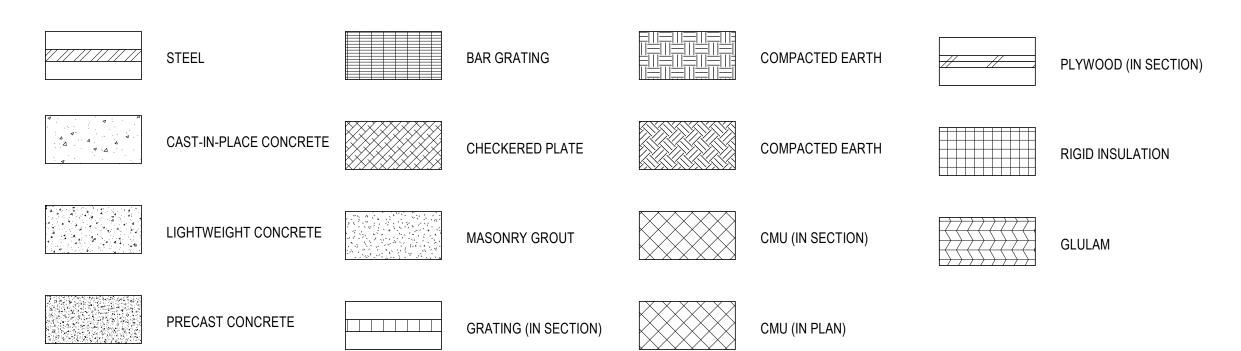
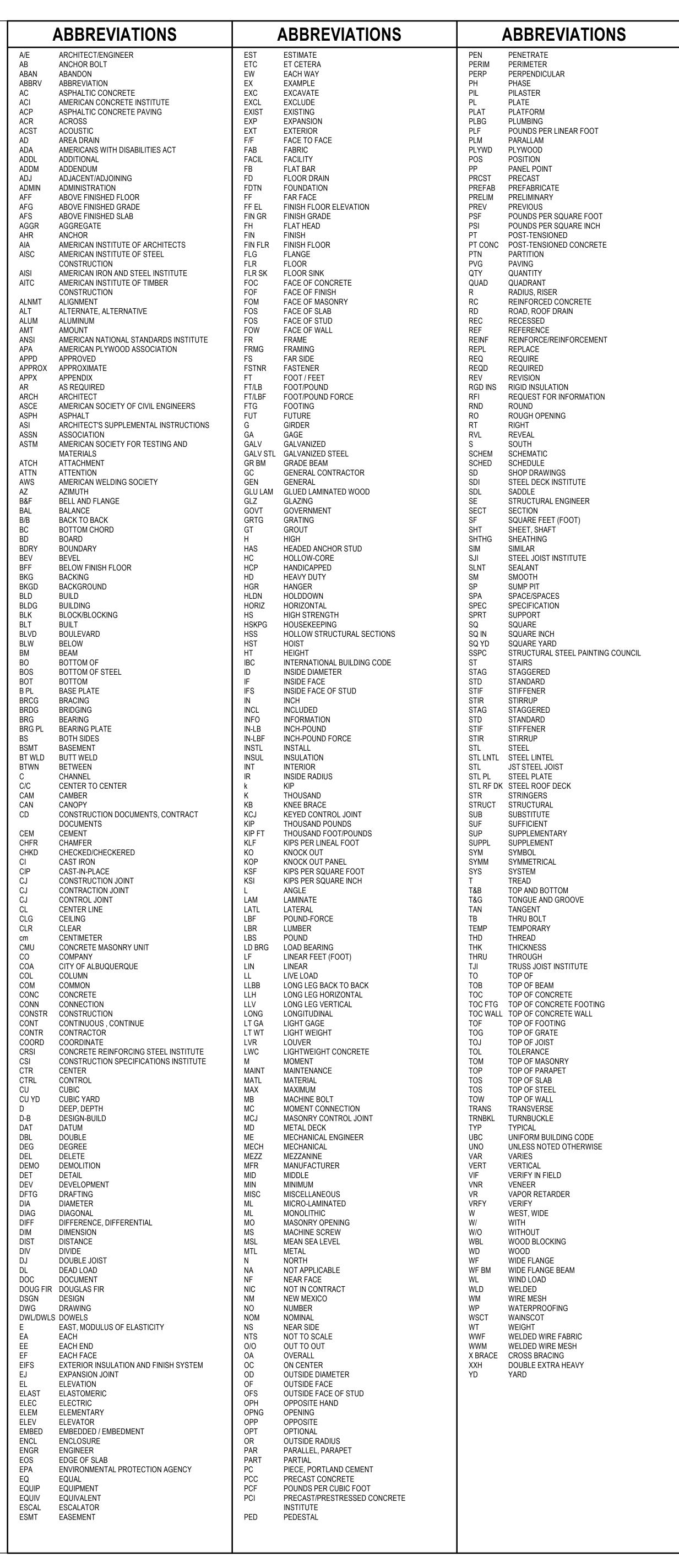
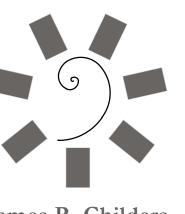


STRUCTURAL MATERIALS LEGEND







James R. Childers Architect, Inc.

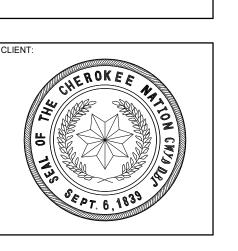
45 South 4th Street Fort Smith, AR 72901 479-783-2480

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PROFESSIONAL SEAL:

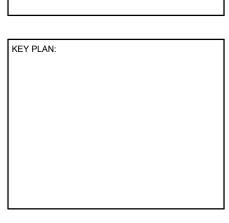
NOTE: THIS STRUCTURAL
PACKAGE MAY BE USED FOR
PROCUREMENT OF STRUCTURAL
STEEL SHAPES. ANY CHANGES TO
THE PROJECT, INCLUDING, BUT
NOT LIMITED TO: LOADING
REQUIREMENTS, GEOMETRY
CHANGES IN PLAN OR ELEVATION,
SPACE USAGE REVISIONS, OR
VALUE ENGINEERING MAY AFFECT
THE STRUCTURAL STEEL MEMBER
REQUIREMENTS OF THE ALREADY
PROCURED STEEL.





Steopathic Medicin
The Cherokee Nation
HEQUAH, OKLAHOMA





PROJECT PHASE:

BULK STEEL PACKAGE
NOT FOR CONSTRUCTION

DATE DESCRIPTION

DATE: JOB NUMBER: 17-13

SHEET NUMBER:

ABBREVIATIONS AND LEGEND

ACTIVE EARTH PRESSURE FOR ON SITE SOIL BACKFILL 35 PCF UNRESTRAINED PASSIVE EARTH PRESSURE 230 PCF ¬EARTH PRESSURE AT REST 50 PCF UNRESTRAINED USOIL FRICTION FACTOR 0.26 SOIL BEARING CAPACITY 3500 PSF

GENERAL STRUCTURAL NOTES GENERAL STRUCTURAL NOTES

100 PSF

300 LBS

100 PSF

100 PSF

150 PSF

150 PSF

150 PSF

15 PSF

10 PSF

2000 LBS

R1 = 1.0

R2 = 1.0

PG = 10 **PSF**

PF = 16 PSF

GCPI = 0.18

IS = 1.25

SS = 0.152G

S1 = 0.081G

SDS = 0.122G

SD1 = 0.092G

CS = 0.043

24 INCHES

NONE

SPREAD FOOTINGS: 4000 PSF CONTINUOUS FOOTINGS: 3500 PSF

R = 3V = 0.043W

STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE

CE = 0.9

IS = 1.10

CT = 1.0

STAIRS AND EXIT-WAYS*

LIBRARY/ BOOKSTORE

SUSPENDED EQUIPMENT

CONCENTRATED LOAD

GROUND SNOW LOAD FLAT ROOF SNOW LOAD**

THERMAL FACTOR

RISK CATEGORY

SHORT PERIOD

SHORT PERIOD

1 SECOND PERIOD

SEISMIC DESIGN CATEGORY

1 SECOND PERIOD

NATURAL FREQUENCY

EXPOSURE

SNOW EXPOSURE FACTOR

SNOW LOAD IMPORTANCE FACTOR

ULTIMATE DESIGN WIND SPEED

SEISMIC IMPORTANCE FACTOR

INTERNAL PRESSURE COEFFICIENT

STRUCTURE IS FLEXIBLE

POUR OR UNTIL CONCRETE HAS ATTAINED THE SPECIFIED DESIGN STRENGTH.

INCLUDING THE FOLLOWING REQUIREMENTS FROM STANDARDS - 29 CFR, SECTION 1926.

BEAMS, CONCRETE COLUMNS, AND CONCRETE BEAMS.

SUBPART R:

STEEL ERECTION.

THE CONTRACTOR SHALL SUBMIT FOR PRIOR APPROVAL THE END OF POUR LOCATIONS FOR CONCRETE GRADE

THE CONTRACTOR SHALL BE RESPONSIBLE FOR ADHERING TO ALL APPLICABLE STANDARDS SET FORTH BY OSHA

A. THE STEEL ERECTION CONTRACTOR SHALL NOT ERECT STEEL UNLESS THEY HAVE RECEIVED WRITTEN

NOTIFICATION FROM THE CONTRACTOR THAT THE CONCRETE IN THE FOOTINGS, PIERS AND WALLS OR THE MORTAR IN THE MASONRY PIERS AND WALLS HAS ATTAINED, ON THE BASIS OF AN APPROPRIATE ASTM

STANDARD TEST METHOD OF FIELD-CURED SAMPLES, EITHER 75 PERCENT OF THE INTENDED MINIMUM

COMPRESSIVE DESIGN STRENGTH OR SUFFICIENT STRENGTH TO SUPPORT THE LOADS IMPOSED DURING

(PER IBC 1607.4)

ASSEMBLY AREAS

SAFER ROOM

PARTITIONS

STORAGE

*MINIMUM CONCENTRATED LOAD

REDUCTION FACTOR BASED ON TRIB AREA

**INCLUDES 5 PSF RAIN-ON SNOW SURCHARGE LOAD

PROVIDE STRUCTURAL ENGINEER A COPY OF WRITTEN NOTIFICATION WHEN IT IS PROVIDED TO THE STEEL ERECTOR. ANCHOR RODS (ANCHOR BOLTS) SHALL NOT BE REPAIRED, REPLACED OR FIELD-MODIFIED WITHOUT THE

APPROVAL OF THE PROJECT STRUCTURAL ENGINEER OF RECORD. PRIOR TO ERECTION OF COLUMNS, THE CONTRACTOR SHALL PROVIDE WRITTEN NOTIFICATION TO THE STEEL ERECTOR IF THERE HAS BEEN ANY REPAIR. REPLACEMENT OR MODIFICATION OF THE ANCHOR RODS (ANCHOR BOLTS).

PROVIDE STRUCTURAL ENGINEER A COPY OF WRITTEN NOTIFICATION WHEN IT IS PROVIDED TO THE STEEL ERECTOR.

: NO MODIFICATION THAT AFFECTS THE STRENGTH OF A STEEL JOIST OR STEEL JOIST GIRDER SHALL BE MADE WITHOUT THE APPROVAL OF THE PROJECT STRUCTURAL ENGINEER OF RECORD.

. METAL DECKING HOLES AND OPENINGS SHALL NOT BE CUT UNTIL IMMEDIATELY PRIOR TO BEING PERMANENTLY FILLED WITH THE EQUIPMENT OR STRUCTURE, OR SHALL BE IMMEDIATELY COVERED.

PROTECTION: PROPER PRECAUTIONS SHALL BE TAKEN AT ALL TIMES TO PROTECT VEHICULAR AND PEDESTRIAN TRAFFIC FROM ANY DAMAGE OR INJURY WHICH MAY BE CAUSED. EITHER DIRECTLY OR INDIRECTLY. BY THE WORK INCLUDED ON THESE DRAWINGS. SUCH PRECAUTIONS SHALL INCLUDE THE ERECTION AND MAINTENANCE OF FENCES, BARRICADES, RAILINGS, GUARDS, SIGNS, COVERINGS, LIGHTS, AND OTHER PRECAUTIONS AS MAY BE REQUIRED. IF AT ANY TIME. IN THE OPINION OF THE OWNER OR THE OWNER'S REPRESENTATIVE. PROPER PRECAUTIONS ARE NOT BEING TAKEN TO SECURE THIS PROTECTION, THE CONTRACTOR SHALL AT NO ADDITIONAL COST TO THE OWNER, INSTALL AND MAINTAIN SUCH ADDITIONAL PROTECTION AS MAY BE DIRECTED

POLLUTION CONTROLS: USE WATER SPRINKLING, TEMPORARY ENCLOSURES, AND OTHER SUITABLE METHODS TO LIMIT DUST AND DIRT RISING AND SCATTERING IN THE AIR TO LOWEST PRACTICAL LEVEL. COMPLY WITH GOVERNING REGULATIONS PERTAINING TO ENVIRONMENTAL PROTECTION

TYPICAL DETAIL SHEETS:

THE S7.00 SERIES SHEETS IN THESE DRAWINGS CONTAIN TYPICAL STRUCTURAL DETAILS FOR VARIOUS BUILDING MATERIALS. SOME OF THESE DETAILS MAY NOT BE PART OF THIS PROJECT

DRAWINGS:

DO NOT SCALE DRAWINGS

WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL NOTES, AND SPECIFICATIONS, THE MORE STRINGENT REQUIREMENTS SHALL GOVERN. DETAILS ON DRAWINGS TAKE PRECEDENCE OVER GENERAL NOTES. AND TYPICAL DETAILS. DETAILS NOTED "TYPICAL" APPLY TO ALL SIMILAR CONDITIONS. WHERE NO SPECIFIC DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ELSEWHERE ON THE PROJECT.

FAST-TRACK/PHASED CONSTRUCTION:

THE STRUCTURAL PORTION OF THIS PROJECT IS BEING DESIGNED, BID, PERMITTED, AND CONSTRUCTED PRIOR TO THE COMPLETION OF ARCHITECTURAL, ENGINEERING, AND OTHER DESIGN TEAM CONSTRUCTION DOCUMENTS. THE OWNER, ARCHITECT, AND CONTRACTOR SHALL BE AWARE THAT THIS ACCELERATED STRUCTURAL SCHEDULE CREATES INHERENT RISK OF FUTURE CHANGES DUE TO DESIGN COORDINATION WITH OTHER DISCIPLINES. WHILE EVERY EFFORT HAS BEEN MADE TO MINIMIZE THESE CHANGES. THE RISK OF ADDED COSTS DUE TO THESE CHANGES SHALL BE UNDERSTOOD AND ACCEPTED BY ALL PARTIES.

DRAWINGS THAT DO NOT HAVE AN ENGINEERING SEAL BY THE STRUCTURAL ENGINEER OF RECORD OR NOT LABELED AS CONSTRUCTION DRAWINGS ARE PRELIMINARY AND SUBJECT TO CHANGE. IF THESE DOCUMENTS ARE BEING USED FOR PRICING, BIDDING, STEEL MILL ORDER, OR PREPARATION OF SHOP DRAWINGS, THE CONTRACTOR SHALL ANTICIPATE FUTURE DRAWING REVISIONS THAT MAY AFFECT THIS WORK OR INCREASE CONSTRUCTION COSTS. THE STRUCTURAL ENGINEER SHALL NOT BE RESPONSIBLE FOR ANY CHANGE ORDER COSTS INCURRED DUE TO THESE DRAWING REVISIONS, AND THE CONTRACTOR SHALL CONSIDER THESE ANTICIPATED COSTS IN ANY BIDS OR PRICE GUARANTEES TO THE OWNER.

JSE THE MOST CURRENT SET OF DRAWINGS IN PREPARATION OF ALL SUBMITTALS. ALL SUBMITTALS SHALL LIST THE DATE OF THE DRAWINGS USED TO PREPARE THE SUBMITTAL. SUBMITTALS PREPARED FROM OUTDATED DRAWINGS MAY BE REJECTED. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ACQUIRING THE LATEST SET OF CONSTRUCTION DRAWINGS AND DISTRIBUTING TO THE APPROPRIATE PARTIES.

CAST-IN-PLACE CONCRETE:

ALL CONCRETE SHALL CONFORM TO THE SPECIFICATIONS FOR STRUCTURAL CONCRETE, ACI 301-10.

ALL EXPOSED EDGES OF CONCRETE SHALL HAVE A 3/4" CHAMFER UNLESS NOTED OTHERWISE.

NORMALWEIGHT CONCRETE:

NOT ALLOWED. COMPLY WITH ACI 305R.

A. F'C = 4500 PSI @ 28 DAYS – ALL CONCRETE EXPOSED TO FREEZE/THAW CYCLES AND OCCASIONAL MOISTURE, INCLUDING CONCRETE FLAT WORK, EXPOSED BUILDING STEM WALLS, SITE WALLS, ETC. EXTERIOR CONCRETE SHALL MEET EXPOSURE CATEGORY AND CLASS F1 ACCORDING TO ACI 318 TABLE

B. F'C = 3000 PSI @ 28 DAYS - ALL FOOTINGS, TIE BEAMS, GRADE BEAMS. C. $F'C = 3000 \, PSI \, @ \, 28 \, DAYS - ALL INTERIOR SLABS ON GRADE, UNLESS NOTED OTHERWISE.$

D. F'C = 3500 PSI @ 28 DAYS - ALL CONCRETE FILL OVER METAL DECK, UNLESS NOTED OTHERWISE. E. F'C = 4000 PSI @ 28 DAYS – ALL CAST-IN-PLACE CONCRETE COLUMNS, PEDESTALS, RETAINING WALLS, AND

ELEVATED BEAMS. F. F'C = 4000 PSI @ 28 DAYS – ALL ELEVATED CAST-IN-PLACE SLABS.

G. F'C = 4000 PSI @ 28 DAYS – ALL SLABS ON GRADE AND ELEVATED SLABS TO RECEIVE POLISHED CONCRETE

FIRE RATED SLABS: COORDINATE AIR CONTENT REQUIREMENTS WITH ARCHITECTURAL DRAWINGS.

CONCRETE MIX DESIGNS (INCLUDING AIR CONTENT, WATER TO CEMENT RATIOS, AND OTHER CRITERIA) SHALL CONFORM TO THE REQUIREMENTS SET FORTH IN ACI 318 TABLE 19.3.2.1, BASED ON THE EXPOSURE CATEGORIES AND CLASSES DEFINED IN ACI 318 TABLE 19.3.1.1. USE AIR ENTRAINING ADMIXTURE IN ALL EXTERIOR CONCRETE AIR CONTENT IN FIRE RATED SLABS SHALL ALSO COMPLY WITH THE REQUIREMENTS IN THE SPECIFIED UL LISTING.

COLD WEATHER CONCRETING: PROTECT CONCRETE WORK FROM PHYSICAL DAMAGE OR REDUCED STRENGTH

STRENGTH OF THE CONCRETE, REDUCE DELIVERY TIME OF READY MIX CONCRETE, LOWER THE TEMPERATURE

OF MATERIALS, OR ADD RETARDER TO ENSURE THAT THE CONCRETE IS PLASTIC. RETEMPERING WITH WATER IS

CAUSED BY FROST, FREEZING OR LOW TEMPERATURES. COMPLY WITH ACI 306.1 HOT WEATHER CONCRETING: WHEN HOT WEATHER CONDITIONS EXIST THAT WOULD IMPAIR THE QUALITY AND

SLAB CURING: ALL INTERIOR CONCRETE SLABS, EXCEPT EXPOSED INTEGRALLY COLORED SLABS, ARE TO BE CURED WITH A MOISTURE RETAINING COVER FOR THE FIRST 7 DAYS (MINIMUM) AFTER PLACEMENT.

THE CONTRACTOR IS ALLOWED TO CAST FOUNDATIONS AGAINST EXCAVATED SOIL SURFACES, PROVIDED THE FOLLOWING IS ADHERED TO:

A. THE SIDE SLOPES OF THE EXCAVATION SHALL BE ABLE TO MAINTAIN VERTICAL SLOPE WITHOUT SOIL

SLOUGHAGE. . THE BOTTOM WIDTH OF THE EXCAVATION SHALL BE ONE INCH WIDER MINIMUM ON EACH SIDE THAN THE

SPECIFIED FOOTING WIDTH. THE SIDE WALLS OF THE EXCAVATION SHALL BE BATTERED A MINIMUM OF ONE INCH HORIZONTAL TO TWELVE INCHES VERTICAL

IF SANDY OR LOOSE MATERIALS ARE ENCOUNTERED, THE FOOTING MUST BE FORMED.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVAL OF ANY SOIL SLOUGHAGE FROM THE WET CONCRETE DURING THE CASTING OPERATION

THE CONTRACTOR AGREES TO REMOVE AND RECAST ANY FOOTING WHERE THE ABOVE CONDITIONS ARE NOT

EXPOSED SITE WALLS, RETAINING WALLS, AND STEM WALLS GREATER THAN 30 FEET IN LENGTH SHALL HAVE CONTROL JOINTS INSTALLED AT THE FOLLOWING MAXIMUM SPACING:

12'-0" ON CENTER FOR WALLS 6'-0" MAXIMUM HEIGHT 18'-0" ON CENTER FOR WALLS 10'-0" MAXIMUM HEIGHT 20'-0" ON CENTER FOR WALLS GREATER THAN 10'-0" IN HEIGHT

ALL CONCRETE EXPOSED TO GROUND SHALL BE MANUFACTURED WITH PORTLAND CEMENT TYPE I OR TYPE II.

SEE SHEET S7.11 FOR TYPICAL CONCRETE DETAILS.

REINFORCING STEEL

ALL REINFORCING STEEL SHALL BE FABRICATED AND PLACED IN ACCORDANCE WITH THE BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI 318-14), AND DETAILS AND DETAILING OF CONCRETE REINFORCEMENT (ACI 315-99).

FIELD-BENT BARS, WHICH SHALL CONFORM TO ASTM A615 GRADE 40.

ALL REINFORCING STEEL SHALL CONFORM TO ASTM A615 GRADE 60; EXCEPT STIRRUPS, TIES AND INDICATED

ALL WELDED WIRE FABRIC SHALL BE DEFORMED AND SHALL CONFORM TO ASTM A479. PROVIDE IN FLAT SHEETS

TENSION AND COMPRESSION LAPS IN REINFORCING SHALL CONFORM TO THE LAP SPLICE SCHEDULE ON SHEET S6.01 AND BE IN ACCORDANCE WITH ACI 318, CHAPTER 12, UNLESS NOTED OTHERWISE.

ALL HORIZONTAL REINFORCING IN FOOTINGS, WALLS AND BEAMS SHALL BE CONTINUOUS AROUND CORNERS OR HAVE BENT (CORNER) BARS OF THE SAME SIZE AND SPACING AS THE HORIZONTAL BARS AND LAP 30 BAR DIAMETERS (24" MINIMUM).

GENERAL STRUCTURAL NOTES

CONCRETE COVER FOR REINFORCING SHALL BE AS FOLLOWS UNLESS OTHERWISE NOTED: A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: B. CONCRETE CAST AGAINST FORMS BUT EXPOSED TO EARTH OR WEATHER:

. BARS LARGER THAN NO. 5:

2. BARS NO. 5 OR SMALLER: C. CONCRETE NOT EXPOSED TO WEATHER OR NOT IN CONTACT WITH GROUND COLUMNS, GIRDERS AND BEAMS: 2. STRUCTURAL SLABS, WALLS AND JOISTS (NO. 11 AND SMALLER):

1 1/2" FROM TOP OF SLAB D. SLAB ON GRADE: E. STRUCTURAL SLABS ON METAL DECK: 1" FROM TOP OF SLAB

FORM TIES SHALL BE EITHER OF THE THREADED OR SNAP-OFF TYPE SO THAT NO METAL WILL BE LEFT WITHIN 1 INCH OF THE SURFACE OF THE WALL. FOLLOWING REMOVAL OF FORM TIES. RECESSES ARE TO BE CAREFULLY FILLED AND POINTED WITH MORTAR.

REINFORCING SHALL NOT BE TACK WELDED OR WELDED IN ANY MANNER UNLESS SPECIFICALLY DETAILED ON THE STRUCTURAL PLANS.

BAR SUPPORTS AND SPACERS FOR REINFORCING SHALL BE PROVIDED IN ACCORDANCE WITH ACI 315-99. REINFORCING SHALL BE SECURELY TIED TO SUPPORTS

CHAIRS WITH 22 GAGE SAND PLATES OR PRECAST BLOCKS SHALL BE PROVIDED FOR ALL REINFORCING OF CONCRETE IN CONTACT WITH GRADE.

DECK CHAIRS SHALL BE PROVIDED FOR ALL WELDED WIRE FABRIC IN SLABS OVER METAL DECK.

POST INSTALLED ANCHORS

THE STRUCTURAL DESIGN IS BASED ON THE POST INSTALLED ANCHORING SYSTEMS NOTED BELOW. SINCE ANCHOR CAPACITIES VARY BY MANUFACTURER, THE CONTRACTOR SHALL USE ONLY THE SYSTEMS NOTED BELOW UNLESS AN ALTERNATE IS APPROVED BY THE ENGINEER OF RECORD. ALTERNATE ANCHORING SYSTEMS MAY REQUIRE RE-DESIGN TO VERIFY ANCHOR QUANTITIES, SPACING, AND EMBED DEPTHS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY ADDITIONAL CONSTRUCTION AND RE-DESIGN COSTS ASSOCIATED WITH THE ALTERNATE ANCHORING SYSTEM.

ALL ADHESIVE (EPOXY) FOR POST INSTALLED ANCHORS AND/OR REBAR INTO GROUT FILLED MASONRY SHALL BE HILTI HIT HY 70 ADHESIVE ANCHORING SYSTEM. INSTALLATION SHALL BE PER THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.

ALL ADHESIVE (EPOXY) FOR POST INSTALLED ANCHORS AND/OR REBAR INTO HOLLOW MASONRY AND/OR BRICK SHALL BE HILTI HIT HY 270 ADHESIVE ANCHORING SYSTEM. INSTALLATION SHALL BE PER THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.

ALL POST INSTALLED MECHANICAL ANCHORS INTO CONCRETE SHALL BE HILTI KWIK HUS EZ (KH-EZ) SCREW

ANCHOR. INSTALLATION SHALL BE PER THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.

ANCHOR LENGTHS SHOWN FOR ATTACHMENT TO CONCRETE AND/OR MASONRY ARE REQUIRED EMBEDMENT LENGTHS. THE CONTRACTOR SHALL PROVIDE ANCHORS WITH ADDITIONAL LENGTH TO FACILITATE THE REQUIRED CONNECTION.

SUBMIT ALL PROPOSED ANCHORING SYSTEMS INCLUDING ICC-ES REPORTS TO STRUCTURAL ENGINEER FOR REVIEW PRIOR TO INSTALLATION. THE ICC-ES FORMS SHALL MEET THE REQUIREMENTS OF THE IBC REFERENCED IN THESE NOTES.

ALL POST-INSTALLED ANCHORS SHALL BE INSTALLED WITH SPECIAL INSPECTION AS DICTATED BY THE RESPECTIVE PRODUCT'S ICC-ES EVALUATION SERVICE REPORT

THE CONTRACTOR SHALL ARRANGE AN ANCHOR MANUFACTURER'S REPRESENTATIVE TO PROVIDE ONSITE INSTALLATION TRAINING, UNLESS ALL PERSONNEL INSTALLING ANCHORS ARE CERTIFIED IN ACCORDANCE WITH ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM, OR EQUIVALENT APPROVED BY THE ENGINEER OF RECORD.

INSTALLATION OF ADHESIVE ANCHORS IN HORIZONTAL TO VERTICALLY OVERHEAD ORIENTATION SHALL BE CONTINUOUSLY INSPECTED DURING INSTALLATION BY AN INSPECTOR SPECIALLY APPROVED FOR THAT PURPOSE BY THE BUILDING OFFICIAL. INSTALLATION SHALL BE DONE BY A CERTIFIED ADHESIVE ANCHOR INSTALLER (AAI) AS CERTFIEID THROUGH ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM, OR EQUIVALENT. PROOF OF CURRENT CERTIFICATION SHALL BE SUBMITTED TO THE ENGINEER PRIOR TO COMMENCEMENT OF INSTALLATION, AND INSPECTION REPORTS SHALL BE PROVIDED TO THE ENGINEER OF RECORD AND THE BUILDING

STRUCTURAL AND MISCELLANEOUS STEEL:

ALL STRUCTURAL STEEL SHALL BE DETAILED AND FABRICATED IN ACCORDANCE WITH THE AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS".

ALL WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A992, GRADE 50, UNLESS NOTED OTHERWISE.

ALL MISCELLANEOUS STEEL MEMBERS, SUCH AS CHANNELS, ANGLES, FLAT BARS, AND PLATES SHALL CONFORM TO ASTM A36 UNLESS NOTED OTHERWISE.

ALL RECTANGULAR AND SQUARE STRUCTURAL TUBING SHALL CONFORM TO ASTM A500, GRADE B, FY = 46 KSI OR ASTM 1085, GRADE B, FY = 50 KSI.

ALL ROUND STRUCTURAL TUBING SHALL CONFORM TO ASTM A500, GRADE B, FY = 42 KSI OR ASTM 1085, GRADE B, FY = 50 KSI.

ALL STRUCTURAL PIPE SHALL CONFORM TO ASTM A53, TYPE E OR S, GRADE B, FY = 35 KSI.

BOLTS SHALL CONFORM TO ASTM A325N TENSION CONTROL BOLTS UNLESS NOTED OTHERWISE, WITH SIZES AS SHOWN ON THE DRAWINGS. WHERE CLEARANCE WITHIN A CONNECTION DOES NOT PERMIT THE USE OF TENSION CONTROL BOLTS, STANDARD A325N BOLTS SHALL BE USED AND INSPECTED IN ACCORDANCE WITH THE AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS".

ALL BOLTS SHALL BE INSTALLED IN A SNUG TIGHT CONDITION EXCEPT AT MOMENT CONNECTIONS, BRACED FRAME CONNECTIONS, AND AT CONNECTIONS DETAILED WITH A325SC BOLTS. AT THESE LOCATIONS, THE BOLTS SHALL BE TIGHTENED SO AS TO SHEAR THE SPLINE OFF THE BOLT.

ANCHOR BOLTS EMBEDDED IN CONCRETE SHALL BE ASTM F1554 GRADE 36 THREADED RODS WITH DOUBLE NUTS. PROVIDE FLAT WASHERS BETWEEN NUTS AND BASEPLATE SURFACES. ANCHOR BOLT LENGTHS SHOWN FOR ATTACHMENT TO CONCRETE AND/OR MASONRY ARE REQUIRED EMBEDMENT LENGTHS. THE CONTRACTOR SHALL PROVIDE ANCHOR BOLTS WITH ADDITIONAL BOLT LENGTH TO FACILITATE THE REQUIRED CONNECTION.

ANCHOR BOLT FLAT WASHERS SHALL BE PROVIDED IN ACCORDANCE WITH TABLE 14-2 OF AISC 360, AISC MANUAL OF STEEL CONSTRUCTION LATEST EDITION.

ALL WELDING SHALL BE DONE IN ACCORDANCE WITH THE LATEST STANDARDS OF THE AWS STRUCTURAL

ALL BOLT HOLES THAT ARE REQUIRED TO BE FIELD DRILLED SHALL BE DRILLED WITH A MAG DRILL. FLAME CUTTING OF HOLES OR ENLARGING OF MISALIGNED HOLES WILL NOT BE ALLOWED.

HEADED CONCRETE ANCHORS AND SHEAR CONNECTORS SHALL BE MADE FROM STEEL CONFORMING TO ASTM A108 AND MEET THE MECHANICAL PROPERTIES OF TYPE B, AS REQUIRED BY CHAPTER 7 OF AWS D1.1 "STRUCTURAL WELDING CODE-STEEL", LATEST EDITION. STRUCTURAL STEEL TO RECEIVE SHEAR CONNECTORS SHALL BE FREE OF PAINT. WELDING PREQUALIFICATION REQUIRED.

COMPOSITE FLOORS:

THE METAL DECK FOR COMPOSITE FLOORS SHALL BE UNSHORED UNLESS NOTED OTHERWISE.

THE SHEAR CONNECTORS SHALL BE 3/4" DIAMETER X 3" AT 1 1/2" DEEP DECK AND 3/4" DIAMETER X 4 1/2" AT 3" DEEP BRACING INSTALLATION. DECK UNLESS NOTED OTHERWISE. THE SHEAR CONNECTORS SHALL BE MADE FROM STEEL CONFORMING TO ASTM A108 AND MEET THE MECHANICAL PROPERTIES OF TYPE B, AS REQUIRED BY CHAPTER 7 OF AWS D1.1 "STRUCTURAL WELDING CODE STEEL", LATEST EDITION. STRUCTURAL STEEL TO RECEIVE SHEAR CONNECTIONS SHALL BE FREE OF PAINT. WELDING PREQUALIFICATION REQUIRED.

THE SHEAR CONNECTIONS SHALL NOT BE ADDED UNTIL THE METAL FLOOR DECK IS INSTALLED.

WHERE SHEAR CONNECTIONS AND PUDDLE WELDS COINCIDE, THE SHEAR CONNECTOR MAY REPLACE THE PUDDLE WELD.

CAMBERED BEAMS SHALL HAVE THE CAMBER PUT IN AT 1/3 POINTS OR ALONG A PARABOLIC CURVE.

HE CONTRACTOR SHALL SURVEY THE CAMBER OF THE BEAMS AFTER THE BEAMS HAVE BEEN ERECTED. THE CONTRACTOR SHALL SUBMIT THE SURVEY TO THE ENGINEER FOR REVIEW. THE CONTRACTOR SHALL NOT POUR THE SLAB UNTIL THE ENGINEER HAS REVIEWED AND APPROVED THE BEAM CAMBERS.

CONTRACTOR SHALL SHORE BEAMS WITH A CAMBER MORE THAN 1/2" LOWER THAN SPECIFIED. THE BEAM SHALL

BE ALLOWED TO DEFLECT TO LEVEL.

GENERAL STRUCTURAL NOTES

THE TOPPING SLAB SHALL BE POURED AND PLACED TO THE ELEVATION INDICATED ON THE DRAWINGS.

THE CONCRETE FOR THE SLAB SHALL BE POURED AND PLACED TO THE ELEVATION INDICATED ON THE DRAWINGS WHILE MAINTAINING THE MINIMUM THICKNESS. SPREAD CONCRETE OVER AREA OF INFLUENCE TO ROUGH DEPTH

IN ORDER TO LOAD BEAMS AND GIRDERS PRIOR TO SETTING SCREED ELEVATIONS. THE WEIGHT OF THE WET CONCRETE WILL CAUSE DEFLECTIONS OF THE STEEL FRAMING. THEREFORE, CONCRETE OVERRRUNS ARE TO BE ANTICIPATED BY THE CONTRACTOR.

CONTRACTOR SHALL CONTINUOUSLY MONITOR THE THICKNESS AND ELEVATIONS DURING CONCRETE PLACING

PROVIDE #4 X 6'-0" AT 12" ON CENTER OVER ALL GIRDERS OF COMPOSITE FLOORS.

PROVIDE DECK CHAIRS FOR ALL WELDED WIRE FABRIC IN SLABS OVER METAL DECK.

PROVIDE #4 X 6'-0" AT 12" ON CENTER OVER SHORED BEAMS THAT ARE NOT ALLOWED TO DEFLECT TO LEVEL

PROVIDE WELDED WIRE FABRIC AS INDICATED ON DRAWINGS IN FLAT SHEETS ONLY.

STEEL DECK:

ALL STEEL DECK SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE STEEL DECK INSTITUTE SPECIFICATIONS.

SEE PLANS FOR STEEL DECK TYPE, GAGE, FINISH AND CONNECTIONS.

PROVIDE A MINIMUM OF 1 1/2" BEARING FOR ALL STEEL DECK

ALL SPLICES AND LAPS SHALL BE A MINIMUM OF 2" IN LENGTH AND SHALL BE LOCATED DIRECTLY ABOVE SUPPORTS.

ALL DECKING SHALL BE CONTINUOUS OVER TWO OR MORE SPANS.

POWDER DRIVEN FASTENERS SHALL BE EQUIVALENT TO: HILTI X-HSN 24 FOR STEEL BASE MATERIAL IF UP TO 3/8". HILTI ENP-19 FOR STEEL BASE MATERIAL tf 1/4" OR THICKER]

COLD-FORMED METAL FRAMING (43 MILS OR HEAVIER):

ALL COLD-FORMED METAL FRAMING SHALL CONFORM TO THE LATEST EDITION OF AISI STANDARD S100 "NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS".

WALLS SHALL BE PROVIDED WITH MANUFACTURER'S STANDARD BRIDGING: (EITHER WELDED 2 1/2" x 43 MILS STUD OR CLIPPED COLD-ROLLED CHANNEL 1 1/2" x 54 MILS). PROVIDE BRIDGING AT 4'-0" ON CENTER MAXIMUM FOR LOAD BEARING WALLS AND EXTERIOR WALLS.

PROVIDE ALL MISCELLANEOUS ACCESSORIES AND FOLLOW ERECTION PROCEDURES AS PER MANUFACTURER'S SPECIFICATIONS AND RECOMMENDATIONS UNLESS NOTED OTHERWISE.

MANUFACTURERS ASSOCIATION (SSMA) SPECIFICATIONS. ALL TRACK SHALL BE ANCHORED TO CONCRETE WITH 1/2" DIAMETER x 3 1/2" EMBED EXPANSION ANCHORS SPACED

COLD-FORMED METAL FRAMING SHALL MEET THE MINIMUM PROPERTIES AS SHOWN IN THE STEEL STUD

AT 4'-0" ON CENTER UNLESS SHOWN OTHERWISE ON PLANS. SECURE STUDS TO TOP AND BOTTOM TRACKS BY WELDING AT BOTH INSIDE AND OUTSIDE FLANGES OR WITH A MINIMUM OF 1-#10 SELF-DRILLING SCREW PER LOCATION UNLESS NOTED OTHERWISE.

ALL COMPONENTS OF BUILT-UP STUD SECTIONS, INCLUDING COLUMNS, HEADERS, ETC. SHALL BE WELDED TOGETHER UTILIZING 1/8" FILLET WELDS, 1" LONG AT 12" OC, ALONG THE FULL LENGTH OF EACH FLANGE TO

FASTEN WELD CLIPS TO STUDS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS AND LOAD DATA TO PROVIDE AN ALLOWABLE LOAD OF [700# MINIMUM] IN THE HORIZONTAL DIRECTION AND [700# MINIMUM] IN THE VERTICAL DIRECTION. SEE SHEET S7.31 FOR TYPICAL WELD CLIP DETAIL.

FASTEN SLIDE CLIPS TO STUDS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS AND LOAD DATA TO PROVIDE AN ALLOWABLE LOAD OF [700# MINIMUM] IN THE HORIZONTAL DIRECTION. SEE SHEET S7.31 FOR TYPICAL SLIDE CLIP DETAIL.

SEE SHEET S7.31 FOR TYPICAL COLD-FORMED DETAILS.

FOR CMU OR BRICK VENEER (5" MAXIMUM, 3" MINIMUM THICKNESS) ATTACHMENT TO STRUCTURAL MASONRY PROVIDE ADJUSTABLE INTEGRAL ANCHOR TIES. ADJUSTABLE INTEGRAL ANCHOR TIES SHALL BE CORROSION RESISTANT AND HAVE TWO PINTLE LEGS MINIMUM WITH W2.8 (3/16") WIRE OR APPROVED EQUAL. PROVIDE DUR-O WALL DA370 ADJUSTABLE INTEGRAL ANCHOR TIES OR APPOVED EQUAL.

FOR CMU OR BRICK VENEER (5" MAXIMUM. 3" MINIMUM THICKNESS) ATTACHMENT TO STRUCTURAL CONCRETE PROVIDE ADJUSTABLE ANCHOR TIES. ADJUSTABLE ANCHOR TIES SHALL BE CORROSION RESISTANT AND HAVE A TWO PINTLE LEGS MINIMUM WITH A MINIMUM W2.8 (3/16") WIRE. ATTACH TO CONCRETE WITH 2-1/4" DIAMETER CONCRETE SCREWS, HILTI KWIKCON 11 x 1 1/2" OR APPROVED EQUAL

FOR CMU OR BRICK VENEER (5" MAXIMUM, 3" MINIMUM THICKNESS) ATTACHMENT TO STRUCTURAL COLD FORMED METAL STUDS, PROVIDE ADJUSTABLE ANCHOR TIES. ADJUSTABLE ANCHOR TIES SHALL BE CORROSION RESISTANT AND HAVE TWO PINTLE LEGS MINIMUM W2.8 (3/16") WIRE. PROVIDE DUR-O-WALL DA213 ADJUSTABLE ANCHOR TIE OR APPROVED EQUAL. ATTACH THROUGH SHEATHING TO STUDS WITH 2-1/4" x 1 1/2" CORROSION RESISTANT TEK

SEE TYPICAL DETAILS ON SHEET S7.11, S7.21, AND S7.31 FOR VENEER TIE SPACING.

PROVIDE ADDITIONAL ANCHORS AROUND ALL OPENINGS LARGER THAN 16" IN EITHER DIMENSION. SPACE ANCHORS WITHIN 12" OF OPENING PERIMETER AND MATCH HORIZONTAL OR VERTICAL ANCHOR TIE SPACING.

COORDINATE VENEER LOCATION, TYPE, BOND PATTERN, ETC. WITH ARCHITECTURAL DRAWINGS.

GLASS CURTAIN WALL SYSTEM

ALL LATERAL AND GRAVITY SUPPORT FOR THE GLASS CURTAIN WALL SYSTEM SHALL BE PER THE MANUFACTURER'S RECOMMENDATIONS. SHOP DRAWINGS AND STAMPED CALCULATIONS SHALL BE SUBMITTED FOR APPROVAL BY THE ENGINEER OF RECORD AND THE ARCHITECT PRIOR TO INSTALLATION.

THE ENGINEER STAMPING THE SHOP DRAWINGS SHALL BE REGISTERED IN THE STATE THAT THE PROJECT IS LOCATED.

THE BEAMS AT [ALL] FLOORS HAVE BEEN DESIGNED TO SUPPORT THE GRAVITY LOAD OF THE GLASS CURTAIN WALL SYSTEM. THE GLASS CURTAIN WALL SYSTEM SHALL BE LATERALLY SUPPORTED AT ALL FLOORS AND ROOF

SEISMIC BRACING OF NON-STRUCTURAL COMPONENTS:

SEISMIC BRACING AND RESTRAINTS FOR MECHANICAL/ELECTRICAL EQUIPMENT AND SYSTEMS SHALL BE PROVIDED BY THE CONTRACTOR PER THE 2015 INTERNATIONAL BUILDING CODE (2015 IBC) AND THE ASCE 7-10, MINIMUM DESIGN LOADS FOR BUILDINGS AND STRUCTURES.

SEE THESE GENERAL STRUCTURAL NOTES FOR THE SITE-SPECIFIC SEISMIC DESIGN CRITERIA.

THE CONTRACTOR IS RESPONSIBLE FOR DESIGNING AND INSTALLING THE SEISMIC BRACING AND RESTRAINTS. STAMPED SHOP DRAWINGS, INCLUDING CALCULATIONS, SHALL BE SUBMITTED FOR APPROVAL PRIOR TO ANY

THE ENGINEER STAMPING THE SHOP DRAWINGS SHALL BE REGISTERED IN THE STATE THAT THE PROJECT IS LOCATED.

THE STRUCTURE HAS BEEN DESIGNED FOR A KONE [MODEL #] ELEVATOR.

ALL STRUCTURAL SUPPORTS, FLOOR PENETRATION SIZES AND PIT DIMENSIONS HAVE BEEN DESIGNED BASED ON THE ABOVE INFORMATION. SHOULD THE ACTUAL ELEVATOR(S) SELECTED DIFFER FROM THE INFORMATION PROVIDED ABOVE, THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY ADDITIONAL CONSTRUCTION AND REDESIGN COSTS ASSOCIATED WITH THE ALTERNATE ELEVATOR(S)

THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING ALL ELEVATOR PIT AND FLOOR PENETRATION LOCATIONS AND DIMENSIONS

DEFERRED SUBMITTALS:

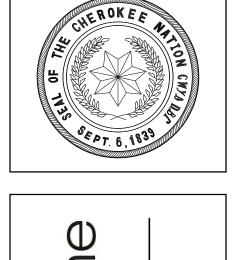
THE DEFERRED SUBMITTALS LISTED BELOW ARE THOSE PORTIONS OF THE DESIGN THAT ARE NOT COMPLETED AT THE TIME OF APPLICATION AND ARE TO BE SUBMITTED TO THE BUILDING OFFICIAL AND APPROVED PRIOR TO THE INSTALLATION OF THOSE ITEMS. THE MANUFACTURER, CONSULTANT, OR CONTRACTOR, AS APPROPRIATE, SHALL PROVIDE SUBMITTALS TO THE ENGINEER OF RECORD FOR REVIEW FOR THE FOLLOWING ITEMS.

- PRECAST PRESTRESSED CONCRETE COLD-FORMED METAL (LIGHTGAGE) FRAMING
- METAL STAIRS, RAMPS, LADDERS, AND GUARDRAILS
- DEWATERING/DRAINAGE SYSTEM BEHIND BASEMENT WALLS. INCLUDING ITS TIE IN TO THE STORM WATER
- SEISMIC BRACING OF ALL ARCHITECTURAL, LIGHTING, THEMING, FIRE SUPPRESSION, AND MEP COMPONENTS CURTAINWALL SYSTEMS
- TEMPORARY AND PERMANENT SHORING OF EXISTING STRUCTURES AND SOILS WHERE REQUIRED FOR NEW CONSTRUCTION
 - NOTES

lames R. Childers Architect, Inc. 45 South 4th Street Fort Smith, AR 72901 479-783-2480 www.childersarchitect.com ROFFSSIONAL SEA **NOTE:** THIS STRUCTURAL

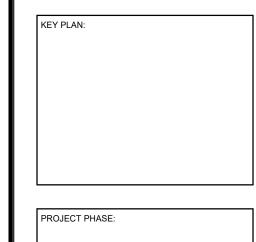
PACKAGE MAY BE USED FOR PROCUREMENT OF STRUCTURA STEEL SHAPES. ANY CHANGES TO THE PROJECT, INCLUDING, BUT NOT LIMITED TO: LOADING REQUIREMENTS. GEOMETRY CHANGES IN PLAN OR ELEVATION, SPACE USAGE REVISIONS, OR VALUE ENGINEERING MAY AFFECT THE STRUCTURAL STEEL MEMBER REQUIREMENTS OF THE ALREADY PROCURED STEEL

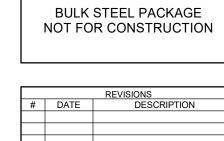












JOB NUMBER: 03-15-19 17-13

> SHEET NUMBER: S0.02

GENERAL STRUCTURAL

SCHEDULE OF STRUCTURAL SPECIAL INSPECTIONS

- 1. SPECIAL INSPECTIONS / TESTING "SPECIAL STRUCTURAL INSPECTION" SHALL NOT RELIEVE THE OWNER OR THEIR AGENT FROM HAVING THE INSPECTIONS OF THE JURISDICTION BUILDING DEPARTMENT PER SECTION 110 OF THE IBC PERFORMED. BOTH THE JURISDICTION BUILDING DEPARTMENT INSPECTIONS AND "SPECIAL STRUCTURAL INSPECTION" SHALL BE PERFORMED.
- 2. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR NOTIFYING THE JURISDICTION BUILDING OFFICIAL AND SPECIAL INSPECTOR WHEN WORK IS READY FOR INSPECTION.
- 3. REPORTING FOR SPECIAL INSPECTION SPECIAL INSPECTION AND TESTING REPORTS SHALL BE COMPLETED AND DISTRIBUTED AT THE COMPLETION OF EACH TASK. IF A TASK IS TO TAKE LONGER THAN THREE (3) DAYS, PROVIDE REPORTS FOR EACH DAY. PROVIDE COPIES OF REPORTS TO CONTRACTOR, OWNER, ARCHITECT AND STRUCTURAL ENGINEER OF RECORD. SPECIAL INSPECTOR TO KEEP A NON-COMPLIANCE LIST DOCUMENTING ITEMS INSPECTED NOT MEETING APPROVED CONSTRUCTION DOCUMENTS AND WHEN / HOW RESOLVED.
- 4. SEE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING CONSTRUCTION DOCUMENTS FOR ADDITIONAL NON-STRUCTURAL SPECIAL INSPECTION ITEMS.
- 5. SPECIAL INSPECTION OF SHOP FABRICATED MEMBERS AND ASSEMBLIES SHALL BE IN ACCORDANCE WITH SECTION 1704.2, UNLESS FABRICATOR IS APPROVED TO PERFORM WORK WITHOUT SPECIAL INSPECTION.
- 6. IN ACCORDANCE WITH IBC CHAPTER 17, THE OWNER OR THE OWNER'S AGENT, OTHER THAN THE CONTRACTOR, SHALL EMPLOY ONE OR MORE APPROVED AGENCIES TO PROVIDE SPECIAL INSPECTIONS AND TESTS, DURING CONSTRUCTION FOR THE TYPES OF WORK LISTED BELOW THESE SPECIAL INSPECTIONS AND TESTS ARE IN ADDITION TO THE INSPECTIONS BY THE BUILDING OFFICIAL IDENTIFIED IN IBC SECTION 110

7. DEFINITIONS:

- * SPECIAL INSPECTION: INSPECTION AS HEREIN REQUIRED BY A QUALIFIED SPECIAL INSPECTOR COMPETENT WITH THE MATERIALS, INSTALLATION, FABRICATION, ERECTION OR PLACEMENT OF COMPONENTS AND CONNECTIONS REQUIRING SPECIAL EXPERTISE TO ENSURE COMPLIANCE WITH APPROVED CONSTRUCTION DOCUMENTS AND REFERENCED STANDARDS (SEE SECTION 1704).
- * CONTINUOUS SPECIAL INSPECTION: FULL-TIME OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED. * PERIODIC SPECIAL INSPECTION: THE PART-TIME OR INTERMITTENT OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK HAS BEEN OR IS BEING PERFORMED AND AT THE COMPLETION OF THE WORK.

ITEM	DESCRIPTION OF REQUIREMENTS	REQUIRED (YES/NO)
SPECIAL INSPECTION OF STRUCTURAL STEEL	TO BE PERFORMED IN ACCORDANCE WITH CHAPTER N OF AISC 360-10	YES
SPECIAL INSPECTION AND VERIFICATION OF STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.2	YES
SPECIAL INSPECTIONS AND VERIFICATIONS FOR CONCRETE CONSTRUCTION	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.3	YES
SPECIAL INSPECTIONS AND VERIFICATIONS FOR WOOD CONSTRUCTION	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.5	NO
SPECIAL INSPECTIONS AND VERIFICATIONS OF SOILS	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.6, THE GEOTECHNICAL REPORT LISTED IN THE GENERAL FOUNDATION NOTES, AND ANY OTHER REQUIREMENTS LISTED IN THE GENERAL FOUNDATION NOTES	YES

GENERAL STRUCTURAL NOTES

TEMPORARY SHORING OF EXCAVATIONS:

THE TEMPORARY SHORING OF EXCAVATIONS SHALL BE SOIL NAIL/SHOTCRETE SYSTEM, SHEET PILING, OR APPROVED EQUAL.

THE SHORING SHALL NOT BE DRIVEN OR INSTALLED IN ANY MANNER THAT COULD POTENTIALLY DAMAGE EXISTING STRUCTURES OR CAUSE HUMAN DISCOMFORT.

THE CONTRACTOR SHALL LOCATE ALL EXISTING UNDERGROUND UTILITIES PRIOR TO INSTALLING SHORING. PROVISIONS SHALL BE MADE TO AVOID EXISTING UTILITIES.

THE SHORING AS SHOWN ON THE PLANS IS FOR GRAPHICAL REPRESENTATION ONLY. THE CONTRACTOR

SHALL VERIFY THE EXACT LOCATION AND CONFIGURATION OF THE SHORING.

ANY SHORING THAT REMAINS IN PLACE SHALL NOT HAMPER FUTURE CONSTRUCTION.

THE CONTRACTOR IS RESPONSIBLE FOR DESIGNING AND INSTALLING THE TEMPORARY SHORING. STAMPED SHOP DRAWINGS, INCLUDING CALCULATIONS, SHALL BE SUBMITTED FOR APPROVAL PRIOR TO ANY SHORING INSTALLATION.

THE ENGINEER STAMPING THE SHOP DRAWINGS SHALL BE REGISTERED IN THE STATE THAT THE PROJECT IS LOCATED.

DEMOLITION:

CONDITION.

PROCEEDING.

NOTCHING OR CUTTING ANY STRUCTURAL MEMBER IN THE FIELD IS PROHIBITED, UNLESS DETAILED OTHERWISE ON THE STRUCTURAL PLANS.

CONTRACTOR SHALL BE RESPONSIBLE FOR ADHERING TO ALL APPLICABLE STANDARDS SET FORTH BY

PRIOR TO STARTING DEMOLITION WORK, THE CONTRACTOR SHALL MAKE AN INSPECTION OF ALL SURROUNDING IMPROVEMENTS TO REMAIN, TO DETERMINE AND RECORD THEIR EXISTING PHYSICAL

SHORING AND BRACING: THE CONTRACTOR SHALL FURNISH ALL SHORING, BRACING, AND INCIDENTALS NECESSARY AND REQUIRED FOR THE PROPER SUPPORT AND SAFETY OF ALL MEMBERS AFFECTED BY DEMOLITION WORK.

WHERE DEMOLITION WOULD AFFECT THE STRUCTURAL INTEGRITY OF THE REMAINING STRUCTURE, THE CONTRACTOR SHALL PROVIDE TEMPORARY SUPPORTS. THE CONTRACTOR SHALL NOTIFY THE ENGINEER OF ANY FIELD CONDITION WHICH WOULD PRESENT A HAZARDOUS CONDITION TO THE STRUCTURE BEFORE

PROTECTION: PROPER PRECAUTIONS SHALL BE TAKEN AT ALL TIMES TO PROTECT VEHICULAR AND PEDESTRIAN TRAFFIC FROM ANY DAMAGE OR INJURY WHICH MAY BE CAUSED, EITHER DIRECTLY OR INDIRECTLY, BY THE WORK INCLUDED ON THESE DRAWINGS. SUCH PRECAUTIONS SHALL INCLUDE THE ERECTION AND MAINTENANCE OF FENCES, BARRICADES, RAILINGS, GUARDS, SIGNS, COVERINGS, LIGHTS, AND OTHER PRECAUTIONS AS MAY BE REQUIRED. IF AT ANY TIME, IN THE OPINION OF THE OWNER OR THE OWNER'S REPRESENTATIVE, PROPER PRECAUTIONS ARE NOT BEING TAKEN TO SECURE THIS PROTECTION, THE CONTRACTOR SHALL AT NO ADDITIONAL COST TO THE OWNER, INSTALL AND MAINTAIN SUCH

POLLUTION CONTROLS: USE WATER SPRINKLING, TEMPORARY ENCLOSURES, AND OTHER SUITABLE METHODS TO LIMIT DUST AND DIRT RISING AND SCATTERING IN THE AIR TO LOWEST PRACTICAL LEVEL. COMPLY WITH GOVERNING REGULATIONS PERTAINING TO ENVIRONMENTAL PROTECTION.

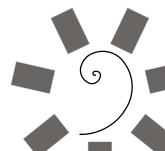
ADDITIONAL PROTECTION AS MAY BE DIRECTED BY THE OWNER.

REMOVE DEBRIS FROM THE SITE AS IT ACCUMULATES. UNLESS OTHERWISE NOTED, DO NOT STORE, SELL, BURN, OR OTHERWISE DISPOSE OF DEBRIS ON THE SITE. REMOVAL OF DEBRIS INCLUDES CLEARING OF ALL LOWER LEVELS AND SIMILAR BELOW GRADE STRUCTURES. REMOVE ALL DEBRIS IN SUCH A MANNER AS TO PREVENT SPILLAGE. KEEP ALL PAVEMENTS AND AREAS ADJACENT TO THE SITE CLEAN AND FREE FROM MUD, DIRT, AND DEBRIS AT ALL TIMES.

USE OF EXPLOSIVES: THE CONTRACTOR IS ABSOLUTELY PROHIBITED FROM USING DYNAMITE OR ANY OTHER EXPLOSIVES IN ANY OF THE WORK OR OPERATIONS SHOWN ON THESE PLANS AT THE PROJECT

DEMOLITION SHALL BE PERFORMED IN A MANNER THAT WILL NOT DAMAGE ADJOINING SURFACES INDICATED TO REMAIN. SURFACES SHALL BE PATCHED, IF REQUIRED, TO PROVIDE A SUITABLE SUBSTRATE FOR NEW CONSTRUCTION.

SPECIFIC DEMOLITION NOTES ARE NOT TO BE CONSIDERED ALL INCLUSIVE OR COMPLETE IN THEMSELVES. CONTRACTOR SHALL PROVIDE ALL DEMOLITION INCIDENTAL TO OR REQUIRED FOR CONSTRUCTION WHETHER SPECIFICALLY NOTED OR NOT.



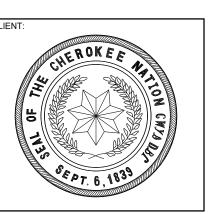
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NOTE: THIS STRUCTURAL PACKAGE MAY BE USED FOR PROCUREMENT OF STRUCTURAL STEEL SHAPES. ANY CHANGES TO THE PROJECT, INCLUDING, BUT NOT LIMITED TO: LOADING REQUIREMENTS, GEOMETRY CHANGES IN PLAN OR ELEVATION, SPACE USAGE REVISIONS, OR VALUE ENGINEERING MAY AFFECT THE STRUCTURAL STEEL MEMBER REQUIREMENTS OF THE ALREADY PROCURED STEEL.









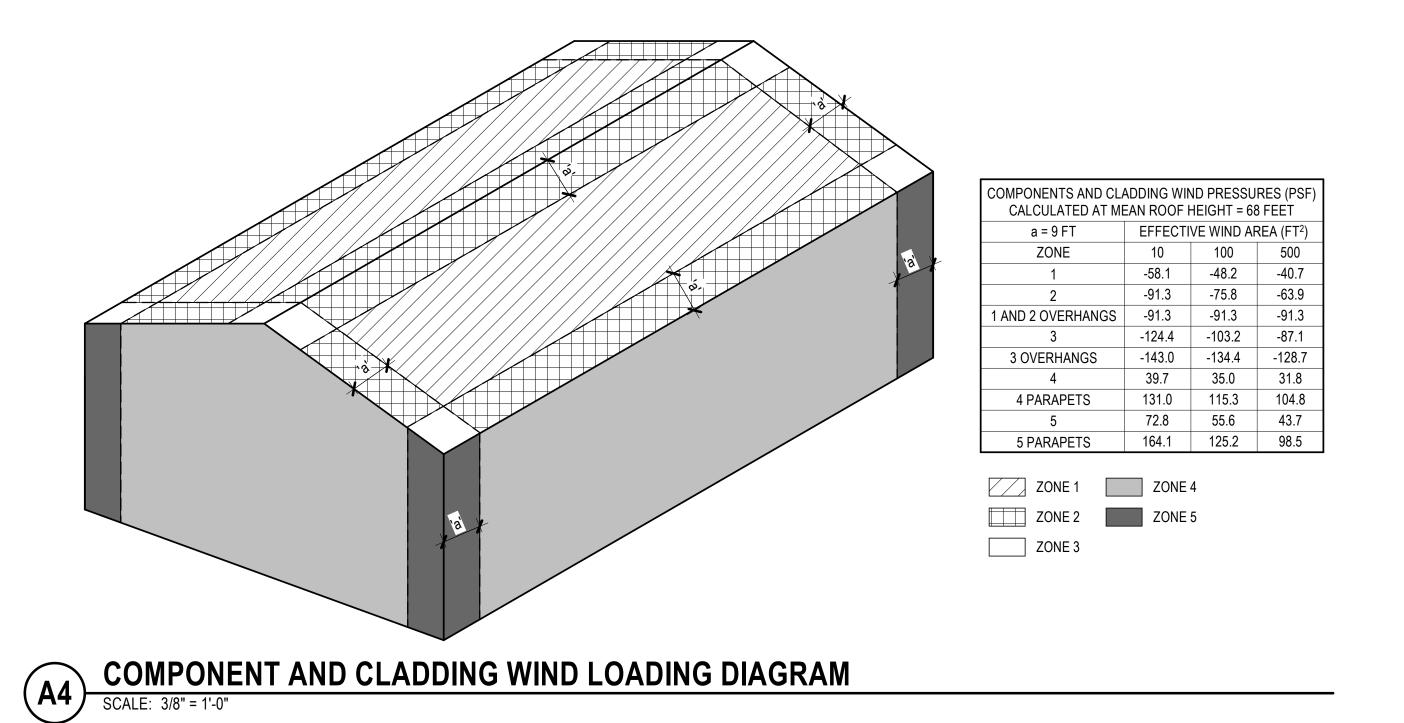
PROJECT PHASE:
BULK STEEL PACKAGE NOT FOR CONSTRUCTION

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#	DATE	DESCRIPTION

DATE:	JOB NUMBER:
03-15-19	9 17-13

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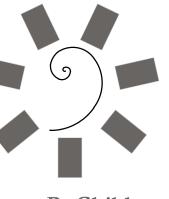
SPECIAL INSPECTION **TABLES**



- . SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- . REFERENCE FINISH FLOOR ELEVATION 100'-0" = MEAN SEA

FINISH FLOOR ELEVATION. SEE CIVIL DRAWINGS.

- 3. NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE CONCRETE WALLS AND MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
 - 4. DIMENSIONS ARE TO THE FACE OF CONCRETE UNLESS NOTED OTHERWISE.
- 5. PROVIDE SLAB JOINTS AT 10'-0" ON CENTER MAXIMUM. THE AREA OF THE CONTROL JOINT SHALL NOT EXCEED A 2.1 RATION CONTROL JOINTS SHALL BE LOCATED AT COLUMN LINES WHERE THE LAYOUT PERMITS. AT RE-ENTRANT CORNERS THAT DO NOT HAVE CONTROL JOINTS, PROVIDE 2-#4 x 3'-0" DIAGONAL TO THE RE-ENTRANT CORNER.
- 3. STRUCTURAL COLD FORMED METAL STUDS SHALL BE 8" WIDE UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING BY OTHERS.
- SEE S7.00 SERIES SHEETS FOR TYPICAL FOUNDATION SECTIONS AND DETAILS.
- 8. SEE SHEET S6.01 FOR SCHEDULES.
- 9. UNDER SLAB FRENCH DRAIN SYSTEM REQUIRED BELOW THE SLAB ON GRADE IN THE BASEMENT. SEE GEOTECHNICAL REPORT SECTION 12 FOR ADDITIONAL INFORMATION.



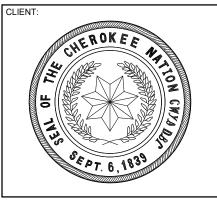
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THE STRUCTURAL STEEL MEMBER
REQUIREMENTS OF THE ALREADY
PROCURED STEEL.

PROFESSIONAL SEAL:





SHEET KEYNOTE

FOOT. COORDINATE EXACT SIZE AND LOCATION WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.

FLOOR DRAIN / MOP SINK, SLOPE SLAB TO DRAIN 1/8" PER

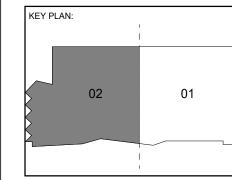
- ELEVATOR SUMP PIT, COORDINATE EXACT SIZE AND LOCATION WITH ELEVATOR MANUFACTURER, SEE A4 / S5.61
- 3. HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE B4 / S5.61
- 4. TOP OF FOOTING ELEVATION = 79'-6". ELEVATION SHALL BE COORDINATED WITH TOP OF ELEVATOR PIT FOOTING. CONTRACTOR TO COORDINATE.
- HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE D4 / S5.61



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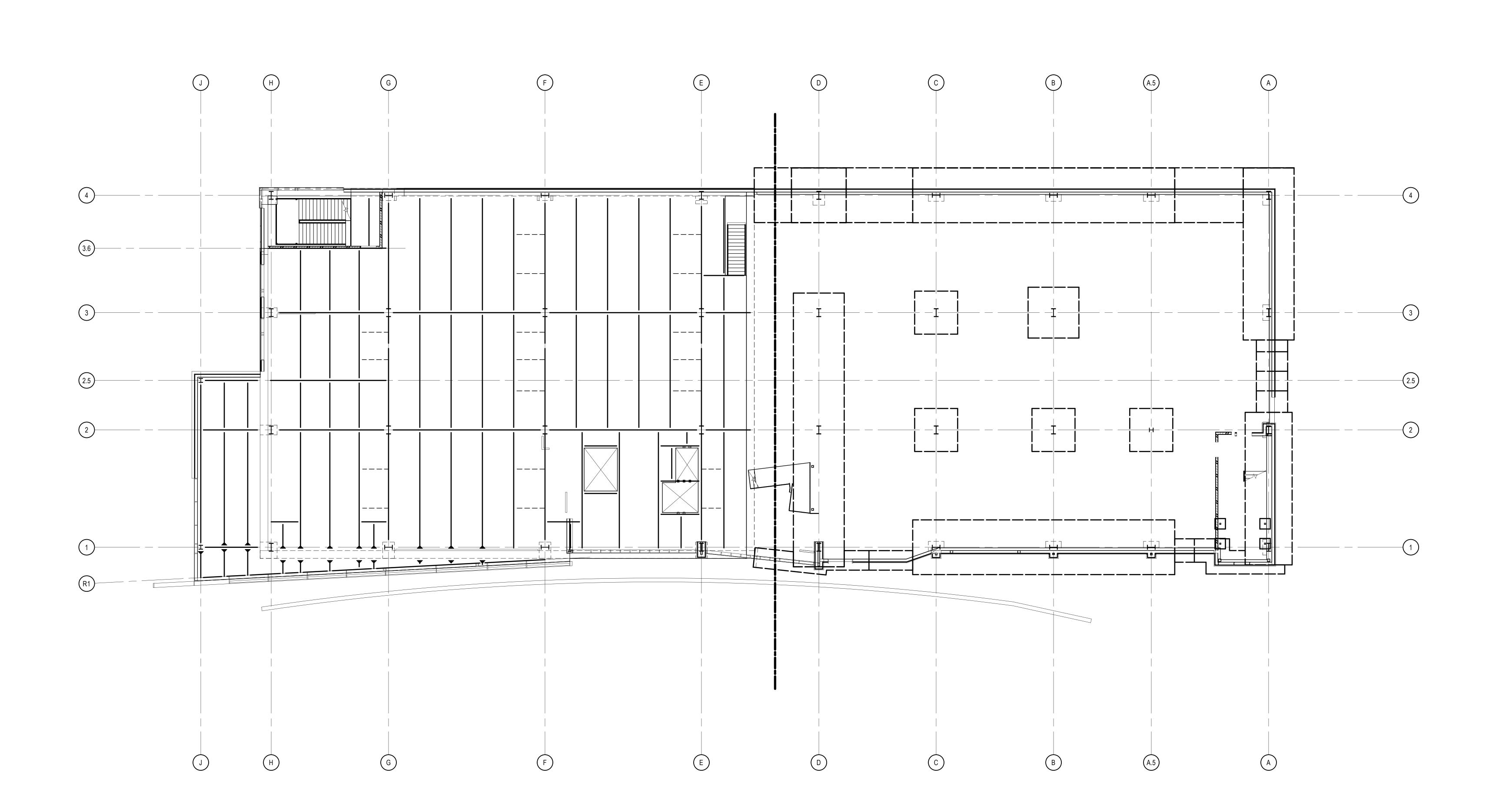


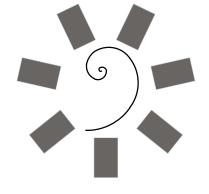


PROJECT PHASE:
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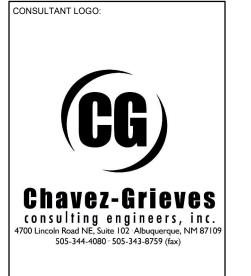
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03-15-19	17-13
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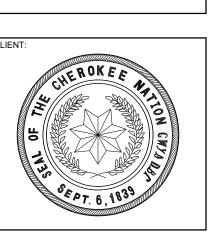




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PROCURED STEEL.

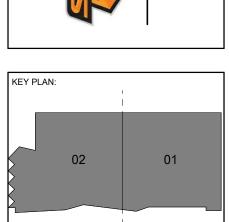




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PROJECT PHASE:

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#	DATE	DESCRIPTION

DATE: JOB NUMBER: 17-13

SHEET NUMBER: S1.10

FIRST FLOOR FRAMING PLAN - OVERALL PLAN

- 1. SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE CONCRETE WALLS AND MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING
- DIMENSIONS ARE TO THE FACE OF CONCRETE OR STUDS UNLESS NOTED OTHERWISE.

THE AISC CODE OF STANDARD PRACTICES.

. SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.

OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF

- BEAMS ARE SPACED EQUALLY BETWEEN GRIDS UNLESS NOTED OTHERWISE.
- 6. STRUCTURAL COLD FORMED METAL STUDS SHALL BE 8" WIDE UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING BY OTHERS.
- 7. SEE S7.00 SERIES SHEETS FOR TYPICAL DETAILS.
- 8. SEE SHEET S6.01 FOR SCHEDULES.
- . ALL MOMENT FRAMES LABELED ON PLAN UTILIZE SIDEPLATE PROPRIETARY MOMENT CONNECTIONS. SEE S8.00 SERIES SHEETS
- DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
- DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE DRAWINGS.
- 12. DIMENSIONS SHOWN ON PLAN AS FOLLOWS ARE CONCRETE COLUMN DIMENSIONS IN INCHES: 38x36, 50x36,ETC.



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NOTE: THIS STRUCTURAL

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STEEL SHAPES. ANY CHANGES TO

THE PROJECT, INCLUDING, BUT

NOT LIMITED TO: LOADING REQUIREMENTS, GEOMETRY

CHANGES IN PLAN OR ELEVATION, SPACE USAGE REVISIONS, OR

VALUE ENGINEERING MAY AFFECT THE STRUCTURAL STEEL MEMBER

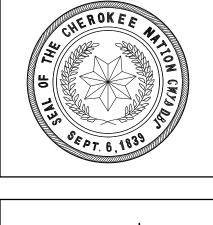
REQUIREMENTS OF THE ALREADY

PROCURED STEEL.

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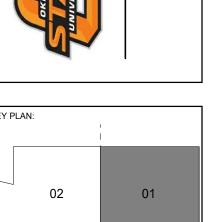
SHEET KEYNOTE

- . HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST, COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE B4 / S5.62 , C4 / S5.62 , D4 / S5.62
- 2. OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A5 / S5.52 FOR SUPPORT.
- . BEAM SPLICE LOCATION. SEE B4 / S5.52 FOR SPLICE DETAIL.
- 4. HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE A4 / S5.62, B4 / S5.62, C4 / S5.62, D4 / S5.62, AND C3 / S5.62
- 5. W8x31 OUTRIGGER.
- TOTAL NUMBER OF CHORD REINFORCEMENT BARS AT EXTENTS SHOWN. CHORD REINFORCEMENT SHALL BE LOCATED AS INDICATED ON PLAN. PROVIDE 130% LAP SPLICES WHEN REQUIRED.
- 3-#7 SLAB REINFORCING BARS. EXTEND BARS 130% OF A LAP SPLICE LENGTH BEYOND OPENING, OR PROVIDE STD 90 DEGREE HOOK WHERE REQUIRED.
- BOTTOM FLANGE BRACING AT EQUAL SPACING, UNLESS NOTED OTHERWISE. SEE B1 / S5.52
- 9. BOTTOM FLANGE BRACING SPACED AT 10' 0" ON CENTER MAXIMUM, UNLESS NOTED OTHERWISE. SEE A1 / S5.52



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PROJECT PHASE:

BULK STEEL PACKAGE
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		REVISIONS
#	DATE	DESCRIPTION

DATE: JOB NUMBER:

03-15-19 17-1

SHEET NUMBER:

S1.11

FIRST FLOOR FOUNDATION AND FRAMING PLAN -SECTOR 1

- 1. SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE CONCRETE WALLS AND MOMENT

FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF

- THE AISC CODE OF STANDARD PRACTICES.

 3. DIMENSIONS ARE TO THE FACE OF CONCRETE OR STUDS UNLESS NOTED OTHERWISE.
- 4. SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS
- NOT SHOWN.

 5. BEAMS ARE SPACED EQUALLY BETWEEN GRIDS UNLESS NOTEI
- OTHERWISE.
- STRUCTURAL COLD FORMED METAL STUDS SHALL BE 8" WIDE UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING BY OTHERS.
- 7. SEE S7.00 SERIES SHEETS FOR TYPICAL DETAILS.
- 8. SEE SHEET S6.01 FOR SCHEDULES.
- 9. ALL MOMENT FRAMES LABELED ON PLAN UTILIZE SIDEPLATE PROPRIETARY MOMENT CONNECTIONS. SEE S8.00 SERIES SHEETS
- 10. DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
- DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE DRAWINGS.
- 12. DIMENSIONS SHOWN ON PLAN AS FOLLOWS ARE CONCRETE COLUMN DIMENSIONS IN INCHES: 38x36, 50x36,ETC.



James R. Childers

Architect, Inc.

45 South 4th Street Fort Smith, AR 72901

479-783-2480

NOTE: THIS STRUCTURAL

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STEEL SHAPES. ANY CHANGES TO

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CHANGES IN PLAN OR ELEVATION, SPACE USAGE REVISIONS, OR

VALUE ENGINEERING MAY AFFECT THE STRUCTURAL STEEL MEMBER

REQUIREMENTS OF THE ALREADY

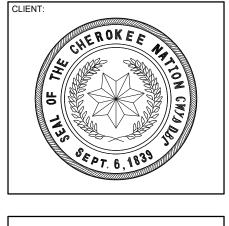
PROCURED STEEL.

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 HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST, COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE B4 / S5.62, C4 / S5.62, D4 / S5.62

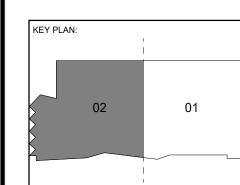
- OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A5 / S5.52 FOR SUPPORT.
- B. BEAM SPLICE LOCATION. SEE B4 / S5.52 FOR SPLICE DETAIL.
- 4. HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE A4 / S5.62, B4 / S5.62, C4 / S5.62, D4 / S5.62, AND C3 / S5.62
- 5. W8x31 OUTRIGGER.
- 6. TOTAL NUMBER OF CHORD REINFORCEMENT BARS AT EXTENTS SHOWN. CHORD REINFORCEMENT SHALL BE LOCATED AS INDICATED ON PLAN. PROVIDE 130% LAP SPLICES WHEN REQUIRED.
- 3-#7 SLAB REINFORCING BARS. EXTEND BARS 130% OF A LAP SPLICE LENGTH BEYOND OPENING, OR PROVIDE STD 90 DEGREE HOOK WHERE REQUIRED.
- B. BOTTOM FLANGE BRACING AT EQUAL SPACING, UNLESS NOTED OTHERWISE. SEE B1 / S5.52
- 9. BOTTOM FLANGE BRACING SPACED AT 10' 0" ON CENTER MAXIMUM, UNLESS NOTED OTHERWISE. SEE A1 / S5.52



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PR	OJECT PHASE:
	BULK STEEL PACKAGE NOT FOR CONSTRUCTION

			REVISIONS
	#	DATE	DESCRIPTION

DATE:	JOB NUMBER:	
03-15-19	17-13	
SHEET NUMBER:		
S1.12		

FIRST FLOOR FRAMING

PLAN - SECTOR 2

FIRST FLOOR FRAMING PLAN - SECTOR 2

SCALE: 1/8" = 1'-0"

- . SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- 2. NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE CONCRETE WALLS AND MOMENT

FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF

DIMENSIONS ARE TO THE FACE OF CONCRETE OR STUDS UNLESS NOTED OTHERWISE.

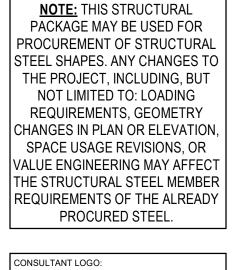
THE AISC CODE OF STANDARD PRACTICES.

- 4. SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS
- NOT SHOWN. . BEAMS ARE SPACED EQUALLY BETWEEN GRIDS UNLESS NOTED
- 6. STRUCTURAL COLD FORMED METAL STUDS SHALL BE 8" WIDE UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING
- . SEE S7.00 SERIES SHEETS FOR TYPICAL DETAILS.
- 8. SEE SHEET S6.01 FOR SCHEDULES.

OTHERWISE.

BY OTHERS.

- 9. ALL MOMENT FRAMES LABELED ON PLAN UTILIZE SIDEPLATE PROPRIETARY MOMENT CONNECTIONS. SEE S8.00 SERIES
- DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
- DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE DRAWINGS.
- 12. DIMENSIONS SHOWN ON PLAN AS FOLLOWS ARE CONCRETE COLUMN DIMENSIONS IN INCHES: 38x36, 50x36,ETC.



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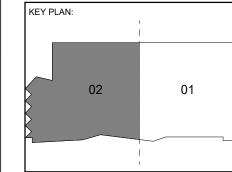
- 1. HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST, COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE B4 / S5.62, C4 / S5.62, D4 / S5.62
- 2. OPERABLE PARTITION BELOW. COORDINATE EXACT
- 3. BEAM SPLICE LOCATION. SEE B4 / S5.52 FOR SPLICE DETAIL.
- 4. HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE A4 / S5.62,
- 5. W8x31 OUTRIGGER.
- 6. TOTAL NUMBER OF CHORD REINFORCEMENT BARS AT
- 7. 3-#7 SLAB REINFORCING BARS. EXTEND BARS 130% OF A LAP SPLICE LENGTH BEYOND OPENING, OR PROVIDE STD 90 DEGREE HOOK WHERE REQUIRED.
- 8. BOTTOM FLANGE BRACING AT EQUAL SPACING, UNLESS NOTED OTHERWISE. SEE B1 / S5.52
- MAXIMUM, UNLESS NOTED OTHERWISE. SEE A1 / S5.52

SHEET KEYNOTE

- LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A5 / S5.52 FOR SUPPORT.
- B4 / S5.62, C4 / S5.62, D4 / S5.62, AND C3 / S5.62
- EXTENTS SHOWN. CHORD REINFORCEMENT SHALL BE LOCATED AS INDICATED ON PLAN. PROVIDE 130% LAP SPLICES WHEN REQUIRED.
- 9. BOTTOM FLANGE BRACING SPACED AT 10' 0" ON CENTER







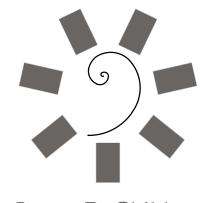
PROJECT PHASE:
BULK STEEL PACKAGE NOT FOR CONSTRUCTION

		REVISIONS
#	DATE	DESCRIPTION

03-15-19

S1.13

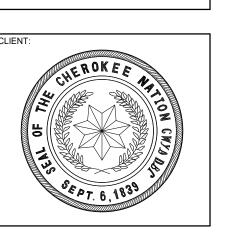
FIRST FLOOR SLAB REINFORCING PLAN -SECTOR 2



PROFESSIONAL SEAL:

NOTE: THIS STRUCTURAL
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REQUIREMENTS OF THE ALREADY
PROCURED STEEL.

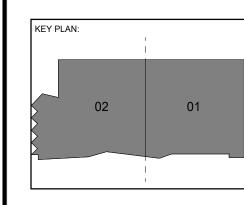




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PROJECT PHASE:

BULK STEEL PACKAGE

NOT FOR CONSTRUCTION

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		REVISIONS
#	DATE	DESCRIPTION

DATE: JOB NUMBER: 17-13

S1.20

SECOND FLOOR FRAMING PLAN -OVERALL PLAN

- 1. SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE CONCRETE WALLS AND MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
- 3. DIMENSIONS ARE TO THE FACE OF CONCRETE OR STUDS UNLESS NOTED OTHERWISE.
- 4. SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
- 5. BEAMS ARE SPACED EQUALLY BETWEEN GRIDS UNLESS NOTED
- 6. STRUCTURAL COLD FORMED METAL STUDS SHALL BE 8" WIDE UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING BY OTHERS.
- 7. SEE S7.00 SERIES SHEETS FOR TYPICAL DETAILS.
- 8. SEE SHEET S6.01 FOR SCHEDULES.

OTHERWISE.

- 9. ALL MOMENT FRAMES LABELED ON PLAN UTILIZE SIDEPLATE PROPRIETARY MOMENT CONNECTIONS. SEE S8.00 SERIES SHEETS
- DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
- 11. DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE DRAWINGS
- 12. DIMENSIONS SHOWN ON PLAN AS FOLLOWS ARE CONCRETE COLUMN DIMENSIONS IN INCHES: 38x36, 50x36,ETC.



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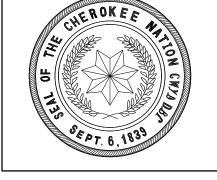
CHANGES IN PLAN OR ELEVATION, SPACE USAGE REVISIONS, OR

PACKAGE MAY BE USED FOR

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SHEET KEYNOTE

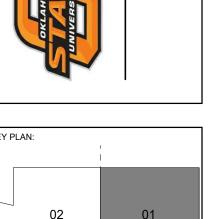
- HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST, COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE B4 / S5.62, C4 / S5.62, D4 / S5.62
- OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A5 / S5.52 FOR SUPPORT.
- 3. BEAM SPLICE LOCATION. SEE B4 / S5.52 FOR SPLICE DETAIL.
- 4. HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE A4 / S5.62, B4 / S5.62, C4 / S5.62, D4 / S5.62, AND C3 / S5.62
- 5. W8x31 OUTRIGGER.
- 6. TOTAL NUMBER OF CHORD REINFORCEMENT BARS AT EXTENTS SHOWN. CHORD REINFORCEMENT SHALL BE LOCATED AS INDICATED ON PLAN. PROVIDE 130% LAP SPLICES WHEN REQUIRED.
- 7. 3-#7 SLAB REINFORCING BARS. EXTEND BARS 130% OF A LAP SPLICE LENGTH BEYOND OPENING, OR PROVIDE STD 90 DEGREE HOOK WHERE REQUIRED.
- . BOTTOM FLANGE BRACING AT EQUAL SPACING, UNLESS NOTED OTHERWISE. SEE B1 / S5.52
- 9. BOTTOM FLANGE BRACING SPACED AT 10' 0" ON CENTER MAXIMUM, UNLESS NOTED OTHERWISE. SEE A1 / S5.52



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PROJECT PHASE:

BULK STEEL PACKAGE
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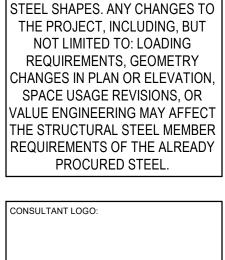
		REVISIONS
#	DATE	DESCRIPTION

DATE: JOB NUMBER: 17-13

S1.21

SECOND FLOOR FRAMING PLAN -SECTOR 1

- SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- 2. NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE CONCRETE WALLS AND MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
- 3. DIMENSIONS ARE TO THE FACE OF CONCRETE OR STUDS UNLESS NOTED OTHERWISE.
- 4. SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
- BEAMS ARE SPACED EQUALLY BETWEEN GRIDS UNLESS NOTED OTHERWISE.
- 6. STRUCTURAL COLD FORMED METAL STUDS SHALL BE 8" WIDE UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING BY OTHERS.
- . SEE S7.00 SERIES SHEETS FOR TYPICAL DETAILS.
- 8. SEE SHEET S6.01 FOR SCHEDULES.
- 9. ALL MOMENT FRAMES LABELED ON PLAN UTILIZE SIDEPLATE PROPRIETARY MOMENT CONNECTIONS. SEE S8.00 SERIES SHEETS
- DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
- DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE DRAWINGS.
- 12. DIMENSIONS SHOWN ON PLAN AS FOLLOWS ARE CONCRETE COLUMN DIMENSIONS IN INCHES: 38x36, 50x36,ETC.



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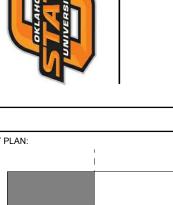


- . HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST, COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE B4 / S5.62, C4 / S5.62, D4 / S5.62
- OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A5 / S5.52 FOR SUPPORT.
- 3. BEAM SPLICE LOCATION. SEE B4 / S5.52 FOR SPLICE DETAIL.
- 4. HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE A4 / S5.62, B4 / S5.62, C4 / S5.62, D4 / S5.62, AND C3 / S5.62
- 5. W8x31 OUTRIGGER.
- 6. TOTAL NUMBER OF CHORD REINFORCEMENT BARS AT EXTENTS SHOWN. CHORD REINFORCEMENT SHALL BE LOCATED AS INDICATED ON PLAN. PROVIDE 130% LAP SPLICES WHEN REQUIRED.
- 3-#7 SLAB REINFORCING BARS. EXTEND BARS 130% OF A LAP SPLICE LENGTH BEYOND OPENING, OR PROVIDE STD 90 DEGREE HOOK WHERE REQUIRED.
- . BOTTOM FLANGE BRACING AT EQUAL SPACING, UNLESS NOTED OTHERWISE. SEE B1 / S5.52
- . BOTTOM FLANGE BRACING SPACED AT 10' 0" ON CENTER MAXIMUM, UNLESS NOTED OTHERWISE. SEE A1 / S5.52



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PROJECT PHASE:

BULK STEEL PACKAGE

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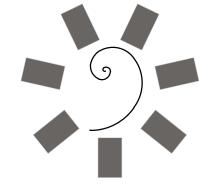
		REVISIONS
#	DATE	DESCRIPTION

DATE: JOB NUMBER: 17-13

SHEET NUMBER:

S1.22

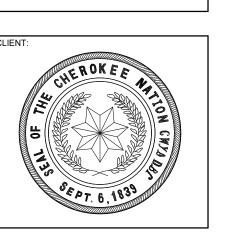
SECOND FLOOR FRAMING PLAN -SECTOR 2



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REQUIREMENTS OF THE ALREADY
PROCURED STEEL.

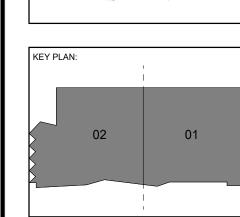




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PROJECT PHASE:

BULK STEEL PACKAGE

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		REVISIONS
#	DATE	DESCRIPTION

DATE: JOB NUMBER: 17-13

SHEET NUMBER:

THIRD FLOOR FRAMING PLAN - OVERALL PLAN

S1.30

- 1. SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE CONCRETE WALLS AND MOMENT

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THE AISC CODE OF STANDARD PRACTICES.

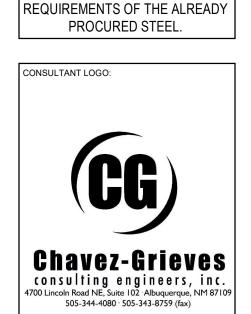
3. DIMENSIONS ARE TO THE FACE OF CONCRETE OR STUDS

UNLESS NOTED OTHERWISE.

- 4. SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
- 5. BEAMS ARE SPACED EQUALLY BETWEEN GRIDS UNLESS NOTE: OTHERWISE.
- 6. STRUCTURAL COLD FORMED METAL STUDS SHALL BE 8" WIDE UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING
- . SEE S7.00 SERIES SHEETS FOR TYPICAL DETAILS.
- 8. SEE SHEET S6.01 FOR SCHEDULES.

BY OTHERS.

- 9. ALL MOMENT FRAMES LABELED ON PLAN UTILIZE SIDEPLATE PROPRIETARY MOMENT CONNECTIONS. SEE S8.00 SERIES SHEETS
- DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
- DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE DRAWINGS.
- 12. DIMENSIONS SHOWN ON PLAN AS FOLLOWS ARE CONCRETE COLUMN DIMENSIONS IN INCHES: 38x36, 50x36,ETC.



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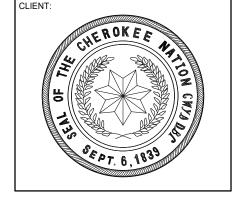
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VALUE ENGINEERING MAY AFFECT THE STRUCTURAL STEEL MEMBER

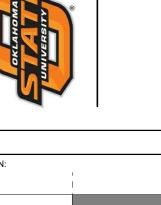
SHEET KEYNOTE

- 1. HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST, COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE B4 / S5.62, C4 / S5.62, D4 / S5.62
- OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A5 / S5.52 FOR SUPPORT.
- BEAM SPLICE LOCATION. SEE B4 / S5.52 FOR SPLICE DETAIL
- 4. HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE A4 / S5.62, B4 / S5.62, C4 / S5.62, D4 / S5.62, AND C3 / S5.62
- 5. W8x31 OUTRIGGER.
- 6. TOTAL NUMBER OF CHORD REINFORCEMENT BARS AT EXTENTS SHOWN. CHORD REINFORCEMENT SHALL BE LOCATED AS INDICATED ON PLAN. PROVIDE 130% LAP SPLICES WHEN REQUIRED.
- 7. 3-#7 SLAB REINFORCING BARS. EXTEND BARS 130% OF A LAP SPLICE LENGTH BEYOND OPENING, OR PROVIDE STD 90 DEGREE HOOK WHERE REQUIRED.
- 8. BOTTOM FLANGE BRACING AT EQUAL SPACING, UNLESS NOTED OTHERWISE. SEE B1 / S5.52
- BOTTOM FLANGE BRACING SPACED AT 10' 0" ON CENTER MAXIMUM, UNLESS NOTED OTHERWISE. SEE A1 / S5.52



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PROJECT PHASE:

BULK STEEL PACKAGE NOT FOR CONSTRUCTION

		REVISIONS
#	DATE	DESCRIPTION

DATE: JOB NUMBER: 17-13
SHEET NUMBER:

S1.31

THIRD FLOOR FRAMING PLAN - SECTOR 1

- 1. SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE CONCRETE WALLS AND MOMENT
- OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
 - UNLESS NOTED OTHERWISE.

 4. SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS

DIMENSIONS ARE TO THE FACE OF CONCRETE OR STUDS

FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING

- NOT SHOWN.

 5. BEAMS ARE SPACED EQUALLY BETWEEN GRIDS UNLESS NOTE:
- 6. STRUCTURAL COLD FORMED METAL STUDS SHALL BE 8" WIDE UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING BY OTHERS.
- . SEE S7.00 SERIES SHEETS FOR TYPICAL DETAILS.
- 8. SEE SHEET S6.01 FOR SCHEDULES.

OTHERWISE.

- 9. ALL MOMENT FRAMES LABELED ON PLAN UTILIZE SIDEPLATE PROPRIETARY MOMENT CONNECTIONS. SEE S8.00 SERIES SHEETS
- DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
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- 12. DIMENSIONS SHOWN ON PLAN AS FOLLOWS ARE CONCRETE COLUMN DIMENSIONS IN INCHES: 38x36, 50x36,ETC.



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SHEET KEYNOTE

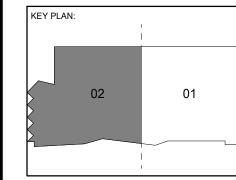
- . HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST, COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE B4 / S5.62, C4 / S5.62, D4 / S5.62
- OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A5 / S5.52 FOR SUPPORT.
- 3. BEAM SPLICE LOCATION. SEE B4 / S5.52 FOR SPLICE DETAIL.
- 4. HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE A4 / S5.62, B4 / S5.62, C4 / S5.62, D4 / S5.62, AND C3 / S5.62
- 5. W8x31 OUTRIGGER.
- 6. TOTAL NUMBER OF CHORD REINFORCEMENT BARS AT EXTENTS SHOWN. CHORD REINFORCEMENT SHALL BE LOCATED AS INDICATED ON PLAN. PROVIDE 130% LAP SPLICES WHEN REQUIRED.
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- BOTTOM FLANGE BRACING SPACED AT 10' 0" ON CENTER MAXIMUM, UNLESS NOTED OTHERWISE. SEE A1 / S5.52



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	l	PROJECT PHASE:
		BULK STEEL PACKAGE NOT FOR CONSTRUCTION
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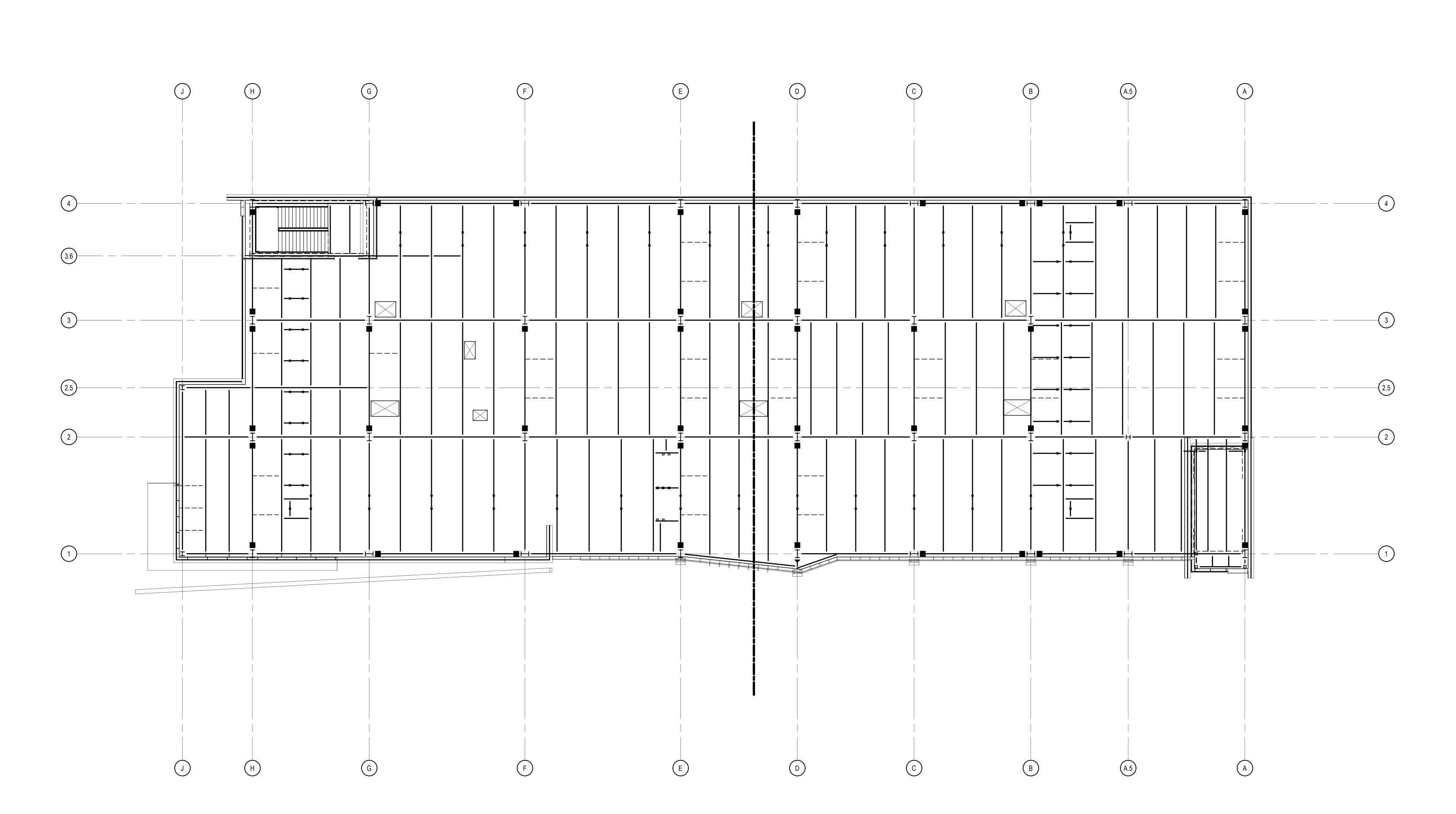
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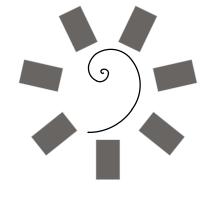
DATE: JOB NUMBER: 17-13

SHEET NUMBER:

S1.32

THIRD FLOOR FRAMING PLAN - SECTOR 2

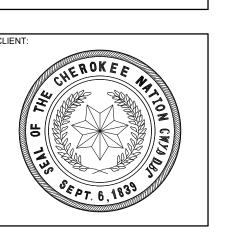




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REQUIREMENTS, GEOMETRY
CHANGES IN PLAN OR ELEVATION,
SPACE USAGE REVISIONS, OR
VALUE ENGINEERING MAY AFFECT
THE STRUCTURAL STEEL MEMBER
REQUIREMENTS OF THE ALREADY
PROCURED STEEL.

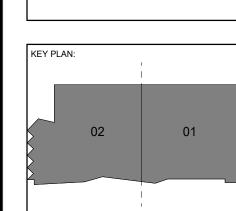


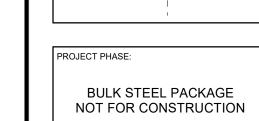












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#	DATE	DESCRIPTION
	#	# DATE

DATE:	JOB NUMBER:
03-15-19	17-13
SHEET NUMBER:	

ROOF FRAMING PLAN -OVERALL PLAN

S1.40

- . SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- 2. NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE CONCRETE WALLS AND MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
- DIMENSIONS ARE TO THE FACE OF CONCRETE OR STUDS UNLESS NOTED OTHERWISE.
- 4. SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
- 5. BEAMS ARE SPACED EQUALLY BETWEEN GRIDS UNLESS NOTED OTHERWISE.
- 5. STRUCTURAL COLD FORMED METAL STUDS SHALL BE 8" WIDE UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING
- 7. SEE S7.00 SERIES SHEETS FOR TYPICAL DETAILS.
- 8. SEE SHEET S6.01 FOR SCHEDULES.

BY OTHERS.

- DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
- 10. DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE



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NOTE: THIS STRUCTURAL

PROCUREMENT OF STRUCTURAL

STEEL SHAPES. ANY CHANGES TO

THE PROJECT, INCLUDING, BUT

NOT LIMITED TO: LOADING

REQUIREMENTS, GEOMETRY CHANGES IN PLAN OR ELEVATION,

SPACE USAGE REVISIONS, OR VALUE ENGINEERING MAY AFFECT

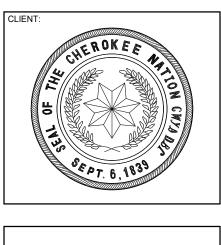
THE STRUCTURAL STEEL MEMBER REQUIREMENTS OF THE ALREADY PROCURED STEEL.

PACKAGE MAY BE USED FOR

PROFESSIONAL SEAL:

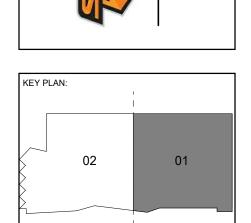
SHEET KEYNOTE

- 1. MECHANICAL UNIT, COORDINATE EXACT SIZE AND LOCATION WITH MECHANICAL DRAWINGS.
- MECHANICAL OPENING. COORDINATE EXACT SIZE AND LOCATION WITH MECHANICAL DRAWINGS. SEE B2 / S7.41 AND C2 / S7.41 FOR TYPICAL FRAMING.
- 3. HSS6x4x3/16 EXTERIOR CLADDING SUPPORT.
- 4. OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A4 / S5.62, B4 / S5.62, C4 / S5.62, AND D4 / S5.62 FOR SUPPORT.
- 5. HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE A4 / S5.62, B4 / S5.62, C4 / S5.62, AND D4 / S5.62
- 6. HSS8x2x3/16 BETWEEN OUTRIGGERS.
- 7. W12x26.
- 8. BOTTOM FLANGE BRACING AT EQUAL SPACING, UNLESS NOTED OTHERWISE ON PLAN. SEE B1 / S5.52
- OTHERWISE ON PLAN A1 / S5.52
- 10. 2-4x4x1/4 BRACE TO CHANNEL FRAMING BELOW.
- 11. HSS4x4x1/4 BRACE BELOW.



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PF	ROJECT PHASE:
	BULK STEEL PACKAGE NOT FOR CONSTRUCTION

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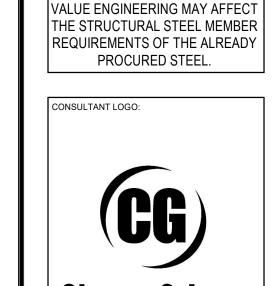
03-15-19 17-13

SHEET NUMBER:

S1.41

ROOF FRAMING PLAN -SECTOR 1

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 - STRUCTURAL COLD FORMED METAL STUDS SHALL BE 8" WIDE UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING BY OTHERS.
 - 7. SEE S7.00 SERIES SHEETS FOR TYPICAL DETAILS.
 - 8. SEE SHEET S6.01 FOR SCHEDULES.
 - DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
 - DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE DRAWINGS.



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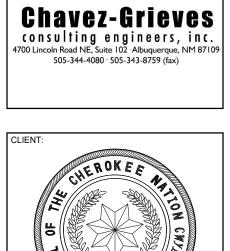
SPACE USAGE REVISIONS, OR

THE PROJECT, INCLUDING, BUT

PROFESSIONAL SEAL:

SHEET KEYNOTE

- MECHANICAL UNIT, COORDINATE EXACT SIZE AND LOCATION WITH MECHANICAL DRAWINGS.
- MECHANICAL OPENING. COORDINATE EXACT SIZE AND LOCATION WITH MECHANICAL DRAWINGS. SEE B2 / S7.41 AND C2 / S7.41 FOR TYPICAL FRAMING.
- 3. HSS6x4x3/16 EXTERIOR CLADDING SUPPORT.
- 4. OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A4 / S5.62, B4 / S5.62, C4 / S5.62, AND D4 / S5.62 FOR SUPPORT.
- HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE A4 / S5.62, B4 / S5.62, C4 / S5.62, AND D4 / S5.62
- 6. HSS8x2x3/16 BETWEEN OUTRIGGERS.
- 7. W12x26.
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- BOTTOM FLANGE BRACING AT EACH BEAM, UNLESS NOTED OTHERWISE ON PLAN A1 / S5.52
- 10. 2-4x4x1/4 BRACE TO CHANNEL FRAMING BELOW.
- 11. HSS4x4x1/4 BRACE BELOW.



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PROJECT PHASE:

BULK STEEL PACKAGE

NOT FOR CONSTRUCTION

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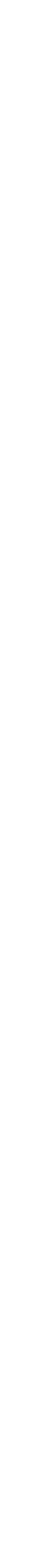
03-15-19 17-13

SHEET NUMBER:

S1.42

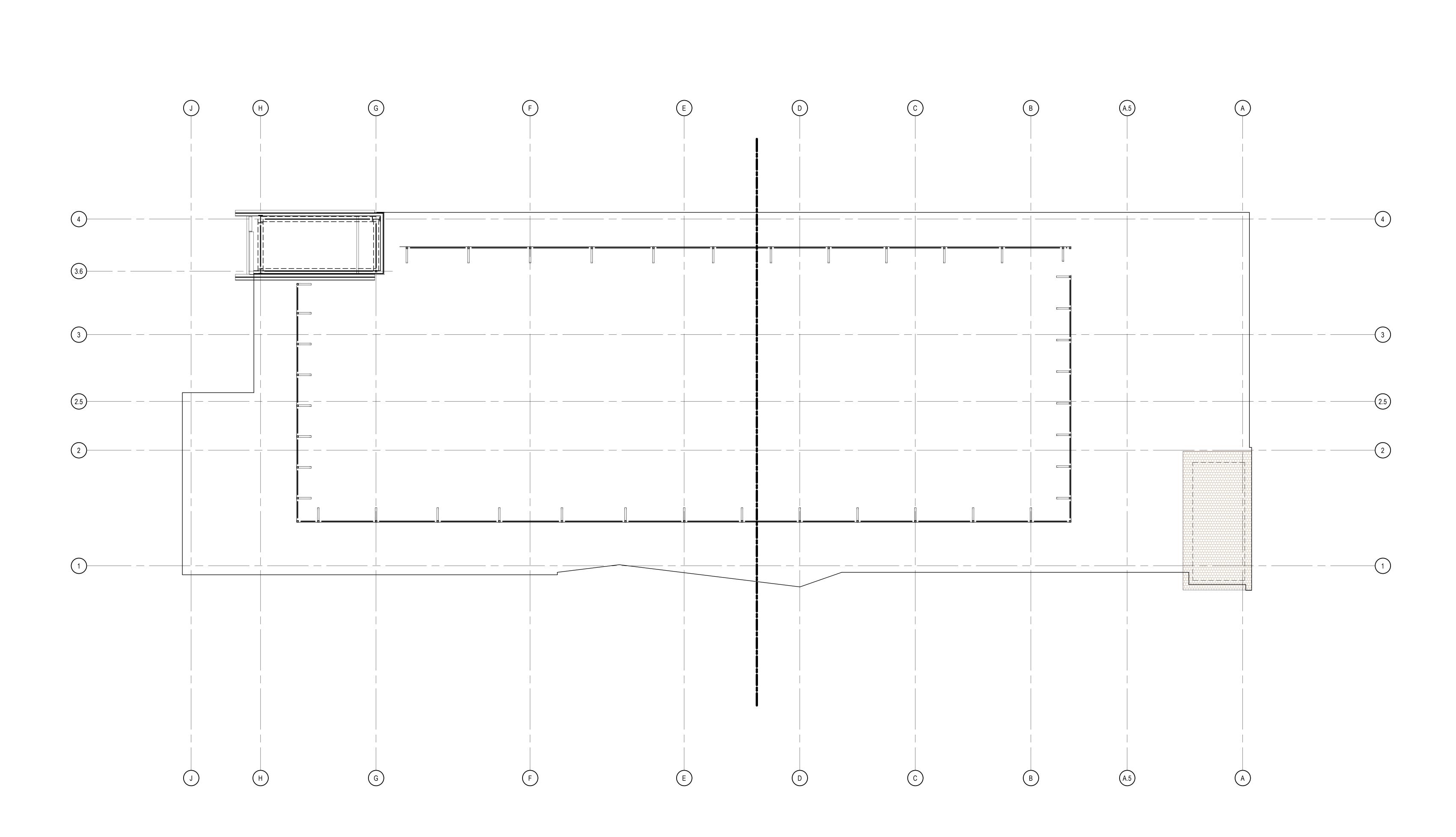
ROOF FRAMING PLAN -SECTOR 2

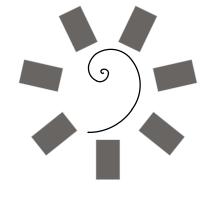
ROOF FRAMING PLAN - SECTOR 2



1/2"=1'-0"

3/8"=1-0"



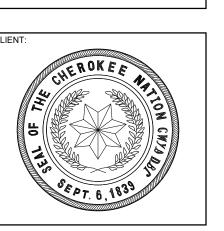


PROFESSIONAL SEAL:

NOTE: THIS STRUCTURAL

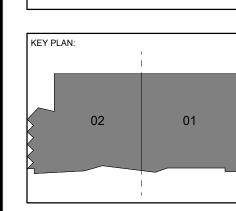
PACKAGE MAY BE USED FOR PROCUREMENT OF STRUCTURAL STEEL SHAPES. ANY CHANGES TO THE PROJECT, INCLUDING, BUT NOT LIMITED TO: LOADING REQUIREMENTS, GEOMETRY CHANGES IN PLAN OR ELEVATION, SPACE USAGE REVISIONS, OR VALUE ENGINEERING MAY AFFECT THE STRUCTURAL STEEL MEMBER REQUIREMENTS OF THE ALREADY PROCURED STEEL.





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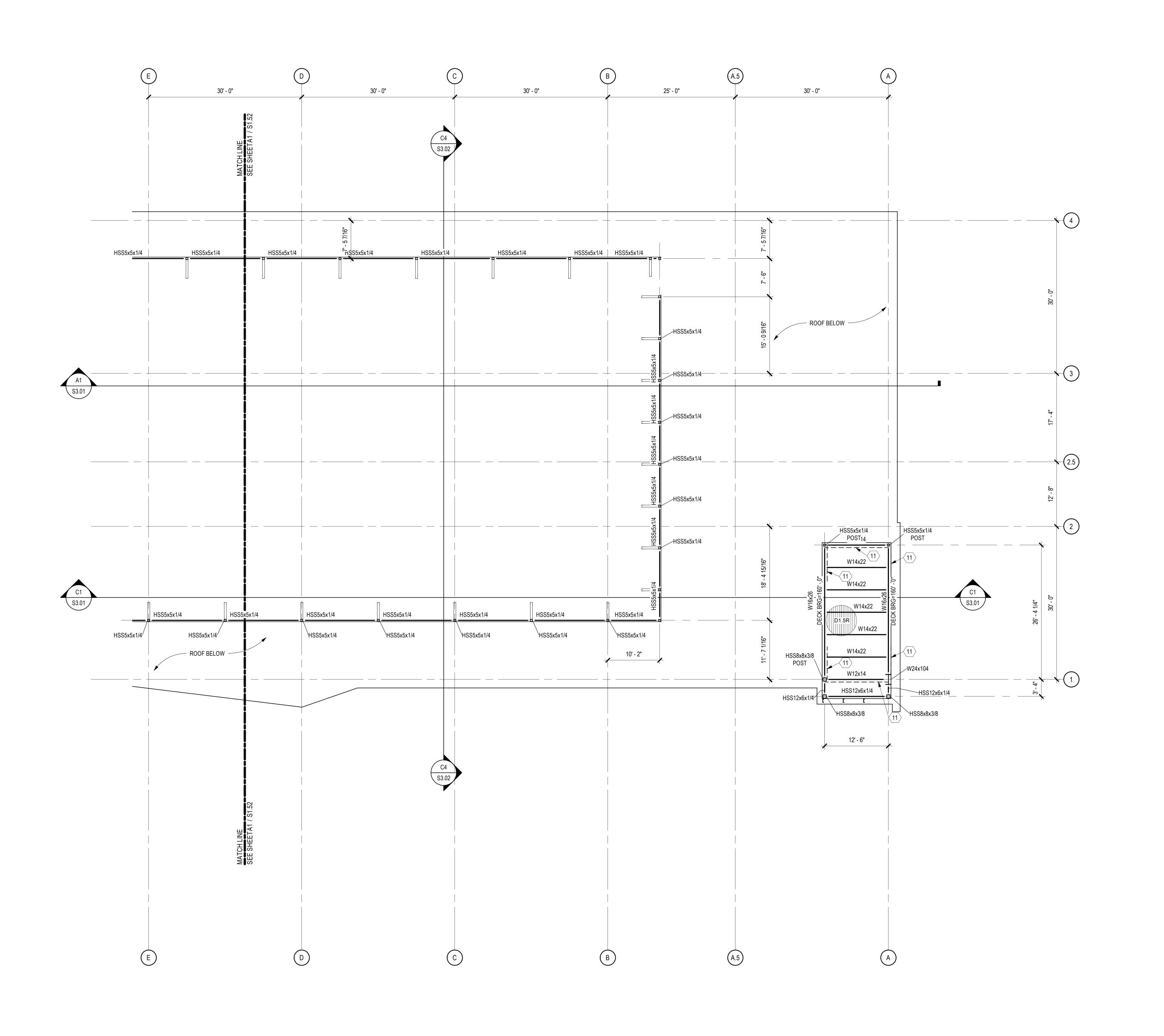


	PROJECT PHASE:
	BULK STEEL PACKAGE NOT FOR CONSTRUCTION

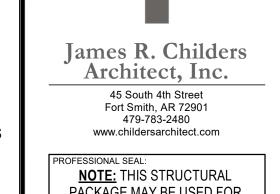
		REVISIONS
#	DATE	DESCRIPTION

DATE:	JOB NUMBER:
03-15-19	17-13
SHEET NUMBER:	
S	1.50

HIGH ROOF FRAMING PLAN - OVERALL PLAN



- . SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- . NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE CONCRETE WALLS AND MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
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- 7. SEE S7.00 SERIES SHEETS FOR TYPICAL DETAILS.
- 8. SEE SHEET S6.01 FOR SCHEDULES.
- DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
- DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE DRAWINGS.



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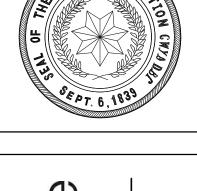


MECHANICAL UNIT, COORDINATE EXACT SIZE AND LOCATION

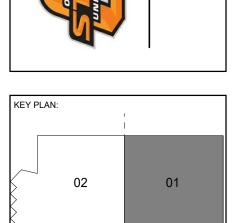
WITH MECHANICAL DRAWINGS.

SHEET KEYNOTE

- MECHANICAL OPENING. COORDINATE EXACT SIZE AND LOCATION WITH MECHANICAL DRAWINGS. SEE B2 / S7.41 AND C2 / S7.41 FOR TYPICAL FRAMING.
- HSS6x4x3/16 EXTERIOR CLADDING SUPPORT.
- 4. OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A4 / S5.62, B4 / S5.62, C4 / S5.62, AND D4 / S5.62 FOR SUPPORT.
- HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE A4 / S5.62, B4 / S5.62, C4 / S5.62, AND D4 / S5.62
- 6. HSS8x2x3/16 BETWEEN OUTRIGGERS.
- . W12x26.
- BOTTOM FLANGE BRACING AT EQUAL SPACING, UNLESS NOTED OTHERWISE ON PLAN. SEE B1 / S5.52
- 9. BOTTOM FLANGE BRACING AT EACH BEAM, UNLESS NOTED OTHERWISE ON PLAN A1 / S5.52
- 10. 2-4x4x1/4 BRACE TO CHANNEL FRAMING BELOW.
- 11. HSS4x4x1/4 BRACE BELOW.







PROJECT PHASE: BULK STEEL PACKAGE NOT FOR CONSTRUCTION

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#	DATE	DESCRIPTION		

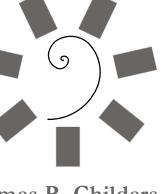
03-15-19 S1.51

HIGH ROOF FRAMING PLAN - SECTOR 1

HIGH ROOF FRAMING PLAN - SECTOR 1

SCALE: 1/8" = 1'-0"

- . SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
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- DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
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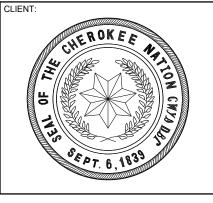


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- MECHANICAL UNIT, COORDINATE EXACT SIZE AND LOCATION WITH MECHANICAL DRAWINGS.
- MECHANICAL OPENING. COORDINATE EXACT SIZE AND AND C2 / S7.41 FOR TYPICAL FRAMING.
- 3. HSS6x4x3/16 EXTERIOR CLADDING SUPPORT.
- 4. OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A4 / S5.62,
- 5. HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE A4 / S5.62, B4 / S5.62 , C4 / S5.62 , AND D4 / S5.62
- 6. HSS8x2x3/16 BETWEEN OUTRIGGERS.
- 7. W12x26.
- 9. BOTTOM FLANGE BRACING AT EACH BEAM, UNLESS NOTED OTHERWISE ON PLAN A1 / S5.52
- 10. 2-4x4x1/4 BRACE TO CHANNEL FRAMING BELOW.
- 11. HSS4x4x1/4 BRACE BELOW.

SHEET KEYNOTE

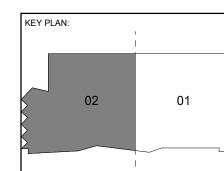
- LOCATION WITH MECHANICAL DRAWINGS. SEE B2 / S7.41
- B4 / S5.62, C4 / S5.62, AND D4 / S5.62 FOR SUPPORT.

- 8. BOTTOM FLANGE BRACING AT EQUAL SPACING, UNLESS NOTED OTHERWISE ON PLAN. SEE B1 / S5.52









PROJECT PHASE:
BULK STEEL PACKAGE
NOT FOR CONSTRUCTIO

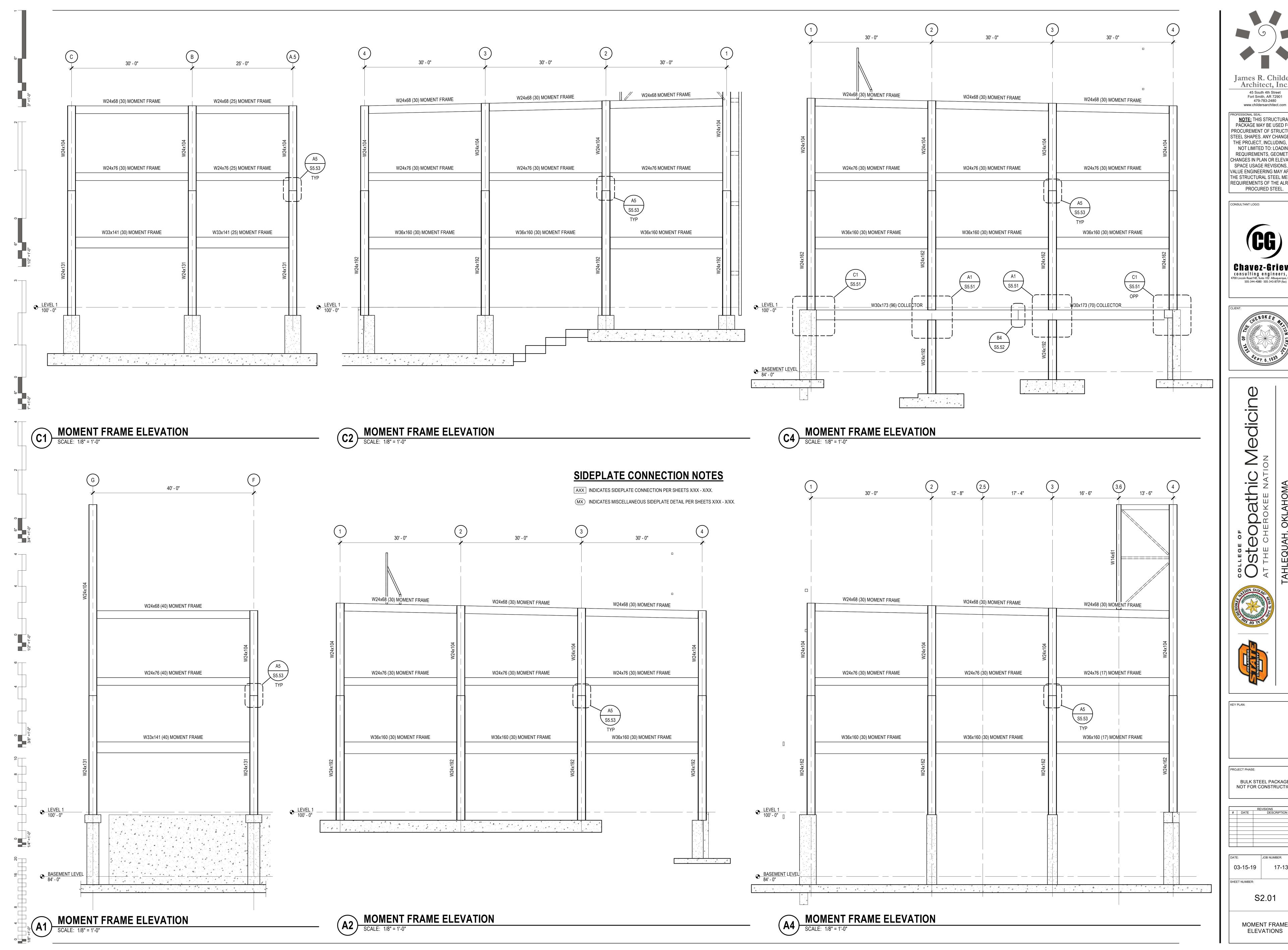
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HIGH ROOF FRAMING PLAN - SECTOR 2

HIGH ROOF FRAMING PLAN - SECTOR 2

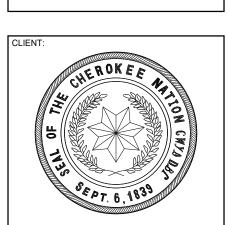
SCALE: 1/8" = 1'-0"



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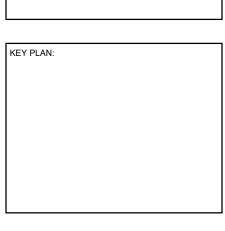
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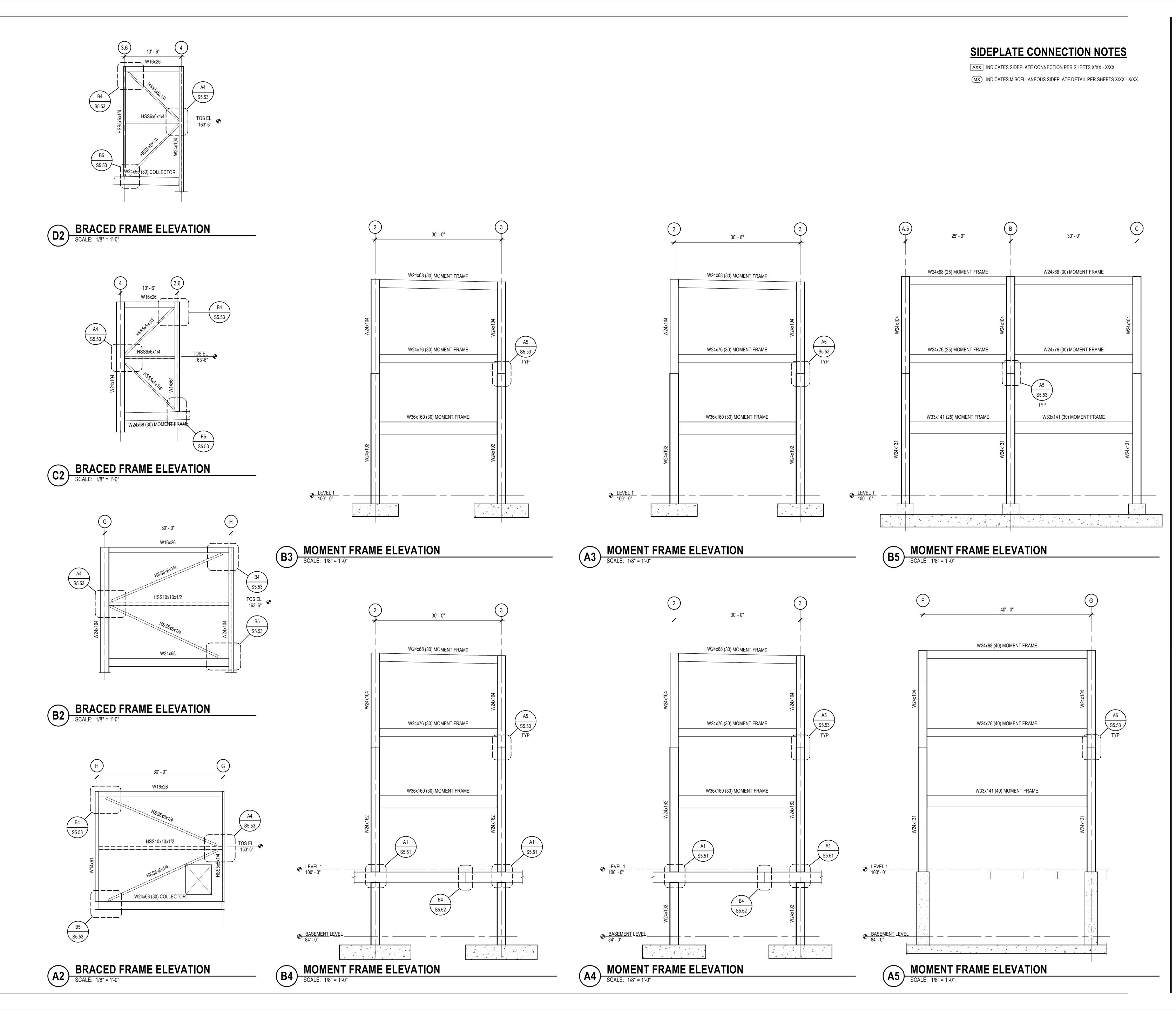


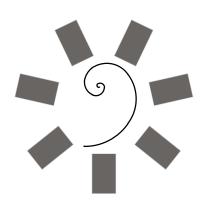


BULK STEEL PACKAGE NOT FOR CONSTRUCTION

03-15-19

MOMENT FRAME **ELEVATIONS**





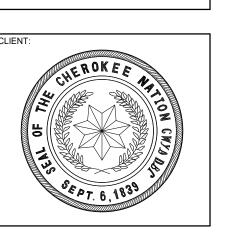
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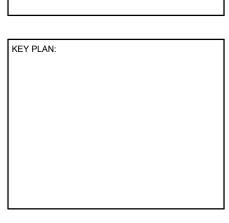


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PROJECT PHASE:

BULK STEEL PACKAGE
NOT FOR CONSTRUCTION

DATE DESCRIPTION

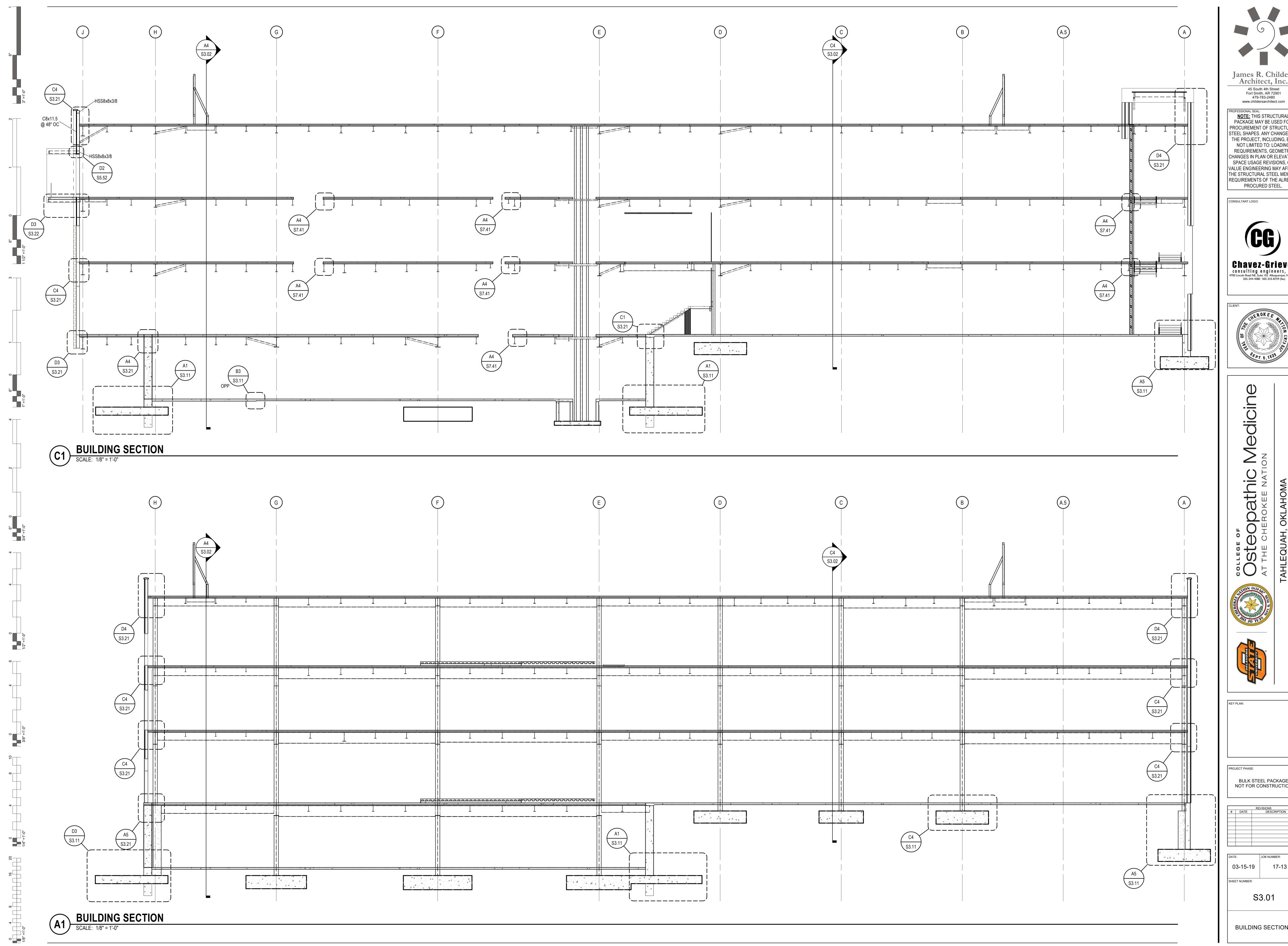
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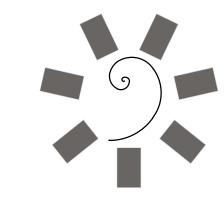
03-15-19 17-13

SHEET NUMBER:

S2.02

MOMENT FRAME AND BRACED FRAME ELEVATIONS

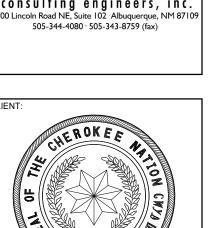


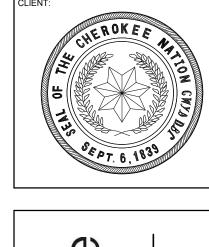


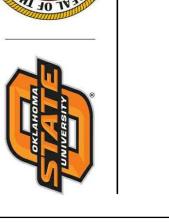
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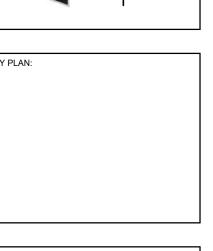
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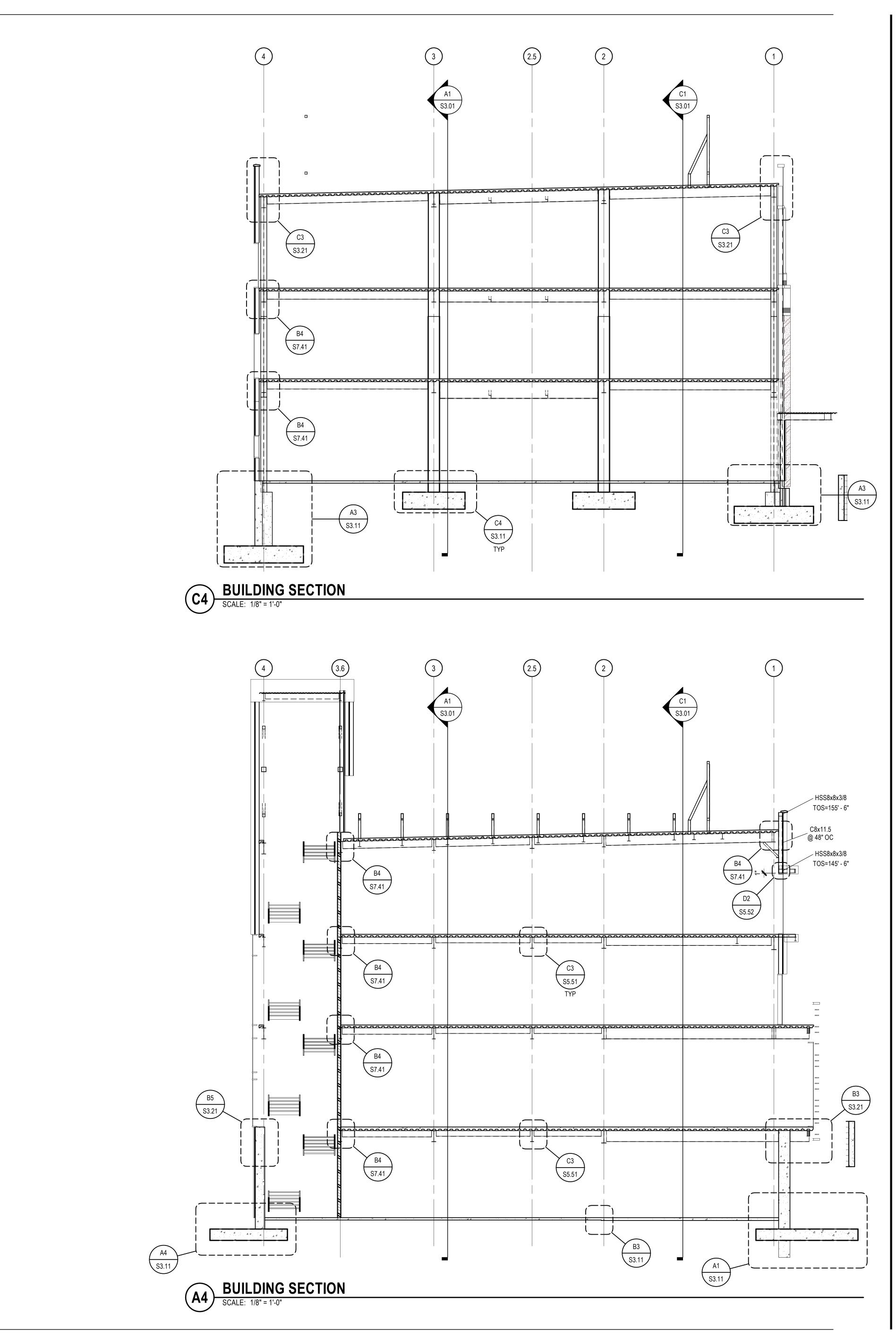


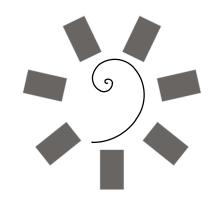


BULK STEEL PACKAGE NOT FOR CONSTRUCTION

S3.01

BUILDING SECTIONS





PROFESSIONAL SEAL:

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CHANGES IN PLAN OR ELEVATION,

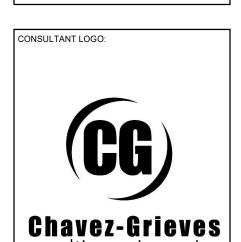
SPACE USAGE REVISIONS, OR

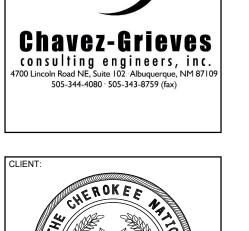
VALUE ENGINEERING MAY AFFECT

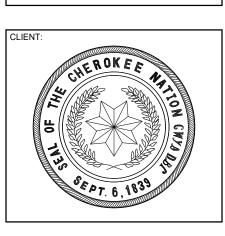
THE STRUCTURAL STEEL MEMBER

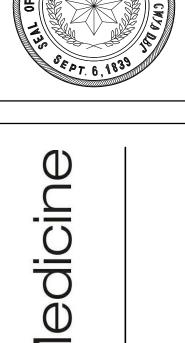
REQUIREMENTS OF THE ALREADY

PROCURED STEEL.



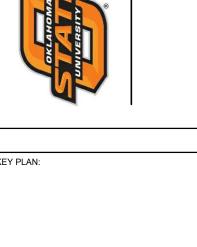


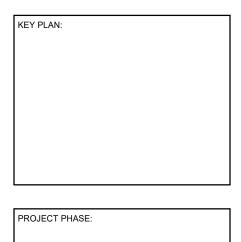












PROJECT PHASE:

BULK STEEL PACKAGE

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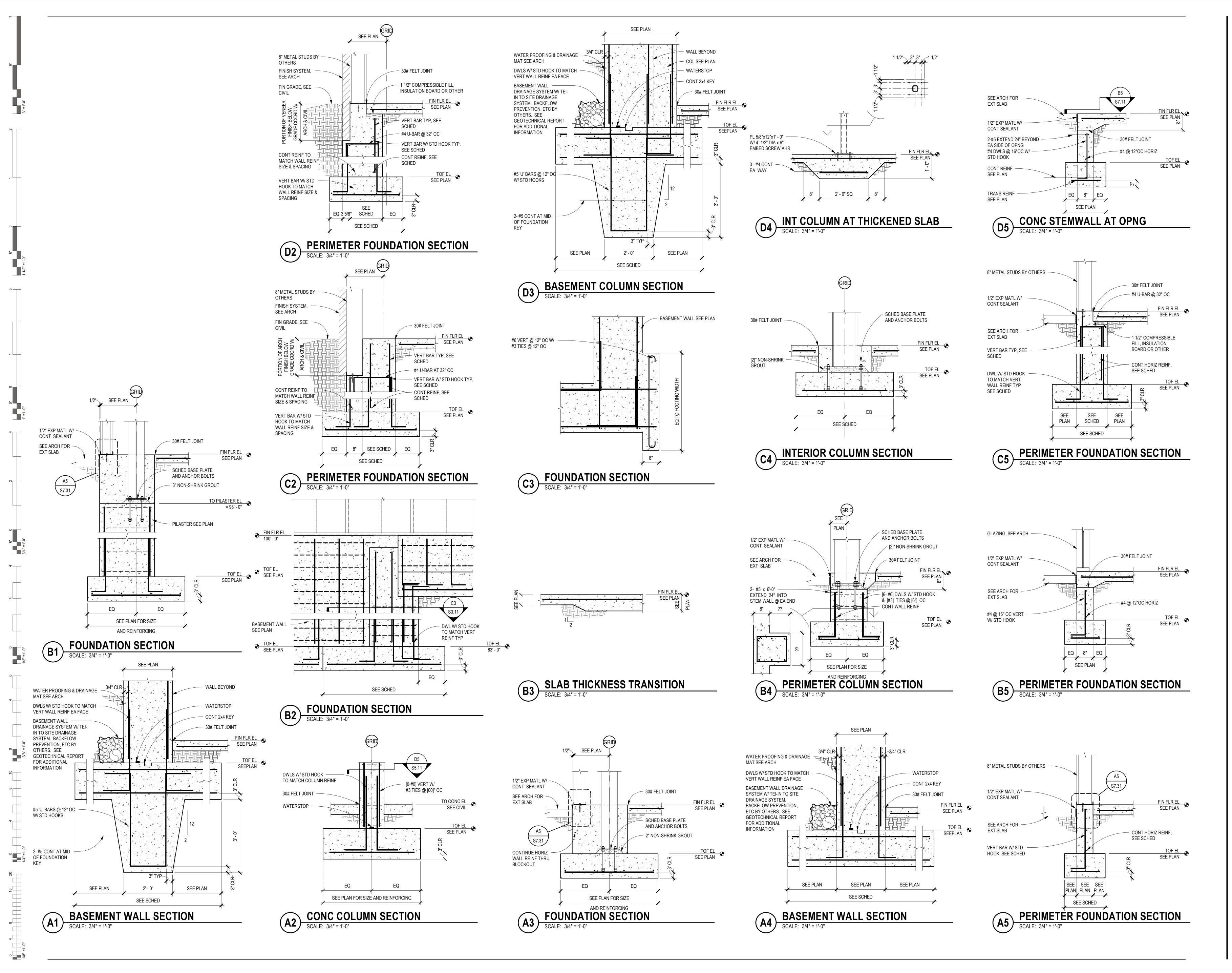
REVISIONS
DATE DESCRIPTION

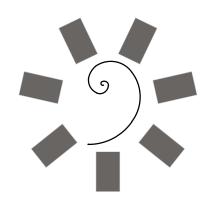
DATE: JOB NUMBER: 17-13

SHEET NUMBER:

S3.02

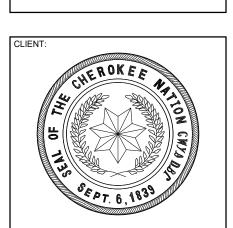
BUILDING SECTIONS

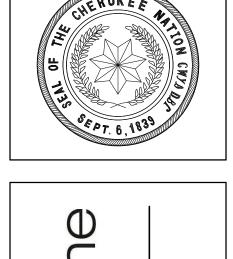




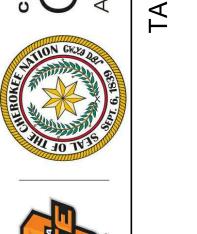
PROFESSIONAL SEAL **NOTE:** THIS STRUCTURAL PACKAGE MAY BE USED FOR PROCUREMENT OF STRUCTURA STEEL SHAPES. ANY CHANGES T THE PROJECT, INCLUDING, BUT NOT LIMITED TO: LOADING REQUIREMENTS, GEOMETRY CHANGES IN PLAN OR ELEVATION SPACE USAGE REVISIONS, OR VALUE ENGINEERING MAY AFFEC THE STRUCTURAL STEEL MEMBER REQUIREMENTS OF THE ALREADY PROCURED STEEL.

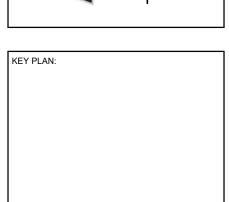


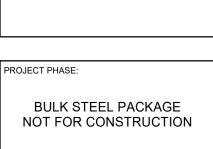










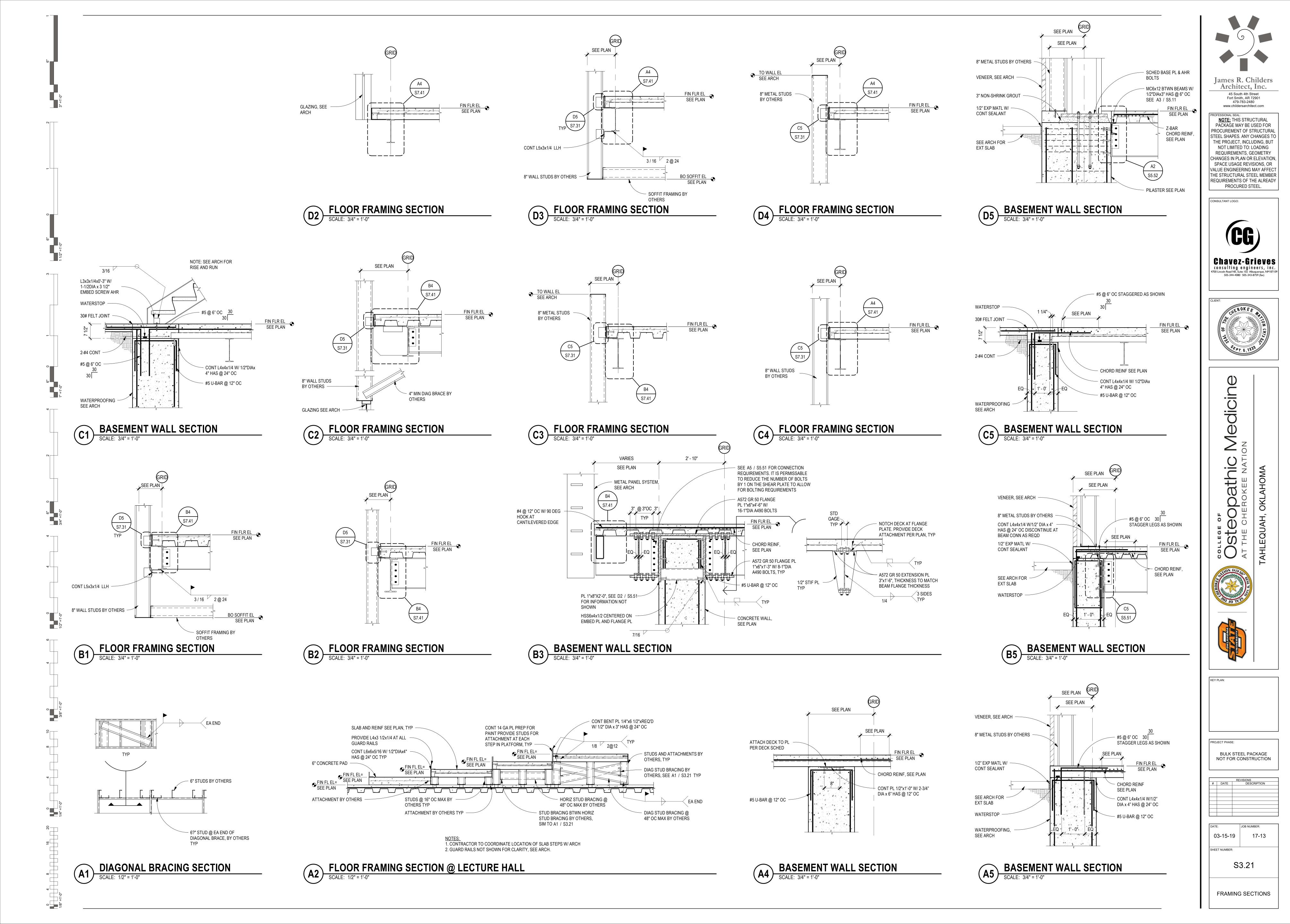


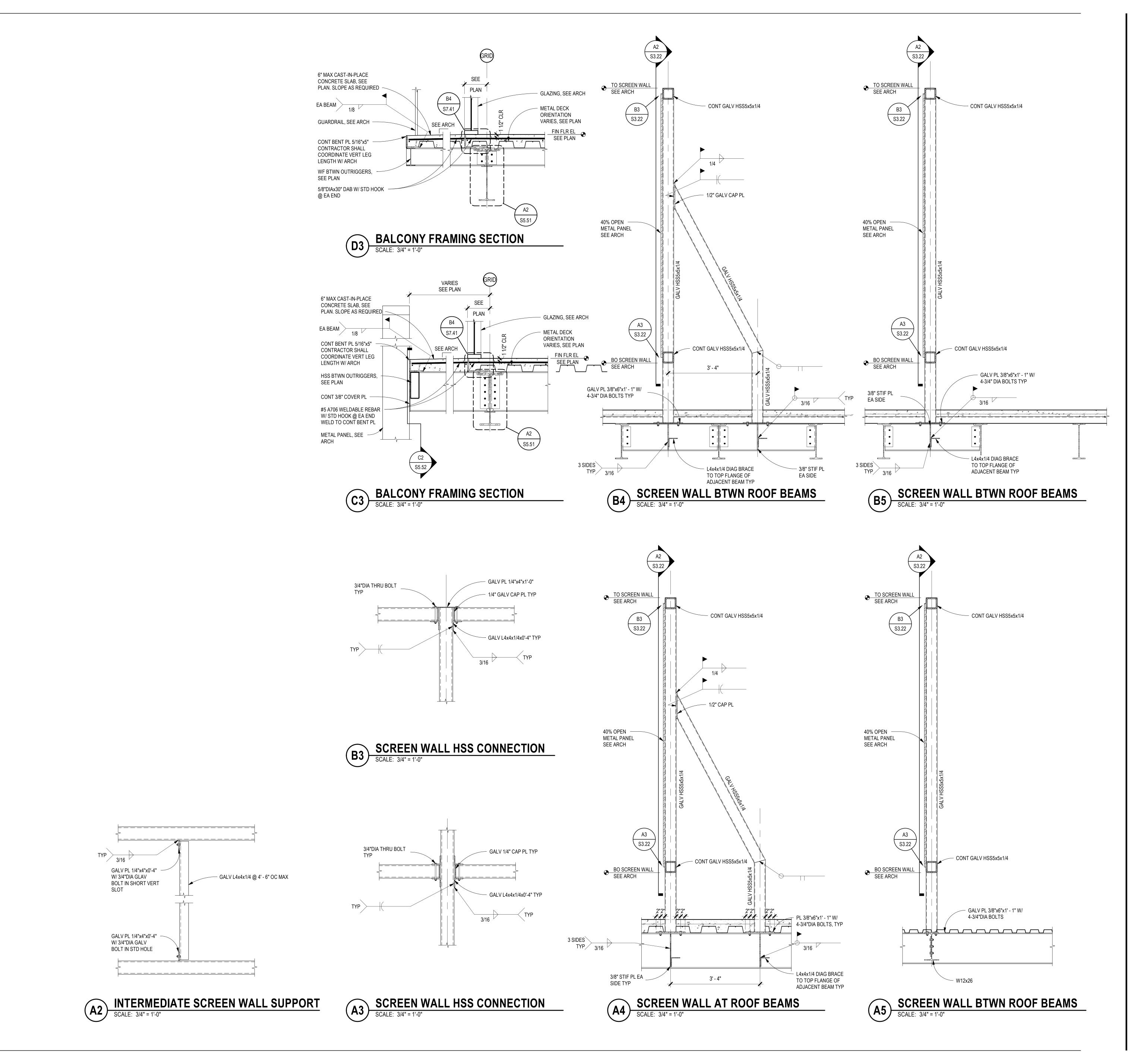
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DATE DESCRIPTION

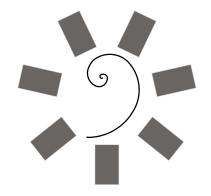
03-15-19 SHEET NUMBER:

S3.11

FOUNDATION SECTIONS







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STEEL SHAPES. ANY CHANGES TO

THE PROJECT, INCLUDING, BUT

NOT LIMITED TO: LOADING

REQUIREMENTS, GEOMETRY

CHANGES IN PLAN OR ELEVATION,

SPACE USAGE REVISIONS, OR

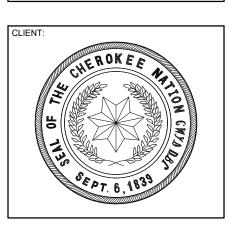
VALUE ENGINEERING MAY AFFECT

THE STRUCTURAL STEEL MEMBER

REQUIREMENTS OF THE ALREADY

PROCURED STEEL.

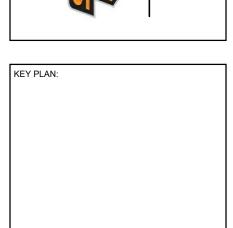




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TAHIFOLIAH OKLAHOMA





PROJECT PHASE:

BULK STEEL PACKAGE

NOT FOR CONSTRUCTION

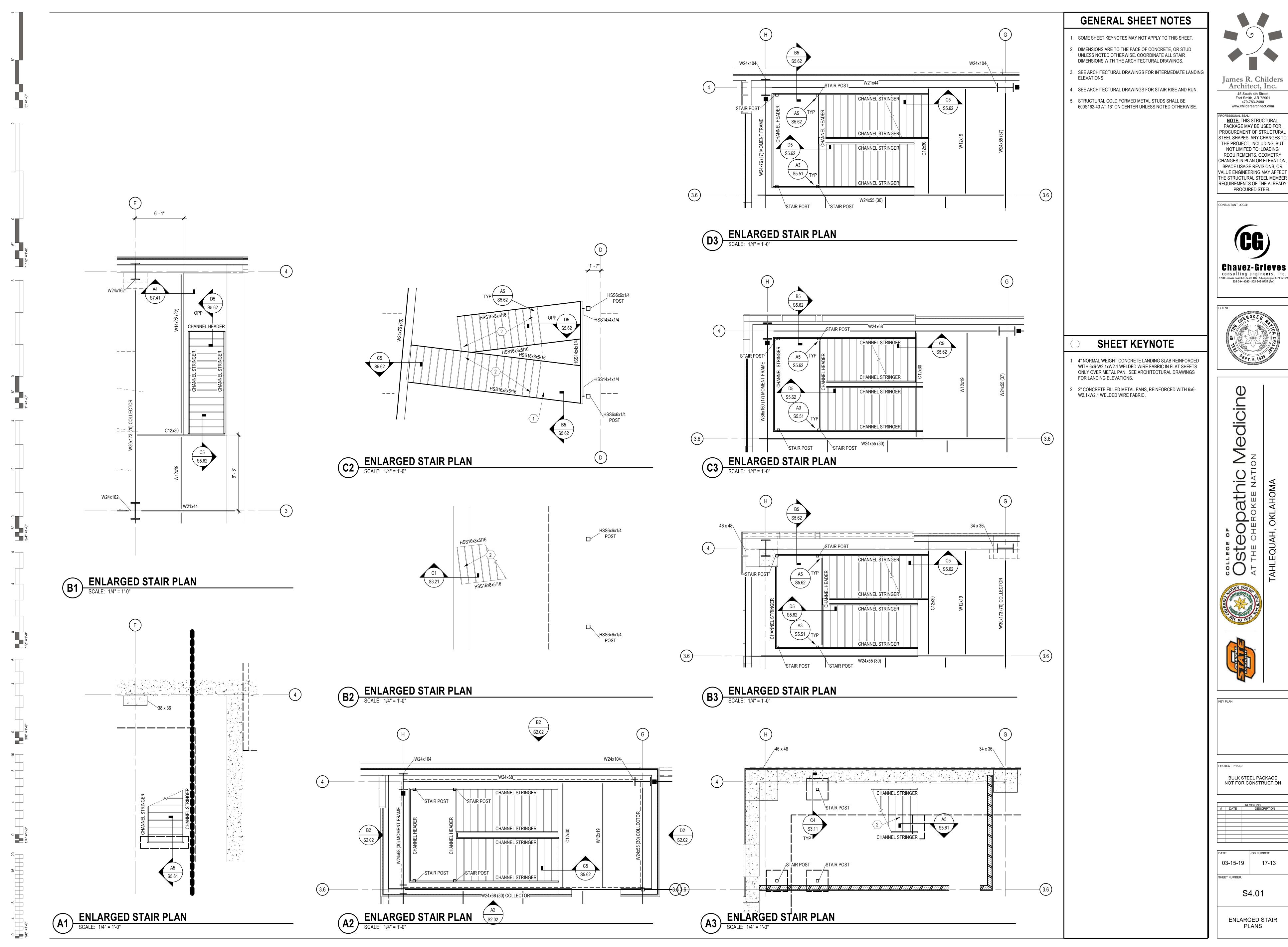
REVISIONS
DATE DESCRIPTION

DATE: JOB NUMBER: 17-13

SHEET NUMBER:

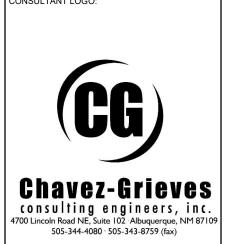
S3.22

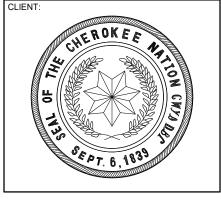
FRAMING SECTIONS



James R. Childers

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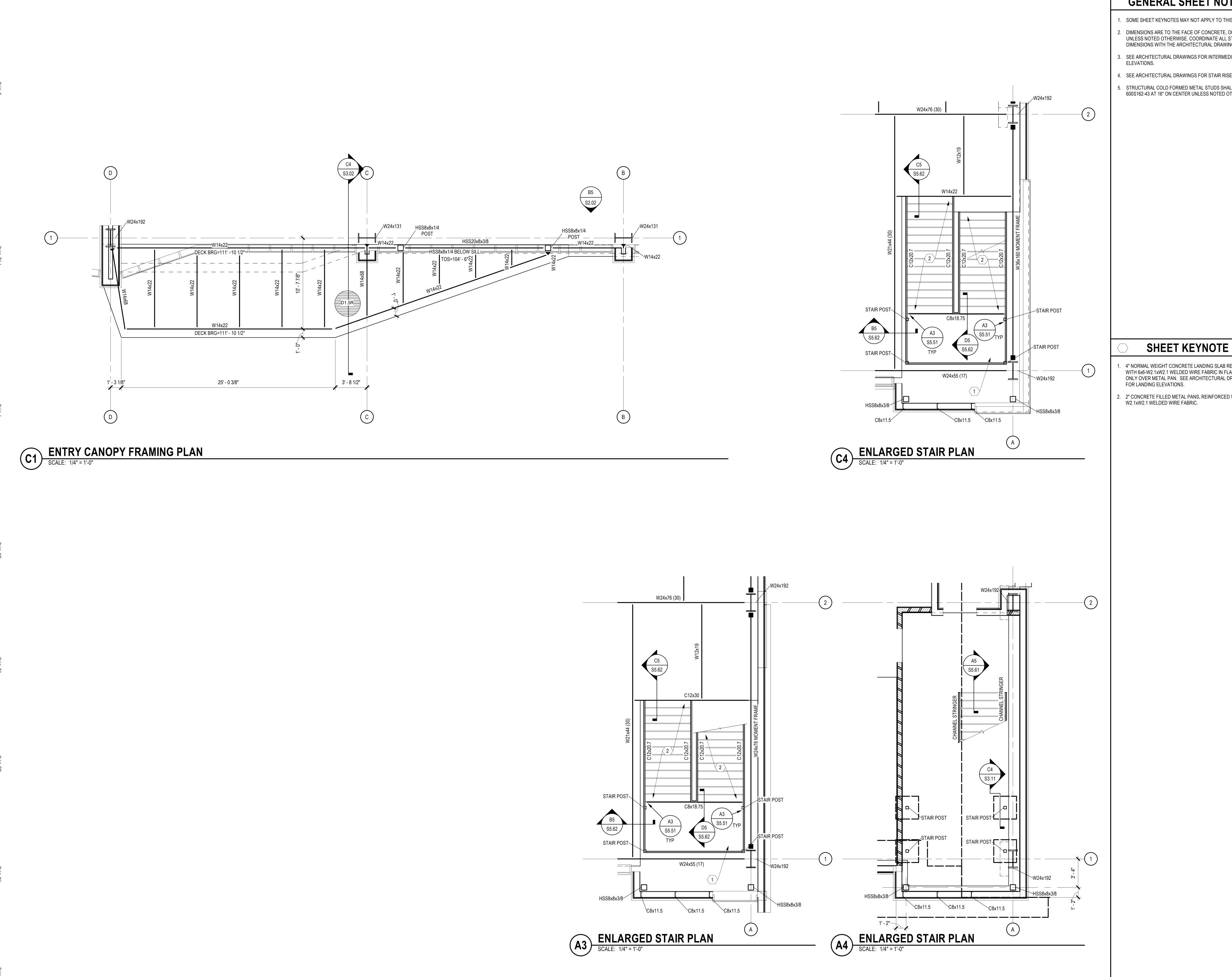




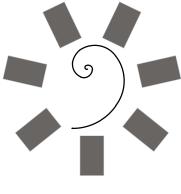
BULK STEEL PACKAGE NOT FOR CONSTRUCTION

S4.01

PLANS



- 1. SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- 2. DIMENSIONS ARE TO THE FACE OF CONCRETE, OR STUD UNLESS NOTED OTHERWISE. COORDINATE ALL STAIR DIMENSIONS WITH THE ARCHITECTURAL DRAWINGS.
- 3. SEE ARCHITECTURAL DRAWINGS FOR INTERMEDIATE LANDING
- 4. SEE ARCHITECTURAL DRAWINGS FOR STAIR RISE AND RUN.
- 5. STRUCTURAL COLD FORMED METAL STUDS SHALL BE 600S162-43 AT 16" ON CENTER UNLESS NOTED OTHERWISE.

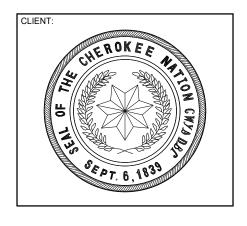


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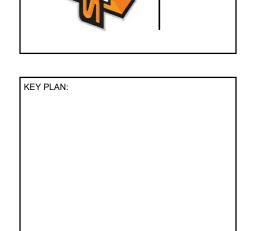


4" NORMAL WEIGHT CONCRETE LANDING SLAB REINFORCED WITH 6x6-W2.1xW2.1 WELDED WIRE FABRIC IN FLAT SHEETS ONLY OVER METAL PAN. SEE ARCHITECTURAL DRAWINGS

2. 2" CONCRETE FILLED METAL PANS, REINFORCED WITH 6x6-W2.1xW2.1 WELDED WIRE FABRIC.







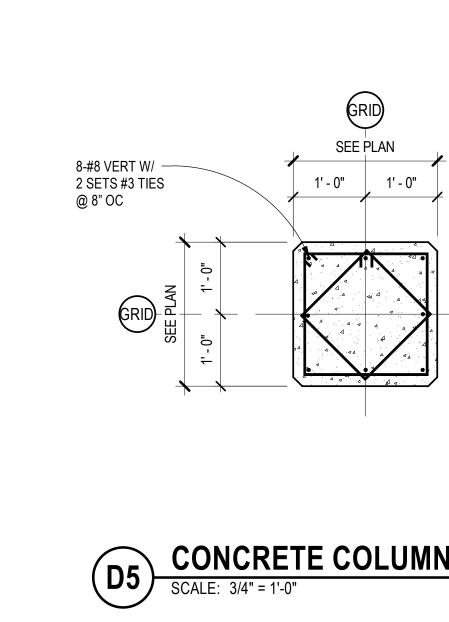
PROJECT PHASE: BULK STEEL PACKAGE NOT FOR CONSTRUCTION

		REVISIONS
#	DATE	DESCRIPTION

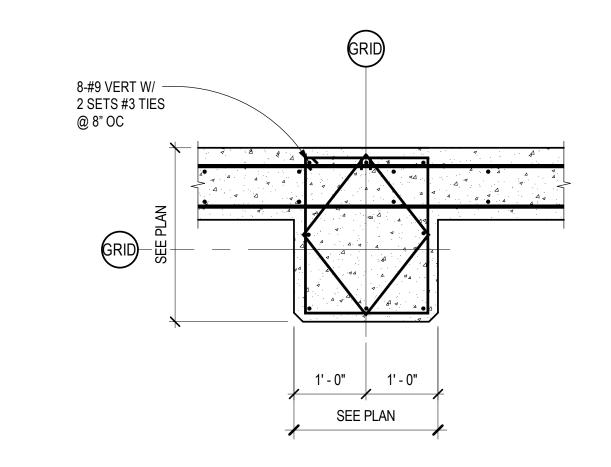
03-15-19

S4.02

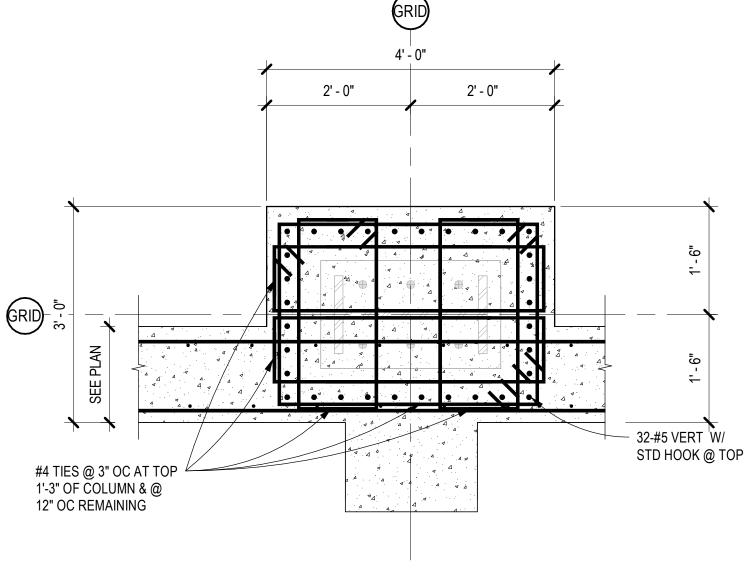
ENLARGED PLANS



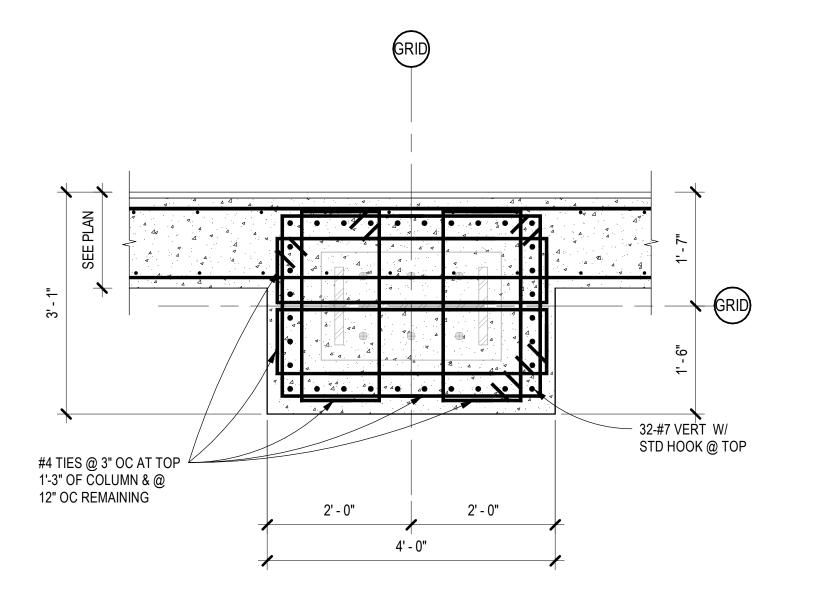
CONCRETE COLUMN DETAIL



C5 CONCRETE COLUMN DETAIL SCALE: 3/4" = 1'-0"



PILASTER DETAIL SCALE: 3/4" = 1'-0"





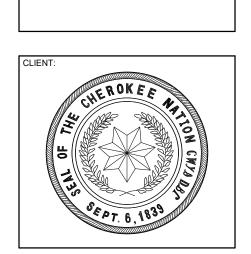


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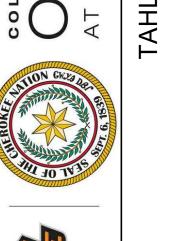
NOTE: THIS STRUCTURAL

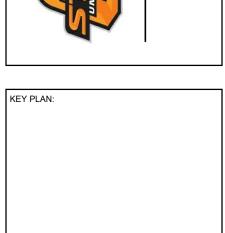
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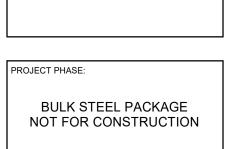








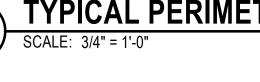




		REVISIONS
#	DATE	DESCRIPTION

DATE:	JOB NUMBER:	
03-15-19	17-13	
SHEET NUMBER:		
S5.11		

CONCRETE DETAILS



TYPICAL PERIMETER CHANNEL AT BASEMENT LID PLAN

SCALE: 3/4" = 1'-0"

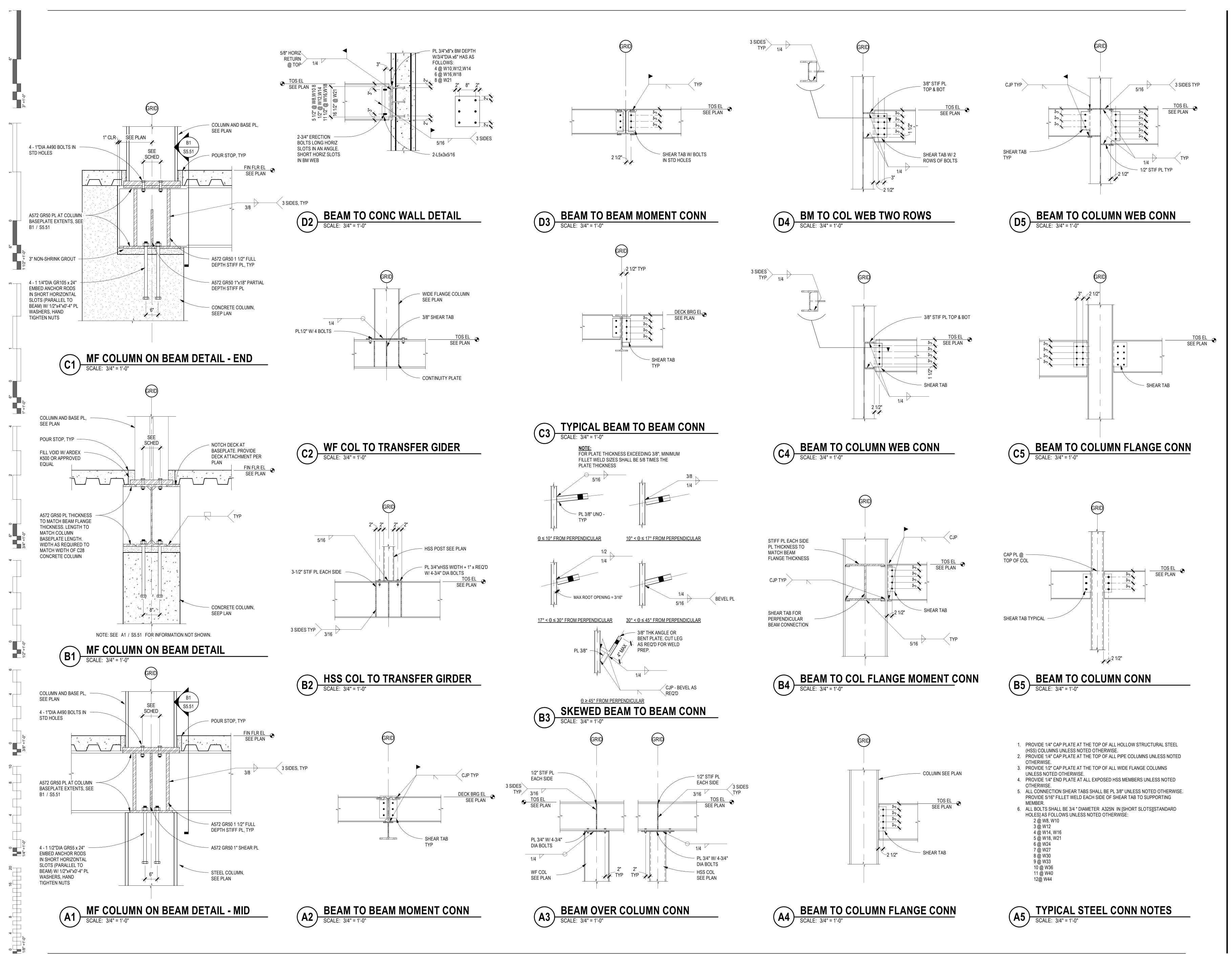
EXTEND CHANNEL
 TO ADJACENT BEAM

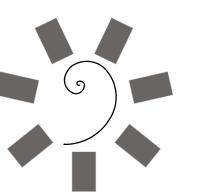
BASEMENT WALL SEE PLAN

TO ADJACENT BEAM

COLUMN SEE PLAN

CONC PILASTER SEE PLAN



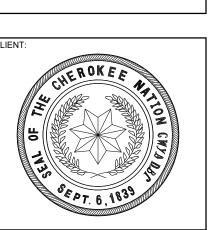


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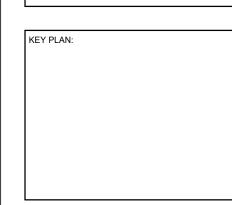


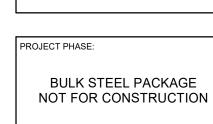


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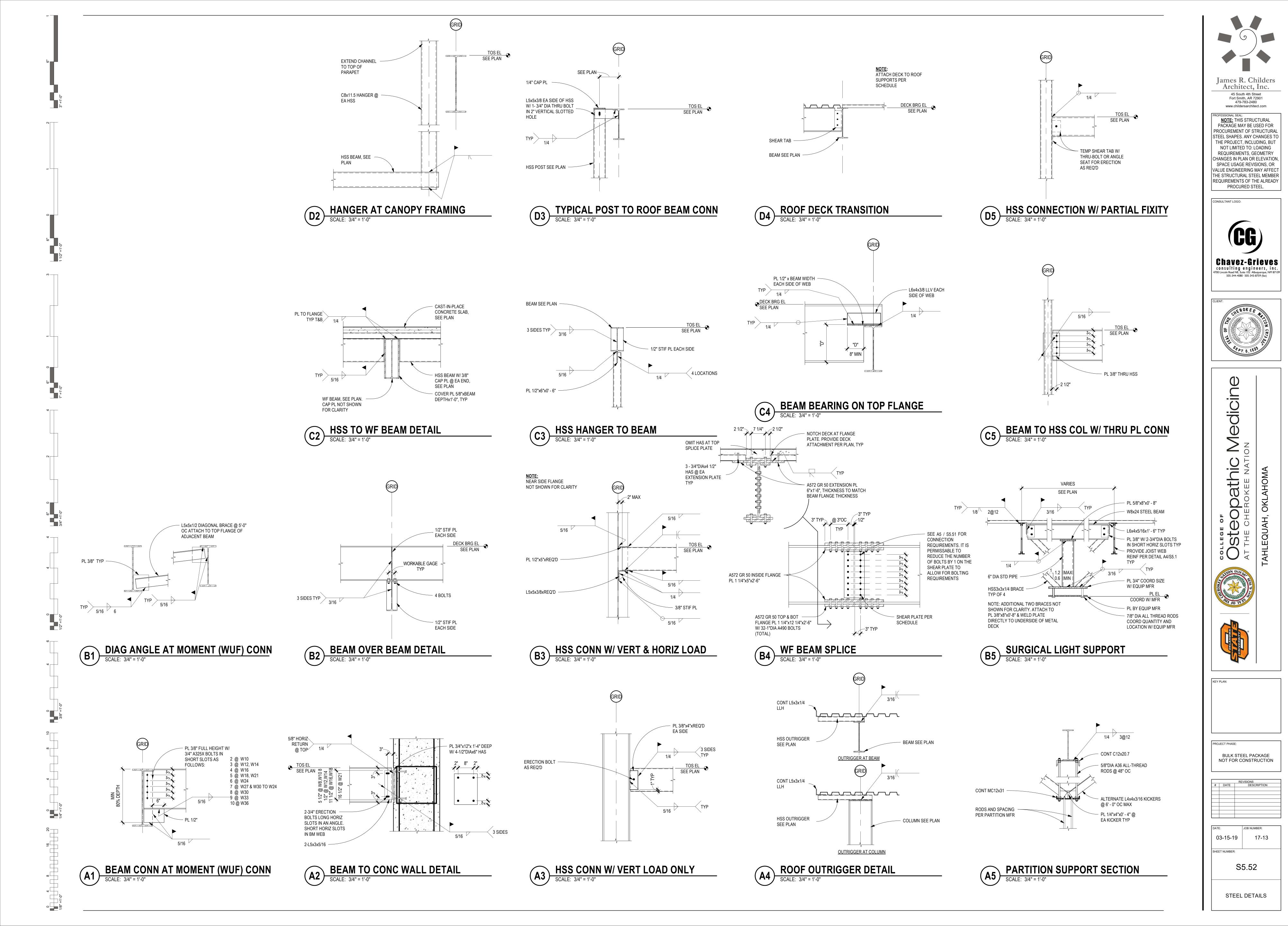


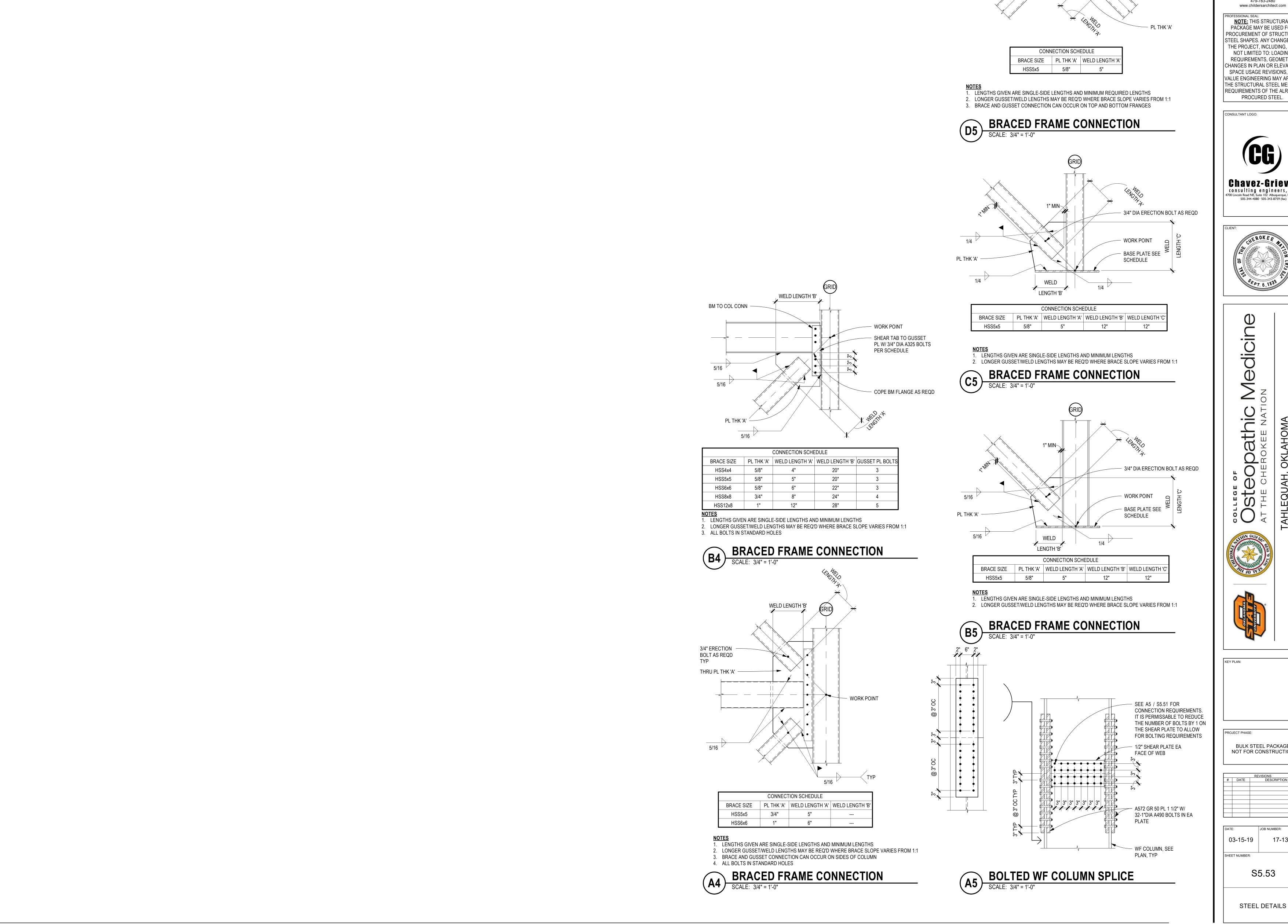
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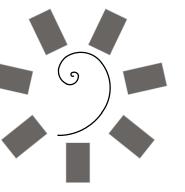
03-15-19 SHEET NUMBER:

S5.51

STEEL DETAILS







3/4" ERECTION BOLT AS REQD TYP

5/16 FILLET AT HSS8x8

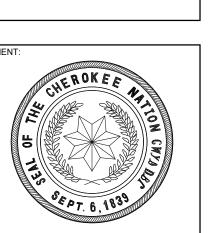
WORK POINT -

5/16 FILLET AT HSS8x8 1/4

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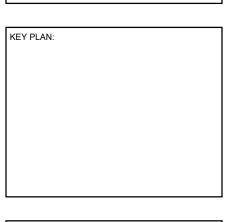




teopathic







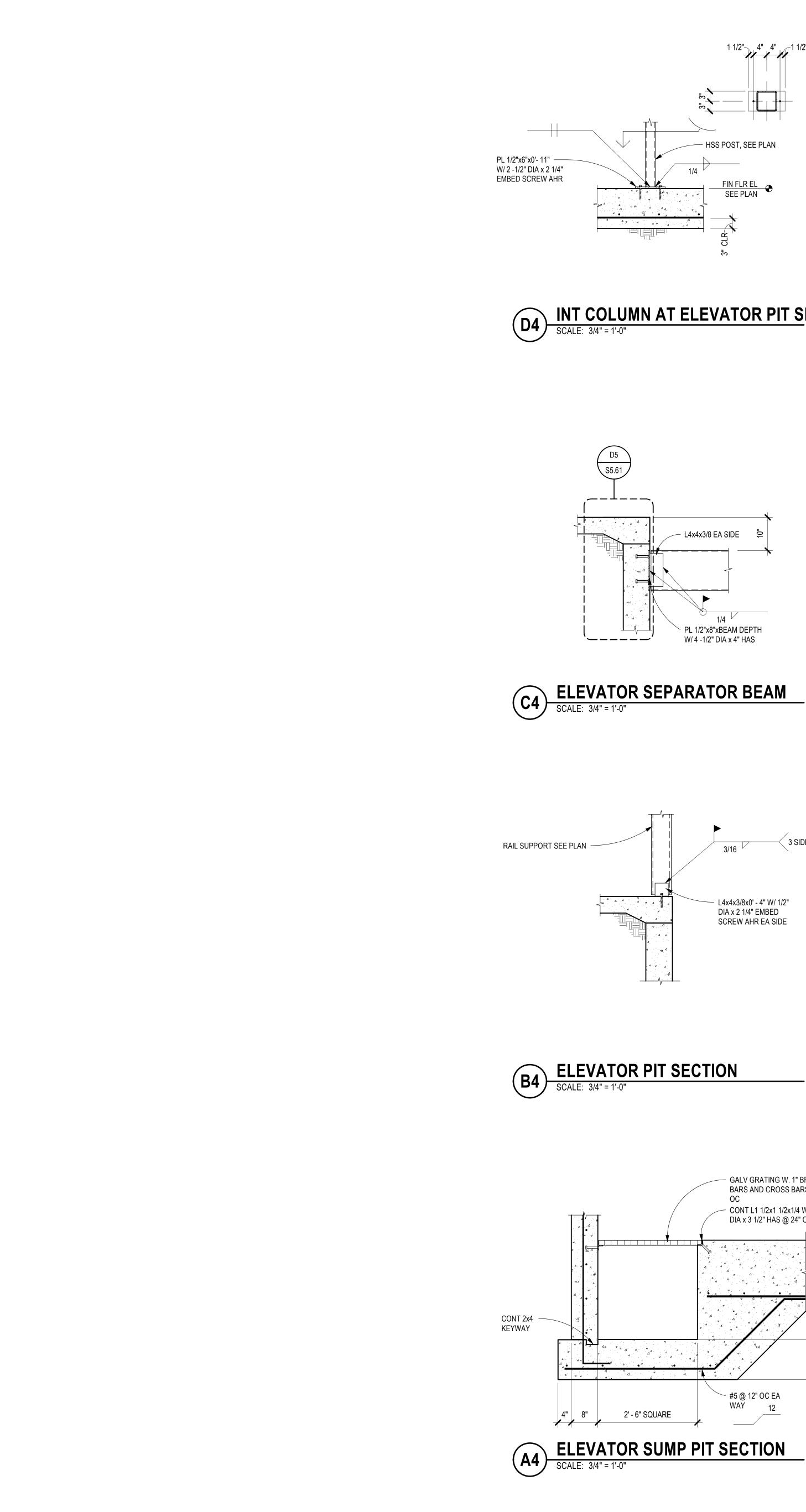
PROJECT PHASE: BULK STEEL PACKAGE NOT FOR CONSTRUCTION

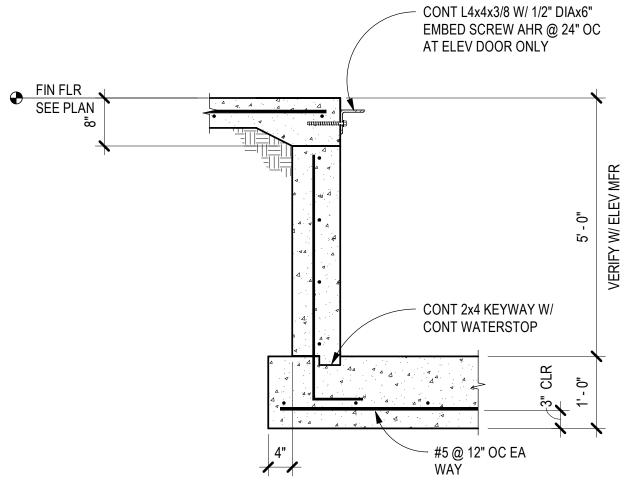
	REVISIONS				
#	DATE	DESCRIPTION			

17-13 03-15-19 SHEET NUMBER:

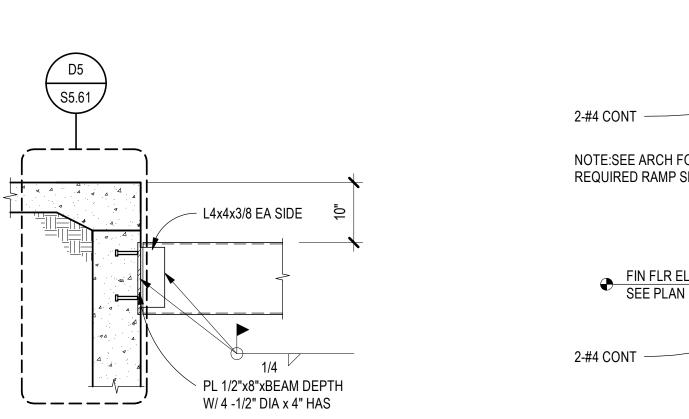
S5.53

STEEL DETAILS



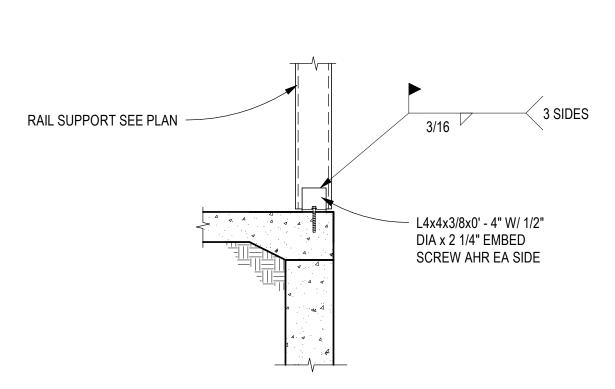


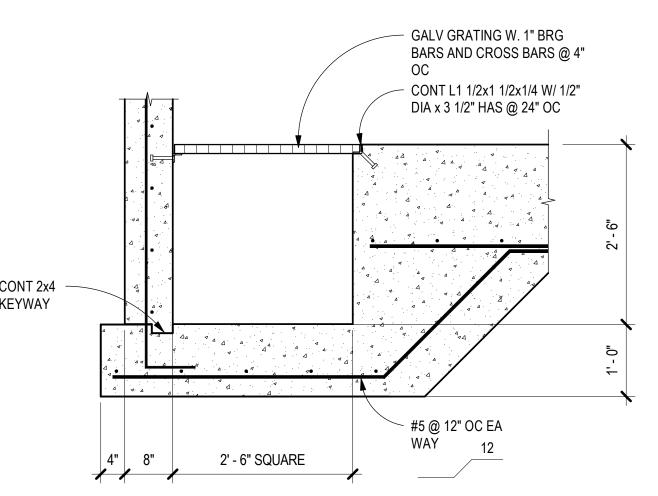
D4 INT COLUMN AT ELEVATOR PIT SLAB SCALE: 3/4" = 1'-0"



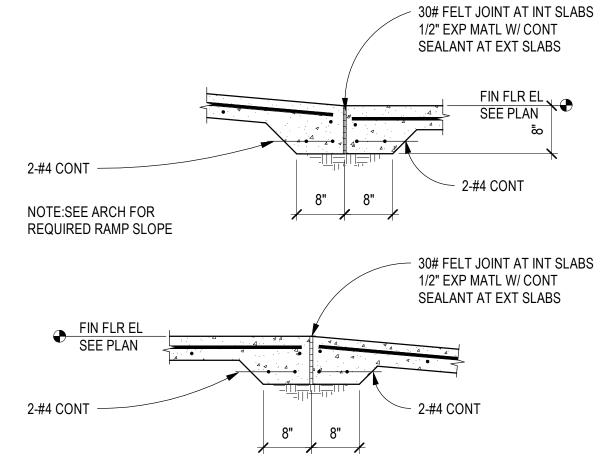
1 1/2" 4" 4" 1 1/2"

FIN FLR EL SEE PLAN

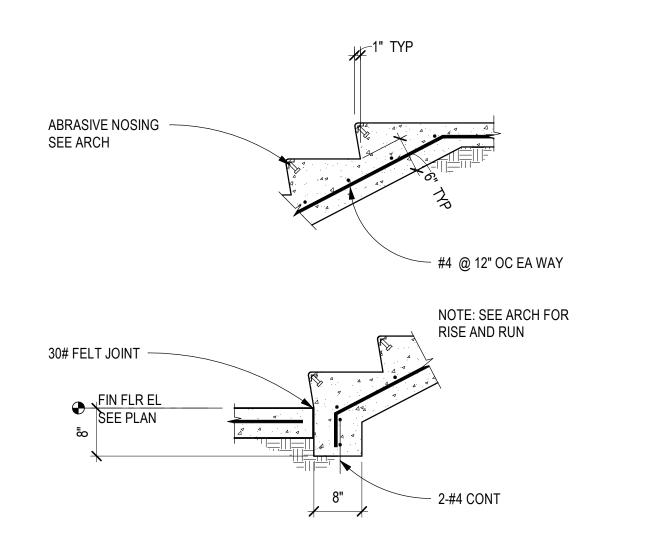




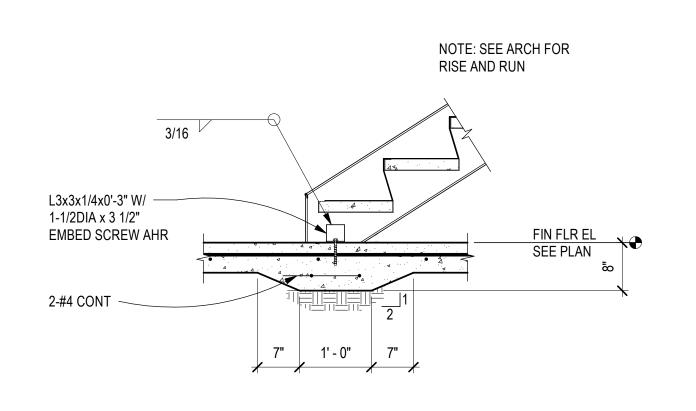




C5 TYPICAL RAMP SECTION SCALE: 3/4" = 1'-0"

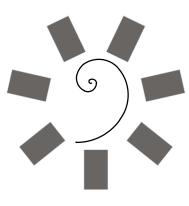


TYPICAL CONC STAIR SECTION SCALE: 3/4" = 1'-0"



TYPICAL STAIR BASE DETAIL

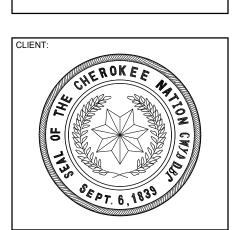
SCALE: 3/4" = 1'-0"



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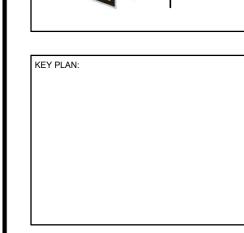
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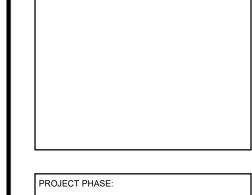




Osteopathic Medicin





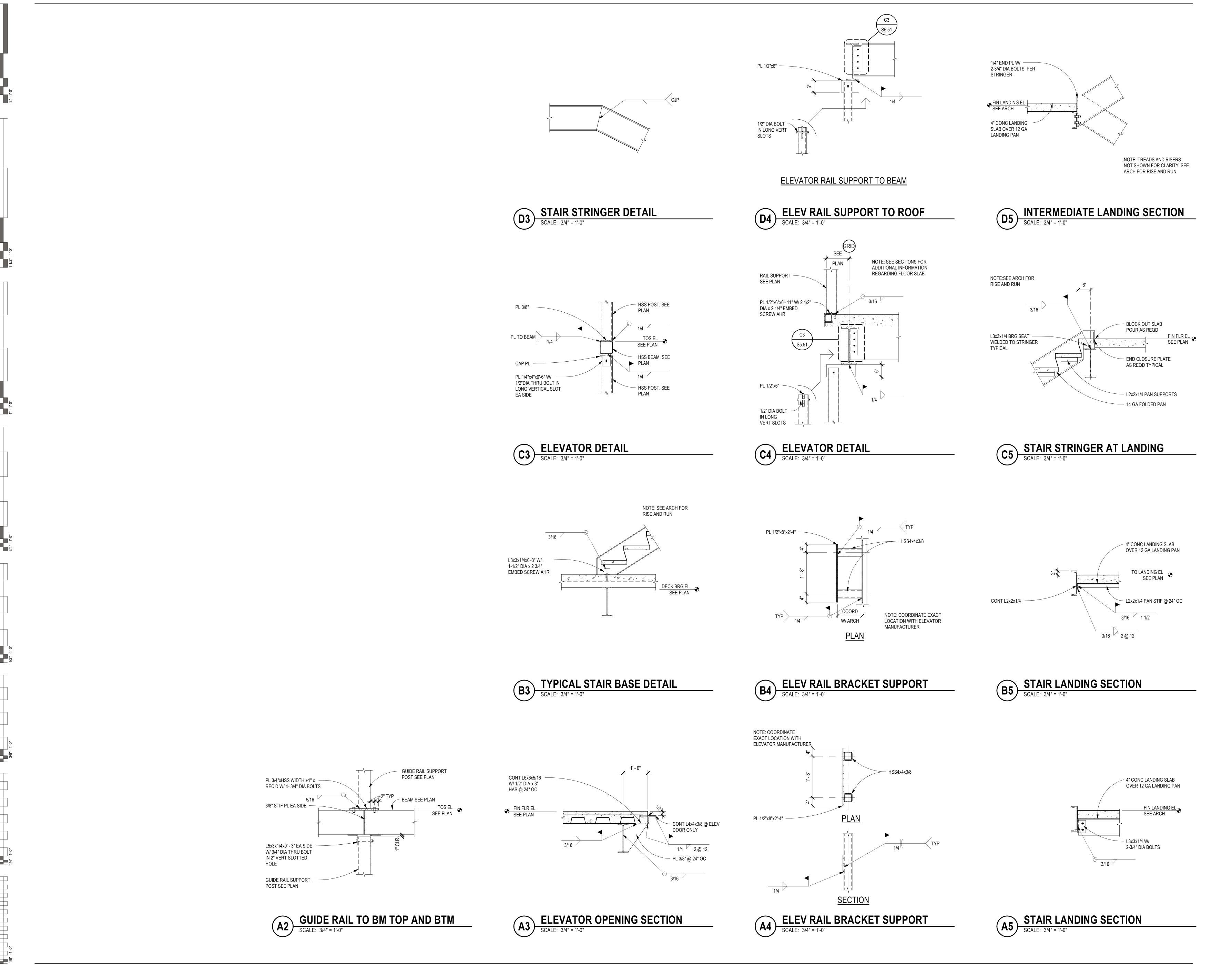


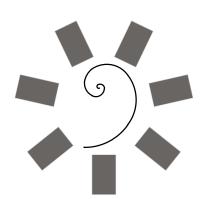
BULK STEEL PACKAGE NOT FOR CONSTRUCTION

		REVISIONS
#	DATE	DESCRIPTION

03-15-19

S5.61 TYPICAL VERTICAL
CIRCULATION
FOUNDATION DETAILS

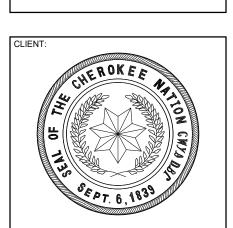




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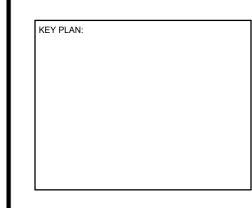




Medicir Osteopathic
AT THE CHEROKEE NATIO







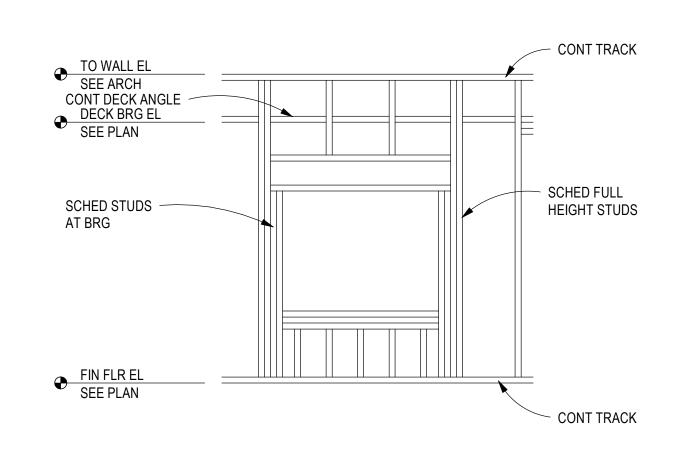
PROJECT PHASE: BULK STEEL PACKAGE NOT FOR CONSTRUCTION

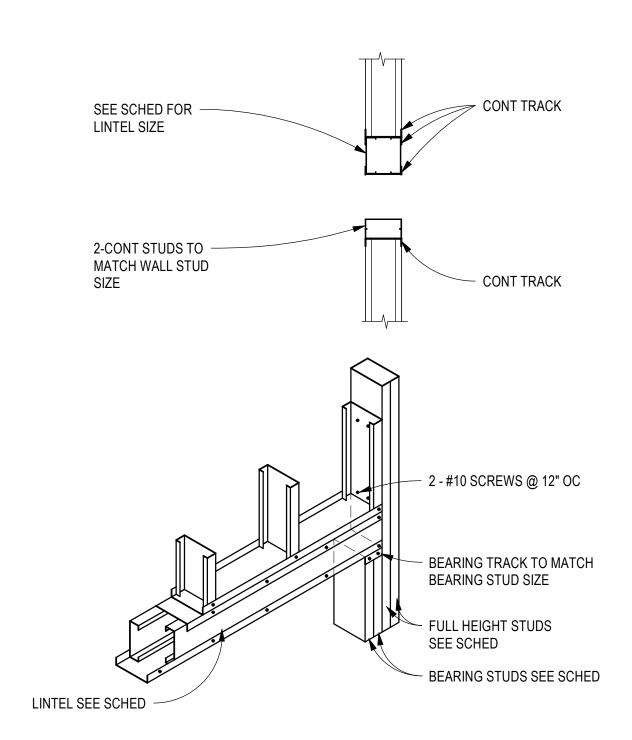
		REVISIONS
#	DATE	DESCRIPTION

03-15-19

SHEET NUMBER: S5.62

TYPICAL VERTICAL CIRCULATION FRAMING DETAILS





REI	NFORCIN	IG LAP SC	HEDULE					
REINFORCEMENT TYPE	#6 AND 3000 PSI	SMALLE 4000 PSI	R (#db) 5000 PSI	#7 AN 3000 PSI	D LARGEF 4000 PSI	R (#db) 5000 PSI	MINIMUM LENGTH (IN)	COMMENTS
CONTINUOUS WALL FOOTINGS AND STEMWALLS	30	30	30	30	30	30	18	
RETAINING WALLS AND BASEMENT WALL VERTICAL REINFORCING	57	50	45	72	62	56	12	
RETAINING WALLS AND BASEMENT WALL HORIZONTAL REINFORCING	57	50	45	72	62	56	12	
CONCRETE COLUMNS NOT SUPPORTING LATERAL FORCES	30	30	30	30	30	30	12	
CONCRETE COLUMNS SUPPORTING LATERAL FORCES	57	50	45	72	62	56	12	
TOP FLEXURAL REINFORCEMENT, INCLUDING BEAMS, GRADE BEAMS, AND COMBINED COLUMN FOOTING AT BRACED FRAME AND MOMENT FRAMES	57	50	45	72	62	56	12	
BOTTOM FLEXURAL REINFORCEMENT, INCLUDING BEAMS, GRADE BEAMS, AND COMBINED COLUMN FOOTING AT BRACED FRAME AND MOMENT FRAMES	57	50	45	55	62	56	12	
SLABS-ON-GRADE	30	30	30	30	30	30	12	
MINIMUM EMBEDMENT OF STANDARD HOOKS INTO CONCRETE BASE	16	14	12	16	14	14	6	INCREASE LENGTH FOR # 11 BARS AND LARGER BY A FACTOR OF 1.4
ALL REBAR LAPS IN CMU	· · · · · ·	72			72		12	

WALL HEIGHT ABOVE GRADE	HEIGHT RETAINED				- 011 - 11 - 17 - 111 111 10	WALL SCHEDULE					TO WALL FL
		F00 ⁻	TING SIZE	FOOTING F	REINFORCING		WALL				TO WALL EL SEE CIVIL DWGS
"HW"	"HR"	WIDTH	THICKNESS	REINF #1	REINF #2	REINFO	ORCING	REINF	SPLICE LENGTH	REMARKS	PROVIDE WATERPROCEING
FT-IN	FT-IN	FT-IN	FT-IN	NUMBER - SIZE	SIZE - SPACING	VERT SIZE - SPACING	HORIZ SIZE - SPACING	LOCATION	"LS"		PER ARCH DWGS
0' - 0" - 0' - 6"	0' - 0" - 4' - 0"	2'-6"	1'-0"	3 - #5 CONT	#4 @ 48" OC	#4 @ 18" OC	#4 @ 12" OC	4"	24"	BEARING PRESSURE 2000 PSF	WALL REINF
0' - 0" - 3' - 6"	0' - 0" - 5' - 0"	3'-6"	1'-0"	3 - #5 CONT	#4 @ 48" OC	#4 @ 14" OC	#4 @ 12" OC	4"	24"	BEARING PRESSURE 1500 PSF	DWL W/ STD HOOK TO MAX PROVIDE 12"x12"x12" GRA' MIRAFI 140N GEOTEXTILE FILTI BETWEEN SOIL & GRAVEL @ E/
0' - 0" - 0 - 6"	4' - 1" - 7' - 0"	5'-0"	1'-0"	5 - #5 CONT T&B	#5 @ 24" OC T&B	#5 @ 10" OC	#4 @ 12" OC	4"	36"	BEARING PRESSURE 1500 PSF	HOLE
0' - 0" - 4' - 0"	0' - 0" - 4' - 0"	3'-6"	1'-0"	3 - #5 CONT	#4 @ 48" OC T&B	#5 @ 18" OC	#4 @ 12" OC	4"	36"	BEARING PRESSURE 1500 PSF	REINF LOCATION SEE SCHED
0' - 0" - 4' - 0"	4' - 1" - 7' - 1"	5'-0"	1'-0"	5 - #5 CONT	#5 @ 18" OC T&B	#5 @ 10" OC	#4 @ 12" OC	4"	36"	BEARING PRESSURE 2000 PSF	TOF EL COORD W/ CIVIL
ΓE:											ĬĘ Į
		OF WALL WITH									REINF #2
OVIDE CONCRETE FIMES THE WALL H	WALL VERTICAL CONTF IEIGHT AND AT ALL STE	PS IN TOP OF									EQ 8" EQ WIDTH
	0' - 0" - 3' - 6" 0' - 0" - 0 - 6" 0' - 0" - 4' - 0" 0' - 0" - 4' - 0" EE: ORDINATE EXACT CHITECTURAL AND COVIDE CONCRETE TIMES THE WALL HERE	0' - 0" - 3' - 6" 0' - 0" - 5' - 0" 0' - 0" - 0 - 6" 4' - 1" - 7' - 0" 0' - 0" - 4' - 0" 0' - 0" - 4' - 0" 0' - 0" - 4' - 0" 4' - 1" - 7' - 1" EE: ORDINATE EXACT LOCATION AND EXTENT CHITECTURAL AND CIVIL DWGS. OVIDE CONCRETE WALL VERTICAL CONTROL TIMES THE WALL HEIGHT AND AT ALL STE LL. SEE DETAIL C2/S7.11 FOR CONTROL J	0' - 0" - 3' - 6"	0' - 0" - 3' - 6" 0' - 0" - 5' - 0" 3'-6" 1'-0" 0' - 0" - 0 - 6" 4' - 1" - 7' - 0" 5'-0" 1'-0" 0' - 0" - 4' - 0" 0' - 0" - 4' - 0" 3'-6" 1'-0" 0' - 0" - 4' - 0" 4' - 1" - 7' - 1" 5'-0" 1'-0" EE: ORDINATE EXACT LOCATION AND EXTENT OF WALL WITH HITECTURAL AND CIVIL DWGS. OVIDE CONCRETE WALL VERTICAL CONTROL JOINTS AT IMES THE WALL HEIGHT AND AT ALL STEPS IN TOP OF IL. SEE DETAIL C2/S7.11 FOR CONTROL JOINT	0' - 0" - 3' - 6" 0' - 0" - 5' - 0" 3' - 6" 1' - 0" 3 - #5 CONT 0' - 0" - 0 - 6" 4' - 1" - 7' - 0" 5' - 0" 1' - 0" 5 - #5 CONT T&E 0' - 0" - 4' - 0" 0' - 0" - 4' - 0" 3' - 6" 1' - 0" 3 - #5 CONT 0' - 0" - 4' - 0" 4' - 1" - 7' - 1" 5' - 0" 1' - 0" 5 - #5 CONT EE: ORDINATE EXACT LOCATION AND EXTENT OF WALL WITH CHITECTURAL AND CIVIL DWGS. OVIDE CONCRETE WALL VERTICAL CONTROL JOINTS AT TIMES THE WALL HEIGHT AND AT ALL STEPS IN TOP OF I.L. SEE DETAIL C2/S7.11 FOR CONTROL JOINT	0'-0"-3'-6" 0'-0"-5'-0" 3'-6" 1'-0" 3-#5 CONT #4 @ 48" OC 0'-0"-0-6" 4'-1"-7'-0" 5'-0" 1'-0" 5-#5 CONT T&B #5 @ 24" OC T&B 0'-0"-4'-0" 0'-0"-4'-0" 1'-0" 3-#5 CONT #4 @ 48" OC T&B 0'-0"-4'-0" 4'-1"-7'-1" 5'-0" 1'-0" 5-#5 CONT #5 @ 18" OC T&B FE: ORDINATE EXACT LOCATION AND EXTENT OF WALL WITH HITECTURAL AND CIVIL DWGS. OVIDE CONCRETE WALL VERTICAL CONTROL JOINTS AT HIMES THE WALL HEIGHT AND AT ALL STEPS IN TOP OF LL. SEE DETAIL C2/S7.11 FOR CONTROL JOINT	0'-0"-3'-6" 0'-0"-5'-0" 3'-6" 1'-0" 3-#5 CONT #4 @ 48" OC #4 @ 14" OC 0'-0"-0-6" 4'-1"-7'-0" 5'-0" 1'-0" 5-#5 CONT T&B #5 @ 24" OC T&B #5 @ 10" OC 0'-0"-4'-0" 0'-0"-4'-0" 3'-6" 1'-0" 3-#5 CONT #4 @ 48" OC T&B #5 @ 18" OC 0'-0"-4'-0" 4'-1"-7'-1" 5'-0" 1'-0" 5-#5 CONT #5 @ 18" OC T&B #5 @ 10" OC E: ORDINATE EXACT LOCATION AND EXTENT OF WALL WITH HITECTURAL AND CIVIL DWGS. INVIDE CONCRETE WALL VERTICAL CONTROL JOINTS AT IMES THE WALL HEIGHT AND AT ALL STEPS IN TOP OF IL. SEE DETAIL C2/S7.11 FOR CONTROL JOINT	0'-0"-3'-6" 0'-0"-5'-0" 3'-6" 1'-0" 3-#5 CONT #4 @ 48" OC #4 @ 14" OC #4 @ 12" OC 0'-0"-0-6" 4'-1"-7'-0" 5-0" 1'-0" 5-#5 CONT T&B #5 @ 24" OC T&B #5 @ 10" OC #4 @ 12" OC 0'-0"-4'-0" 0'-0"-4'-0" 3'-6" 1'-0" 3-#5 CONT #4 @ 48" OC T&B #5 @ 18" OC #4 @ 12" OC 0'-0"-4'-0" 4'-1"-7'-1" 5'-0" 1'-0" 5-#5 CONT #5 @ 18" OC T&B #5 @ 10" OC #4 @ 12" OC EE: ORDINATE EXACT LOCATION AND EXTENT OF WALL WITH CHITECTURAL AND CIVIL DWGS. OVIDE CONCRETE WALL VERTICAL CONTROL JOINTS AT TIMES THE WALL HEIGHT AND AT ALL STEPS IN TOP OF IL. SEE DETAIL C2/S7.11 FOR CONTROL JOINTS AT TIMES THE WALL HEIGHT AND AT ALL STEPS IN TOP OF IL. SEE DETAIL C2/S7.11 FOR CONTROL JOINTS	0'-0"-3'-6" 0'-0"-5'-0" 3'-6" 1'-0" 3-#5 CONT #4 @ 48" OC #4 @ 14" OC #4 @ 12" OC 4" 0'-0"-0-6" 4'-1"-7'-0" 5'-0" 1'-0" 3-#5 CONT T&B #5 @ 24" OC T&B #5 @ 10" OC #4 @ 12" OC 4" 0'-0"-4'-0" 0'-0"-4'-0" 3'-6" 1'-0" 3-#5 CONT #4 @ 48" OC T&B #5 @ 18" OC #4 @ 12" OC 4" 0'-0"-4'-0" 4'-1"-7'-1" 5'-0" 1'-0" 5-#5 CONT #5 @ 18" OC T&B #5 @ 10" OC #4 @ 12" OC 4" EE: ORDINATE EXACT LOCATION AND EXTENT OF WALL WITH HITECTURAL AND CIVIL DWGS. OVIDE CONCRETE WALL VERTICAL CONTROL JOINTS AT IMES THE WALL HEIGHT AND AT ALL STEPS IN TOP OF IL. SEE DETAIL C2/57.11 FOR CONTROL JOINTS	0'-0"-3'-6" 0'-0"-5'-0" 3'-6" 1'-0" 3-#5 CONT #4 @ 48" OC #4 @ 14" OC #4 @ 12" OC 4" 24" 0'-0"-0-6" 4'-1"-7'-0" 5-0" 1'-0" 5-#5 CONT T&B #5 @ 24" OC T&B #5 @ 10" OC #4 @ 12" OC 4" 36" 0'-0"-4'-0" 0'-0"-4'-0" 3'-6" 1'-0" 3-#5 CONT #4 @ 48" OC T&B #5 @ 18" OC #4 @ 12" OC 4" 36" 0'-0"-4'-0" 4'-1"-7'-1" 5-0" 1'-0" 5-#5 CONT #5 @ 18" OC T&B #5 @ 10" OC #4 @ 12" OC 4" 36" E: ORDINATE EXACT LOCATION AND EXTENT OF WALL WITH HITECTURAL AND CIVIL DWGS. WIDE CONCRETE WALL VERTICAL CONTROL JOINTS AT TIMES THE WALL HEIGHT AND AT ALL STEPS IN TOP OF IL. SEE DETAIL CZ/ST.11 FOR CONTROL JOINTS	PRESSURE 2000 PSF 0'-0"-3'-6" 0'-0"-5'-0" 3'-6" 1'-0" 3-#5 CONT #4 @ 48" OC #4 @ 14" OC #4 @ 12" OC 4" 24" BEARING PRESSURE 1500 PSF 0'-0"-0-6" 4'-1"-7'-0" 5-0" 1'-0" 5-#5 CONT T&B #5 @ 24" OC T&B #5 @ 10" OC #4 @ 12" OC 4" 36" BEARING PRESSURE 1500 PSF 0'-0"-4'-0" 0'-0"-4'-0" 3'-6" 1'-0" 3-#5 CONT #4 @ 48" OC T&B #5 @ 18" OC #4 @ 12" OC 4" 36" BEARING PRESSURE 1500 PSF 0'-0"-4'-0" 4'-1"-7'-1" 5'-0" 1'-0" 5-#5 CONT #5 @ 18" OC T&B #5 @ 10" OC #4 @ 12" OC 4" 36" BEARING PRESSURE 1500 PSF E: ORDINATE EXACT LOCATION AND EXTENT OF WALL WITH HITECTURAL AND CIVIL DWGS. WIDE CONCRETE WALL VERTICAL CONTROL JOINTS AT IMES THE WALL HEIGHT AND AT ALL STEPS IN TOP OF IL. SEE DETAIL C2057.11 FOR CONTROL JOINTS AT IMES THE WALL HEIGHT AND AT ALL STEPS IN TOP OF IL. SEE DETAIL C2057.11 FOR CONTROL JOINT

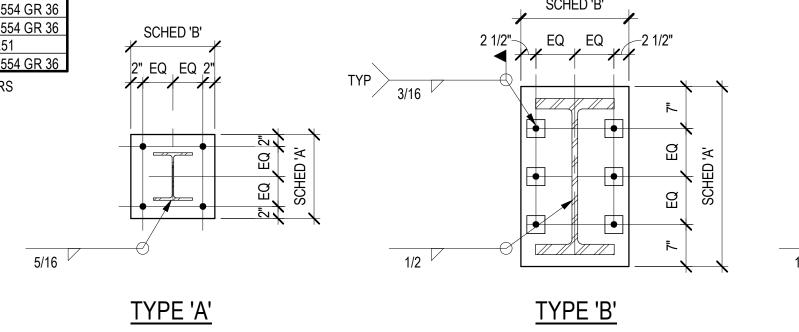
									DECK SCHEE	DULE			
		SLAB METAL DECK		CK	DECK ATTACHMENTS								
MARK	COMPOSITE SLAB	THICK	MATL	REINF	THICK	TYPE	GAGE	FINISH	ATTACH PERP TO RIBS	ATTACH PARALLEL TO RIBS	ATTACH SIDELAPS	DECK THICKNESS	COMMENTS
D1.5R					1 1/2"	В	20	PAINTED	4-5/8 "DIA PUDDLE WELDS PER 36 "WIDE SHEET	5/8" DIA PUDDLE WELDS @ 12" OC	#10 SCREWS @ 12" OC	1 1/2"	
D6R	Х	3"	NW CONC	6x6 - W2.1xW2.1 WELD WIRE FABRIC IN FLAT SHEETS	3"	VLI	18	[GALVANIZED]	[4-5/8 "DIA PUDDLE WELDS] PER 36 "WIDE SHEET	[5/8" DIA PUDDLE WELDS] @ [12]" OC	[#10} SCREWS @ [12]" OC	6"	
D6RA		6"	NW CONC	#4 @ 12" OC EA WAY								6"	SLOPE PER ARCHITECTURAL DRAWINGS INCLUDE CRYSTALLINE WATERPROOFING ADMIZTURE IN CONCRETE
D7.5F	Х	4 1/2"	NW CONC	#5 @ 12" OC EA WAY	3"	VLI	18	[GALVANIZED]	[4-5/8 "DIA PUDDLE WELDS] PER 36 "WIDE SHEET	[5/8" DIA PUDDLE WELDS] @ [12]" OC	[#10] SCREWS @ [12]" OC	7 1/2"	

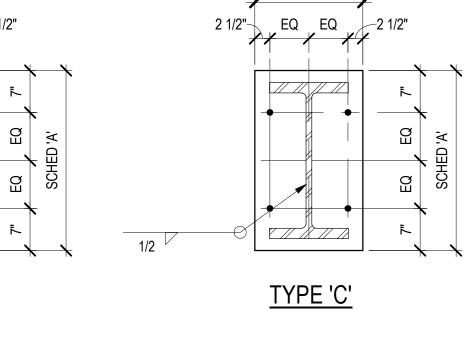
	SLAB-ON-GRADE SCHEDULE										
	SLAB										
MARK	THICKNESS	MATL	REINFORCING	BEARING STRATA	COMMENTS						
S5	5"	CONC	#4 @ 18" OC EA WAY	15 MIL VAPOR RETARDER OVER LASER LEVELED SUBGRADE PER GEN STRUCT NOTES	THE FINISHED TOP SURFACE SHOULD BE FLAT AND LEVEL WITH SUFFICIENT FINES TO FILL BETWEEN COARSE AGGREGATE. IF THIS IS NOT THE CASE, PROVIDE UP TO 1/2 INCH BLOTTER LAYER OF SAND						
S6	5"	CONC	#4 @ 12" OC EA WAY	15 MIL VAPOR RETARDER OVER LASER LEVELED SUBGRADE PER GEN STRUCT NOTES	THE FINISHED TOP SURFACE SHOULD BE FLAT AND LEVEL WITH SUFFICIENT FINES TO FILL BETWEEN COARSE AGGREGATE. IF THIS IS NOT THE CASE, PROVIDE UP TO 1/2 INCH BLOTTER LAYER OF SAND						

HSS6x6 A PL 3/4"x14"x1' - 2" 4 NOTE: FOR GRAVITY BASE PLATES, PROVID	ANCHOR BOLTS QTY x SIZE x GRADE - 3/4" DIA x 9" F1554 GR 36 - 3/4" DIA x 9" F1554 GR 36 DE MAXIMUM OVERSIZED	STD WASHER W/ DBL HEX NUTS PER AISC 360-10 TABLE 14-2	SCHED EMBED	TOP OF BASE PLATE TOP OF CONCRETE STD WASHER W/ DBL HEX NUTS	STD WASHER W/ DBL HEX NUTS PER AISC 360-10 TABLE 14-2	SCHED EMBED SEE NOTE	TOP OF BASE PLATE TOP OF CONCRETE PLATE WASHER 1/2"x3" SQ W/ DBL HEX NUTS
HOLES AND PLATE WASHERS WITH SIZE AN 360-10 TABLE 14-2.	ND THICKNESS PER AISC					NOTE: FIRST 1'-0" OF EMBEDDED PORT ANCHOR BOLT SHALL BE UNTHREADE	

GRAVITY ANCHOR BOLT

		ATERAL BASE PLAT	E SCHEDULE	GRAVITY ANCHUR BU	<u>JL1</u>	<u>L/</u>
	BASE	PLATE	ANCHOR BOLTS			
MARK	TYPE	SIZE "T"x"A"x"B"	QTY x SIZE x GRADE			
BP1	В	PL 2 1/4"x30"x1'-6"	6 - 1 1/4" DIA x 18" F1554 GR 36		SCHED 'B'	
BP2	В	PL 2 1/2"x30"x1'-6"	6 - 1 1/4" DIA x 18" F1554 GR 36	SCHED 'B'		
BP3	С	PL 2 1/4"x30"x1'-6"	SEE A5 / S5.51	/ /	2 1/2" EQ EQ 2	1/2"
BP4	В	PL 2 1/4"x30"x1'-6"	6 - 1 1/4" DIA x 24" F1554 GR 36	2" EQ EQ 2"	711 111	
NOTE: FOR	LATERA	L BASE PLATES, PRO	OVIDE PLATE WASHERS	TYP 2/4/		
		CKNESS PER AISC 36		3/16		<u>"</u>
						Ø



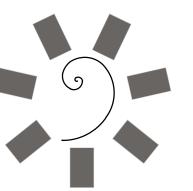


LATERAL ANCHOR BOLT

WALL SCHEDULE										
MARK	VENEER	WALL	VERTICAL	HORIZONTAL	GRADE	COMMENTS				
WC8		8" CONC	#5 @ 12" OC	#5 @ 12" OC	A615					
WC8A		8" CONC	#5 @ 12" OC	#5 @ 12" OC	A615	SEE D2/S3.11				
WC15		15" CONC	#7 @ 12" OC EA FACE	#4 @ 12" OC EA FACE	A615					
WC16		16" CONC	#7 @ 12" OC EA FACE	#4 @ 12" OC EA FACE	A615	SEE C2/S3.11				
WC16A		16" CONC	#7 @ 12" OC EA FACE	#4 @ 12" OC EA FACE	A615					
WC20		16" CONC	#7 @ 6" OC EA FACE	#5 @ 12" OC EA FACE	A615					
WC24		24" CONC	#7 @ 6" OC EA FACE	#5 @ 12" OC EA FACE	A615					

		SP	OT FOOTING SO	CHEDULE		
		SIZE		REINF	FORCING	
MARK	WIDTH	LENGTH	DEPTH	BOTTOM	TOP	COMMENTS
F132	11' - 0"	11' - 0"	3' - 0"	12 - #8 EA WAY	12 - #8 EA WAY	
F144	12' - 0"	12' - 0"	3' - 0"	15 - #8 EA WAY	15 - #8 EA WAY	
F156	13' - 0"	13' - 0"	3' - 3"	15 - #8 EA WAY	15 - #8 EA WAY	
F168	14' - 0"	14' - 0"	1' - 3"	15 - #8 EA WAY	15 - #8 EA WAY	
F180	15' - 0"	15' - 0"	3' - 3"	22 - #9 EA WAY	22 - #9 EA WAY	
F192	16' - 0"	16' - 0"	3' - 6"	24 - #9 EA WAY	24 - #9 EA WAY	
F204	17' - 0"	17' - 0"	3' - 6"	28 - #9 EA WAY	28 - #9 EA WAY	
F468	12' - 0"	39' - 0"	3' - 0"			
F528	13' - 0"	44' - 0"	3' - 0"			
F804	14' - 0"	67' - 0"	3' - 0"			
F840	13' - 0"	70' - 0"	3' - 0"			

	CONTINUOUS FOOTING SCHEDULE										
	SI	ZE	REINFO	REINFORCING							
MARK	WIDTH	DEPTH	CONTINUOUS	TRANSVERSE	COMMENTS						
CF60	5' - 0"	1' - 6"	4 - #5	#5 @ 18" OC							
CF72	6' - 0"	1' - 6"	4 - #5	#5 @ 18" OC							
CF96	8' - 0"	2' - 0"	4 - #5	#5 @ 18" OC							
CF168		2' - 0"	18 - #7 TOP & BOT	#7 @ 12" OC TOP & BOT							
CF168B	14' - 0"	3' - 0"	4 - #5	#5 @ 18" OC							
CF216		2' - 0"	22 - #7 TOP & BOT	#7 @ 12" OC TOP & BOT							



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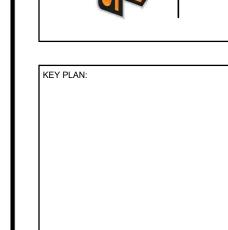
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BULK STEEL PACKAGE NOT FOR CONSTRUCTION

		REVISIONS
#	DATE	DESCRIPTION

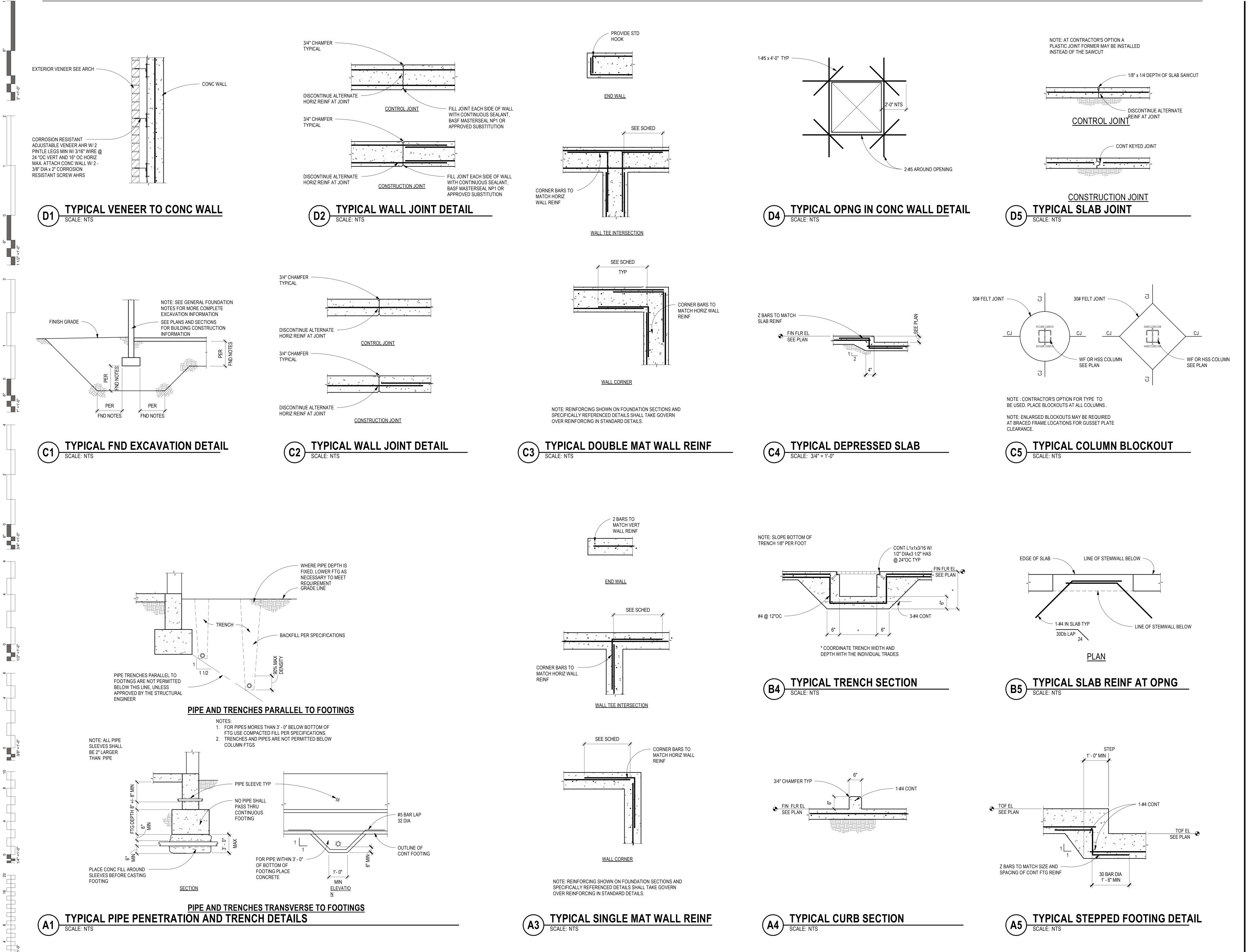
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03-15-19	17-13
SHEET NUMBER:	1

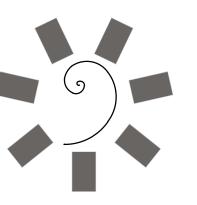
S6.01

SCHEDULES

TYPICAL CONCRETE SITE RETAINING WALL SCHEDULE AND DETAIL

SCALE: 3/4" = 1'-0"



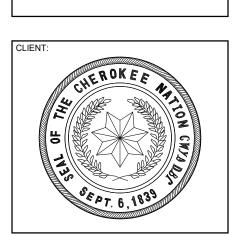


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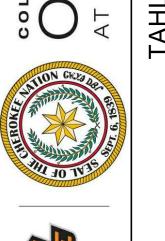
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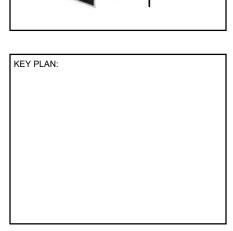
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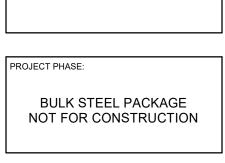




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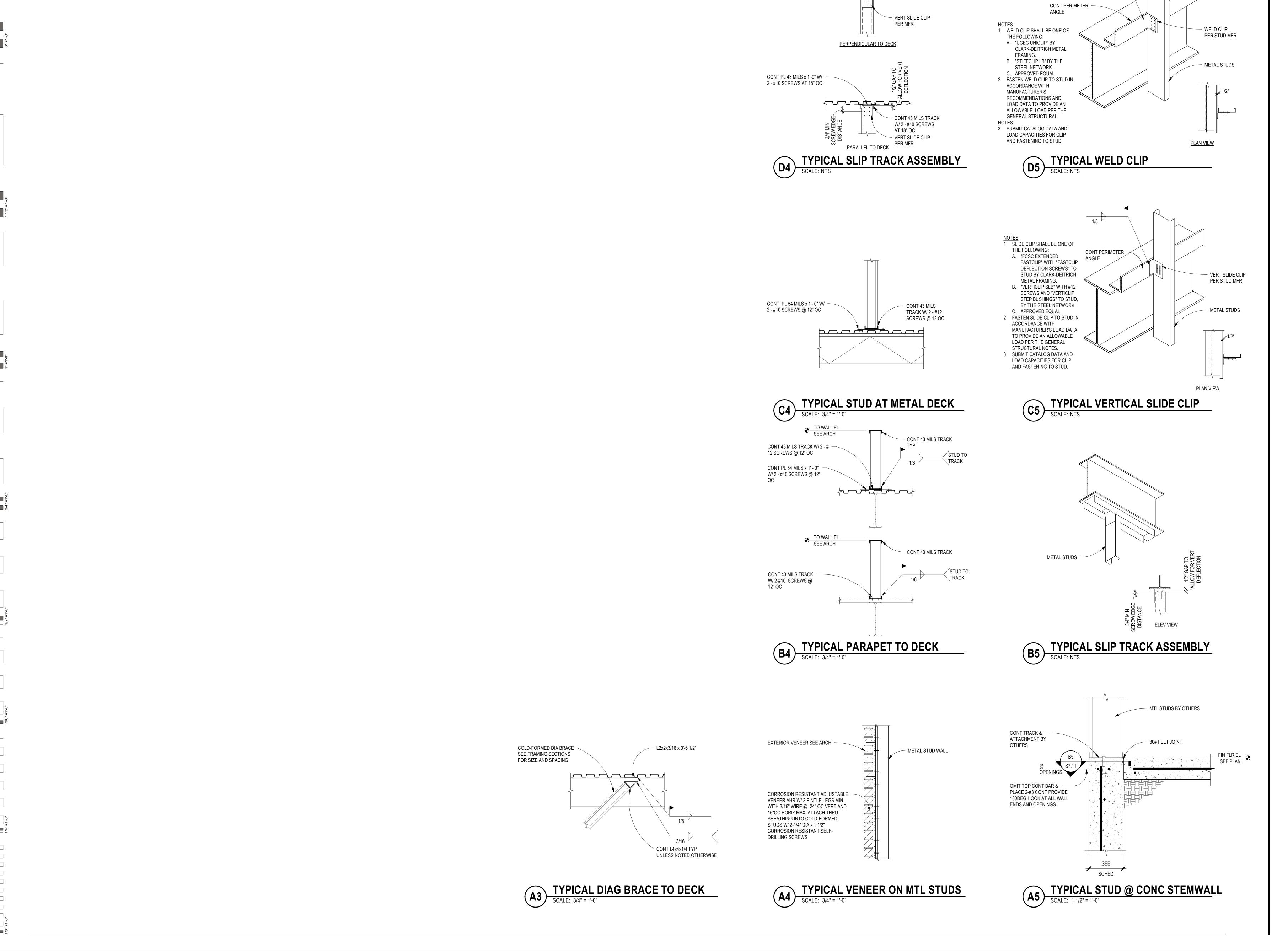


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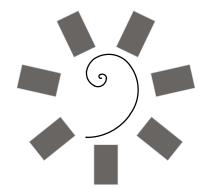
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TYPICAL CONCRETE **DETAILS**



CONT 600T125-43 W/ 2 - #10 ----

SCREWS AT 18" OC



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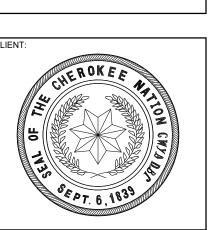
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THE STRUCTURAL STEEL MEMBER
REQUIREMENTS OF THE ALREADY
PROCURED STEEL.





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KEY PLAN:

PROJECT PHASE:

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DATE DESCRIPTION

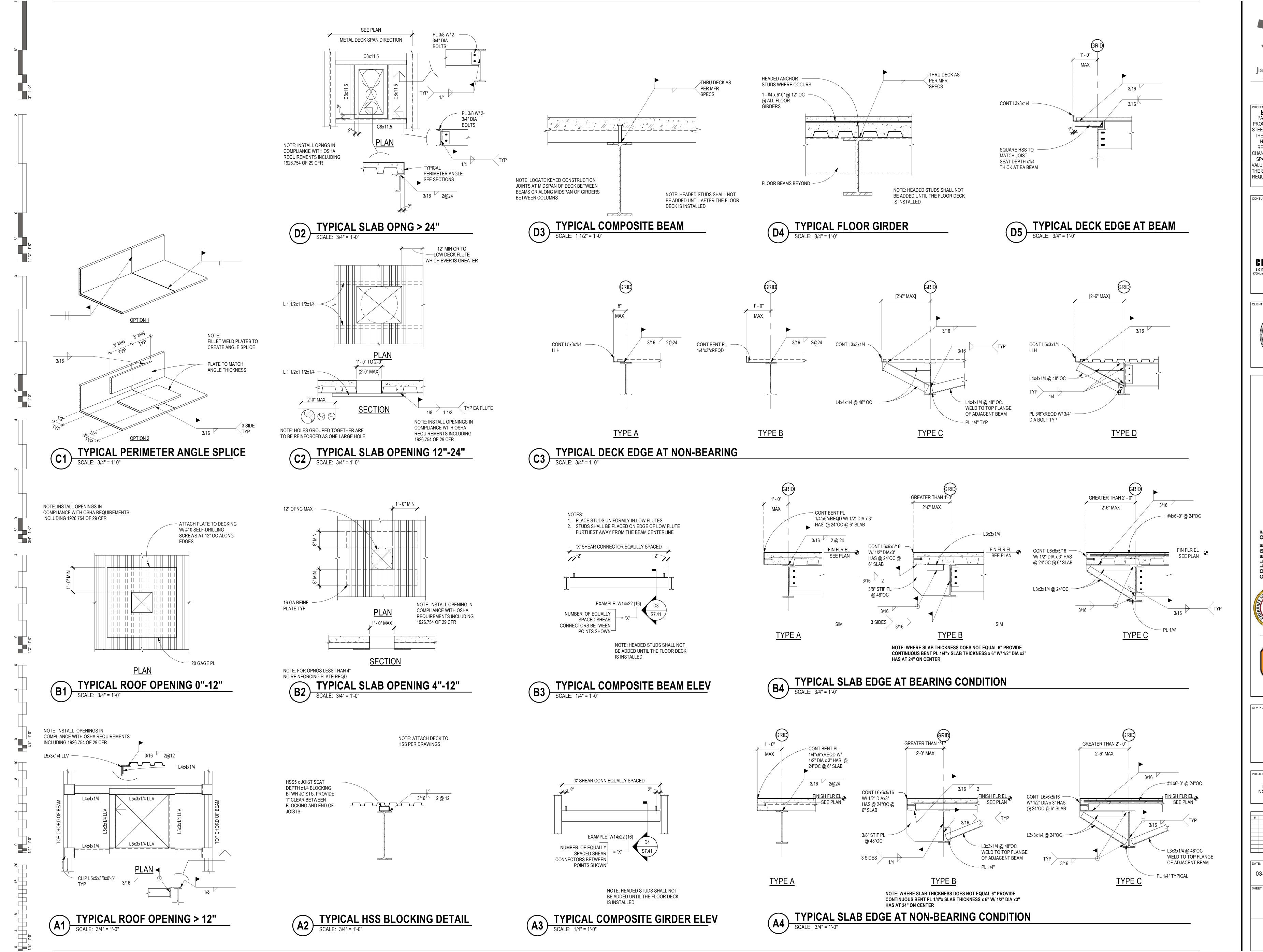
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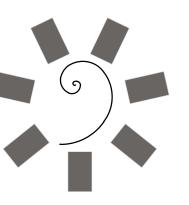
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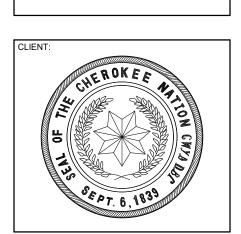
TYPICAL COLD-FORMED DETAILS

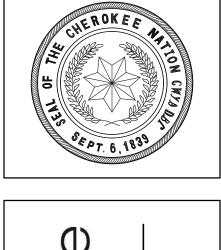




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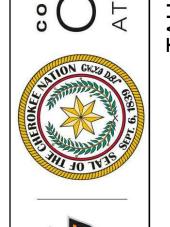


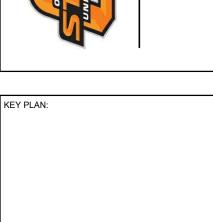


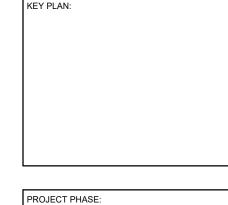


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REVISIONS DESCRIPTION

JOB NUMBER: 17-13 03-15-19 SHEET NUMBER:

S7.41

TYPICAL STEEL **DETAILS**

1. IN ADDITION TO THE REQUIRED SUBMITTALS SPECIFIED BY THE BALANCE OF THE CONTRACT DOCUMENTS, THE FOLLOWING SUBMITTALS SHALL BE SENT TO SIDEPLATE SYSTEMS, INC. ELECTRONICALLY VIA THE STRUCTURAL ENGINEER OF RECORD FOR THEIR REVIEW AND DISPOSITION:

a. QUALITY CONTROL PROGRAM (REQUIRED IF NOT AISC CERTIFIED) b. ONE ELECTRONIC COPY OF ALL STRUCTURAL STEEL DRAWINGS THAT EITHER DIRECTLY PERTAINS TO AND/OR AFFECTS THE SHOP FABRICATION OR FIELD ERECTION OF THE SIDEPLATE STEEL FRAME CONNECTION SYSTEM, INCLUDING THE INITIAL SUBMITTAL AND ALL CORRECTED RE-SUBMITTALS OF AFFECTED DRAWINGS. SIDEPLATE SYSTEMS, INC. SHALL BE GIVEN, AS A MINIMUM, THE SAME SPECIFIED REVIEW TIME (NOT LESS THAN SEVEN BUSINESS DAYS) AS THE ENGINEER OF RECORD.

a. PRIOR TO THE START OF DETAILING OF THE SHOP DRAWINGS, THE FABRICATION CONTRACTOR SHALL FORMALLY REQUEST A PRE-DETAILING MEETING FROM SIDEPLATE SYSTEMS, INC. THIS MEETING IS TYPICALLY A WEBINAR TO DISCUSS BEST PRACTICES FOR THE DETAILING OF THE SIDEPLATE CONNECTIONS, AND TO CREATE A PROACTIVE FORUM TO ANSWER ANY QUESTIONS.

a. PRIOR TO THE START OF FABRICATION, THE FABRICATION CONTRACTOR SHALL FORMALLY REQUEST A PRE-FABRICATION MEETING FROM SIDEPLATE SYSTEMS, INC. THIS MEETING IS TYPICALLY A WEBINAR TO DISCUSS BEST PRACTICES FOR THE FABRICATION OF THE SIDEPLATE CONNECTIONS, AND TO CREATE A PROACTIVE FORUM TO ANSWER ANY QUESTIONS.

a. PRIOR TO THE START OF STEEL ERECTION, THE ERECTION CONTRACTOR SHALL FORMALLY REQUEST A PRE-ERECTION MEETING FROM SIDEPLATE SYSTEMS, INC. THIS MEETING IS TYPICALLY A WEBINAR TO DISCUSS BEST PRACTICES FOR FIELD ERECTION OF THE SIDEPLATE BEAMS AND COLUMNS, AND TO CREATE A PROACTIVE FORUM TO ANSWER ANY QUESTIONS.

1. THE GOVERNING CODES SHALL CONSIST OF ANSI/AWS D1.1-2010 (AWS D1.1), AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES (APRIL 14, 2010), 2009 RCSC SPECIFICATIONS FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS, AND ALL APPLICABLE BUILDING AND JURISDICTIONAL CODES AND PROJECT STANDARDS SPECIFIED IN THE PROJECT SPECIFICATION STRUCTURAL STEEL SECTION. WHERE THE REQUIREMENTS DIFFER BETWEEN SIDEPLATE CONNECTION NOTES, THE GENERAL STRUCTURAL NOTES, AND THE GOVERNING CODES, THE MORE STRINGENT SECTION CRITERIA SHALL CONTROL. 2. ALPHA AND NUMERIC DESIGNATORS {X} & {#} USED HEREIN TO SIMPLIFY THE IDENTIFICATION OF PLATES, ANGLES, AND WELDS ARE DEFINED

SIDE PLATE FOR UNIAXIAL CONNECTIONS

BEAM FLANGE COVER PLATE, AS REQUIRED

VERTICAL SHEAR PLATE OR FLAT BAR, AS REQUIRED

{D} HORIZONTAL SHEAR PLATE OR FLAT BAR, AS REQUIRED

VERTICAL ANGLE WELDED TO THE VERTICAL SHEAR PLATE {C}, AS REQUIRED

VERTICAL SHEAR ELEMENT (VSE) WHICH CONSISTS OF PLATE {C} AND ANGLE {E} MATERIAL, AS REQUIRED

{G} LONGITUDINAL ANGLE WELDED TO THE OUTSIDE FACE OF SIDE PLATE {A}, AS REQUIRED

LONGITUDINAL ANGLE WELDED TO THE BOTTOM BEAM FLANGE (OR TOP BEAM FLANGE AS REQUIRED)

HORIZONTAL PLATE WELDED TO THE OUTSIDE FACE OF SIDE PLATE {A}, AS REQUIRED

FILLET WELD CONNECTING SIDE PLATE {A} TO HORIZONTAL SHEAR PLATE {D} OR COLUMN FILLET (AND/OR FLARE BEVEL) WELD CONNECTING INSIDE FACE OF SIDE PLATE {A} TO COLUMN

FILLET WELD CONNECTING HORIZONTAL SHEAR PLATE (D) TO COLUMN, AS REQUIRED

FILLET WELD TO CONSTRUCT VSE {F} AND TO CONNECT IT TO THE WEB OF THE BEAM, AS REQUIRED

FILLET (AND/OR PJP) WELD CONNECTING BEAM FLANGE TIPS TO COVER PLATE {B} AND/OR LONGITUDINAL ANGLE {H}, AS REQUIRED

{5a} FILLET WELD CONNECTING OUTSIDE FACE OF BEAM FLANGE TO COVER PLATE {B} AND/OR LONGITUDINAL ANGLE {H}, AS REQUIRED

({5p}) PJP WELD CONNECTING ANGLE {H} TO BEVELED BEAM FLANGE

(5b) FILLET WELD CONNECTING COVER PLATE (B) EDGE TO TOP FACE OF BEAM FLANGE, ACROSS ITS WIDTH

{8} FILLET (AND/OR PJP) WELD CONNECTING LONGITUDINAL ANGLE {G} (AND/OR PLATE {T}) TO SIDE PLATE {A}, AS REQUIRED

({8p}) PJP WELD CONNECTING PLATE {T} TO SIDE PLATE {A} AND/OR CONNECTING BUILT UP ANGLE {H} PLATES TOGETHER, AS REQUIRED

{9} FILLET WELD CONNECTING SIDE PLATE {A} TO COLUMN FACE, WRAPPED AROUND THREE SIDES OF SIDE PLATE {A}

{10} FILLET WELD TO CONSTRUCT SIDE PLATE SLOTTED INTERLOCK ASSEMBLY

[10p] PJP WELD TO CONSTRUCT SIDE PLATE SLOTTED INTERLOCK ASSEMBLY

{10r} REINFORCING FILLET WELD TO CONSTRUCT SIDE PLATE SLOTTED INTERLOCK ASSEMBLY

3. ALPHA DESIGNATORS, USED HEREIN TO SIMPLIFY THE IDENTIFICATION OF DIMENSIONS OF THE SIDEPLATE CONNECTIONS, ARE DEFINED BELOW: GAP PHYSICAL SEPARATION BETWEEN THE END OF THE MOMENT FRAME BEAM AND THE ADJOINING FACE OF THE COLUMN FLANGE

A EXTENSION OF SIDE PLATE {A} FROM THE FACE OF THE COLUMN

B DEPTH OF SIDE PLATE {A}

C LENGTH OF COVER PLATE {B} AND/OR LONGITUDINAL ANGLE {H}

D LENGTH OF SLOT FROM THE TOE OF THE RADIUS IN THE COVER PLATE {B}, AS REQUIRED

EDGE DISTANCE OF BOLT HOLES IN COVER PLATE {B}, AS REQUIRED

G GAGE DISTANCE TO CENTERLINE OF BOLT HOLES IN ANGLES (G) AND (H), AND PLATE (T), AS REQUIRED

H ADDED DIMENSION TO COLUMN FLANGE WIDTH TO DEFINE TOTAL COVER PLATE (B) WIDTH

DISTANCE FROM END OF THE BEAM TO CENTERLINE OF VERTICAL BOLT HOLES IN VSE {F}, AS REQUIRED

RADIUS OF SLOT DIMENSION IN COVER PLATE {B}

S HORIZONTAL SPACING BETWEEN BOLT HOLES

ADDED DIMENSION TO COLUMN FLANGE WIDTH FOR ALLOWABLE SPREAD OF SIDE PLATES (A)

a. ALL PLATE MATERIAL SHALL HAVE A MINIMUM YIELD STRENGTH (F_y) OF 50 KSI. b. ANGLE AND BAR MATERIAL SHALL HAVE A HIGH STRENGTH STEEL SPECIFICATION AND SHALL HAVE A MINIMUM YIELD STRENGTH (Fy) OF 50

2. <u>HIGH STRENGTH BOLTS/FASTENERS</u>:
a. BOLTS SHALL BE TYPE 1 OR TYPE 3 AND SHALL BE A490 HEAVY HEX, F2280 TWIST-OFF-TYPE TENSION-CONTROL BOLT ASSEMBLIES, OR F3148 FIXED SPLINE BOLT ASSEMBLIES. THE BOLT HEAD SHALL BE DISTINCTIVELY MARKED WITH A MINIMUM MARKING OF A490, A490TC, OR 144 RESPECTIVELY. AN ALTERNATIVE DESIGN THAT MEETS THE REQUIREMENTS OF RCSC SECTION 2.8 MAY BE USED, WITH THE WRITTEN APPROVAL FROM SIDEPLATE SYSTEMS, INC.

b. WASHERS SHALL BE ORDINARY THICKNESS AND ASTM F436 TYPE 1 OR TYPE 3.

c. NUTS SHALL BE ASTM A563 GRADE DH OR DH3. d. THE BOLT ASSEMBLY SHALL BE COVERED IN A LIGHT PROTECTIVE OIL. F2280 AND F3148 ASSEMBLIES SHALL ONLY BE LUBRICATED BY THE

e. THE MILL TEST REPORT (MTR) MUST HAVE DOCUMENTED LOT TRACEABILITY, STATEMENT OF DIMENSIONAL RESULTS, FULL CHEMICAL AND MECHANICAL TEST RESULTS TO THE SPECIFICATIONS ABOVE. f. THE USE OF FINGER SHIMS ARE ACCEPTABLE PER BOLTING SECTION 8.

3. ROLLED SHAPES: a. ALL ROLLED SHAPES USED FOR COLUMNS AND BEAMS IN CONSTRUCTING SIDEPLATE MOMENT FRAMES SHALL BE ASTM A992 GRADE 50

4. <u>HSS TUBE SHAPES</u>:
a. ALL HSS SHAPES USED FOR COLUMNS AND BEAMS IN CONSTRUCTING SIDEPLATE MOMENT FRAMES SHALL, AS A MINIMUM, BE ASTM A500 GRADE B OR GRADE C OR ASTM1085.

1. THE STEEL FABRICATION AND ERECTION SUBCONTRACTORS SHALL EMPLOY A DISTORTION CONTROL PROGRAM PRIOR TO THE START OF SIDEPLATE MOMENT FRAME FABRICATION. THE DISTORTION CONTROL PROGRAM SHALL BE IN ACCORDANCE WITH THE PROVISIONS OF AWS D1.1 SECTION 5.21 AND 5.22 TO ENSURE THAT THE FOLLOWING ARE MAINTAINED: DIMENSIONAL ACCURACY FRAMING AND ALIGNMENT TOLERANCES

 COMPLIANCE WITH AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES, SECTION 7.0, ERECTION PROVISIONS CONTROL OF DISTORTION AND WELD SHRINKAGE

1. WELDER QUALIFICATION: THE PERFORMANCE OF ALL WELDERS, WELDING OPERATORS AND TACK WELDERS SHALL BE QUALIFIED IN CONFORMANCE WITH AWS D1.1, SECTION 4, PART C TO DEMONSTRATE ABILITY TO PRODUCE SOUND WELDS.

BOLTING

1. BOLTS/FASTENERS SHALL BE INSTALLED TO PRETENSIONED CONDITION USING ONE OF THE METHODS PRESCRIBED HERE: TURN-OF-NUT (A490), CALIBRATED WRENCH (A490), TWIST-OFF-TYPE TENSION-CONTROL BOLT (F2280), OR TORQUE AND ANGLE METHOD(F3148). 2. FOR ALL PRETENSIONING METHODOLOGIES, ALL FASTENER ASSEMBLIES WITHIN THE JOINT SHALL FIRST BE BROUGHT TO A SNUG TIGHT CONDITION, FOLLOWED BY A SYSTEMATIC PRETENSIONING PROCESS. PRETENSIONING SHALL BEGIN AT THE MOST RIGID PART OF THE JOINT AND CONTINUE IN A MANNER THAT WILL MINIMIZE THE RELAXATION OF PREVIOUSLY PRETENSIONED FASTENERS, UNTIL THE CONNECTED PLIES ARE IN AS FIRM CONTACT AS POSSIBLE.

3. REUSE OF A490, F2280, AND F3148 BOLT ASSEMBLIES SHALL NOT BE ALLOWED. TOUCHING UP OR RE-TIGHTENING BOLTS THAT MAY HAVE BEEN LOOSENED BY THE INSTALLATION OF ADJACENT BOLTS SHALL NOT BE CONSIDERED TO BE A REUSE. 4. ALL BOLT HOLES SHALL BE ALIGNED TO PERMIT INSERTION OF THE BOLTS WITHOUT UNDUE DAMAGE TO THE THREADS.

5. THE BOLT LENGTH USED SHALL BE SUCH THAT THE BOLT THREAD EXTENDS BEYOND OR IS AT LEAST FLUSH WITH THE OUTER FACE OF THE NUT WHEN PROPERLY INSTALLED. 6. FASTENER COMPONENTS SHALL BE PROTECTED FROM DIRT AND MOISTURE IN CLOSED CONTAINERS AT THE SITE OF INSTALLATION.

7. F2280 OR F3148 ASSEMBLIES AND ALTERNATIVE DESIGN FASTENERS THAT MEET THE SPECIFIED REQUIREMENTS PREVIOUSLY MENTIONED SHALL NOT BE RE-LUBRICATED. EXCEPT BY THE MANUFACTURER. 8. FINGER SHIMS MAY BE USED UP TO 1/4 INCH WITHOUT RESTRICTION, SHIM REQUIREMENTS GREATER THAN 1/4 INCH SHALL BE SUBMITTED TO

SIDEPLATE SYSTEMS INC FOR APPROVAL PRIOR TO USE WASHERS SHALL BE ASTM F436 ORDINARY THICKNESS AND SHALL BE USED UNDER THE NUT OF THE FASTENER ASSEMBLY SO AS TO PROVIDE A HARDENED NON-GALLING SURFACE OF THE TURNED ELEMENT. WHEN USING THE TURN-OF-NUT OR CALIBRATED WRENCH METHOD, THE TURNED ELEMENT MUST BE THE SAME AS WAS USED WHEN PERFORMING PREINSTALLATION VERIFICATION TESTING.

1. THE FABRICATOR AND ERECTOR SHALL BE RESPONSIBLE FOR QUALITY CONTROL BY PROVIDING, AS A MINIMUM, IN-PROCESS VISUAL INSPECTION OF ALL FABRICATION AND ERECTION ACTIVITIES TO ENSURE THAT MATERIALS AND WORKMANSHIP MEET THE REQUIREMENTS OF THE CONTRACT DOCUMENTS, AND SHALL INCLUDE WORK PERFORMED PRIOR TO ASSEMBLY. SUCH WORK SHALL INCLUDE, BUT NOT BE LIMITED TO, VERIFYING THAT EFFECTIVE PROCEDURES AND METHODS HAVE BEEN EMPLOYED IN THE FORM OF A DISTORTION CONTROL PROGRAM TO ACCOUNT FOR AND COUNTERACT THE EFFECTS OF WELD SHRINKAGE, EXISTING BEAM SWEEP AND CAMBER, AND CHANGES IN MOMENT FRAME GEOMETRY DUE TO SKEWED AND CURVED DESIGN CONFIGURATIONS (AS OCCURS), TO ENSURE COMPLIANCE WITH SPECIFIED ERECTION AND ALIGNMENT TOLERANCES. QC INSPECTION SHALL INCLUDE HOLD POINTS FOR THE FOLLOWING:

VERIFICATION THAT ACTUAL COLUMN FLANGE WIDTH IS AT LEAST NOMINAL COLUMN FLANGE WIDTH WHERE THE SIDE PLATES {A} ARE TO BE INSTALLED. IN THE UNLIKELY EVENT ACTUAL COLUMN FLANGE WIDTH IS LESS THAN NOMINAL, BUT WITHIN AISC STANDARD MILL TOLERANCES (-3/16 INCH MAX), CONTACT SIDEPLATE SYSTEMS, INC FOR APPROPRIATE RECOMMENDATIONS. MINIMUM CLEAR DIMENSION SHALL BE VERIFIED AFTER PLACEMENT OF WELD {2}, COOLING OF WELD {2}, AND REMOVAL OF TEMPORARY SHOP CONSTRUCTION AID(S). VERIFY THAT A MINIMUM ACTUAL COLUMN FLANGE WIDTH DIMENSION OCCURS ANYWHERE IN BETWEEN

THE SIDE PLATES {A} FROM TOP TO BOTTOM. THE SIDE PLATES SHALL BE PARALLEL TO ONE ANOTHER. IN NO CASE SHALL THEY BE LESS THAN THE ACTUAL COLUMN FLANGE WIDTH. MAXIMUM SPREAD DIMENSION OF SIDE PLATE {A} SHALL NOT EXCEED ACTUAL COLUMN FLANGE WIDTH PLUS THE SCHEDULED SPREAD DIMENSION Y. THE FIELD CONSTRUCTION AID SHALL BE PLACED AND HOLD THE SIDE PLATES IN THIS FLARED CONDITION UNTIL THE BEAM HAS BEEN SAFELY ERECTED. IN NO CASE SHALL THE SPREAD CAUSE PERMANENT DEFORMATION IN THE SIDE PLATES. 4. VERIFICATION OF BOLT HOLE ELEVATION AND SPACING FOR POSITION OF SIDE PLATE (A) AND PROPER POSITION AND ELEVATION OF

VERIFICATION OF PERPENDICULAR ALIGNMENT BETWEEN THE TOP COVER PLATE (B) AND BOTTOM ANGLES (H) TO THE WEB OF THE BEAM, TO MINIMIZE, IF NOT ELIMINATE, ANY MISALIGNMENT OF BOLT HOLES DUE TO BEAM FLANGE TILT WHEN THE BEAM HAS BEEN

2. VERIFICATION OF BOLT HOLE SPACING AND POSITION ON COVER PLATE (B) AND ANGLES (H). CONSIDERATION SHALL BE GIVEN TO THE CUPPING EFFECT OF THE TOP COVER PLATE {B}, DUE TO WELD SHRINKAGÉ. VERIFICATION OF THE DISTANCE BETWEEN EXTERIOR ANGLE {H} FACES AND THEIR RESPECTIVE BOLT HOLE PLACEMENT TO EACH OTHER (VERTICALLY AND HORIZONTALLY).

4. VERIFICATION THAT IN NO CASE SHALL THE OUTSIDE FACE OF VSE {F} EXTEND BEYOND THE OUTSIDE FACES OF THE LONGITUDINAL VERIFICATION THAT VERTICAL PLACEMENT OF VSE {F} IS IN THE CORRECT LOCATION.

2. FILLET WELD FIT-UP TOLERANCES: a. THE PARTS TO BE JOINED BY FILLET WELDS SHALL BE BROUGHT INTO AS CLOSE CONTACT AS PRACTICABLE, USING AS NECESSARY SUITABLE CLAMPING MEANS. THE ROOT OPENING (I.E., THE FIT-UP GAP) SHALL NOT EXCEED 1/4 INCH. FOR FILLET WELD ROOT GAPS GREATER THAN 1/16 INCH, THE LEG SIZE (I.E., THE SPECIFIED SIZE) OF FILLET WELD SHALL BE INCREASED BY THE AMOUNT OF THE ROOT

a. THE ROUGHNESS OF ALL THERMAL-CUT SURFACES SHALL BE NO GREATER THAN AN ANSI SURFACE ROUGHNESS VALUE OF 1000 MICRO-INCHES. ROUGHNESS EXCEEDING THIS VALUE AND NOTCHES OR GOUGES NOT MORE THAN 3/16 INCH DEEP SHALL BE REMOVED BY MACHINING OR GRINDING. NOTCHES OR GOUGES IN THE THERMALLY CUT EDGES DEEPER THAN 3/16 INCH SHALL BE REPAIRED PER AWS. 4. TENSION CALIBRATION FOR PRE-INSTALLATION

TENSION CALIBRATION SHALL BE USED TO CONFIRM THE SUITABILITY OF THE COMPLETE FASTENER ASSEMBLY, AND THE PROCEDURE TO BE USED BY THE BOLTING CREW.

GREATER THAN 1/4 INCH, CONTACT SIDEPLATE SYSTEMS, INC.

PIPE/TUBE COLUMN APPLICATIONS FOR NO LESS THAN THE REQUIRED RATED TIME

ANGLES (G).

QUALITY ASSURANCE IN ADDITION TO ALL OTHER QUALITY ASSURANCE INSPECTION ACTIVITIES, THE OWNER'S VERIFICATION INSPECTOR SHALL BE RESPONSIBLE FOR: a. TO ASSURE THE PROPER AMPERAGE AND VOLTAGE OF THE WELDING PROCESS, THE USE OF HAND HELD CALIBRATED AMP AND VOLT METERS SHALL BE USED. THIS EQUIPMENT SHALL BE USED BY THE FABRICATOR AND THE INSPECTOR. AMPERAGE AND VOLTAGE SHALL BE MEASURED NEAR THE ARC. TRAVEL SPEED AND ELECTRODE STICK OUT SHALL BE VERIFIED TO BE IN COMPLIANCE WITH THE APPROVED

b. VISUAL INSPECTION SHALL BE PERFORMED ON ALL SHOP WELDS. . EACH WELDER EMPLOYED ON THE PROJECT SHALL UNDERSTAND ALL THE REQUIREMENTS OF THE WELDING PROCEDURE

SPECIFICATION(S) BEFORE WELDING ON THE PROJECT. d. AS-BUILT BEAM TO COLUMN GAP PER CONNECTION SCHEDULE IS ALLOWED TO BE INSTALLED WITH A TOLERANCE OF PLUS OR MINUS 1/2 INCH.

a. THE SURFACES ADJACENT TO THE BOLT HEAD AND NUT SHALL BE FREE OF DIRT AND OTHER FOREIGN MATERIAL OTHER THAN THE

b. FAYING SURFACES ARE PERMITTED TO BE UNCOATED AND COATED WITH ANY COATINGS OF ANY FORMULATION OR GALVANIZATION. . AFTER THE CONNECTIONS HAVE BEEN ASSEMBLED, VISUALLY ENSURE THAT THE PLIES OF THE CONNECTED ELEMENTS HAVE BEEN BROUGHT INTO AS CLOSE OF CONTACT AS PRACTICABLE WITH ONE ANOTHER. GAPS UP TO 1/8 INCH BETWEEN THE SURFACES SHALL BE ALLOWED. GAPS GREATER THAN 1/8 INCH UP TO 1/4 INCH SHALL HAVE FINGER SHIMS INSTALLED BEFORE PRETENSIONING. FOR GAPS

1. SIDEPLATE CONNECTIONS REQUIRING THIS TYPE OF FINISH SHALL FOLLOW THE SAME CONSTRUCTION SEQUENCING AS PREVIOUSLY OUTLINED WITH THE FOLLOWING MODIFICATIONS: a. HORIZONTAL SHEAR PLATES (D) SHALL HAVE AN INCREASED CLIP SIZE WHICH SHALL BE 1 5/8 INCH BY 1 5/8 INCH TO PROVIDE ADEQUATE VENTILATION AND DRAINAGE. CONTACT SIDEPLATE SYSTEMS, INC. IN THE EVENT THAT THE GALVANIZING CONTRACTOR SPECIFICATIONS REQUIRE A LARGER OPENING THAN THAT SPECIFIED HEREIN. b. SEAL WELDING SHALL BE ALLOWED ON THE PLATES (B) AND ANGLES.

c. ANY DEVIATIONS TO THESE MODIFICATIONS SHALL BE COORDINATED WITH SIDEPLATE SYSTEMS, INC. AND THE SEOR.

1. WHEN REQUIRED BY THE GOVERNING CODE FOR CERTAIN TYPES OF CONSTRUCTION, SIDEPLATE CONNECTIONS SHALL HAVE A FIRE-RESISTANCE RATING LIKE THAT OF A STEEL "STRUCTURAL FRAME". THE MINIMUM THICKNESS OF SPRAY-APPLIED FIRE-RESISTIVE MATERIAL (SFRM) FOR STEEL SIDEPLATE CONNECTIONS PLATES THAT ARE NOT ENCASED IN CONCRETE, SHALL BE DETERMINED JUST LIKE THAT OF A PIPE/TUBE COLUMN SECTION WITH A CONSTANT STEEL WALL THICKNESS USING THE THICKNESS OF SIDE PLATE {A} FOR EACH SIDEPLATE CONNECTION ID PER THE SIDEPLATE CONNECTION SCHEDULE, WHICH ARE UNIFORMLY HEATED AND PROTECTED (THE FIRE EXPOSURE OF A PIPE/TUBE COLUMN IS DIRECTLY ANALOGOUS TO A PLATE WITH A 1-SIDED FIRE EXPOSURE AND PROTECTION). THE SFRM SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ASTM E119 AND LISTED FOR FIRE RESISTIVE

. AS REQUIRED, WHEN NO VERTICAL SHEAR ELEMENT {F} EXISTS IN THE BEAM, SPRAY THE MINIMUM THICKNESS OF SFRM BETWEEN INSIDE OF SIDE PLATE {A} AND BEAM WEB COVERING ALL SURFACES INCLUDING COLUMN FLANGE. NOTE: THIS DOES NOT NECESSITATE FILLING THE

4. WHEN VERTICAL SHEAR ELEMENT {F} IS USED, THE CONTRACTOR SHALL PROVIDE THE MEANS, TYPICALLY DONE WITH A LAYERING TECHNIQUE, FOR FIREPROOFING ACROSS THE BOTTOM OF THE GAP. 5. SEE GRAPHIC NUMBER 10 IN FIELD ERECTION OF THE SIDEPLATE BOLTED SYSTEM FOR FIREPROOFING ACROSS THE BOTTOM OF THE GAP.

1. IN ORDER TO SAFEGUARD THE AUTHORIZED USE AND INTELLECTUAL PROPERTY OF THE PATENTED SIDEPLATE CONNECTION TECHNOLOGY, THE STEEL FABRICATION SUBCONTRACTOR SHALL SATISFY THE FOLLOWING REQUIREMENTS:

a. A NOTICE OF INTELLECTUAL PROPERTY, IDENTICAL TO THAT PROVIDED ON THIS SHEET, SHALL BE AFFIXED ON EACH SHEET OF SHOP DETAIL AND FIELD ERECTION DRAWINGS CONTAINING SIDEPLATE SYSTEM INFORMATION WHICH DISCLOSES IN ANY WAY THE SIDEPLATE CONNECTION CONCEPT PRIOR TO RELEASING SUCH INFORMATION FOR ITS INTENDED USE. SUCH NOTICE SHALL BE PROVIDED TO THE STEEL FABRICATION SUBCONTRACTOR BY SIDEPLATE SYSTEMS, INC. IN A FORMAT (E.G. WORD OR AUTOCAD) SUITABLE TO THE NEEDS OF

b. PATENT LABELS SHALL BE APPLIED ON THE OUTSIDE FACE OF ONE OF THE TWO BOTTOM HORIZONTAL SHEAR PLATES (D) OF EACH MOMENT CONNECTION AND ON ONE END OF THE BEAM WEB IN COMPLIANCE WITH THE PATENT AND INTELLECTUAL PROPERTY LAWS.

1. THE CONTRACTOR SHALL ASSUME FULL AND COMPLETE RESPONSIBILITY FOR THE MEANS AND METHODS OF CONSTRUCTING THE STEEL FRAME USING THE SIDEPLATE BOLTED SYSTEM. CONSTRUCTION MEANS AND METHODS SHALL BE COMPLIANT WITH THE CURRENT PROVISIONS OF AWS D1.1, THE AISC 360 CODE OF STANDARD PRACTICE, THE RCSC HIGH-STRENGTH BOLTING SPECIFICATIONS, AND THE CONSTRUCTION GUIDELINES PROVIDED HEREIN AND SHALL INCLUDE, BUT ARE NOT LIMITED TO: a. DIMENSIONAL VERIFICATION AND CONTROL

FABRICATION AND ERECTION PROCEDURES (INCLUDING METHODS FOR CONTROLLING DISTORTION DUE TO WELD SHRINKAGE, AND FOR CONTROLLING COMBINED MILL, FABRICATION AND ERECTION TOLERANCES) CONSTRUCTION AIDS SUCH AS ERECTION RIGGING AND SHORING

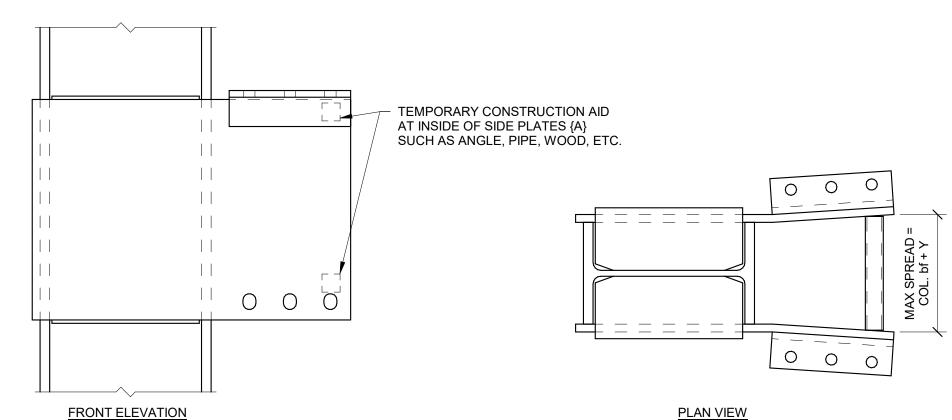
d. PROPER BOLT HOLE ALIGNMENT e. PROPER PRETENSIONING OF BOLTS

THE SEQUENCE OF CONSTRUCTION OPTIONS PROVIDED BELOW IN THESE CONSTRUCTION GUIDELINES HAVE PROVEN TO BE SUCCESSFUL BY STEEL FABRICATORS AND ERECTORS TO COST EFFICIENTLY CONSTRUCT THE BOLTED SIDEPLATE CONNECTION SYSTEM. VARIATIONS TO THESE CONSTRUCTION SEQUENCE OPTIONS PROVIDED BELOW SHALL BE SUBMITTED FOR REVIEW AND DISPOSITION TO SIDEPLATE SYSTEMS, INC. . A PRE-FABRICATION COORDINATION MEETING WITH A SIDEPLATE SYSTEMS, INC. REPRESENTATIVE IS REQUIRED FOR ALL PROJECTS. THE PRE-

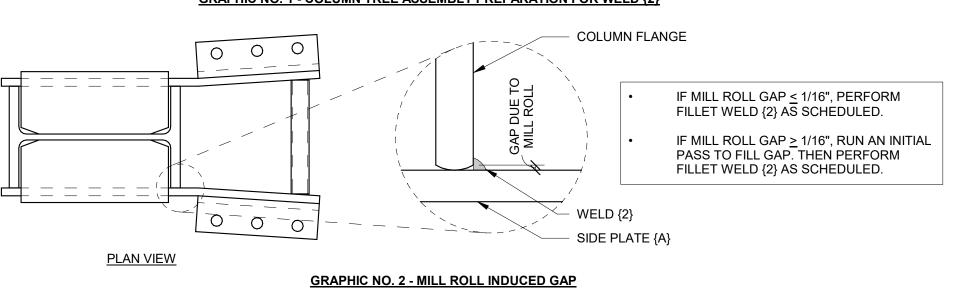
SHOP FABRICATION OF THE SIDEPLATE BOLTED SYSTEM 1. WATCH OUR SIDEPLATE COLUMN ASSEMBLY VIDEO AT

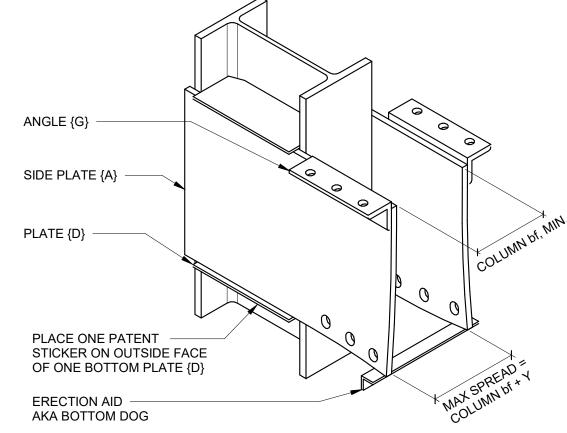
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FABRICATION COORDINATION MEETING IS INTENDED TO SHARE BEST PRACTICES AND COMMON MISTAKES TO AVOID.



GRAPHIC NO. 1 - COLUMN TREE ASSEMBLY PREPARATION FOR WELD {2}

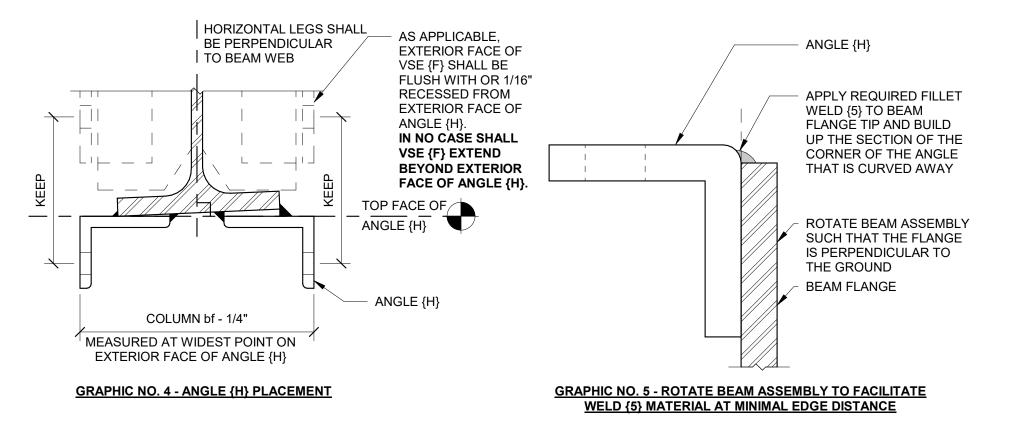




GRAPHIC NO. 3 - COMPLETED SIDEPLATE BOLTED COLUMN TREE ASSEMBLY

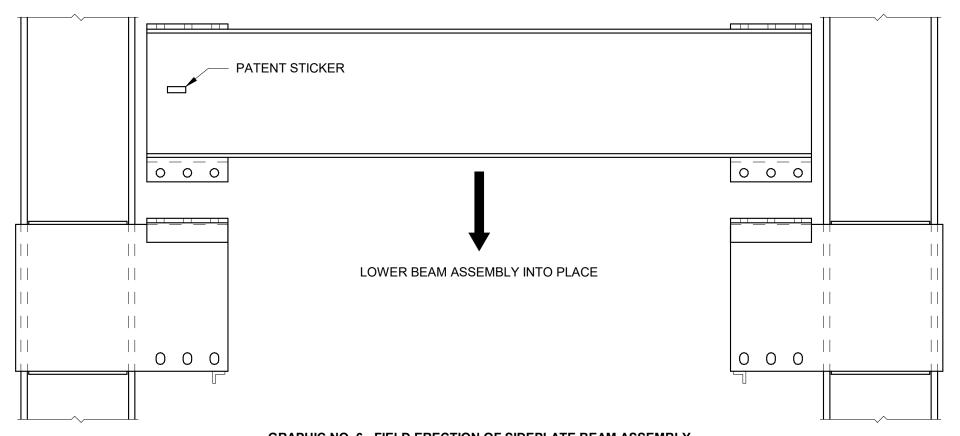
2. WATCH OUR SIDEPLATE BEAM ASSEMBLY VIDEO AT

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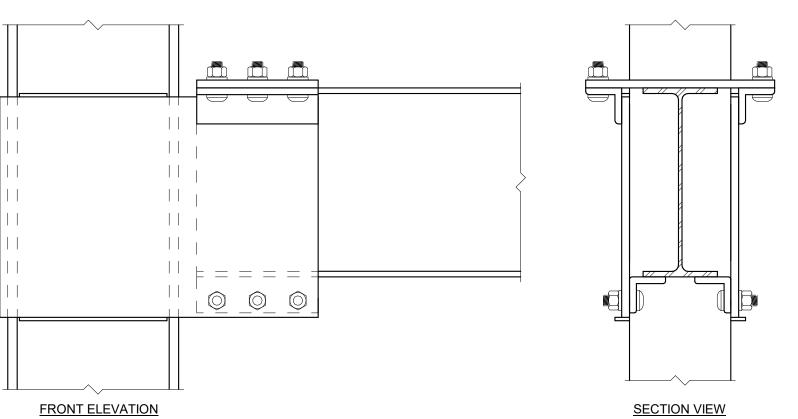


FIELD ERECTION OF SIDEPLATE BOLTED SYSTEM 3. WATCH OUR SIDEPLATE FIELD ERECTION VIDEO AT

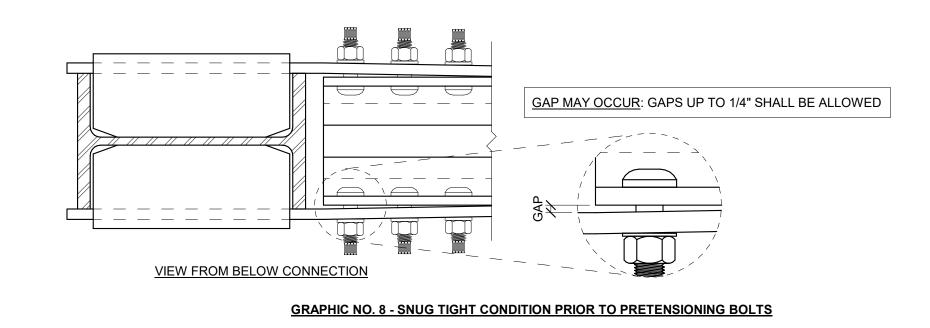
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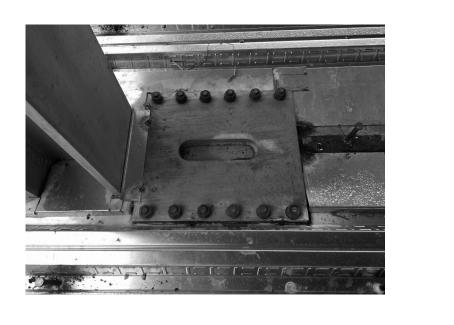


GRAPHIC NO. 6 - FIELD ERECTION OF SIDEPLATE BEAM ASSEMBLY



GRAPHIC NO. 7 - COMPLETED SIDEPLATE BOLTED CONNECTION



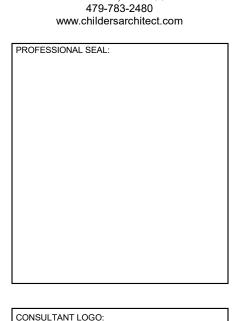


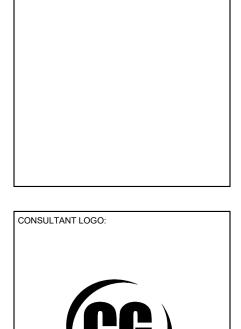
GRAPHIC NO. 9 - TYPICAL GAP CLOSURE AT THE TOP OF THE GAP



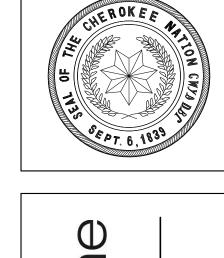
GRAPHIC NO. 10 - FIREPROOFING ACROSS THE BOTTOM OF THE GAP

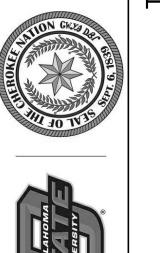


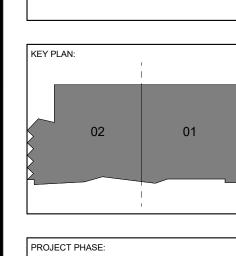


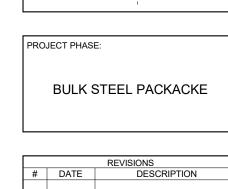


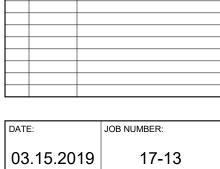












SHEET NUMBER:

SIDEPLATE GENERAL NOTES AND CONSTRUCTION **GUIDELINES**

INTELLECTUAL PROPERTY RIGHTS NOTICE The SIDEPLATE® steel frame connection system is covered by one or more of U.S. Pat. Nos. 6,138,427; 6,516,583; 6,591,573; 7,178,296; 8,122,671; 8,122,672; 8,146,322; 8,176,706; 8,205,408; and 9,091,065 and foreign counterparts. Other U.S. and foreign applications pending. SIDEPLATE® is a registered trademark of MiTek Holdings, Inc., an affiliate of SidePlate Systems, Inc. Copyright © 2019 SidePlate Systems, Inc. All rights reserved. Without limitation, this drawing and the information hereon may be used only following payment of a license fee to SidePlate Systems, Inc. and for the design, construction, operation, repair,

maintenance, restoration or demolition of the building(s) specifically identified.

0 0 0 0 T.O.STEEL 0 0 0 1 1/4" X 1 1/2" ↔ ____ SLOTTED HOLES AT SIDE PLATE {A}, TYPICAL SLOTTED HOLE AT SIDE PLATE {A}, TYPICAL OF BEAM - SIDE PLATE {A} - SIDE PLATE {A} MULTIPLE VERTICAL BOLT HOLES SINGLE VERTICAL BOLT HOLE

NOTE(S):

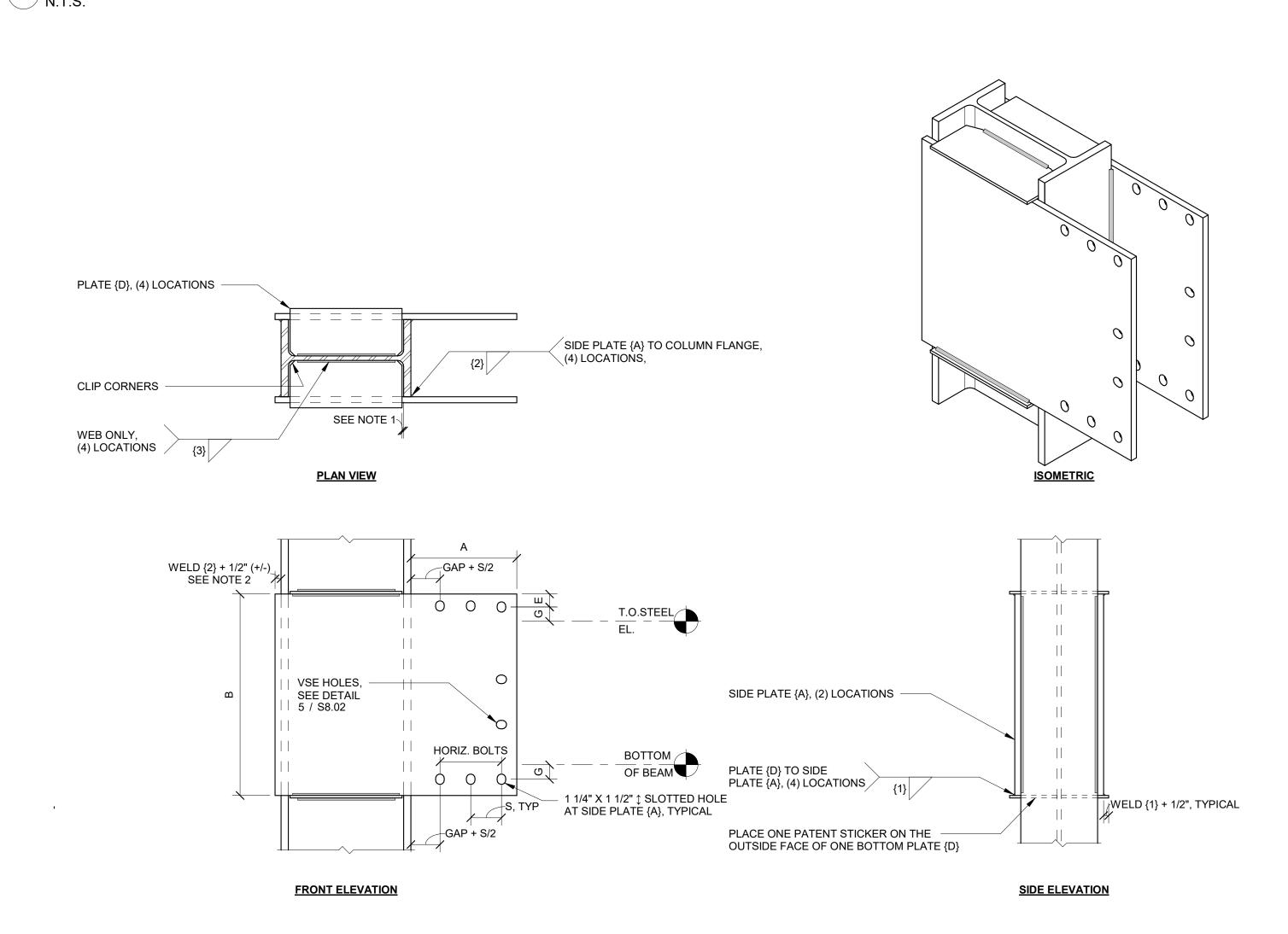
1. SEE COLUMN SCHEDULE FOR BOLT QUANTITY.

5 SIDE PLATE {A} VSE BOLT HOLE DETAIL N.T.S.

		COLUMN PANEL ZONE DESIGN								SIDEPLATE {A} EXTENSION DESIGN										
ID	MEMBER SIZE PLATE THICKNESS			WELD			MISCELLANEOUS			MEMBER SIZE	DIMENSION					MISCELLANEOUS				
	COLUMN	{A}	{D}	{1}	{2}	{3} WEB	{3} FLANGE	PLATE {D} OPTION NUMBER	COORDINATE WITH DETAIL	DESIGN SIDE	BEAM	GAP	А	В	G	S	Y	BOLT Ø	HORIZONTAL BOLTS	VERTICAI BOLTS

4 A TYPE NARROW COLUMN CONNECTION SCHEDULE N.T.S.

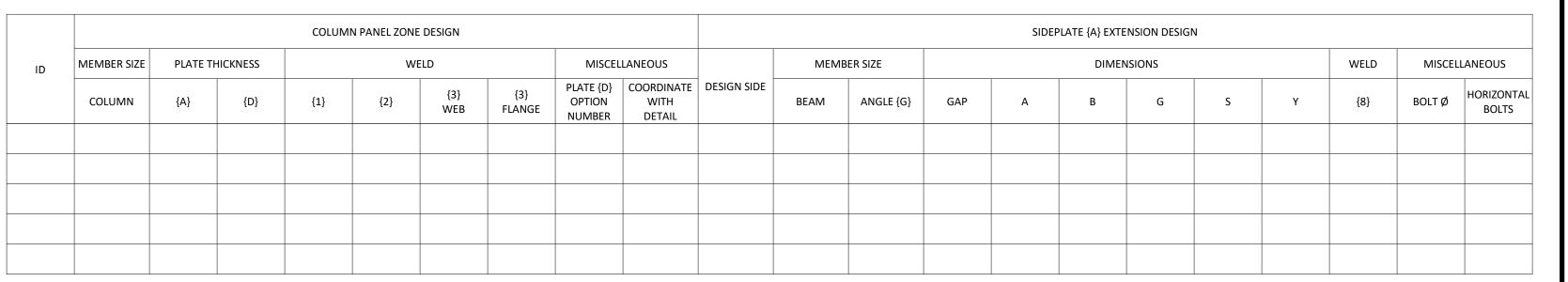
3 A TYPE NARROW BOLTED CONNECTION N.T.S.



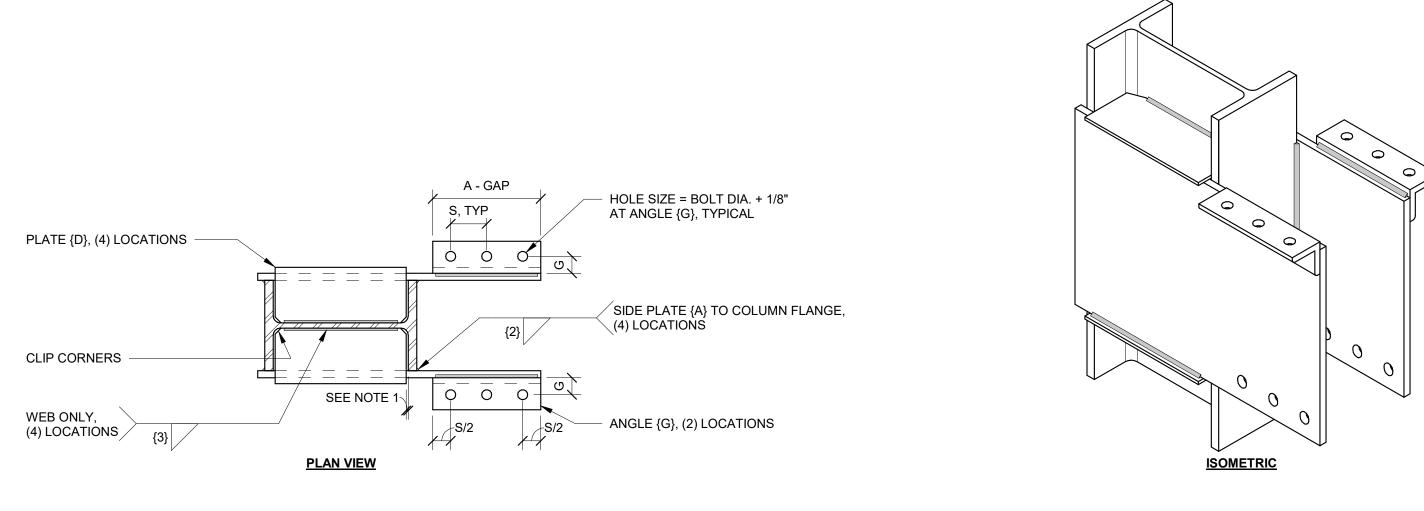
NOTE(S):

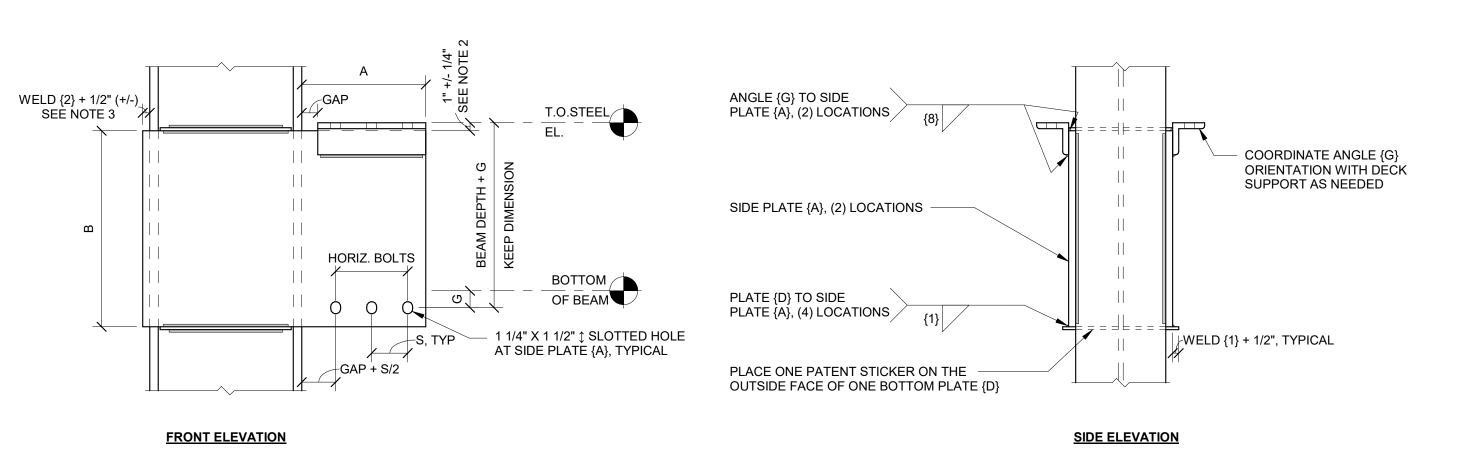
1. DIMENSION BETWEEN PLATE {D} AND INSIDE FACE OF COLUMN FLANGE SHALL NOT EXCEED 1/4 INCH. THIS DIMENSION MAY VARY DEPENDING ON THE DETAILER'S PREFERENCE TO ACCOMMODATE MILL TOLERANCE AND/OR THE UNIFORMITY OF PIECE MARKS. 2. THE 1/2 INCH OVERHANG ON THE SIDE PLATE {A} IS TO ENSURE SUFFICIENT ROOM FOR WELD {2}, THE +/- TOLERANCE IS APPLIED SO THAT IF DESIRED, THE DETAILER CAN MAKE THE SIDE PLATES {A} THE SAME LENGTH WITH SLIGHTLY VARYING COLUMN DEPTHS WITHIN A GROUP OF THE SAME CONNECTION ID'S.

1 A TYPE BOLTED CONNECTION N.T.S.



2 A TYPE COLUMN CONNECTION SCHEDULE N.T.S.

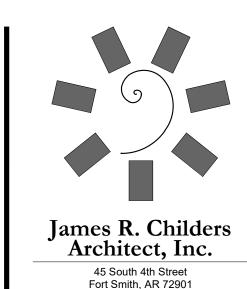




NOTE(S):

1. DIMENSION BETWEEN PLATE {D} AND INSIDE FACE OF COLUMN FLANGE SHALL NOT EXCEED 1/4 INCH. THIS DIMENSION MAY VARY DEPENDING ON THE DETAILER'S PREFERENCE TO ACCOMMODATE MILL TOLERANCE 2. THE +/- 1/4 INCH TOLERANCE FOR PLACEMENT OF ANGLES {G} IS TO ENSURE CORRECT TOP OF STEEL PLACEMENT RELATIVE TO THE CENTERLINE OF THE BOTTOM HORIZONTAL ROW OF BOLT HOLES. THE PLACEMENT OF ANGLES {G} SHALL NEVER BE MEASURED FROM THE BOTTOM EDGE OF SIDE PLATE {A} TO ESTABLISH THE CORRECT TOP OF STEEL.

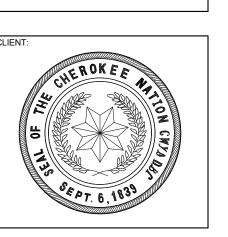
3. THE 1/2 INCH OVERHANG ON THE SIDE PLATE {A} IS TO ENSURE SUFFICIENT ROOM FOR WELD {2}, THE +/- TOLERANCE IS APPLIED SO THAT IF DESIRED, THE DETAILER CAN MAKE THE SIDE PLATES {A} THE SAME LENGTH WITH SLIGHTLY VARYING COLUMN DEPTHS WITHIN A GROUP OF THE SAME CONNECTION ID'S.



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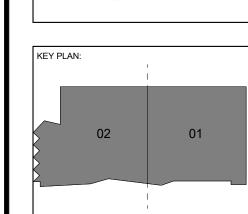


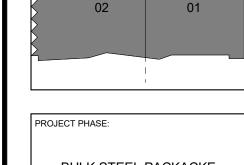


Steopathic
THE CHEROKEE NATIO









PROJECT PHASE:
BULK STEEL PACKACKE

			REVISIONS
1	#	DATE	DESCRIPTION

DATE:	JOB NUMBER:						
03.15.2019	17-13						
SHEET NUMBER:							
S	8.02						

SIDEPLATE COLUMN DETAILS, A TYPE

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> 0 0 0 0 0 0 0 1 1/4" X 1 1/2" ↔ ____ SLOTTED HOLES AT SIDE PLATE {A}, 1 1/4" X 1 1/2" ↔ SLOTTED HOLE AT SIDE PLATE {A}, 0 0 0 0-SIDE PLATE {A} - SIDE PLATE {A} MULTIPLE VERTICAL BOLT HOLES SINGLE VERTICAL BOLT HOLE

5 SIDE PLATE {A} VSE BOLT HOLE DETAIL N.T.S.

NOTE(S):

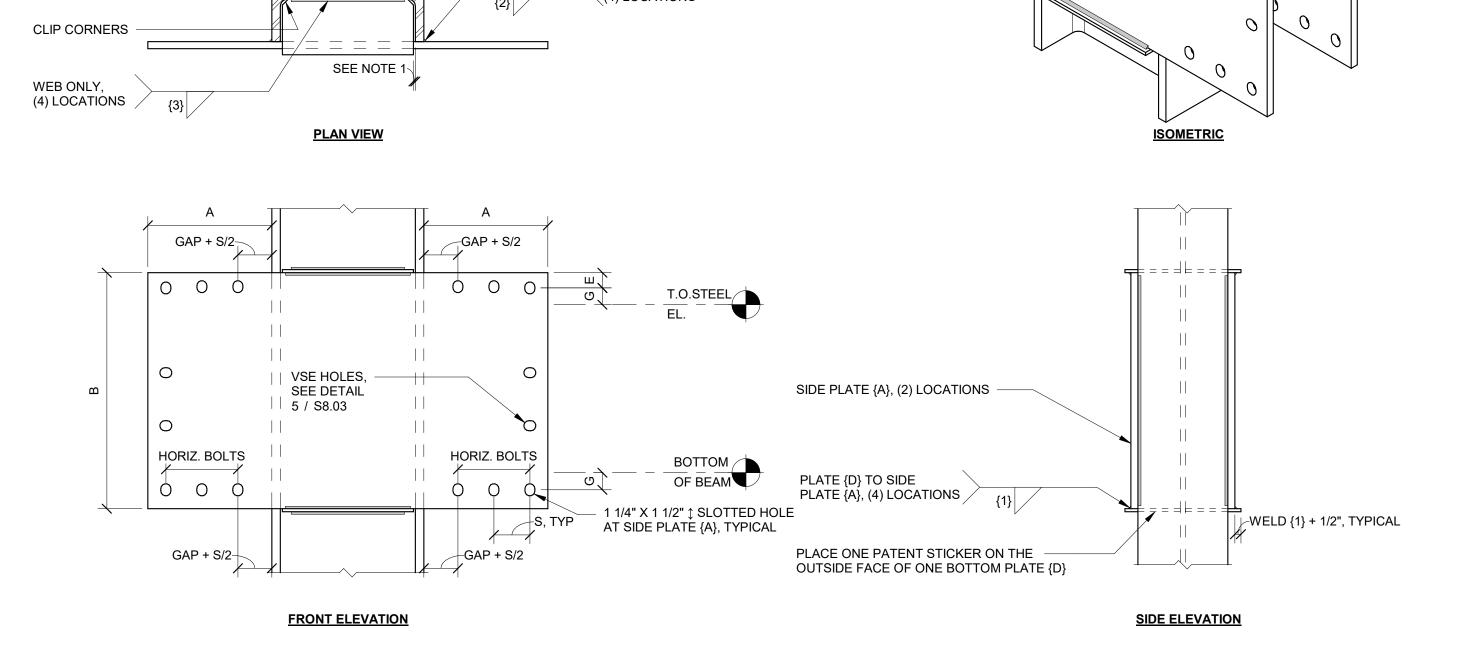
1. SEE COLUMN SCHEDULE FOR BOLT QUANTITY.

	COLUMN PANEL ZONE DESIGN								SIDEPLATE {A} EXTENSION DESIGN											
ID	MEMBER SIZE	PLATE THICKNESS		WELD			MISCE	LLANEOUS		MEMBER SIZE		DIMENSION					MISCELLANEOUS		JS	
	COLUMN	{A}	{D}	{1}	{2}	{3} WEB	{3} FLANGE	PLATE {D} OPTION NUMBER	COORDINATE WITH DETAIL	DESIGN SIDE	BEAM	GAP	А	В	G	S	Y	BOLT Ø	HORIZONTAL BOLTS	VERTICA BOLTS

3 B TYPE NARROW BOLTED CONNECTION N.T.S.

ID	MEMBER SIZE	PLATE TH	HICKNESS	WELD				MISCEL	LANEOUS		MEMBER SIZE		DIMENSION						MISCELLANEOUS		
	COLUMN	{A}	{D}	{1}	{2}	{3} WEB	{3} FLANGE	PLATE {D} OPTION NUMBER	COORDINATE WITH DETAIL	DESIGN SIDE	BEAM	GAP	А	В	G	S	Υ	BOLT Ø	HORIZONTAL BOLTS	VERTICA BOLTS	

E NARROW COLUMN CONNECTION SCHEDULE	
PLATE {D}, (4) LOCATIONS ———	
SIDE PLATE {A} TO COLUMN FLANGE, (4) LOCATIONS	



NOTE(S):

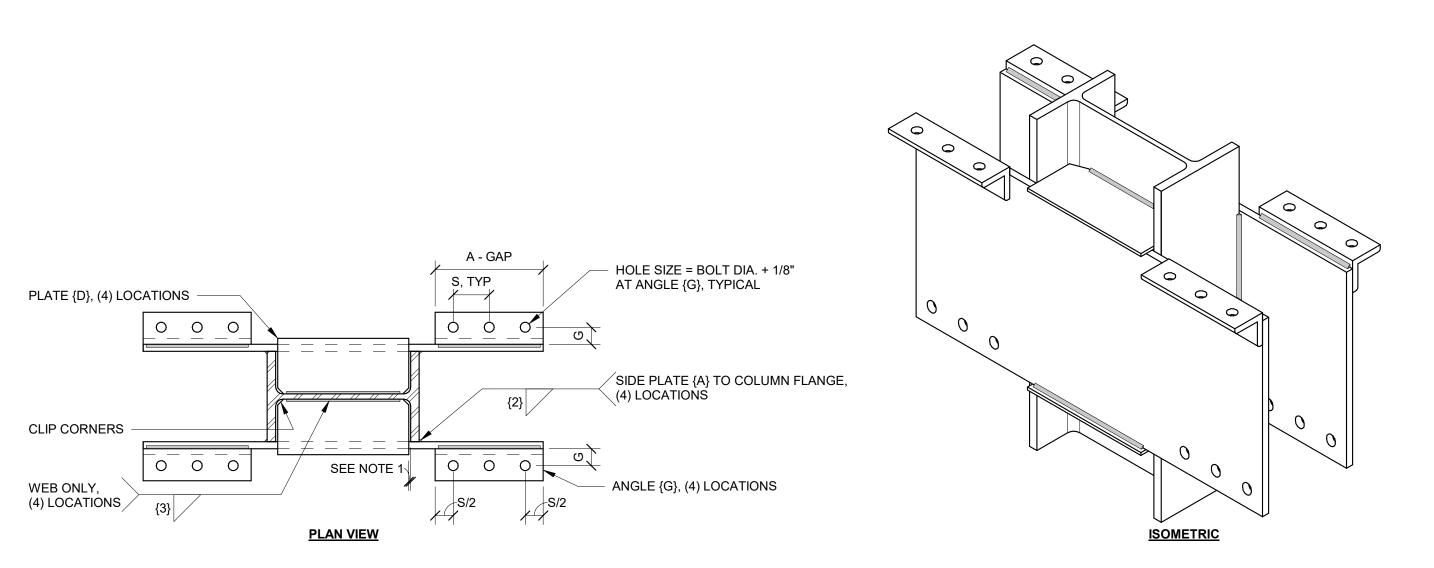
1. DIMENSION BETWEEN PLATE {D} AND INSIDE FACE OF COLUMN FLANGE SHALL NOT EXCEED 1/4 INCH. THIS DIMENSION MAY VARY DEPENDING ON THE DETAILER'S PREFERENCE TO ACCOMMODATE MILL TOLERANCE AND/OR THE UNIFORMITY OF PIECE MARKS.

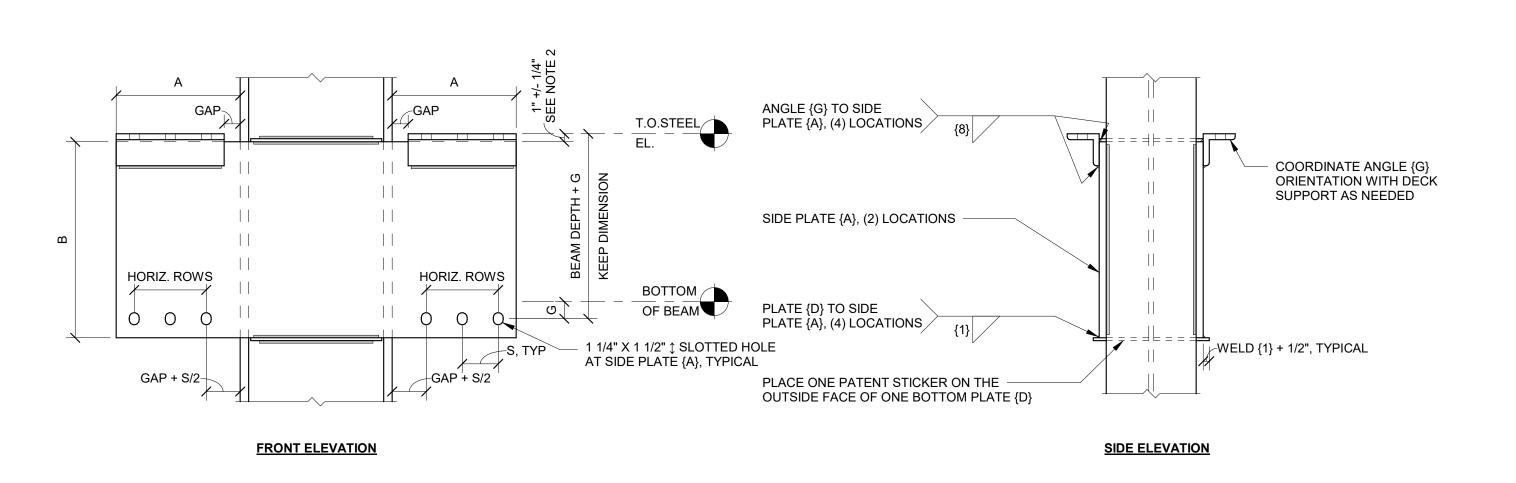
1 B TYPE BOLTED CONNECTION N.T.S.

MEMBER SIZE PLATE THICKNESS MISCELLANEOUS MEMBER SIZE DIMENSIONS WELD MISCELLANEOUS PLATE {D} COORDINATE DESIGN SIDE HORIZONTAL OPTION WITH NUMBER DETAIL BEAM ANGLE {G} B TYPE COLUMN CONNECTION SCHEDULE
N.T.S.

SIDEPLATE {A} EXTENSION DESIGN

COLUMN PANEL ZONE DESIGN



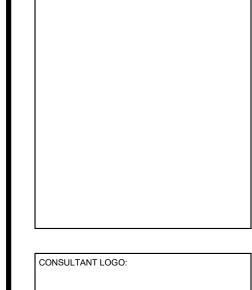


NOTE(S):

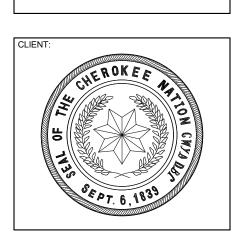
1. DIMENSION BETWEEN PLATE (D) AND INSIDE FACE OF COLUMN FLANGE SHALL NOT EXCEED 1/4 INCH. THIS DIMENSION MAY VARY DEPENDING ON THE DETAILER'S PREFERENCE TO ACCOMMODATE MILL TOLERANCE AND/OR THE UNIFORMITY OF PIECE MARKS.
2. THE +/- 1/4 INCH TOLERANCE FOR PLACEMENT OF ANGLES {G} IS TO ENSURE CORRECT TOP OF STEEL PLACEMENT RELATIVE TO THE CENTERLINE OF THE BOTTOM HORIZONTAL ROW OF BOLT HOLES. THE PLACEMENT

OF ANGLES {G} SHALL NEVER BE MEASURED FROM THE BOTTOM EDGE OF SIDE PLATE {A} TO ESTABLISH THE CORRECT TOP OF STEEL.

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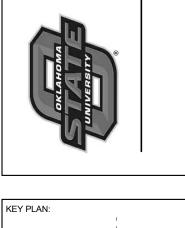


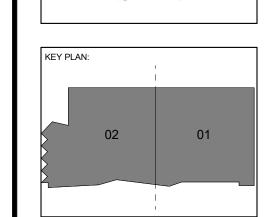


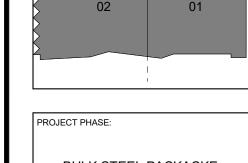








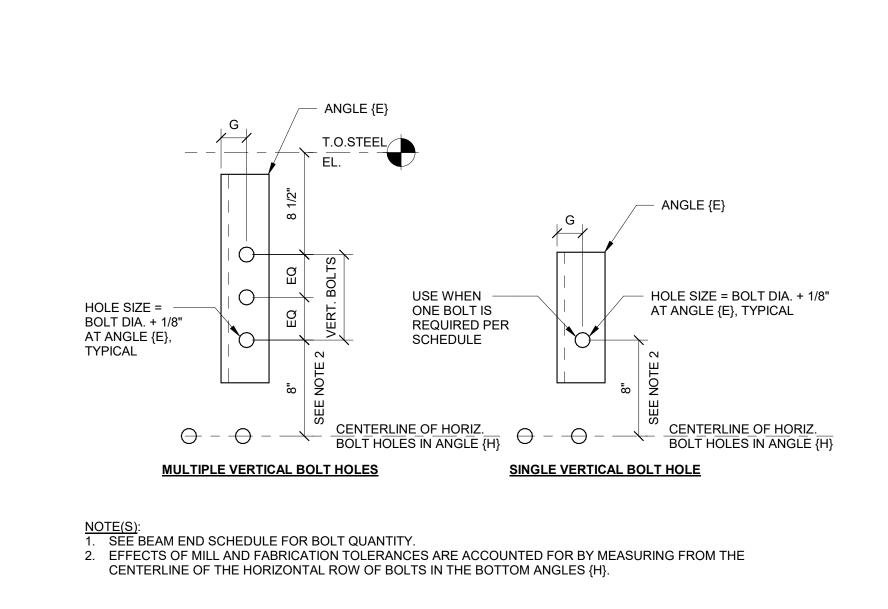




	BULKS	STEEL PACKACKE
		REVISIONS
#	DATE	DESCRIPTION

		REVISIONS
#	DATE	DESCRIPTION

DATE:	JOB NUMBER:									
03.15.2019	17-13									
SHEET NUMBER:										
S8.03										
	36.03									



HORIZONTAL BOLTS

VSE OPTION

SIZE THICKNESS

ANGLE {E}

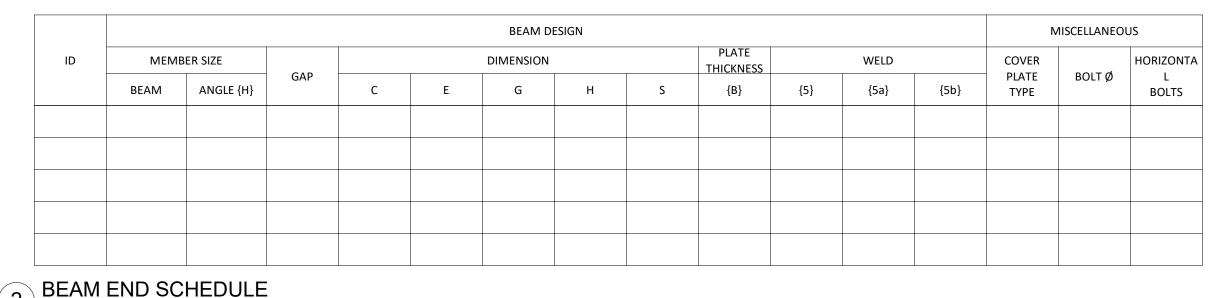
ANGLE {E}

VSE

7 VSE {F} HOLE DETAIL N.T.S.

MISCELLANEOUS

COVER PLATE TYPE



0 0 0

HORIZ. BOLTS

S/2

HORIZ. BOLTS

FRONT ELEVATION

NOTE(S):

1. USE SLOTTED OR RECTANGULAR COVER PLATE {B} PER SCHEDULE. FOR RECTANGULAR COVER PLATE, SEE DETAIL 3 / S8.04

1/2 COL. DEPTH + GAP-

BEAM FLANGE TO PLATE {B}, (2) LOCATIONS

PLACE ONE PATENT STICKER

HOLE SIZE = BOLT DIA. + 1/8" -AT ANGLE {H}, TYPICAL

ANGLE {H}, (2) LOCATIONS (5)

AT ONE END OF BEAM **BEAM FLANGE TO**

ANGLE {H}, (2) LOCATIONS

PLAN VIEW

PLATE {B} TO BEAM FLANGE

✓ PLATE {B} TO BEAM FLANGE

1 1/4" X 1 1/2" ↑ SLOTTED HOLE

AT PLATE {B}, TYPICAL

2 BEAM END SCHEDULE N.T.S.

S/2

NOTE(S):
1. FOR ITEMS NOT NOTED, SEE DETAIL 1 / S8.04

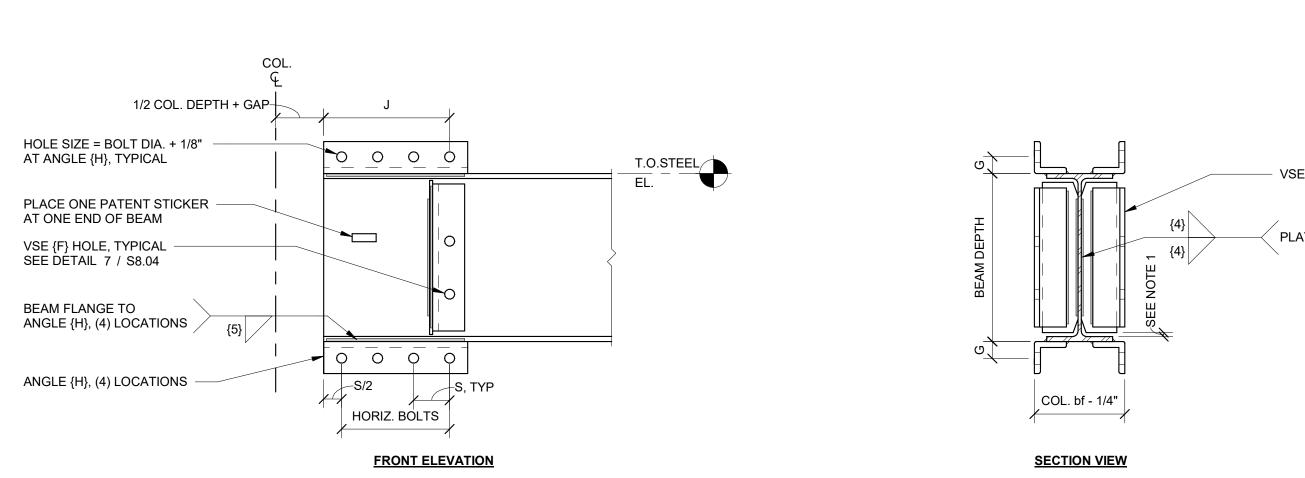
3 RECTANGULAR COVER PLATE {B} N.T.S.

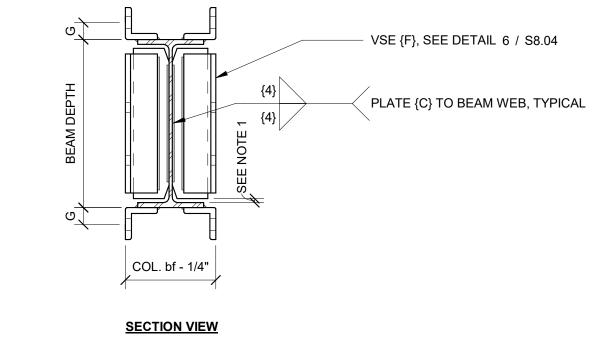
S, TYP

HORIZ. BOLTS



VERTICAL BOLTS



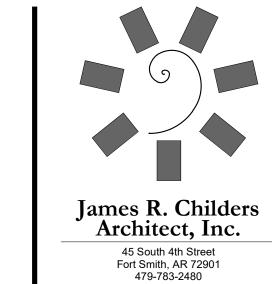


ISOMETRIC VIEW

NOTE(S):

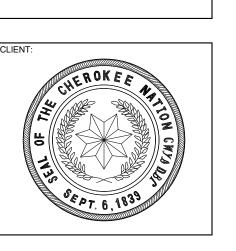
1. DIMENSION BETWEEN PLATE {C} AND INSIDE FACE OF BEAM FLANGE SHALL NOT EXCEED 1/4 INCH, AND MAY VARY DEPENDING ON BEAM MILL TOLERANCES. PLATE {C} SHALL BE CENTERED ON THE DEPTH OF THE BEAM.

1 BEAM END DETAIL N.T.S.



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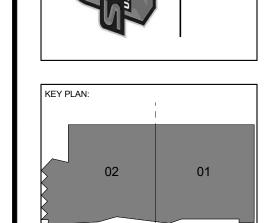










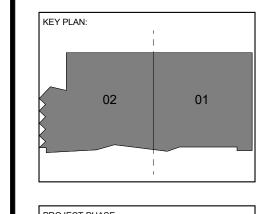


ISOMETRIC VIEW

COL. bf + H

COL. bf - 1/4"

SECTION VIEW



PROJECT PHASE:
BULK STEEL PACKACKE

BULK STEEL PACKACKE				
REVISIONS				
#	DATE	DESCRIPTION		

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DATE	<u> </u>	JOB NUMBER:	
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BEAM FLANGE TO ANGLE {H}

DATE:	JOB NUMBER:				
03.15.2019	17-13				
SHEET NUMBER:					
S8.04					

SIDEPLATE BEAM **DETAILS**

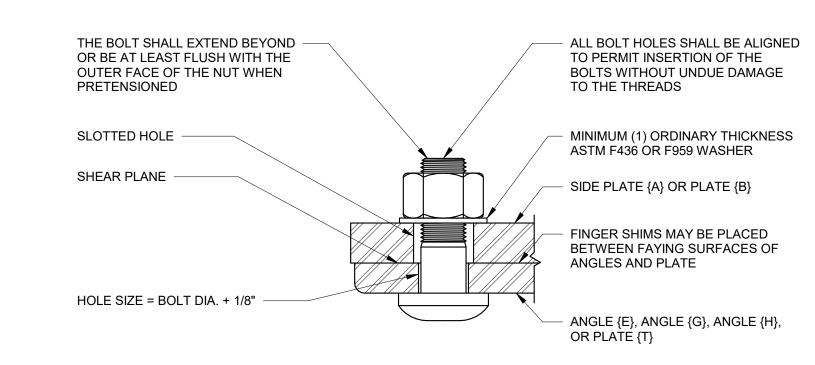
4 NARROW BEAM END DETAIL N.T.S.

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WHEN THE BEAM WEB IS NOT VERTICAL, IT MAY CAUSE THE ANGLES TO BIND MAKING IT DIFFICULT TO LOWER THE **BEAM INTO PLACE** SIDE PLATES HELD APART BY TEMORARY CONSTRUCTION AID **CORRECTLY RIGGED BEAM** INCORRECTLY RIGGED BEAM



- NOTE(S):

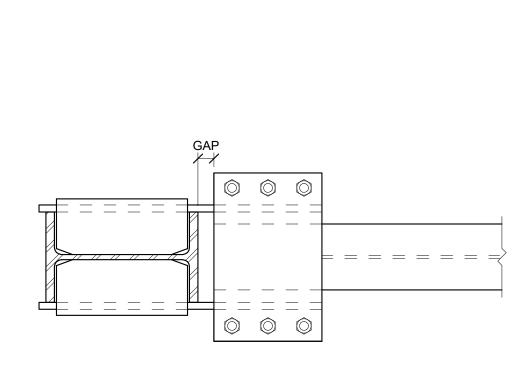
 1. BOLTS SHALL BE INSTALLED AS SHOWN TO KEEP THREADS OUTSIDE OF SHEAR PLANE.

 2. BOLTS SHALL BE SYSTEMATICALLY INSTALLED AS OUTLINED IN THE BOLTING SPECIFICATIONS. FIRST TO A SNUG TIGHT CONDITION, AND THEN PRETENSIONED.
- 3. THE USE OF FINGER SHIMS ARE ALLOWED FOR GAPS GREATER THAN 1/8 INCH UP TO 1/4 INCH. CONTACT SIDEPLATE SYSTEMS, INC. IF GAPS ARE GREATER THAN 1/4 INCH.
- 4. NUT SHALL BE ASTM A563. THE BOLT/FASTENER ASSEMBLY SHALL BE COVERED IN A LIGHT PROTECTIVE OIL.
 FOLLOW QUALITY CONTROL SECTION FOR EXPOSURE LIMITATION ON BOLTS/FASTENERS.

BOLT Ø HORIZONTAL BEAM BEAM ERECTION SCHEDULE

MISCELLANEOUS

3 FIELD BOLTING DETAIL N.T.S.





FRONT ELEVATION

3. BOTTOM DOG SHALL BE REMOVED. IT IS RECOMMENDED THAT IT BE REMOVED BY TORCH

CUTTING A 'V' SECTION OUT OF ONE OF THE ANGLE LEGS TO ALLEVIATE THE LOAD AND THEN

PROCEED TO REMOVE IT. IT IS NOT RECOMMENDED TO USE A GRINDING WHEEL TO REMOVE

4. BOLTS SHALL BE STUFFED INTO HOLES IN THE BEAM COVER PLATE {B} AND THE SIDE PLATES {A}.

6. THE WELD REMNANTS OF THE BOTTOM DOG MAY REMAIN IN PLACE AND DO NOT NEED TO BE

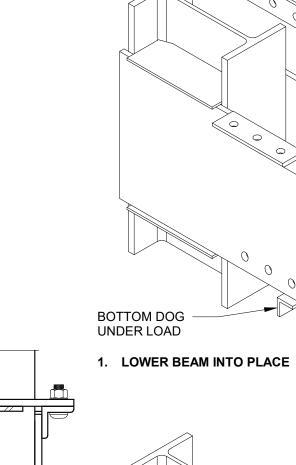
TYPICAL SEQUENCE OF ERECTION:

1. LOWER THE BEAM INTO PLACE FROM ABOVE.

2. STUFF A FEW BOLTS TO SECURE ASSEMBLY.

THE WELDS WHILE THE DOG IS UNDER LOAD!

5. SYSTEMATICALLY TIGHTEN BOLTS PER RCSC SPECIFICATIONS.



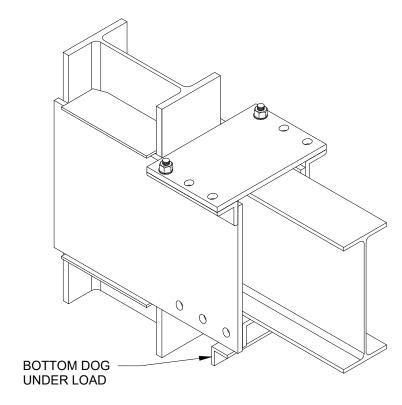
SET FIRST

REMOVED

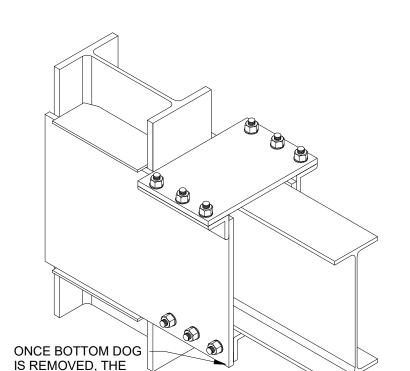
BOTTOM DOG

BOTTOM ROW AND THEN THE TOP ROW.

SECTION VIEW



2. STUFF A FEW BOLTS TO SECURE ASSEMBLY



IS REMOVED, THE SIDE PLATES WILL CLAMP TOGETHER

AND ELIMINATE GAPS 3. CAREFULLY REMOVE BOTTOM DOG AS IT IS UNDER LOAD. 4. SYSTEMATICALLY TIGHTEN BOLTS PER

THEN STUFF ALL REMAINING BOLTS STARTING WITH THE SPECIFICATIONS.

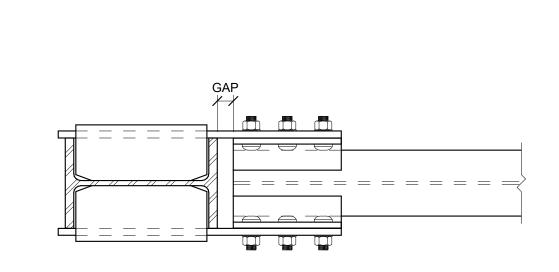
NARROW BEAM ERECTION SCHEDULE

MISCELLANEOUS

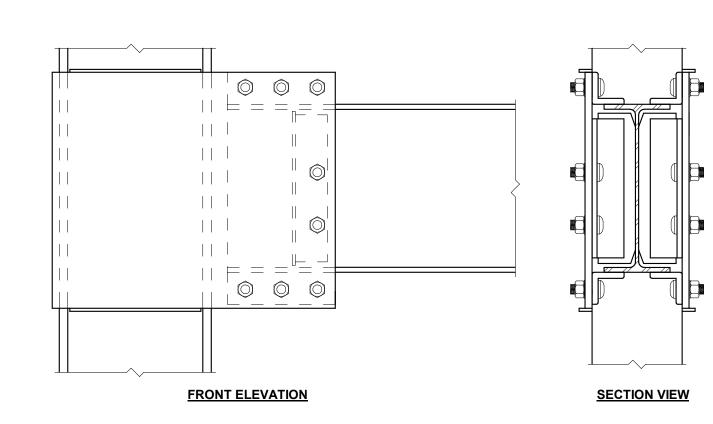
BEAM BOLT Ø

HORIZONTAL VERTICAL

BOLTS BOLTS



PLAN VIEW



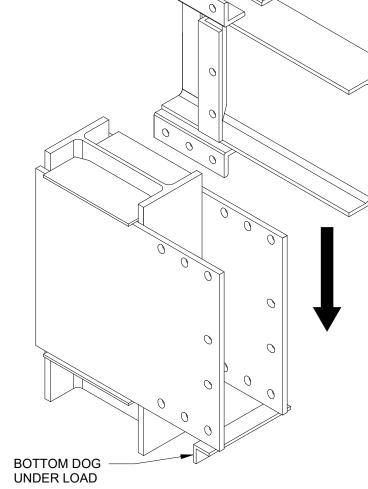
TYPICAL SEQUENCE OF ERECTION:

1. LOWER THE BEAM INTO PLACE FROM ABOVE.

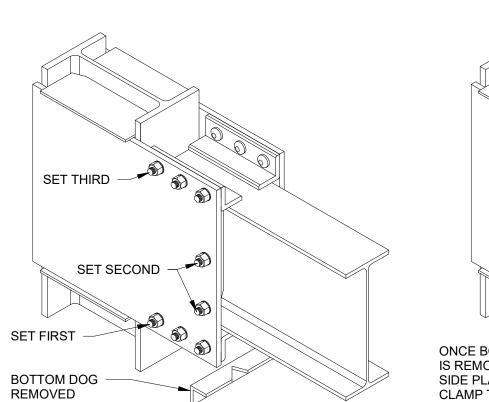
2. STUFF A FEW BOLTS TO SECURE ASSEMBLY. 3. BOTTOM DOG SHALL BE REMOVED. IT IS RECOMMENDED THAT IT BE REMOVED BY TORCH CUTTING A 'V' SECTION OUT OF ONE OF THE ANGLE LEGS TO ALLEVIATE THE LOAD AND THEN

PROCEED TO REMOVE IT. IT IS NOT RECOMMENDED TO USE A GRINDING WHEEL TO REMOVE THE WELDS WHILE THE DOG IS UNDER LOAD! 4. BOLTS SHALL BE STUFFED INTO HOLES IN THE BEAM COVER PLATE {B} AND THE SIDE PLATES {A}. 5. SYSTEMATICALLY TIGHTEN BOLTS PER RCSC SPECIFICATIONS.

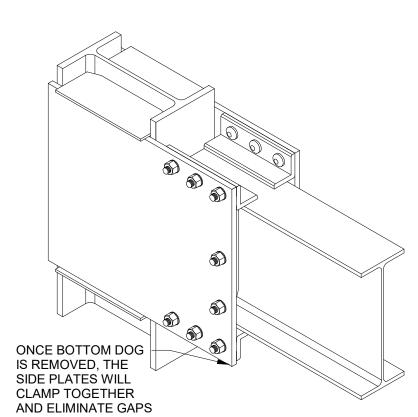
6. THE WELD REMNANTS OF THE BOTTOM DOG MAY REMAIN IN PLACE AND DO NOT NEED TO BE



1. LOWER BEAM INTO PLACE



3. CAREFULLY REMOVE BOTTOM DOG AS IT IS UNDER LOAD. THEN STUFF ALL REMAINING BOLTS STARTING WITH THE BOTTOM ROW, THEN THE VERTICAL ROW, AND LAST THE



2. STUFF A FEW BOLTS TO SECURE ASSEMBLY

BOTTOM DOG

UNDER LOAD

4. SYSTEMATICALLY TIGHTEN BOLTS PER SPECIFICATIONS.

5 NARROW BEAM ERECTION DETAIL N.T.S.

GROUND SMOOTH.

GROUND SMOOTH.

CONSULTANT LOGO:

Chavez-Grieves consulting engineers, inc. 4700 Lincoln Road NE, Suite 102 Albuquerque, NM 87109 505-344-4080 · 505-343-8759 (fax)

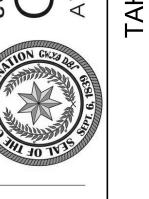
James R. Childers

Architect, Inc.

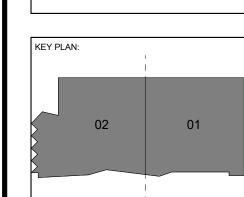
45 South 4th Street Fort Smith, AR 72901 479-783-2480 www.childersarchitect.com

PROFESSIONAL SEAL:









PROJECT PHASE: BULK STEEL PACKACKE

REVISIONS
DATE DESCRIPTION

03.15.2019 17-13 SHEET NUMBER:

SIDEPLATE FIELD **ERECTION DETAILS**

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5 NARROW CONFIGURATION DECK SUPPORT DETAIL N.T.S.

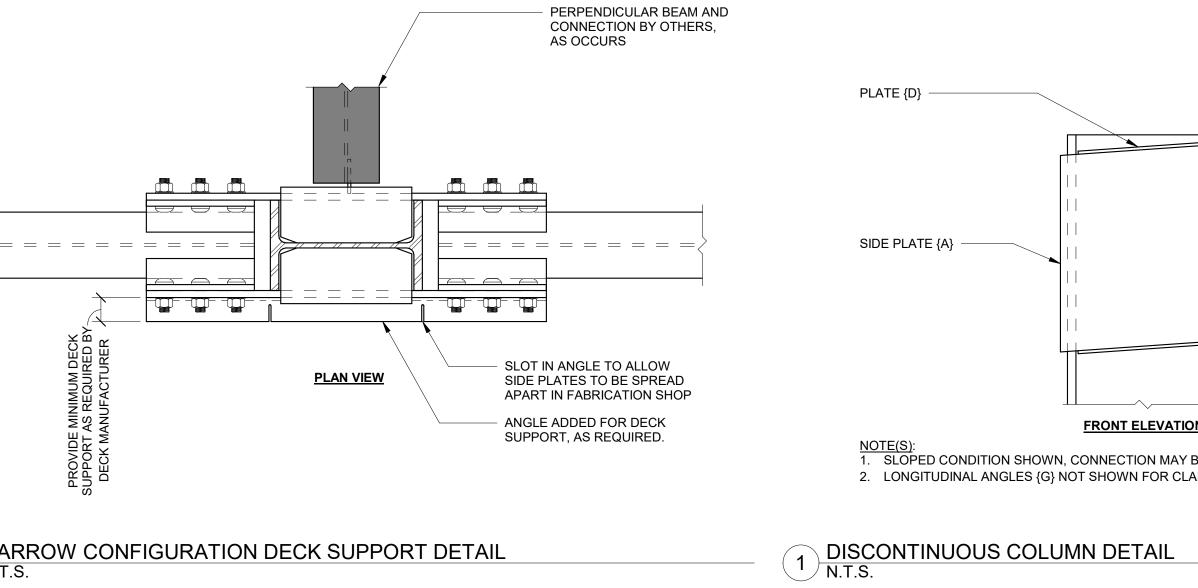
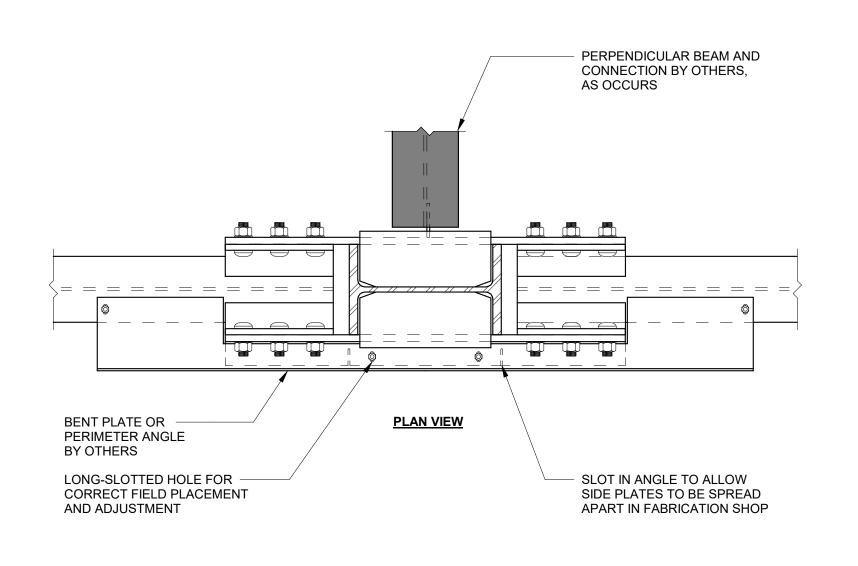


PLATE {D}

SIDE PLATE {A} -



- HORIZONTAL SHEAR PLATE {D}, (4) LOCATIONS. COLUMN WEB SIDE PLATE {A} PLATE {D} TO COLUMN WEB, (4) LOCATIONS PLATE {D} TO INSIDE FACE OF SIDE PLATE {A}, (4) LOCATIONS SIDE ELEVATION NOTE(S):
1. LONGITUDINAL ANGLES {G} NOT SHOWN FOR CLARITY.

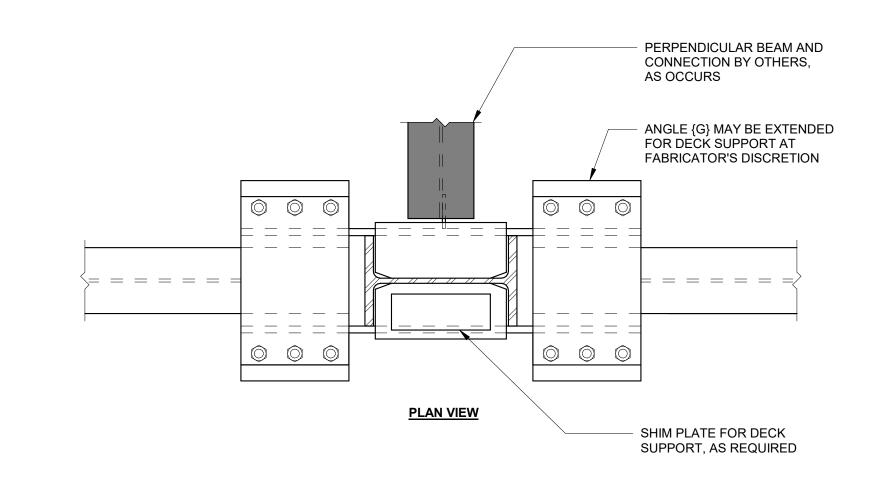
11 0 0 0

FRONT ELEVATION

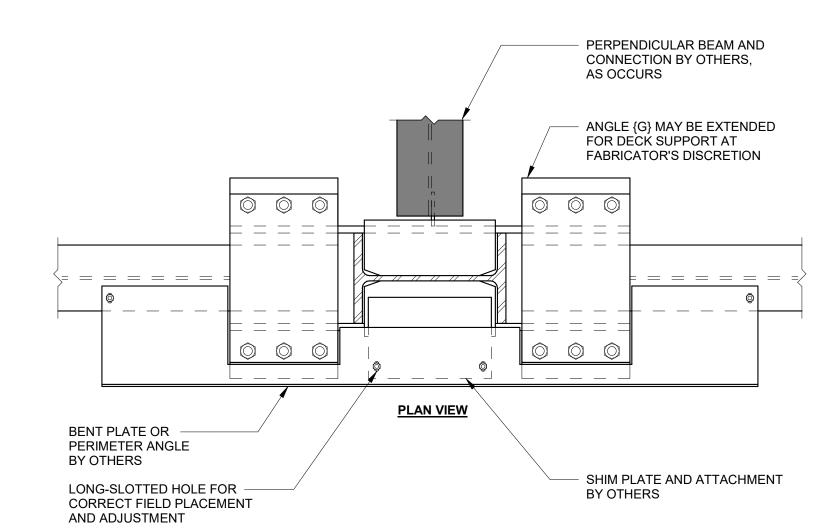
NOTE(S):

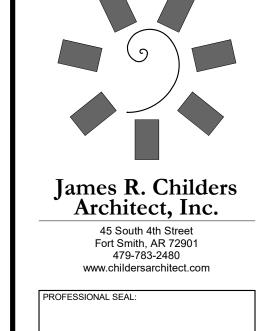
1. SLOPED CONDITION SHOWN, CONNECTION MAY BE FLAT. 2. LONGITUDINAL ANGLES (G) NOT SHOWN FOR CLARITY.

3 DECK SUPPORT DETAIL N.T.S.



4 SLAB EDGE DETAIL N.T.S.





CONSULTANT LOGO:

Chavez-Grieves
consulting engineers, inc.
4700 Lincoln Road NE, Suite 102 · Albuquerque, NM 87109
505-344-4080 · 505-343-8759 (fax)

Steopathic THE CHEROKEE NATION

PROJECT PHASE:

BULK STEEL PACKACKE

REVISIONS
DATE DESCRIPTION

03.15.2019 17-13

SIDEPLATE MISC DETAILS AND

COORDINATION ITEMS

SHEET NUMBER: