CHEROKEE NATION
W.W. HASTINGS HOSPITAL

100% SCHEMATIC DESIGN SUBMITTAL

RedFern Group

PROFESSIONAL SEAL:

CONSULTANT LOGO:

CLIENT:

SEAL OF THE CHEROKEE NATION
Sept. 6, 1839

CHEROKEE NATION
W.W. HASTINGS HOSPITAL
TAHLEQUAH, OKLAHOMA

PROJECT PHASE:
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REVISIONS:

#	DATE	DESCRIPTION

DATE:	JOB NUMBER:
03-21-14	13-13

SHEET NUMBER:
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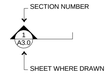
COVER

ABBREVIATIONS

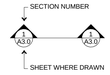
@	AT
A/C	AIR CONDITIONING
AB	ANCHOR BOLT
ABC	AGGREGATE BASE COURSE
AC	ACOUSTICAL CEILING
ADJ	ADJACENT
AFF	ABOVE FINISH FLOOR
AGG	AGGREGATE
ALT	ALTERNATE
ALUM	ALUMINUM
ANCH	ANCHOR, ANCHORAGE
ANOD	ANODIZED
AP	ACCESS PANEL
BD	BOARD
BFF	BELOW FINISH FLOOR
BLDG	BUILDING
BLK(G)	BLOCK(ING)
BM	BENCH MARK
BO	BOTTOM OF
BOT	BOTTOM
BRG	BEARING
BRK	BRICK
BSMT	BASEMENT
BU	BUILT UP
CMWP	COMPOSITE METAL WALL PANEL
C	CHANNEL
C/C	CENTER TO CENTER
CAB	CABINET
CB	CHALK BOARD, CATCH BASIN
CEM	CEMENT
CG	CORNER GUARD
CI	CAST IRON
CIP	CAST IN PLACE
CJ	CONTROL JOINT
CLG	CEILING
CLR	CLEAR FLOOR SPACE
CMP	CORRUGATED METAL PIPE
CMTS	COMMENTS
CMU	CONCRETE MASONRY UNIT
CO	CASED OPENING
COL	COLUMN
COMB	COMBINATION
CONC	CONCRETE
CONN	CONNECTION
CONST	CONSTRUCTION
CONT	CONTINUOUS OR CONTINUE
CORR	CORRIDOR, CORRUGATED
CT	CARPET TILE
CR	CLEAN ROOM
CSK	COUNTERSINK
CTB	CERAMIC TILE - BASE
CTF	CERAMIC TILE - FLOOR
CTW	CERAMIC TILE - WALL
CU	CUBIC
CU FT	CUBIC FOOT (FEET)
CW	COLD WATER
D	DEPTH
DEMO	DEMOLISH, DEMOLITION
DF	DRINKING FOUNTAIN
DIA	DIAMETER
DIAG	DIAGONAL
DIM	DIMENSION
DISP	DISPENSER
DN	DOWN
DO	DITTO
DP	DAMPPROOFING
DR	DOOR
DS	DOWNSPOUT
DTL	DETAIL
DWG	DRAWING
DWC	DRYWALL CHANNELS (HAT)
E	EAST
EA	EACH
EF	EACH FACE
EG	END GUARD
EJ	EXPANSION JOINT
ELEC	ELECTRIC(AL)
ELEV	ELEVATION, ELEVATOR
EMER	EMERGENCY
EP	EPOXY PAINT
EQ	EQUAL
EQUIP	EQUIPMENT
ES	EXPOSED STRUCTURE
EST	ESTIMATE
EW	EACH WAY
EWC	ELECTRIC WATER COOLER
EXC	EXCAVATE(ION)
EXH	EXHAUST
EXIST	EXISTING
EXP	EXPANSION, EXPOSED
EXT	EXTERIOR
EIFS	EXT. INSULATION FINISH SYSTEM
F	FAHRENHEIT
FA	FIRE ALARM
FAB	FABRICATE
FD	FURR-DOWN
FDN	FOUNDATION
FE(C)	FIRE EXTINGUISHER (CABINET)
FF	FINISH FLOOR
FHC	FIRE HOSE CABINET
FHR	FIRE HOSE RACK
FIN	FINISH(ED)
FL	FLOOR
FLA	FLASHING
FLEX	FLEXIBLE
FLUOR	FLUORESCENT
FO	FACE OF
FP	FIREPROOF
FRP	FIBER REINFORCED PLASTIC
FT	FOOT, FEET
FTG	FOOTING
FUR	FURRED(ING)
FUT	FUTURE
GA	GAGE, (GAUGE)
GALV	GALVANIZED
GC	GENERAL CONTRACTOR
GD	GRADE
GL	GLASS, GLAZING
GWB	GYPSUM WALLBOARD
GYP	GYPSUM
H(T)	HEIGHT
HB	HOSE BIB
HC	HOLLOW CORE
HDBD	HARDBOARD
HD	HEAVY DUTY
HDWD	HARDWOOD
HDWR	HARDWARE
HM	HOLLOW METAL
HORZ	HORIZONTAL
HP	HIGH POINT
HR	HOUR
HVAC	HEATING/ VENTILATING/ AIR CONDITIONING
HW	HOT WATER
ID	INSIDE DIAMETER
IE	INVERT ELEVATION
IN	INCH(ES)
INCL	INCLUDING
INSUL	INSULATION
INT	INTERIOR
JC	JANITORS CLOSET
JST	JOIST
JT	JOINT
KD	KNOCKED DOWN
KIT	KITCHEN
KO	KNOCK OUT
KP	KICK PLATE
L	LENGTH, LONG
LAB	LABORATORY
LAM	LAMINATE
LAV	LAVATORY
LF	LINEAL FEET
LH	LEFT HAND
LL	LIVE LOAD
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LP	LOW POINT
LT	LIGHT
LTWT	LIGHTWEIGHT
LVR	LOUVER
MAS	MASONRY
MAX	MAXIMUM
MATL	MATERIAL
MED	MEDIUM
MECH	MECHANICAL
MET	METAL
MEZZ	MEZZANINE
MFR	MANUFACTURER
MH	MANHOLE
MIRR	MIRRORED
MM	MILLIMETER
MIN	MINIMUM
MISC	MISCELLANEOUS
MO	MASONRY OPENING
MOD	MODULAR
MOV	MOVABLE
MTD	MOUNTED
MWP	METAL WALL PANEL (SYSTEM)
N	NORTH
NA	NOT APPLICABLE
NIC	NOT IN CONTRACT
NOM	NOMINAL
NRC	NOISE REDUCTION COEFFICIENTS
NTS	NOT TO SCALE
NO	NUMBER
OA	OVERALL
OC	ON CENTER(S)
OD	OUTSIDE DIAMETER
OFF	OFFICE
OH	OVERHEAD, OPPOSITE HAND
OPG	OPENING
OPP	OPPOSITE
ORD	OVERFLOW ROOF DRAIN
P(#)	PARTITION TYPE
P	PAINT(ED)
PAR	PARALLEL
PART	PARTITION
PART BD	PARTICLE BOARD
PC	PIECE
PCC	PRECAST CONCRETE
PCF	POUNDS PER CUBIC FOOT
PERIM	PERIMETER
PERP	PERPENDICULAR
PF	PERFORATE(D)
PJ	PANEL JOINT
PL	PLASTER, PLATE, PROPERTY LINE
PLBG	PLUMBING
PLAM	PLASTIC LAMINATE
PLF	POUNDS PER LINEAL FOOT
PNL	PANEL
PO	POWER OPERATED
PR	PAIR
PREFAB	PREFABRICATED
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PT	POINT
PTD	PAPER TOWEL DISPENSER
PTR	PAPER TOWEL RECEPTOR
PVC	POLYVINYL CHLORIDE
PVMT	PAVEMENT
PWD	PLYWOOD
QT	QUARRY TILE
QTR	QUARTER
QTY	QUANTITY
(R)	RECESSED
R	RADIUS, RISER ROUND
RA	RETURN AIR
RB	RESILIENT BASE
RCP	REINFORCED CONCRETE PIPE, REFLECTED CEILING PLAN
RD	ROOF DRAIN
RE(F)	REFERENCE
RELO	RELOCATION
REPL	REPLACE
RES	RESILIENT
REV	REVISION(S), REVISED
RF	RIGID FIBERGLASS
RFG	ROOFING
RH	RIGHT HAND
RM	ROOM
RO	ROUGH OPENING
ROW	RIGHT OF WAY
REQ'D	REQUIRED
RWL	RAINWATER LEADER
S	SEALED, SOUTH
(S)	SURFACE MOUNTED
SC	SOLID CORE, SEALED CONCRETE
SCHED	SCHEDULE
SECT	SECTION
SD	STORM DRAIN
SDG	SIDING
SF	SQUARE FEET
SHT	SHEET
SHWR	SHOWER
SIM	SIMILAR
SL	SHEET LINOLEUM
SM	SHEET METAL
SP	SOUNDPROOF
SPECS	SPECIFICATIONS
SPKR	SPEAKER
SPL	SPECIAL
SQ	SQUARE
SS	SERVICE SINK
SST	STAINLESS STEEL
ST	STEEL, STREET
STC	SOUND TRANSMISSION COEFFICIENT
STD	STANDARD
STOR	STORAGE
STRUC	STRUCTURAL
SUSP	SUSPENDED
SV	SEAMLESS VINYL
SW	SWITCH
SWR	SEWER
SYM	SYMMETRY(ICAL)
T	THICKNESS, TEE
TD	TRENCH DRAIN
T&G	TONGUE & GROOVE
TB	TACKBOARD
TEL	TELEPHONE
TEMP	TEMPERATURE, TEMPORARY
TERR	TERRAZZO
THK	THICK(NESS)
TO	TOP OF
TOC	TOP OF CONCRETE
TOIL	TOILET
TOP	TOP OF PARAPET
TOS	TOP OF STEEL
TOW	TOP OF WALL
TPD	TOILET PAPER DISPENSER
TV	TELEVISION
TYP	TYPICAL
U	URINAL
UG	UNDERGROUND
UL	UNDERWRITERS LABORATORY
UNF	UNFINISHED
UNO	UNLESS NOTED OTHERWISE
VB	VAPOR BARRIER
VCT	VINYL COMPOSITION TILE
VERT	VERTICAL
VIF	VERIFY IN FIELD
VNR	VENEER
VPL	VENEER PLASTER
VWC	VINYL WALL COVERING
W	WIDTH, WIDE, WEST
W/	WITH
W/O	WITHOUT
W/W	WALL TO WALL
WC	WATER CLOSET
WD	WOOD
WDW	WINDOW
WH	WALL HUNG, WATER HEATER
WI	WROUGHT IRON
WM	WIRE MESH
WP	WATERPROOFING
WR	WATER REPELLENT
WS	WEATHER-STRIP
WSCT	WAINSCOT
WT	WEIGHT
WWF	WELDED WIRE FABRIC
YD	YARD

ANNOTATION SYMBOLS

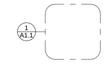
WALL SECTION



BUILDING SECTION



ENLARGED AREA DETAIL

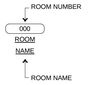


1 DRAWING TITLE
SCALE:

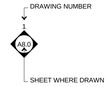
DOOR MARK

WINDOW MARK

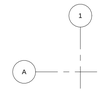
ROOM MARK



EXTERIOR / INTERIOR ELEVATION



COLUMN GRID



ELEVATION INDICATOR

INDEX OF DRAWINGS

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CLIENT:

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TAHLEQUAH, OKLAHOMA

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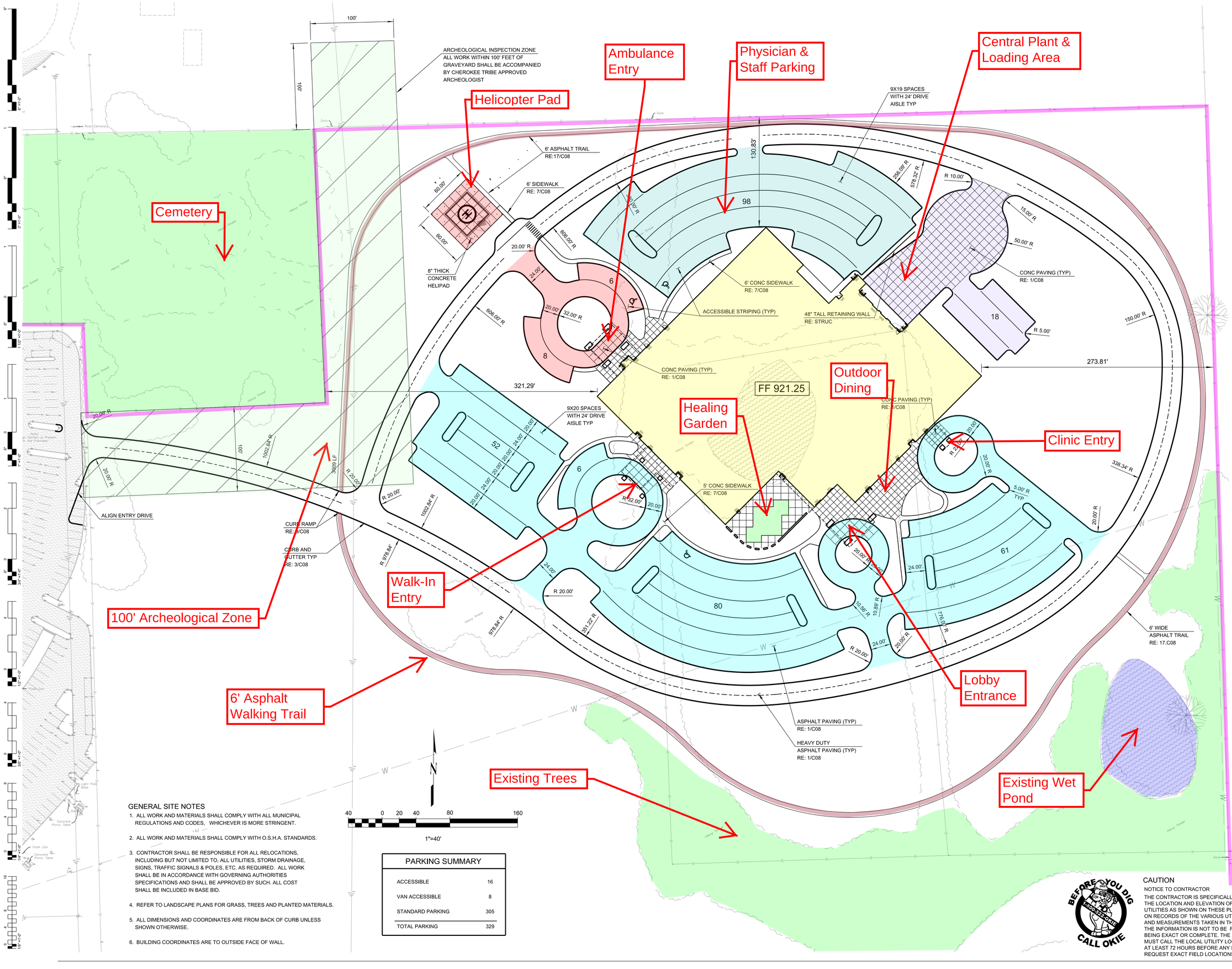
ABBREVIATIONS, SYMBOLS, & INDEX



GENERAL SITE NOTES

1. ALL WORK AND MATERIALS SHALL COMPLY WITH ALL MUNICIPAL REGULATIONS AND CODES, WHICHEVER IS MORE STRINGENT.
2. ALL WORK AND MATERIALS SHALL COMPLY WITH O.S.H.A. STANDARDS.
3. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL RELOCATIONS, INCLUDING BUT NOT LIMITED TO, ALL UTILITIES, STORM DRAINAGE, SIGNS, TRAFFIC SIGNALS & POLES, ETC. AS REQUIRED. ALL WORK SHALL BE IN ACCORDANCE WITH GOVERNING AUTHORITIES SPECIFICATIONS AND SHALL BE APPROVED BY SUCH. ALL COST SHALL BE INCLUDED IN BASE BID.
4. REFER TO LANDSCAPE PLANS FOR GRASS, TREES AND PLANTED MATERIALS.
5. ALL DIMENSIONS AND COORDINATES ARE FROM BACK OF CURB UNLESS SHOWN OTHERWISE.
6. BUILDING COORDINATES ARE TO OUTSIDE FACE OF WALL.

PARKING SUMMARY	
ACCESSIBLE	16
VAN ACCESSIBLE	8
STANDARD PARKING	305
TOTAL PARKING	329

CAUTION

NOTICE TO CONTRACTOR

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BEFORE YOU DIG 811 1-800-522-OKIE CALL OKIE

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PROFESSIONAL SEAL:

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CONSULTANT LOGO:

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Structural Consultants, Inc.

Structural and Civil Consultants
200 East Brady Street
Tulsa, Oklahoma 74103
918.584.5858, Fax 918.584.8689

OKLAHOMA CA #1460
EXP DATE 6/30/15

CLIENT:

SEAL OF THE CHEROKEE NATION SEPT. 6, 1839

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C04

SITE PLAN

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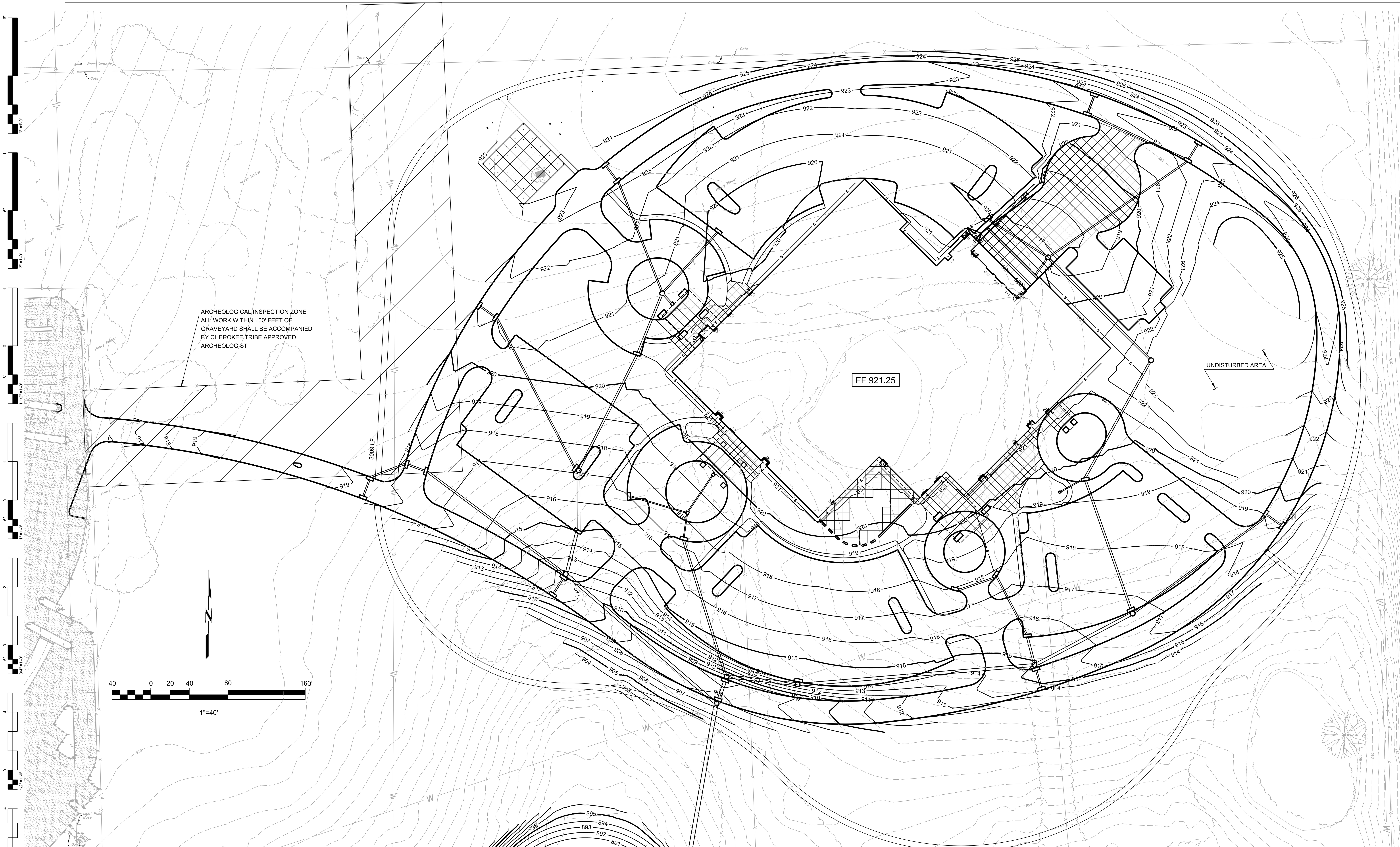

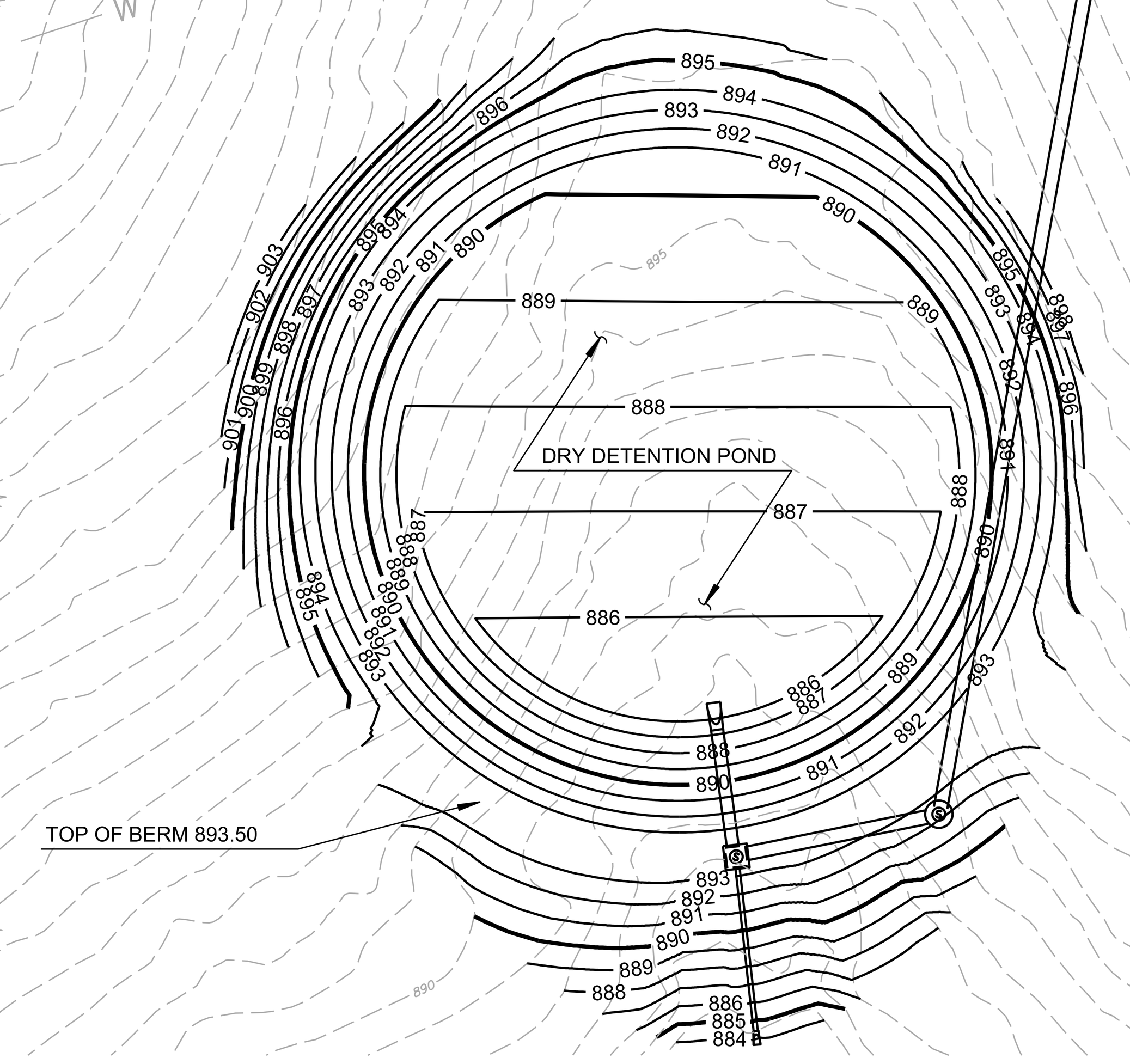

GENERAL GRADING NOTES

1. SITE GRADING SHALL NOT PROCEED UNTIL EROSION CONTROL MEASURES HAVE BEEN INSTALLED & INSPECTED AND APPROVED BY LOCAL AUTHORITIES.
2. ALL CUT OR FILL SLOPES SHALL BE 3:1 OR FLATTER UNLESS OTHERWISE NOTED.
3. EXISTING DRAINAGE STRUCTURES TO BE INSPECTED AND REPAIRED AS NEEDED, AND EXISTING PIPES ARE TO BE CLEANED OUT TO REMOVE ALL SILT AND DEBRIS.
4. IF ANY EXISTING STRUCTURES TO REMAIN ARE DAMAGED DURING CONSTRUCTION IT SHALL BE THE CONTRACTORS RESPONSIBILITY TO REPAIR OR REPLACE THE EXISTING STRUCTURE AS NECESSARY.
5. ALL STORM PIPE ENTERING STRUCTURES SHALL BE GROUTED TO ASSURE CONNECTION AT STRUCTURE IS WATER TIGHT.
6. CONTRACTOR SHALL ASSURE POSITIVE DRAINAGE AWAY FROM BUILDINGS AND STRUCTURES FOR ALL GRASSED AND PAVED AREAS.
7. CONSTRUCTION SHALL COMPLY WITH ALL APPLICABLE GOVERNING CODES AND BE CONSTRUCTED TO THE SAME.
8. CONTRACTOR IS RESPONSIBLE FOR TEMPORARY ACCESS ROADS AND SHALL MAINTAIN POSITIVE DRAINAGE OF ENTIRE SITE THROUGHOUT CONSTRUCTION AND AVOID PONDING OR RUTTING. TEMPORARY DEWATERING, INCLUDING PUMPING, MAY BE REQUIRED AND SHALL BE INCLUDED IN THE SCOPE OF WORK.
9. SIDEWALKS SHALL HAVE MAX 2% CROSS SLOPE.

BEFORE YOU DIG
CALL OKIE

CAUTION

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C05

GRADING PLAN

GENERAL UTILITY NOTES:

1. PRIOR TO CONSTRUCTION LOCATION OF SITE UTILITIES SHALL BE VERIFIED BY CONTRACTOR WITH THE PROPER UTILITY COMPANY PROVIDING SERVICE.
2. CONTRACTOR SHALL NOTIFY THE UTILITY AUTHORITIES INSPECTORS BEFORE CONNECTING TO ANY EXISTING LINE IN ACCORDANCE WITH LOCAL REQUIREMENTS.
3. RESTRAINED JOINTS SHALL BE PROVIDED ON 4" AND LARGER WATER LINES AT ALL BENDS TEES AND FIRE HYDRANTS A MINIMUM 2 JOINTS BOTH SIDES OF FITTING PER AWWA MINIMUM STANDARDS.
4. ALL TRENCHING, PIPE LAYING AND BACKFILLING SHALL BE IN ACCORDANCE WITH FEDERAL OSHA REGULATIONS.
5. REFER TO PLUMBING AND/OR FIRE PROTECTION SHEETS FOR FIRE LINE LEAD-IN LOCATION AND DETAIL.
6. ALL PIPING SHALL BE INSTALLED WITH A MINIMUM OF 30" OF COVER, UNLESS NOTED OTHERWISE.
7. CONTRACTOR IS RESPONSIBLE FOR COMPLYING TO THE SPECIFICATIONS OF THE LOCAL AUTHORITIES WITH REGARDS TO MATERIALS AND INSTALLATION OF THE WATER AND SEWER LINES.
8. CONTRACTOR IS TO COORDINATE WITH ALL UTILITY COMPANIES FOR INSTALLATION REQUIREMENTS AND SPECIFICATIONS.
9. CONTRACTOR SHALL UTILIZE AWWA AND FACTORY MUTUAL TEST AND CERTIFICATIONS FOR ALL UNDERGROUND FIRE PROTECTION LINES AS A MINIMUM. LOCAL OR STATE AUTHORITIES MAY REQUIRE MORE STRINGENT TESTING WHICH SHALL BE PROVIDED BY THE GC IF REQUIRED.
10. UTILITY TRENCH DETAIL RE: XX/CXXX.
11. REFER TO PLUMB FOR CONTINUATION OF UTILITIES AT BUILDING.
12. CONSTRUCT CLAY TRENCH PLUG THAT EXTENDS AT LEAST 5 FEET OUT FROM THE FACE OF THE BUILDING EXTERIOR. THE PLUG MATERIAL SHALL CONSIST OF CLAY COMPACTED AT A WATER CONTENT AT OR ABOVE THE SOILS OPTIMUM WATER CONTENT. THE CLAY FILL SHALL BE PLACED TO COMPLETELY SURROUND THE UTILITY LINE AND BE COMPACTED TO AT LEAST 95% STANDARD PROCTOR DENSITY.

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CALL OKIE

CAUTION

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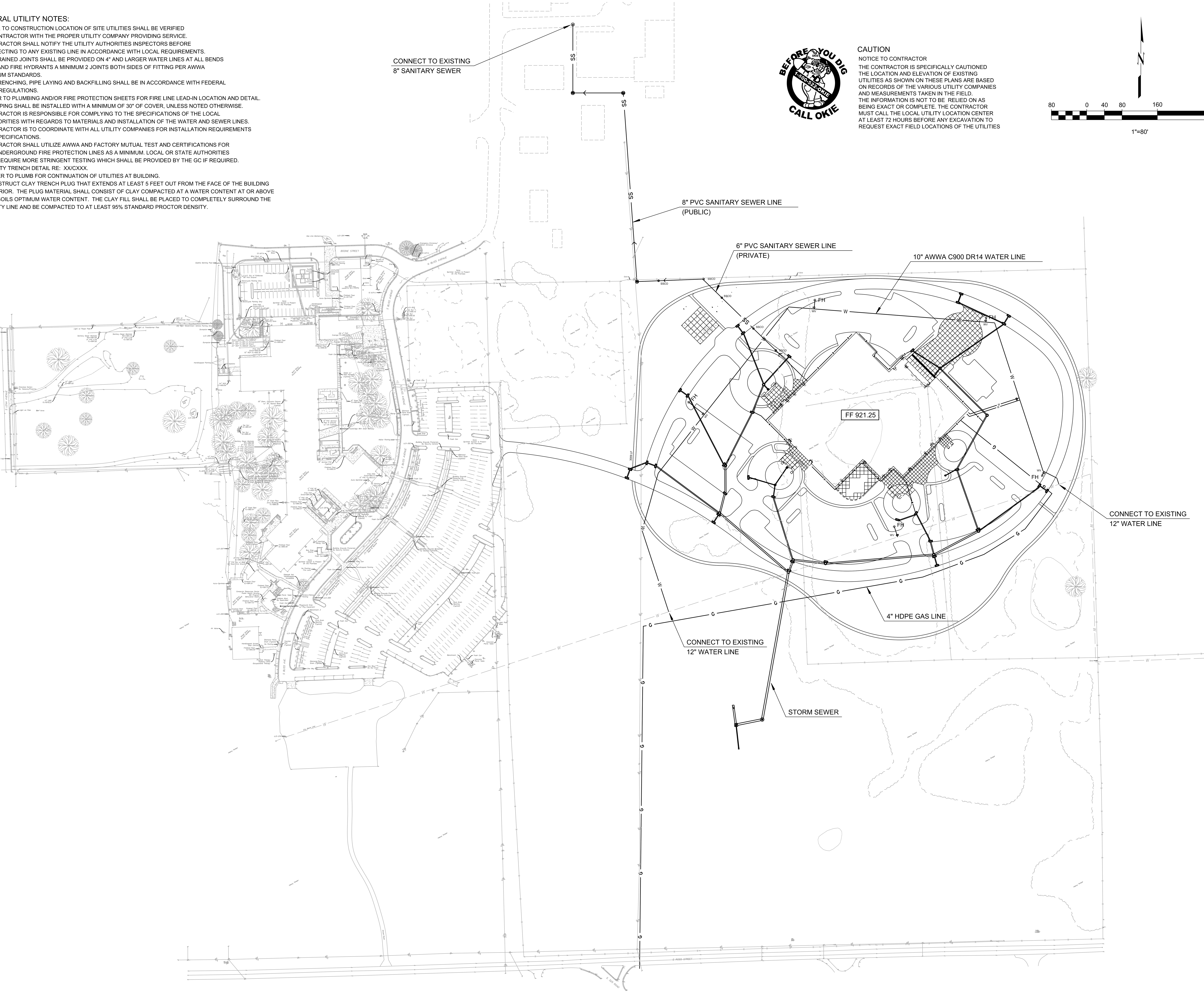
1"=80'



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C06

OVERALL UTILITY PLAN

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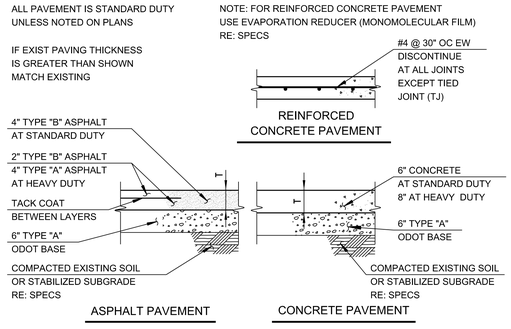

1 PAVEMENT DETAIL
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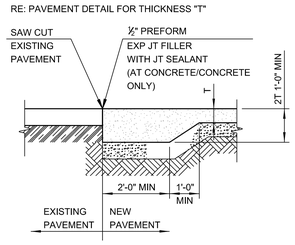
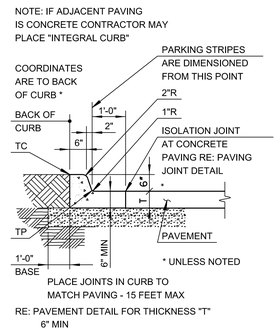
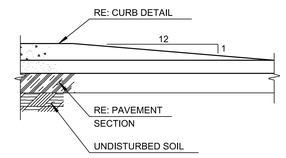


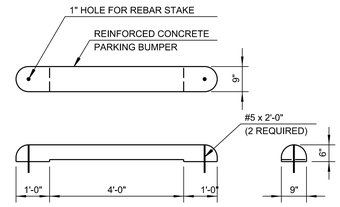
2 PAVEMENT TRANSITION
SCALE: NTS



3 CURB AND GUTTER (BARRIER)
SCALE: NTS



4 CURB TERMINATION
SCALE: NTS



5 PARKING BUMPER
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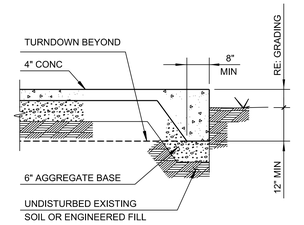
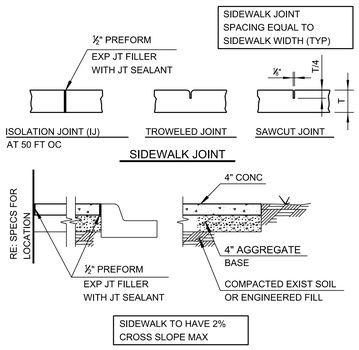
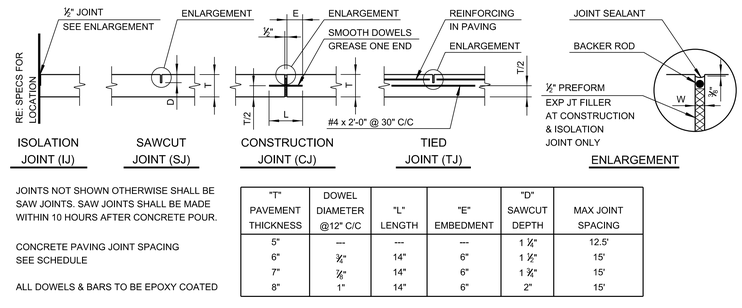


6 STOOP
SCALE: NTS



7 SIDEWALK
SCALE: NTS



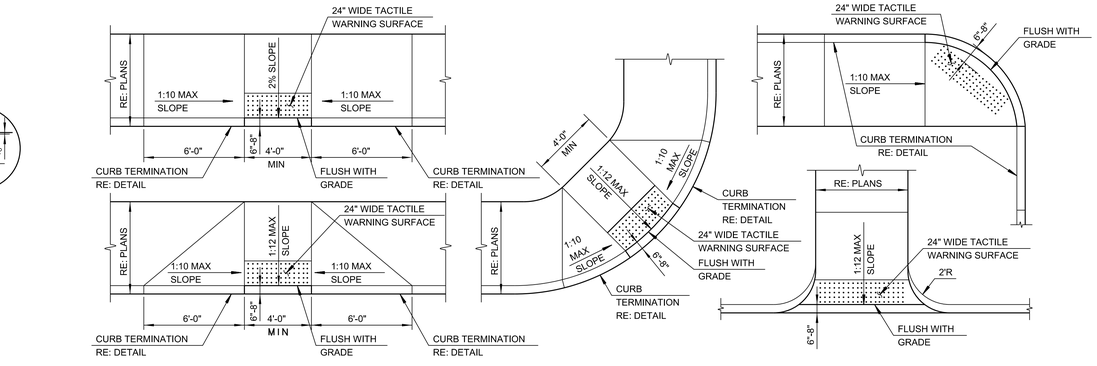
JOINTS NOT SHOWN OTHERWISE SHALL BE SAW JOINTS. SAW JOINTS SHALL BE MADE WITHIN 10 HOURS AFTER CONCRETE POUR.

CONCRETE PAVING JOINT SPACING
SEE SCHEDULE

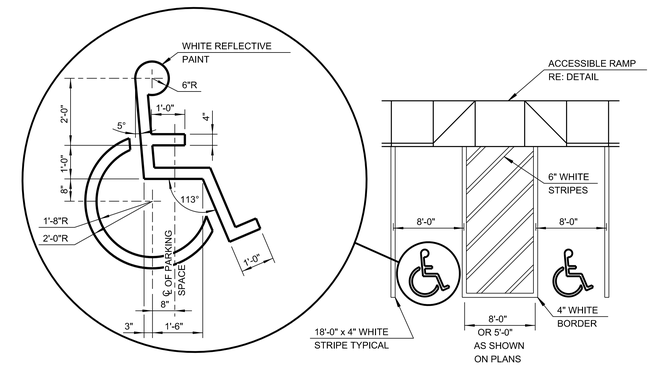
ALL DOWELS & BARS TO BE EPOXY COATED

"T" PAVEMENT THICKNESS	DOWEL DIAMETER @12" C/C	"L" LENGTH	"E" EMBEDMENT	"D" SAWCUT DEPTH	MAX JOINT SPACING
5"	---	---	---	1 ¼"	12.5'
6"	¾"	14"	6"	1 ½"	15'
7"	⅞"	14"	6"	1 ¾"	15'
8"	1"	14"	6"	2"	15'

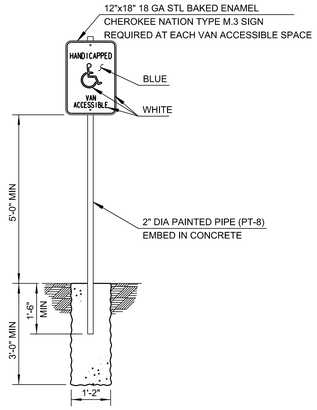
8 PAVING JOINT DETAIL
SCALE: NTS



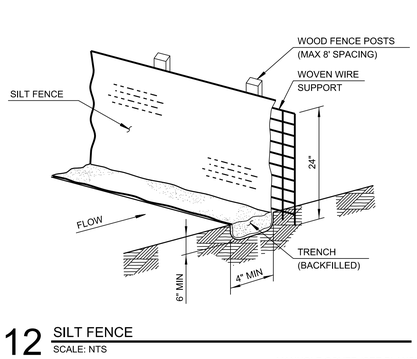
9 CURB RAMPS
SCALE: NTS



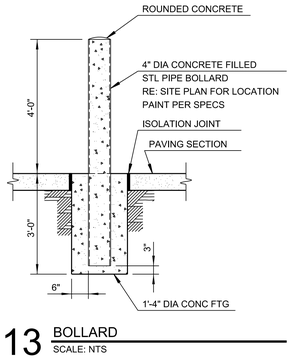
10 ACCESSIBLE STRIPING
SCALE: NTS



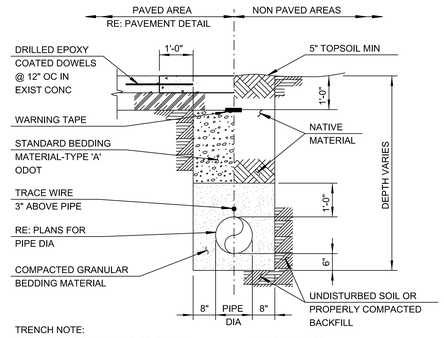
11 ACCESSIBLE SIGNAGE
SCALE: NTS



12 SILT FENCE
SCALE: NTS



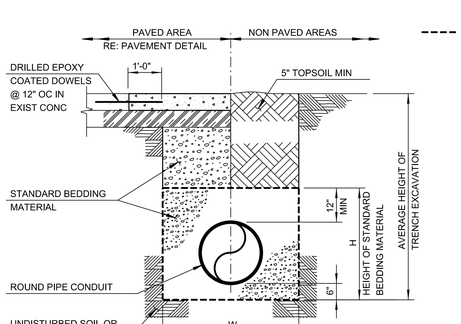
13 BOLLARD
SCALE: NTS



TRENCH NOTE:
BRACING AND SHEATHING OR OTHER TRENCH PROTECTION TO BE PROVIDED TO MEET APPLICABLE STATE AND OSHA SAFETY STANDARDS. ALL SUCH TRENCH PROTECTION TO BE RESPONSIBILITY OF THE CONTRACTOR.

BEDDING NOTE:
UNDER PAVING ODOT TYPE "A" AGGREGATE BASE MUST BE TO THE TOP OF THE TRENCH. ODOT TYPE "A" AGGREGATE BASE TO BE COMPACTED TO 95% STANDARD PROCTOR.

14 PIPE TRENCH-ALL PIPES EXCEPT STORM SEWER
SCALE: NTS



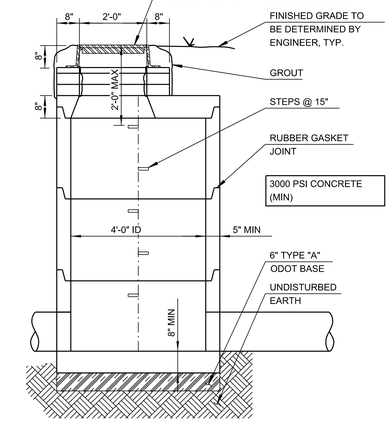
LIMITS OF STANDARD BEDDING MATERIAL QUANTITIES FOR BEDDING MATERIAL DO NOT INCLUDE THE SPACE WITHIN AND BOUNDED BY THE OUTER SURFACE OF THE PIPE CONDUIT.

TRENCH NOTE:
BRACING AND SHEATHING OR OTHER TRENCH PROTECTION TO BE PROVIDED TO MEET APPLICABLE STATE AND OSHA SAFETY STANDARDS. ALL SUCH TRENCH PROTECTION TO BE RESPONSIBILITY OF THE CONTRACTOR.

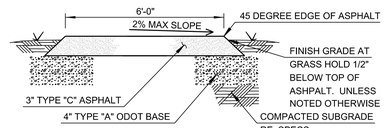
BEDDING NOTE:
STANDARD BEDDING MATERIAL MUST BE ODOT TYPE "A" AGGREGATE BASE. UNDER PAVING ODOT TYPE "A" AGGREGATE BASE MUST BE TO THE TOP OF THE TRENCH. ODOT TYPE "A" AGGREGATE BASE TO BE COMPACTED TO 95% STANDARD PROCTOR.

PIPE SIZE, IN.	TRENCH WIDTH, FT.
ID ≤ 24"	OD + 2'
24" > ID ≤ 36"	OD + 2.5'
36" > ID ≤ 60"	OD + 3.5'
ID > 60"	OD + 4'

15 STANDARD PIPE BEDDING DETAIL FOR STORM SEWER
SCALE: NTS



16 MANHOLE
SCALE: NTS



17 ASPHALT TRAIL
SCALE: NTS

RedFern Group

PROFESSIONAL SEAL:

THIS DOCUMENT IS PRELIMINARY IN NATURE AND IS NOT A FINAL, SIGNED AND SEALED DOCUMENT.

CONSULTANT LOGO:

wallace

Wallace Engineering
Structural Consultants, Inc.

Structual and Civil Consultants
200 East Brady Street
Tulsa, Oklahoma 74103
918.584.5858, Fax 918.584.8689

OKLAHOMA CA #1460
EXP DATE 6/30/15

CLIENT:

SEAL OF THE CHEROKEE NATION
SEPT. 6, 1839

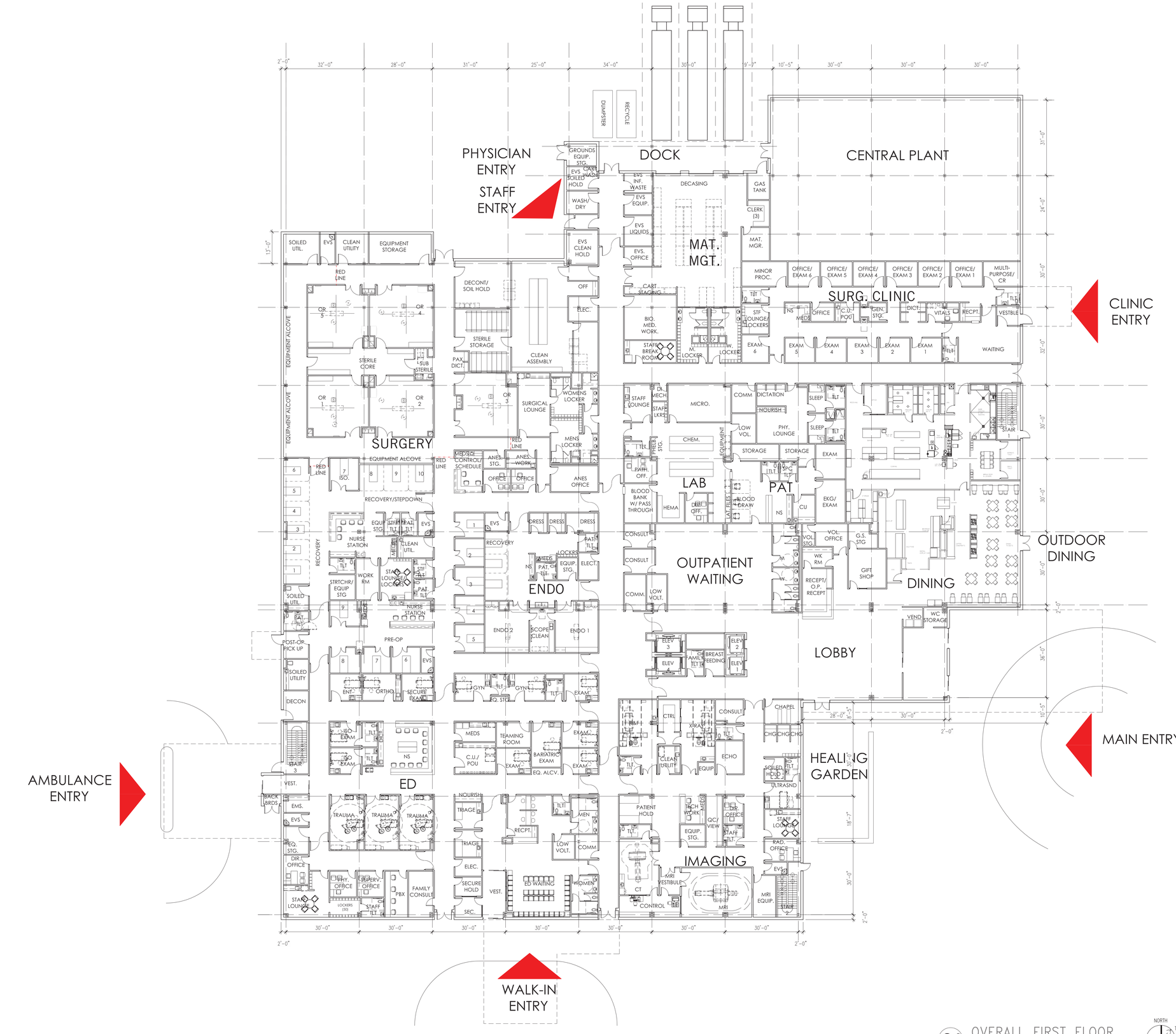
CHEROKEE NATION
W.W. HASTINGS HOSPITAL
TAHLEQUAH, OKLAHOMA

PROJECT PHASE:
100%
SCHEMATIC DESIGN
SUBMITTAL

REVISIONS		
#	DATE	DESCRIPTION

DATE: 03-21-14
JOB NUMBER: 13-13
SHEET NUMBER: C08
DETAIL SHEET

3/20/2014 12:56:31 PM \\civil_server\projects\13\40101 Cherokee Nation WW Hastings\Dgn\CI08Det.dgn



01 OVERALL FIRST FLOOR
1/16" = 1'-0"

NORTH
TRUE NORTH

RedFern Group

PROFESSIONAL SEAL:

CONSULTANT LOGO:

CLIENT:

CHEROKEE NATION
W.W. HASTINGS HOSPITAL
TAHLEQUAH, OKLAHOMA

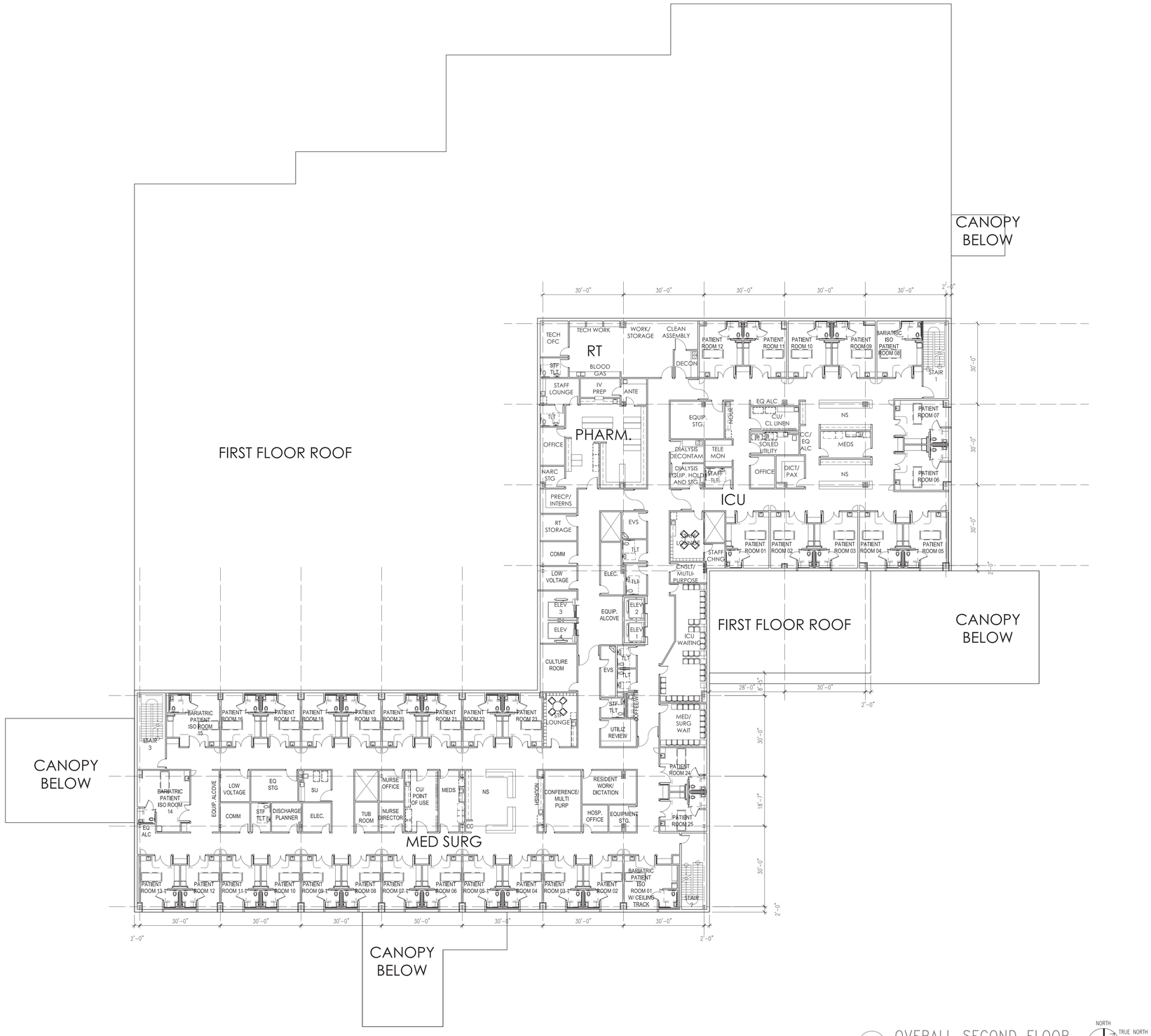
PROJECT PHASE:
100% SCHEMATIC DESIGN (NOT FOR CONSTRUCTION)

REVISIONS		
#	DATE	DESCRIPTION

DATE:	JOB NUMBER:
03-21-14	13-13

SHEET NUMBER:
A 1.1

FIRST FLOOR



FIRST FLOOR ROOF

01 OVERALL SECOND FLOOR
1/16" = 1'-0"

NORTH
TRUE NORTH

RedFern Group

PROFESSIONAL SEAL:

CONSULTANT LOGO:

CLIENT:

CHEROKEE NATION
W.W. HASTINGS HOSPITAL
TAHLEQUAH, OKLAHOMA

PROJECT PHASE:
100% SCHEMATIC DESIGN
(NOT FOR CONSTRUCTION)

REVISIONS		
#	DATE	DESCRIPTION

DATE:	JOB NUMBER:
03-21-14	13-13

SHEET NUMBER:
A 1.2

SECOND FLOOR
1/16" = 1'-0"

01 OVERALL THIRD FLOOR
1/16" = 1'-0"

NORTH TRUE NORTH

RedFern Group

PROFESSIONAL SEAL:

CONSULTANT LOGO:

CLIENT:

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W.W. HASTINGS HOSPITAL
TAHLEQUAH, OKLAHOMA

PROJECT PHASE:
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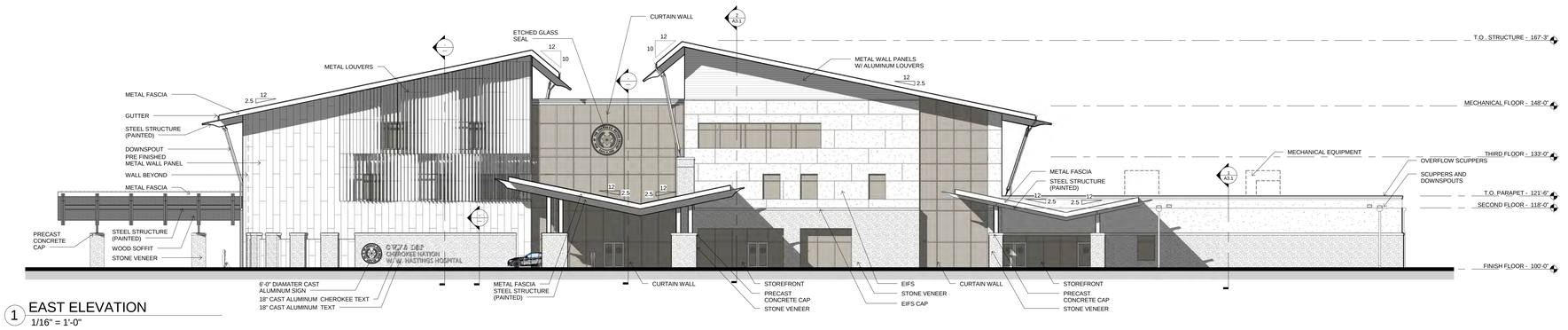
REVISIONS		
#	DATE	DESCRIPTION

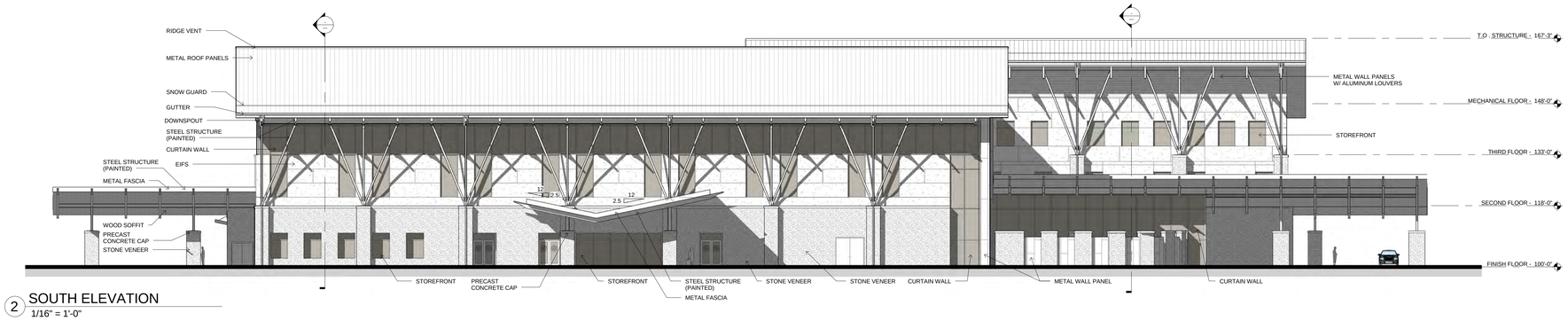
DATE:	JOB NUMBER:
03-21-14	13-13

SHEET NUMBER:
A 1.3

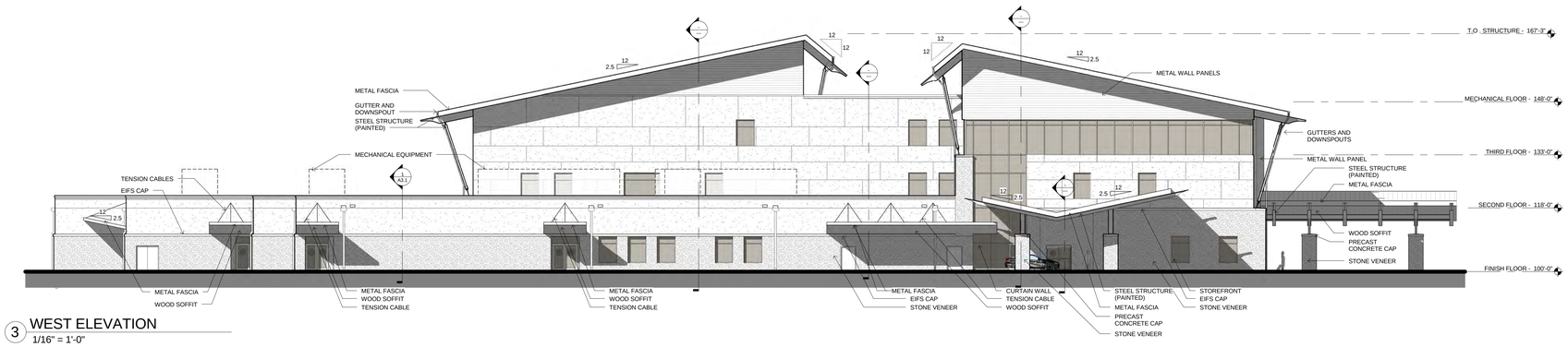
THIRD FLOOR



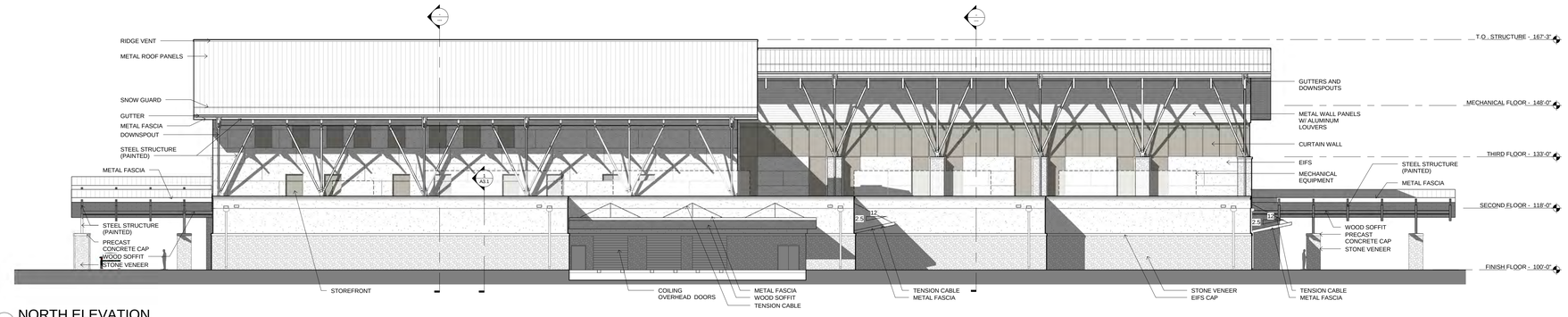
1 EAST ELEVATION
1/16" = 1'-0"



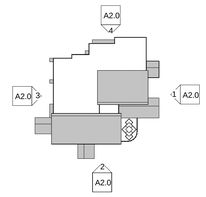
2 SOUTH ELEVATION
1/16" = 1'-0"



3 WEST ELEVATION
1/16" = 1'-0"



4 NORTH ELEVATION
1/16" = 1'-0"



KEY PLAN

RedFern Group

PROFESSIONAL SEAL:

CONSULTANT LOGO:

CLIENT:

CHEROKEE NATION
W.W. HASTINGS HOSPITAL
TAHLEQUAH, OKLAHOMA

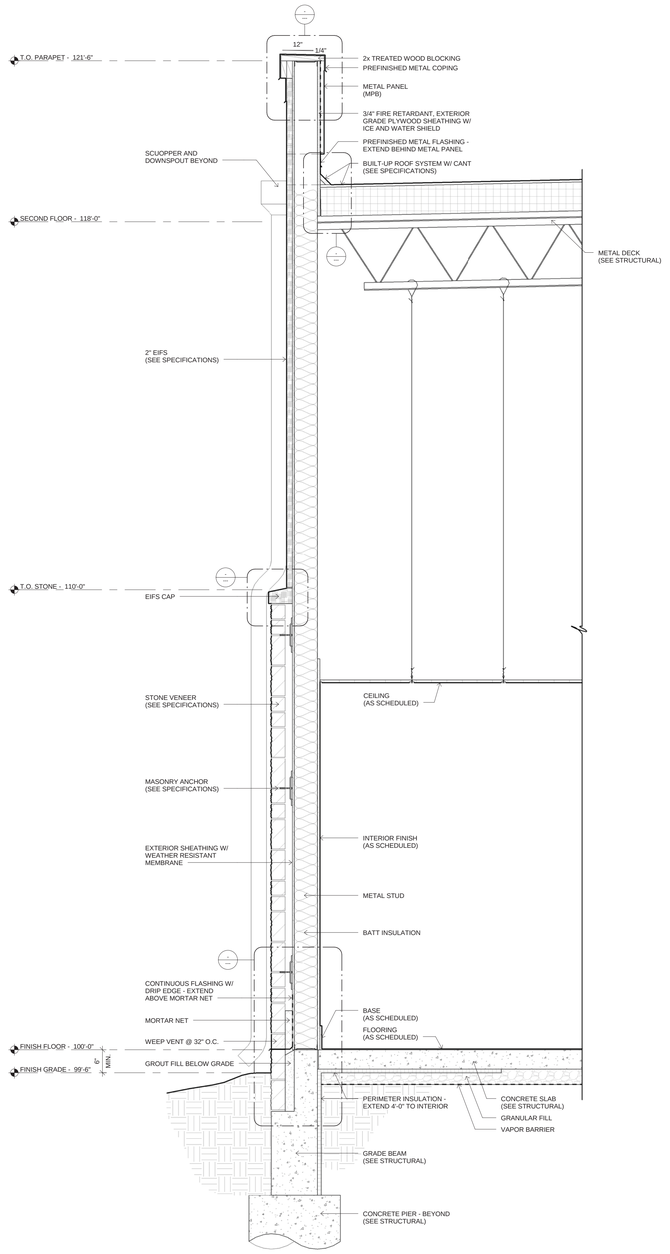
PROJECT PHASE:
100% SCHEMATIC DESIGN SUBMITTAL

REVISIONS		
#	DATE	DESCRIPTION

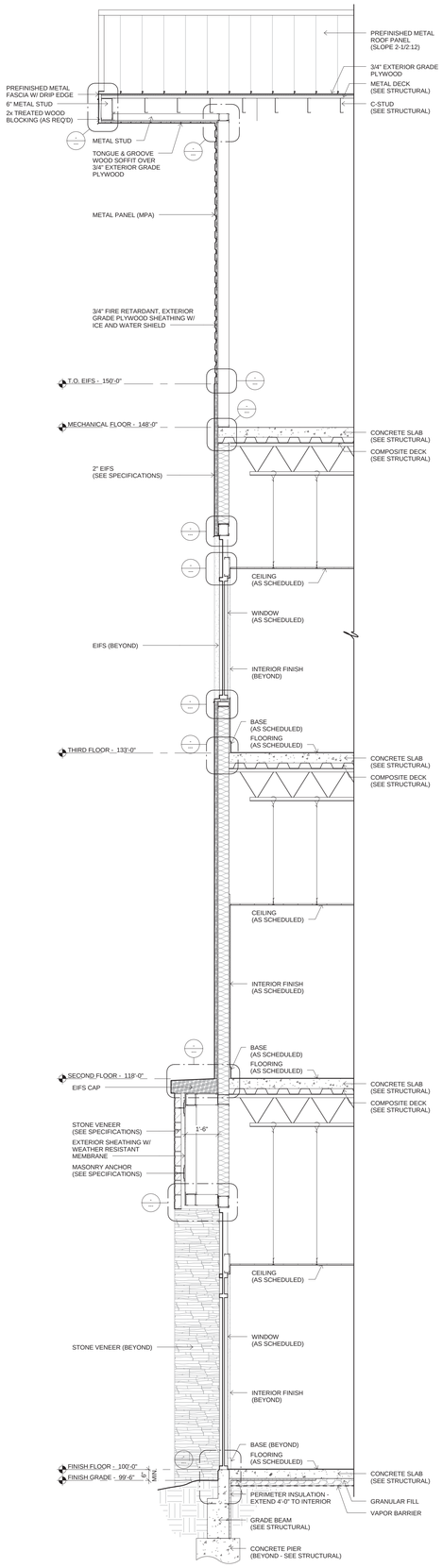
DATE:	JOB NUMBER:
03-21-14	13-13

SHEET NUMBER:
A2.0

EXTERIOR ELEVATIONS



1 WALL SECTION
3/4" = 1'-0"



2 WALL SECTION
3/8" = 1'-0"

RedFern Group

PROFESSIONAL SEAL:

CONSULTANT LOGO:

CLIENT:

CHEROKEE NATION
W.W. HASTINGS HOSPITAL
TAHLEQUAH, OKLAHOMA

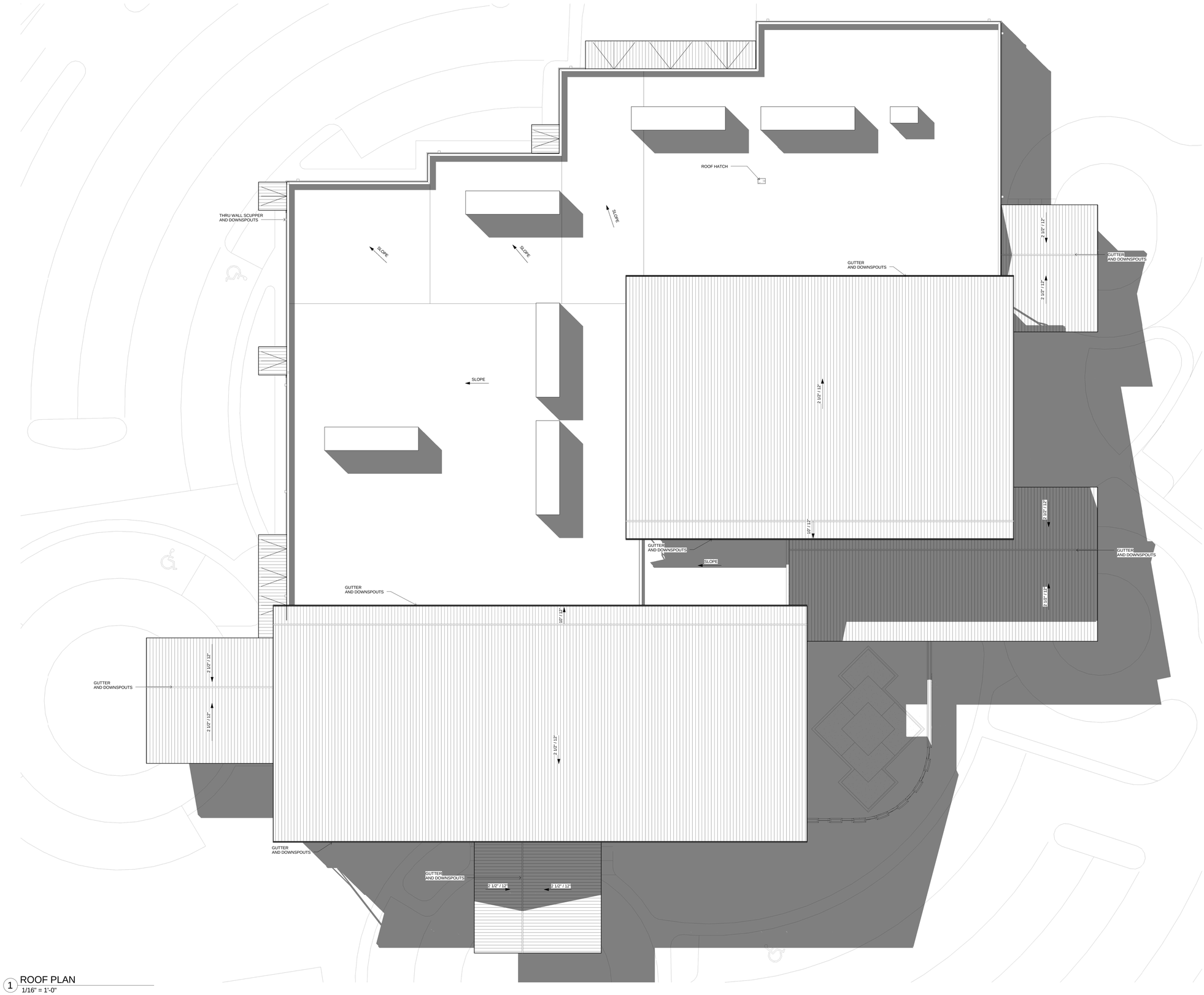
PROJECT PHASE:
100% SCHEMATIC DESIGN SUBMITTAL

REVISIONS		
#	DATE	DESCRIPTION

DATE:	JOB NUMBER:
03-21-14	13-13

SHEET NUMBER:
A3.1

WALL SECTIONS



1 ROOF PLAN
1/16" = 1'-0"

RedFern Group

PROFESSIONAL SEAL:

CONSULTANT LOGO:

CLIENT:

SEAL OF THE CHEROKEE NATION
ADOPTED SEPT. 6, 1839

CHEROKEE NATION
W.W. HASTINGS HOSPITAL
TAHLEQUAH, OKLAHOMA

PROJECT PHASE:
100% SCHEMATIC DESIGN SUBMITTAL

REVISIONS		
#	DATE	DESCRIPTION

DATE: 03-21-14
JOB NUMBER: 13-13
SHEET NUMBER: A4.0
ROOF PLAN

CODE ANALYSIS

GENERAL DESCRIPTION

PROJECT NAME: HASTINGS HOSPITAL
PROJECT LOCATION: TAHLEQUAH, OKLAHOMA
APPLICABLE CODES: IBC 2009, NFPA101 - 2000 & ICC/ANSI A 117.1 - 2009

TOTAL BUILDING AREA: 155,015 SF

	IBC - 2009	IBC - 2009 CODE REFERENCE	NFPA 101 - 2000	NFPA 101 - 2000 CODE REFERENCE
OCCUPANCY CLASSIFICATION	INSTITUTIONAL	304.1	INSTITUTIONAL	.3.34, 6.1.11.1 3.3.178.3, & CHAPTER 38
CONSTRUCTION TYPE	TYPE 1B	TABLE 601	II 222	18.1.6.2
HEIGHT & AREA LIMITS				
MAXIMUM HEIGHT ALLOWED:	75 FEET (4) STORY	TABLE 503, 504.2		
BASIC ALLOWABLE AREA (PER FLOOR):	UNLIMITED	TABLE 503		
AUTO. SPRINKLER SYSTEM INCREASE (PER FLOOR):	UNLIMITED	506.3		
FRONTAGE INCREASE (PER FLOOR):	NOT REQUIRED	506.2		
TOTAL AREA (PER FLOOR):	UNLIMITED (ALLOWABLE)	85,015 SF ACTUAL		
FIRE RESISTIVE REQUIREMENTS				
PRIMARY STRUCTURAL FRAME:	2 HOUR (1HR FOR ONLY ROOF)	TABLE 601, SECTION 202	0 HOUR	A.8.2.1.2
FLOOR CONSTRUCTION:	2 HOUR	TABLE 601, SECTION 202	0 HOUR	A.8.2.1.2
SHAFT ENCLOSURES:	2 HOUR			
CONNECTING LESS THAN 4 FLOORS:	2 HOUR	SECTION 708.4	I HOUR	8.6.5(2)
CONNECTING 4 OF MORE FLOORS:	2 HOUR	SECTION 708.4	2 HOUR	8.6.5(1)
ROOF CONSTRUCTION:	1 HOUR	TABLE 601	0 HOUR	A.8.2.1.2
ROOF SURFACING:				
MODIFIED BITUMEN ROOFING:	MIN. SLOPE 1/4" PER FOOT	1507.11.1		
ROOF INSULATION :	FM4450 OR UL1256	1508.1		
ROOF COVERING:	ASTM STANDARDS	1507.11.2		
METAL ROOF PANELS: STANDING SEAM		1504.3.2		
PRE-PAINTED STEEL:	ASTM A755	TABLE 1507.4.3 (1)		

WALLS

	FIRE RESISTANCE		OPENING PROTECTIVES		FIRE RESISTANCE			
	RATING	REFERENCE	RATING	REFERENCE	RATING	REFERENCE		
EXTERIOR BEARING:	2 HOUR	TABLE 601	0 HOUR	TABLE 705.8(2)	0 HOUR	A.8.2.1.2		
EXTERIOR NON-BEARING: (GREATER THAN 30 FEET OF SEPARATION)	0 HOUR	TABLE 602	0 HOUR	TABLE 705.8(2)	0 HOUR	A.8.2.1.2		
INTERIOR BEARING:	2 HOUR	TABLE 601			0 HOUR	A.8.2.1.2		
INTERIOR NON-BEARING:	0 HOUR	TABLE 601			0 HOUR	A.8.2.1.2		
CORRIDORS:	SMOKE PART.	407.3	SMOKE RESISTNT	407.3.1	0 HOUR	38.3.6.1(3)		
WITH OCCUPANCY LOAD GREATER THAN 10:	N/A	TABLE 1018.1						
WITH OCCUPANCY LOAD GREATER THAN 30:	0 HR (NOTE 6)	TABLE 1018.1						
EXIT PASSAGEWAYS:	2 HOUR	SECTION 1023.3	90 MIN.	TABLE 715.4				
HORIZONTAL EXIT WALLS:	2 HOUR	SECTION 1025.2	1 1/2 HOUR	TABLE 715.4				

SPECIAL HAZARD PROTECTION

(OCCUPANCY SEPARATION)	WALL RATING	DOOR RATING	IBC REFERENCE	WALL HOUR	DOOR REFERENCE	NFPA REFERENCE
BOILER & FUEL-FIRED HEATER ROOMS:	1 HOUR (NOTE 6 & 7)	1 HOUR (NOTE 6 & 7)	TABLE 508.2.5, SECTION 508.2.2	0 (NOTE 6 & 8)	0 (NOTE 6 & 8)	8.7.1.1(2) & 38.3.2.1
WASTE & LINEN	1 HOUR	45 MIN.				
PAINT SHOP	1 HOUR	45 MIN.				
LAB	1 HOUR	45 MIN.				
LAUNDRY	1 HOUR	45 MIN.				
BATTERY STORAGE	2 HOUR	90 MIN.				
FIRE PUMP	1 HOUR	45 MIN.				

DAMPERS

FIRE DAMPERS:	RATED IN FIRE BARRIER	SECTION 716.5.4		
SMOKE DAMPERS:	IN SMOKE BARRIER	SECTION 716.5.7		SECTION 8.5.5

COMPARTMENTATION

SMOKE BARRIERS:	REQUIRED		REQUIRED	SECTION 38.3.7

EXIT REQUIREMENTS

DOORS:				
MIN. EXIT DOOR WIDTH FOR MOVEMENT OF BEDS:	41 1/2" CLEAR WIDTH.			
			32" CLEAR	7.2.1.2.3.2
			NO REQUIREMENT	
			NO REQUIREMENT	
			50 PERSONS	7.2.1.4.2(1)
			REQUIRED	7.2.1.5.1
			44" MINIMUM	7.3.3.1
			44" MINIMUM	38.2.3.2
			7'-6"	7.1.5.1
			TWO REQUIRED	7.4.1.1
			500 - 1,000	7.4.1.2(1)
			1,000+	7.4.1.2(2)
			200 FEET (NOTE 6)	18.2.6.3
			20 FEET (NOTE 6)	18 2.5.2.1
			75 FEET (NOTE 6)	18 2.5.3.1

	IBC - 2009	IBC - 2009 CODE REFERENCE	NFPA 101 - 2000	NFPA 101 - 2000 CODE REFERENCE
EGRESS PATHS				
EXIT LIGHTING	REQUIRED	1006.1	REQUIRED	38.2.8 & SECTION 7.8
EMERGENCY LIGHTING	REQUIRED	1006.3	REQUIRED	38.2.9.1 (3) & SEC. 7.9
EXIT SIGNS & ILLUMINATION & TACTILE SIGNS	REQUIRED	1011	REQUIRE LIGHTING	7.10.2 & SEC.7.10.5.1
FIRE PROTECTION SYSTEMS				
AUTOMATIC SPRINKLERS	PROVIDED	903.3.1.1	PROVIDED NFPA 13	
PORTABLE FIRE EXTINGUISHERS	REQUIRED	906.1	REQUIRED NFPA 10	38.3.5 & 9.7.4.1
DETECTION & ALARM SYSTEMS	REQUIRED	907.2	REQUIRED	38.3.4.1 (3)
SMOKE DETECTORS	REQUIRED			
MANUAL FIRE ALARM	REQUIRED	907.2.2		
EMPLOYEE EXTINGUISHER TRAINING	SEE NFPA		REQUIRED PERIODICALLY	38.7.3
CARBON MONOXIDE	NOTE 4			
HEAT				
INTERIOR FINISHES				
EXIT ENCLOSURES, EXIT PASSAGEWAYS (WALLS & CEILINGS) / EXITS	CLASS B	TABLE 803.9	CLASS A OR CLASS B	TBL. A.10.2.2 & SEC. 38.3.3.2.1
CORRIDORS	CLASS B	TABLE 803.9	CLASS A OR CLASS B	TBL. A.10.2.2 & SEC. 38.3.3.2.1
ROOMS AND ENCLOSED SPACES / OTHER SPACES	CLASS B OR C	TABLE 803.9	CLASS A, B OR C	TBL. A.10.2.2 & SEC. 38.3.3.2.2
FLOORS (EXIT PASSAGEWAYS & CORRIDORS ONLY)	CLASS II (NOTE 6)	SECTION 804.4.1	CLASS I OR II	TBL. A.10.2.2 & SEC. 38.3.3.3.2

2010 ADAAG

ACCESSIBLE FACILITIES		
DRINKING FOUNTAINS	50% ACCESSIBLE, NOT LESS THAN 1	
SPOUT HEIGHT	36" AFF (WHEELCHAIR) / 38" - 43" AFF (STANDING)	
TOILET FACILITIES		
FLOOR SPACE (TURNING)	60" DIAMETER	
WATER CLOSET		
LOCATION	℄ (ACCESSIBLE WATER CLOSETS)	
SEAT (HEIGHT)	17" TO 19" MAXIMUM AFF	
CLEAR FLOOR SPACE - NO STALL	60" MIN. FROM SIDE WALL / 56" MIN. FROM REAR WALL	
CLEAR FLOOR SPACE - IN STALL	60" MIN. WIDTH / 56" MIN. DEPTH WALL HUNG 59" MIN. DEPTH FLOOR MOUNTED	
GRAB BARS	TOP AT 33" TO 36" AFF	
SIDE WALL BARS (HORIZONTAL)	12" FROM BACK WALL AND EXTEND TO 54"	
SIDE WALL BARS (VERTICAL)	18" LONG WITH BOTTOM AT 39" - 41" AFF & ℄ LOCATED 39" - 41" FROM REAR WALL	
REAR WALL BARS (HORIZONTAL)	24" MIN. CENTERED ON WATER CLOSET WHERE PERMITTED BAR SHALL BE 36"	
URINALS	RIM AT 17" MAX. AFF (WALL HUNG)	
CLEAR FLOOR SPACE	30" WIDE x 48" DEEP	
LAVATORIES & SINKS	RIM AT 34" MAX. AFF / CLEARANCE OF 27" MIN AFF BOTTOM OF APRON	
CLEAR FLOOR SPACE	30" x 48" MIN. CLEAR / EXTEND UP TO 25" MAX. UNDER LAVATORY	
EXPOSED PIPES AND SURFACES	INSULATED OR PROTECTED	
MIRRORS (REFLECTING SURFACE)	BOTTOM AT 40" MAX. AFF OVER LAVATORY	
CABINETS AND COUNTERS	34" MAX. HEIGHT / 28" MIN. HEIGHT - KNEE SPACE 27" CLEAR HEIGHT	
PARALLEL APPROACH - CABINETS & COUNTERS	34" MAX. HEIGHT / 28" MIN. HEIGHT	
KITCHEN FACILITIES		
KITCHEN SINKS & LAVATORIES		
CLEAR FLOOR SPACE	A CLEAR FLOOR SPACE COMPLYING WITH SECTION 305.3. POSITIONED FOR FORWARD APPROACH SHALL BE PROVIDED. KNEE AND TOE CLEARANCE COMPLYING WITH SECTION 306 SHALL BE PROVIDED. EXCEPTIONS: A PARALLEL APPROACH COMPLYING WITH SECTION 305 SHALL BE PERMITTED TO A KITCHEN SINK IN A SPACE WHERE A COOK TOP OR CONVENTIONAL RANGE IS NOT PROVIDED.	
HEIGHT & CLEARANCES	THE FRONT OF LAVATORIES AND SINKS SHALL BE 34" MAX. ABOVE THE FLOOR OR GROUND, MEASURED TO THE HIGHER OF THE FIXTURE RIM OR COUNTER SURFACE.	
FAUCETS	FAUCETS SHALL COMPLY WITH SECTION 309. HAND-OPERATED, SELF-CLOSING FAUCETS SHALL REMAIN OPEN FOR 10 SECONDS MIN.	

NOTES:

James R. Childers Architect, Inc.
314 Lexington Avenue
Fort Smith, AR 72901
479-783-2480
www.childersarchitect.com

PROFESSIONAL SEAL:

CONSULTANT LOGO:

CLIENT:

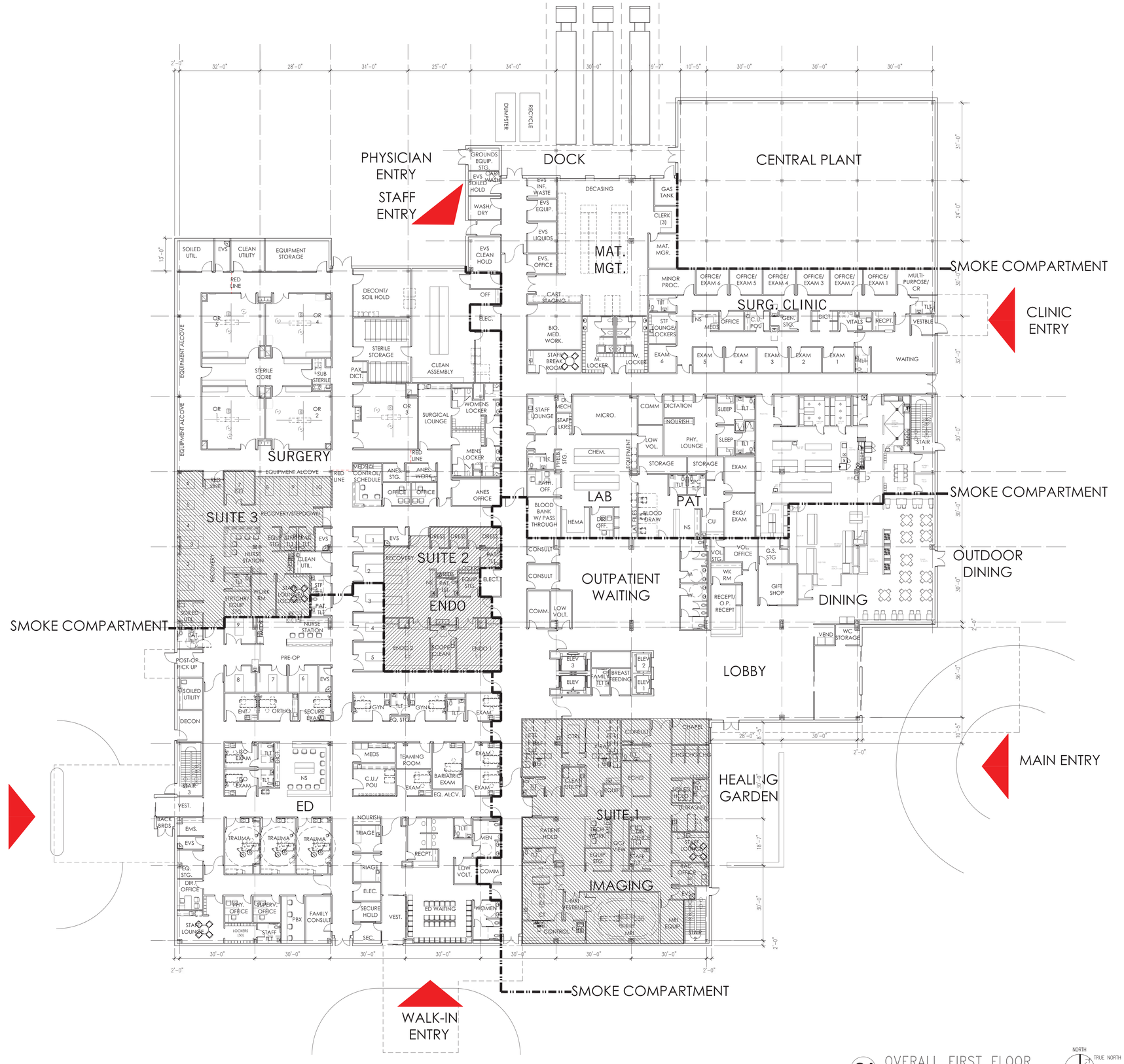
CHEROKEE NATION
W.W. HASTINGS HOSPITAL
TAHLEQUAH, OKLAHOMA

PROJECT PHASE:
100% SCHEMATIC DESIGN (NOT FOR CONSTRUCTION)

REVISIONS

#	DATE	DESCRIPTION

DATE: 03-21-14
JOB NUMBER: 13-13
SHEET NUMBER: A9.0
CODE ANALYSIS



01 OVERALL FIRST FLOOR
1/16" = 1'-0"

NORTH
TRUE NORTH

RedFern Group

PROFESSIONAL SEAL:

CONSULTANT LOGO:

CLIENT:

CHEROKEE NATION
W.W. HASTINGS HOSPITAL
TAHLEQUAH, OKLAHOMA

PROJECT PHASE:
100% SCHEMATIC DESIGN (NOT FOR CONSTRUCTION)

REVISIONS		
#	DATE	DESCRIPTION

DATE:	JOB NUMBER:
03-21-14	13-13

SHEET NUMBER:
A 9.1

FIRST FLOOR PRELIMINARY LIFE SAFETY PLAN

01 OVERALL SECOND FLOOR
1/16" = 1'-0"

NORTH
TRUE NORTH

RedFern Group

PROFESSIONAL SEAL:

CONSULTANT LOGO:

CLIENT:

SEAL OF THE CHEROKEE NATION
SEPT. 6, 1839

CHEROKEE NATION
W.W. HASTINGS HOSPITAL
TAHLEQUAH, OKLAHOMA

PROJECT PHASE:
100% SCHEMATIC DESIGN (NOT FOR CONSTRUCTION)

REVISIONS		
#	DATE	DESCRIPTION

DATE: 03-21-14
JOB NUMBER: 13-13
SHEET NUMBER: A 9.2

SECOND FLOOR PRELIMINARY LIFE SAFETY PLAN

01 OVERALL THIRD FLOOR
1/16" = 1'-0"

RedFern Group

PROFESSIONAL SEAL:

CONSULTANT LOGO:

CLIENT:

CHEROKEE NATION
W.W. HASTINGS HOSPITAL
TAHLEQUAH, OKLAHOMA

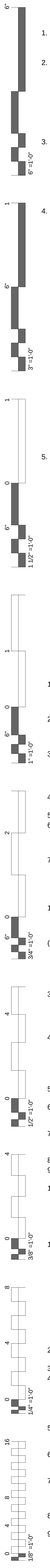
PROJECT PHASE:
100% SCHEMATIC DESIGN (NOT FOR CONSTRUCTION)

REVISIONS		
#	DATE	DESCRIPTION

DATE:	JOB NUMBER:
03-21-14	13-13

SHEET NUMBER:
A 9.3

THIRD FLOOR PRELIMINARY LIFE SAFETY PLAN



DESIGN PARAMETERS

1.		BUILDING CODE	IBC 2009
		OCCUPANCY CATEGORY	IV
2.		LIVE LOADS	
	A.	ROOF	20 PSF
	B.	PATIENT ROOMS	40 PSF
	C.	OPERATING ROOMS, LABORATORIES	60 PSF
	D.	CORRIDORS	80 PSF
	E.	LOBBIES	100 PSF
	F.	STAIRS	100 PSF
3.		ROOF SNOW LOAD	
	A.	GROUND SNOW LOAD, Pg	10 PSF
	B.	FLAT ROOF SNOW LOAD, Pf	12 PSF
	C.	SNOW EXPOSURE FACTOR, Ce	1.0
	D.	SNOW LOAD IMPORTANCE FACTOR, I	1.2
	E.	THERMAL FACTOR, Ct	1.0
4.		WIND DESIGN DATA	
	A.	BASIC WIND SPEED (3 SECOND GUST), V	90 MPH
	B.	WIND IMPORTANCE FACTOR, I	1.00
	C.	WIND EXPOSURE CATEGORY	C
	D.	INTERNAL PRESSURE COEFFICIENT, GCpi	+/- 0.18
	E.	DESIGN WIND PRESSURE ON COMPONENTS AND CLADDING	
		1.) WALLS (150 SQUARE FEET EFFECTIVE WIND AREA)	
		END ZONES	19.2 PSF
		INTERIOR ZONES	17.7 PSF
		2.) ROOF, ASD (NET UPLIFT)	
		CORNER ZONES	23.1 PSF
		END ZONES	23.1 PSF
		INTERIOR ZONES	19.5 PSF
	F.	WIDTH OF END ZONE	12 FT
	E.	DESIGN WIND PRESSURE ON COMPONENTS AND CLADDING (TOWER)	
		1.) WALLS (200 SQUARE FEET EFFECTIVE WIND AREA)	
		END ZONES	33.1 PSF
		INTERIOR ZONES	22 PSF
		2.) ROOF, ASD (NET UPLIFT)	
		CORNER ZONES	61.1 PSF
		END ZONES	44.0 PSF
		INTERIOR ZONES	26.9 PSF
	F.	WIDTH OF END ZONE	8.5 FT
5.		EARTHQUAKE DESIGN DATA	
	A.	SEISMIC IMPORTANCE FACTOR, I	1.5
	B.	MAPPED SPECTRAL RESPONSE ACCELERATION, Ss	18.5%
	C.	MAPPED SPECTRAL RESPONSE ACCELERATION, S1	7.8%
	D.	SITE CLASS	D
	E.	SPECTRAL RESPONSE COEFFICIENT, Sds	0.197
	F.	SPECTRAL RESPONSE COEFFICIENT, Sd1	0.125
	G.	SEISMIC DESIGN CATEGORY	C
	H.	STRUCTURAL SYSTEM (DIRECTION 1)	--
		1.) BASIC SEISMIC FORCE-RESISTING SYSTEM TYPE	H. STRUCTURAL STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE
		2.) VERTICAL ELEMENT TYPE	BRACED FRAMES
		3.) DESIGN BASE SHEAR, LRFD	-- W
		4.) SEISMIC RESPONSE COEFFICIENT, Cs	--
		5.) RESPONSE MODIFICATION FACTOR, R	--
	J.	ANALYSIS PROCEDURE	EQUIVALENT LATERAL FORCE

GENERAL NOTES

GENERAL

1. STRUCTURAL ELEMENTS ARE NON-SELF SUPPORTING AND REQUIRE INTERACTION WITH OTHER ELEMENTS FOR STABILITY AND RESISTANCE TO LATERAL FORCES. FRAMING AND WALLS SHALL BE TEMPORARILY BRACED BY THE CONTRACTOR UNTIL PERMANENT BRACING, FLOOR AND ROOF DECKS, AND WALLS HAVE BEEN INSTALLED AND CONNECTIONS BETWEEN THESE ELEMENTS HAVE BEEN MADE.
2. THE SPECIFICATIONS AND STRUCTURAL DRAWINGS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE METHOD OF CONSTRUCTION, UNLESS NOTED OTHERWISE. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE MEANS, METHODS, TECHNIQUES, SEQUENCES, AND OPERATION OF CONSTRUCTION AND SAFETY PRECAUTIONS AND PROGRAMS INCIDENTAL THERETO.
3. THE SIZE AND LOCATION OF EQUIPMENT PADS AND PENETRATIONS THROUGH THE STRUCTURE FOR MECHANICAL, ELECTRICAL, AND PLUMBING WORK SHALL BE VERIFIED BY THE CONTRACTOR. PENETRATIONS SHALL BE SUBJECT TO APPROVAL BY THE STRUCTURAL ENGINEER. REFER TO MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR OPENING LOCATIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS.
4. USE ONLY DIMENSIONS INDICATED ON THE DRAWINGS. DO NOT SCALE DRAWINGS OR USE ANY DIMENSIONS TAKEN FROM ELECTRONIC DRAWING FILES.
5. ASSUME EQUAL SPACING IF NOT INDICATED ON DRAWINGS.
6. THE SPECIFICATIONS ARE AN INTEGRAL PART OF THE CONTRACT DOCUMENTS AND SHALL BE USED IN CONJUCTION WITH THE STRUCTURAL DRAWINGS. WHERE REQUIREMENTS INDICATED ON THE STRUCTURAL DRAWINGS DIFFER FROM THE SPECIFICATIONS, NOTIFY THE ARCHITECT AND THE STRUCTURAL ENGINEER.
7. ARCHITECTURAL, MECHANICAL AND ELECTRICAL COMPONENTS AND SYSTEMS SHALL BE DESIGNED AND CONSTRUCTED TO RESIST SEISMIC FORCES AS DETERMINED IN CHAPTER 13 OF ASCE 7.

FOUNDATIONS

1. FOUNDATION DESIGNS, SUBGRADE PREPARATION NOTES, AND STRUCTURAL EARTH MOVING SPECIFICATION ARE BASED ON THE RECOMMENDATIONS PROVIDED IN THE GEOTECHNICAL REPORT NUMBER _____, BY: ___________________________ DATED: _______________ (AND ADDENDUM DATED ______________)

(2. PIER DESIGNS ARE BASED ON A NET ALLOWABLE END BEARING PRESSURE OF _____PSF IN COMBINATION WITH AN ALLOWABLE SIDE FRICTION OF ____PSF FOR THAT PORTION OF THE PIER EXTENDING MORE THAN ___FEET INTO APPROVED BEARING MATERIAL. SIDE FRICTION MAY BE USED TO RESIST BOTH UPWARD AND DOWNWARD FORCES. ALL PIERS SHALL EXTEND A MINIMUM OF __FEET INTO APPROVED BEARING MATERIAL AS NOTED IN THE GEOTECHNICAL REPORT. FOR BIDDING PURPOSES ONLY, DEPTH TO BOTTOM OF PIERS SHALL BE ASSUMED TO BE ____FEET BELOW FINISHED FLOOR.)

3. CONTRACTOR AND TESTING LABORATORY REPRESENTATIVE SHALL READ THE GEOTECHNICAL REPORT AND BECOME THOROUGHLY FAMILIAR WITH SITE AND SUBGRADE INFORMATION GIVEN THEREIN. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DETERMINING EXACT QUANTITIES OF CUT AND FILL FOR ESTIMATING AND CONSTRUCTION. SUBGRADE SHALL BE PREPARED AS NOTED IN THE STRUCTURAL EARTH MOVING SPECIFICATION.
4. A QUALIFIED AND REGISTERED GEOTECHNICAL ENGINEER, LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED AND WORKING FOR THE TESTING LABORATORY, SHALL DETERMINE CONFORMANCE OF THE FOUNDATION BEARING STRATA WITH THE FOUNDATION DESIGN CRITERIA ABOVE, AND ALL OTHER CONTRACT DOCUMENTS. TESTING LABORATORY SHALL NOTIFY CONTRACTOR, ARCHITECT AND CONSULTING ENGINEER OF ANY CONDITIONS NOT IN ACCORDANCE WITH FOUNDATION DESIGN CRITERIA OR CONTRACT DOCUMENTS.
5. USE ONLY STRUCTURAL FILL MATERIAL AS NOTED IN THE STRUCTURAL EARTH MOVING SPECIFICATION FOR FILL BELOW BUILDING AND FIVE FEET BEYOND THE EDGES OF THE BUILDING.
6. EXTERIOR FOOTINGS (GRADE BEAMS) SHALL BEAR AT OR BELOW MINIMUM BEARING DEPTH. MINIMUM BEARING DEPTH IS ____ INCHES BELOW ADJACENT FINISHED GRADE. THICKENED SLAB EDGE FOR STOOPS, CANOPIES, ETC. SHALL EXTEND _____ INCHES BELOW GRADE UNLESS NOTED OTHERWISE.
7. FOUNDATION WALLS SHALL HAVE ADEQUATE TEMPORARY BRACING INSTALLED BY THE CONTRACTOR BEFORE BACKFILL IS PLACED AGAINST THEM. TEMPORARY BRACING SHALL NOT BE REMOVED UNTIL WALL IS PERMANENTLY BRACED.
8. FOOTINGS SHALL BE POURED AGAINST UNDISTURBED SOIL, UNLESS NOTED OTHERWISE.
9. AVOID DAMAGE TO UNDERGROUND UTILITIES SUCH AS WATER MAINS, SANITARY SEWERS, BURIED CABLES, ETC., WHICH MIGHT EXTEND ACROSS OR ADJOIN SITE.
10. IT IS ANTICIPATED THAT TEMPORARY CASING MAY BE REQUIRED TO CONTROL GROUND WATER INFLOW AND PREVENT CAVING OF THE PIER EXCAVATION SIDEWALLS. SEE THE GEOTECHNICAL REPORT FOR MORE INFORMATION.

CONCRETE

1. MINIMUM COMPRESSIVE STRENGTH (f'c) AT THE END OF 28 DAYS SHALL BE AS FOLLOWS:

A. PIERS, GRADE BEAMS	3000 PSI
B. FOUNDATION WALLS AND PEDESTALS	4000 PSI
C. SLABS-ON-GRADE	3000 PSI
D. SLABS ON COMPOSITE DECK (SUSPENDED SLABS)	3000 PSI
E. STEEL STAIR PANS (SLABS ON NON-COMPOSITE DECK)	3000 PSI
F. EXTERIOR CONCRETE/SIDEWALKS	4500 PSI

REFER TO SPECIFICATIONS FOR MAXIMUM WATER/CEMENT RATIOS, MINIMUM CEMENT CONTENTS AND OTHER MIX DESIGN REQUIREMENTS. CONCRETE SHALL BE NORMAL WEIGHT (145 PCF), UNLESS NOTED OTHERWISE.

2. EXTERIOR CONCRETE AND CONCRETE EXPOSED TO FREEZE-THAW CYCLES SHALL BE AIR-ENTRAINED. REFER TO SPECIFICATIONS FOR AIR CONTENT.
3. MATERIALS OR ADMIXTURES SHALL NOT CONTAIN ANY CALCIUM CHLORIDE.
4. REINFORCING STEEL SHALL MEET THE FOLLOWING:

A. DEFORMED BARS	ASTM A615, GRADE 60
B. WELDABLE DEFORMED BARS	ASTM A706, GRADE 60
C. WELDED WIRE FABRIC	ASTM A185
D. STEEL FIBERS	ASTM A820

5. WHERE DOWELS ARE INDICATED BUT NOT SIZED, PROVIDE DOWELS THAT MATCH SIZE AND LOCATION OF MAIN REINFORCING STEEL AND LAP SPLICE WITH THE MAIN REINFORCING STEEL. REINFORCING BARS SHALL BE SPLICED AS NOTED IN THE REINFORCING LAP SCHEDULE.
6. REFER TO ACI 318 LATEST EDITION FOR CONCRETE COVER, ACI 315 LATEST EDITION FOR DETAILING PRACTICES AND FABRICATION, AND ACI 301 LATEST EDITION FOR STANDARD PRACTICE FOR MIXING AND PLACING CONCRETE.
7. "C.J." INDICATES SAW CUT CONTRACTION JOINT OR DOWELED CONSTRUCTION JOINT IN SLAB-ON-GRADE. REFERENCE SPECIFICATIONS FOR ACCEPTED SAW CUT METHODS. SLAB POURS SHALL BE SEPARATED BY A DOWELED CONSTRUCTION JOINT. CONTRACTION/CONSTRUCTION JOINTS SHALL BE LOCATED AS SHOWN ON PLANS OR AS DIRECTED BY THE STRUCTURAL ENGINEER.
8. PROVIDE CORNER BARS THAT MATCH AND LAP CONTINUOUS REINFORCEMENT SIZE AND QUANTITY AT INTERSECTIONS AND CORNERS OF WALLS AND FOUNDATIONS.
9. PROVIDE #3 Z-BAR SPACERS AT 24 INCHES ON CENTER EACH WAY FOR CONCRETE WALLS HAVING REINFORCING STEEL IN BOTH FACES.

GENERAL NOTES

10. ANCHORS INSTALLED IN HARDENED CONCRETE SHALL ONLY BE USED WHERE SPECIFIED ON THE CONTRACT DRAWING. THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE ENGINEER-OF-RECORD PRIOR TO INSTALLING POST-INSTALLED ANCHORS IN PLACE OF MISSING OR MISPLACED CAST-IN-PLACE ANCHORS. CARE SHALL BE TAKEN IN PLACING POST-INSTALLED ANCHORS TO AVOID CONFLICTS WITH EXISTING REINFORCING. HOLES SHALL BE DRILLED, DRY AND CLEANED AND ANCHORS INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S PUBLISHED WRITTEN INSTRUCTIONS AND APPLICABLE ESR REPORT. REFERENCE DETAILS FOR ANCHOR SIZE AND EMBEDMENT. SUBSTITUTION REQUESTS FOR PRODUCTS OTHER THAN THOSE SPECIFIED ON THE CONTRACT DRAWINGS SHALL BE SUBMITTED BY THE CONTRACTOR TO THE ENGINEER-OF-RECORD ALONG WITH CALCULATIONS THAT ARE SIGNED AND SEALED BY THE QUALIFIED PROFESSIONAL ENGINEER RESPONSIBLE FOR THEIR PREPARATION AND LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED. THE CALCULATIONS SHALL DEMONSTRATE THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE PERTINENT EQUIVALENT PERFORMANCE VALUES (MINIMUM) OF THE SPECIFIED PRODUCT USING THE APPROPRIATE DESIGN PROCEDURE AND/OR STANDARD(S) AS REQUIRED BY THE BUILDING CODE. ALLOWABLE SUBSTITUTIONS FOR POST-INSTALLED ANCHORS IN CONCRETE ARE:
    A. HILTI HIT RE 500-SD EPOXY ADHESIVE (ICC-ES ESR-2322).
    B. HILIT HIT HY 150 MAX-SD ADHESIVE (ICC-ES ESR-3013).
    C. HILTI KWIK BOLT TZ EXPANSION ANCHOR (ICC-ES ESR-1917).
    D. SIMPSON STRONG-TIE SET-XP EPOXY ADHESIVE (ICC-ES ESR-2508).
    E. SIMPSON STRONG-TIE AT-XP ADHESIVE (IAPMO UES ER-263).
    F. SIMPSON STRONG-TIE STRONG BOLT WEDGE ANCHOR (ICC-ES ESR-1771) - INTERIOR APPLICATIONS ONLY.
    G. SIMPSON STRONG-TIE STRONG BOLT 2 WEDGE ANCHOR (ICC-ES ESR-3037) - EXTERIOR APPLICATIONS.
11. INCLUDE AN ALLOWANCE IN THE BID PRICE FOR _________POUNDS OF REINFORCING STEEL TO BE FABRICATED AND PLACED AS DIRECTED BY ARCHITECT OR ENGINEER. ALLOWANCE IS TO INCLUDE, BUT IS NOT LIMITED TO, MATERIAL, DETAILING, FABRICATION, SHIPPING, INSTALLATION, OVERHEAD AND PROFIT.

STRUCTURAL STEEL

1. STRUCTURAL STEEL SHALL MEET THE FOLLOWING MINIMUM YIELD STRESS (Fy):

.	YIELD	ASTM SPECIFICATION
A. W, WT SHAPES:	50 KSI	A992
B. BARS, PLATES, CHANNELS, ANGLES:	36 KSI	A36
C. SQUARE, RECTANGULAR HSS:	46 KSI	A500, GRADE B
D. ROUND HSS:	42 KSI	A500, GRADE B
E. STRUCTURAL STEEL PIPE:	35 KSI	A53, GRADE B
F. ANCHOR RODS:	36 KSI	F1554
G. ALL-THREAD RODS:	36 KSI	A36
H. HEADED STUD ANCHORS:	65 KSI TENSILE STRESS	A108, GRADES 1010-1020

2. BOLTS FOR STEEL BEAM AND COLUMN CONNECTIONS SHALL BE 3/4-INCH DIAMETER ASTM A325-N HIGH-STRENGTH BOLTS UNLESS NOTED OTHERWISE. ALL BOLTED CONNECTIONS ARE BEARING TYPE UNLESS NOTED OTHERWISE. ALL BOLTS SHALL BE TIGHTENED SNUG TIGHT UNLESS NOTED OTHERWISE. FOR PRETENSIONED OR SLIP-CRITICAL JOINTS, THE METHOD OF INSTALLATION SHALL BE TURN-OF-NUT WITH MATCH MARKING, TWIST-OFF-TYPE TENSION CONTROL BOLT ASSEMBLIES (ASTM F1852), OR DIRECT TENSION INDICATORS (ASTM F959).
3. WELDING SHALL MEET ANSI / AWS D1.1, STRUCTURAL WELDING CODE LATEST REVISION. ELECTRODES SHALL BE 70 KSI, LOW HYDROGEN.
4. PROVIDE DOUBLE NUTS AND DOUBLE WASHERS FOR STEEL COLUMN ANCHOR BOLTS TO ALLOW FOR ADJUSTMENT IN BASE PLATE ELEVATION. PROVIDE 1 1/2 INCH NON-SHRINK GROUT UNDER BASE PLATE AFTER ERECTION. USE 2 1/2 INCH NON-SHRINK GROUT WHEN COLUMN ANCHOR BOLTS ARE 1 1/4 INCH DIAMETER OR LARGER. NON-SHRINK GROUT SHALL BE NON-METALLIC WITH A MINIMUM COMPRESSIVE STRENGTH OF 5,000 PSI AT 28 DAYS.
5. ALL CONNECTIONS NOT FULLY DETAILED ON THE STRUCTURAL DRAWINGS SHALL BE DESIGNED AND DETAILED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED, EMPLOYED OR RETAINED BY THE STEEL FABRICATOR. THE DESIGN AND DETAILING SHALL COMPLY WITH ALL APPLICABLE CODES AND SPECIFICATION SECTIONS.
6. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR INCLUDING THE COSTS FOR ALL MISCELLANEOUS STEEL IN THEIR BID REGARDLESS OF WHETHER THOSE ITEMS ARE INDICATED ON THE STRUCTURAL DRAWINGS. THESE COSTS SHALL INCLUDE BUT ARE NOT LIMITED TO MISCELLANEOUS STEEL ITEMS SHOWN ON ARCHITECTURAL, CIVIL, MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS.
7. INCLUDE AN ALLOWANCE IN THE BID PRICE FOR _______POUNDS OF MISCELLANEOUS STRUCTURAL STEEL TO BE FABRICATED AND PLACED AS DIRECTED BY ARCHITECT OR ENGINEER. ALLOWANCE IS TO INCLUDE, BUT IS NOT LIMITED TO, MATERIAL, DETAILING, FABRICATION, SHIPPING, INSTALLATION, OVERHEAD AND PROFIT.

STEEL JOISTS

1. STEEL JOISTS SHALL BE AS INDICATED ON THE PLANS AND SHALL BE IN ACCORDANCE WITH THE SPECIFICATIONS OF THE STEEL JOIST INSTITUTE (SJI) AND MEET THE FOLLOWING:
    A. JOISTS SHALL BE DESIGNED FOR THE UNIFORM LOAD CAPACITY (AS SPECIFIED IN THE SJI STANDARD LOAD TABLES) IN ADDITION TO THE CONCENTRATED LOADS SHOWN ON PLANS AND DETAILS.
    B. JOISTS THAT SUPPORT CONCENTRATED LOADS SHALL HAVE THEIR CHORDS DESIGNED TO WITHSTAND ALL BENDING STRESSES, OR THE LOADS SHALL OCCUR WITHIN 3 INCHES OF JOIST PANEL POINTS, OR THE JOIST SHALL BE REINFORCED PER THE "JOIST REINFORCING DETAIL" SHOWN HEREIN. CONCENTRATED LOADS SHALL BE CENTERED ON JOISTS AND NOT ATTACHED TO THE EDGE OF CHORD ANGLES.
    C. JOISTS SHALL RESIST THE NET UPLIFT PRESSURE AS INDICATED ON THE "ROOF (NET UPLIFT)" SECTION OF THE DESIGN PARAMETERS FOR "DESIGN WIND PRESSURE ON COMPONENTS AND CLADDING". THIS PRESSURE SHALL ACT ALONE. AN ALLOWABLE STRESS INCREASE IS NOT PERMITTED.
    D. FOR ALL MEMBERS THAT REQUIRE SPECIFIC ORIENTATION, PROVIDE TAG AT ONE END AND DEFINE LOCATION OF TAGGED END ON ERECTION DRAWINGS.
    E. JOIST MANUFACTURER SHALL DETERMINE THE SEAT DEPTH AND WIDTH OF BEARING AND COORDINATE THE SAME WITH THE STEEL FABRICATOR. THE FOLLOWING SEAT DEPTHS ARE ASSUMED ON THE DRAWINGS: 2 1/2 INCH FOR K-SERIES JOISTS (5 INCH FOR LH AND DLH SERIES JOISTS).
2. K-SERIES JOISTS SHALL BE WELDED TO SUPPORTING STEEL WITH MINIMUM 1/8 INCH FILLET WELDS 2 INCHES LONG EACH SIDE OR WITH TWO 1/2 INCH DIAMETER ASTM A307 BOLTS OR THE EQUIVALENT, UNLESS NOTED OTHERWISE. WHEN NEAR OR AT A COLUMN, BOLT JOIST TO SUPPORTING STEEL IN CONFORMANCE WITH OSHA.

(2. (LH AND DLH-SERIES JOISTS SHALL BE WELDED TO SUPPORTING STEEL WITH MINIMUM 1/4 INCH FILLET WELDS 2 INCHES LONG EACH SIDE OR WITH TWO 3/4 INCH DIAMETER ASTM A307 BOLTS OR THE EQUIVALENT, UNLESS NOTED OTHERWISE.)

3. JOIST BRIDGING AND ERECTION STABILITY SHALL BE PROVIDED IN ACCORDANCE WITH THE OCCUPATIONAL SAFETY AND HAZARD ADMINISTRATION (OSHA) AND THE SPECIFICATIONS OF THE STEEL JOIST INSTITUTE (SJI).
4. JOIST RTU LOADS ARE PROVIDED ON THE ROOF FRAMING PLAN, REFERENCE PLANS AND DETAILS FOR LOAD LOCATIONS, VALUES AND SUPPORT FRAMING.
5. JOIST MANUFACTURER SHALL DESIGN THE COMPRESSION CHORD OF ALL JOISTS SUPPORTING ROOF TOP UNITS, SKY LIGHTS, AND OTHER STRUCTURES FOR AN UNBRACED LENGTH APPLICABLE TO THE CONDITIONS AT THE PROJECT WHERE THE UNBRACED LENGTH IS GREATER THAN THE SJI MAXIMUM. (REFERENCE ARCHITECTURAL AND MECHANICAL DRAWINGS)
6. DESIGN JOISTS FOR INTERNAL ROOF DRAINLINE LOCATIONS, IF REQUIRED. ADD 50 PLF FOR 8 INCH DIAMETER AND SMALLER, ADD 75 PLF FOR 10 INCH DIAMETER, ADD 102 PLF FOR 12 INCH DIAMETER, ADD 122 PLF FOR 14 INCH DIAMETER, ADD 200 PLF FOR 18 INCH DIAMETER. REFERENCE MECHANICAL DRAWINGS FOR EXACT LOCATION.
7. JOIST DESIGNS SHALL BE PERFORMED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED, EMPLOYED OR RETAINED BY THE JOIST MANUFACTURER.
8. SHOP DRAWING SHALL BE REVIEWED BY THE ARCHITECT AND STRUCTURAL ENGINEER OF RECORD PRIOR TO JOIST FABRICATION.

STEEL DECK

1. ROOF DECK
    A. ROOF DECK SHALL BE GALVANIZED TYPE "B". DEPTH SHALL BE AS SHOWN ON DRAWINGS.
    B. ROOF DECK IS REQUIRED TO ACT AS A DIAPHRAGM. CONNECTIONS SHALL BE IN ACCORDANCE WITH STEEL DECK INSTITUTE SPECIFICATIONS. REFER TO THE ROOF DIAPHRAGM CONNECTION DIAGRAM FOR ATTACHMENT.
    C. DECKING SHALL BE CONTINUOUS OVER A MINIMUM OF (3) SPANS UNLESS NOTED OTHERWISE.
    D. NO HANGING LOADS SHALL BE ATTACHED TO ROOF DECK.
2. FLOOR DECK
    A. COMPOSITE FLOOR DECK SHALL BE GALVANIZED COMPOSITE STEEL DECK. DEPTH SHALL BE AS SHOWN ON DRAWINGS.
    B. CONNECTIONS SHALL BE IN ACCORDANCE WITH STEEL DECK INSTITUTE SPECIFICATIONS AND AS FOLLOWS:
    1) PERPENDICULAR SUPPORTS - (4) 5/8 INCH DIAMETER PUDDLE WELDS PER SHEET (1) WELD AT EACH LOW RIB
    2) PARALLEL SUPPORTS - 5/8 INCH DIAMETER PUDDLE WELDS AT 12 INCHES ON CENTER.
    3) SIDELAPS - BUTTON PUNCHED AT 12 INCHES ON CENTER UNLESS NOTED OTHERWISE.
    4) DECKING SHALL BE CONTINUOUS OVER A MINIMUM OF (3) SPANS, UNLESS NOTED OTHERWISE.

COLD FORMED METAL FRAMING

1. ALL COLD FORMED METAL FRAMING SHALL HAVE A MINIMUM THICKNESS OF 33 MILS (20 GA) AND SHALL BE SPACED AT A MAXIMUM OF 16 INCHES ON CENTER UNLESS NOTED OTHERWISE AND SHALL MEET THE MINIMUM STRUCTURAL PROPERTIES FROM THE AMERICAN IRON AND STEEL INSTITUTE - NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING LATEST EDITION. MINIMUM FLANGE WIDTH OF FRAMING MEMBERS SHALL BE 1 5/8 INCH AND THE LIP LENGTH OF THE C-SHAPE PORTION SHALL BE A MINIMUM OF 1/2 INCH.
2. WALL STUDS AS BACKING TO MASONRY VENEER SHALL SHALL HAVE A MINIMUM THICKNESS OF 43 MILS (18 GA).
3. METAL FRAMING SHALL BE IN ACCORDANCE WITH THE FOLLOWING, UNLESS NOTED OTHERWISE:
    A. 54 MILS (16 GA) AND HEAVIER — ASTM A1003, GRADE 50 TYPE H (ST50H)
    B. 43 MILS (18 GA) AND LIGHTER — ASTM A1003, GRADE 33 TYPE H (ST33H)
    C. ACCESSORIES, TRACK AND OTHER MEMBERS — ASTM A1003, GRADE 33 TYPE H (ST33H), MINIMUM
4. TRACK SHALL BE 54 MILS (16 GA) MINIMUM FOR WALL STUDS 54 MILS (16 GA) OR LIGHTER. TRACK SHALL MATCH WALL STUD THICKNESS FOR WALL STUDS 68 MILS (14 GA) AND HEAVIER. TRACKS SHALL BE ANCHORED AS FOLLOWS:
    TO STEEL - HILTI X-U, 0.157 INCH DIAMETER KNURLED SHANK FASTENERS AT 12 INCHES ON CENTER (ESR-2269) OR APPROVED EQUAL, UNLESS NOTED OTHERWISE.
    TO CONCRETE - HILTI X-U, 0.157 INCH DIAMETER KNURLED SHANK FASTENERS AT 8 INCHES ON CENTER WITH 1 1/2 INCH EMBEDMENT (ESR-2269) OR APPROVED EQUAL, UNLESS NOTED OTHERWISE.
5. DO NOT WELD 33 MILS (20 GA) AND LIGHTER FRAMING, UNLESS SPECIFICALLY NOTED ON THE PLANS AND DETAILS.
6. CONNECTIONS SHALL CONSIST OF ANY OF THE FOLLOWING AS NOTED ON THE PLANS:
    A. SELF-DRILLING SCREWS OF TYPE AND SIZE AS SHOWN ON THE DETAILS.
    B. WELDS SHALL BE PERFORMED BY OPERATORS QUALIFIED IN ACCORDANCE WITH SECTION 6.0 OF AWS D1.3, SHEET METAL.
7. METAL FRAMING SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S WRITTEN RECOMMENDATIONS. HORIZONTAL BRACING FOR WALL STUDS SHALL BE PLACED AT 48 INCHES ON CENTER OR AS PER MANUFACTURER'S WRITTEN RECOMMENDATIONS IF LESS THAN 48 INCHES ON CENTER. HORIZONTAL BRIDGING FOR JOISTS SHALL BE PLACED AT 8'-0" ON CENTER OR AS PER MANUFACTURER'S WRITTEN RECOMMENDATIONS IF LESS THAN 8'-0" ON CENTER. APPLIED FINISH MATERIALS SHALL NOT BE CONSIDERED BRIDGING OR FLANGE BRACING UNLESS NOTED OTHERWISE.
8. ALL AXIALLY LOADED WALL STUDS SHALL HAVE FULL FLANGE BEARING AGAINST UPPER AND LOWER TRACK WEB PRIOR TO ATTACHMENT TO TRACK. SPLICES IN AXIALLY LOADED WALL STUDS ARE NOT ALLOWED.

ABBREVIATIONS

A.B.	ANCHOR BOLTS
A.F.F.	ABOVE FINISHED FLOOR
ACI	AMERICAN CONCRETE INSTITUTE
AESS	ARCHITECTURALLY EXPOSED STRUCTURAL STEEL
ARCH.	ARCHITECTURAL
B.L.	BLOCK LINTEL
B.O.	BOTTOM OF
B.O.D.	BOTTOM OF DECK
BAL.	BALANCE
BLDG.	BUILDING
BRG.	BEARING
C.J.	CONTRACTION JOINT
C.L.	CENTER LINE
CLR.	CLEAR
CMU	CONCRETE MASONRY UNIT
COL.	COLUMN
CONC.	CONCRETE
CONST.	CONSTRUCTION
CONT.	CONTINUOUS
D.B.A.	DEFORMED BAR ANCHOR
DIA.	DIAMETER
DWG.	DRAWING
E.F.	EACH FACE
E.J.	EXPANSION JOINT
E.O.D.	EDGE OF DECK
E.O.S.	EDGE OF SLAB
E.W.	EACH WAY
ELEV.	ELEVATION
EQ.	EQUAL
EXIST.	EXISTING
F.F.E.	FINISHED FLOOR ELEVATION
F.S.	FAR SIDE
FDN.	FOUNDATION
FTG.	FOOTING
G.B.	GRADE BEAM
GA.	GAGE
GALV.	GALVANIZED
H.S.A.	HEADED STUD ANCHOR
HORIZ.	HORIZONTAL
IBC	INTERNATIONAL BUILDING CODE
INFO.	INFORMATION
J.B.E.	JOIST BEARING ELEVATION
JT.	JOINT
K	UNIT OF 1,000 POUNDS (KIP)
KSI	KIPS PER SQUARE INCH
LBS.	POUNDS
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LONG.	LONGITUDINAL
MAX.	MAXIMUM
MECH.	MECHANICAL
MFR.	MANUFACTURER
MIN.	MINIMUM
MISC.	MISCELLANEOUS
N.I.C.	NOT IN CONTRACT
N.S.	NEAR SIDE
N.T.S.	NOT TO SCALE
NO.	NUMBER
O.C.	ON CENTER
O.D.	OUTSIDE DIAMETER
O.H.	OPPOSITE HAND
P.A.F.	POWER ACTUATED FASTENER
P.M.E.J	PREMOLDED EXPANSION JOINT
PCF	POUNDS PER CUBIC FOOT
PLF	POUNDS PER LINEAR FOOT
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
QTY.	QUANTITY
R.O.	ROUGH OPENING
RE:	REFER
REINF.	REINFORCING
REQD.	REQUIRED
RTU	ROOF TOP UNIT
S.D.S.	SELF-DRILLING SCREWS
SCHED.	SCHEDULE
SIM.	SIMILAR
SPECS.	SPECIFICATIONS
STD.	STANDARD
STL.	STEEL
T&B	TOP AND BOTTOM
T.O.	TOP OF
T.O.P.	TOP OF PIER
T.O.W.	TOP OF WALL
TRANS.	TRANSVERSE
TYP.	TYPICAL
U.N.O.	UNLESS NOTED OTHERWISE
VERT.	VERTICAL
W.P.	WORK POINT
W.W.F.	WELDED WIRE FABRIC
WT.	WEIGHT

DEFERRED STRUCTURAL SUBMITTALS

1. THE FOLLOWING STRUCTURAL COMPONENTS SHALL BE DESIGNED AND SUBMITTED BY OTHERS FOR APPROVAL IN ACCORDANCE WITH DRAWINGS AND SPECIFICATIONS.
    A. STRUCTURAL STEEL CONNECTIONS OF FRAMING AND BRACING ELEMENTS.
    B. STEEL, SELF-SUPPORTING STAIRS.
    C. COLD FORMED METAL WALL FRAMING AND ATTACHMENTS TO STRUCTURE.
    D. STOREFRONT AND CURTAINWALL FRAMING, ACCESSORIES, AND ATTACHMENTS TO STRUCTURE.
2. DOCUMENTS FOR DEFERRED STRUCTURAL SUBMITTAL ITEMS SHALL BE DESIGNED, SEALED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED. THE DEFERRED SUBMITTAL DOCUMENTS SHALL BE SUBMITTED TO THE ARCHITECT OR ENGINEER OF RECORD WHO SHALL REVIEW THEM AND FORWARD THEM TO THE BUILDING OFFICIAL AS REQUESTED WITH A NOTATION INDICATING THAT THE DEFERRED SUBMITTAL DOCUMENTS HAVE BEEN REVIEWED AND BEEN FOUND TO BE IN GENERAL CONFORMANCE TO THE DESIGN OF THE BUILDING. THE DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THE DESIGN AND SUBMITTAL DOCUMENTS HAVE BEEN APPROVED BY THE BUILDING OFFICIAL.

SPECIAL INSPECTIONS

1. THE OWNER SHALL EMPLOY ONE OR MORE SPECIAL INSPECTORS TO PROVIDE INSPECTIONS DURING CONSTRUCTION ON THE TYPES OF WORK LISTED IN THE STATEMENT OF SPECIAL INSPECTIONS PER SECTION 1704 OF THE IBC. THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON WHO SHALL DEMONSTRATE COMPETENCE, TO THE SATISFACTION OF THE BUILDING OFFICIAL, FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION. THESE INSPECTIONS ARE IN ADDITION TO THE INSPECTIONS SPECIFIED IN THE PROJECT SPECIFICATIONS.
2. SPECIAL INSPECTORS SHALL KEEP RECORDS OF INSPECTIONS. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. REPORTS SHALL INDICATE THAT WORK INSPECTED WAS DONE IN CONFORMANCE TO APPROVED CONSTRUCTION DOCUMENTS. DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF THE DISCREPANCIES ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE PRIOR TO COMPLETION OF THAT PHASE OF WORK. A FINAL REPORT DOCUMENTING REQUIRED SPECIAL INSPECTIONS AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED AT A POINT IN TIME AGREED UPON BY THE PERMIT APPLICANT AND THE BUILDING OFFICIAL PRIOR TO THE START OF WORK.
3. THE CONTRACTOR IS RESPONSIBLE FOR NOTIFYING THE SPECIAL INSPECTOR REGARDING INDIVIDUAL INSPECTION FOR ITEMS LISTED ON THE STATEMENT OF SPECIAL INSPECTIONS AND AS NOTED ON THE BUILDING DEPARTMENT APPROVED PLANS. ADEQUATE NOTICE AND ACCESS TO APPROVED PLANS SHALL BE PROVIDED SO THAT THE SPECIAL INSPECTOR HAS TIME TO BECOME FAMILIAR WITH THE PROJECT.
4. FABRICATORS OF STRUCTURAL LOAD-BEARING MEMBERS AND ASSEMBLIES SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1704.2 OF THE IBC.
    (provide additional requirements for special inspection for seismic resistance in structures assigned to Category C, D, E, or F)

(for projects that require structural observation in accordance with Section 1710 of the 2009 IBC – this must be included with the "Statement of Special Inspections")

STRUCTURAL OBSERVATION REQUIREMENTS

1. A REPRESENTATIVE OF THE ENGINEER OF RECORD (or name of other registered design professional or firm employed by the owner) WILL PERFORM THE VISUAL OBSERVATION OF THE STRUCTURAL SYSTEM FOR GENERAL CONFORMANCE TO THE APPROVED CONSTRUCTION DOCUMENTS AT SIGNIFICANT CONSTRUCTION STAGES AND AT COMPLETION OF THE STRUCTURAL SYSTEM. STRUCTURAL OBSERVATION DOES NOT INCLUDE OR WAIVE THE RESPONSIBILITY FOR THE INSPECTION REQUIRED OF THE BUILDING OFFICIAL OR THE SPECIAL INSPECTOR.
2. A PRE-CONSTRUCTION MEETING SHALL BE HELD AND ATTENDED BY THE ARCHITECT, ENGINEER OF RECORD, GENERAL CONTRACTOR, SUBCONTRACTORS, AND SPECIAL INSPECTORS.
3. THE GENERAL CONTRACTOR SHALL NOTIFY THE ENGINEER OF RECORD (or name of other registered design professional or firm employed by the owner) AT LEAST 48 HOURS PRIOR TO COMPLETING CONSTRUCTION OPERATIONS THAT REQUIRE STRUCTURAL OBSERVATION (BY CALLING (918) 584-5858 TO SCHEDULE A SITE VISIT.)
4. AT A MINIMUM, THE FOLLOWING SIGNIFICANT CONSTRUCTION STAGES REQUIRE A SITE VISIT AND AN OBSERVATION REPORT FROM THE STRUCTURAL OBSERVER:
    (A. AT FIRST DAY OF PIER DRILLING.)
    B. AFTER INSTALLATION OF FIRST FOUNDATION REINFORCING AND BEFORE CONCRETE PLACEMENT.
    C. AFTER INSTALLATION OF CONCRETE WALL REINFORCING AND BEFORE CONCRETE PLACEMENT.
    (E. AFTER INSTALLATION OF CONCRETE FLOOR (/ROOF) REINFORCING AND BEFORE CONCRETE PLACEMENT.)
    F. AFTER INSTALLATION OF COMPOSITE FLOOR DECK AND SHEAR STUDS AND BEFORE CONCRETE PLACEMENT.
    G. AFTER ERECTION OF STRUCTURAL STEEL AND BEFORE METAL DECK PLACEMENT.
    H. AFTER INSTALLATION AND FASTENING OF METAL DECK AND BEFORE PLACING INSULATION.
5. AT THE CONCLUSION OF THE WORK INCLUDED IN THE PERMIT, THE STRUCTURAL OBSERVER SHALL SUBMIT TO THE BUILDING OFFICIAL A WRITTEN STATEMENT THAT THE SITE VISITS HAVE BEEN MADE AND IDENTIFY ANY REPORTED DEFICIENCIES THAT, TO THE BEST OF THE STRUCTURAL OBSERVER'S KNOWLEDGE, HAVE NOT BEEN RESOLVED.

STATEMENT OF SPECIAL INSPECTIONS (IBC 2009)

		REQUIRED VERIFICATION AND INSPECTION TASK	FREQUENCY OF INSPECTION CONTINUOUS	FREQUENCY OF INSPECTION PERIODIC
I.		STEEL CONSTRUCTION (IBC TABLE 1704.3)		
1.		MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS AND WASHERS:		
	A.	IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	---	X
	B.	MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED.	---	X
2.		INSPECTION OF HIGH STRENGTH BOLTING:		
	A.	SNUG-TIGHT JOINT.	---	X
	B.	PRETENSIONED AND SLIP-CRITICAL JOINTS USING TURN-OF-NUT WITH MATCHMARKING, TWIST-OFF BOLT OR DIRECT TENSION INDICATOR METHODS OF INSTALLATION.	---	X
	C.	PRETENSIONED AND SLIP-CRITICAL JOINTS USING TURN-OF-NUT WITHOUT MATCHMARKING OR CALIBRATED WRENCH METHODS OF INSTALLATION.	X	---
3.		MATERIAL VERIFICATION OF STRUCTURAL STEEL AND COLD-FORMED STEEL DECK:		
	A.	FOR STRUCTURAL STEEL, IDENTIFICATION MARKINGS TO CONFORM TO AISC 360.	---	X
	B.	FOR OTHER STEEL, IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	---	X
	C.	MANUFACTURER'S CERTIFIED TEST REPORTS.	---	X
4.		MATERIAL VERIFICATION OF WELD FILLER MATERIALS:		
	A.	IDENTIFICATION MARKINGS TO CONFORM TO AWS SPECIFICATION IN THE APPROVED CONSTRUCTION DOCUMENTS.	---	X
	B.	MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED.	---	X
5.		INSPECTION OF WELDING:		
	A.	STRUCTURAL STEEL AND COLD-FORMED STEEL DECK:	---	---
		1) COMPLETE AND PARTIAL JOINT PENETRATION GROOVE WELDS.	X	---
		2) MULTIPASS FILLET WELDS.	X	---
		3) SINGLE-PASS FILLET WELDS > 5/16"	X	---
		4) PLUG AND SLOT WELDS.	X	---
		5) SINGLE-PASS FILLET WELDS < OR = 5/16"	---	X
		6) FLOOR AND ROOF DECK WELDS.	---	X
	B.	REINFORCING STEEL:	---	---
		1) VERIFICATION OF WELDABILITY OF REINFORCING STEEL OTHER THAN ASTM A 706	---	X
		2) SHEAR REINFORCEMENT	X	---
		3) OTHER REINFORCING STEEL.	---	X
6.		INSPECTION OF STEEL FRAME JOINT DETAILS FOR COMPLIANCE		
	A.	DETAILS SUCH AS BRACING AND STIFFENING.	---	X
	B.	MEMBER LOCATIONS.	---	X
	C.	APPLICATION OF JOINT DETAILS AT EACH CONNECTION.	---	X
II.		CONCRETE CONSTRUCTION (IBC TABLE 1704.4)		
1.		INSPECTION OF REINFORCING STEEL, INCLUDING PRESTRESSING TENDONS, AND PLACEMENT.	---	X
2.		INSPECTION OF REINFORCING STEEL WELDING IN ACCORDANCE WITH TABLE 1704.3, ITEM 5B.	---	---
3.		INSPECTION OF BOLTS TO BE INSTALLED IN CONCRETE PRIOR TO AND DURING PLACEMENT OF CONCRETE WHERE ALLOWABLE LOADS HAVE BEEN INCREASED OR WHERE STRENGTH DESIGN IS USED.	X	---
4.		INSPECTION OF ANCHORS INSTALLED IN HARDENED CONCRETE.	---	X
5.		VERIFYING USE OF REQUIRED DESIGN MIX.	---	X
6.		AT THE TIME FRESH CONCRETE IS SAMPLED TO FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	X	---
7.		INSPECTION OF CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	X	---
8.		INSPECTION FOR MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	---	X
11.		VERIFICATION OF IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.	---	X
12.		INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	---	X
V.		REQUIRED VERIFICATION AND INSPECTION OF SOILS (IBC TABLE 1704.7)		
1.		VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	---	X
2.		VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	---	X
3.		PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	---	X
4.		VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	X	---
5.		PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	---	X
VI.		REQUIRED VERIFICATION AND INSPECTION OF CAST-IN-PLACE DEEP FOUNDATION ELEMENTS (TABLE 1704.9)		
1.		OBSERVE DRILLING OPERATIONS AND MAINTAIN COMPLETE AND ACCURATE RECORDS FOR EACH ELEMENT.	X	---
2.		VERIFY PLACEMENT LOCATIONS AND PLUMBNESS, CONFIRM ELEMENT DIAMETERS, BELL DIAMETERS (IF APPLICABLE), LENGTHS, EMBEDMENT INTO BEDROCK (IF APPLICABLE) AND ADEQUATE END BEARING STRATA CAPACITY. RECORD CONCRETE OR GROUT VOLUMES.	X	---
3.		FOR CONCRETE ELEMENTS, PERFORM ADDITIONAL INSPECTIONS IN ACCORDANCE WITH REQUIREMENTS FOR CONCRETE CONSTRUCTION.	---	---
		CONTINUOUS SHALL MEAN: THE FULL-TIME OBSERVATION OF WORK PERIODIC SHALL MEAN: THE PART-TIME OR INTERMITTANT OBSERVATION OF WORK AND AT THE COMPLETION OF WORK		

RedFern Group

PROFESSIONAL SEAL:

CONSULTANT LOGO:

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200 East Brady Street
Tulsa, Oklahoma 74103
918.584.5858, Fax 918.584.8689
OKCA #1460, Exp. 06.30.15

CLIENT:

SEAL OF THE CHEROKEE NATION
SEPT. 6, 1839

CHEROKEE NATION
W.W. HASTINGS HOSPITAL
TAHLEQUAH, OKLAHOMA

PROJECT PHASE:
100% SCHEMATIC DESIGN SUBMITTAL

REVISIONS		
#	DATE	DESCRIPTION

DATE:	JOB NUMBER:
03-21-14	13-13

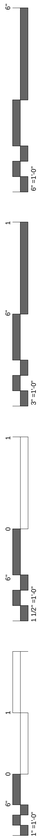
SHEET NUMBER:
S0.1

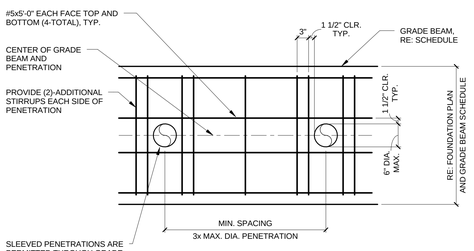
GENERAL NOTES

2009 IBC CONCRETE REINFORCING LAP SCHEDULE		
BAR SIZE	LAP LENGTH (f'c=3000psi)	
	TOP BARS (NOTE 1)	OTHER
#3	28"	22"
#4	38"	29"
#5	47"	36"
#6	56"	43"
#7	81"	63"
#8	93"	72"
#9	105"	81"
#10	118"	91"

NOTES
1. "TOP BARS" IS WHERE HORIZONTAL REINFORCEMENT IS PLACED SUCH THAT MORE THAN 12 INCHES OF FRESH CONCRETE IS CAST BELOW THE SPLICE.

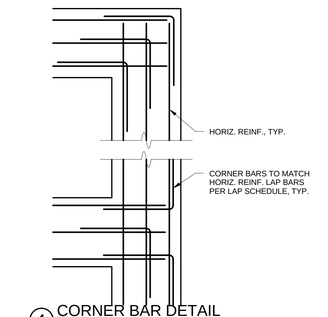
8 CONCRETE REINFORCING LAP SCHEDULE
3/4" = 1'-0"



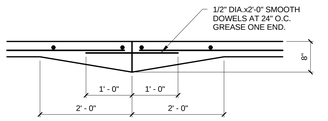


NOTIFY STRUCTURAL ENGINEER OF RECORD IF PROPOSED BEAM PENETRATIONS WILL NOT SATISFY THE CRITERIA SPECIFIED IN THIS DETAIL

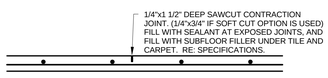
7 TYPICAL GRADE BEAM PENETRATION
3/4" = 1'-0"



4 CORNER BAR DETAIL
3/4" = 1'-0"

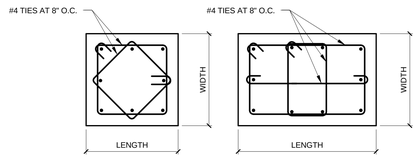


CONSTRUCTION JOINT (CONST. JT.)



CONTRACTION JOINT (C.J.)

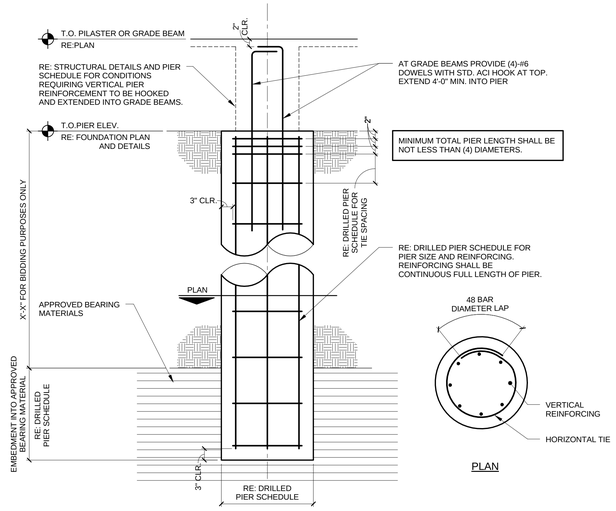
3 SLAB JOINT DETAILS
3/4" = 1'-0"



8 VERTICAL BARS | 10 VERTICAL BARS

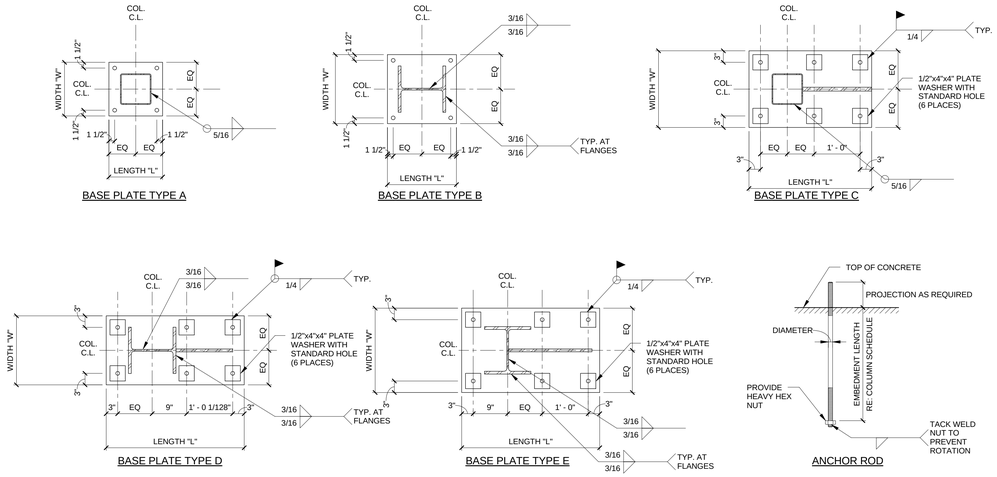
CONCRETE PILASTER SCHEDULE							
	SIZE		VERTICAL REINFORCING		TIES		
MARK	WIDTH	LENGTH	QUANTITY	SIZE	SIZE	SPACING	NOTES

6 PILASTER DETAILS AND SCHEDULE
3/4" = 1'-0"



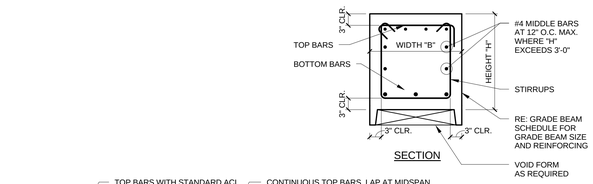
DRILLED PIER SCHEDULE							
		EMBEDMENT INTO APPROVED BEARING MATERIAL	VERTICAL REINFORCING		TIES		
MARK	DIAMETER		QUANTITY	SIZE	SIZE	SPACING	NOTES

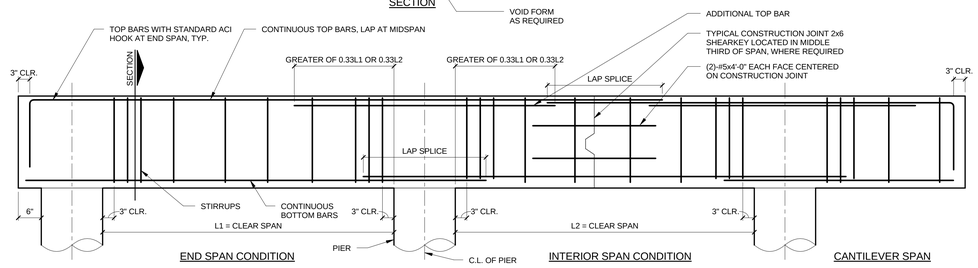
2 TYPICAL DRILLED PIER DIAGRAM AND SCHEDULE
3/4" = 1'-0"



STEEL COLUMN SCHEDULE									
		BASE PLATE				ANCHOR BOLTS			
MARK	TYPE	TYPE	T	W	L	NUMBER	DIA.	EMBED	NOTES

5 ANCHOR ROD AND BASE PLATE DIAGRAMS
3/4" = 1'-0"





NOTES:
1. REFER TO GRADE BEAM SCHEDULE FOR BEAM SIZE AND REINFORCING.
2. PROVIDE DOWELS FROM PIER INTO GRADE BEAM, U.N.O., RE: TYPICAL DRILLED PIER DIAGRAM.
3. ADDITIONAL TOP BARS SHALL BE THE LARGER OF THE TWO ADJACENT SPANS.
4. SINGLE-SPAN GRADE BEAMS SHALL HAVE TOP BARS HOOKED AT EACH END.
5. USE OF EARTH TO FORM THE GRADE BEAMS MAY ONLY BE USED IF THE SIDES OF THE TRENCH CAN BE CUT AND MAINTAINED IN A VERTICAL AND SMOOTH CONDITION UNTIL CONCRETE HAS BEEN PLACED. OTHERWISE GRADE BEAMS MUST BE FORMED AND CLEAR COVER MAY BE REDUCED FROM 3" TO 2".

GRADE BEAM SCHEDULE												
	SIZE		TOP BARS				BOTTOM BARS		STIRRUPS			
			CONTINUOUS		ADDITIONAL		CONTINUOUS					
MARK	WIDTH B	HEIGHT H	QUANTITY	SIZE	QUANTITY	SIZE	QUANTITY	SIZE	SIZE	TYPE	SPACING (EACH END)	NOTES

1 GRADE BEAM DIAGRAM AND SCHEDULE
3/4" = 1'-0"

RedFern Group

PROFESSIONAL SEAL:

CONSULTANT LOGO:

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W.W. HASTINGS HOSPITAL
TAHLEQUAH, OKLAHOMA

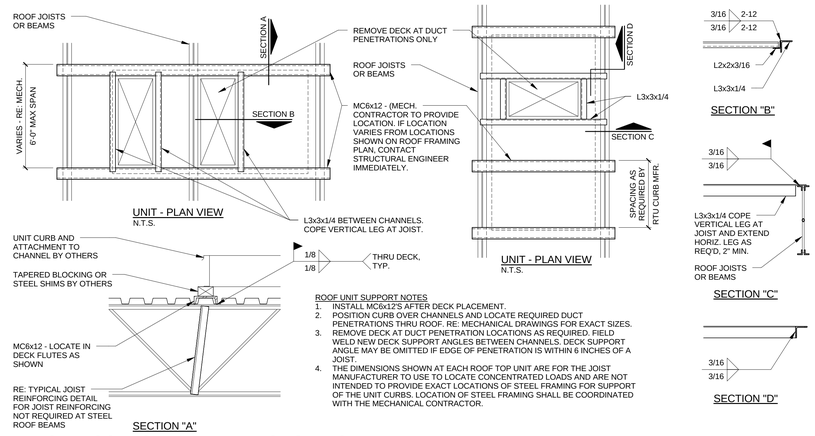
PROJECT PHASE:

100% SCHEMATIC DESIGN SUBMITTAL

REVISIONS		
#	DATE	DESCRIPTION

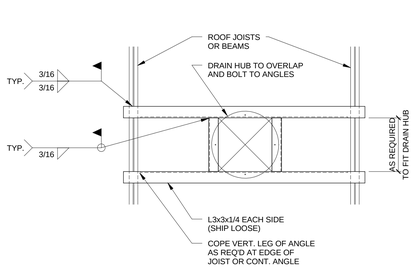
DATE: 03-21-14
JOB NUMBER: 13-13
SHEET NUMBER: S0.2

TYPICAL DETAILS AND SCHEDULES

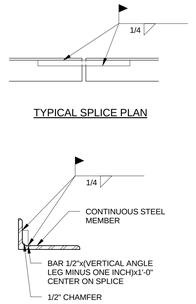


ROOF UNIT SUPPORT NOTES
1. INSTALL MC6x12'S AFTER DECK PLACEMENT.
2. POSITION CURB OVER CHANNELS AND LOCATE REQUIRED DUCT PENETRATIONS THRU ROOF. RE: MECHANICAL DRAWINGS FOR EXACT SIZES.
3. REMOVE DECK AT DUCT PENETRATION LOCATIONS AS REQUIRED. FIELD WELD NEW DECK SUPPORT ANGLES BETWEEN CHANNELS. DECK SUPPORT ANGLE MAY BE OMITTED IF EDGE OF PENETRATION IS WITHIN 6 INCHES OF A JOIST.
4. THE DIMENSIONS SHOWN AT EACH ROOF TOP UNIT ARE FOR THE JOIST MANUFACTURER TO USE TO LOCATE CONCENTRATED LOADS AND ARE NOT INTENDED TO PROVIDE EXACT LOCATIONS OF STEEL FRAMING FOR SUPPORT OF THE UNIT CURBS. LOCATION OF STEEL FRAMING SHALL BE COORDINATED WITH THE MECHANICAL CONTRACTOR.

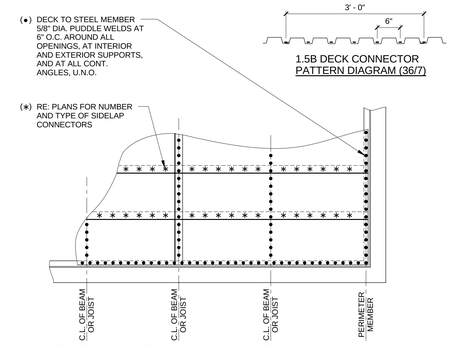
12 TYPICAL ROOF TOP UNIT AND DECK SUPPORT
3/4" = 1'-0"



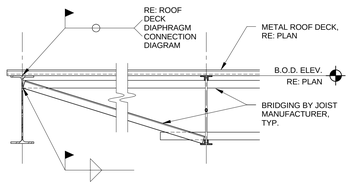
11 ROOF DRAIN FRAMING
3/4" = 1'-0"



10 CHORD ANGLE SPLICE DETAIL
1 1/2" = 1'-0"

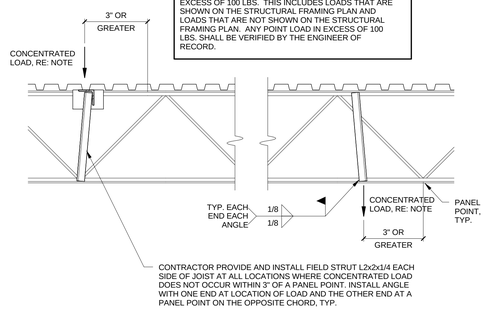


9 ROOF DECK DIAPHRAGM CONNECTION DIAGRAM
3/4" = 1'-0"

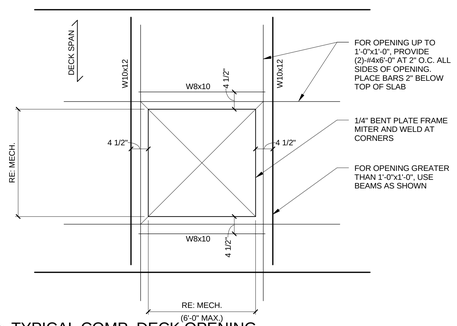


8 TYPICAL BRIDGING CONNECTION
3/4" = 1'-0"

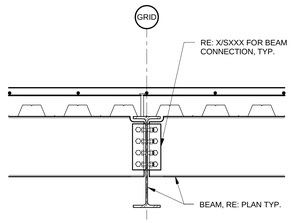
PROVIDE JOIST REINFORCING IN ACCORDANCE WITH THIS DETAIL FOR ALL CONCENTRATED LOADS IN EXCESS OF 100 LBS. THIS INCLUDES LOADS THAT ARE SHOWN ON THE STRUCTURAL FRAMING PLAN AND LOADS THAT ARE NOT SHOWN ON THE STRUCTURAL FRAMING PLAN. ANY POINT LOAD IN EXCESS OF 100 LBS. SHALL BE VERIFIED BY THE ENGINEER OF RECORD.



7 JOIST REINFORCING DETAIL
3/4" = 1'-0"

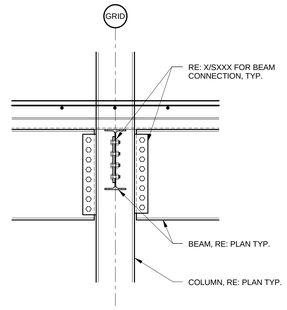


6 TYPICAL COMP. DECK OPENING
3/4" = 1'-0"



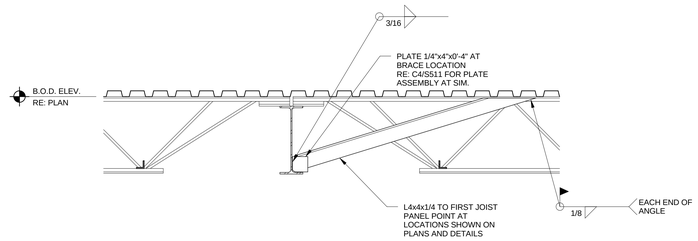
TYPICAL BEAM TO BEAM CONNECTION

5 TYP. BEAM TO BEAM CONNECTION
3/4" = 1'-0"

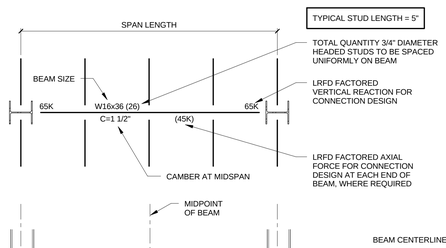


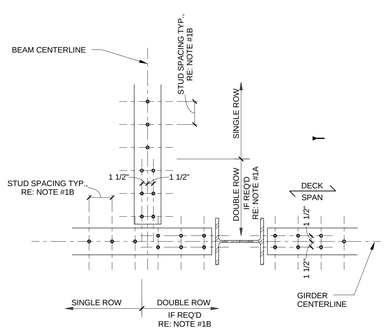
TYPICAL BEAM TO COLUMN CONNECTION

4 TYP. BEAM TO COLUMN CONNECTION
3/4" = 1'-0"



3 BRACE CONNECTION AT JOIST TO BEAM
3/4" = 1'-0"





COMPOSITE FRAMING NOTES
1. STUD PLACEMENT:
   A.) FOR BEAMS WITH DECK SPAN PERPENDICULAR TO THE BEAM.
   a.) REFER TO STUD LAYOUT AND POSITIONING DIAGRAMS AT LEFT.
   b.) LAYOUT STUDS ALONG BEAM LENGTH PRIOR TO STUD ATTACHMENT.
   c.) IF TOTAL QUANTITY OF STUDS EXCEEDS SPAN LENGTH IN FEET, LAYOUT A SINGLE ROW OF STUDS AT 12" O.C. AND LAYOUT REMAINING STUDS IN EQUAL QUANTITIES FROM EACH END OF BEAM AT 12" O.C. IN A DOUBLE ROW OF STUDS. CENTER DOUBLE STUD ROW ABOUT CENTERLINE OF BEAM. REFER TO DETAIL "C" FOR STUD PLACEMENT
   d.) IF TOTAL QUANTITY OF STUDS IS LESS THAN THE SPAN LENGTH IN FEET, PLACE STUDS AT 24" O.C. AND PLACE REMAINING STUDS IN EQUAL QUANTITIES FROM EACH END OF BEAM AT VACANT DECK FLUTE LOCATIONS AT 24" O.C. SUCH THAT THE STUD SPACING AT END IS 12" O.C.
   e.) IN ALL CASES, THERE SHALL BE AN EQUAL NUMBER OF SHEAR STUDS ON EACH SIDE OF THE MIDPOINT OF BEAM.
   B.) FOR BEAMS WITH DECK SPAN PARALLEL TO THE BEAM.
   a.) LAYOUT STUDS ALONG BEAM LENGTH PRIOR TO STUD ATTACHMENT.
   b.) SPACE STUDS IN A SINGLE ROW BETWEEN SUPPORTS AT A UNIFORM SPACING, SPACING SHALL NOT BE LESS THAN 4 1/2".
   c.) IF SPACING WOULD BE LESS THAN 4 1/2", LAYOUT A SINGLE ROW OF STUDS AT 4 1/2" ON CENTER AND LAYOUT THE REMAINING STUDS IN EQUAL QUANTITIES FROM EACH END OF THE BEAM IN A DOUBLE ROW OF STUDS. CENTER DOUBLE STUD ROW ABOUT CENTERLINE OF BEAM. REFER TO DETAIL "C" FOR STUD PLACEMENT.
2. STEEL FABRICATOR SHALL DESIGN BEAM CONNECTIONS TO COLUMNS OR TO BEAMS FOR THE REACTIONS SHOWN ON THE PLANS. REACTIONS INDICATED ARE FACTORED FOR USE WITH LOAD AND RESISTANCE FACTOR DESIGN (LRFD) METHOD. IF NO REACTION IS SHOWN ON PLAN, DESIGN FOR THE LRFD FACTORED REACTION SHOWN IN THE STEEL BEAM MINIMUM CONNECTION SCHEDULE.
3. INDICATES FIELD BOLTED MOMENT CONNECTION.
4. IF SPECIFIED CAMBER IS NOT MET IN THE ERECTED STEEL, CONTRACTOR SHALL PROVIDE SHORING FOR STEEL FLOOR BEAMS DURING FLOOR SLAB POURS.

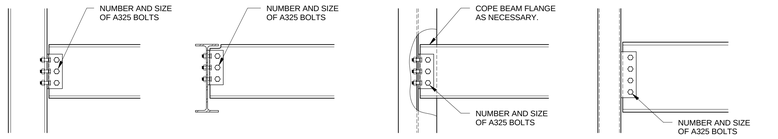
2 COMPOSITE BEAM DIAGRAMS AND COMPOSITE FRAMING NOTES
3/4" = 1'-0"

STEEL BEAM MINIMUM CONNECTION SCHEDULE

BEAM	MINIMUM BOLT DIA.	MINIMUM NUMBER OF BOLTS	MINIMUM LRFD FACTORED END REACTION (KIPS)	NOTE
W8	3/4"	2	24	1,2,3,4
W10	3/4"	2	24	1,2,3,4
W12	3/4"	3	38	1,2,3,4
W14	3/4"	3	38	1,3,4
W16	3/4"	4	52	1,3,4
W18	3/4"	4	52	1,3,4
W21	3/4"	5	64	1,3,4
W24	3/4"	5	64	1,3,4
W27	3/4"	7	88	1,3,4
W30	3/4"	7	88	1,3,4
W33	3/4"	8	101	1,3,4

THE STEEL FABRICATOR SHALL BE RESPONSIBLE FOR DESIGN AND ADEQUACY OF ALL CONNECTIONS THAT ARE NOT FULLY DETAILED ON THE CONTRACT DOCUMENTS. RE: PLANS AND SCHEDULE FOR LRFD FACTORED LOADS, AND RE: STEEL BEAM MINIMUM CONNECTION SCHEDULE FOR MINIMUM CONNECTION REQUIRED.

BEAM MINIMUM CONNECTION SCHEDULE NOTES
1. UNLESS INDICATED ON FRAMING PLAN AND CORRESPONDING DETAILS OR NOTES BELOW, SCHEDULE INDICATES THE MINIMUM NUMBER OF BOLTS ALLOWED FOR BEAM CONNECTIONS.
2. MINIMUM BOLT SCHEDULE APPLIES TO CHANNEL SECTIONS OF SAME NOMINAL DEPTHS, U.N.O.
3. CONNECTIONS SHALL BE DESIGNED FOR LRFD FACTORED END REACTIONS SHOWN ON PLANS. IF NO REACTION IS SHOWN ON PLAN, DESIGN FOR REACTION IN SCHEDULE.
4. RE: PLAN FOR REQUIRED MOMENT CONNECTION.



1 STEEL BEAM MINIMUM CONNECTION SCHEDULE AND DETAILS
3/4" = 1'-0"

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SEPT. 6, 1839

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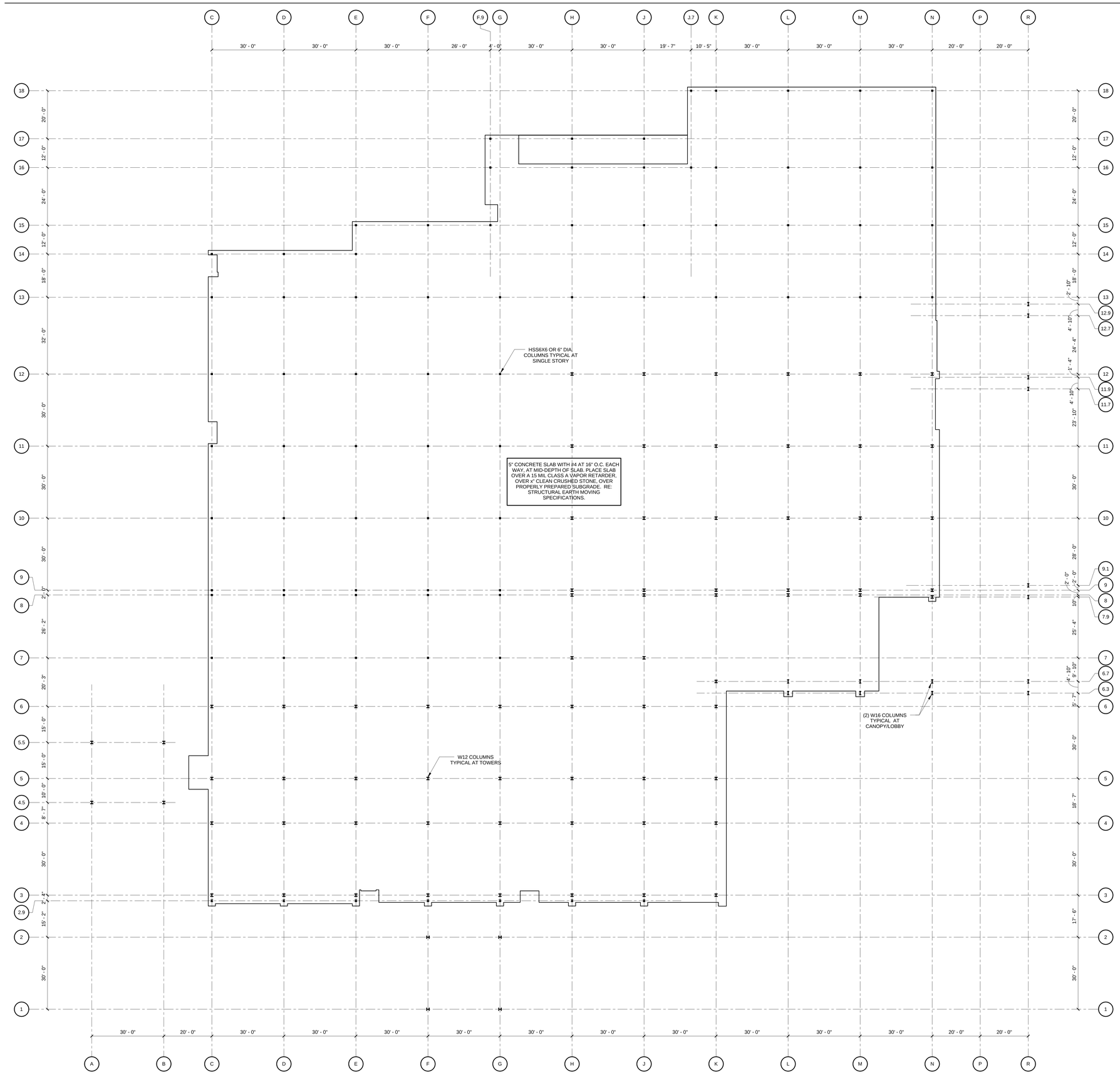
TAHLEQUAH, OKLAHOMA

PROJECT PHASE:
100% SCHEMATIC DESIGN SUBMITTAL

REVISIONS

#	DATE	DESCRIPTION

DATE: 03-21-14
JOB NUMBER: 13-13
SHEET NUMBER: S0.3
TYPICAL DETAILS



1 FOUNDATION PLAN
1/16" = 1'-0"

SCHEMATIC DESIGN NOTES:

PIER AND GRADE BEAMS:

THE BUILDING EXTERIOR WALL WILL BE SUPPORTED BY A GRADE BEAM. THE GRADE BEAM WILL BE SUPPORTED BY CONCRETE DRILLED PIERS. INDIVIDUAL CONCRETE DRILLED PIERS WILL SUPPORT THE INTERIOR BUILDING COLUMNS. IT IS ANTICIPATED THAT THE DRILLED PIERS MAY HAVE TO BE CASED TO PENETRATE THROUGH THE SUBSURFACE LAYERS WITH GRAVEL AND COBBLE. THE DESIGN PARAMETERS ARE STILL TO BE DETERMINED PENDING THE FINAL GEOTECHNICAL ENGINEERING REPORT. GRADE BEAMS WILL EXTEND A MINIMUM OF 24" BELOW EXTERIOR GRADE.

SLABS-ON-GRADE:

INTERIOR BUILDING SLABS WILL BE 5" THICK REINFORCED WITH STANDARD REBAR. SLABS WILL BE PLACED OVER A 4 INCH LAYER OF FREE DRAINING GRANULAR MATERIAL OVER A 15 MIL POLYETHYLENE VAPOR BARRIER OVER PROPERLY PREPARED SUBGRADE. ANY REQUIRED LOW PLASTICITY SUBGRADE MATERIAL IS PENDING THE FINAL GEOTECHNICAL REPORT. THE LOW PLASTICITY MATERIAL MAY EITHER BE IMPORTED MATERIAL OR CHEMICALLY STABILIZED ON SITE SOILS. THE CONCRETE SLAB WILL HAVE CONTROL JOINTS OR DOWELED CONSTRUCTION JOINTS SPACED AT A MAXIMUM OF 15 FEET ON CENTER FOR SHRINKAGE CRACK CONTROL. THE CONCRETE STRENGTH USED FOR FLOOR SLABS ON GRADE WILL BE 3,000 POUNDS PER SQUARE INCH AT 28 DAYS.

FOUNDATION PLAN NOTES

1. REFERENCE ARCHITECTURAL PLANS FOR DIMENSIONS NOT SHOWN. COORDINATE SLAB ELEVATIONS AND SLOPES WITH ARCHITECTURAL PLANS.
2. REFERENCE MECHANICAL AND ARCHITECTURAL DRAWINGS FOR SIZE AND LOCATION OF SLAB PENETRATIONS.
3. REFERENCE S0.2 FOR PIER (P), GRADE BEAM (GB), PILASTER (Z) AND COLUMN (C) SCHEDULES.
4. SLAB AND WALL BLOCKOUTS AT COLUMNS AND BRACED FRAMES SHALL BE SIZED AS REQUIRED TO PROPERLY INSTALL AND CONNECT COLUMNS AND DIAGONAL BRACES.
5. AT STAIR STRINGERS CONNECT EACH STRINGER TO A 12" DEEP x 2'-0" WIDE THICKENED SLAB-ON-GRADE WITH (2) - #4 CONTINUOUS. USE L4x4x1/4x0'-9" AT EACH STRINGER WITH (2)-1/2" DIA. HILTI HAS THREADED RODS WITH HILTI HIT-RE 500 SD EPOXY ADHESIVE, 4 1/2" EMBEDMENT.
6. THICKEN SLAB AT FLOOR BOXES AND CONDUIT TO MAINTAIN A MINIMUM X" SLAB THICKNESS. RE: ELECTRICAL FOR LOCATION OF PADS
7. HOUSKEEPING PADS SHALL BE 4" N.W. CONCRETE PAD PLACED ON SLAB. REINFORCE PAD WITH #3 AT 12" O.C., E.W. RE: MECH. FOR SIZE AND LOCATION

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PROJECT PHASE:

100% SCHEMATIC DESIGN SUBMITTAL

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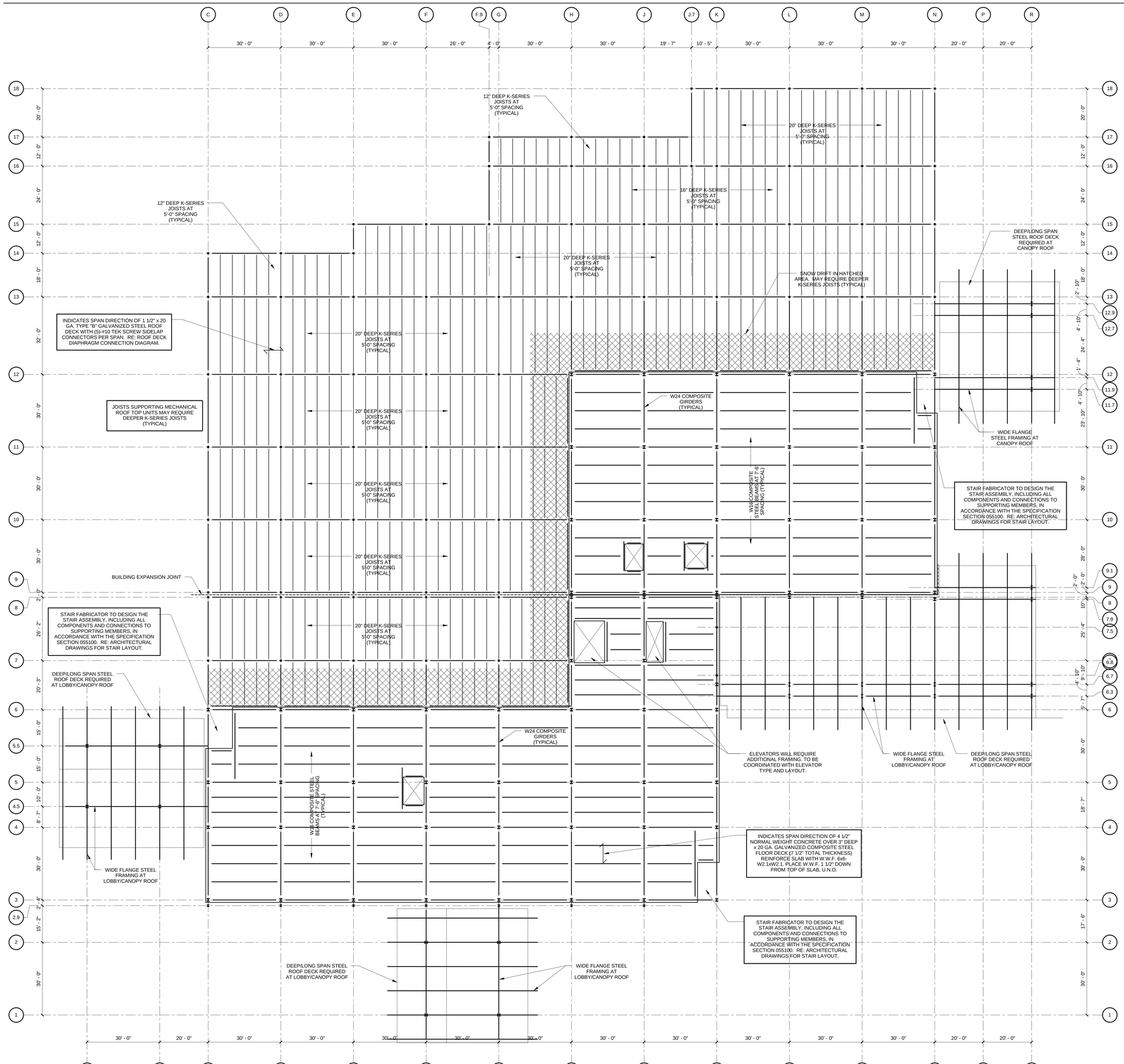
03-21-14	13-13

SHEET NUMBER:

S1.0

FOUNDATION PLAN



1 SECOND FLOOR AND LOW ROOF FRAMING PLAN
1/16" = 1'-0"

SCHEMATIC DESIGN NOTES:

FLOOR FRAMING:

THE BUILDING FLOORS ARE A 3" COMPOSITE DECK WITH 4 ½" NORMAL WEIGHT CONCRETE SUPPORTED BY COMPOSITE STEEL BEAMS. STEEL WIDE FLANGE COLUMNS SUPPORT THE BEAMS. THE TYPICAL FLOORS WILL BE DESIGNED FOR A SUPERIMPOSED LIVE LOAD OF 80 POUNDS PER SQUARE FOOT, WHICH WILL MEET OR EXCEED THE CODE MINIMUM REQUIREMENTS FOR THE PROPOSED USES. IMPACT AND CYCLIC LOADING WILL BE CONSIDERED FOR VIBRATION CONTROL. THE MECHANICAL FLOOR WILL BE DESIGNED FOR A SUPERIMPOSED LIVE LOAD OF 100 POUNDS PER SQUARE FOOT WITH ADDITIONAL CONCENTRATED LOADS AT ALL EQUIPMENT LOCATIONS.

ROOF FRAMING.

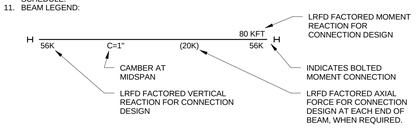
THE MULTISTORY BUILDING ROOD IS 1 ½" STEEL DECK (TYPE B) SUPPORTED BY STEEL BEAMS THAT CANTILEVER BEYOND THE EXTERIOR BUILDING WALLS TO SUPPORT THE ROOF OVER HANG. STEEL WIDE FLANGE COLUMNS SUPPORT THE BEAMS. THE SINGLE STORY BUILDING ROOF IS 1 1/2" STEEL DECK (TYPE B) SUPPORTED BY STEEL JOISTS AND BEAMS. STEEL PIPE OR TUBE COLUMNS SUPPORT THE BEAMS. THE MAIN LOBBY AND CANOPY ROOF IS STEEL DECK SUPPORTED BY STEEL WIDE FLANGE BEAMS. DOUBLE WIDE FLANGE COLUMNS SUPPORT THE STEEL BEAMS.

LATERAL BRACING:

THE LATERAL BRACING FOR THE STRUCTURE WILL BE ACCOMPLISHED USING THE STEEL ROOF DECK AS A DIAPHRAGM. THE DECK WILL TRANSFER THE LATERAL LOADS TO STEEL BRACES THAT WILL TRANSFER THE LOADS TO THE FOUNDATIONS.

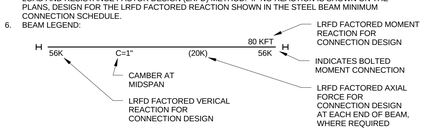
ROOF FRAMING PLAN NOTES

1. "B.O.D." INDICATES BOTTOM OF DECK ELEVATION.
2. ATTACH ROOF DECK TO SUPPORTS PER THE ROOF DECK DIAPHRAGM CONNECTION DIAGRAM.
3. REFERENCE MECHANICAL AND ARCHITECTURAL DRAWINGS FOR SIZE AND LOCATION OF DECK PENETRATIONS.
4. NO HANGING LOADS SHALL BE APPLIED TO ROOF DECK.
5. ✳ INDICATES CONCENTRATED LOAD AT STEEL JOIST BOTTOM CHORD OR TOP CHORD. LOAD MAY OCCUR UP TO 1'-0" AWAY FROM LOCATION SHOWN. LOADS SHOWN ARE SERVICE LEVEL VALUES WHICH SHALL BE FACTORED PER THE INTERNATIONAL BUILDING CODE FOR USE WITH THE LOAD AND RESISTANCE FACTOR DESIGN (LRFD) METHOD. REFERENCE THE TYPICAL ROOF TOP UNIT AND DECK SUPPORT DETAIL. REFERNCE MECHANICAL FOR EXACT UNIT LOCATIONS.
6. REFERENCE x/Sxxx FOR JOIST REINFORCING DETAIL AT ALL LOCATIONS WHERE CONCENTRATED LOADS DO NOT OCCUR AT JOIST PANEL POINTS.
7. STEEL JOIST MANUFACTURER SHALL COORDINATE MECHANICAL DUCT LOCATIONS TO AVOID CONFLICT WITH BRIDGING.
8. --- INDICATES KICKER, RE: x/Sxxx
9. REFERENCE x/Sxxx FOR ROOF DRAIN SUPPORT FRAMING. REFERENCE MEP AND ARCHITECTURAL FOR ROOF DRAIN LOCATIONS.
10. STEEL FABRICATOR SHALL DESIGN BEAM CONNECTIONS TO COLUMNS OR TO BEAMS FOR THE TOTAL REACTIONS SHOWN ON THE PLANS. REACTIONS INDICATED ARE FACTORED FOR USE WITH LOAD AND RESISTANCE FACTOR DESIGN (LRFD) METHOD. IF NO REACTION IS SHOWN ON THE PLANS, DESIGN FOR THE LRFD FACTORED REACTION SHOWN IN THE STEEL BEAM MINIMUM CONNECTION SCHEDULE.
11. BEAM LEGEND:



FLOOR FRAMING PLAN NOTES

1. REFERENCE ARCHITECTURAL PLANS FOR DIMENSIONS NOT SHOWN. COORDINATE SLAB ELEVATIONS AND SLOPES WITH ARCHITECTURAL PLANS.
2. REFERENCE MECHANICAL AND ARCHITECTURAL DRAWINGS FOR SIZE AND LOCATION OF SLAB PENETRATIONS.
3. FLOOR BEAMS, METAL DECK AND SLABS ARE DESIGNED AS AN UNSHORED (U.N.O.) COMPOSITE FLOOR SYSTEM REQUIRING SHEAR CONNECTORS (HEADED STUDS) WELDED THROUGH THE METAL DECK DIRECTLY TO THE TOP FLANGES OF BEAMS AND GIRDERS. REFERENCE DETAIL x/Sxxx FOR STUD REQUIREMENTS AND LAYOUT. DO NOT PAINT SURFACES WHICH RECEIVE HEADED STUDS.
4. AT ALL EDGES OF OPENINGS WHERE EDGE ANGLE IS NOT NOTED, A MINIMUM OF 3/16" BENT PLATE x x" VERTICAL x AS REQUIRED TO CENTERLINE OF BEAM SHALL BE INSTALLED. WELD PLATE TO TOP FLANGE WITH 3/16"x2" FILLET WELDS AT 12" O.C., (3)-WELDS MINIMUM.
5. STEEL FABRICATOR SHALL DESIGN BEAM CONNECTIONS TO COLUMNS OR TO BEAMS FOR THE TOTAL REACTIONS SHOWN ON THE PLANS. REACTIONS INDICATED ARE FACTORED FOR USE WITH LOAD AND RESISTANCE FACTOR DESIGN (LRFD) METHOD. IF NO REACTION IS SHOWN ON THE PLANS, DESIGN FOR THE LRFD FACTORED REACTION SHOWN IN THE STEEL BEAM MINIMUM CONNECTION SCHEDULE.
6. BEAM LEGEND:



7. PLACING CONDUIT IN SLAB ON METAL DECK IS NOT PERMITTED.

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CLIENT:

THE CHEROKEE NATION
SEPT. 6, 1839

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TAHLEQUAH, OKLAHOMA

PROJECT PHASE:

100% SCHEMATIC DESIGN SUBMITTAL

REVISIONS

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DATE: 03-21-14

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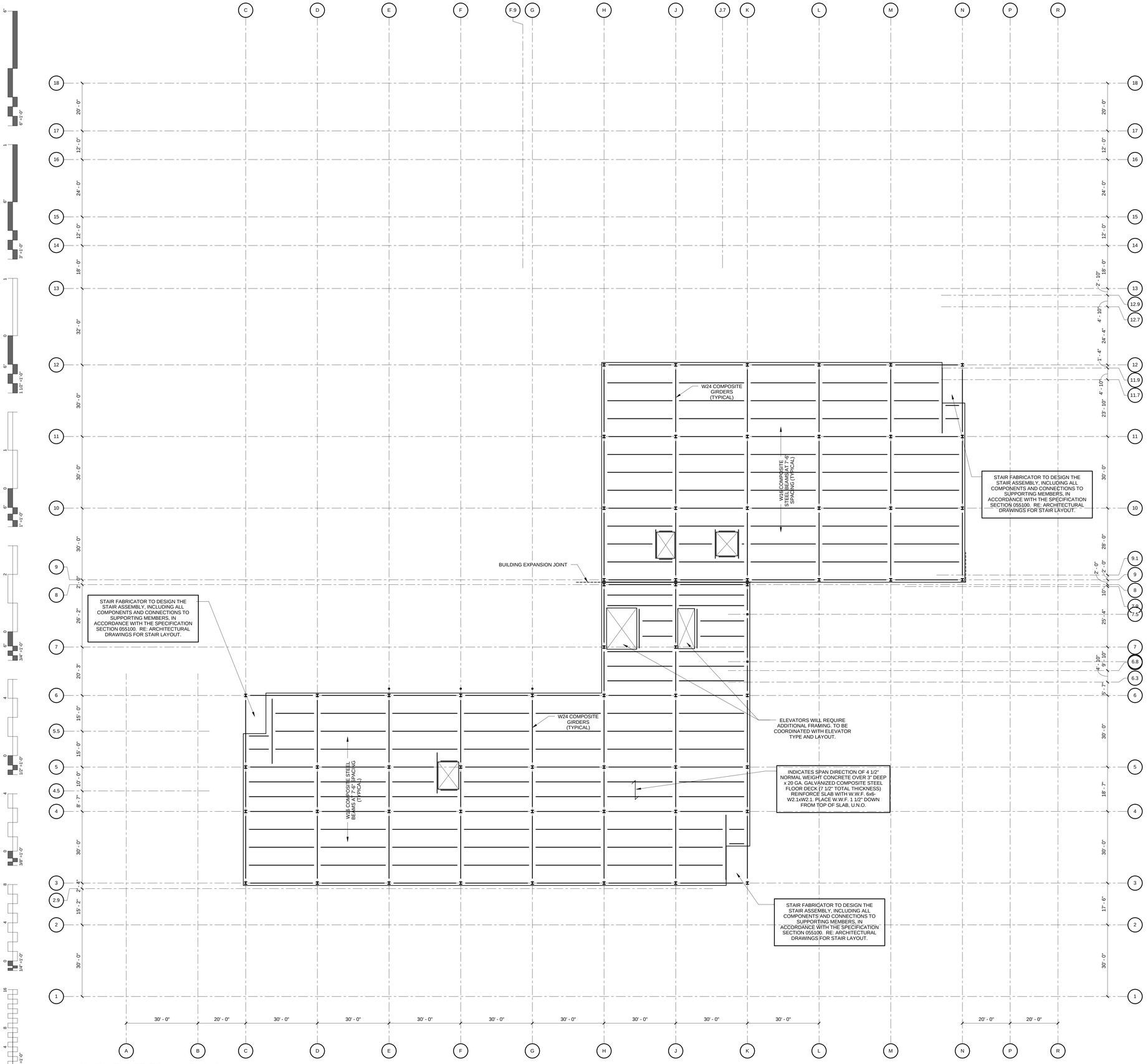
SHEET NUMBER: S1.1

SECOND FLOOR AND LOW ROOF FRAMING PLAN



1 THIRD FLOOR FRAMING PLAN
1/16" = 1'-0"

SCHEMATIC DESIGN NOTES:

FLOOR FRAMING:

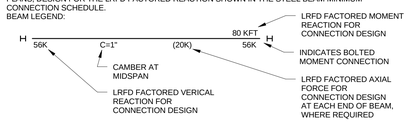
THE BUILDING FLOORS ARE A 3" COMPOSITE DECK WITH 4 ½" NORMAL WEIGHT CONCRETE SUPPORTED BY COMPOSITE STEEL BEAMS. STEEL WIDE FLANGE COLUMNS SUPPORT THE BEAMS. THE TYPICAL FLOORS WILL BE DESIGNED FOR A SUPERIMPOSED LIVE LOAD OF 80 POUNDS PER SQUARE FOOT, WHICH WILL MEET OR EXCEED THE CODE MINIMUM REQUIREMENTS FOR THE PROPOSED USES. IMPACT AND CYCLIC LOADING WILL BE CONSIDERED FOR VIBRATION CONTROL. THE MECHANICAL FLOOR WILL BE DESIGNED FOR A SUPERIMPOSED LIVE LOAD OF 100 POUNDS PER SQUARE FOOT WITH ADDITIONAL CONCENTRATED LOADS AT ALL EQUIPMENT LOCATIONS.

LATERAL BRACING:

THE LATERAL BRACING FOR THE STRUCTURE WILL BE ACCOMPLISHED USING THE STEEL ROOF DECK AS A DIAPHRAGM. THE DECK WILL TRANSFER THE LATERAL LOADS TO STEEL BRACES THAT WILL TRANSFER THE LOADS TO THE FOUNDATIONS.

FLOOR FRAMING PLAN NOTES

1. REFERENCE ARCHITECTURAL PLANS FOR DIMENSIONS NOT SHOWN. COORDINATE SLAB ELEVATIONS AND SLOPES WITH ARCHITECTURAL PLANS.
2. REFERENCE MECHANICAL AND ARCHITECTURAL DRAWINGS FOR SIZE AND LOCATION OF SLAB PENETRATIONS.
3. FLOOR BEAMS, METAL DECK AND SLABS ARE DESIGNED AS AN UNSHORED (U.N.O.) COMPOSITE FLOOR SYSTEM REQUIRING SHEAR CONNECTORS (HEADED STUDS) WELDED THROUGH THE METAL DECK DIRECTLY TO THE TOP FLANGES OF BEAMS AND GIRDERS. REFERENCE DETAIL x/Sxxx FOR STUD REQUIREMENTS AND LAYOUT. DO NOT PAINT SURFACES WHICH RECEIVE HEADED STUDS.
4. AT ALL EDGES OF OPENINGS WHERE EDGE ANGLE IS NOT NOTED, A MINIMUM OF 3/16" BENT PLATE x x" VERTICAL x AS REQUIRED TO CENTERLINE OF BEAM SHALL BE INSTALLED. WELD PLATE TO TOP FLANGE WITH 3/16"x2" FILLET WELDS AT 12" O.C., (3)-WELDS MINIMUM.
5. STEEL FABRICATOR SHALL DESIGN BEAM CONNECTIONS TO COLUMNS OR TO BEAMS FOR THE TOTAL REACTIONS SHOWN ON THE PLANS. REACTIONS INDICATED ARE FACTORED FOR USE WITH LOAD AND RESISTANCE FACTOR DESIGN (LRFD) METHOD. IF NO REACTION IS SHOWN ON THE PLANS, DESIGN FOR THE LRFD FACTORED REACTION SHOWN IN THE STEEL BEAM MINIMUM CONNECTION SCHEDULE.
6. BEAM LEGEND:



7. PLACING CONDUIT IN SLAB ON METAL DECK IS NOT PERMITTED.

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SEAL OF THE CHEROKEE NATION
Sept. 6, 1839

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PROJECT PHASE:

100% SCHEMATIC DESIGN SUBMITTAL

REVISIONS

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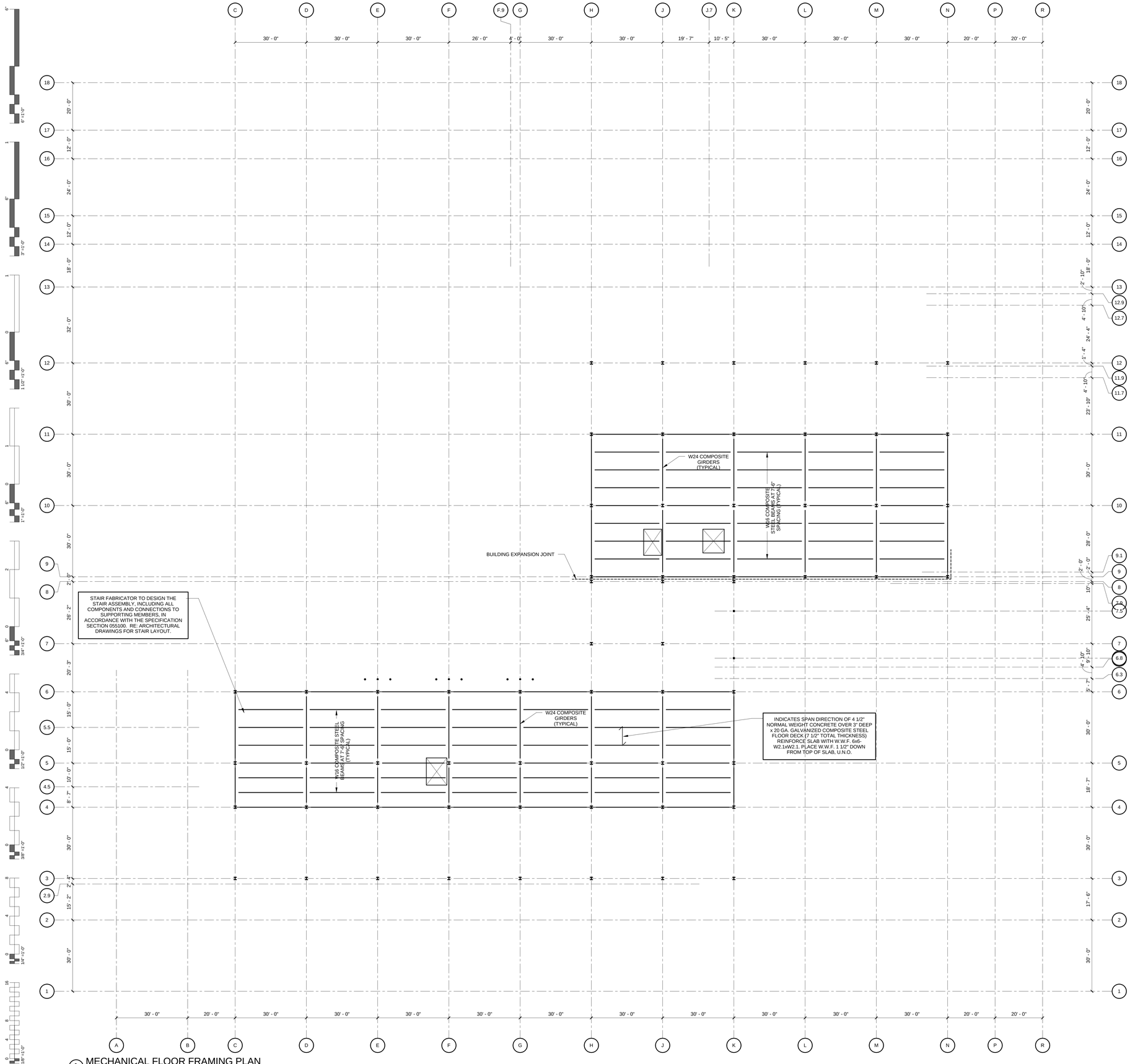
DATE: 03-21-14

JOB NUMBER: 13-13

SHEET NUMBER:

S1.2

THIRD FLOOR FRAMING PLAN



1 MECHANICAL FLOOR FRAMING PLAN
1/16" = 1'-0"

SCHEMATIC DESIGN NOTES:

FLOOR FRAMING:

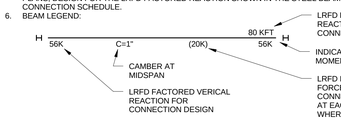
THE BUILDING FLOORS ARE A 3" COMPOSITE DECK WITH 4 ½" NORMAL WEIGHT CONCRETE SUPPORTED BY COMPOSITE STEEL BEAMS. STEEL WIDE FLANGE COLUMNS SUPPORT THE BEAMS. THE TYPICAL FLOORS WILL BE DESIGNED FOR A SUPERIMPOSED LIVE LOAD OF 80 POUNDS PER SQUARE FOOT, WHICH WILL MEET OR EXCEED THE CODE MINIMUM REQUIREMENTS FOR THE PROPOSED USES. IMPACT AND CYCLIC LOADING WILL BE CONSIDERED FOR VIBRATION CONTROL. THE MECHANICAL FLOOR WILL BE DESIGNED FOR A SUPERIMPOSED LIVE LOAD OF 100 POUNDS PER SQUARE FOOT WITH ADDITIONAL CONCENTRATED LOADS AT ALL EQUIPMENT LOCATIONS.

LATERAL BRACING:

THE LATERAL BRACING FOR THE STRUCTURE WILL BE ACCOMPLISHED USING THE STEEL ROOF DECK AS A DIAPHRAGM. THE DECK WILL TRANSFER THE LATERAL LOADS TO STEEL BRACES THAT WILL TRANSFER THE LOADS TO THE FOUNDATIONS.

FLOOR FRAMING PLAN NOTES

1. REFERENCE ARCHITECTURAL PLANS FOR DIMENSIONS NOT SHOWN. COORDINATE SLAB ELEVATIONS AND SLOPES WITH ARCHITECTURAL PLANS.
2. REFERENCE MECHANICAL AND ARCHITECTURAL DRAWINGS FOR SIZE AND LOCATION OF SLAB PENETRATIONS.
3. FLOOR BEAMS, METAL DECK AND SLABS ARE DESIGNED AS AN UNSHORED (U.N.O.) COMPOSITE FLOOR SYSTEM REQUIRING SHEAR CONNECTORS (HEADED STUDS) WELDED THROUGH THE METAL DECK DIRECTLY TO THE TOP FLANGES OF BEAMS AND GIRDERS. REFERENCE DETAIL x/Sxxx FOR STUD REQUIREMENTS AND LAYOUT. DO NOT PAINT SURFACES WHICH RECEIVE HEADED STUDS.
4. AT ALL EDGES OF OPENINGS WHERE EDGE ANGLE IS NOT NOTED, A MINIMUM OF 3/16" BENT PLATE x "-VERTICAL + AS REQUIRED TO CENTERLINE OF BEAM SHALL BE INSTALLED. WELD PLATE TO TOP FLANGE WITH 3/16"x2" FILLET WELDS AT 12" O.C.. (3)-WELDS MINIMUM.
5. STEEL FABRICATOR SHALL DESIGN BEAM CONNECTIONS TO COLUMNS OR TO BEAMS FOR THE TOTAL REACTIONS SHOWN ON THE PLANS. REACTIONS INDICATED ARE FACTORED FOR USE WITH LOAD AND RESISTANCE FACTOR DESIGN (LRFD) METHOD. IF NO REACTION IS SHOWN ON THE PLANS, DESIGN FOR THE LRFD FACTORED REACTION SHOWN IN THE STEEL BEAM MINIMUM CONNECTION SCHEDULE.
6. BEAM LEGEND:



7. PLACING CONDUIT IN SLAB ON METAL DECK IS NOT PERMITTED.

RedFern Group

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PROJECT PHASE:

100% SCHEMATIC DESIGN SUBMITTAL

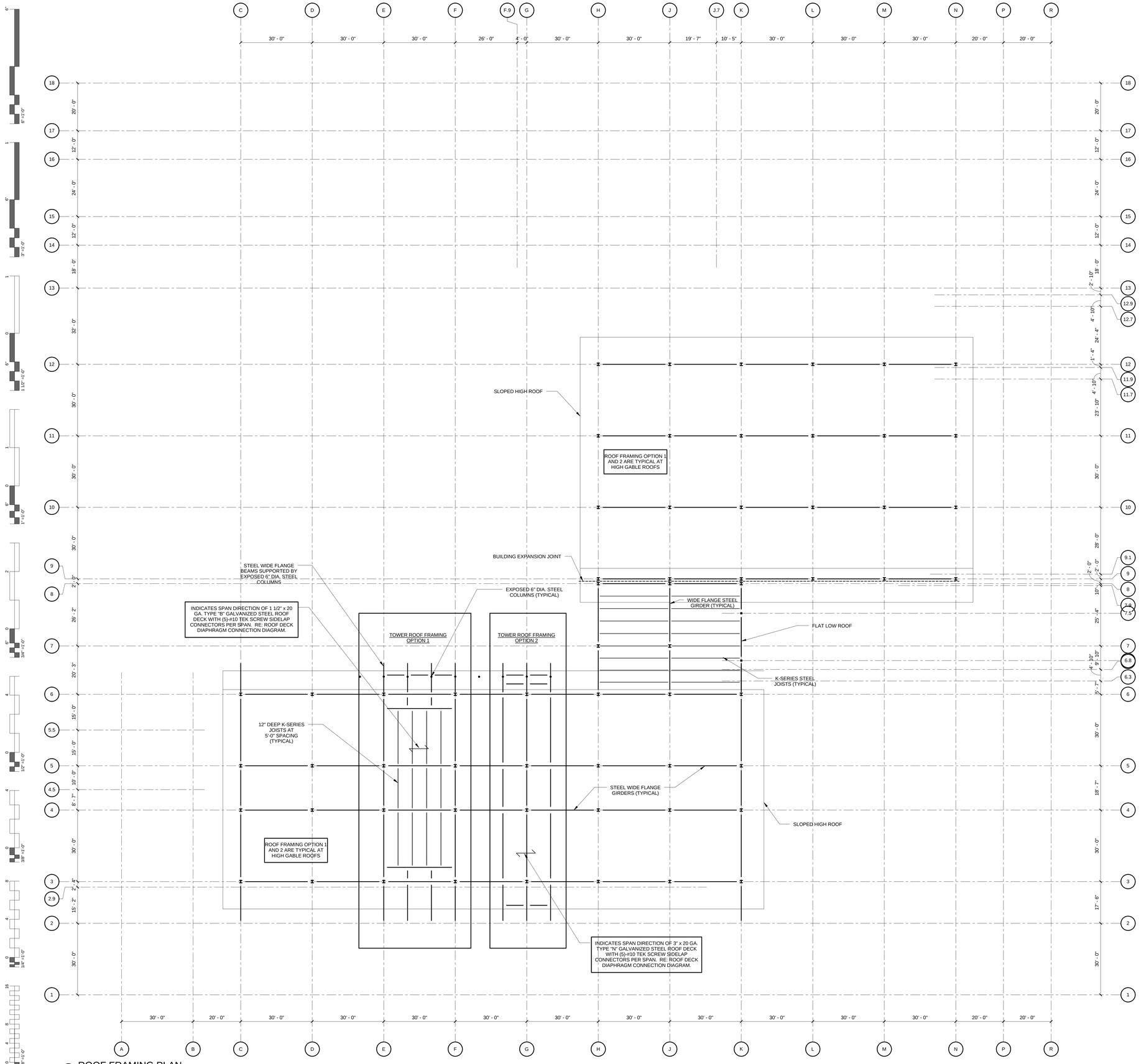
REVISIONS		
#	DATE	DESCRIPTION

DATE:	JOB NUMBER:
03-21-14	13-13

SHEET NUMBER:

S1.3

MECH. FLOOR FRAMING PLAN



1 ROOF FRAMING PLAN
1/16" = 1'-0"

SCHEMATIC DESIGN NOTES:

ROOF FRAMING.

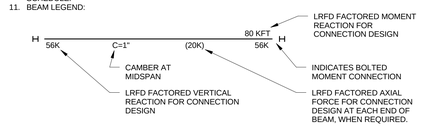
THE MULTISTORY BUILDING ROOD IS 1 ½" STEEL DECK (TYPE B) SUPPORTED BY STEEL BEAMS THAT CANTILEVER BEYOND THE EXTERIOR BUILDING WALLS TO SUPPORT THE ROOF OVER HANG. STEEL WIDE FLANGE COLUMNS SUPPORT THE BEAMS. THE SINGLE STORY BUILDING ROOF IS 1 1/2" STEEL DECK (TYPE B) SUPPORTED BY STEEL JOISTS AND BEAMS. STEEL PIPE OR TUBE COLUMNS SUPPORT THE BEAMS. THE MAIN LOBBY AND CANOPY ROOF IS STEEL DECK SUPPORTED BY STEEL WIDE FLANGE BEAMS. DOUBLE WIDE FLANGE COLUMNS SUPPORT THE STEEL BEAMS.

LATERAL BRACING:

THE LATERAL BRACING FOR THE STRUCTURE WILL BE ACCOMPLISHED USING THE STEEL ROOF DECK AS A DIAPHRAGM. THE DECK WILL TRANSFER THE LATERAL LOADS TO STEEL BRACES THAT WILL TRANSFER THE LOADS TO THE FOUNDATIONS.

ROOF FRAMING PLAN NOTES

1. "B.O.D." INDICATES BOTTOM OF DECK ELEVATION.
2. ATTACH ROOF DECK TO SUPPORTS PER THE ROOF DECK DIAPHRAGM CONNECTION DIAGRAM.
3. REFERENCE MECHANICAL AND ARCHITECTURAL DRAWINGS FOR SIZE AND LOCATION OF DECK PENETRATIONS.
4. NO HANGING LOADS SHALL BE APPLIED TO ROOF DECK.
5. ✳ INDICATES CONCENTRATED LOAD AT STEEL JOIST BOTTOM CHORD OR TOP CHORD. LOAD MAY OCCUR UP TO 1'-0" AWAY FROM LOCATION SHOWN. LOADS SHOWN ARE SERVICE LEVEL VALUES WHICH SHALL BE FACTORED PER THE INTERNATIONAL BUILDING CODE FOR USE WITH THE LOAD AND RESISTANCE FACTOR DESIGN (LRFD) METHOD. REFERENCE THE TYPICAL ROOF TOP UNIT AND DECK SUPPORT DETAIL. REFERNCE MECHANICAL FOR EXACT UNIT LOCATIONS.
6. REFERENCE x/Sxxx FOR JOIST REINFORCING DETAIL AT ALL LOCATIONS WHERE CONCENTRATED LOADS DO NOT OCCUR AT JOIST PANEL POINTS.
7. STEEL JOIST MANUFACTURER SHALL COORDINATE MECHANICAL DUCT LOCATIONS TO AVOID CONFLICT WITH BRIDGING.
8. --- INDICATES KICKER, RE: x/Sxxx
9. REFERENCE x/Sxxx FOR ROOF DRAIN SUPPORT FRAMING. REFERENCE MEP AND ARCHITECTURAL FOR ROOF DRAIN LOCATIONS.
10. STEEL FABRICATOR SHALL DESIGN BEAM CONNECTIONS TO COLUMNS OR TO BEAMS FOR THE TOTAL REACTIONS SHOWN ON THE PLANS. REACTIONS INDICATED ARE FACTORED FOR USE WITH LOAD AND RESISTANCE FACTOR DESIGN (LRFD) METHOD. IF NO REACTION IS SHOWN ON THE PLANS, DESIGN FOR THE LRFD FACTORED REACTION SHOWN IN THE STEEL BEAM MINIMUM CONNECTION SCHEDULE.
11. BEAM LEGEND:



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PROJECT PHASE:

100% SCHEMATIC DESIGN SUBMITTAL

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S1.4

ROOF FRAMING PLAN