COLLEGE OF
Osteopathic Medicine
AT THE CHEROKEE NATION

BID PACKAGE 03
(FOUNDATION)

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REGISTERED ARCHITECT
03-20-2019

CONSULTANT LOGO:

CLIENT:

COLLEGE OF Osteopathic Medicine AT THE CHEROKEE NATION
TAHLEQUAH, OKLAHOMA

KEY PLAN:

PROJECT PHASE:
BID PACKAGE 03

REVISIONS		
#	DATE	DESCRIPTION

DATE: 03/20/19
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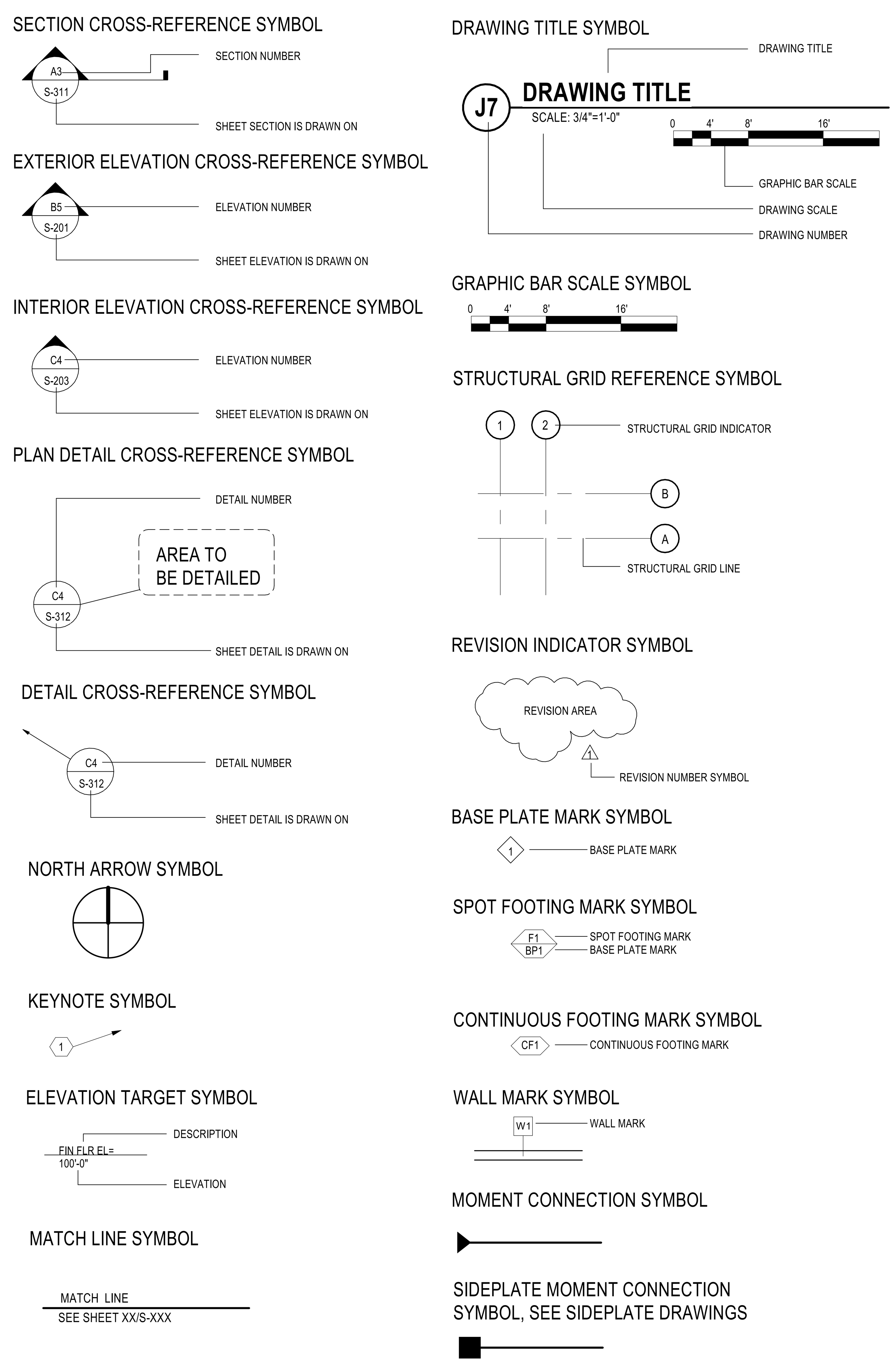
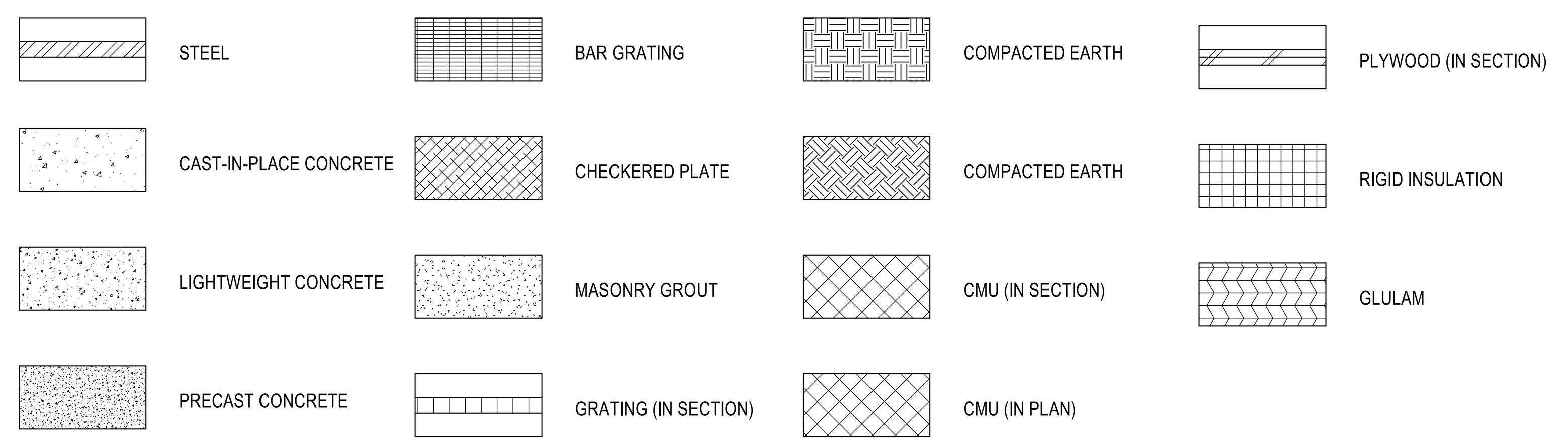
COVER / INDEX

STRUCTURAL GRAPHIC SYMBOLS



STRUCTURAL MATERIALS LEGEND



ABBREVIATIONS

A/E	ARCHITECT/ENGINEER
AB	ANCHOR BOLT
ABAN	ABANDON
ABBRV	ABBREVIATION
AC	ASPHALTIC CONCRETE
ACI	AMERICAN CONCRETE INSTITUTE
ACP	ASPHALTIC CONCRETE PAVING
ACR	ACROSS
ACST	ACOUSTIC
AD	AREA DRAIN
ADA	AMERICANS WITH DISABILITIES ACT
ADDL	ADDITIONAL
ADDM	ADDENDUM
ADJ	ADJACENT/ADJOINING
ADMIN	ADMINISTRATION
AFF	ABOVE FINISHED FLOOR
AFG	ABOVE FINISHED GRADE
AFS	ABOVE FINISHED SLAB
AGGR	AGGREGATE
AHR	ANCHOR
AIA	AMERICAN INSTITUTE OF ARCHITECTS
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION
AISI	AMERICAN IRON AND STEEL INSTITUTE
AITC	AMERICAN INSTITUTE OF TIMBER CONSTRUCTION
ALNMT	ALIGNMENT
ALT	ALTERNATE, ALTERNATIVE
ALUM	ALUMINUM
AMT	AMOUNT
ANSI	AMERICAN NATIONAL STANDARDS INSTITUTE
APA	AMERICAN PLYWOOD ASSOCIATION
APPD	APPROVED
APPROX	APPROXIMATE
APPX	APPENDIX
AR	AS REQUIRED
ARCH	ARCHITECT
ASCE	AMERICAN SOCIETY OF CIVIL ENGINEERS
ASPH	ASPHALT
ASI	ARCHITECT'S SUPPLEMENTAL INSTRUCTIONS
ASSN	ASSOCIATION
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS
ATCH	ATTACHMENT
ATTN	ATTENTION
AWS	AMERICAN WELDING SOCIETY
AZ	AZIMUTH
B&F	BELL AND FLANGE
BAL	BALANCE
B/B	BACK TO BACK
BC	BOTTOM CHORD
BD	BOARD
BDRY	BOUNDARY
BEV	BEVEL
BFF	BELOW FINISH FLOOR
BKG	BACKING
BKGD	BACKGROUND
BLD	BUILD
BLDG	BUILDING
BLK	BLOCK/BLOCKING
BLT	BUILT
BLVD	BOULEVARD
BLW	BELOW
BM	BEAM
BO	BOTTOM OF
BOS	BOTTOM OF STEEL
BOT	BOTTOM
B PL	BASE PLATE
BRCG	BRACING
BRDG	BRIDGING
BRG	BEARING
BRG PL	BEARING PLATE
BS	BOTH SIDES
BSMT	BASEMENT
BT WLD	BUTT WELD
BTWN	BETWEEN
C	CHANNEL
C/C	CENTER TO CENTER
CAM	CAMBER
CAN	CANOPY
CD	CONSTRUCTION DOCUMENTS, CONTRACT DOCUMENTS
CEM	CEMENT
CHFR	CHAMFER
CHKD	CHECKED/CHECKERED
CI	CAST IRON
CIP	CAST-IN-PLACE
CJ	CONSTRUCTION JOINT
CJ	CONTRACTION JOINT
CJ	CONTROL JOINT
CL	CENTER LINE
CLG	CEILING
CLR	CLEAR
cm	CENTIMETER
CMU	CONCRETE MASONRY UNIT
CO	COMPANY
COA	CITY OF ALBUQUERQUE
COL	COLUMN
COM	COMMON
CONC	CONCRETE
CONN	CONNECTION
CONSTR	CONSTRUCTION
CONT	CONTINUOUS , CONTINUE
CONTR	CONTRACTOR
COORD	COORDINATE
CRSI	CONCRETE REINFORCING STEEL INSTITUTE
CSI	CONSTRUCTION SPECIFICATIONS INSTITUTE
CTR	CENTER
CTRL	CONTROL
CU	CUBIC
CU YD	CUBIC YARD
D	DEEP, DEPTH
D-B	DESIGN-BUILD
DAT	DATUM
DBL	DOUBLE
DEG	DEGREE
DEL	DELETE
DEMO	DEMOLITION
DET	DETAIL
DEV	DEVELOPMENT
DFTG	DRAFTING
DIA	DIAMETER
DIAG	DIAGONAL
DIFF	DIFFERENCE, DIFFERENTIAL
DIM	DIMENSION
DIST	DISTANCE
DIV	DIVIDE
DJ	DOUBLE JOIST
DL	DEAD LOAD
DOC	DOCUMENT
DOUG FIR	DOUGLAS FIR
DSGN	DESIGN
DWG	DRAWING
DWL/DWLS	DOWELS
E	EAST, MODULUS OF ELASTICITY
EA	EACH
EE	EACH END
EF	EACH FACE
EIFS	EXTERIOR INSULATION AND FINISH SYSTEM
EJ	EXPANSION JOINT
EL	ELEVATION
ELAST	ELASTOMERIC
ELEC	ELECTRIC
ELEM	ELEMENTARY
ELEV	ELEVATOR
EMBED	EMBEDDED / EMBEDMENT
ENCL	ENCLOSURE
ENGR	ENGINEER
EOS	EDGE OF SLAB
EPA	ENVIRONMENTAL PROTECTION AGENCY
EQ	EQUAL
EQUIP	EQUIPMENT
EQUIV	EQUIVALENT
ESCAL	ESCALATOR
ESMT	EASEMENT
EST	ESTIMATE
ETC	ET CETERA
EW	EACH WAY
EX	EXAMPLE
EXC	EXCAVATE
EXCL	EXCLUDE
EXIST	EXISTING
EXP	EXPANSION
EXT	EXTERIOR
F/F	FACE TO FACE
FAB	FABRIC
FACIL	FACILITY
FB	FLAT BAR
FD	FLOOR DRAIN
FDTN	FOUNDATION
FF	FAR FACE
FF EL	FINISH FLOOR ELEVATION
FIN GR	FINISH GRADE
FH	FLAT HEAD
FIN	FINISH
FIN FLR	FINISH FLOOR
FLG	FLANGE
FLR	FLOOR
FLR SK	FLOOR SINK
FOC	FACE OF CONCRETE
FOF	FACE OF FINISH
FOM	FACE OF MASONRY
FOS	FACE OF SLAB
FOS	FACE OF STUD
FOW	FACE OF WALL
FR	FRAME
FRMG	FRAMING
FS	FAR SIDE
FSTNR	FASTENER
FT	FOOT / FEET
FT/LB	FOOT/POUND
FT/LBF	FOOT/POUND FORCE
FTG	FOOTING
FUT	FUTURE
G	GIRDER
GA	GAGE
GALV	GALVANIZED
GALV STL	GALVANIZED STEEL
GR BM	GRADE BEAM
GC	GENERAL CONTRACTOR
GEN	GENERAL
GLU LAM	GLUED LAMINATED WOOD
GLZ	GLAZING
GOVT	GOVERNMENT
GRTG	GRATING
GT	GROUT
H	HIGH
HAS	HEADED ANCHOR STUD
HC	HOLLOW-CORE
HCP	HANDICAPPED
HD	HEAVY DUTY
HGR	HANGER
HLDN	HOLDDOWN
HORIZ	HORIZONTAL
HS	HIGH STRENGTH
HSKPG	HOUSEKEEPING
HSS	HOLLOW STRUCTURAL SECTIONS
HST	HOIST
HT	HEIGHT
IBC	INTERNATIONAL BUILDING CODE
ID	INSIDE DIAMETER
IF	INSIDE FACE
IFS	INSIDE FACE OF STUD
IN	INCH
INCL	INCLUDED
INFO	INFORMATION
IN-LB	INCH-POUND
IN-LBF	INCH-POUND FORCE
INSTL	INSTALL
INSUL	INSULATION
INT	INTERIOR
IR	INSIDE RADIUS
k	KIP
K	THOUSAND
KB	KNEE BRACE
KCJ	KEYED CONTROL JOINT
KIP	THOUSAND POUNDS
KIP FT	THOUSAND FOOT/POUNDS
KLF	KIPS PER LINEAL FOOT
KO	KNOCK OUT
KOP	KNOCK OUT PANEL
KSF	KIPS PER SQUARE FOOT
KSI	KIPS PER SQUARE INCH
L	ANGLE
LAM	LAMINATE
LATL	LATERAL
LBF	POUND-FORCE
LBR	LUMBER
LBS	POUND
LD BRG	LOAD BEARING
LF	LINEAR FEET (FOOT)
LIN	LINEAR
LL	LIVE LOAD
LLBB	LONG LEG BACK TO BACK
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LONG	LONGITUDINAL
LT GA	LIGHT GAGE
LT WT	LIGHT WEIGHT
LVR	LOUVER
LWC	LIGHTWEIGHT CONCRETE
M	MOMENT
MAINT	MAINTENANCE
MATL	MATERIAL
MAX	MAXIMUM
MB	MACHINE BOLT
MC	MOMENT CONNECTION
MCJ	MASONRY CONTROL JOINT
MD	METAL DECK
ME	MECHANICAL ENGINEER
MECH	MECHANICAL
MEZZ	MEZZANINE
MFR	MANUFACTURER
MID	MIDDLE
MIN	MINIMUM
MISC	MISCELLANEOUS
ML	MICRO-LAMINATED
ML	MONOLITHIC
MO	MASONRY OPENING
MS	MACHINE SCREW
MSL	MEAN SEA LEVEL
MTL	METAL
N	NORTH
NA	NOT APPLICABLE
NF	NEAR FACE
NIC	NOT IN CONTRACT
NM	NEW MEXICO
NO	NUMBER
NOM	NOMINAL
NS	NEAR SIDE
NTS	NOT TO SCALE
O/O	OUT TO OUT
OA	OVERALL
OC	ON CENTER
OD	OUTSIDE DIAMETER
OF	OUTSIDE FACE
OFS	OUTSIDE FACE OF STUD
OPH	OPPOSITE HAND
OPNG	OPENING
OPP	OPPOSITE
OPT	OPTIONAL
OR	OUTSIDE RADIUS
PAR	PARALLEL, PARAPET
PART	PARTIAL
PC	PIECE, PORTLAND CEMENT
PCC	PRECAST CONCRETE
PCF	POUNDS PER CUBIC FOOT
PCI	PRECAST/PRESTRESSED CONCRETE INSTITUTE
PED	PEDESTAL
PEN	PENETRATE
PERIM	PERIMETER
PERP	PERPENDICULAR
PH	PHASE
PIL	PILASTER
PL	PLATE
PLAT	PLATFORM
PLBG	PLUMBING
PLF	POUNDS PER LINEAR FOOT
PLM	PARALLAM
PLYWD	PLYWOOD
POS	POSITION
PP	PANEL POINT
PRCST	PRECAST
PREFAB	PREFABRICATE
PRELIM	PRELIMINARY
PREV	PREVIOUS
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PT	POST-TENSIONED
PT CONC	POST-TENSIONED CONCRETE
PTN	PARTITION
PVG	PAVING
QTY	QUANTITY
QUAD	QUADRANT
R	RADIUS, RISER
RC	REINFORCED CONCRETE
RD	ROAD, ROOF DRAIN
REC	RECESSED
REF	REFERENCE
REINF	REINFORCE/REINFORCEMENT
REPL	REPLACE
REQ	REQUIRE
REQD	REQUIRED
REV	REVISION
RGD INS	RIGID INSULATION
RFI	REQUEST FOR INFORMATION
RND	ROUND
RO	ROUGH OPENING
RT	RIGHT
RVL	REVEAL
S	SOUTH
SCHEM	SCHEMATIC
SCHED	SCHEDULE
SD	SHOP DRAWINGS
SDI	STEEL DECK INSTITUTE
SDL	SADDLE
SE	STRUCTURAL ENGINEER
SECT	SECTION
SF	SQUARE FEET (FOOT)
SHT	SHEET, SHAFT
SHTHG	SHEATHING
SIM	SIMILAR
SJI	STEEL JOIST INSTITUTE
SLNT	SEALANT
SM	SMOOTH
SP	SUMP PIT
SPA	SPACE/SPACES
SPEC	SPECIFICATION
SPRT	SUPPORT
SQ	SQUARE
SQ IN	SQUARE INCH
SQ YD	SQUARE YARD
SSPC	STRUCTURAL STEEL PAINTING COUNCIL
ST	STAIRS
STAG	STAGGERED
STD	STANDARD
STIF	STIFFENER
STIR	STIRRUP
STAG	STAGGERED
STD	STANDARD
STIF	STIFFENER
STIR	STIRRUP
STL	STEEL
STL LNTL	STEEL LINTEL
STL	JST STEEL JOIST
STL PL	STEEL PLATE
STL RF DK	STEEL ROOF DECK
STR	STRINGERS
STRUCT	STRUCTURAL
SUB	SUBSTITUTE
SUF	SUFFICIENT
SUP	SUPPLEMENTARY
SUPPL	SUPPLEMENT
SYM	SYMBOL
SYMM	SYMMETRICAL
SYS	SYSTEM
T	TREAD
T&B	TOP AND BOTTOM
T&G	TONGUE AND GROOVE
TAN	TANGENT
TB	THRU BOLT
TEMP	TEMPORARY
THD	THREAD
THK	THICKNESS
THRU	THROUGH
TJI	TRUSS JOIST INSTITUTE
TO	TOP OF
TOB	TOP OF BEAM
TOC	TOP OF CONCRETE
TOC FTG	TOP OF CONCRETE FOOTING
TOC WALL	TOP OF CONCRETE WALL
TOF	TOP OF FOOTING
TOG	TOP OF GRATE
TOJ	TOP OF JOIST
TOL	TOLERANCE
TOM	TOP OF MASONRY
TOP	TOP OF PARAPET
TOS	TOP OF SLAB
TOS	TOP OF STEEL
TOW	TOP OF WALL
TRANS	TRANSVERSE
TRNBKL	TURNBUCKLE
TYP	TYPICAL
UBC	UNIFORM BUILDING CODE
UNO	UNLESS NOTED OTHERWISE
VAR	VARIES
VERT	VERTICAL
VIF	VERIFY IN FIELD
VNR	VENEER
VR	VAPOR RETARDER
VRFY	VERIFY
W	WEST, WIDE
W/	WITH
W/O	WITHOUT
WBL	WOOD BLOCKING
WD	WOOD
WF	WIDE FLANGE
WF BM	WIDE FLANGE BEAM
WL	WIND LOAD
WLD	WELDED
WM	WIRE MESH
WP	WATERPROOFING
WSCT	WAINSCOT
WT	WEIGHT
WWF	WELDED WIRE FABRIC
WWM	WELDED WIRE MESH
X BRACE	CROSS BRACING
XXH	DOUBLE EXTRA HEAVY
YD	YARD

NOTE: THIS STRUCTURAL PACKAGE IS FOR FOUNDATIONS ONLY. ANY CHANGES TO THE PROJECT, INCLUDING, BUT NOT LIMITED TO: LOADING REQUIREMENTS, GEOMETRY CHANGES IN PLAN OR ELEVATION, SPACE USAGE REVISIONS, OR VALUE ENGINEERING MAY AFFECT THE STRUCTURAL STEEL MEMBER REQUIREMENTS SHOWN IN THESE DRAWINGS.

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COLLEGE OF Osteopathic Medicine AT THE CHEROKEE NATION

TAHLEQUAH, OKLAHOMA

KEY PLAN:

PROJECT PHASE: BID PACKAGE 03

REVISIONS		
#	DATE	DESCRIPTION

DATE: 03-20-19

JOB NUMBER: 17-13

SHEET NUMBER: S0.01

ABBREVIATIONS AND LEGEND

GENERAL STRUCTURAL NOTES

FOUNDATION NOTES

GENERAL:

A SUBSURFACE SOIL INVESTIGATION HAS BEEN MADE BY PALMERTON AND PARRISH, INC., PROJECT NO. 255932.

A REPORT OF THAT INVESTIGATION DATED 1/30/19 IS AVAILABLE FOR VIEWING IN THE PROJECT MANUAL.

THE FOUNDATION SYSTEM FOR THIS PROJECT IS SPREAD FOOTINGS OVER ENGINEERED FILL.

ADDITIONAL INFORMATION CONCERNING SPECIFIC SOIL CONDITIONS TO BE ENCOUNTERED IS AVAILABLE IN THE SOILS REPORTS AND SHALL BE REVIEWED BY THE CONTRACTOR.

FIELD OBSERVATION AND TESTS:

THE OWNER SHALL EMPLOY THE SERVICES OF A REGISTERED, LICENSED GEOTECHNICAL ENGINEER TO OBSERVE ALL CONTROLLED EARTHWORK. THE GEOTECHNICAL ENGINEER SHALL PROVIDE CONTINUOUS ON-SITE OBSERVATION BY EXPERIENCED PERSONNEL DURING CONSTRUCTION OF CONTROLLED EARTHWORK. THE CONTRACTOR SHALL NOTIFY THE GEOTECHNICAL ENGINEER AT LEAST TWO WORKING DAYS IN ADVANCE OF ANY FIELD OPERATIONS OF THE CONTROLLED EARTHWORK.

TESTS OF MATERIALS SHALL BE MADE AT THE FOLLOWING MINIMUM RATES. THE ON-SITE GEOTECHNICAL ENGINEER SHALL DETERMINE THE ACTUAL TESTING RATES:

ONE FIELD DENSITY TEST PER 2500 SQUARE FEET OF COMPACTED SUBGRADE, PRIOR TO PLACING STRUCTURAL FILL OR SLAB-ON-GRADE, WITH A MINIMUM OF 3 TESTS.

ONE FIELD DENSITY TEST PER 2500 SQUARE FEET OF STRUCTURAL FILL PLACED OR EACH HORIZONTAL LAYER OF STRUCTURAL FILL, WHICHEVER IS GREATER.

ONE MOISTURE-DENSITY CURVE FOR EACH TYPE OF MATERIAL USED, AS INDICATED BY THE SIEVE ANALYSIS AND THE PLASTICITY INDEX.

THE GEOTECHNICAL ENGINEER SHALL SUBMIT THE RESULTS OF ALL REQUIRED TESTS.

CLEARING AND GRUBBING:

REMOVE ALL EXISTING FOUNDATIONS, CONCRETE SLAB,VEGETATIVE MATTER, STUMPS, MATTED ROOTS AND ROOTS BRUSH, AND RUBBISH FROM THE CONSTRUCTION AREA.

REMOVE ALL TOPSOIL FROM THE CONSTRUCTION AREA. TOPSOIL/VEGETATIVE MATTER STRIPPING ON THE ORDER OF 3-INCHES SHOULD BE ANTICIPATED IN GRASS COVERED AREAS. THIS MATERIAL SHALL NOT BE USED AS FILL MATERIAL, BUT MAY BE STOCKPILED AND LATER USED IN LAWN AND LANDSCAPE AREAS ONLY.

SITE, SUBFLOOR AND BEARING SURFACE PREPARATION:

A REPRESENTATIVE OF THE GEOTECHNICAL ENGINEER SHALL BE PRESENT TO CONFIRM COMPLETE EXCAVATION OF ANY UNCONTROLLED FILL, PROOF ROLLING AND CONSTRUCTION MATERIALS TESTING..

OVEREXCAVATE ALL SOILS UNDERLYING FOOTINGS AND FLOOR SLAB AND ALL UNCONTROLLED FILL TO A MINIMUM DEPTH OF 2 FEET OR TO GRAVELLY CLAYS/CLAYEY GRAVELS, WHICHEVER IS SHALLOWER, AND REPLACE WITH LVC FILL MATERIAL.

OVEREXCAVATE ALL SOILS UNDERLYING SITE RETAINING WALL FOOTINGS TO A MINIMUM DEPTH OF 2 FEET OR TO GRAVELLY CLAYS/CLAYEY GRAVELS, WHICHEVER IS SHALLOWER, AND REPLACE WITH LVC FILL MATERIAL.

ONCE THE BASEMENT EXCAVATION AND UNDERCUTTING OF EXISTING SOILS HAS BEEN COMPLETED THE BUILDING FOOTPRINT SOILS SHALL BE SCANNED WITH GROUND PENETRATING RADAR, GPR, TO SEARCH FOR ANY LARGE SHALLOW SURFACE VOIDS. THE GPR SCANNING SHALL BE PERFORMED BY THE GEOTECHNICAL ENGINEER OF RECORD FOR THE PROJECT.

AFTER GPR SCANNING AND WRITTEN APPROVAL THAT NO VOIDS EXIST, ALL UNDERCUT SUBGRADE SOILS SHALL BE PROOF ROLLED WITH A FULLY LOADED TANDEM AXLE DUMP TRUCK OR SIMILAR RUBBER TIRED CONSTRUCTION EQUIPMENT AND ANY AREAS WHICH RUT OR DEFLECT DURING ROLLING NOTED. ALL SOFT AREAS, IF ANY, IDENTIFIED DURING PROOF-ROLLING SHALL BE UNDERCUT AND REPLACED WITH COMPACTED FILL.

AFTER EVALUATION OF PROOF-ROLLING AND APPROVAL, SCARIFY ALL EXPOSED SUBGRADE SOILS TO A DEPTH OF 8 INCHES, MOISTEN TO OPTIMUM MOISTURE CONTENT (+/- 2%, OR 0 TO 4% FOR CH SOIL TYPES) AND COMPACT TO THE DENSITY SPECIFIED HEREINAFTER.

PLACE ALL STRUCTURAL FILL IN APPROXIMATELY HORIZONTAL LAYERS NOT GREATER THAN 12 INCHES IN LOOSE THICKNESS, MOISTEN TO OPTIMUM MOISTURE CONTENT (+/- 2% OR 0 TO 4% FOR CH SOIL TYPES) AND COMPACT TO DENSITY SPECIFIED HEREINAFTER.

ALL EARTHWORK FOR THE BUILDING PAD SHALL EXTEND A MINIMUM OF 5 FEET BEYOND THE PERIMETER FOOTINGS.

ENGINEERED FILL REQUIREMENTS:

LOW PLASTICITY COHESIVE SOIL OR GRANULAR FILL PER GEOTECHNICAL REPORT:

GRADATION (ASTM D422):

SIEVE SIZE	PERCENT PASSING BY WEIGHT
12"	100
NO. 200	</= 85%

LIQUID LIMIT (ASTM D4318): 50 MAXIMUM

MATERIAL LARGER THAN 12 INCHES SHALL NOT BE PLACED IN THE STRUCTURAL FILL, AND MATERIAL LARGER THAN 4 INCHES SHALL NOT BE PLACED WITHIN TWELVE INCHES OF THE BEARING SURFACES OF SLABS OR FOUNDATIONS.

NO BRUSH, SOD, FROZEN MATERIAL OR OTHER UNSUITABLE MATERIAL SHALL BE PLACED IN THE STRUCTURAL FILL. MATERIAL SHALL BE PLACED IN SUCH A MANNER AS TO RESULT IN A UNIFORMLY COMPACTED FILL.

BASED ON THE REQUIREMENTS FOR THE STRUCTURAL FILL AND THE DESCRIPTION OF THE EXISTING SITE SOILS IN THE PROJECT GEOTECHNICAL REPORT, THE CONTRACTOR IS RESPONSIBLE FOR DETERMINING THE MOST APPROPRIATE METHOD FOR PROVIDING THE REQUIRED STRUCTURAL FILL. DEPENDING ON THE SITE CONDITIONS, APPROPRIATE METHODS COULD INCLUDE REBLENDING OF THE EXISTING SITE SOILS, MIXING THE EXISTING SITE SOILS WITH IMPORTED FILL, OR REMOVING THE EXISTING SITE SOILS ENTIRELY AND REPLACING WITH IMPORTED FILL. SEE SPECIFIC REQUIREMENTS IN GEOTECHNICAL REPORT FOR USE OF ON-SITE SOILS/FILL.

GRANULAR BASE COURSE REQUIREMENTS:

GRADATION (ASTM C136):

SIEVE SIZE	PERCENT PASSING BY WEIGHT
1"	100
3/4"	85-100
NO. 4	45-95
NO. 200	0-8

PLASTICITY INDEX (ASTM D4318): 3 MAXIMUM

THE COURSE AGGREGATE SHALL HAVE A PERCENT WEAR OF 50 OR LESS WHEN TESTED IN ACCORDANCE WITH ASTM C131.

THE FINISHED TOP SURFACE SHOULD BE FLAT AND LEVEL WITH SUFFICIENT FINES TO FILL BETWEEN COARSE AGGREGATE. IF THIS IS NOT THE CASE, PROVIDE UP TO 1/2 INCH BLOTTER LAYER OF SAND.

COMPACTION REQUIREMENTS:

IN ACCORDANCE WITH ASTM D698 (STANDARD PROCTOR), SUBGRADE SOILS AND STRUCTURAL FILL MATERIALS SHALL BE COMPACTED TO THE FOLLOWING PERCENTAGES OF THE MAXIMUM DRY DENSITY AT +/- 2% (OR 0 TO 4% FOR CH SOIL TYPES) OPTIMUM MOISTURE CONTENT:

MATERIAL	MINIMUM PERCENT COMPACTION
STRUCTURAL FILL IN THE BUILDING AREA	95
SUBBASE FOR SLAB SUPPORT	95
SUBGRADE BELOW STRUCTURAL FILL	95
MISCELLANEOUS BACKFILL	90

SITE RETAINING WALL DESIGN CRITERIA:

LOADING CONDITION	EQUIVALENT FLUID PRESSURE
ACTIVE EARTH PRESSURE FOR ON SITE SOIL BACKFILL	35 PCF UNRESTRAINED
PASSIVE EARTH PRESSURE	230 PCF
EARTH PRESSURE AT REST	50 PCF UNRESTRAINED
SOIL FRICTION FACTOR	0.26
SOIL BEARING CAPACITY	3500 PSF

GENERAL STRUCTURAL NOTES

CODES AND MANUALS:

IBC-15 INTERNATIONAL BUILDING CODE 2015
ASCE/SEI 3-91 STRUCTURAL DESIGN OF COMPOSITE SLABS
ASCE/SEI 7-10 MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES
AISC 360-10 SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS
AISC 341-10 SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS
AISC MANUAL OF STEEL CONSTRUCTION 14TH EDITION
SDI DIAPHRAGM DESIGN MANUAL, 3RD EDITION
ANSI/SDI NC1.0-06 STANDARD FOR NONCOMPOSITE STEEL FLOOR DECK
ANSI/SDI RD1.0-06 STANDARD FOR STEEL ROOF DECK
ANSI/SDI C1.0-06 STANDARD FOR COMPOSITE STEEL FLOOR DECK
AISI S100-12 NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS
AISI S200-12 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING – GENERAL PROVISIONS
AISI S210-12 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING – FLOOR AND ROOF SYSTEM DESIGN
AISI S211-07 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING – WALL STUD DESIGN WITH 2012 SUPPLEMENT
AISI S212-12 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING – HEADER DESIGN
AISI S213-12 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING – LATERAL DESIGN WITH 2010 SUPPLEMENT
AISI S214-12 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING – TRUSS DESIGN, WITH SUPPLEMENT 2, DATED 2008
ACI 318-14 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE
ACI 530-13 BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES
ACI 530.1-13 SPECIFICATIONS FOR MASONRY STRUCTURES
AWS D1.1-04 STRUCTURAL WELDING CODE – STEEL
AWS D1.3-98 STRUCTURAL WELDING CODE – SHEET STEEL
AWS D1.4-11 STRUCTURAL WELDING CODE – REINFORCING STEEL

DESIGN CRITERIA:

VERTICAL:

LIVE LOAD	
FLOOR	80 PSF
STAIRS AND EXIT-WAYS*	100 PSF
*MINIMUM CONCENTRATED LOAD	300 LBS
ASSEMBLY AREAS	100 PSF
SAFER ROOM	100 PSF
STORAGE	150 PSF
LIBRARY/ BOOKSTORE	150 PSF
GYM	150 PSF
ADDITIONAL SUPERIMPOSED LOADS	
PARTITIONS	15 PSF
SUSPENDED EQUIPMENT	10 PSF
CONCENTRATED LOAD (PER IBC 1607.4)	2000 LBS
ROOF LIVE LOAD: LR = 20*R1*R2	20 PSF
REDUCTION FACTOR BASED ON TRIB AREA	R1 = 1.0
REDUCTION FACTOR BASED ON ROOF SLOPE	R2 = 1.0
SNOW LOAD	
GROUND SNOW LOAD	PG = 10 PSF
FLAT ROOF SNOW LOAD**	PF = 16 PSF
SNOW EXPOSURE FACTOR	CE = 0.9
SNOW LOAD IMPORTANCE FACTOR	IS = 1.10
THERMAL FACTOR	CT = 1.0
**INCLUDES 5 PSF RAIN-ON SNOW SURCHARGE LOAD	

HORIZONTAL:

WIND	
ULTIMATE DESIGN WIND SPEED	120 MPH
RISK CATEGORY	III
EXPOSURE	C
INTERNAL PRESSURE COEFFICIENT	GCPI = 0.18
NATURAL FREQUENCY	0.685
STRUCTURE IS FLEXIBLE	
SEISMIC	
SEISMIC IMPORTANCE FACTOR	IS = 1.25
MAPPED SPECTRAL RESPONSE ACCELERATIONS	
SHORT PERIOD	SS = 0.152G
1 SECOND PERIOD	S1 = 0.081G
SITE CLASS	C
SPECTRAL RESPONSE COEFFICIENTS	
SHORT PERIOD	SDS = 0.122G
1 SECOND PERIOD	SD1 = 0.092G
SEISMIC DESIGN CATEGORY	B
BASIC SEISMIC FORCE RESISTING SYSTEM: STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE	
SEISMIC RESPONSE COEFFICIENT	CS = 0.043
RESPONSE MODIFICATION FACTOR	R = 3
DESIGN BASE SHEAR	V = 0.043W
ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE	
ALLOWABLE SOIL BEARING PRESSURE =	SPREAD FOOTINGS: 4000 PSF CONTINUOUS FOOTINGS: 3500 PSF
FROST DEPTH =	24 INCHES
FUTURE BUILDING EXPANSION:	NONE

GENERAL:

STRUCTURAL DRAWINGS ARE NOT STAND-ALONE DOCUMENTS AND ARE INTENDED TO BE USED IN CONJUNCTION WITH CIVIL, ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND DRAWINGS FROM OTHER DISCIPLINES. THE CONTRACTOR SHALL COORDINATE ALL REQUIREMENTS OF THE CONTRACT DOCUMENTS INTO THE SHOP DRAWINGS AND FIELD WORK.

COORDINATE DIMENSIONS OF ALL OPENINGS, DEPRESSIONS, BLOCKOUTS, ETC. WITH ARCHITECTURAL DRAWINGS, DRAWINGS FROM OTHER DISCIPLINES, PROJECT SHOP DRAWINGS, AND FIELD CONDITIONS PRIOR TO SHOP DRAWING SUBMITTAL. THE STRUCTURAL DRAWINGS ONLY REPRESENT A PORTION OF THE REQUIREMENTS FOR THE PROJECT.

SEE ARCHITECTURAL PLANS FOR INTERIOR NON-BEARING PARTITION WALLS. PARTITION FRAMING SHALL BE CONNECTED TO THE PRIMARY STRUCTURE TO ALLOW FOR VERTICAL LIVE LOAD DEFLECTIONS OF SPAN/360 FOR FLOOR FRAMING AND SPAN/240 FOR ROOF FRAMING.

CONTRACTOR'S CONSTRUCTION AND/OR ERECTION SEQUENCES SHALL RECOGNIZE AND CONSIDER THE EFFECTS OF THERMAL MOVEMENTS OF STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PERIOD.

THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS IN THE FIELD.

SHOP DRAWINGS SHALL BE FURNISHED AND REVIEWED BEFORE ANY FABRICATION OR ERECTION IS STARTED. THE CONTRACTOR SHALL REVIEW AND APPROVE SHOP DRAWINGS PRIOR TO SUBMITTAL TO THE ARCHITECT FOR REVIEW. POORLY EXECUTED SHOP DRAWINGS WILL BE REJECTED AND SHALL BE RESUBMITTED.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING SAFE AND ADEQUATE SHORING FOR ALL PARTS OF THE STRUCTURE DURING CONSTRUCTION.

TEMPORARY PROVISIONS SHALL BE MADE FOR STRUCTURAL STABILITY DURING CONSTRUCTION. THE STRUCTURE SHOWN ON THE DRAWINGS HAS BEEN DESIGNED FOR STABILITY UNDER FINAL CONFIGURATION.

NOTCHING OR CUTTING ANY STRUCTURAL MEMBER IN THE FIELD IS PROHIBITED.

THE CONTRACTOR SHALL VERIFY THE SIZE AND LOCATION OF FOUNDATIONS UNDER MECHANICAL AND ELECTRICAL EQUIPMENT AS REQUIRED. NO CONCRETE PADS SHALL BE LOCATED ON ROOF UNLESS SHOWN ON THE STRUCTURAL DRAWINGS.

BACKFILL SHALL NOT BE PLACED BEHIND RETAINING WALLS UNTIL CONCRETE HAS ATTAINED 100 PERCENT OF DESIGN STRENGTH.

BACKFILL SHALL NOT BE PLACED BEHIND BASEMENT WALLS UNTIL THE CONCRETE HAS ATTAINED 100 PERCENT OF DESIGN STRENGTH AND THE ELEVATED FLOOR PROVIDING LATERAL SUPPORT AT THE TOP OF THE WALL IS COMPLETELY CONSTRUCTED, OR TEMPORARY BRACING/SHORING OF THE WALL IS PROVIDED. DESIGN OF ANY TEMPORARY WALL BRACING/SHORING IS THE RESPONSIBILITY OF THE CONTRACTOR.

REMOVAL OF FORMS AND SHORING SHALL BE IN ACCORDANCE WITH ACI 347. WHERE CONCRETE MUST SUPPORT SUPERIMPOSED LOADS PRIOR TO ATTAINING THE SPECIFIED DESIGN STRENGTH, RESHORE CONCRETE IN ACCORDANCE WITH ACI 347. RESHORING SHALL NOT BE REMOVED SOONER THAN 28 DAYS FROM THE DATE OF POUR OR UNTIL CONCRETE HAS ATTAINED THE SPECIFIED DESIGN STRENGTH.

THE CONTRACTOR SHALL SUBMIT FOR PRIOR APPROVAL THE END OF POUR LOCATIONS FOR CONCRETE GRADE BEAMS, CONCRETE COLUMNS, AND CONCRETE BEAMS.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR ADHERING TO ALL APPLICABLE STANDARDS SET FORTH BY OSHA, INCLUDING THE FOLLOWING REQUIREMENTS FROM STANDARDS - 29 CFR, SECTION 1926, SUBPART R:

A. THE STEEL ERECTION CONTRACTOR SHALL NOT ERECT STEEL UNLESS THEY HAVE RECEIVED WRITTEN NOTIFICATION FROM THE CONTRACTOR THAT THE CONCRETE IN THE FOOTINGS, PIERS AND WALLS OR THE MORTAR IN THE MASONRY PIERS AND WALLS HAS ATTAINED, ON THE BASIS OF AN APPROPRIATE ASTM STANDARD TEST METHOD OF FIELD-CURED SAMPLES, EITHER 75 PERCENT OF THE INTENDED MINIMUM COMPRESSIVE DESIGN STRENGTH OR SUFFICIENT STRENGTH TO SUPPORT THE LOADS IMPOSED DURING STEEL ERECTION.

GENERAL STRUCTURAL NOTES

PROVIDE STRUCTURAL ENGINEER A COPY OF WRITTEN NOTIFICATION WHEN IT IS PROVIDED TO THE STEEL ERECTOR.

B. ANCHOR RODS (ANCHOR BOLTS) SHALL NOT BE REPAIRED, REPLACED OR FIELD-MODIFIED WITHOUT THE APPROVAL OF THE PROJECT STRUCTURAL ENGINEER OF RECORD.

PRIOR TO ERECTION OF COLUMNS, THE CONTRACTOR SHALL PROVIDE WRITTEN NOTIFICATION TO THE STEEL ERECTOR IF THERE HAS BEEN ANY REPAIR, REPLACEMENT OR MODIFICATION OF THE ANCHOR RODS (ANCHOR BOLTS).

PROVIDE STRUCTURAL ENGINEER A COPY OF WRITTEN NOTIFICATION WHEN IT IS PROVIDED TO THE STEEL ERECTOR.

C. NO MODIFICATION THAT AFFECTS THE STRENGTH OF A STEEL JOIST OR STEEL JOIST GIRDER SHALL BE MADE WITHOUT THE APPROVAL OF THE PROJECT STRUCTURAL ENGINEER OF RECORD.

D. METAL DECKING HOLES AND OPENINGS SHALL NOT BE CUT UNTIL IMMEDIATELY PRIOR TO BEING PERMANENTLY FILLED WITH THE EQUIPMENT OR STRUCTURE, OR SHALL BE IMMEDIATELY COVERED.

PROTECTION: PROPER PRECAUTIONS SHALL BE TAKEN AT ALL TIMES TO PROTECT VEHICULAR AND PEDESTRIAN TRAFFIC FROM ANY DAMAGE OR INJURY WHICH MAY BE CAUSED, EITHER DIRECTLY OR INDIRECTLY, BY THE WORK INCLUDED ON THESE DRAWINGS. SUCH PRECAUTIONS SHALL INCLUDE THE ERECTION AND MAINTENANCE OF FENCES, BARRICADES, RAILINGS, GUARDS, SIGNS, COVERINGS, LIGHTS, AND OTHER PRECAUTIONS AS MAY BE REQUIRED. IF AT ANY TIME, IN THE OPINION OF THE OWNER OR THE OWNER'S REPRESENTATIVE, PROPER PRECAUTIONS ARE NOT BEING TAKEN TO SECURE THIS PROTECTION, THE CONTRACTOR SHALL AT NO ADDITIONAL COST TO THE OWNER, INSTALL AND MAINTAIN SUCH ADDITIONAL PROTECTION AS MAY BE DIRECTED BY THE OWNER.

POLLUTION CONTROLS: USE WATER SPRINKLING, TEMPORARY ENCLOSURES, AND OTHER SUITABLE METHODS TO LIMIT DUST AND DIRT RISING AND SCATTERING IN THE AIR TO LOWEST PRACTICAL LEVEL. COMPLY WITH GOVERNING REGULATIONS PERTAINING TO ENVIRONMENTAL PROTECTION.

TYPICAL DETAIL SHEETS:

THE S7.00 SERIES SHEETS IN THESE DRAWINGS CONTAIN TYPICAL STRUCTURAL DETAILS FOR VARIOUS BUILDING MATERIALS. SOME OF THESE DETAILS MAY NOT BE PART OF THIS PROJECT.

DRAWINGS:

DO NOT SCALE DRAWINGS.

WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL NOTES, AND SPECIFICATIONS, THE MORE STRINGENT REQUIREMENTS SHALL GOVERN. DETAILS ON DRAWINGS TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS. DETAILS NOTED "TYPICAL" APPLY TO ALL SIMILAR CONDITIONS. WHERE NO SPECIFIC DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ELSEWHERE ON THE PROJECT.

FAST-TRACK/PHASED CONSTRUCTION:

THE STRUCTURAL PORTION OF THIS PROJECT IS BEING DESIGNED, BID, PERMITTED, AND CONSTRUCTED PRIOR TO THE COMPLETION OF ARCHITECTURAL, ENGINEERING, AND OTHER DESIGN TEAM CONSTRUCTION DOCUMENTS. THE OWNER, ARCHITECT, AND CONTRACTOR SHALL BE AWARE THAT THIS ACCELERATED STRUCTURAL SCHEDULE CREATES INHERENT RISK OF FUTURE CHANGES DUE TO DESIGN COORDINATION WITH OTHER DISCIPLINES. WHILE EVERY EFFORT HAS BEEN MADE TO MINIMIZE THESE CHANGES, THE RISK OF ADDED COSTS DUE TO THESE CHANGES SHALL BE UNDERSTOOD AND ACCEPTED BY ALL PARTIES.

DRAWINGS THAT DO NOT HAVE AN ENGINEERING SEAL BY THE STRUCTURAL ENGINEER OF RECORD OR NOT LABELED AS CONSTRUCTION DRAWINGS ARE PRELIMINARY AND SUBJECT TO CHANGE. IF THESE DOCUMENTS ARE BEING USED FOR PRICING, BIDDING, STEEL MILL ORDER, OR PREPARATION OF SHOP DRAWINGS, THE CONTRACTOR SHALL ANTICIPATE FUTURE DRAWING REVISIONS THAT MAY AFFECT THIS WORK OR INCREASE CONSTRUCTION COSTS. THE STRUCTURAL ENGINEER SHALL NOT BE RESPONSIBLE FOR ANY CHANGE ORDER COSTS INCURRED DUE TO THESE DRAWING REVISIONS, AND THE CONTRACTOR SHALL CONSIDER THESE ANTICIPATED COSTS IN ANY BIDS OR PRICE GUARANTEES TO THE OWNER.

USE THE MOST CURRENT SET OF DRAWINGS IN PREPARATION OF ALL SUBMITTALS. ALL SUBMITTALS SHALL LIST THE DATE OF THE DRAWINGS USED TO PREPARE THE SUBMITTAL. SUBMITTALS PREPARED FROM OUTDATED DRAWINGS MAY BE REJECTED. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ACQUIRING THE LATEST SET OF CONSTRUCTION DRAWINGS AND DISTRIBUTING TO THE APPROPRIATE PARTIES.

CAST-IN-PLACE CONCRETE:

ALL CONCRETE SHALL CONFORM TO THE SPECIFICATIONS FOR STRUCTURAL CONCRETE, ACI 301-10.

ALL EXPOSED EDGES OF CONCRETE SHALL HAVE A 3/4" CHAMFER UNLESS NOTED OTHERWISE.

NORMALWEIGHT CONCRETE:

A. F'C = 4500 PSI @ 28 DAYS – ALL CONCRETE EXPOSED TO FREEZE/THAW CYCLES AND OCCASIONAL MOISTURE, INCLUDING CONCRETE FLAT WORK, EXPOSED BUILDING STEM WALLS, SITE WALLS, ETC…
EXTERIOR CONCRETE SHALL MEET EXPOSURE CATEGORY AND CLASS F1 ACCORDING TO ACI 318 TABLE 19.3.1.1.
B. F'C = 3000 PSI @ 28 DAYS - ALL FOOTINGS, TIE BEAMS, GRADE BEAMS.
C. F'C = 3000 PSI @ 28 DAYS - ALL INTERIOR SLABS ON GRADE, UNLESS NOTED OTHERWISE.
D. F'C = 3500 PSI @ 28 DAYS - ALL CONCRETE FILL OVER METAL DECK, UNLESS NOTED OTHERWISE.
E. F'C = 4000 PSI @ 28 DAYS – ALL CAST-IN-PLACE CONCRETE COLUMNS, PEDESTALS, RETAINING WALLS, AND ELEVATED BEAMS.
F. F'C = 4000 PSI @ 28 DAYS – ALL ELEVATED CAST-IN-PLACE SLABS.
G. F'C = 4000 PSI @ 28 DAYS – ALL SLABS ON GRADE AND ELEVATED SLABS TO RECEIVE POLISHED CONCRETE FINISH.

FIRE RATED SLABS: COORDINATE AIR CONTENT REQUIREMENTS WITH ARCHITECTURAL DRAWINGS.

CONCRETE MIX DESIGNS (INCLUDING AIR CONTENT, WATER TO CEMENT RATIOS, AND OTHER CRITERIA) SHALL CONFORM TO THE REQUIREMENTS SET FORTH IN ACI 318 TABLE 19.3.2.1, BASED ON THE EXPOSURE CATEGORIES AND CLASSES DEFINED IN ACI 318 TABLE 19.3.1.1. USE AIR ENTRAINING ADMIXTURE IN ALL EXTERIOR CONCRETE. AIR CONTENT IN FIRE RATED SLABS SHALL ALSO COMPLY WITH THE REQUIREMENTS IN THE SPECIFIED UL LISTING.

COLD WEATHER CONCRETING: PROTECT CONCRETE WORK FROM PHYSICAL DAMAGE OR REDUCED STRENGTH CAUSED BY FROST, FREEZING OR LOW TEMPERATURES. COMPLY WITH ACI 306.1.

HOT WEATHER CONCRETING: WHEN HOT WEATHER CONDITIONS EXIST THAT WOULD IMPAIR THE QUALITY AND STRENGTH OF THE CONCRETE, REDUCE DELIVERY TIME OF READY MIX CONCRETE, LOWER THE TEMPERATURE OF MATERIALS, OR ADD RETARDER TO ENSURE THAT THE CONCRETE IS PLASTIC. RETEMPERING WITH WATER IS NOT ALLOWED. COMPLY WITH ACI 305R.

SLAB CURING: ALL INTERIOR CONCRETE SLABS, EXCEPT EXPOSED INTEGRALLY COLORED SLABS, ARE TO BE CURED WITH A MOISTURE RETAINING COVER FOR THE FIRST 7 DAYS (MINIMUM) AFTER PLACEMENT.

THE CONTRACTOR IS ALLOWED TO CAST FOUNDATIONS AGAINST EXCAVATED SOIL SURFACES, PROVIDED THE FOLLOWING IS ADHERED TO:

A. THE SIDE SLOPES OF THE EXCAVATION SHALL BE ABLE TO MAINTAIN VERTICAL SLOPE WITHOUT SOIL SLOUGHAGE.
B. THE BOTTOM WIDTH OF THE EXCAVATION SHALL BE ONE INCH WIDER MINIMUM ON EACH SIDE THAN THE SPECIFIED FOOTING WIDTH.
C. THE SIDE WALLS OF THE EXCAVATION SHALL BE BATTERED A MINIMUM OF ONE INCH HORIZONTAL TO TWELVE INCHES VERTICAL.
D. IF SANDY OR LOOSE MATERIALS ARE ENCOUNTERED, THE FOOTING MUST BE FORMED.
E. THE CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVAL OF ANY SOIL SLOUGHAGE FROM THE WET CONCRETE DURING THE CASTING OPERATION.
F. THE CONTRACTOR AGREES TO REMOVE AND RECAST ANY FOOTING WHERE THE ABOVE CONDITIONS ARE NOT MET.

EXPOSED SITE WALLS, RETAINING WALLS, AND STEM WALLS GREATER THAN 30 FEET IN LENGTH SHALL HAVE CONTROL JOINTS INSTALLED AT THE FOLLOWING MAXIMUM SPACING:

12'-0" ON CENTER FOR WALLS 6'-0" MAXIMUM HEIGHT
18'-0" ON CENTER FOR WALLS 10'-0" MAXIMUM HEIGHT
20'-0" ON CENTER FOR WALLS GREATER THAN 10'-0" IN HEIGHT

ALL CONCRETE EXPOSED TO GROUND SHALL BE MANUFACTURED WITH PORTLAND CEMENT TYPE I OR TYPE II.

SEE SHEET S7.11 FOR TYPICAL CONCRETE DETAILS.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE FABRICATED AND PLACED IN ACCORDANCE WITH THE BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI 318-14), AND DETAILS AND DETAILING OF CONCRETE REINFORCEMENT (ACI 315-99).

ALL REINFORCING STEEL SHALL CONFORM TO ASTM A615 GRADE 60; EXCEPT STIRRUPS, TIES AND INDICATED FIELD-BENT BARS, WHICH SHALL CONFORM TO ASTM A615 GRADE 40.

ALL WELDED WIRE FABRIC SHALL BE DEFORMED AND SHALL CONFORM TO ASTM A479. PROVIDE IN FLAT SHEETS ONLY.

TENSION AND COMPRESSION LAPS IN REINFORCING SHALL CONFORM TO THE LAP SPLICE SCHEDULE ON SHEET S6.01 AND BE IN ACCORDANCE WITH ACI 318, CHAPTER 12, UNLESS NOTED OTHERWISE.

ALL HORIZONTAL REINFORCING IN FOOTINGS, WALLS AND BEAMS SHALL BE CONTINUOUS AROUND CORNERS OR HAVE BENT (CORNER) BARS OF THE SAME SIZE AND SPACING AS THE HORIZONTAL BARS AND LAP 30 BAR DIAMETERS (24" MINIMUM).

GENERAL STRUCTURAL NOTES

CONCRETE COVER FOR REINFORCING SHALL BE AS FOLLOWS UNLESS OTHERWISE NOTED:

A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH:	3"
B. CONCRETE CAST AGAINST FORMS BUT EXPOSED TO EARTH OR WEATHER:	
1. BARS LARGER THAN NO. 5:	2"
2. BARS NO. 5 OR SMALLER:	1 1/2"
C. CONCRETE NOT EXPOSED TO WEATHER OR NOT IN CONTACT WITH GROUND:	
1. COLUMNS, GIRDERS AND BEAMS:	1 1/2"
2. STRUCTURAL SLABS, WALLS AND JOISTS (NO. 11 AND SMALLER):	3/4"
D. SLAB ON GRADE:	1 1/2" FROM TOP OF SLAB
E. STRUCTURAL SLABS ON METAL DECK:	1" FROM TOP OF SLAB

FORM TIES SHALL BE EITHER OF THE THREADED OR SNAP-OFF TYPE SO THAT NO METAL WILL BE LEFT WITHIN 1 INCH OF THE SURFACE OF THE WALL. FOLLOWING REMOVAL OF FORM TIES, RECESSES ARE TO BE CAREFULLY FILLED AND POINTED WITH MORTAR.

REINFORCING SHALL NOT BE TACK WELDED OR WELDED IN ANY MANNER UNLESS SPECIFICALLY DETAILED ON THE STRUCTURAL PLANS.

BAR SUPPORTS AND SPACERS FOR REINFORCING SHALL BE PROVIDED IN ACCORDANCE WITH ACI 315-99. REINFORCING SHALL BE SECURELY TIED TO SUPPORTS.

CHAIRS WITH 22 GAGE SAND PLATES OR PRECAST BLOCKS SHALL BE PROVIDED FOR ALL REINFORCING OF CONCRETE IN CONTACT WITH GRADE.

DECK CHAIRS SHALL BE PROVIDED FOR ALL WELDED WIRE FABRIC IN SLABS OVER METAL DECK.

POST INSTALLED ANCHORS:

THE STRUCTURAL DESIGN IS BASED ON THE POST INSTALLED ANCHORING SYSTEMS NOTED BELOW. SINCE ANCHOR CAPACITIES VARY BY MANUFACTURER, THE CONTRACTOR SHALL USE ONLY THE SYSTEMS NOTED BELOW UNLESS AN ALTERNATE IS APPROVED BY THE ENGINEER OF RECORD. ALTERNATE ANCHORING SYSTEMS MAY REQUIRE RE-DESIGN TO VERIFY ANCHOR QUANTITIES, SPACING, AND EMBED DEPTHS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY ADDITIONAL CONSTRUCTION AND RE-DESIGN COSTS ASSOCIATED WITH THE ALTERNATE ANCHORING SYSTEM.

ALL ADHESIVE (EPOXY) FOR POST INSTALLED ANCHORS AND/OR REBAR INTO GROUT FILLED MASONRY SHALL BE HILTI HIT HY 70 ADHESIVE ANCHORING SYSTEM. INSTALLATION SHALL BE PER THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.

ALL ADHESIVE (EPOXY) FOR POST INSTALLED ANCHORS AND/OR REBAR INTO HOLLOW MASONRY AND/OR BRICK SHALL BE HILTI HIT HY 270 ADHESIVE ANCHORING SYSTEM. INSTALLATION SHALL BE PER THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.

ALL POST INSTALLED MECHANICAL ANCHORS INTO CONCRETE SHALL BE HILTI KWIK HUS EZ (KH-EZ) SCREW ANCHOR. INSTALLATION SHALL BE PER THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.

ANCHOR LENGTHS SHOWN FOR ATTACHMENT TO CONCRETE AND/OR MASONRY ARE REQUIRED EMBEDMENT LENGTHS. THE CONTRACTOR SHALL PROVIDE ANCHORS WITH ADDITIONAL LENGTH TO FACILITATE THE REQUIRED CONNECTION.

SUBMIT ALL PROPOSED ANCHORING SYSTEMS INCLUDING ICC-ES REPORTS TO STRUCTURAL ENGINEER FOR REVIEW PRIOR TO INSTALLATION. THE ICC–ES FORMS SHALL MEET THE REQUIREMENTS OF THE IBC REFERENCED IN THESE NOTES.

ALL POST-INSTALLED ANCHORS SHALL BE INSTALLED WITH SPECIAL INSPECTION AS DICTATED BY THE RESPECTIVE PRODUCT'S ICC-ES EVALUATION SERVICE REPORT

THE CONTRACTOR SHALL ARRANGE AN ANCHOR MANUFACTURER'S REPRESENTATIVE TO PROVIDE ONSITE INSTALLATION TRAINING, UNLESS ALL PERSONNEL INSTALLING ANCHORS ARE CERTIFIED IN ACCORDANCE WITH ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM, OR EQUIVALENT APPROVED BY THE ENGINEER OF RECORD.

INSTALLATION OF ADHESIVE ANCHORS IN HORIZONTAL TO VERTICALLY OVERHEAD ORIENTATION SHALL BE CONTINUOUSLY INSPECTED DURING INSTALLATION BY AN INSPECTOR SPECIALLY APPROVED FOR THAT PURPOSE BY THE BUILDING OFFICIAL. INSTALLATION SHALL BE DONE BY A CERTIFIED ADHESIVE ANCHOR INSTALLER (AAI) AS CERTFIEID THROUGH ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM, OR EQUIVALENT. PROOF OF CURRENT CERTIFICATION SHALL BE SUBMITTED TO THE ENGINEER PRIOR TO COMMENCEMENT OF INSTALLATION, AND INSPECTION REPORTS SHALL BE PROVIDED TO THE ENGINEER OF RECORD AND THE BUILDING OFFICIAL.

STRUCTURAL AND MISCELLANEOUS STEEL:

ALL STRUCTURAL STEEL SHALL BE DETAILED AND FABRICATED IN ACCORDANCE WITH THE AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS".

ALL WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A992, GRADE 50, UNLESS NOTED OTHERWISE.

ALL MISCELLANEOUS STEEL MEMBERS, SUCH AS CHANNELS, ANGLES, FLAT BARS, AND PLATES SHALL CONFORM TO ASTM A36 UNLESS NOTED OTHERWISE.

ALL RECTANGULAR AND SQUARE STRUCTURAL TUBING SHALL CONFORM TO ASTM A500, GRADE B, FY = 46 KSI OR ASTM 1085, GRADE B, FY = 50 KSI.

ALL ROUND STRUCTURAL TUBING SHALL CONFORM TO ASTM A500, GRADE B, FY = 42 KSI OR ASTM 1085, GRADE B, FY = 50 KSI.

ALL STRUCTURAL PIPE SHALL CONFORM TO ASTM A53, TYPE E OR S, GRADE B, FY = 35 KSI.

BOLTS SHALL CONFORM TO ASTM A325N TENSION CONTROL BOLTS UNLESS NOTED OTHERWISE, WITH SIZES AS SHOWN ON THE DRAWINGS. WHERE CLEARANCE WITHIN A CONNECTION DOES NOT PERMIT THE USE OF TENSION CONTROL BOLTS, STANDARD A325N BOLTS SHALL BE USED AND INSPECTED IN ACCORDANCE WITH THE AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS".

ALL BOLTS SHALL BE INSTALLED IN A SNUG TIGHT CONDITION EXCEPT AT MOMENT CONNECTIONS, BRACED FRAME CONNECTIONS, AND AT CONNECTIONS DETAILED WITH A325SC BOLTS. AT THESE LOCATIONS, THE BOLTS SHALL BE TIGHTENED SO AS TO SHEAR THE SPLINE OFF THE BOLT.

ANCHOR BOLTS EMBEDDED IN CONCRETE SHALL BE ASTM F1554 GRADE 36 THREADED RODS WITH DOUBLE NUTS. PROVIDE FLAT WASHERS BETWEEN NUTS AND BASEPLATE SURFACES. ANCHOR BOLT LENGTHS SHOWN FOR ATTACHMENT TO CONCRETE AND/OR MASONRY ARE REQUIRED EMBEDMENT LENGTHS. THE CONTRACTOR SHALL PROVIDE ANCHOR BOLTS WITH ADDITIONAL BOLT LENGTH TO FACILITATE THE REQUIRED CONNECTION.

ANCHOR BOLT FLAT WASHERS SHALL BE PROVIDED IN ACCORDANCE WITH TABLE 14-2 OF AISC 360, AISC MANUAL OF STEEL CONSTRUCTION LATEST EDITION.

ALL WELDING SHALL BE DONE IN ACCORDANCE WITH THE LATEST STANDARDS OF THE AWS STRUCTURAL WELDING CODE.

ALL BOLT HOLES THAT ARE REQUIRED TO BE FIELD DRILLED SHALL BE DRILLED WITH A MAG DRILL. FLAME CUTTING OF HOLES OR ENLARGING OF MISALIGNED HOLES WILL NOT BE ALLOWED.

HEADED CONCRETE ANCHORS AND SHEAR CONNECTORS SHALL BE MADE FROM STEEL CONFORMING TO ASTM A108 AND MEET THE MECHANICAL PROPERTIES OF TYPE B, AS REQUIRED BY CHAPTER 7 OF AWS D1.1 "STRUCTURAL WELDING CODE-STEEL", LATEST EDITION. STRUCTURAL STEEL TO RECEIVE SHEAR CONNECTORS SHALL BE FREE OF PAINT. WELDING PREQUALIFICATION REQUIRED.

COMPOSITE FLOORS:

THE METAL DECK FOR COMPOSITE FLOORS SHALL BE UNSHORED UNLESS NOTED OTHERWISE.

THE SHEAR CONNECTORS SHALL BE 3/4" DIAMETER X 3" AT 1 1/2" DEEP DECK AND 3/4" DIAMETER X 4 1/2" AT 3" DEEP DECK UNLESS NOTED OTHERWISE. THE SHEAR CONNECTORS SHALL BE MADE FROM STEEL CONFORMING TO ASTM A108 AND MEET THE MECHANICAL PROPERTIES OF TYPE B, AS REQUIRED BY CHAPTER 7 OF AWS D1.1 "STRUCTURAL WELDING CODE STEEL", LATEST EDITION. STRUCTURAL STEEL TO RECEIVE SHEAR CONNECTIONS SHALL BE FREE OF PAINT. WELDING PREQUALIFICATION REQUIRED.

THE SHEAR CONNECTIONS SHALL NOT BE ADDED UNTIL THE METAL FLOOR DECK IS INSTALLED.

WHERE SHEAR CONNECTIONS AND PUDDLE WELDS COINCIDE, THE SHEAR CONNECTOR MAY REPLACE THE PUDDLE WELD.

CAMBERED BEAMS SHALL HAVE THE CAMBER PUT IN AT 1/3 POINTS OR ALONG A PARABOLIC CURVE.

THE CONTRACTOR SHALL SURVEY THE CAMBER OF THE BEAMS AFTER THE BEAMS HAVE BEEN ERECTED. THE CONTRACTOR SHALL SUBMIT THE SURVEY TO THE ENGINEER FOR REVIEW. THE CONTRACTOR SHALL NOT POUR THE SLAB UNTIL THE ENGINEER HAS REVIEWED AND APPROVED THE BEAM CAMBERS.

CONTRACTOR SHALL SHORE BEAMS WITH A CAMBER MORE THAN 1/2" LOWER THAN SPECIFIED. THE BEAM SHALL BE ALLOWED TO DEFLECT TO LEVEL.

GENERAL STRUCTURAL NOTES

THE TOPPING SLAB SHALL BE POURED AND PLACED TO THE ELEVATION INDICATED ON THE DRAWINGS.

THE CONCRETE FOR THE SLAB SHALL BE POURED AND PLACED TO THE ELEVATION INDICATED ON THE DRAWINGS WHILE MAINTAINING THE MINIMUM THICKNESS. SPREAD CONCRETE OVER AREA OF INFLUENCE TO ROUGH DEPTH IN ORDER TO LOAD BEAMS AND GIRDERS PRIOR TO SETTING SCREED ELEVATIONS.

THE WEIGHT OF THE WET CONCRETE WILL CAUSE DEFLECTIONS OF THE STEEL FRAMING. THEREFORE, CONCRETE OVERRRUNS ARE TO BE ANTICIPATED BY THE CONTRACTOR.

CONTRACTOR SHALL CONTINUOUSLY MONITOR THE THICKNESS AND ELEVATIONS DURING CONCRETE PLACING OPERATIONS.

PROVIDE #4 X 6'-0" AT 12" ON CENTER OVER ALL GIRDERS OF COMPOSITE FLOORS.

PROVIDE #4 X 6'-0" AT 12" ON CENTER OVER SHORED BEAMS THAT ARE NOT ALLOWED TO DEFLECT TO LEVEL.

PROVIDE WELDED WIRE FABRIC AS INDICATED ON DRAWINGS IN FLAT SHEETS ONLY.

PROVIDE DECK CHAIRS FOR ALL WELDED WIRE FABRIC IN SLABS OVER METAL DECK.

STEEL DECK:

ALL STEEL DECK SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE STEEL DECK INSTITUTE SPECIFICATIONS.

SEE PLANS FOR STEEL DECK TYPE, GAGE, FINISH AND CONNECTIONS.

PROVIDE A MINIMUM OF 1 1/2" BEARING FOR ALL STEEL DECK.

ALL SPLICES AND LAPS SHALL BE A MINIMUM OF 2" IN LENGTH AND SHALL BE LOCATED DIRECTLY ABOVE SUPPORTS.

ALL DECKING SHALL BE CONTINUOUS OVER TWO OR MORE SPANS.

COLD-FORMED METAL FRAMING (43 MILS OR HEAVIER):

ALL COLD-FORMED METAL FRAMING SHALL CONFORM TO THE LATEST EDITION OF AISI STANDARD S100 "NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS".

WALLS SHALL BE PROVIDED WITH MANUFACTURER'S STANDARD BRIDGING: (EITHER WELDED 2 1/2" x 43 MILS STUD OR CLIPPED COLD-ROLLED CHANNEL 1 1/2" x 54 MILS). PROVIDE BRIDGING AT 4'-0" ON CENTER MAXIMUM FOR LOAD BEARING WALLS AND EXTERIOR WALLS.

PROVIDE ALL MISCELLANEOUS ACCESSORIES AND FOLLOW ERECTION PROCEDURES AS PER MANUFACTURER'S SPECIFICATIONS AND RECOMMENDATIONS UNLESS NOTED OTHERWISE.

COLD-FORMED METAL FRAMING SHALL MEET THE MINIMUM PROPERTIES AS SHOWN IN THE STEEL STUD MANUFACTURERS ASSOCIATION (SSMA) SPECIFICATIONS.

ALL TRACK SHALL BE ANCHORED TO CONCRETE NOT MORE THAN 4' - 0" ON CENTER. ATTACHMENT BY OTHERS.

SECURE STUDS TO TOP AND BOTTOM TRACKS, ATTACHMENT BY OTHERS.

WELD CLIPS DESIGNED BY OTHERS. FASTEN WELD CLIPS TO STUDS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS AND LOAD DATA. WELD CLIPS SHALL BE DESIGNED FOR LATERAL LOADS AS SPECIFIED ON S0.03 AND VERTICAL LOADS AS DICTATED BY ARCHITECTURAL FINISHES AND SUPPORT REQUIREMENTS.

SLIDE CLIPS DESIGNED BY OTHERS. FASTEN SLIDE CLIPS TO STUDS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS AND LOAD DATA. SLIDE CLIPS SHALL BE DESIGNED FOR LATERAL LOADS AS SPECIFIED ON S0.03.

SEE SHEET S7.31 FOR TYPICAL COLD-FORMED DETAILS.

VENEER:

FOR CMU OR BRICK VENEER (4" MAXIMUM, 2" MINIMUM THICKNESS) ATTACHMENT TO STRUCTURAL MASONRY, PROVIDE ADJUSTABLE INTEGRAL ANCHOR TIES. ADJUSTABLE INTEGRAL ANCHOR TIES SHALL BE CORROSION RESISTANT AND HAVE TWO PINTLE LEGS MINIMUM WITH W2.8 (3/16") WIRE OR APPROVED EQUAL. PROVIDE DUR-O-WALL DA370 ADJUSTABLE INTEGRAL ANCHOR TIES OR APPOVED EQUAL.

FOR CMU OR BRICK VENEER (4" MAXIMUM, 2" MINIMUM THICKNESS) ATTACHMENT TO STRUCTURAL CONCRETE, PROVIDE ADJUSTABLE ANCHOR TIES. ADJUSTABLE ANCHOR TIES SHALL BE CORROSION RESISTANT AND HAVE A TWO PINTLE LEGS MINIMUM WITH A MINIMUM W2.8 (3/16") WIRE. ATTACH TO CONCRETE WITH 2-1/4" DIAMETER CONCRETE SCREWS, HILTI KWIKCON 11 x 1 1/2" OR APPROVED EQUAL.

FOR CMU OR BRICK VENEER (4" MAXIMUM, 2" MINIMUM THICKNESS) ATTACHMENT TO STRUCTURAL COLD FORMED METAL STUDS, PROVIDE ADJUSTABLE ANCHOR TIES. ADJUSTABLE ANCHOR TIES SHALL BE CORROSION RESISTANT AND HAVE TWO PINTLE LEGS MINIMUM W2.8 (3/16") WIRE. PROVIDE DUR-O-WALL DA213 ADJUSTABLE ANCHOR TIE OR APPROVED EQUAL. ATTACH THROUGH SHEATHING TO STUDS WITH 2-1/4" x 1 1/2" CORROSION RESISTANT TEK SCREWS.

SEE TYPICAL DETAILS ON SHEET S7.11, S7.21, AND S7.31 FOR VENEER TIE SPACING.

PROVIDE ADDITIONAL ANCHORS AROUND ALL OPENINGS LARGER THAN 16" IN EITHER DIMENSION. SPACE ANCHORS WITHIN 12" OF OPENING PERIMETER AND MATCH HORIZONTAL OR VERTICAL ANCHOR TIE SPACING.

COORDINATE VENEER LOCATION, TYPE, BOND PATTERN, ETC. WITH ARCHITECTURAL DRAWINGS.

GLASS CURTAIN WALL SYSTEM:

ALL LATERAL AND GRAVITY SUPPORT FOR THE GLASS CURTAIN WALL SYSTEM SHALL BE PER THE MANUFACTURER'S RECOMMENDATIONS. SHOP DRAWINGS AND STAMPED CALCULATIONS SHALL BE SUBMITTED FOR APPROVAL BY THE ENGINEER OF RECORD AND THE ARCHITECT PRIOR TO INSTALLATION.

THE ENGINEER STAMPING THE SHOP DRAWINGS SHALL BE REGISTERED IN THE STATE THAT THE PROJECT IS LOCATED.

THE GLASS CURTAIN WALL SYSTEM SHALL BE LATERALLY SUPPORTED AT ALL FLOORS AND ROOF LEVEL.

SEISMIC BRACING OF NON-STRUCTURAL COMPONENTS:

SEISMIC BRACING AND RESTRAINTS FOR MECHANICAL/ELECTRICAL EQUIPMENT AND SYSTEMS SHALL BE PROVIDED BY THE CONTRACTOR PER THE 2015 INTERNATIONAL BUILDING CODE (2015 IBC) AND THE ASCE 7-10, MINIMUM DESIGN LOADS FOR BUILDINGS AND STRUCTURES.

SEE THESE GENERAL STRUCTURAL NOTES FOR THE SITE-SPECIFIC SEISMIC DESIGN CRITERIA.

THE CONTRACTOR IS RESPONSIBLE FOR DESIGNING AND INSTALLING THE SEISMIC BRACING AND RESTRAINTS. STAMPED SHOP DRAWINGS, INCLUDING CALCULATIONS, SHALL BE SUBMITTED FOR APPROVAL PRIOR TO ANY BRACING INSTALLATION.

THE ENGINEER STAMPING THE SHOP DRAWINGS SHALL BE REGISTERED IN THE STATE THAT THE PROJECT IS LOCATED.

ELEVATORS:

THE STRUCTURE HAS BEEN DESIGNED FOR A KONE MONOSPACE 500: 2500/4000 (SUBJECT TO CHANGE) ELEVATOR.

ALL STRUCTURAL SUPPORTS, FLOOR PENETRATION SIZES AND PIT DIMENSIONS HAVE BEEN DESIGNED BASED ON THE ABOVE INFORMATION. SHOULD THE ACTUAL ELEVATOR(S) SELECTED DIFFER FROM THE INFORMATION PROVIDED ABOVE, THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY ADDITIONAL CONSTRUCTION AND REDESIGN COSTS ASSOCIATED WITH THE ALTERNATE ELEVATOR(S).

THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING ALL ELEVATOR PIT AND FLOOR PENETRATION LOCATIONS AND DIMENSIONS

DEFERRED SUBMITTALS:

THE DEFERRED SUBMITTALS LISTED BELOW ARE THOSE PORTIONS OF THE DESIGN THAT ARE NOT COMPLETED AT THE TIME OF APPLICATION AND ARE TO BE SUBMITTED TO THE BUILDING OFFICIAL AND APPROVED PRIOR TO THE INSTALLATION OF THOSE ITEMS. THE MANUFACTURER, CONSULTANT, OR CONTRACTOR, AS APPROPRIATE, SHALL PROVIDE SUBMITTALS TO THE ENGINEER OF RECORD FOR REVIEW FOR THE FOLLOWING ITEMS.

- PRECAST PRESTRESSED CONCRETE
- COLD-FORMED METAL (LIGHTGAGE) FRAMING
- METAL STAIRS, RAMPS, LADDERS, AND GUARDRAILS
- DEWATERING/DRAINAGE SYSTEM BEHIND BASEMENT WALLS, INCLUDING ITS TIE IN TO THE STORM WATER SYSTEM
- SEISMIC BRACING OF ALL ARCHITECTURAL, LIGHTING, THEMING, FIRE SUPPRESSION, AND MEP COMPONENTS
- CURTAINWALL SYSTEMS
- TEMPORARY AND PERMANENT SHORING OF EXISTING STRUCTURES AND SOILS WHERE REQUIRED FOR NEW CONSTRUCTION

NOTE: THIS STRUCTURAL PACKAGE IS FOR FOUNDATIONS ONLY. ANY CHANGES TO THE PROJECT, INCLUDING, BUT NOT LIMITED TO: LOADING REQUIREMENTS, GEOMETRY CHANGES IN PLAN OR ELEVATION, SPACE USAGE REVISIONS, OR VALUE ENGINEERING MAY AFFECT THE STRUCTURAL STEEL MEMBER REQUIREMENTS SHOWN IN THESE DRAWINGS.

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SEAL OF THE CHEROKEE NATION
SEPT. 6, 1839

COLLEGE OF Osteopathic Medicine AT THE CHEROKEE NATION
TAHLEQUAH, OKLAHOMA

KEY PLAN:

PROJECT PHASE: BID PACKAGE 03

REVISIONS		
#	DATE	DESCRIPTION

DATE: 03-20-19
JOB NUMBER: 17-13
SHEET NUMBER: S0.02

GENERAL STRUCTURAL NOTES

3" = 1'-0"

1 1/2" = 1'-0"

1" = 1'-0"

3/4" = 1'-0"

1/2" = 1'-0"

3/8" = 1'-0"

1/4" = 1'-0"

1/8" = 1'-0"

SCHEDULE OF STRUCTURAL SPECIAL INSPECTIONS

1. SPECIAL INSPECTIONS / TESTING - "SPECIAL STRUCTURAL INSPECTION" SHALL NOT RELIEVE THE OWNER OR THEIR AGENT FROM HAVING THE INSPECTIONS OF THE JURISDICTION BUILDING DEPARTMENT PER SECTION 110 OF THE IBC PERFORMED. BOTH THE JURISDICTION BUILDING DEPARTMENT INSPECTIONS AND "SPECIAL STRUCTURAL INSPECTION" SHALL BE PERFORMED.

2. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR NOTIFYING THE JURISDICTION BUILDING OFFICIAL AND SPECIAL INSPECTOR WHEN WORK IS READY FOR INSPECTION.

3. REPORTING FOR SPECIAL INSPECTION - SPECIAL INSPECTION AND TESTING REPORTS SHALL BE COMPLETED AND DISTRIBUTED AT THE COMPLETION OF EACH TASK. IF A TASK IS TO TAKE LONGER THAN THREE (3) DAYS, PROVIDE REPORTS FOR EACH DAY. PROVIDE COPIES OF REPORTS TO CONTRACTOR, OWNER, ARCHITECT AND STRUCTURAL ENGINEER OF RECORD. SPECIAL INSPECTOR TO KEEP A NON-COMPLIANCE LIST DOCUMENTING ITEMS INSPECTED NOT MEETING APPROVED CONSTRUCTION DOCUMENTS AND WHEN / HOW RESOLVED.

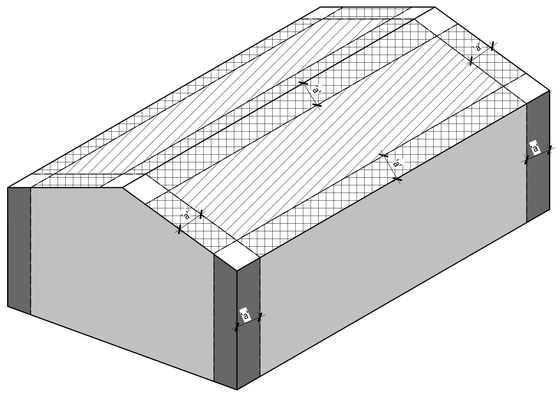
4. SEE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING CONSTRUCTION DOCUMENTS FOR ADDITIONAL NON-STRUCTURAL SPECIAL INSPECTION ITEMS.

5. SPECIAL INSPECTION OF SHOP FABRICATED MEMBERS AND ASSEMBLIES SHALL BE IN ACCORDANCE WITH SECTION 1704.2, UNLESS FABRICATOR IS APPROVED TO PERFORM WORK WITHOUT SPECIAL INSPECTION.

6. IN ACCORDANCE WITH IBC CHAPTER 17, THE OWNER OR THE OWNER'S AGENT, OTHER THAN THE CONTRACTOR, SHALL EMPLOY ONE OR MORE APPROVED AGENCIES TO PROVIDE SPECIAL INSPECTIONS AND TESTS, DURING CONSTRUCTION FOR THE TYPES OF WORK LISTED BELOW THESE SPECIAL INSPECTIONS AND TESTS ARE IN ADDITION TO THE INSPECTIONS BY THE BUILDING OFFICIAL IDENTIFIED IN IBC SECTION 110

7. DEFINITIONS:
   * **SPECIAL INSPECTION:** INSPECTION AS HEREIN REQUIRED BY A QUALIFIED SPECIAL INSPECTOR COMPETENT WITH THE MATERIALS, INSTALLATION, FABRICATION, ERECTION OR PLACEMENT OF COMPONENTS AND CONNECTIONS REQUIRING SPECIAL EXPERTISE TO ENSURE COMPLIANCE WITH APPROVED CONSTRUCTION DOCUMENTS AND REFERENCED STANDARDS (SEE SECTION 1704).
   * **CONTINUOUS SPECIAL INSPECTION:** FULL-TIME OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED.
   * **PERIODIC SPECIAL INSPECTION:** THE PART-TIME OR INTERMITTENT OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK HAS BEEN OR IS BEING PERFORMED AND AT THE COMPLETION OF THE WORK.

ITEM	DESCRIPTION OF REQUIREMENTS	REQUIRED (YES/NO)
SPECIAL INSPECTION OF STRUCTURAL STEEL	TO BE PERFORMED IN ACCORDANCE WITH CHAPTER N OF AISC 360-10	YES
SPECIAL INSPECTION AND VERIFICATION OF STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.2	YES
SPECIAL INSPECTIONS AND VERIFICATIONS FOR CONCRETE CONSTRUCTION	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.3	YES
SPECIAL INSPECTIONS AND VERIFICATIONS FOR WOOD CONSTRUCTION	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.5	NO
SPECIAL INSPECTIONS AND VERIFICATIONS OF SOILS	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.6, THE GEOTECHNICAL REPORT LISTED IN THE GENERAL FOUNDATION NOTES, AND ANY OTHER REQUIREMENTS LISTED IN THE GENERAL FOUNDATION NOTES	YES



COMPONENTS AND CLADDING WIND PRESSURES (PSF) CALCULATED AT MEAN ROOF HEIGHT = 68 FEET			
a = 9 FT	EFFECTIVE WIND AREA (FT²)		
ZONE	10	100	500
1	-58.1	-48.2	-40.7
2	-91.3	-75.8	-63.9
1 AND 2 OVERHANGS	-91.3	-91.3	-91.3
3	-124.4	-103.2	-87.1
3 OVERHANGS	-143.0	-134.4	-128.7
4	39.7	35.0	31.8
4 PARAPETS	131.0	115.3	104.8
5	72.8	55.6	43.7
5 PARAPETS	164.1	125.2	98.5

ZONE 1 ZONE 4
ZONE 2 ZONE 5
ZONE 3

A4 COMPONENT AND CLADDING WIND LOADING DIAGRAM
SCALE: 3/8" = 1'-0"

GENERAL STRUCTURAL NOTES

TEMPORARY SHORING OF EXCAVATIONS:

THE TEMPORARY SHORING OF EXCAVATIONS SHALL BE SOIL NAIL/SHOTCRETE SYSTEM, SHEET PILING, OR APPROVED EQUAL.

THE SHORING SHALL NOT BE DRIVEN OR INSTALLED IN ANY MANNER THAT COULD POTENTIALLY DAMAGE EXISTING STRUCTURES OR CAUSE HUMAN DISCOMFORT.

THE CONTRACTOR SHALL LOCATE ALL EXISTING UNDERGROUND UTILITIES PRIOR TO INSTALLING SHORING. PROVISIONS SHALL BE MADE TO AVOID EXISTING UTILITIES.

THE SHORING AS SHOWN ON THE PLANS IS FOR GRAPHICAL REPRESENTATION ONLY. THE CONTRACTOR SHALL VERIFY THE EXACT LOCATION AND CONFIGURATION OF THE SHORING.

ANY SHORING THAT REMAINS IN PLACE SHALL NOT HAMPER FUTURE CONSTRUCTION.

THE CONTRACTOR IS RESPONSIBLE FOR DESIGNING AND INSTALLING THE TEMPORARY SHORING. STAMPED SHOP DRAWINGS, INCLUDING CALCULATIONS, SHALL BE SUBMITTED FOR APPROVAL PRIOR TO ANY SHORING INSTALLATION.

THE ENGINEER STAMPING THE SHOP DRAWINGS SHALL BE REGISTERED IN THE STATE THAT THE PROJECT IS LOCATED.

DEMOLITION:

NOTCHING OR CUTTING ANY STRUCTURAL MEMBER IN THE FIELD IS PROHIBITED, UNLESS DETAILED OTHERWISE ON THE STRUCTURAL PLANS.

CONTRACTOR SHALL BE RESPONSIBLE FOR ADHERING TO ALL APPLICABLE STANDARDS SET FORTH BY OSHA.

PRIOR TO STARTING DEMOLITION WORK, THE CONTRACTOR SHALL MAKE AN INSPECTION OF ALL SURROUNDING IMPROVEMENTS TO REMAIN, TO DETERMINE AND RECORD THEIR EXISTING PHYSICAL CONDITION.

SHORING AND BRACING: THE CONTRACTOR SHALL FURNISH ALL SHORING, BRACING, AND INCIDENTALS NECESSARY AND REQUIRED FOR THE PROPER SUPPORT AND SAFETY OF ALL MEMBERS AFFECTED BY DEMOLITION WORK.

WHERE DEMOLITION WOULD AFFECT THE STRUCTURAL INTEGRITY OF THE REMAINING STRUCTURE, THE CONTRACTOR SHALL PROVIDE TEMPORARY SUPPORTS. THE CONTRACTOR SHALL NOTIFY THE ENGINEER OF ANY FIELD CONDITION WHICH WOULD PRESENT A HAZARDOUS CONDITION TO THE STRUCTURE BEFORE PROCEEDING.

PROTECTION: PROPER PRECAUTIONS SHALL BE TAKEN AT ALL TIMES TO PROTECT VEHICULAR AND PEDESTRIAN TRAFFIC FROM ANY DAMAGE OR INJURY WHICH MAY BE CAUSED, EITHER DIRECTLY OR INDIRECTLY, BY THE WORK INCLUDED ON THESE DRAWINGS. SUCH PRECAUTIONS SHALL INCLUDE THE ERECTION AND MAINTENANCE OF FENCES, BARRICADES, RAILINGS, GUARDS, SIGNS, COVERINGS, LIGHTS, AND OTHER PRECAUTIONS AS MAY BE REQUIRED. IF AT ANY TIME, IN THE OPINION OF THE OWNER OR THE OWNER'S REPRESENTATIVE, PROPER PRECAUTIONS ARE NOT BEING TAKEN TO SECURE THIS PROTECTION, THE CONTRACTOR SHALL AT NO ADDITIONAL COST TO THE OWNER, INSTALL AND MAINTAIN SUCH ADDITIONAL PROTECTION AS MAY BE DIRECTED BY THE OWNER.

POLLUTION CONTROLS: USE WATER SPRINKLING, TEMPORARY ENCLOSURES, AND OTHER SUITABLE METHODS TO LIMIT DUST AND DIRT RISING AND SCATTERING IN THE AIR TO LOWEST PRACTICAL LEVEL. COMPLY WITH GOVERNING REGULATIONS PERTAINING TO ENVIRONMENTAL PROTECTION.

REMOVE DEBRIS FROM THE SITE AS IT ACCUMULATES. UNLESS OTHERWISE NOTED, DO NOT STORE, SELL, BURN, OR OTHERWISE DISPOSE OF DEBRIS ON THE SITE. REMOVAL OF DEBRIS INCLUDES CLEARING OF ALL LOWER LEVELS AND SIMILAR BELOW GRADE STRUCTURES. REMOVE ALL DEBRIS IN SUCH A MANNER AS TO PREVENT SPILLAGE. KEEP ALL PAVEMENTS AND AREAS ADJACENT TO THE SITE CLEAN AND FREE FROM MUD, DIRT, AND DEBRIS AT ALL TIMES.

USE OF EXPLOSIVES: THE CONTRACTOR IS ABSOLUTELY PROHIBITED FROM USING DYNAMITE OR ANY OTHER EXPLOSIVES IN ANY OF THE WORK OR OPERATIONS SHOWN ON THESE PLANS AT THE PROJECT SITE.

DEMOLITION SHALL BE PERFORMED IN A MANNER THAT WILL NOT DAMAGE ADJOINING SURFACES INDICATED TO REMAIN. SURFACES SHALL BE PATCHED, IF REQUIRED, TO PROVIDE A SUITABLE SUBSTRATE FOR NEW CONSTRUCTION.

SPECIFIC DEMOLITION NOTES ARE NOT TO BE CONSIDERED ALL INCLUSIVE OR COMPLETE IN THEMSELVES. CONTRACTOR SHALL PROVIDE ALL DEMOLITION INCIDENTAL TO OR REQUIRED FOR CONSTRUCTION WHETHER SPECIFICALLY NOTED OR NOT.

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KEY PLAN:

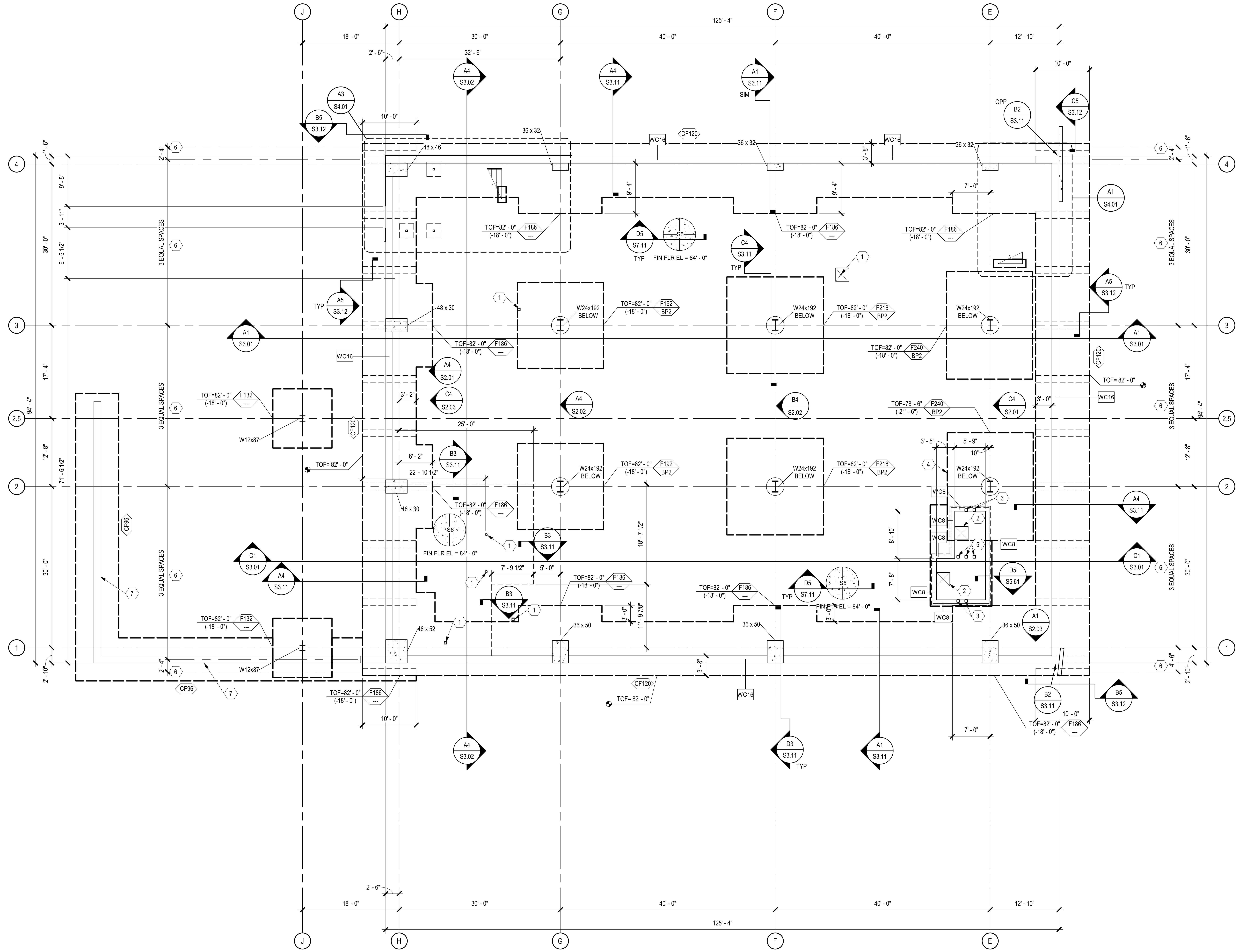
PROJECT PHASE:
BID PACKAGE 03

REVISIONS:		
#	DATE	DESCRIPTION

DATE: 03-20-19
JOB NUMBER: 17-13

SHEET NUMBER:
S0.03

GENERAL STRUCTURAL NOTES AND SPECIAL INSPECTION TABLES



A1 BASEMENT FOUNDATION PLAN - SECTOR 2

SCALE: 1/8" = 1'-0"

GENERAL SHEET NOTES

1. SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
2. REFERENCE FINISH FLOOR ELEVATION 100'-0" = MEAN SEA FINISH FLOOR ELEVATION. SEE CIVIL DRAWINGS.
3. NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE CONCRETE WALLS AND MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
4. DIMENSIONS ARE TO THE FACE OF CONCRETE UNLESS NOTED OTHERWISE.
5. PROVIDE SLAB JOINTS AT 10'-0" ON CENTER MAXIMUM. THE AREA OF THE CONTROL JOINT SHALL NOT EXCEED A 2.1 RATIO. CONTROL JOINTS SHALL BE LOCATED AT COLUMN LINES WHERE THE LAYOUT PERMITS. AT RE-ENTRANT CORNERS THAT DO NOT HAVE CONTROL JOINTS, PROVIDE 2-#4 x 3'-0" DIAGONAL TO THE RE-ENTRANT CORNER.
6. STRUCTURAL COLD FORMED METAL STUDS SHALL BE 8" WIDE UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING BY OTHERS.
7. SEE S7.00 SERIES SHEETS FOR TYPICAL FOUNDATION SECTIONS AND DETAILS.
8. SEE SHEET S6.01 FOR SCHEDULES.
9. UNDER SLAB FRENCH DRAIN SYSTEM REQUIRED BELOW THE SLAB ON GRADE IN THE BASEMENT. SEE GEOTECHNICAL REPORT SECTION 12 FOR ADDITIONAL INFORMATION.

SHEET KEYNOTE

1. FLOOR DRAIN / MOP SINK, SLOPE SLAB TO DRAIN 1/8" PER FOOT. COORDINATE EXACT SIZE AND LOCATION WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
2. ELEVATOR SUMP PIT, COORDINATE EXACT SIZE AND LOCATION WITH ELEVATOR MANUFACTURER, SEE A4 / S5.61
3. HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE B4 / S5.61
4. TOP OF FOOTING ELEVATION = 79'-6". ELEVATION SHALL BE COORDINATED WITH TOP OF ELEVATOR PIT FOOTING. CONTRACTOR TO COORDINATE.
5. HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE D4 / S5.61
6. TRANSVERSE SHEAR KEY AT LOCATIONS/SPACING SHOWN.
7. SITE RETAINING WALL. COORDINATE EXACT SIZE AND EXTENT WITH ARCHITECTURAL AND CIVIL DRAWINGS. SEE RETAINING WALL SCHEDULE A1 / S6.01

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CLIENT:

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COLLEGE OF Osteopathic Medicine AT THE CHEROKEE NATION

TAHLEQUAH, OKLAHOMA

KEY PLAN:

02 01

PROJECT PHASE:

BID PACKAGE 03

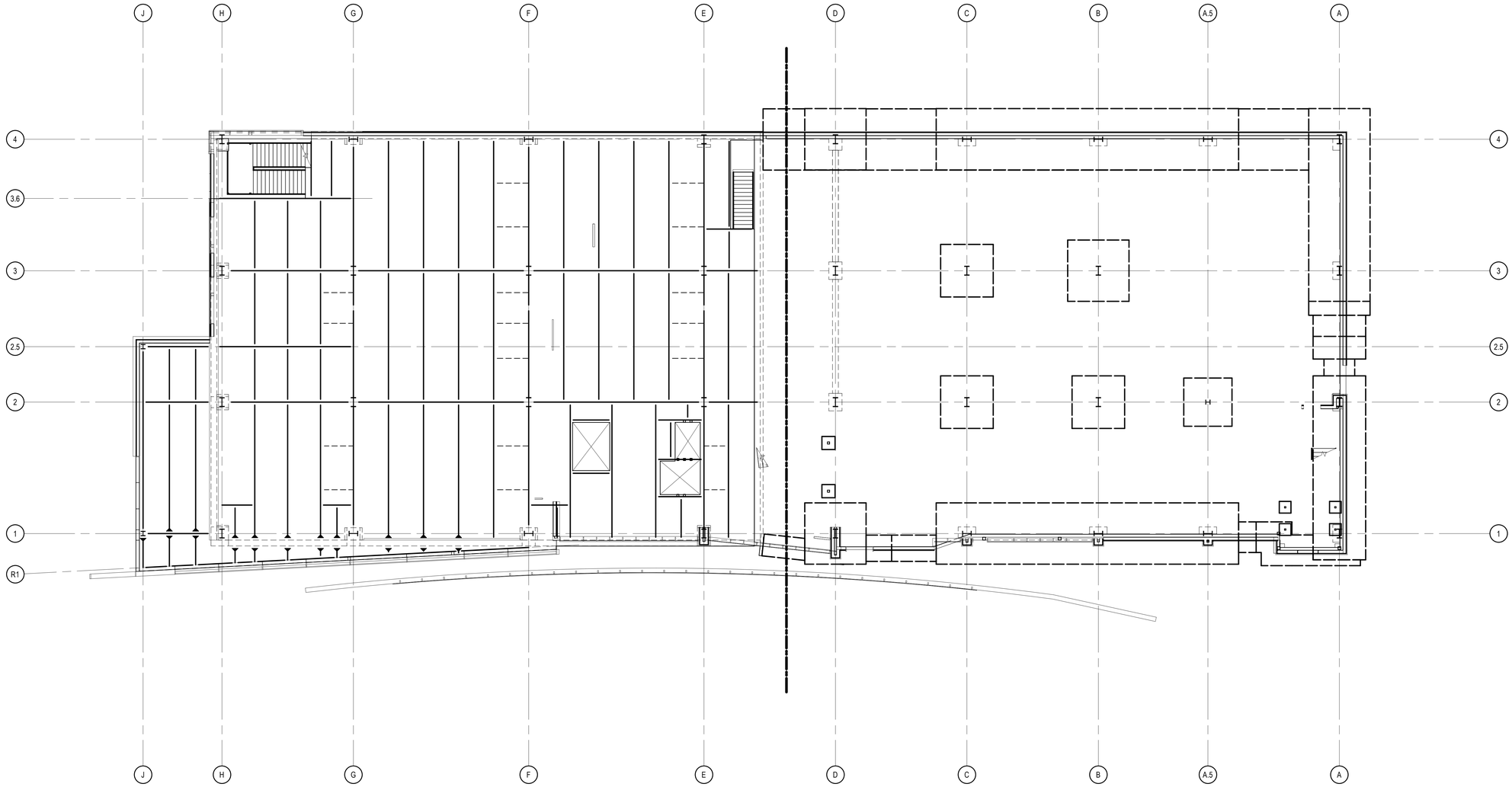
REVISIONS		
#	DATE	DESCRIPTION

DATE: 03-20-19

JOB NUMBER: 17-13

SHEET NUMBER: S1.02

BASEMENT FOUNDATION PLAN - SECTOR 2



A1 **FIRST FLOOR FRAMING PLAN - OVERALL PLAN**
SCALE: 3/32" = 1'-0"

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02 01

PROJECT PHASE:
BID PACKAGE 03

REVISIONS

#	DATE	DESCRIPTION

DATE: 03-20-19
JOB NUMBER: 17-13
SHEET NUMBER: S1.10
FIRST FLOOR FRAMING PLAN - OVERALL PLAN

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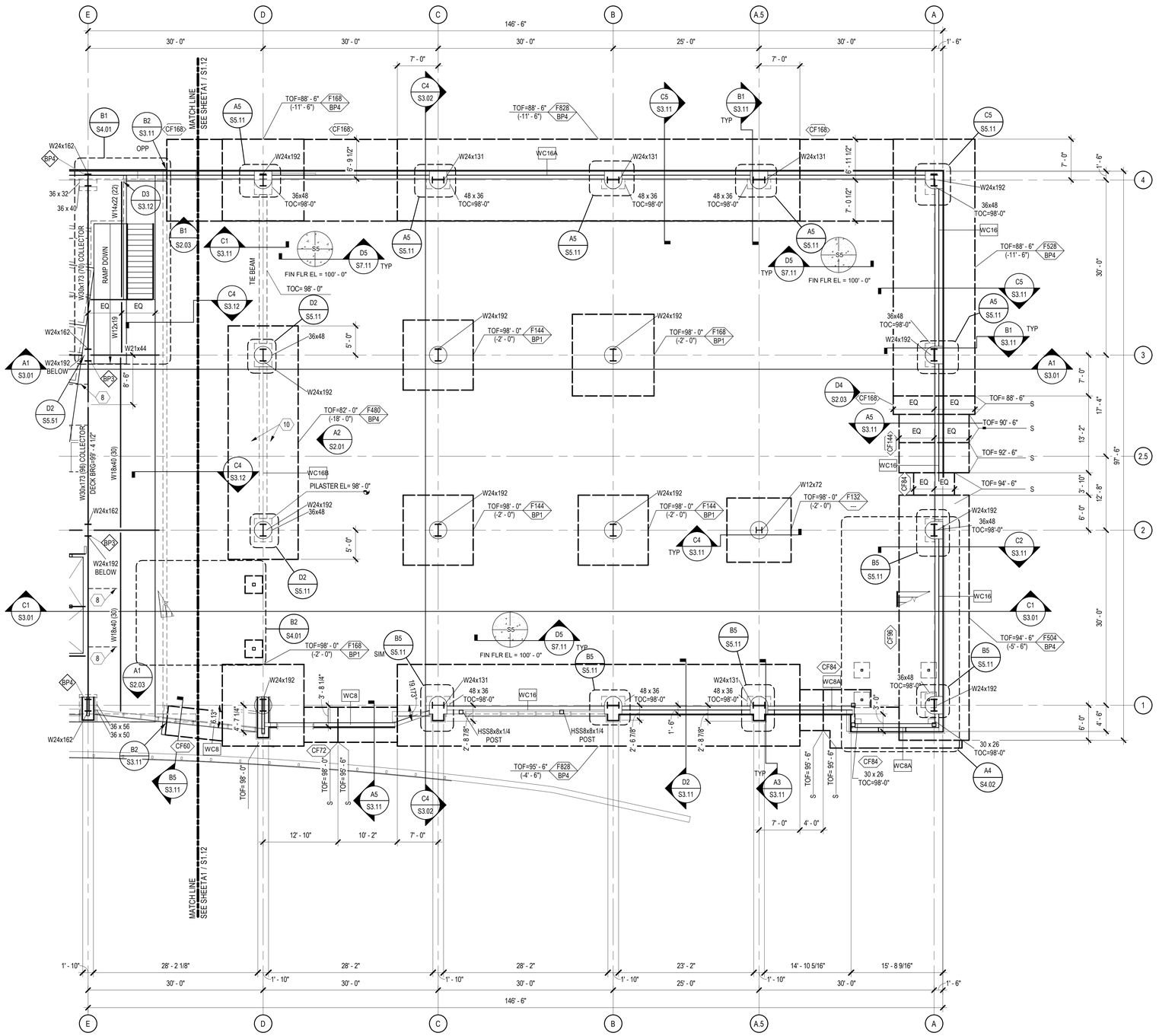

A1 FIRST FLOOR FOUNDATION AND FRAMING PLAN - SECTOR 1

SCALE: 1/8" = 1'-0"

GENERAL SHEET NOTES

1. SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
2. NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE CONCRETE WALLS AND MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
3. DIMENSIONS ARE TO THE FACE OF CONCRETE OR STUDS UNLESS NOTED OTHERWISE.
4. SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
5. BEAMS ARE SPACED EQUALLY BETWEEN GRIDS UNLESS NOTED OTHERWISE.
6. STRUCTURAL COLD FORMED METAL STUDS SHALL BE 8" WIDE UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING BY OTHERS.
7. SEE S7.00 SERIES SHEETS FOR TYPICAL DETAILS.
8. SEE SHEET S6.01 FOR SCHEDULES.
9. ALL MOMENT FRAMES LABELED ON PLAN UTILIZE SIDEPLATE PROPRIETARY MOMENT CONNECTIONS. SEE S8.00 SERIES SHEETS
10. ▶ DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
11. ■ DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE DRAWINGS.
12. DIMENSIONS SHOWN ON PLAN AS FOLLOWS ARE CONCRETE PILASTER DIMENSIONS IN INCHES: 38x36, 50x36, ETC. DIMENSIONS ARE "PLAN WIDTH" x "PLAN HEIGHT". COORDINATE PILASTER REQUIREMENTS WITH SHEET S2.03

SHEET KEYNOTE

1. HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST. COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE B4 / S5.62 , C4 / S5.62, D4 / S5.62
2. OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A5 / S5.52 FOR SUPPORT.
3. BEAM SPLICE LOCATION. SEE B4 / S5.52 FOR SPLICE DETAIL.
4. HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER. SEE A4 / S5.62, B4 / S5.62 , C4 / S5.62, D4 / S5.62 , AND C3 / S5.62
5. W8x31 OUTRIGGER.
6. TOTAL NUMBER OF CHORD REINFORCEMENT BARS AT EXTENTS SHOWN. CHORD REINFORCEMENT SHALL BE LOCATED AS INDICATED ON PLAN. PROVIDE 130% LAP SPLICES WHEN REQUIRED.
7. 3-#7 SLAB REINFORCING BARS. EXTEND BARS 130% OF A LAP SPLICE LENGTH BEYOND OPENING, OR PROVIDE STD 90 DEGREE HOOK WHERE REQUIRED.
8. BOTTOM FLANGE BRACING AT EQUAL SPACING, UNLESS NOTED OTHERWISE. SEE B1 / S5.52
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OKLAHOMA
2019

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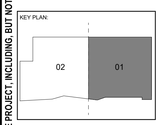
THE CHEROKEE NATION
SEAL OF
SEPT. 6, 1839

COLLEGE OF Osteopathic Medicine
AT THE CHEROKEE NATION
TAHLEQUAH, OKLAHOMA

KEY PLAN:



PROJECT PHASE:

BID PACKAGE 03

REVISIONS		
#	DATE	DESCRIPTION

DATE:	JOB NUMBER:
03-20-19	17-13

SHEET NUMBER:

S1.11

FIRST FLOOR FOUNDATION AND FRAMING PLAN - SECTOR 1

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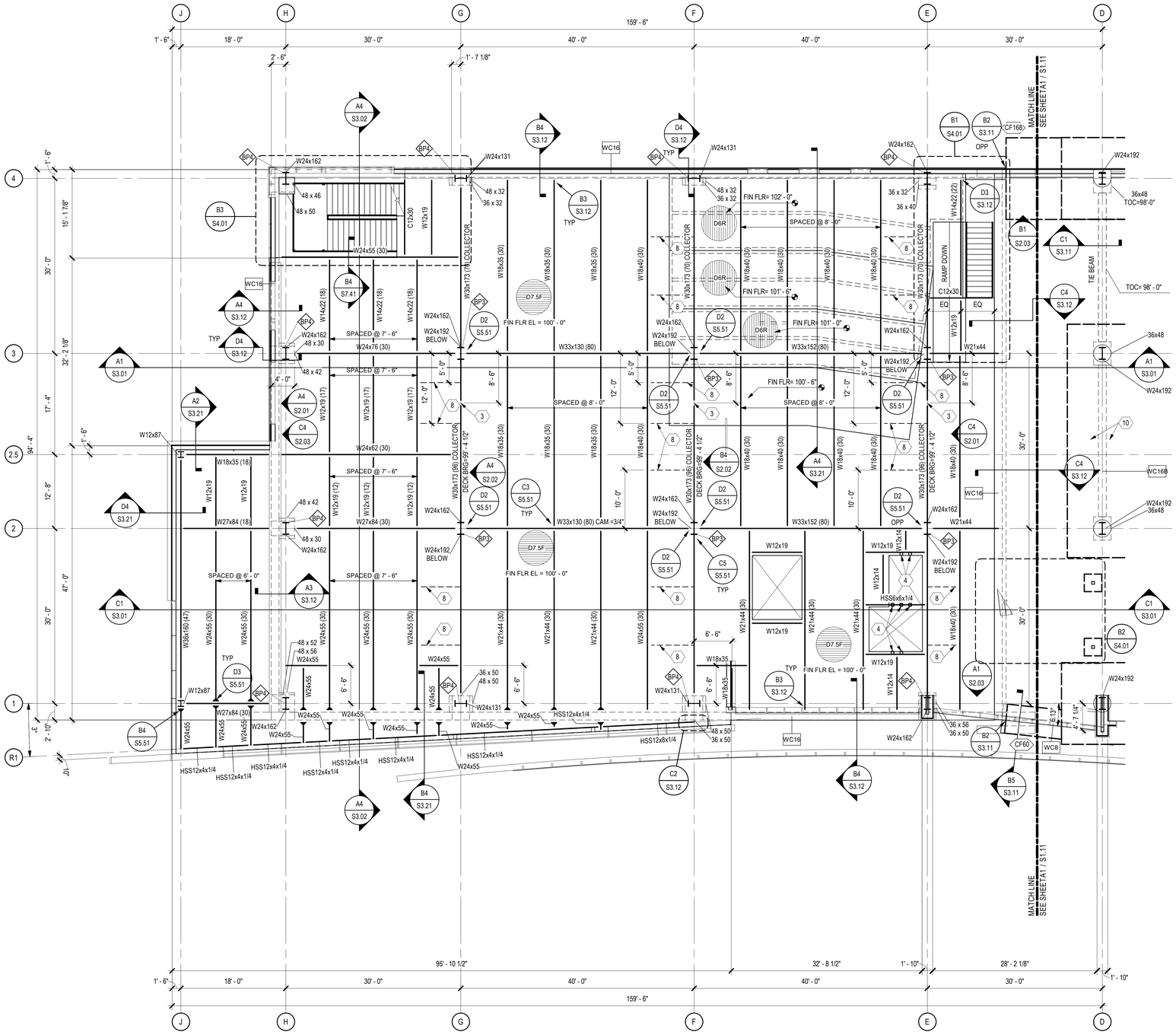

A1 FIRST FLOOR FRAMING PLAN - SECTOR 2

SCALE: 1/8" = 1'-0"

GENERAL SHEET NOTES

1. SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
2. NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE CONCRETE WALLS AND MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
3. DIMENSIONS ARE TO THE FACE OF CONCRETE OR STUDS UNLESS NOTED OTHERWISE.
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5. BEAMS ARE SPACED EQUALLY BETWEEN GRIDS UNLESS NOTED OTHERWISE.
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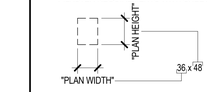

SHEET KEYNOTE

1. HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST, COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE B4 / S5.62 , C4 / S5.62, D4 / S5.62
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CLIENT:

COLLEGE OF Osteopathic Medicine AT THE CHEROKEE NATION
TAHLEQUAH, OKLAHOMA

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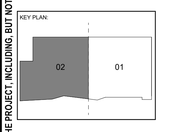
KEY PLAN:



PROJECT PHASE: BID PACKAGE 03

REVISIONS

#	DATE	DESCRIPTION

DATE: 03-20-19
JOB NUMBER: 17-13
SHEET NUMBER: S1.12

FIRST FLOOR FRAMING PLAN - SECTOR 2

A1 FIRST FLOOR FRAMING PLAN - SECTOR 2 SLAB PLAN

SCALE: 1/8" = 1'-0"

GENERAL SHEET NOTES

1. SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
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"PLAN HEIGHT"
36 x 48
"PLAN WIDTH"

SHEET KEYNOTE

1. HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST, COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE B4 / S5.62 , C4 / S5.62 , D4 / S5.62
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3. BEAM SPLICE LOCATION. SEE B4 / S5.52 FOR SPLICE DETAIL.
4. HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE A4 / S5.62, B4 / S5.62 , C4 / S5.62 , D4 / S5.62 , AND C3 / S5.62
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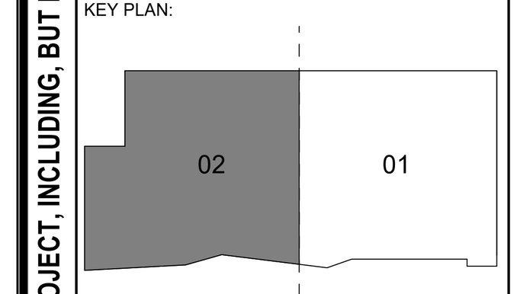
KEY PLAN:



PROJECT PHASE:

BID PACKAGE 03

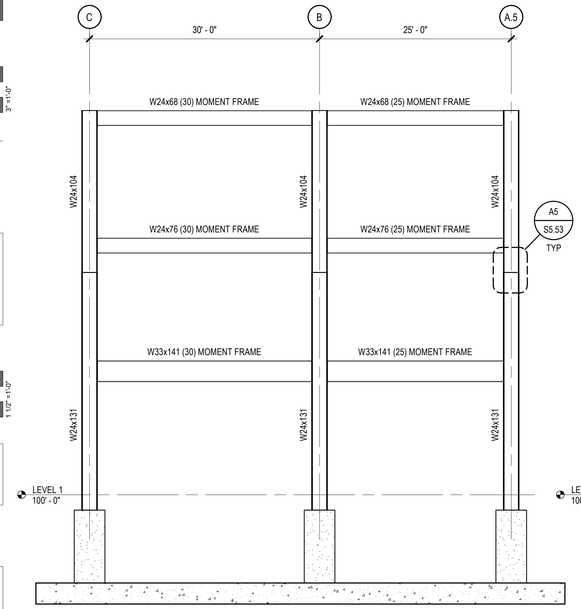
REVISIONS		
#	DATE	DESCRIPTION

DATE: 03-20-19

JOB NUMBER: 17-13

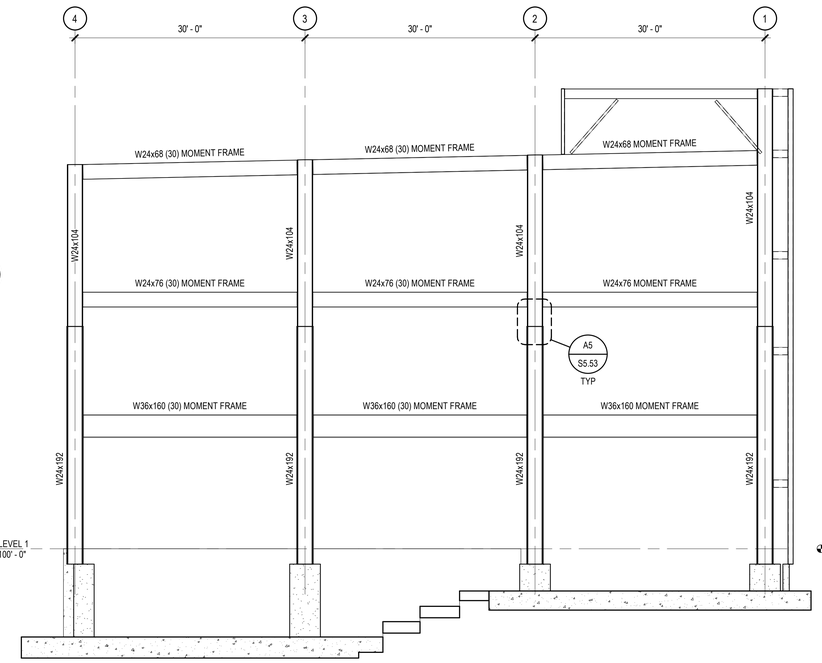
SHEET NUMBER: S1.13

FIRST FLOOR SLAB REINFORCING PLAN - SECTOR 2



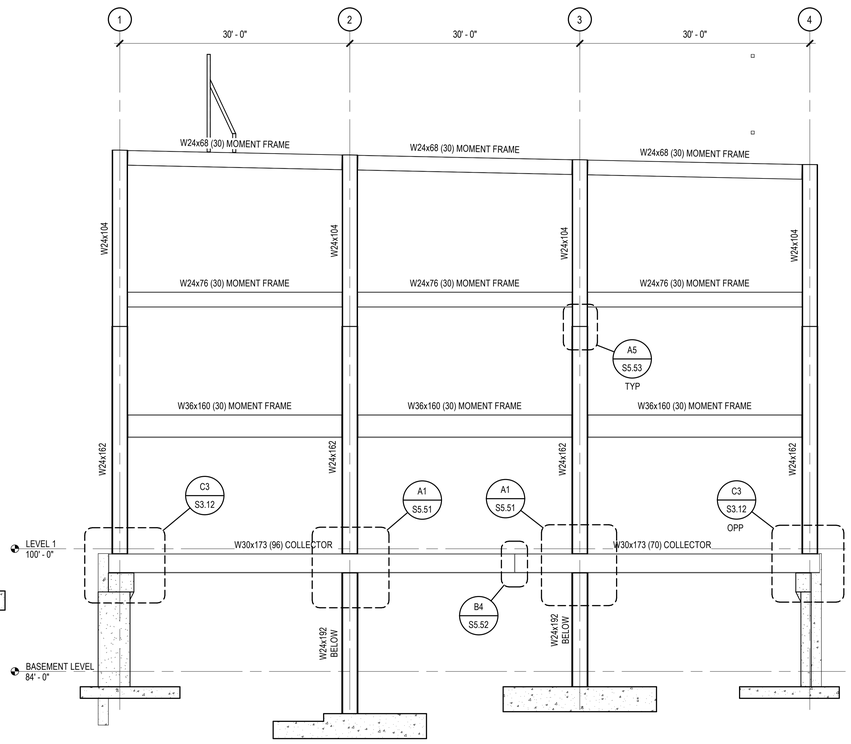
C1 MOMENT FRAME ELEVATION

SCALE: 1/8" = 1'-0"



C2 MOMENT FRAME ELEVATION

SCALE: 1/8" = 1'-0"



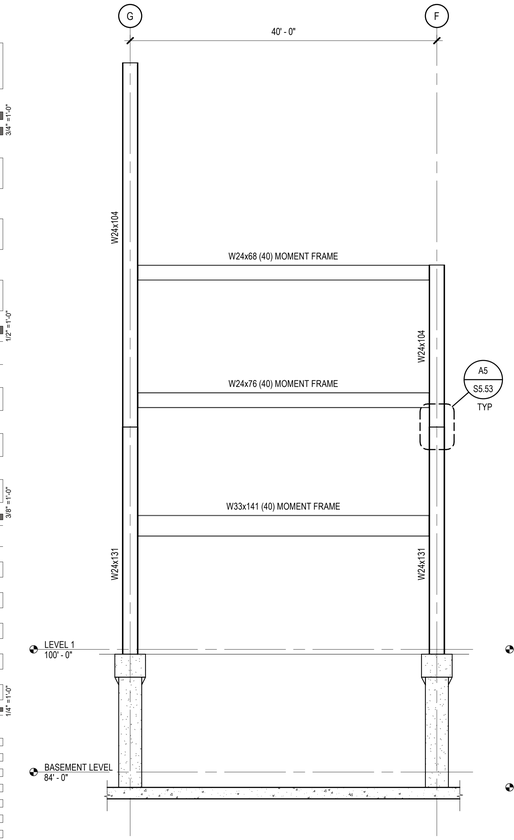
C4 MOMENT FRAME ELEVATION

SCALE: 1/8" = 1'-0"

SIDEPLATE CONNECTION NOTES

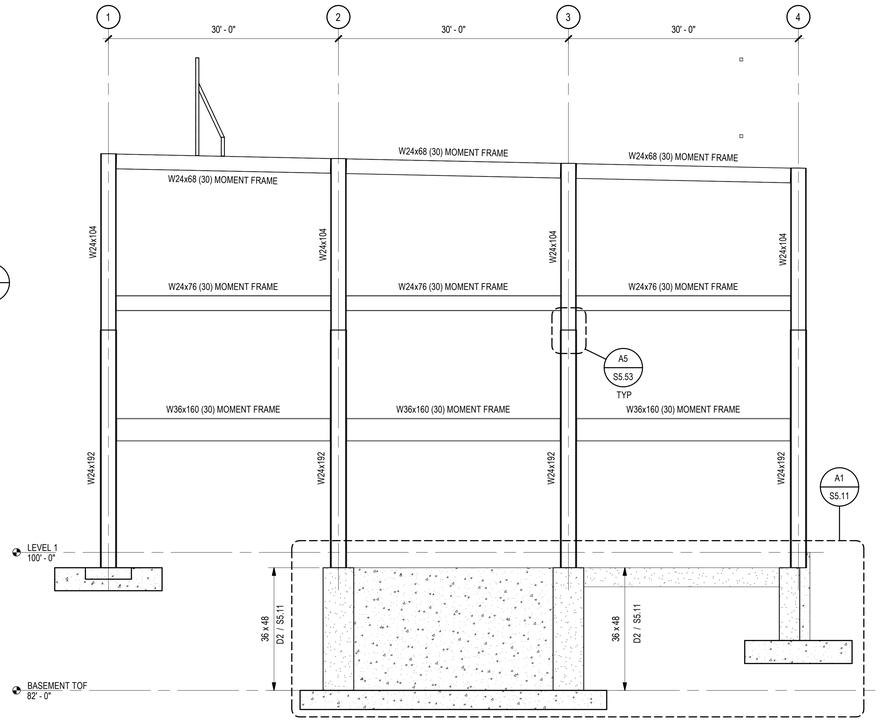
AXX INDICATES SIDEPLATE CONNECTION PER SHEETS X/XX - X/XX.

MX INDICATES MISCELLANEOUS SIDEPLATE DETAIL PER SHEETS X/XX - X/XX.



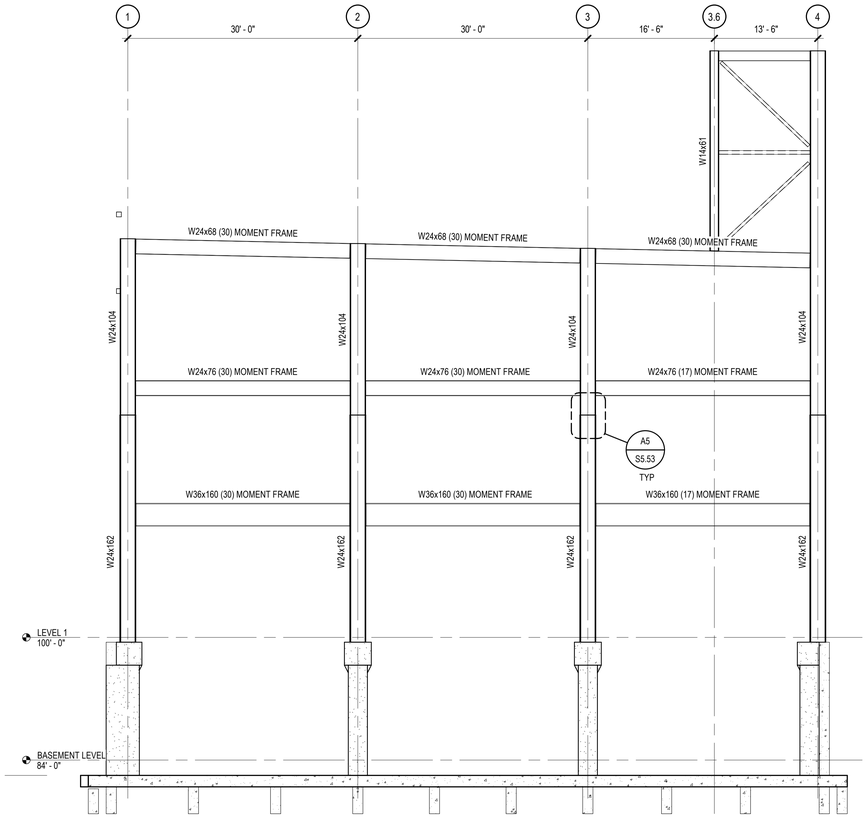
A1 MOMENT FRAME ELEVATION

SCALE: 1/8" = 1'-0"



A2 MOMENT FRAME ELEVATION

SCALE: 1/8" = 1'-0"



A4 MOMENT FRAME ELEVATION

SCALE: 1/8" = 1'-0"

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KEY PLAN:

PROJECT PHASE:
BID PACKAGE 03

REVISIONS:		
#	DATE	DESCRIPTION

DATE: 03-20-19
JOB NUMBER: 17-13

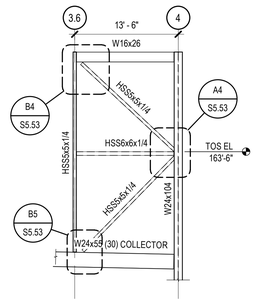
SHEET NUMBER:
S2.01

MOMENT FRAME ELEVATIONS

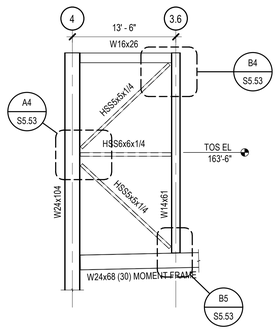
SIDEPLATE CONNECTION NOTES

AXX INDICATES SIDEPLATE CONNECTION PER SHEETS X/XX - X/XX.

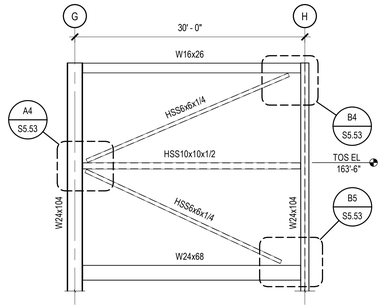
(MX) INDICATES MISCELLANEOUS SIDEPLATE DETAIL PER SHEETS X/XX - X/XX.



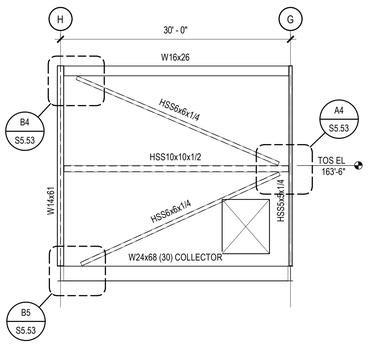
D2 BRACED FRAME ELEVATION
SCALE: 1/8" = 1'-0"



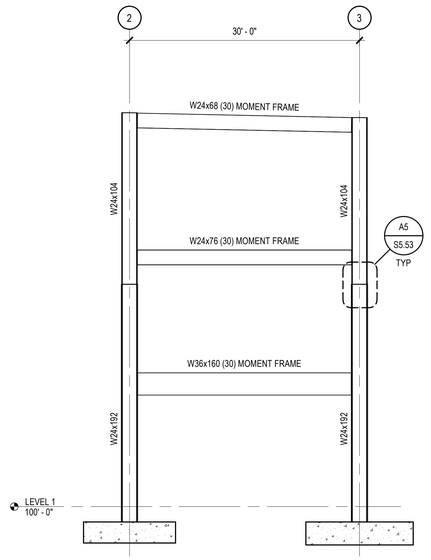
C2 BRACED FRAME ELEVATION
SCALE: 1/8" = 1'-0"



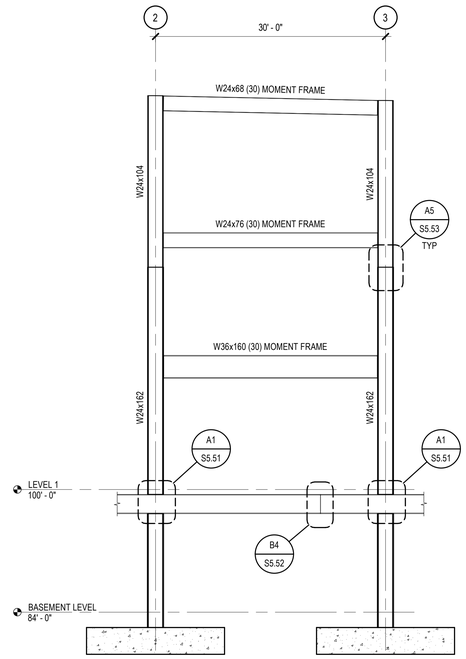
B2 BRACED FRAME ELEVATION
SCALE: 1/8" = 1'-0"



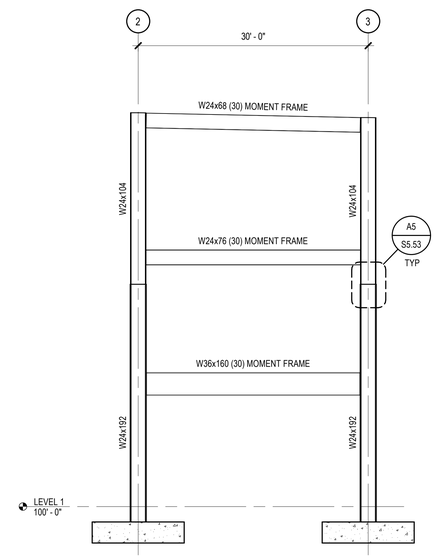
A2 BRACED FRAME ELEVATION
SCALE: 1/8" = 1'-0"



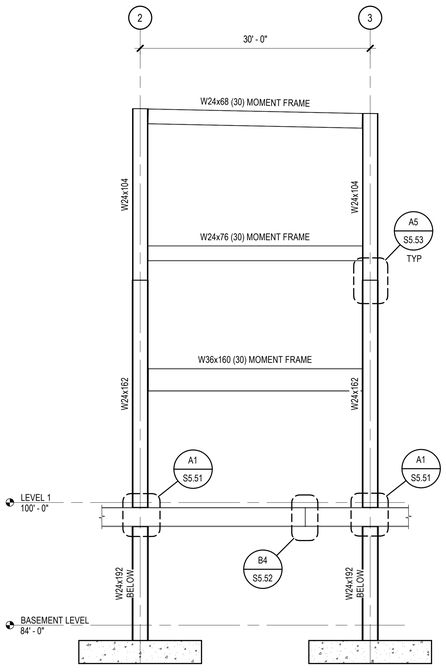
B3 MOMENT FRAME ELEVATION
SCALE: 1/8" = 1'-0"



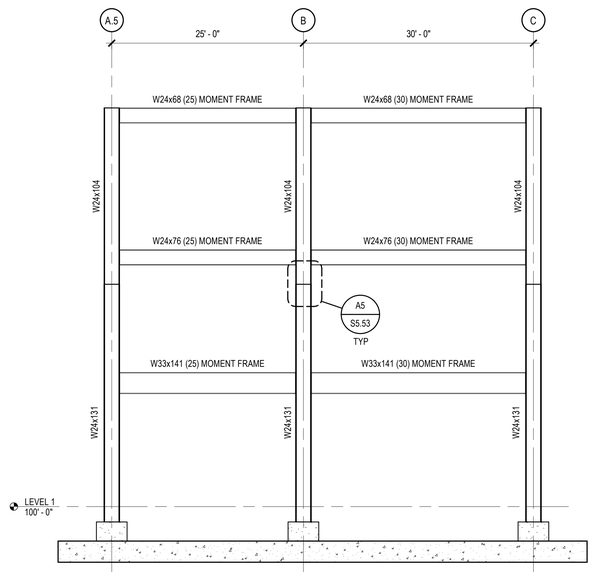
B4 MOMENT FRAME ELEVATION
SCALE: 1/8" = 1'-0"



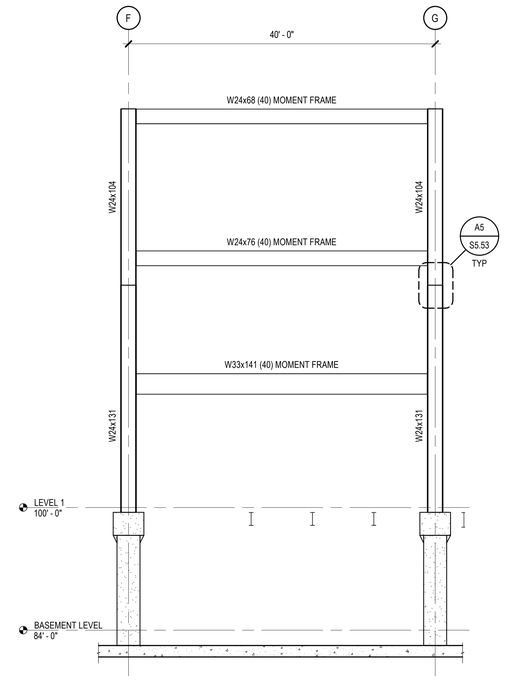
A3 MOMENT FRAME ELEVATION
SCALE: 1/8" = 1'-0"



A4 MOMENT FRAME ELEVATION
SCALE: 1/8" = 1'-0"



B5 MOMENT FRAME ELEVATION
SCALE: 1/8" = 1'-0"



A5 MOMENT FRAME ELEVATION
SCALE: 1/8" = 1'-0"

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KEY PLAN:

PROJECT PHASE:
BID PACKAGE 03

REVISIONS

#	DATE	DESCRIPTION

DATE: 03-20-19
JOB NUMBER: 17-13

SHEET NUMBER:
S2.02

MOMENT FRAME AND BRACED FRAME ELEVATIONS

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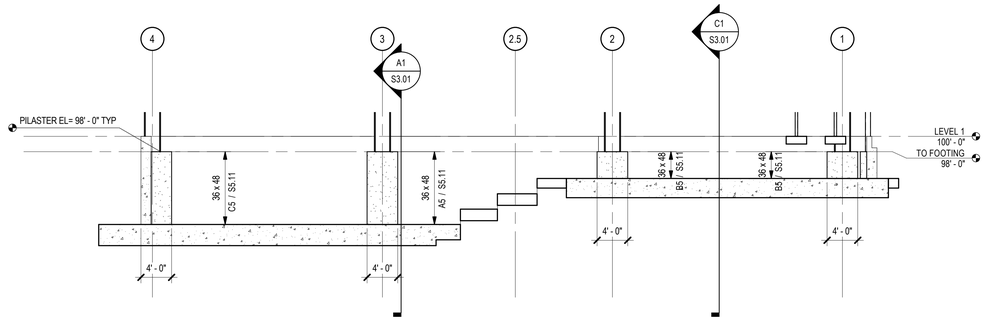
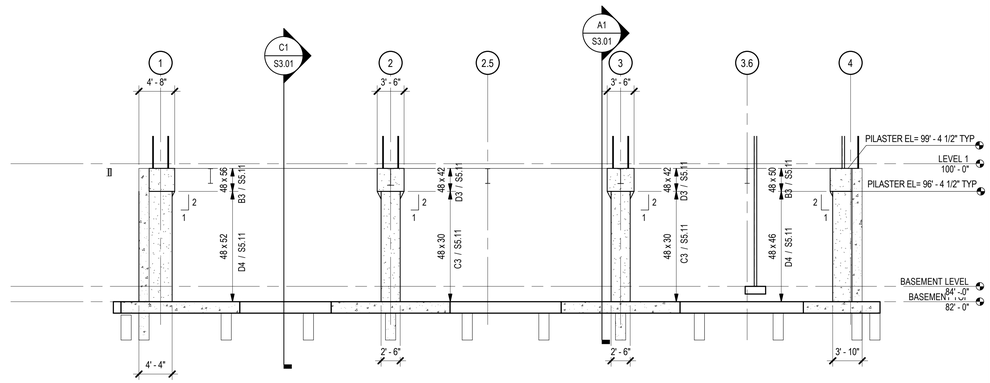


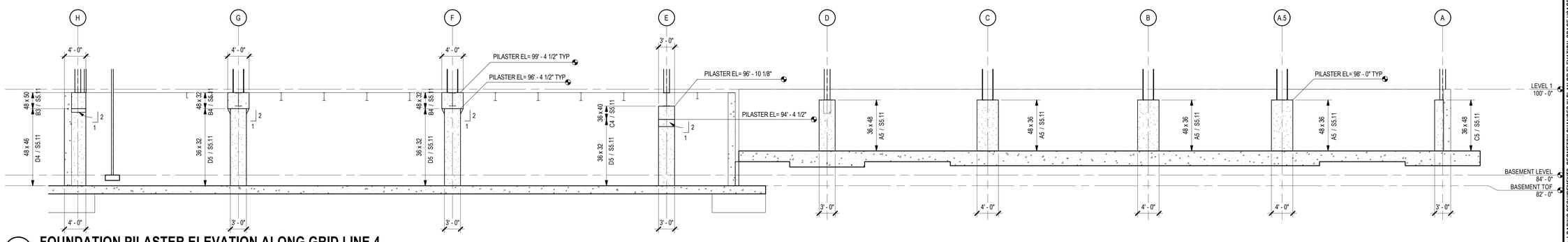
D4 FOUNDATION PILASTER ELEVATION ALONG GRID LINE A

SCALE: 1/8" = 1'-0"



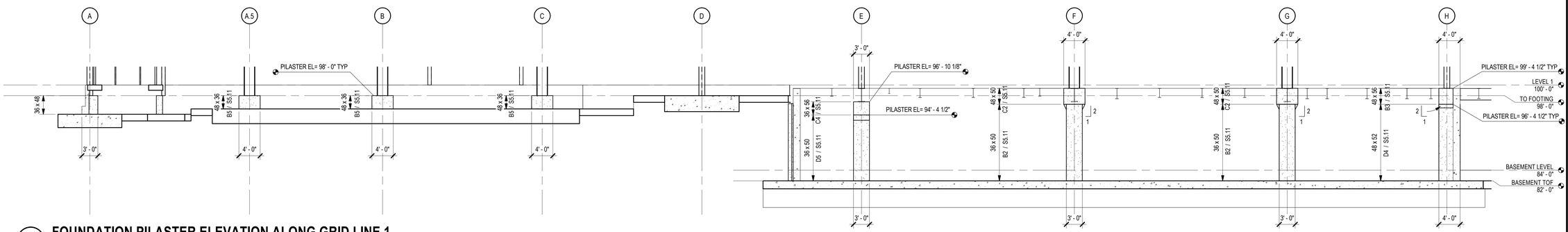
C4 FOUNDATION PILASTER ELEVATION ALONG GRID LINE H

SCALE: 1/8" = 1'-0"



B1 FOUNDATION PILASTER ELEVATION ALONG GRID LINE 4

SCALE: 1/8" = 1'-0"



A1 FOUNDATION PILASTER ELEVATION ALONG GRID LINE 1

SCALE: 1/8" = 1'-0"

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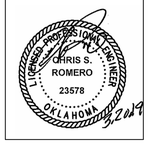
Architect, Inc.
45 South 4th Street
Fort Smith, AR 72901
479-783-2480
www.childersarchitect.com

PROFESSIONAL SEAL:



CONSULTANT LOGO:

Chavez-Grieves
consulting engineers, inc.
4700 Lincoln Road NE, Suite 102 Albuquerque, NM 87109
505-344-4080 505-343-8759 (fax)

CLIENT:

THE CHEROKEE NATION
SEPT. 6, 1839

College of Osteopathic Medicine
AT THE CHEROKEE NATION
TAHLEQUAH, OKLAHOMA

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KEY PLAN:

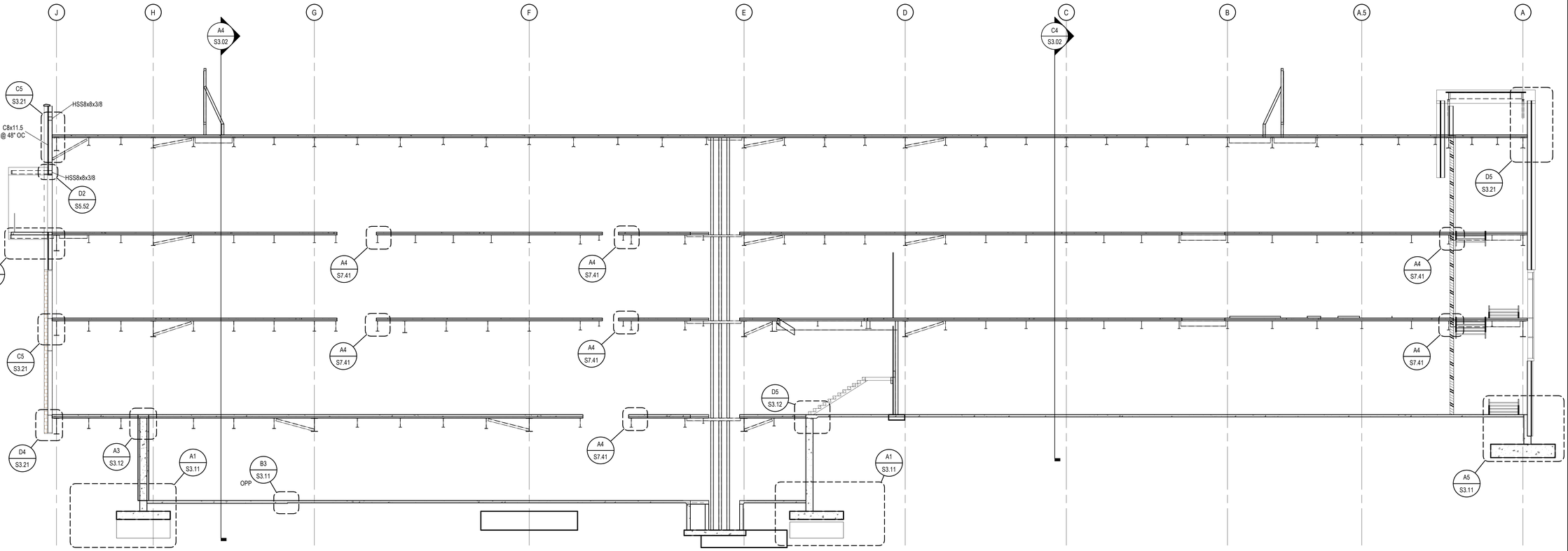
PROJECT PHASE:
BID PACKAGE 03

REVISIONS:		
#	DATE	DESCRIPTION

DATE: 03-20-19
JOB NUMBER: 17-13

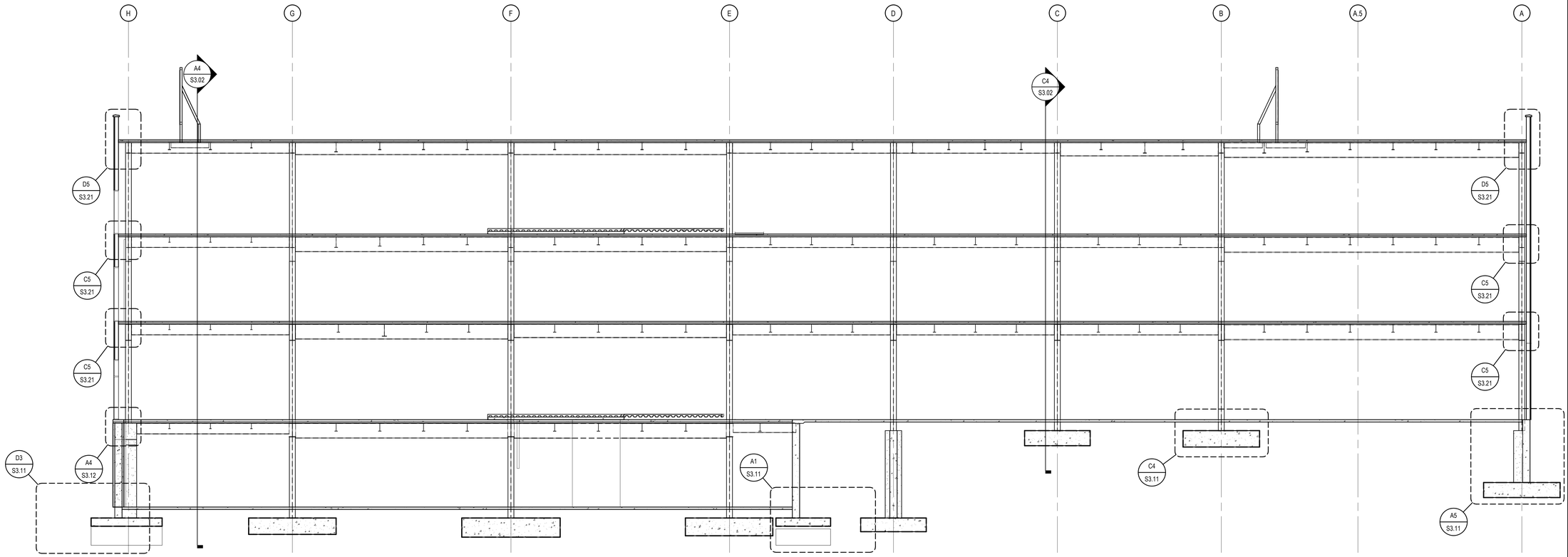
SHEET NUMBER:
S2.03

FOUNDATION PILASTER ELEVATIONS



C1 BUILDING SECTION

SCALE: 1/8" = 1'-0"



A1 BUILDING SECTION

SCALE: 1/8" = 1'-0"

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PROFESSIONAL SEAL:

LICENSED PROFESSIONAL ENGINEER
CHRIS S. ROMERO
23578
OKLAHOMA
2019

CONSULTANT LOGO:

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CLIENT:

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AT THE CHEROKEE NATION
TAHLEQUAH, OKLAHOMA

KEY PLAN:

PROJECT PHASE:
BID PACKAGE 03

REVISIONS		
#	DATE	DESCRIPTION

DATE: 03-20-19
JOB NUMBER: 17-13
SHEET NUMBER: S3.01

BUILDING SECTIONS

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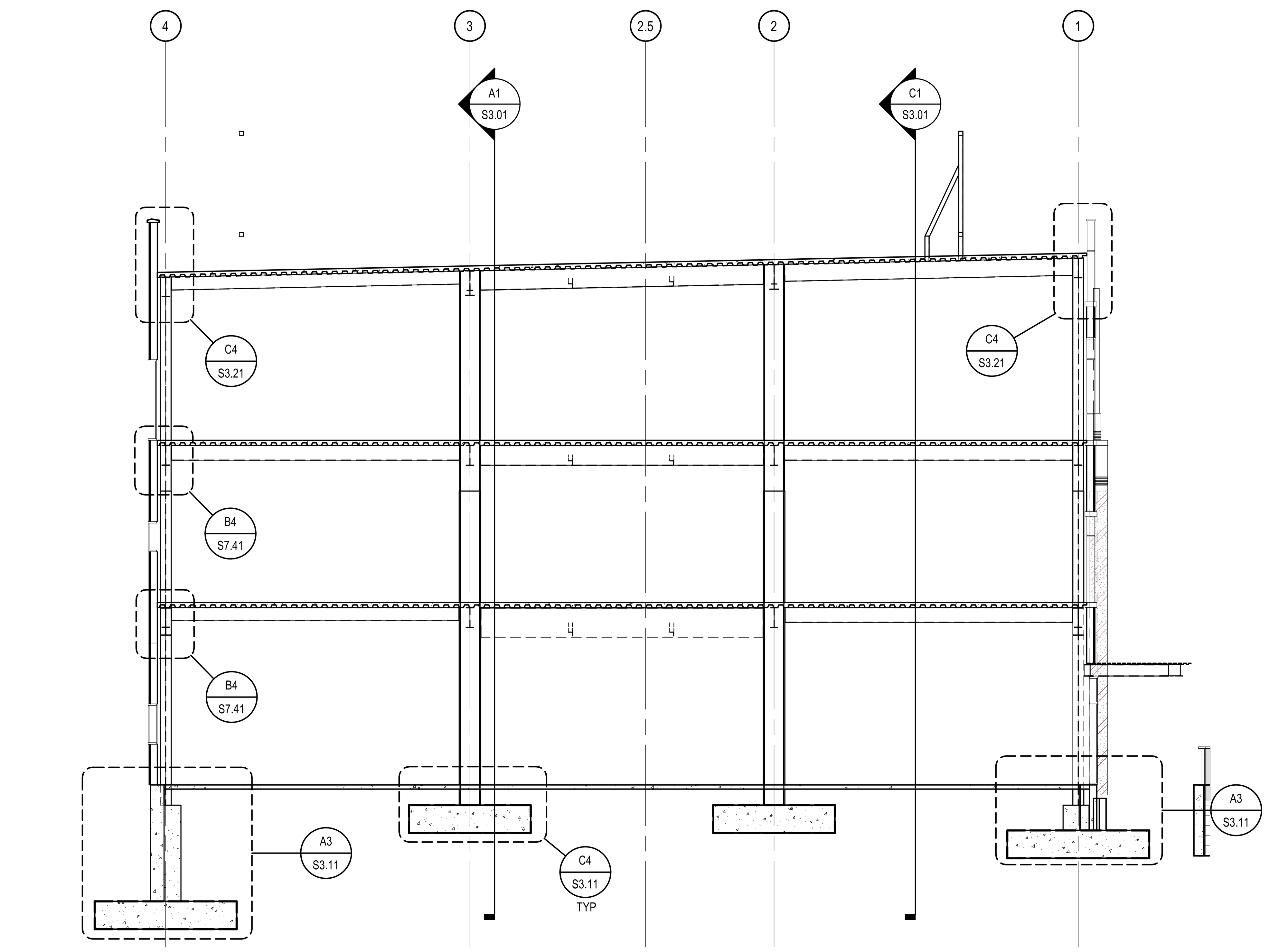

C4 **BUILDING SECTION**
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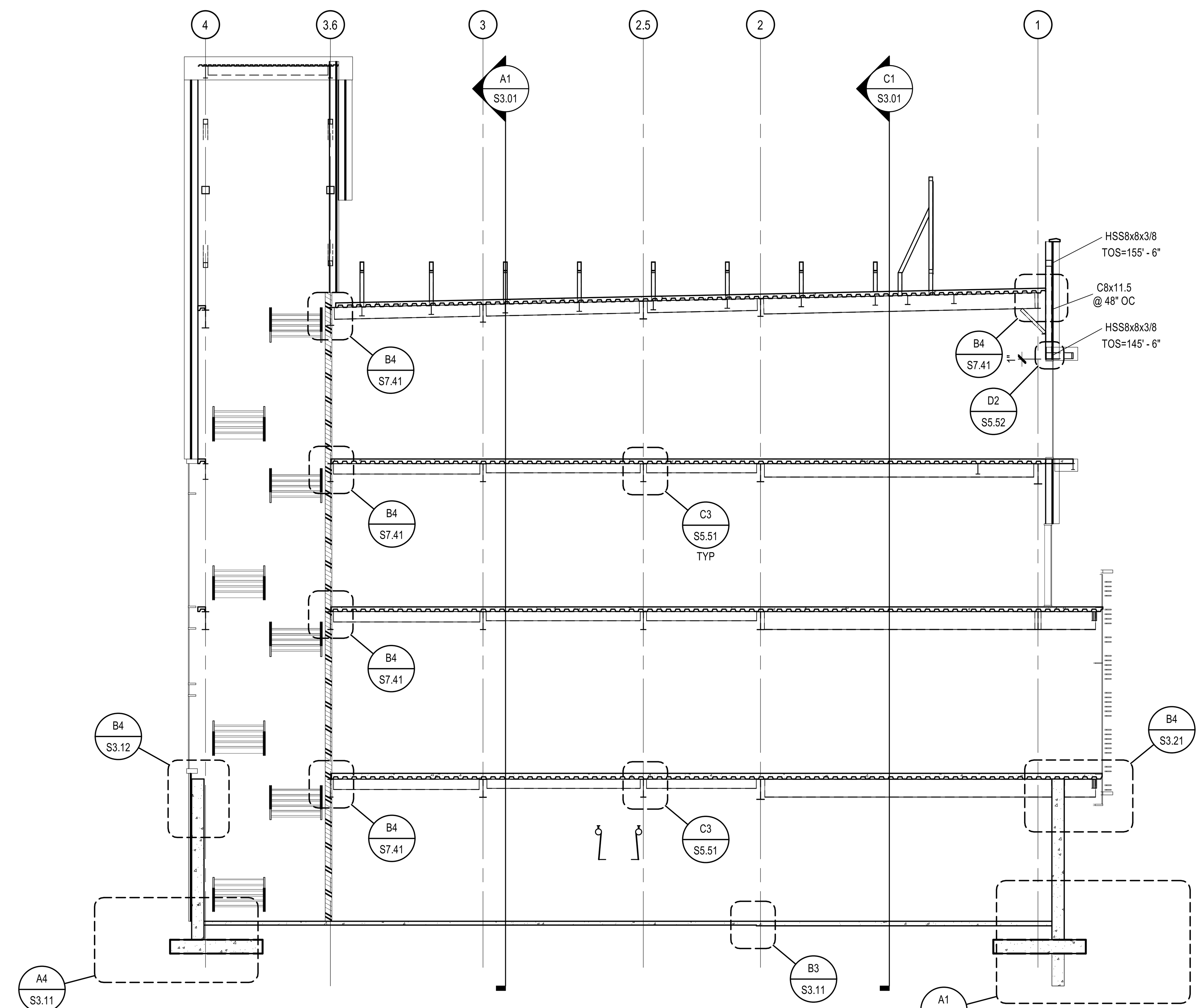

A4 **BUILDING SECTION**
SCALE: 1/8" = 1'-0"

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TAHLEQUAH, OKLAHOMA

OKLAHOMA STATE

KEY PLAN:

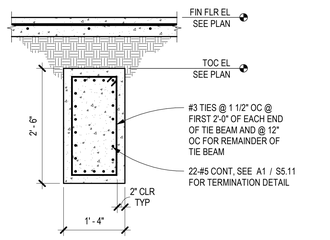
PROJECT PHASE:
BID PACKAGE 03

REVISIONS		
#	DATE	DESCRIPTION

DATE:	JOB NUMBER:
03-20-19	17-13

SHEET NUMBER:
S3.02

BUILDING SECTIONS



C1 TIE BEAM SECTION
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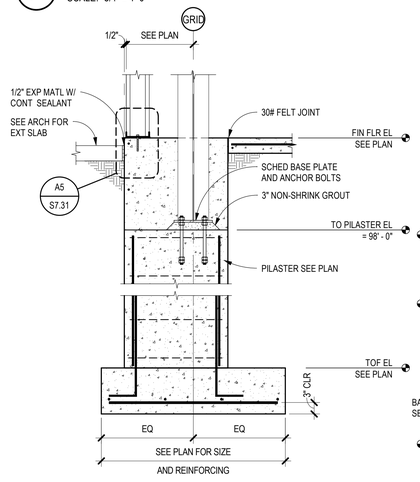
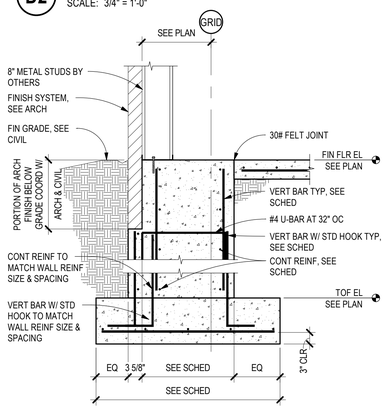


B1 FOUNDATION SECTION
SCALE: 3/4" = 1'-0"

A1 BASEMENT WALL SECTION
SCALE: 3/4" = 1'-0"



C2 PERIMETER FOUNDATION SECTION
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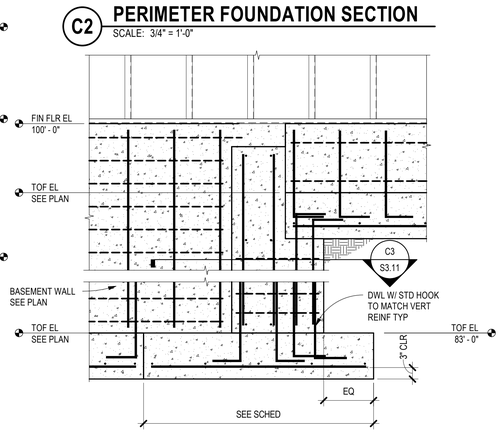
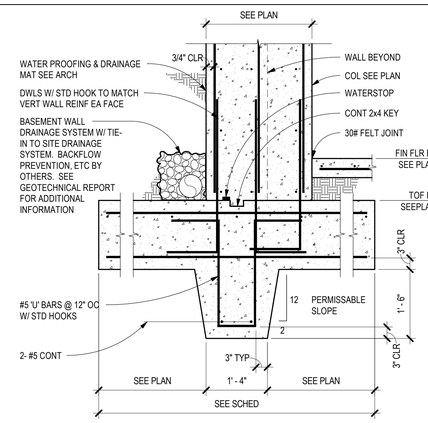


B2 FOUNDATION SECTION
SCALE: 3/4" = 1'-0"

A2 CONC COLUMN SECTION
SCALE: 3/4" = 1'-0"



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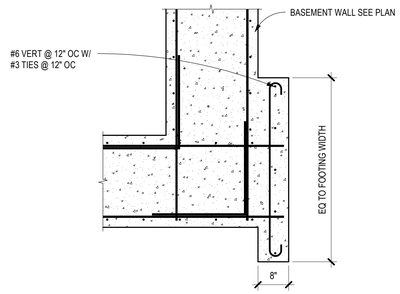
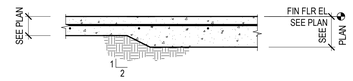
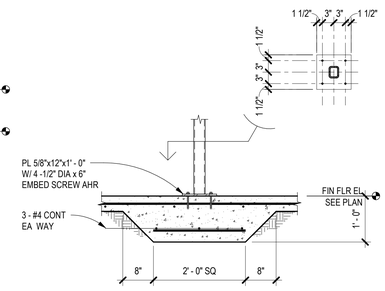


C3 FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



B3 SLAB THICKNESS TRANSITION
SCALE: 3/4" = 1'-0"



D4 INT COLUMN AT THICKENED SLAB
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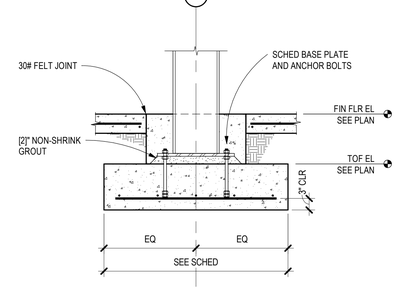

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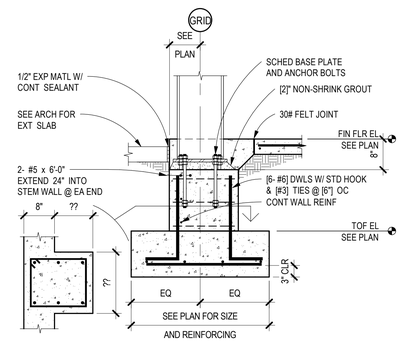

B4 PERIMETER COLUMN SECTION
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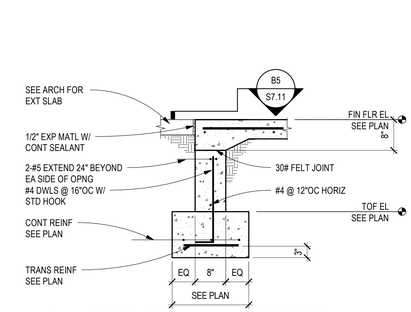
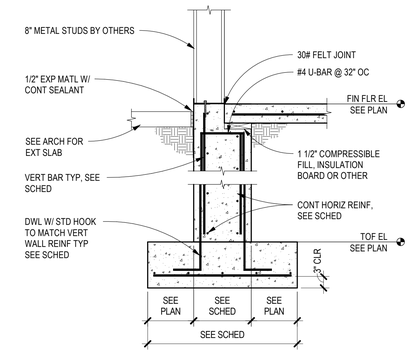
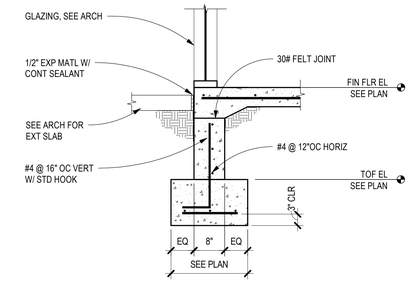


D5 CONC STEMWALL AT OPNG
SCALE: 3/4" = 1'-0"



C5 PERIMETER FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



B5 PERIMETER FOUNDATION SECTION
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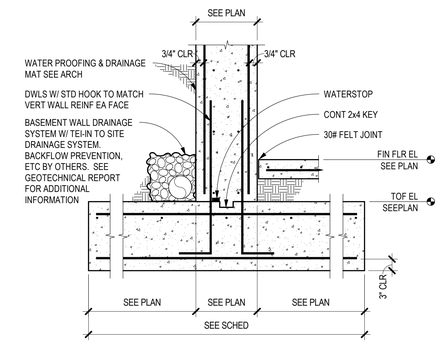
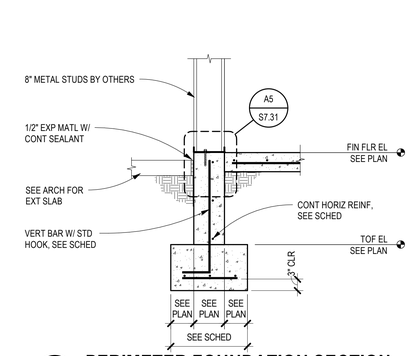


A3 FOUNDATION SECTION
SCALE: 3/4" = 1'-0"

A4 BASEMENT WALL SECTION
SCALE: 3/4" = 1'-0"



A5 PERIMETER FOUNDATION SECTION
SCALE: 3/4" = 1'-0"

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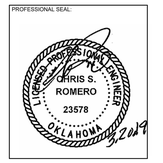

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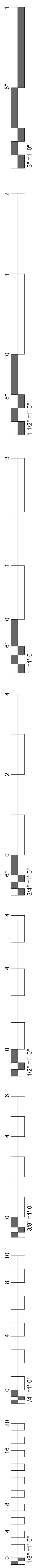
KEY PLAN:

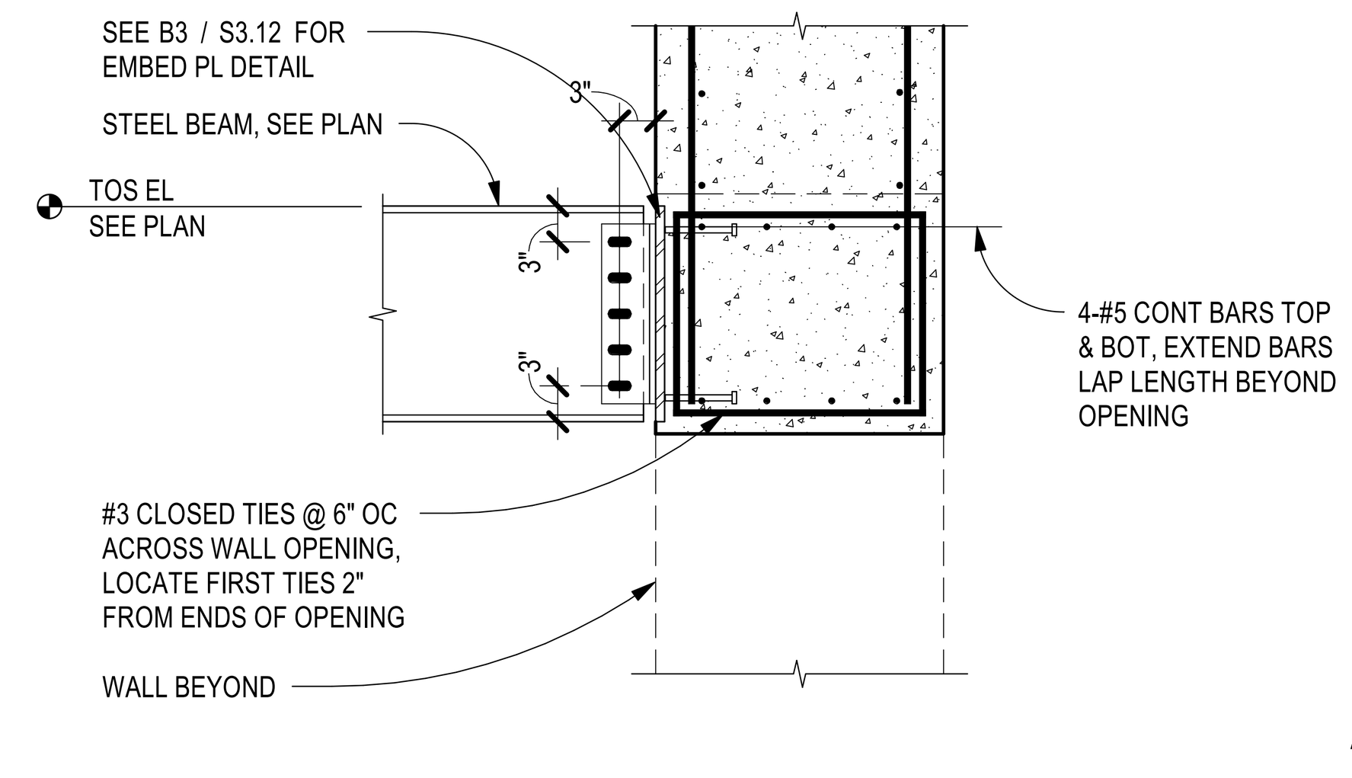
PROJECT PHASE:
BID PACKAGE 03

REVISIONS:		
#	DATE	DESCRIPTION

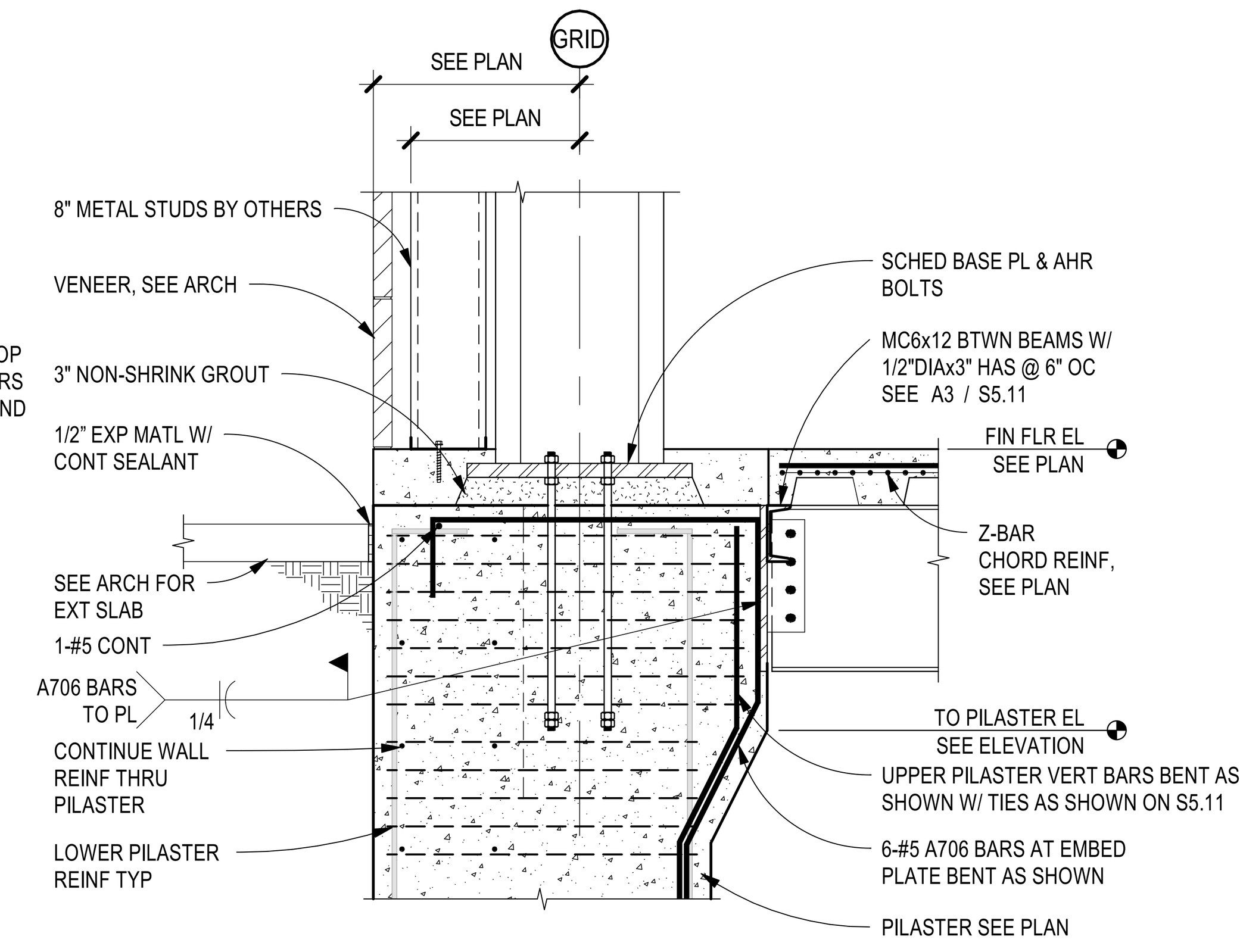
DATE: 03-20-19
JOB NUMBER: 17-13
SHEET NUMBER: S3.11

FOUNDATION SECTIONS





D3 BEAM TO CONC LINTEL DETAIL
SCALE: 3/4" = 1'-0"



D4 BASEMENT WALL SECTION
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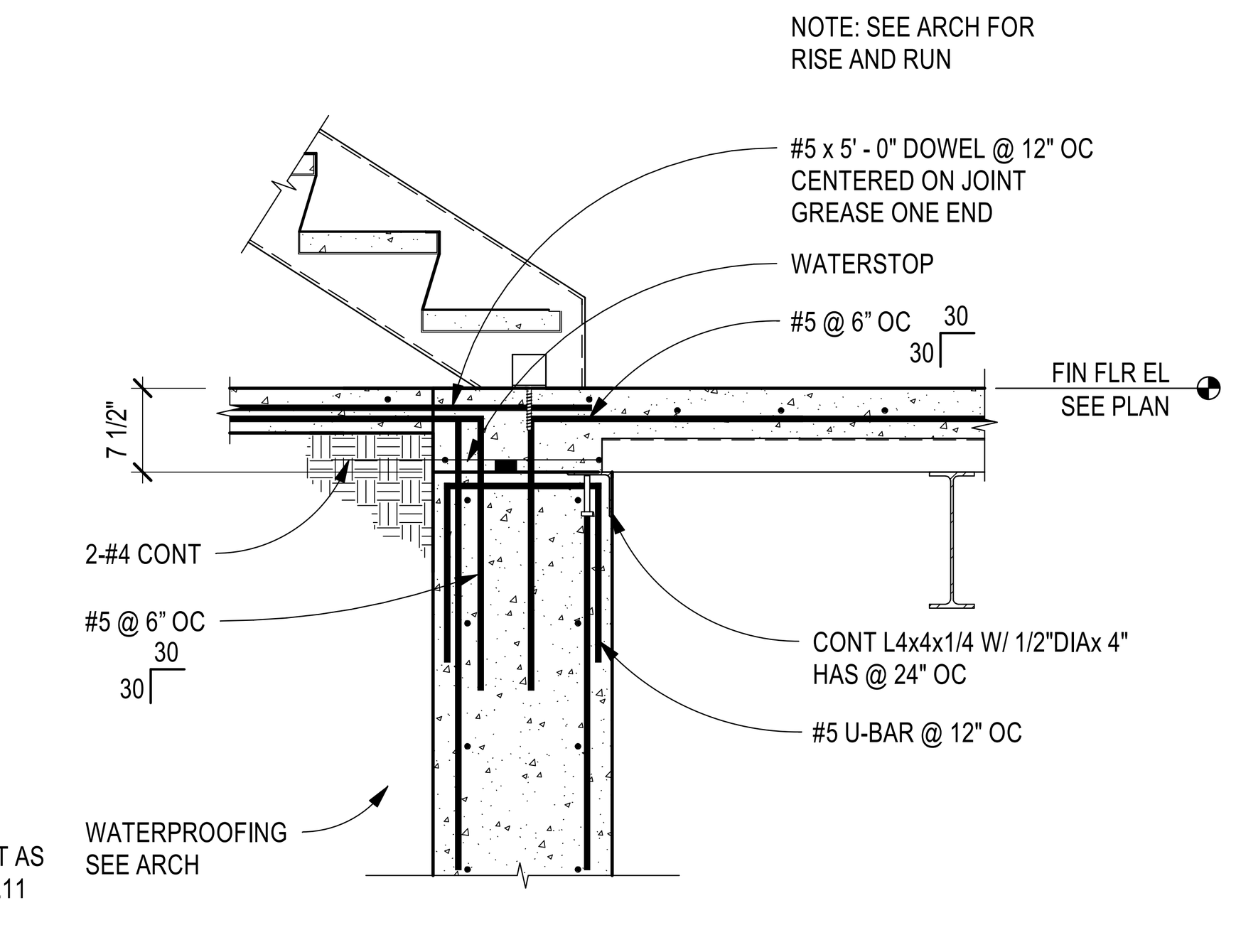

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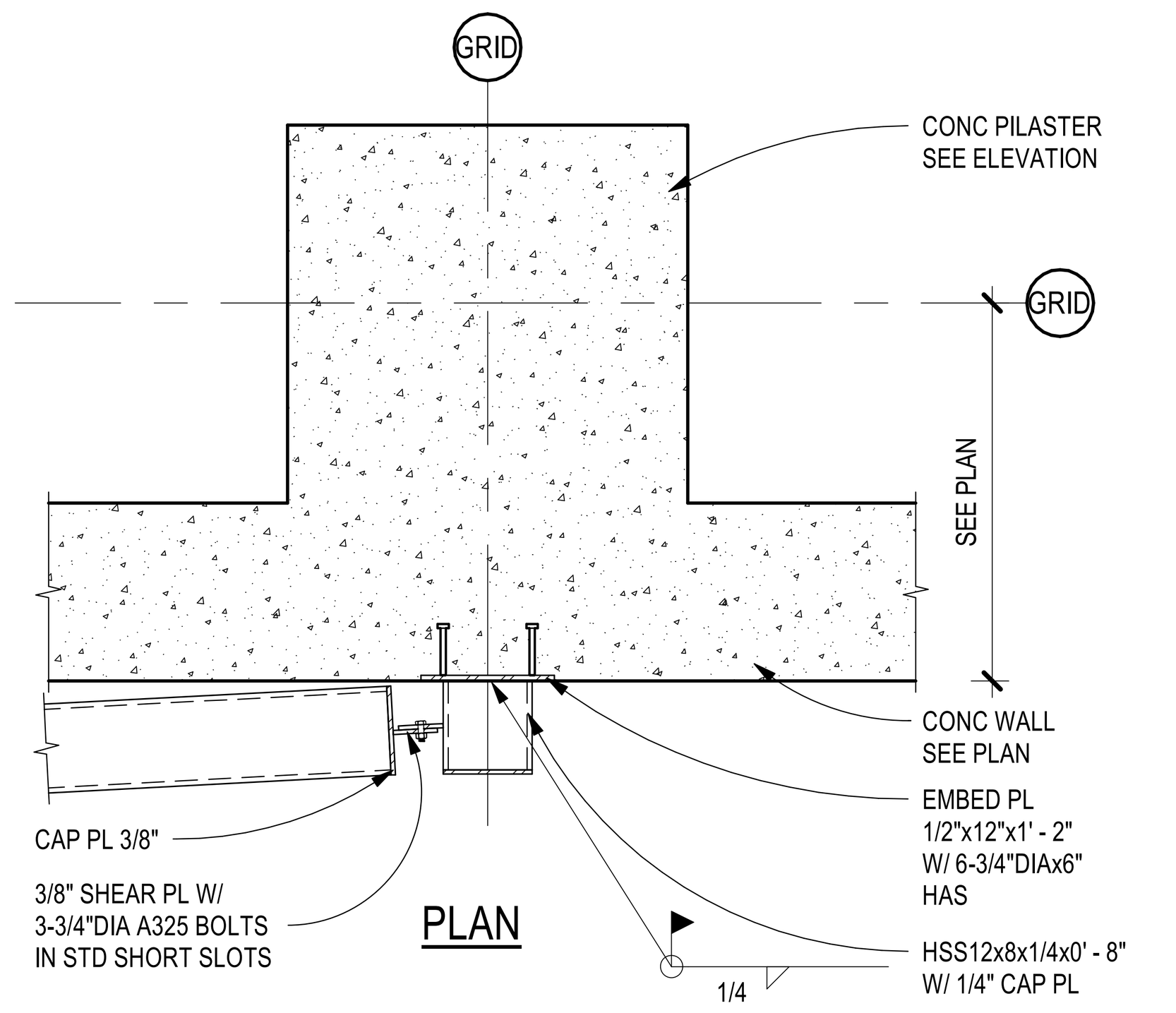
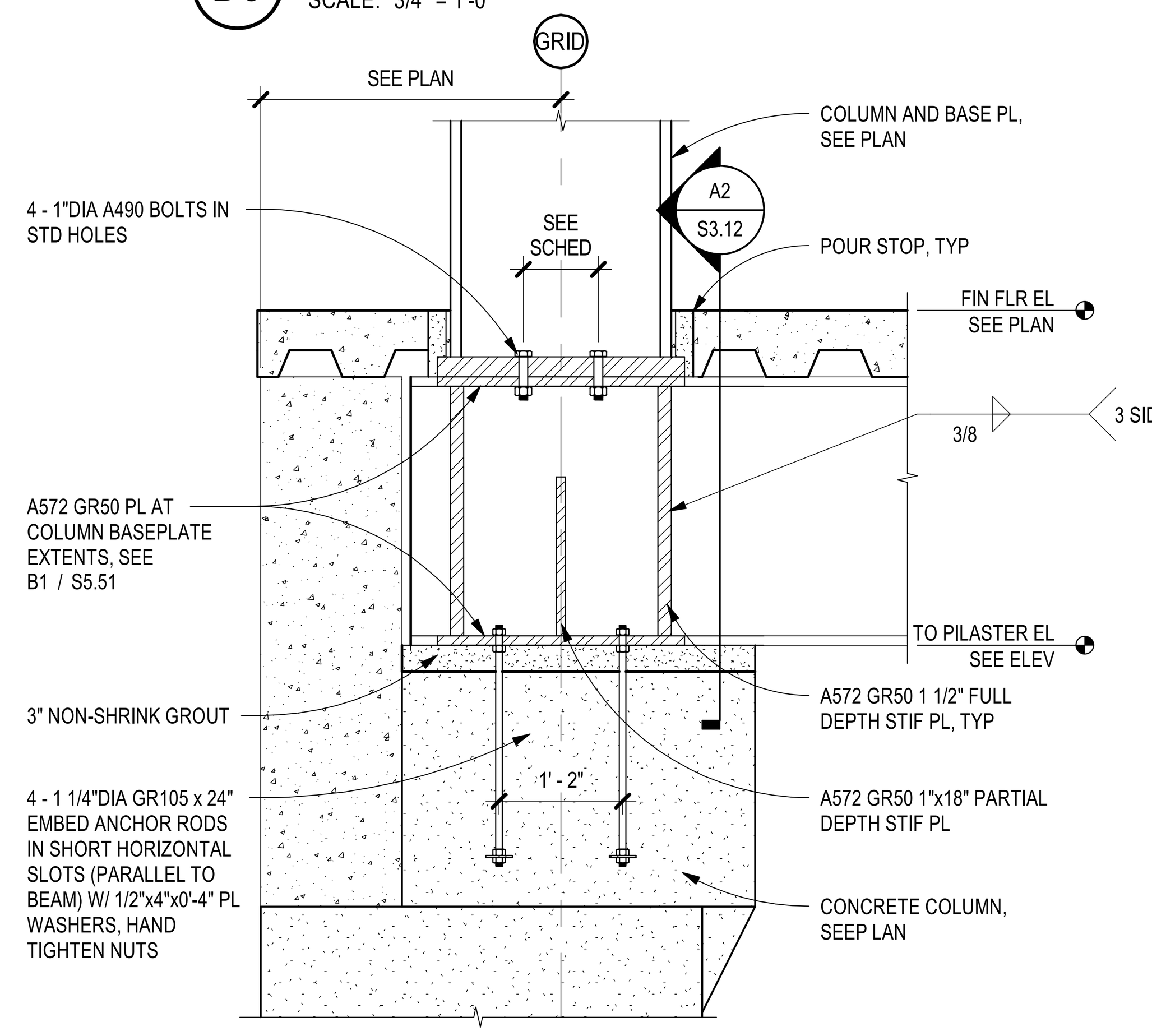


C2 HSS TO CONC WALL DETAIL
SCALE: 3/4" = 1'-0"



C3 PERIM COLUMN SECT @ E-1 & E-4 ONLY
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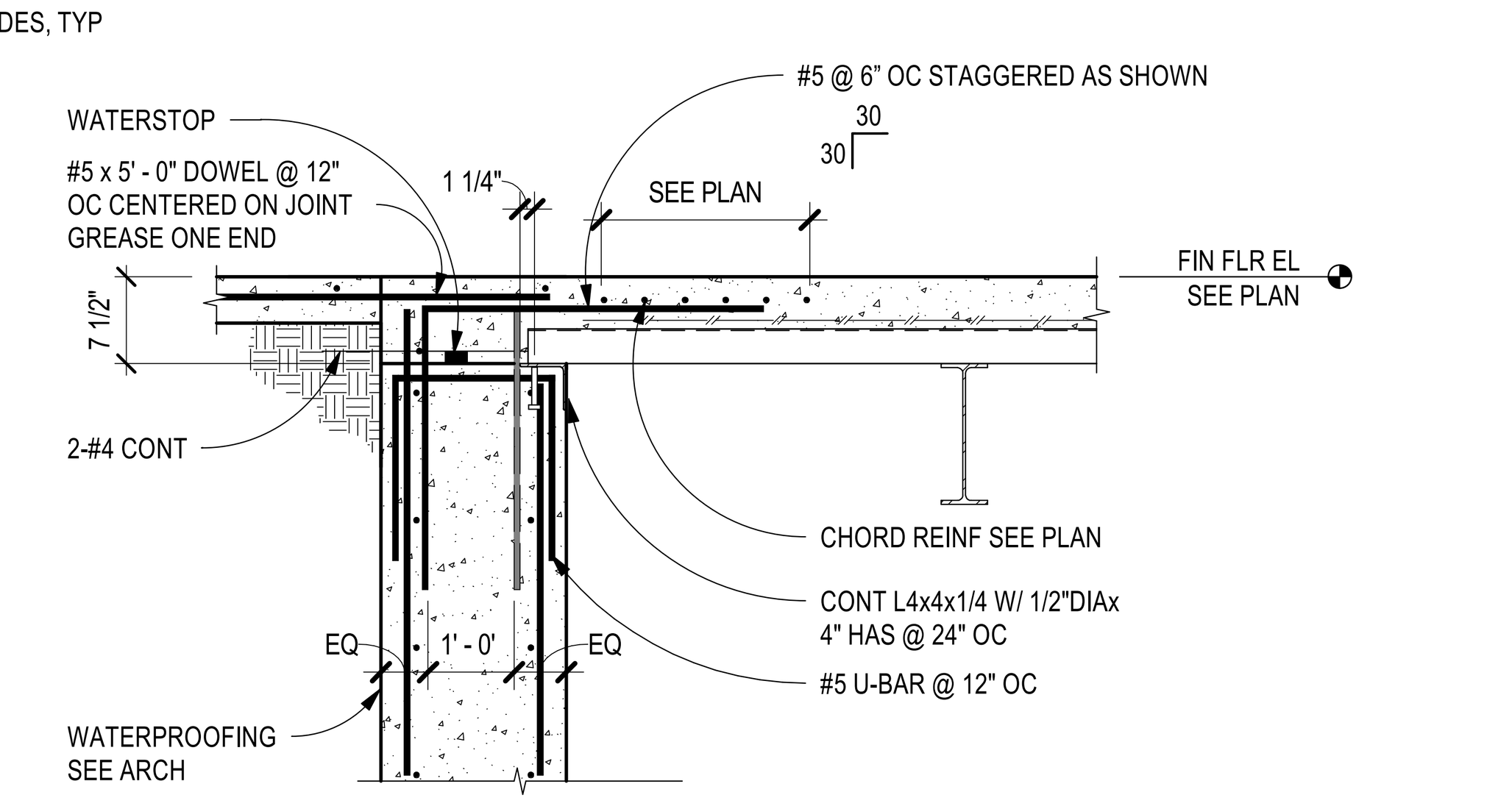

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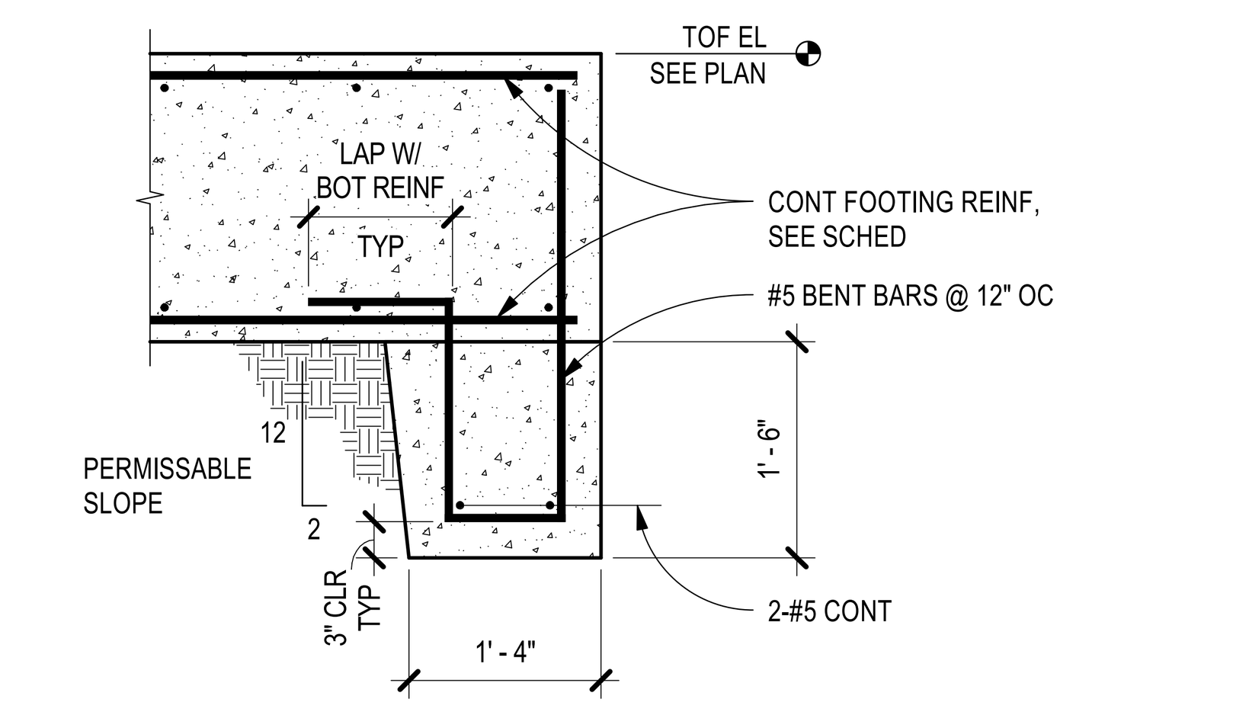

C5 TRANSVERSE FOOTING KEY SECTION
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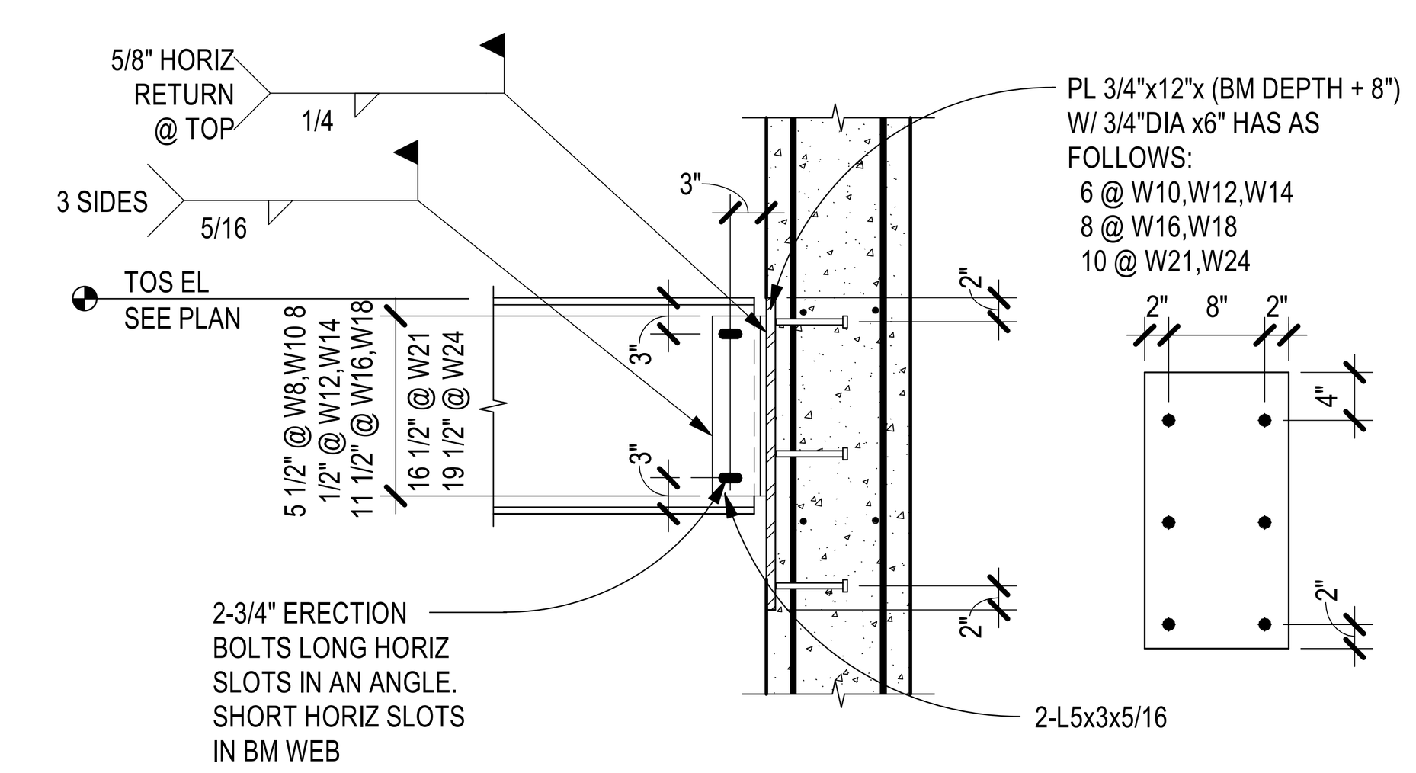
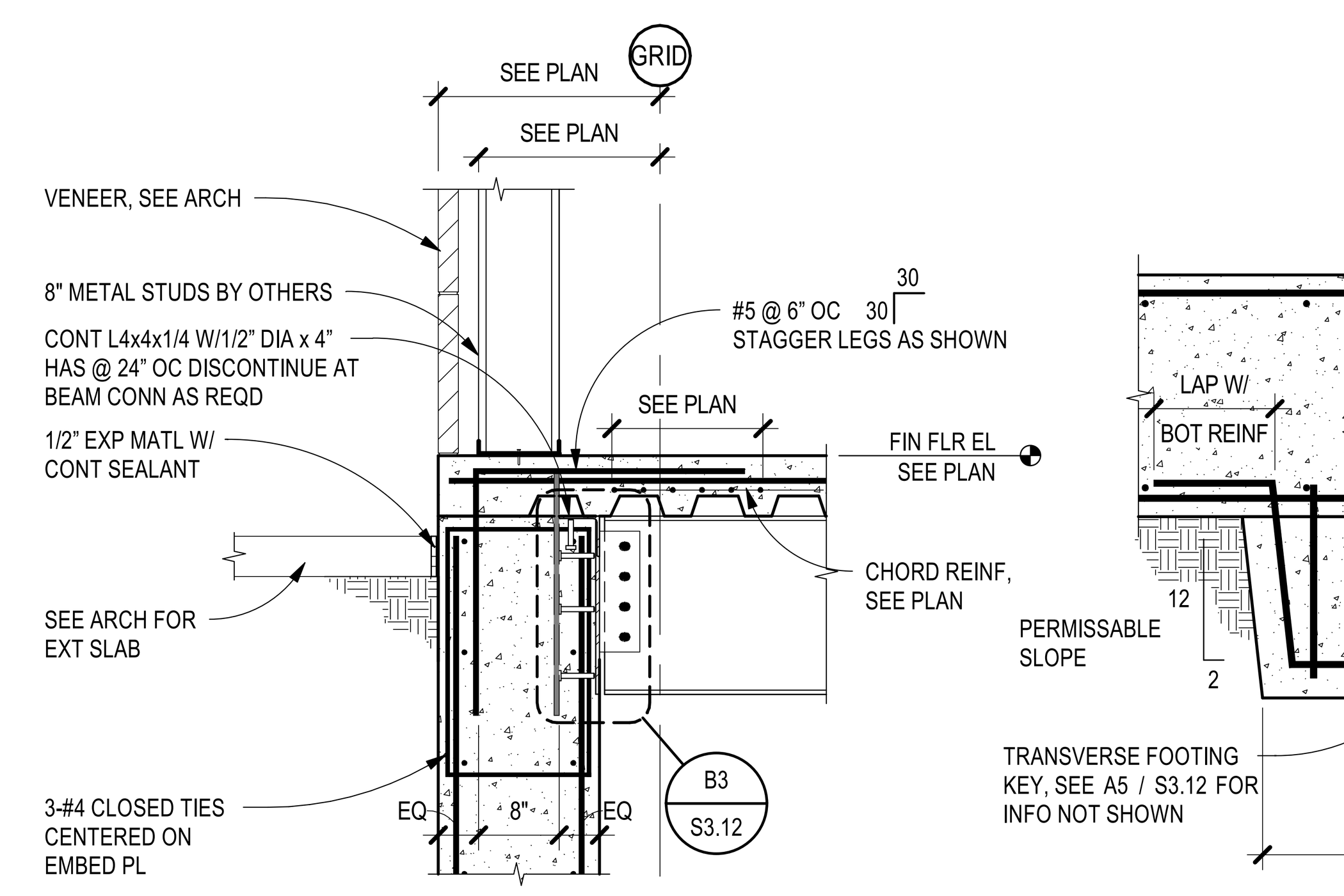


B3 BEAM TO CONC WALL DETAIL
SCALE: 3/4" = 1'-0"



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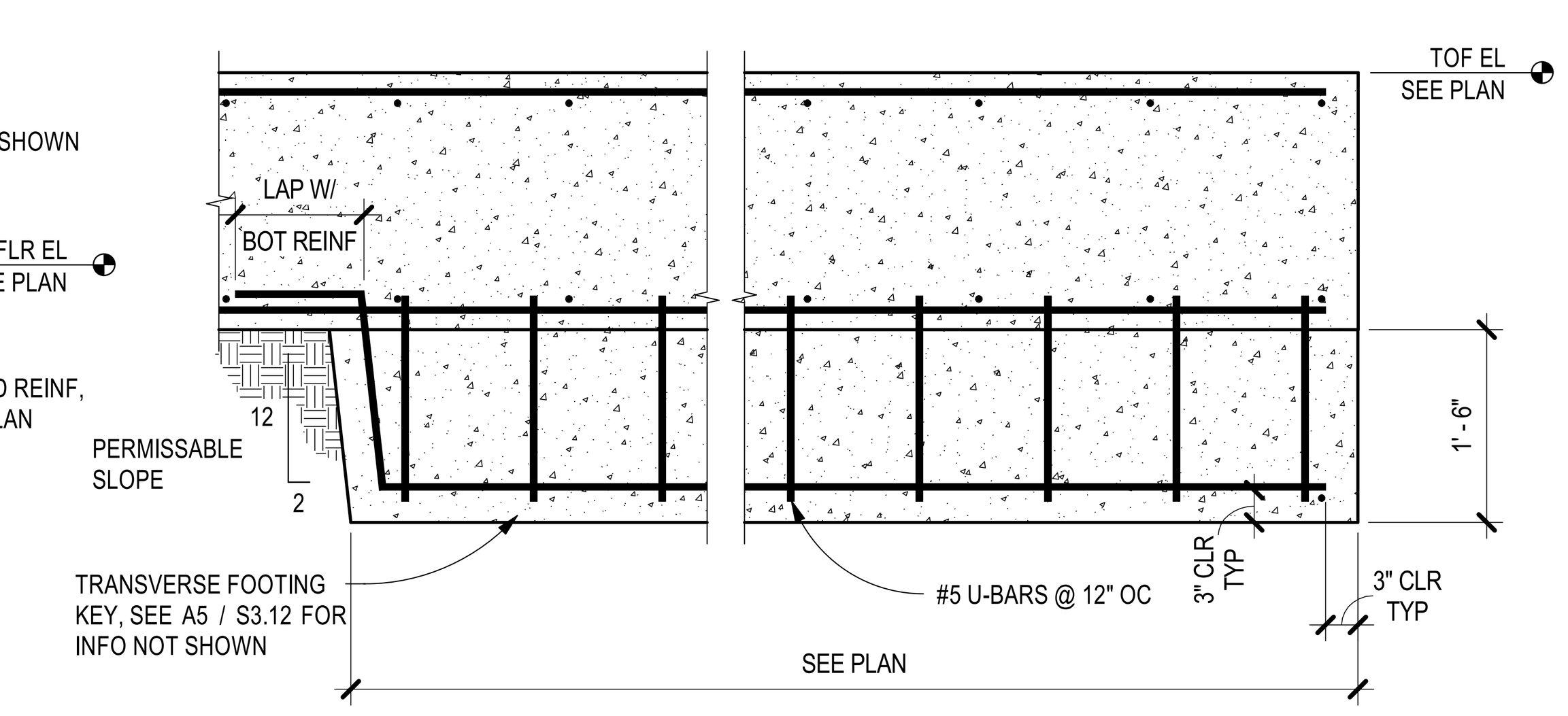

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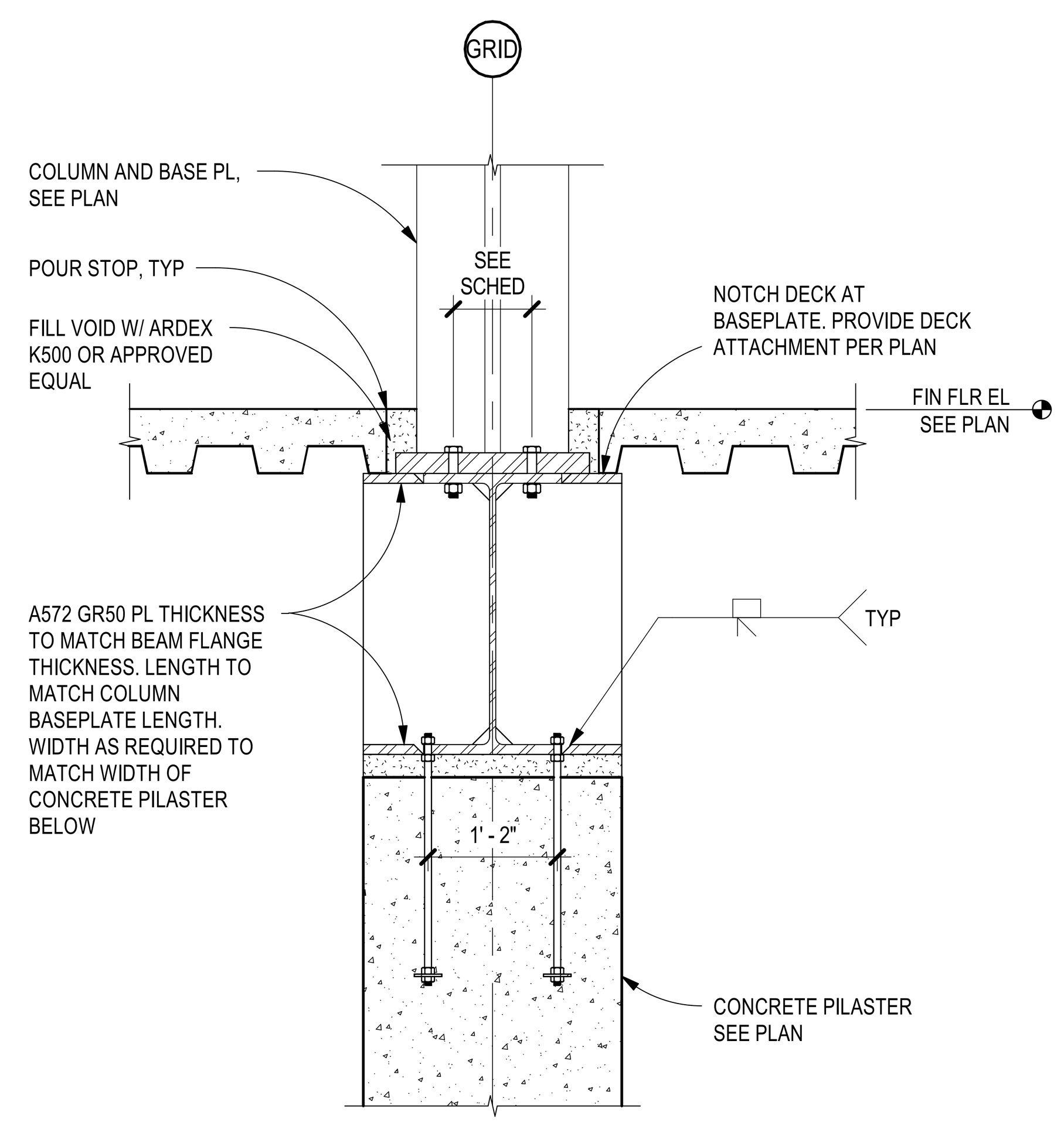
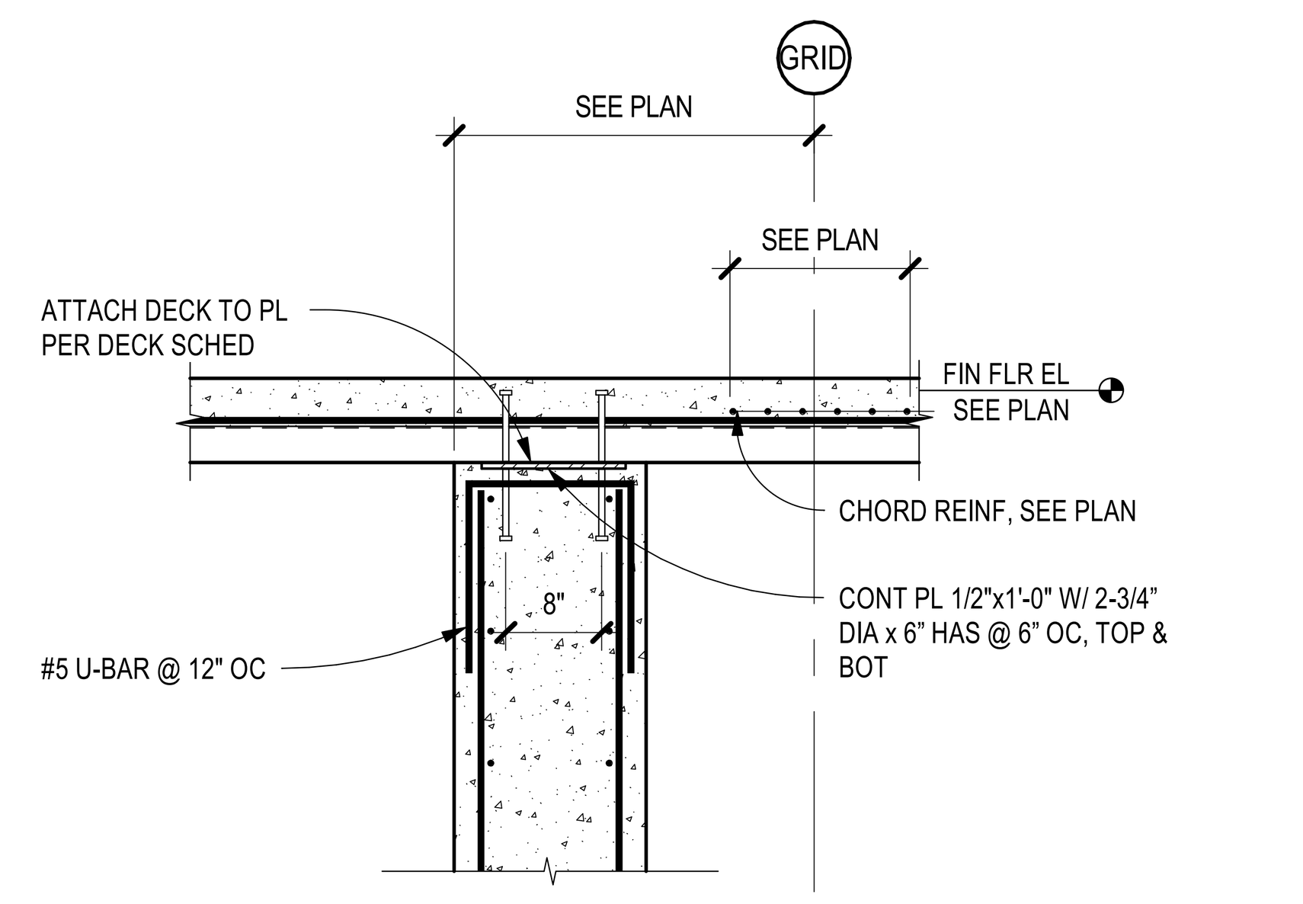


NOTE: SEE A1 / S5.51 FOR INFORMATION NOT SHOWN.

A2 MF COLUMN ON BEAM DETAIL
SCALE: 3/4" = 1'-0"



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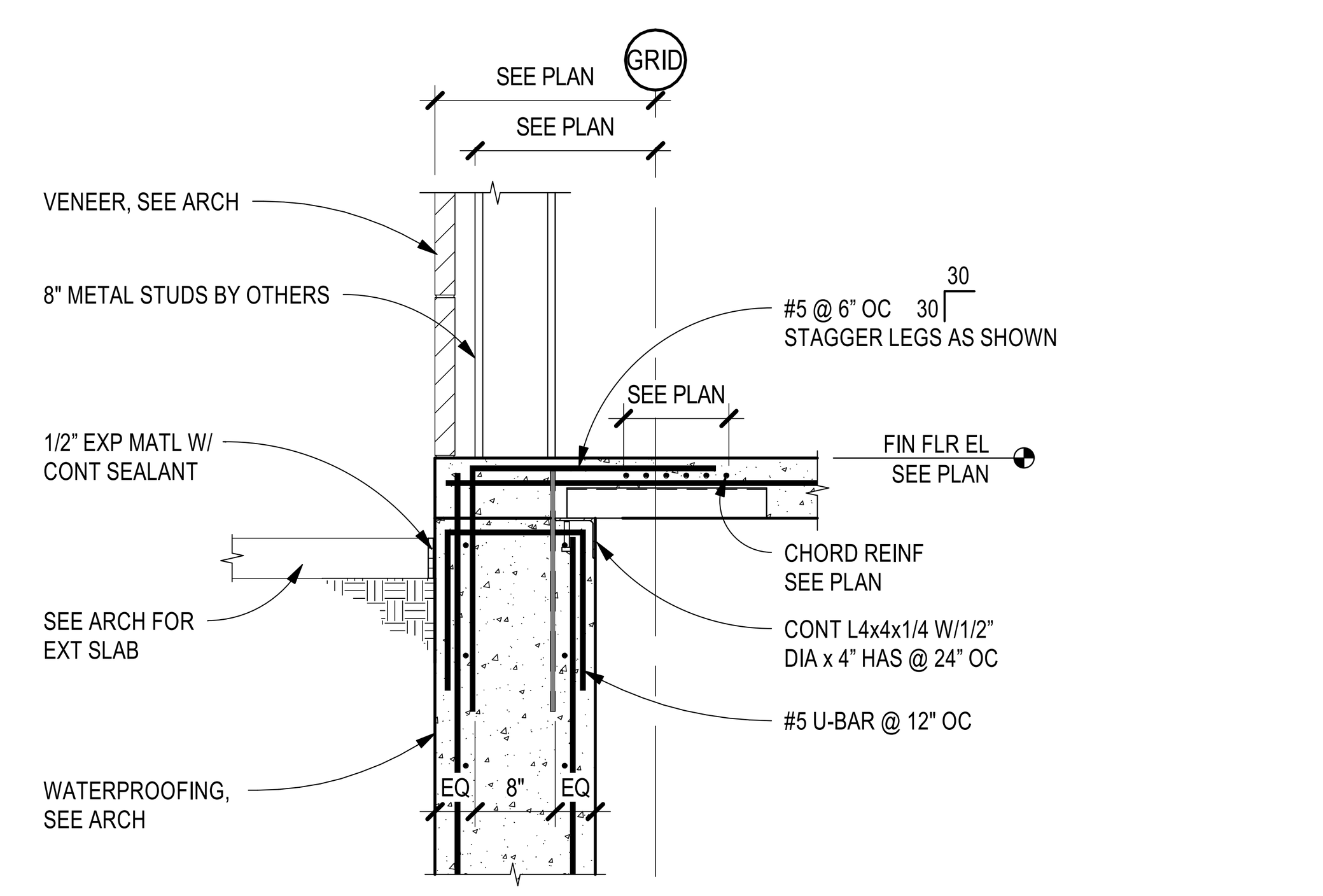

A4 BASEMENT WALL SECTION
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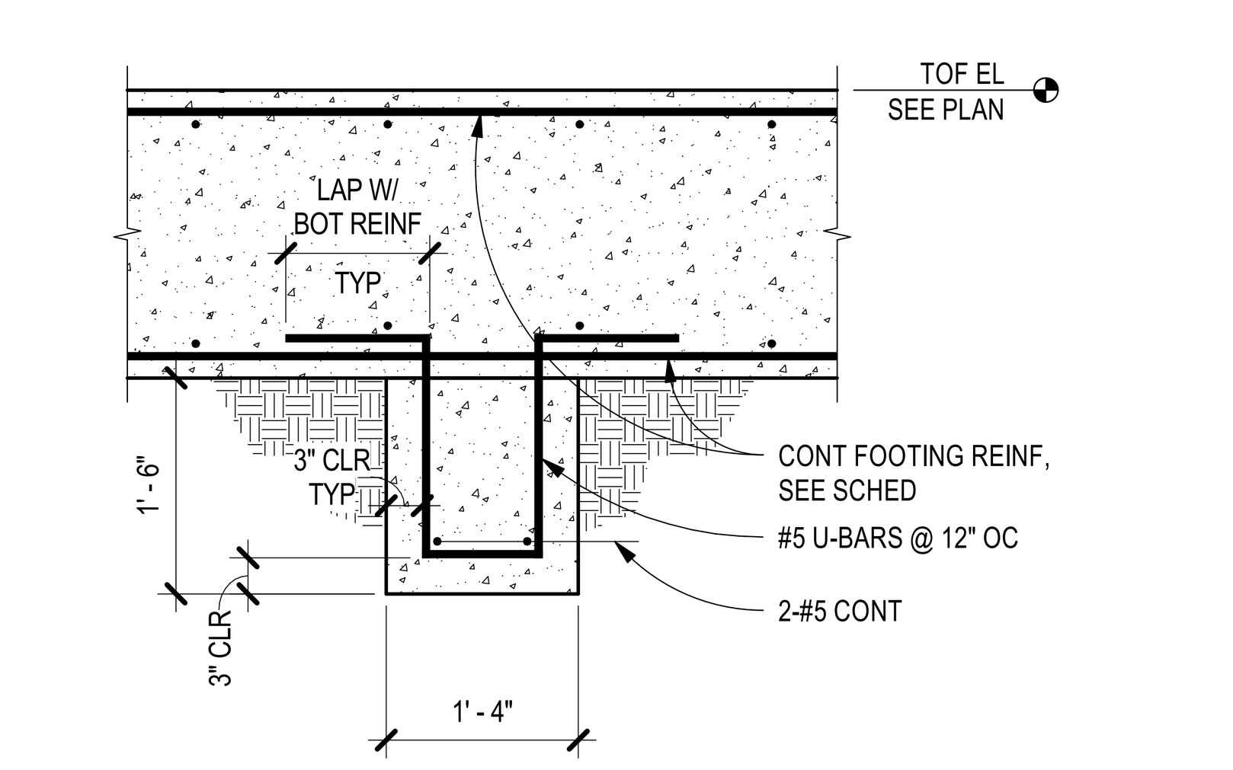

A5 TRANSVERSE FOOTING KEY SECTION
SCALE: 3/4" = 1'-0"

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TAHLEQUAH, OKLAHOMA

KEY PLAN:

PROJECT PHASE:
BID PACKAGE 03

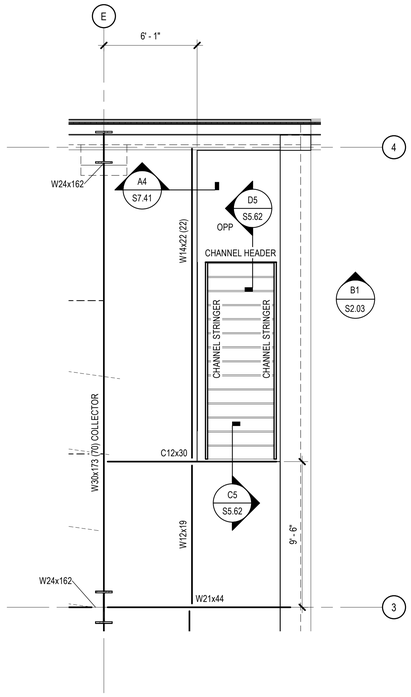
REVISIONS

#	DATE	DESCRIPTION

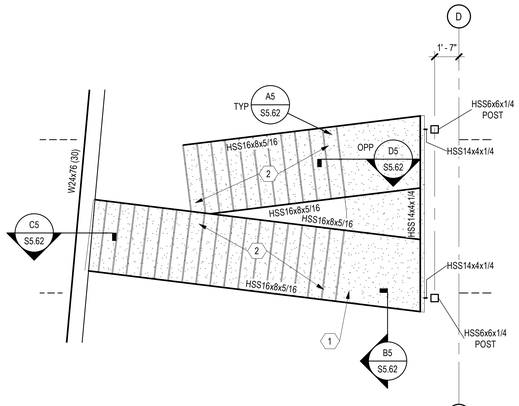
DATE:	JOB NUMBER:
03-20-19	17-13

SHEET NUMBER:
S3.12

FOUNDATION SECTIONS



B1 ENLARGED STAIR PLAN
SCALE: 1/4" = 1'-0"



C2 ENLARGED STAIR PLAN
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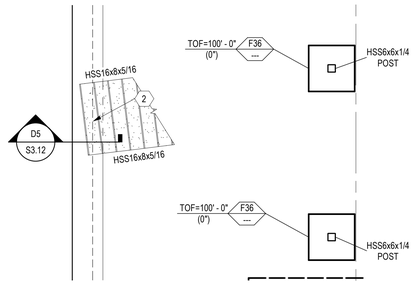

B2 ENLARGED STAIR PLAN
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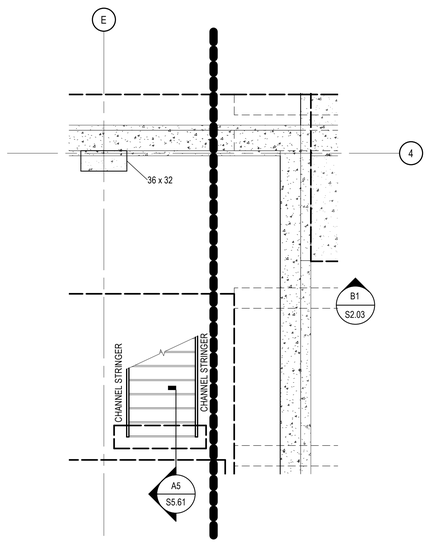
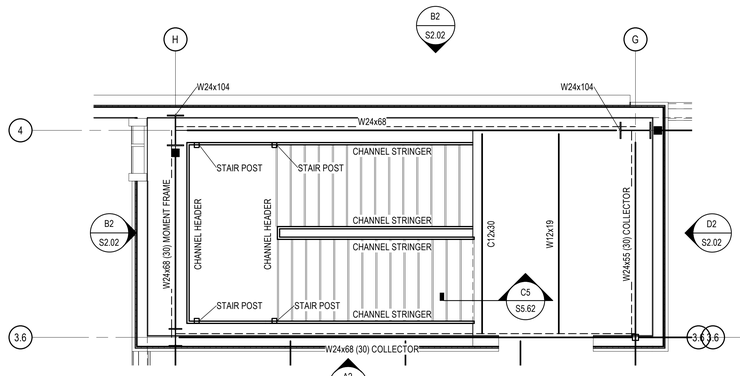


A1 ENLARGED STAIR PLAN
SCALE: 1/4" = 1'-0"



A2 ENLARGED STAIR PLAN
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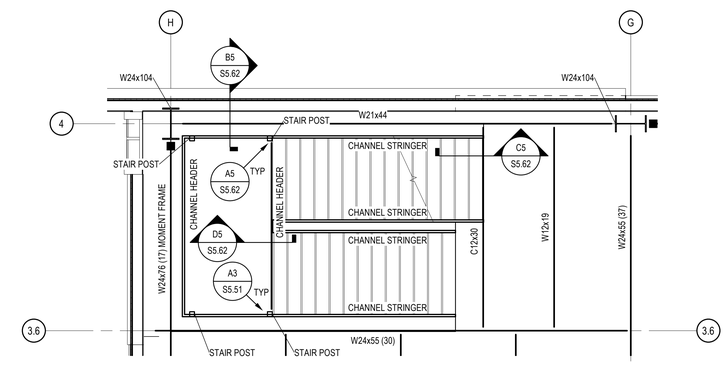
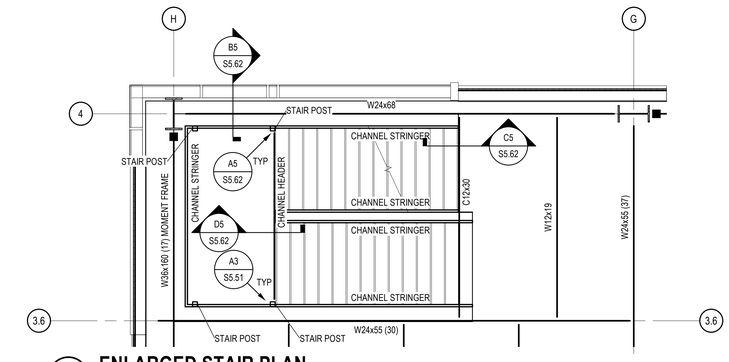
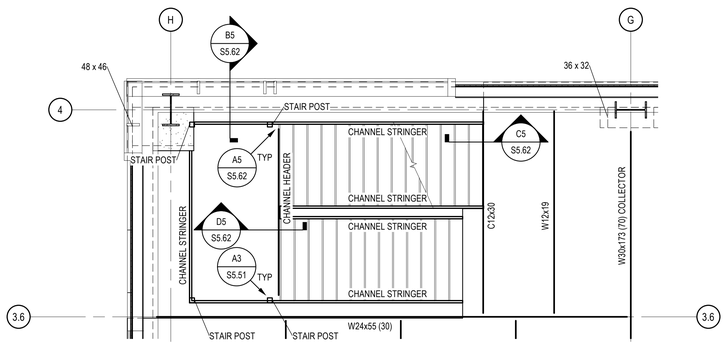


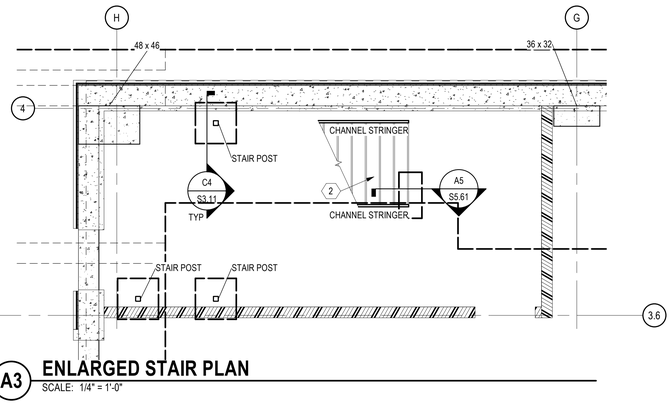
D3 ENLARGED STAIR PLAN
SCALE: 1/4" = 1'-0"



C3 ENLARGED STAIR PLAN
SCALE: 1/4" = 1'-0"



B3 ENLARGED STAIR PLAN
SCALE: 1/4" = 1'-0"



A3 ENLARGED STAIR PLAN
SCALE: 1/4" = 1'-0"

GENERAL SHEET NOTES

1. SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
2. DIMENSIONS ARE TO THE FACE OF CONCRETE, OR STUD UNLESS NOTED OTHERWISE. COORDINATE ALL STAIR DIMENSIONS WITH THE ARCHITECTURAL DRAWINGS.
3. SEE ARCHITECTURAL DRAWINGS FOR INTERMEDIATE LANDING ELEVATIONS.
4. SEE ARCHITECTURAL DRAWINGS FOR STAIR RISE AND RUN.
5. STRUCTURAL COLD FORMED METAL STUDS SHALL BE 600S162-43 AT 16" ON CENTER UNLESS NOTED OTHERWISE.

SHEET KEYNOTE

1. 4" NORMAL WEIGHT CONCRETE LANDING SLAB REINFORCED WITH 6x6-W2.1xW2.1 WELDED WIRE FABRIC IN FLAT SHEETS ONLY OVER METAL PAN. SEE ARCHITECTURAL DRAWINGS FOR LANDING ELEVATIONS.
2. 2" CONCRETE FILLED METAL PANS, REINFORCED WITH 6x6-W2.1xW2.1 WELDED WIRE FABRIC.

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CLIENT:
COLLEGE OF Osteopathic Medicine
AT THE CHEROKEE NATION
TAHLEQUAH, OKLAHOMA

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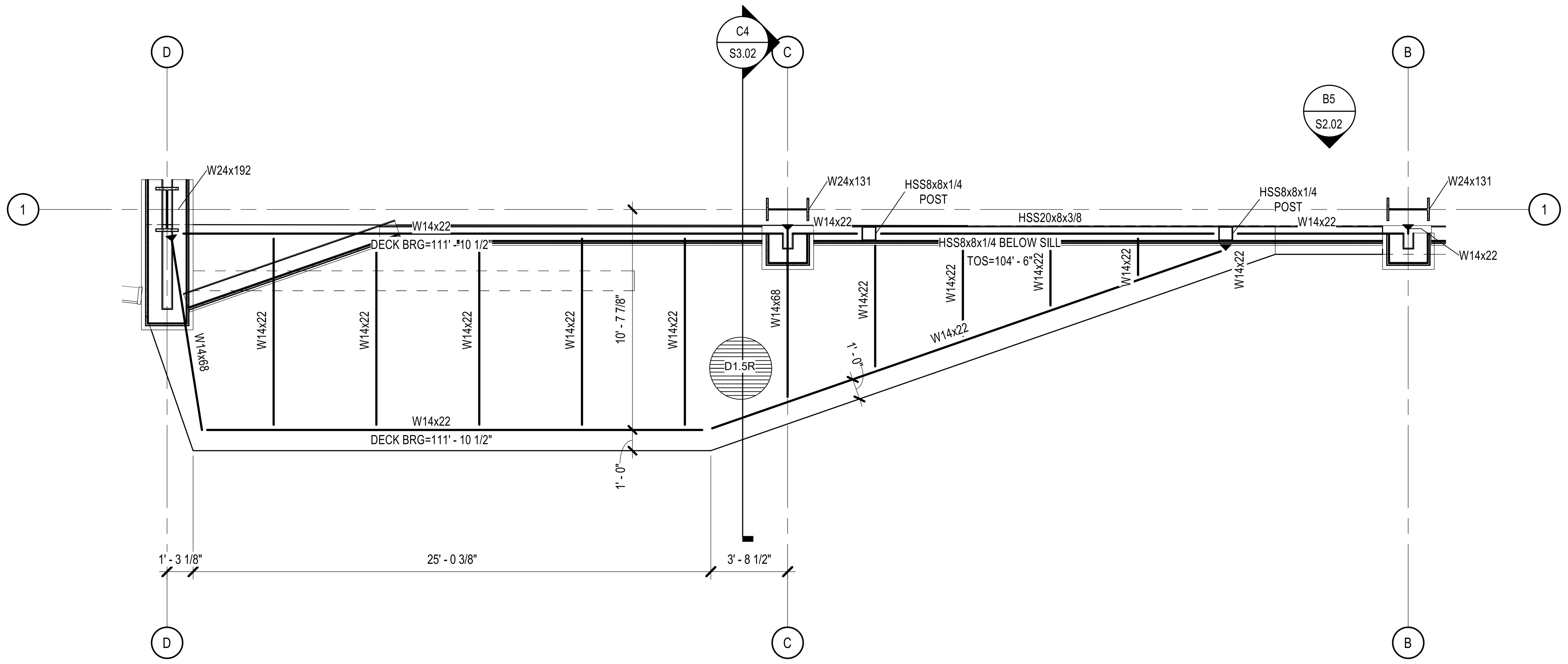
KEY PLAN:

PROJECT PHASE:
BID PACKAGE 03

#	DATE	REVISIONS DESCRIPTION

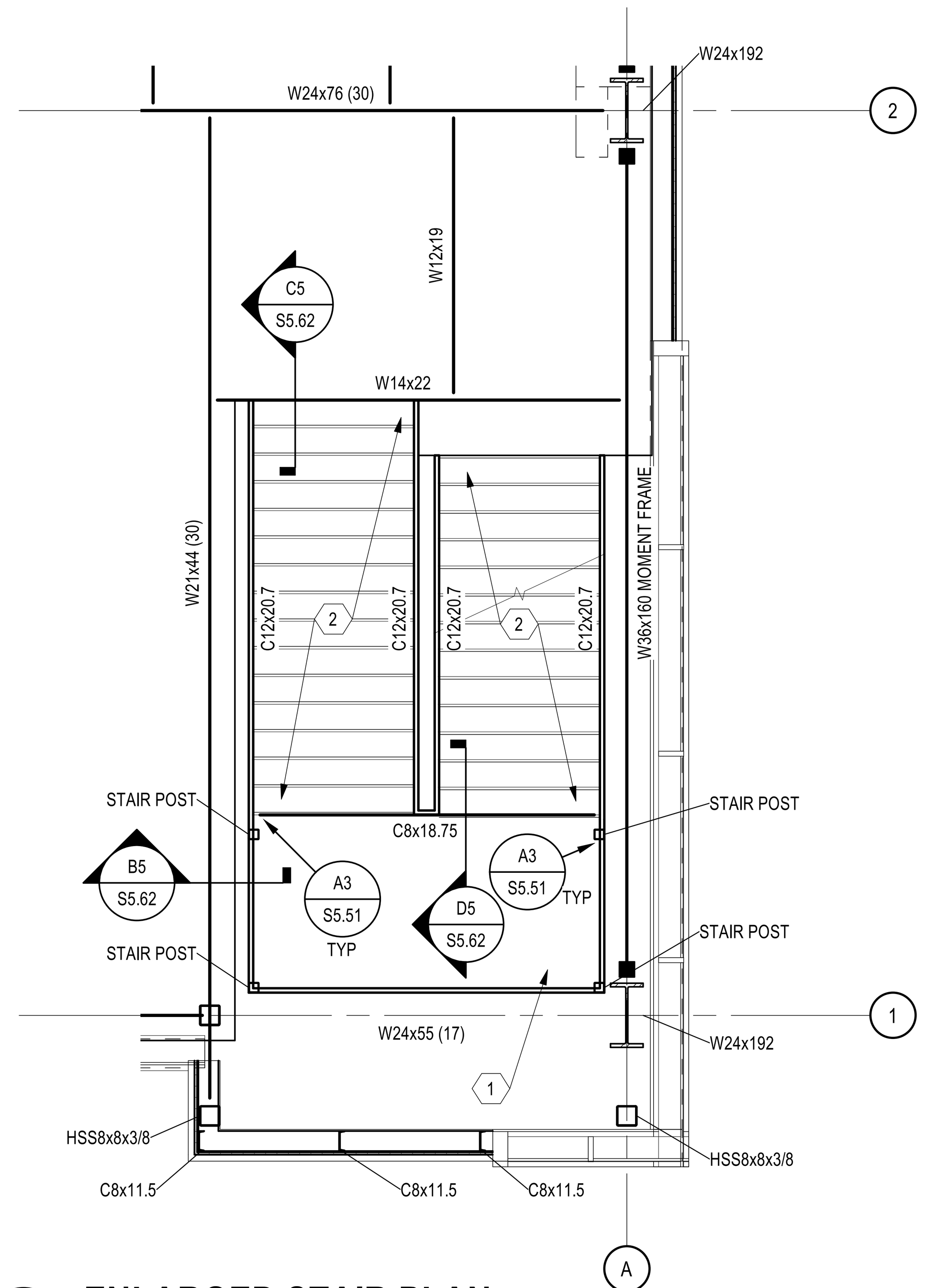
DATE: 03-20-19
JOB NUMBER: 17-13
SHEET NUMBER: S4.01

ENLARGED STAIR PLANS



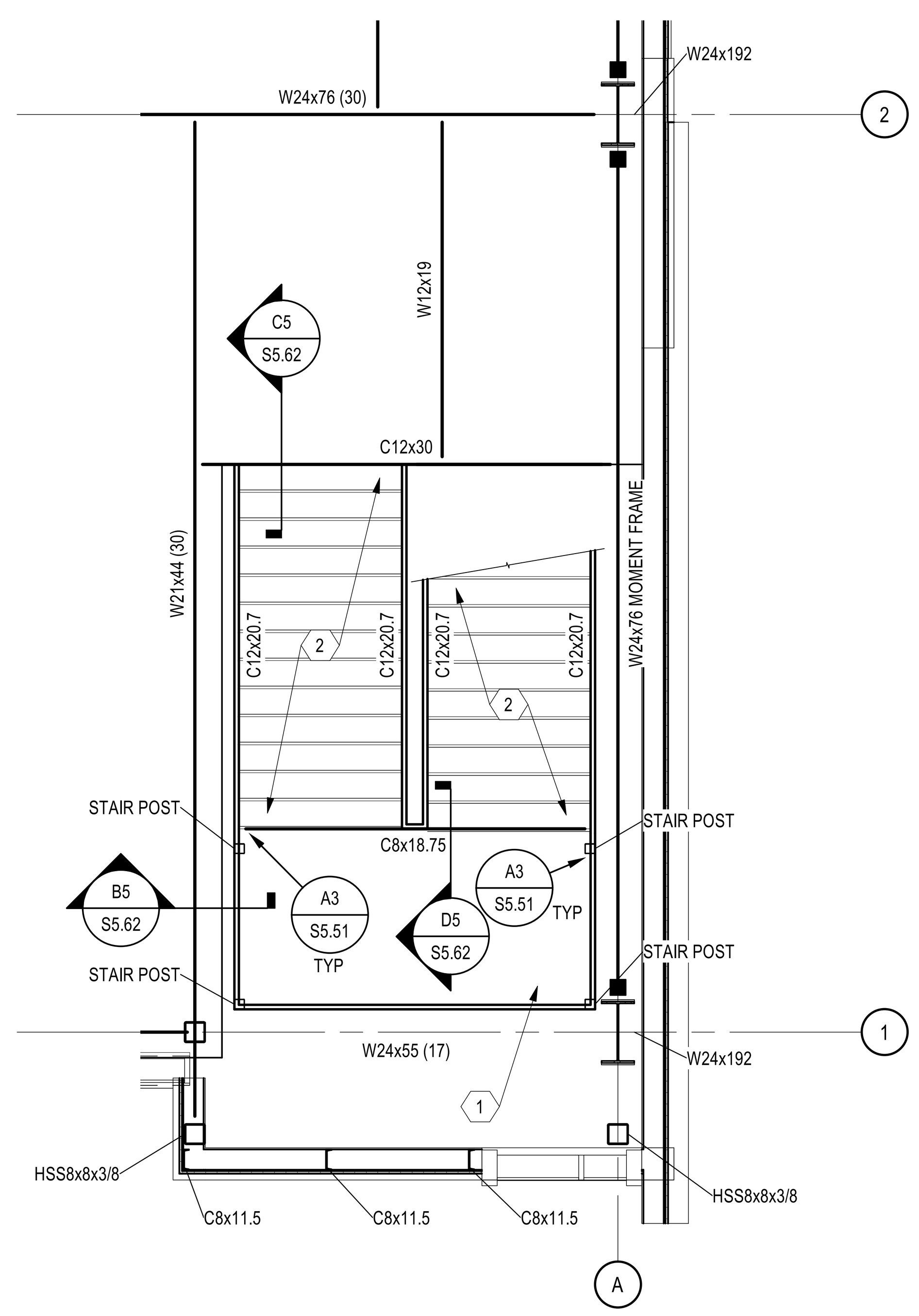
C1 ENTRY CANOPY FRAMING PLAN

SCALE: 1/4" = 1'-0"



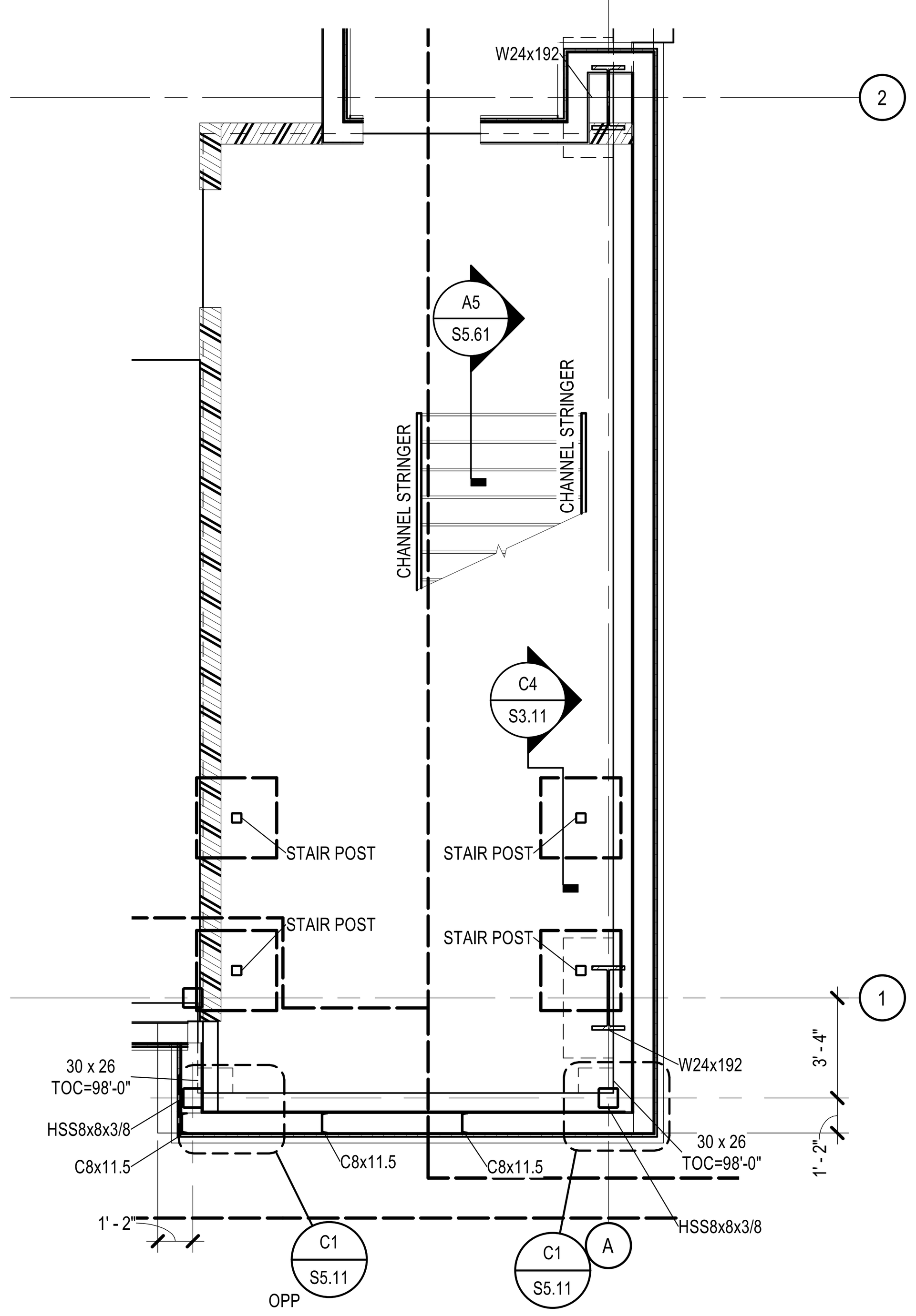
C4 ENLARGED STAIR PLAN

SCALE: 1/4" = 1'-0"



A3 ENLARGED STAIR PLAN

SCALE: 1/4" = 1'-0"



A4 ENLARGED STAIR PLAN

SCALE: 1/4" = 1'-0"

GENERAL SHEET NOTES

1. SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
2. DIMENSIONS ARE TO THE FACE OF CONCRETE, OR STUD UNLESS NOTED OTHERWISE. COORDINATE ALL STAIR DIMENSIONS WITH THE ARCHITECTURAL DRAWINGS.
3. SEE ARCHITECTURAL DRAWINGS FOR INTERMEDIATE LANDING ELEVATIONS.
4. SEE ARCHITECTURAL DRAWINGS FOR STAIR RISE AND RUN.
5. STRUCTURAL COLD FORMED METAL STUDS SHALL BE 600S162-43 AT 16" ON CENTER UNLESS NOTED OTHERWISE.

SHEET KEYNOTE

1. 4" NORMAL WEIGHT CONCRETE LANDING SLAB REINFORCED WITH 6x6-W2.1xW2.1 WELDED WIRE FABRIC IN FLAT SHEETS ONLY OVER METAL PAN. SEE ARCHITECTURAL DRAWINGS FOR LANDING ELEVATIONS.
2. 2" CONCRETE FILLED METAL PANS, REINFORCED WITH 6x6-W2.1xW2.1 WELDED WIRE FABRIC.

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CLIENT:

THE CHEROKEE NATION

COLLEGE OF Osteopathic Medicine AT THE CHEROKEE NATION

TAHLEQUAH, OKLAHOMA

KEY PLAN:

PROJECT PHASE:

BID PACKAGE 03

REVISIONS		
#	DATE	DESCRIPTION

DATE:	JOB NUMBER:
03-20-19	17-13

SHEET NUMBER:

S4.02

ENLARGED PLANS

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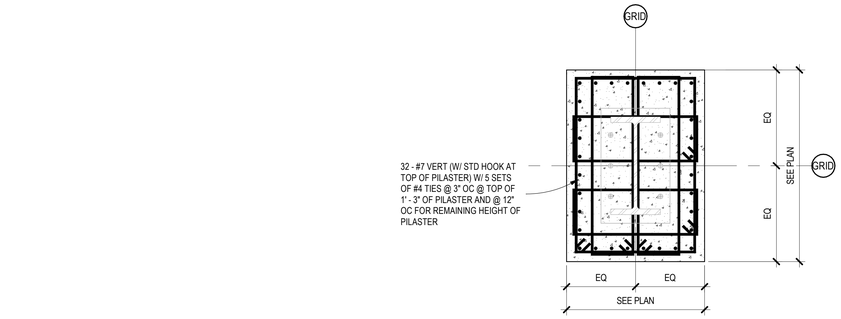
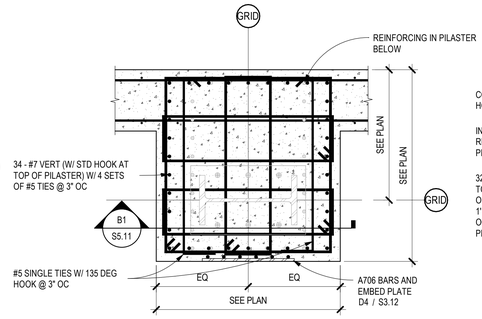
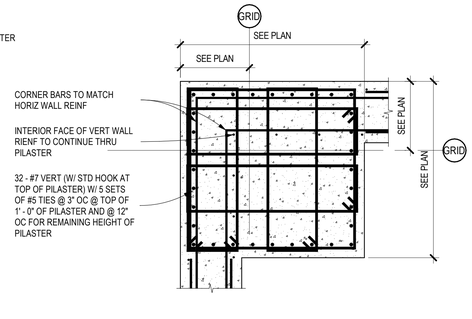


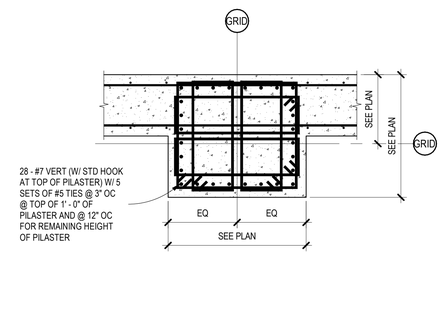
D2 PILASTER DETAIL
SCALE: 3/4" = 1'-0"



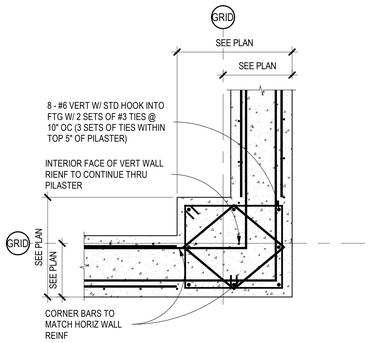
D3 PILASTER DETAIL - UPPER PILASTER
SCALE: 3/4" = 1'-0"



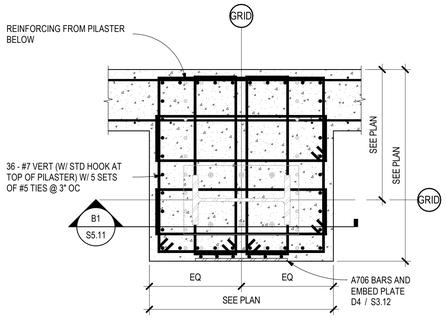
D4 PILASTER DETAIL - LOWER PILASTER
SCALE: 3/4" = 1'-0"



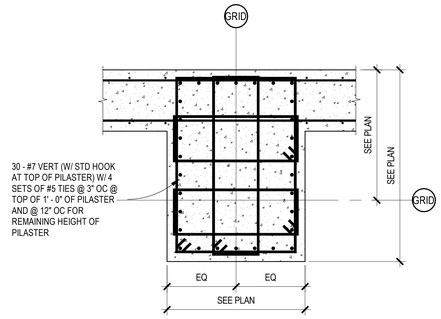
D5 PILASTER DETAIL - LOWER PILASTER
SCALE: 3/4" = 1'-0"



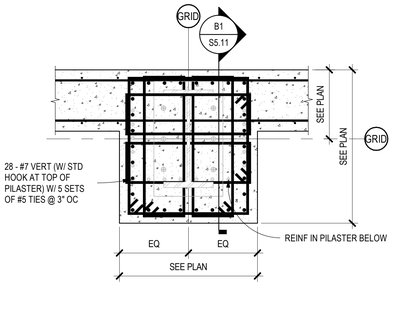
C1 PILASTER DETAIL - STAIR SHAFT
SCALE: 3/4" = 1'-0"



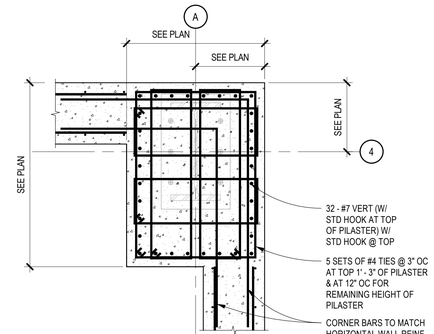
C2 PILASTER DETAIL - UPPER PILASTER
SCALE: 3/4" = 1'-0"



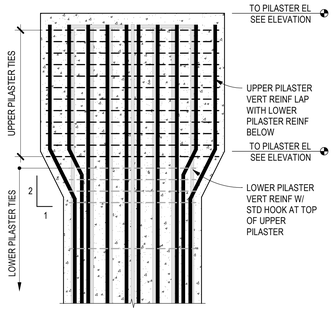
C3 PILASTER DETAIL - LOWER PILASTER
SCALE: 3/4" = 1'-0"



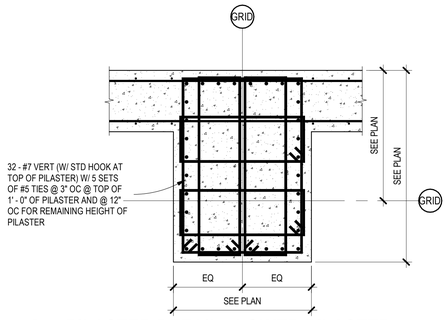
C4 PILASTER DETAIL - UPPER PILASTER
SCALE: 3/4" = 1'-0"



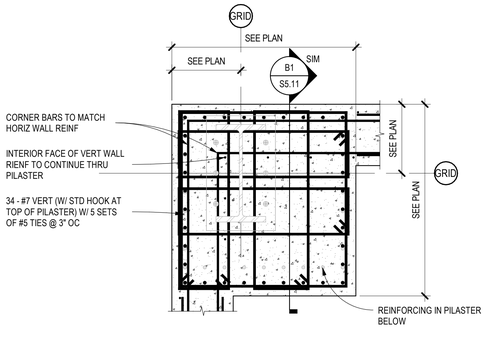
C5 PILASTER DETAIL
SCALE: 3/4" = 1'-0"



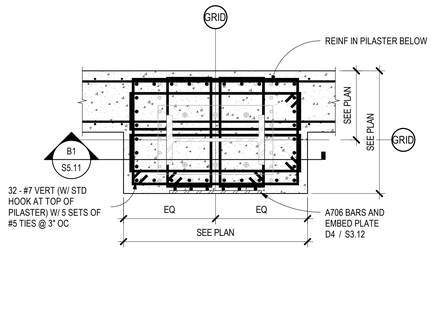
B1 PILASTER SPLICE SECTION
SCALE: 3/4" = 1'-0"



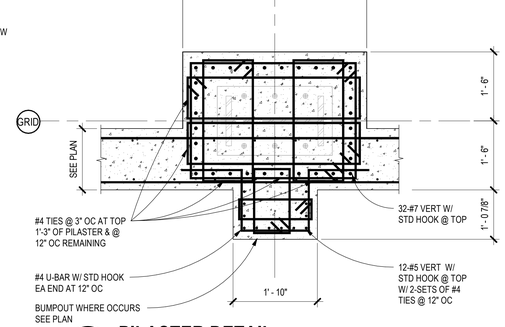
B2 PILASTER DETAIL - LOWER PILASTER
SCALE: 3/4" = 1'-0"



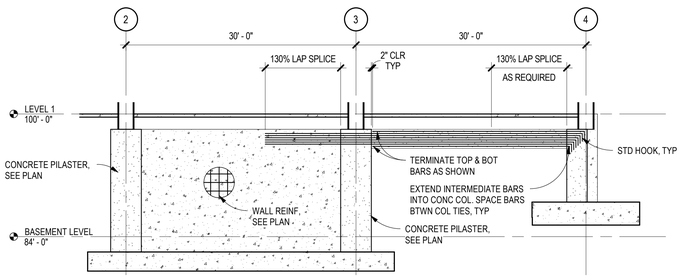
B3 PILASTER DETAIL - UPPER PILASTER
SCALE: 3/4" = 1'-0"



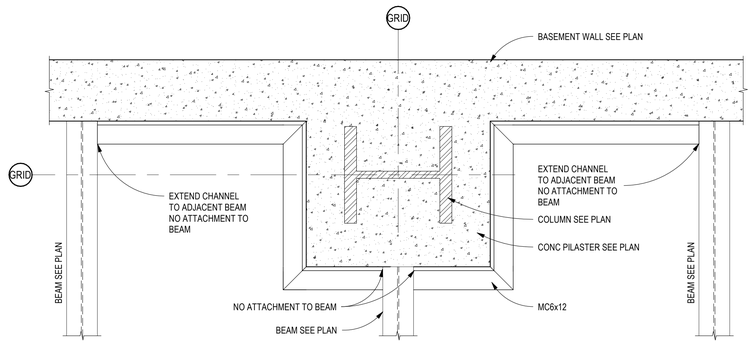
B4 PILASTER DETAIL - UPPER PILASTER
SCALE: 3/4" = 1'-0"



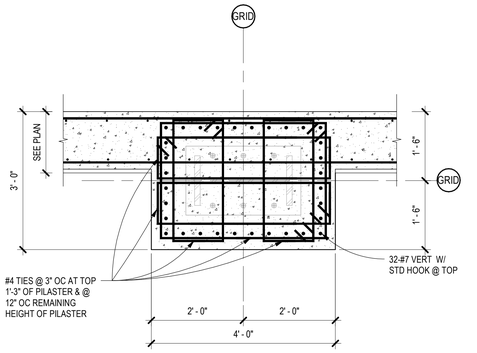
B5 PILASTER DETAIL
SCALE: 3/4" = 1'-0"



A1 SHEAR WALL ELEVATION DETAIL
SCALE: 1/8" = 1'-0"



A3 TYPICAL PERIMETER CHANNEL AT BASEMENT LID PLAN
SCALE: 3/4" = 1'-0"



A5 PILASTER DETAIL
SCALE: 3/4" = 1'-0"

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CLIENT:

COLLEGE OF Osteopathic Medicine
AT THE CHEROKEE NATION
TAHLEQUAH, OKLAHOMA

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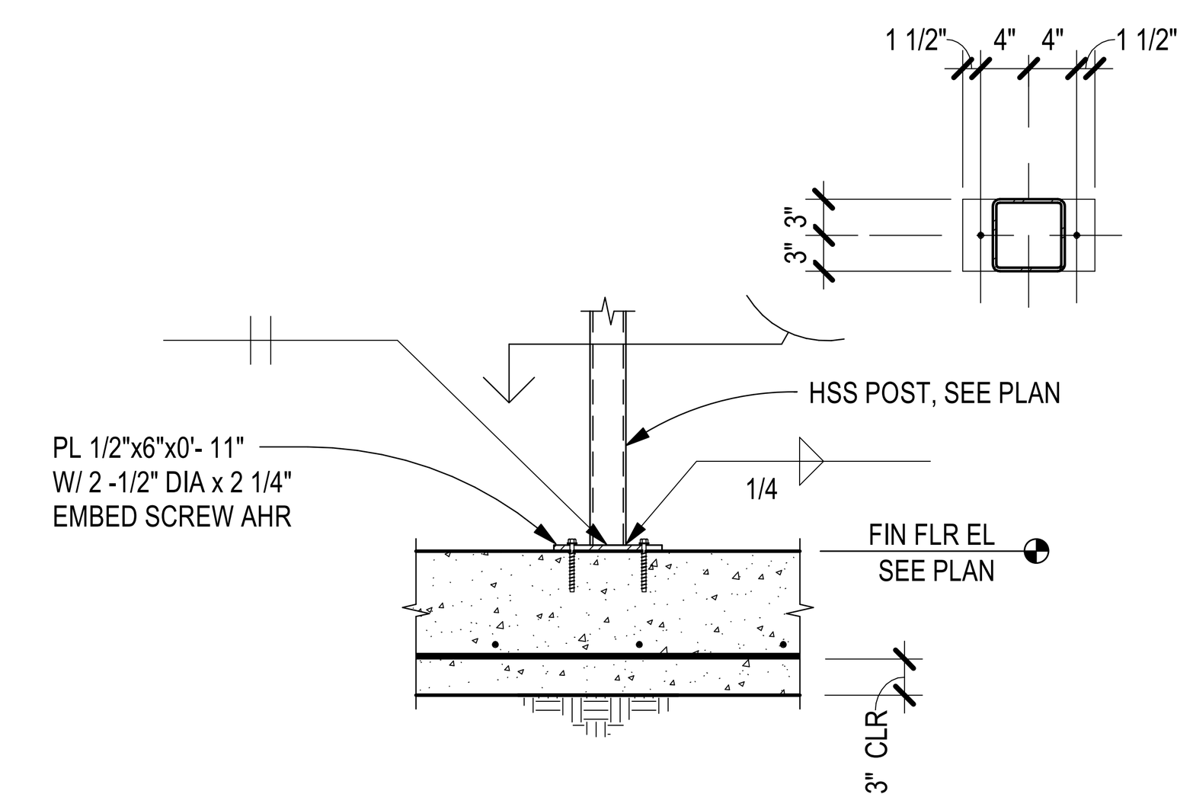
KEY PLAN:

PROJECT PHASE:
BID PACKAGE 03

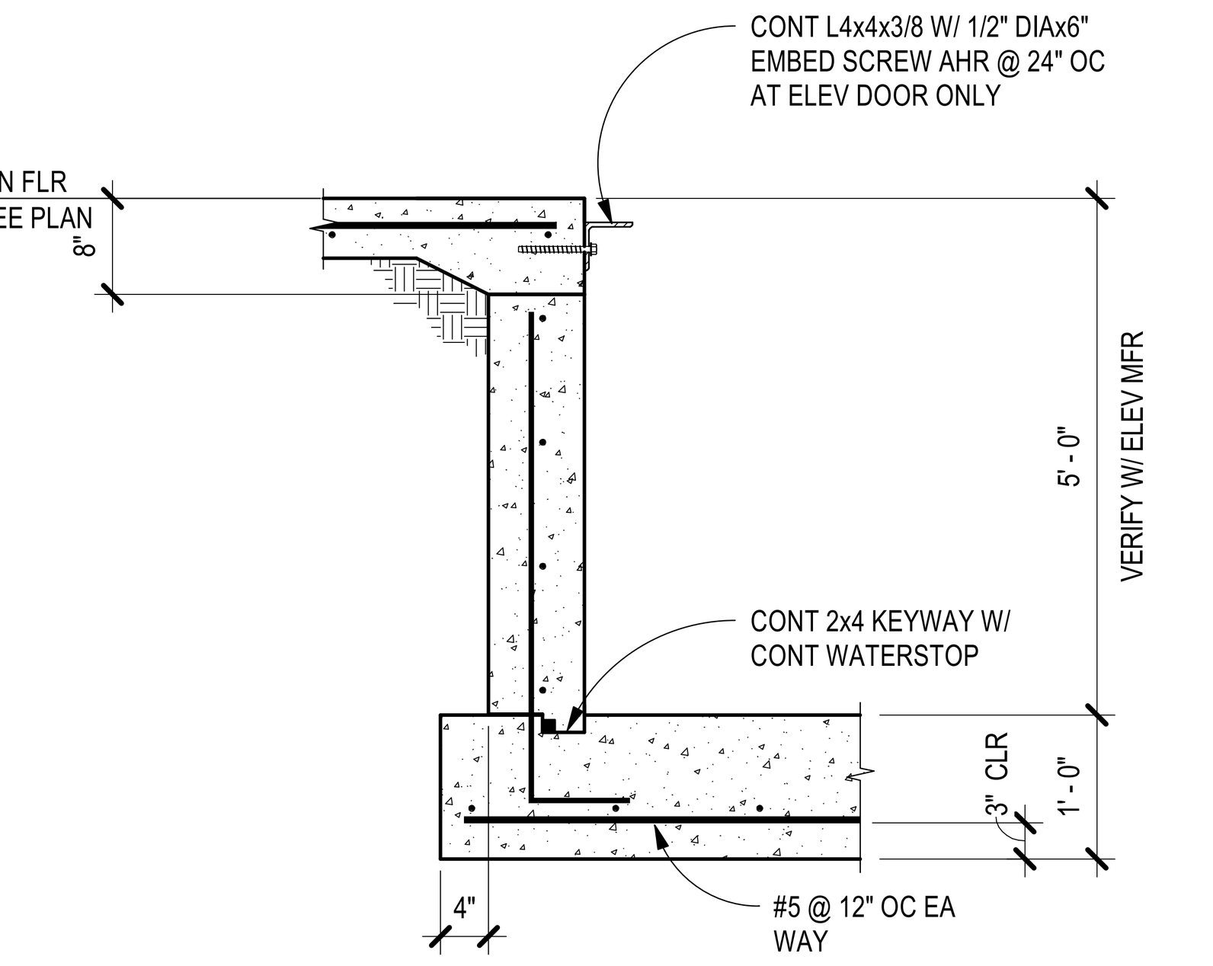
REVISIONS		
#	DATE	DESCRIPTION

DATE: 03-20-19
JOB NUMBER: 17-13
SHEET NUMBER: S5.11

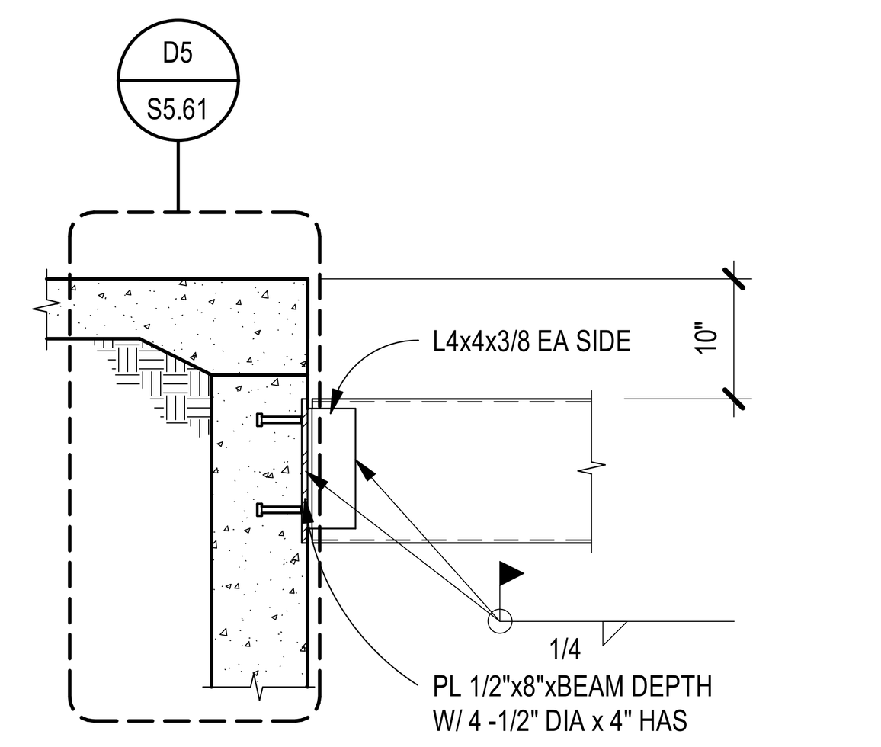
CONCRETE DETAILS



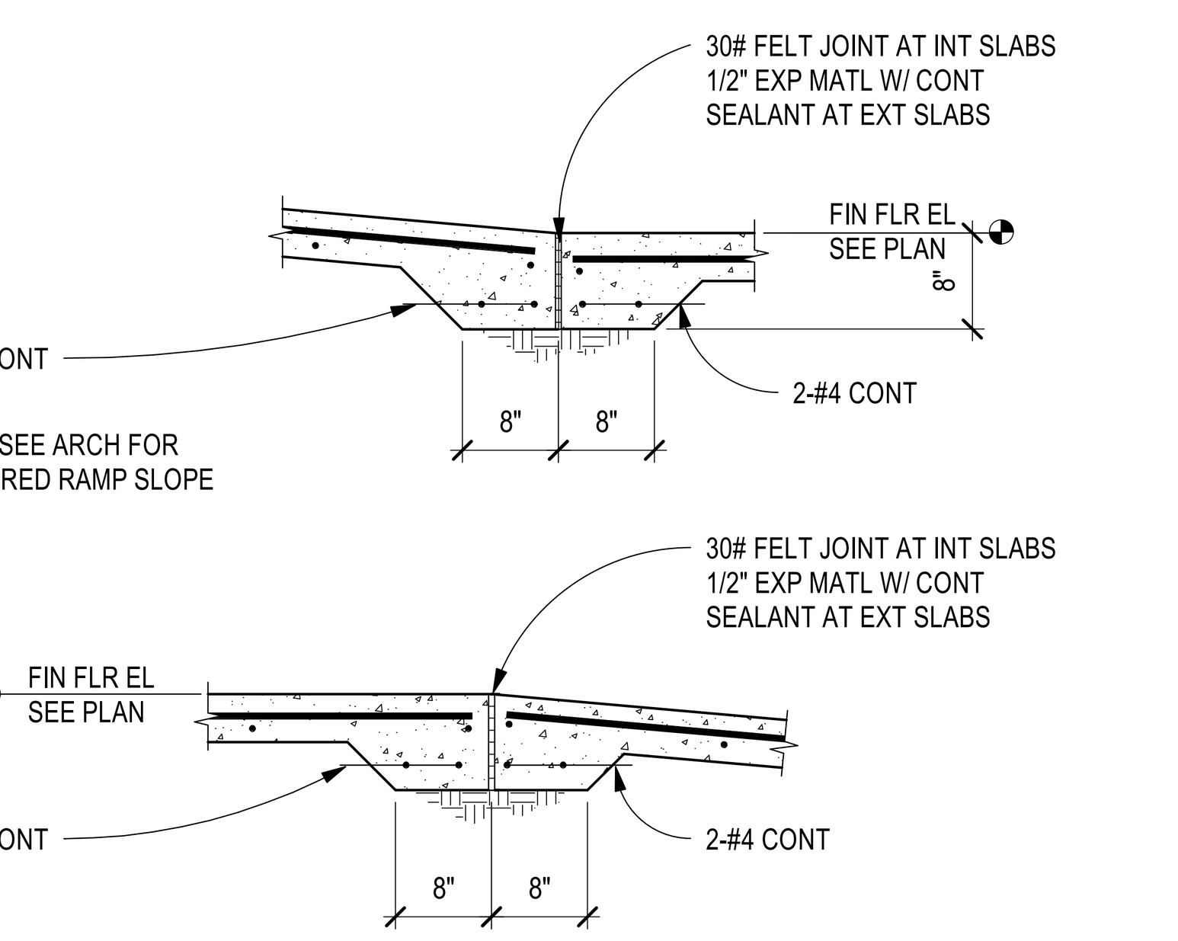
D4 INT COLUMN AT ELEVATOR PIT SLAB
SCALE: 3/4" = 1'-0"



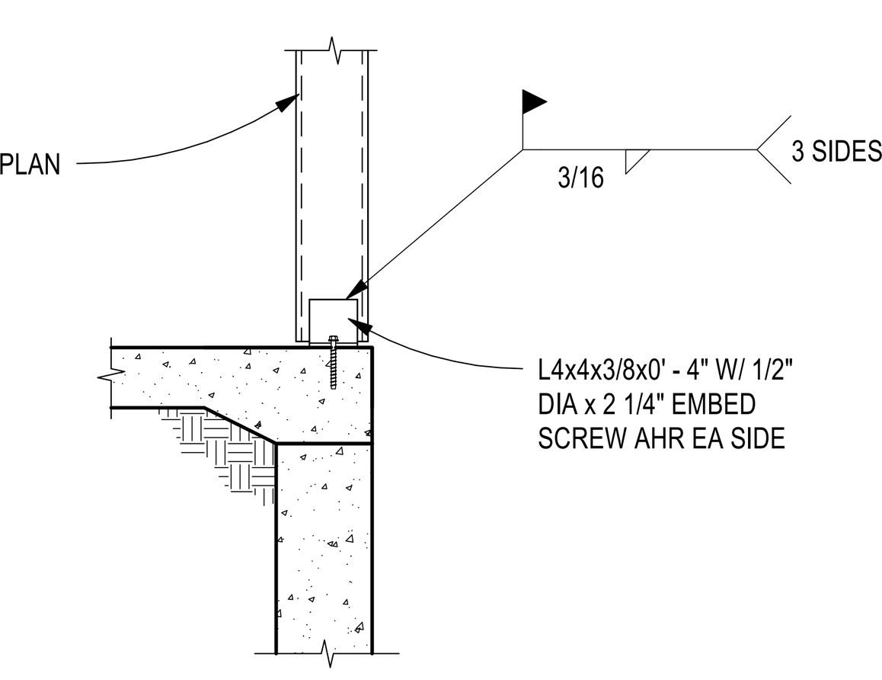
D5 ELEVATOR PIT SECTION
SCALE: 3/4" = 1'-0"



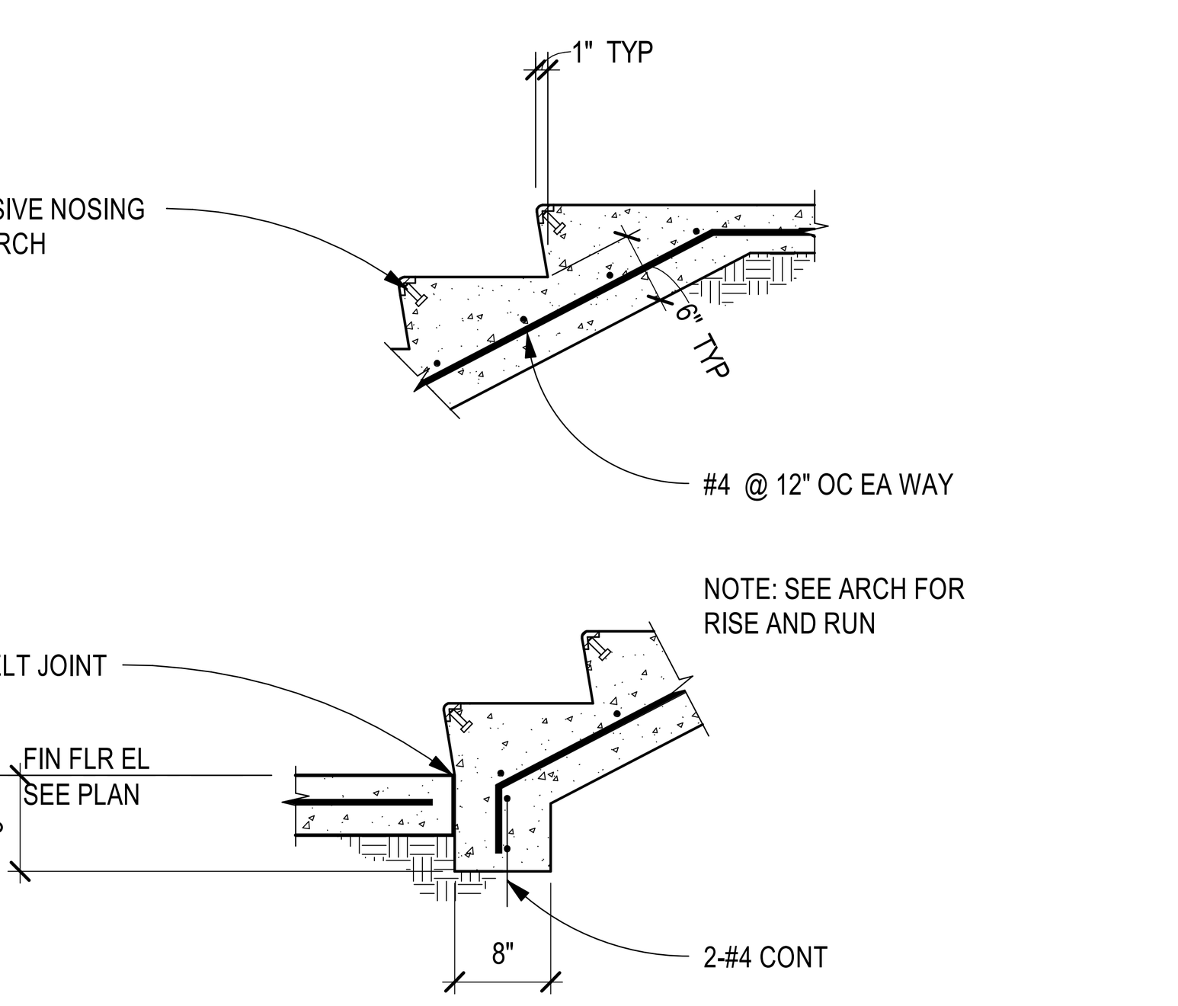
C4 ELEVATOR SEPARATOR BEAM
SCALE: 3/4" = 1'-0"



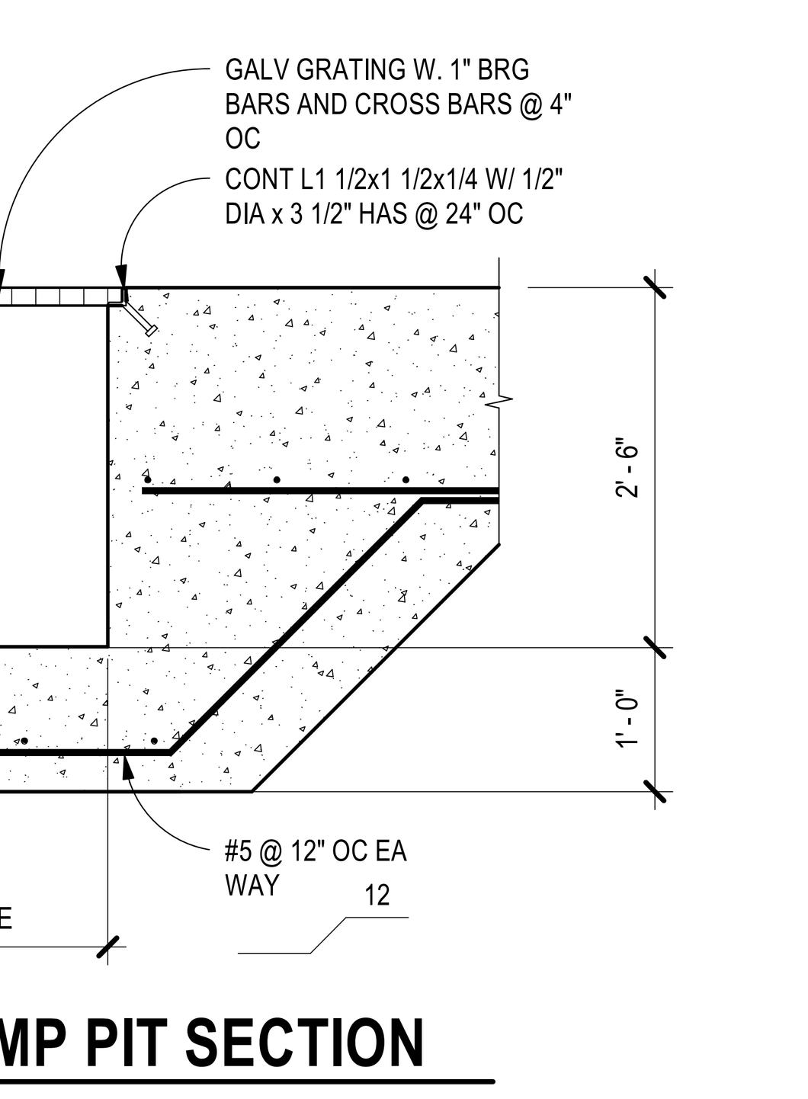
C5 TYPICAL RAMP SECTION
SCALE: 3/4" = 1'-0"



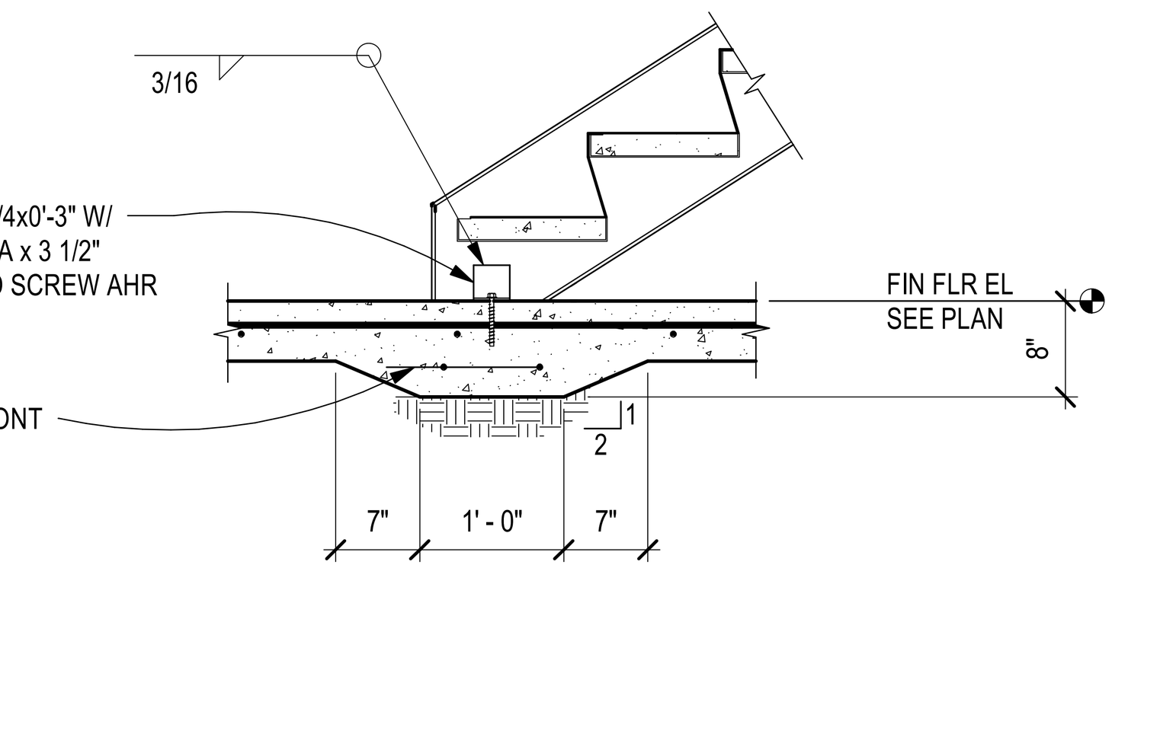
B4 ELEVATOR PIT SECTION
SCALE: 3/4" = 1'-0"



B5 TYPICAL CONC STAIR SECTION
SCALE: 3/4" = 1'-0"



A4 ELEVATOR SUMP PIT SECTION
SCALE: 3/4" = 1'-0"



A5 TYPICAL STAIR BASE DETAIL
SCALE: 3/4" = 1'-0"

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CLIENT:

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SEPT. 6, 1839

COLLEGE OF
Osteopathic Medicine
AT THE CHEROKEE NATION
TAHLEQUAH, OKLAHOMA

KEY PLAN:

PROJECT PHASE:
BID PACKAGE 03

REVISIONS

#	DATE	DESCRIPTION

DATE: 03-20-19
JOB NUMBER: 17-13
SHEET NUMBER: S5.61

TYPICAL VERTICAL
CIRCULATION
FOUNDATION DETAILS

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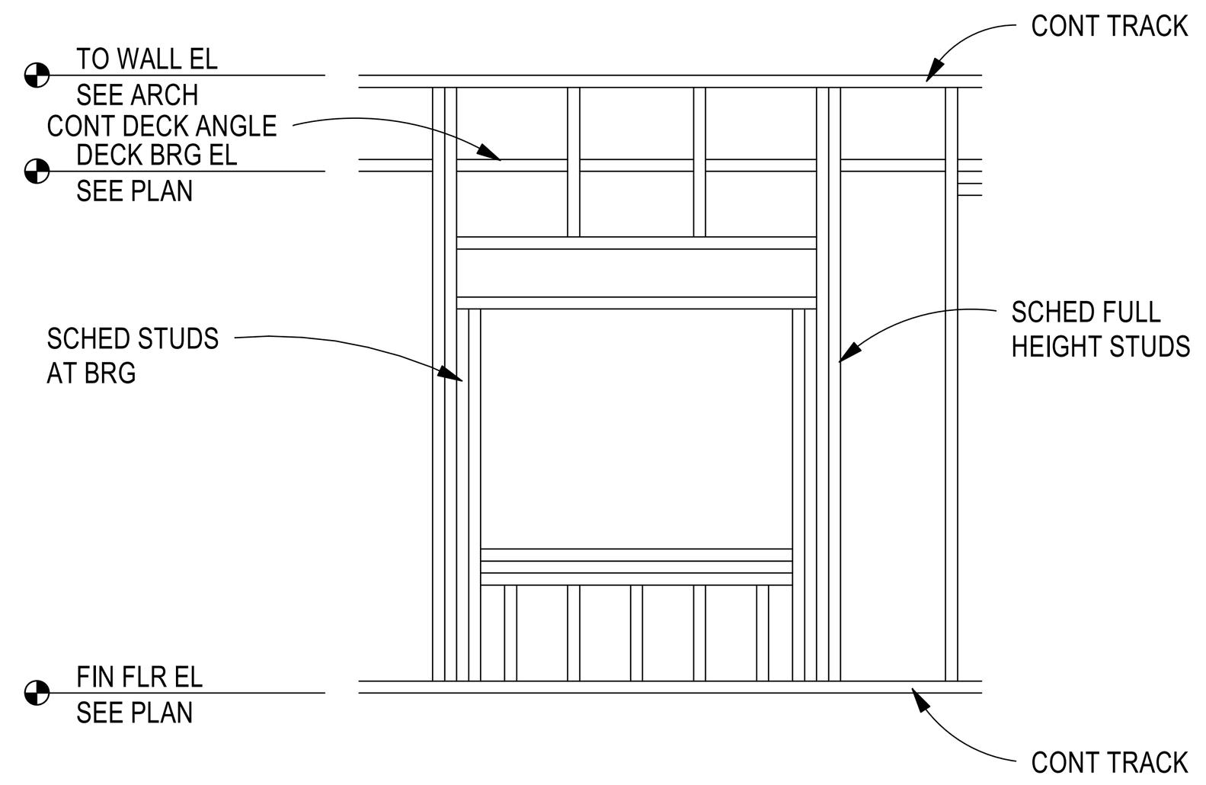
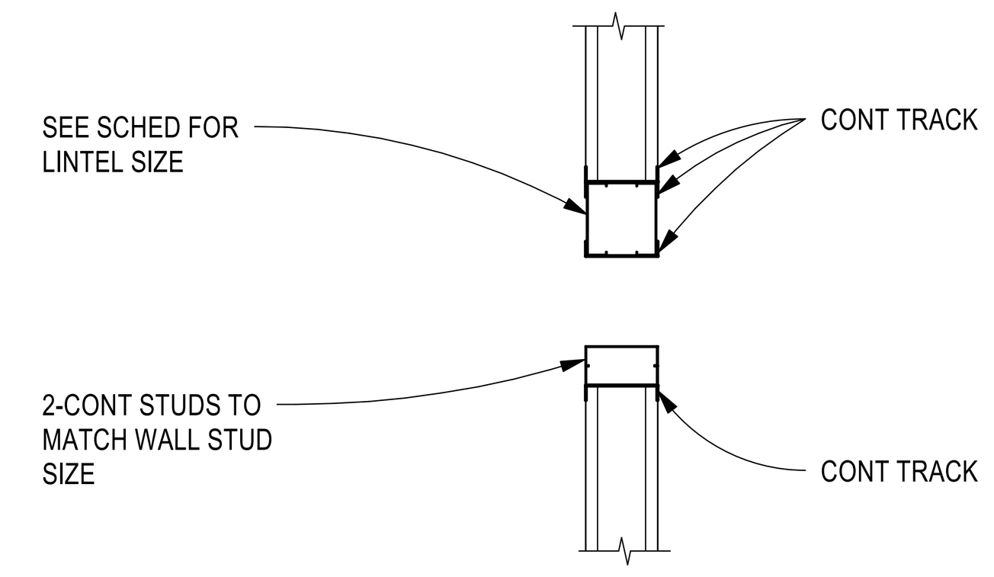
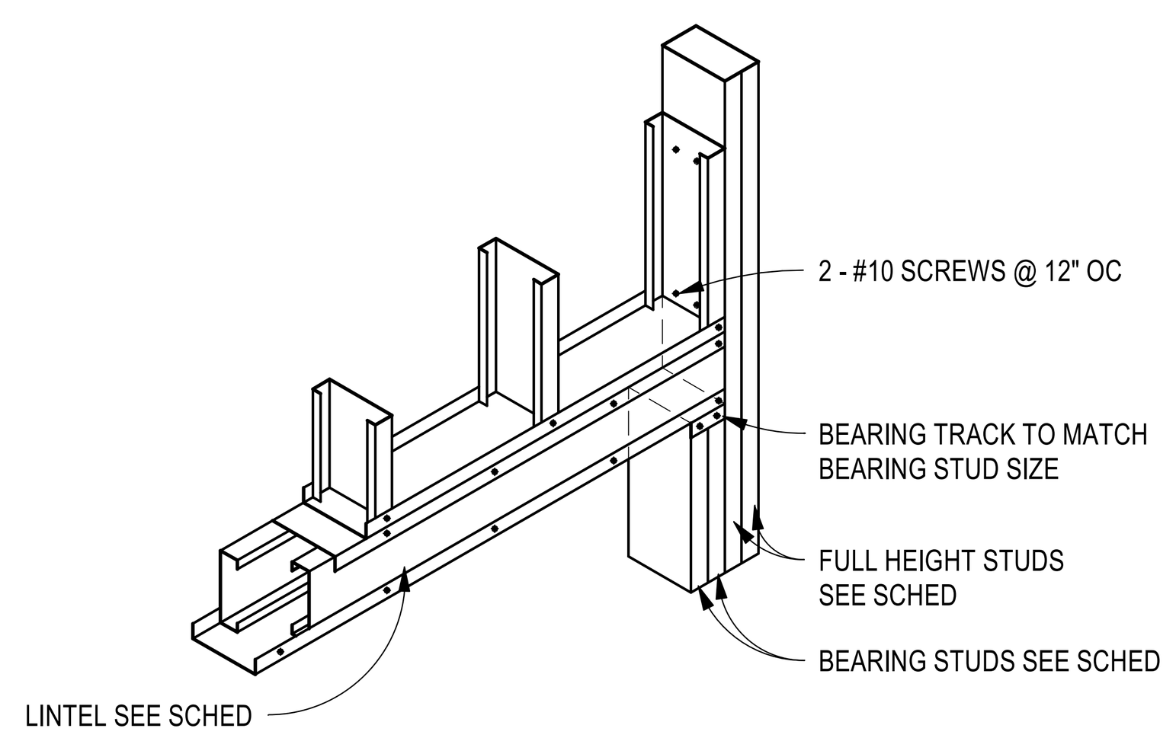
DECK SCHEDULE													
		SLAB			METAL DECK				DECK ATTACHMENTS			TOTAL SLAB / DECK THICKNESS	
MARK	COMPOSITE SLAB	THICK	MATL	REINF	THICK	TYPE	GAGE	FINISH	ATTACH PERP TO RIBS	ATTACH PARALLEL TO RIBS	ATTACH SIDELAPS		COMMENTS
D1.5R		- -	- -	- -	1 1/2"	B	20	PAINTED	4-5/8 "DIA PUDDLE WELDS PER 36 "WIDE SHEET	5/8" DIA PUDDLE WELDS @ 12" OC	#10 SCREWS @ 12" OC	1 1/2"	
D6R	X	3"	NW CONC	6x6 - W2.1xW2.1 WELD WIRE FABRIC IN FLAT SHEETS	3"	VLI	18	GALVANIZED	[4-5/8 "DIA PUDDLE WELDS] PER 36 "WIDE SHEET	[5/8" DIA PUDDLE WELDS] @ [12]" OC	[#10} SCREWS @ [12]" OC	6"	
D6RA		6"	NW CONC	#4 @ 12" OC EA WAY	---	---	---	---	---	---	---	6"	SLOPE PER ARCHITECTURAL DRAWINGS. INCLUDE CRYSTALLINE WATERPROOFING ADMIZTURE IN CONCRETE
D7.5F	X	4 1/2"	NW CONC	#5 @ 12" OC EA WAY	3"	VLI	18	GALVANIZED	[4-5/8 "DIA PUDDLE WELDS] PER 36 "WIDE SHEET	[5/8" DIA PUDDLE WELDS] @ [12]" OC	[#10} SCREWS @ [12]" OC	7 1/2"	PROVIDE 3/4" DIAMETER x 6" LONG HEADED ANCHOR STUDS WHEN SPECIFIED ON PLAN

SLAB-ON-GRADE SCHEDULE					
	SLAB				
MARK	THICKNESS	MATL	REINFORCING	BEARING STRATA	COMMENTS
S5	5"	CONC	#4 @ 18" OC EA WAY	15 MIL VAPOR RETARDER OVER LASER LEVELED SUBGRADE PER GEN STRUCT NOTES	THE FINISHED TOP SURFACE SHOULD BE FLAT AND LEVEL WITH SUFFICIENT FINES TO FILL BETWEEN COARSE AGGREGATE. IF THIS IS NOT THE CASE, PROVIDE UP TO 1/2 INCH BLOTTER LAYER OF SAND
S6	5"	CONC	#4 @ 12" OC EA WAY	15 MIL VAPOR RETARDER OVER LASER LEVELED SUBGRADE PER GEN STRUCT NOTES	THE FINISHED TOP SURFACE SHOULD BE FLAT AND LEVEL WITH SUFFICIENT FINES TO FILL BETWEEN COARSE AGGREGATE. IF THIS IS NOT THE CASE, PROVIDE UP TO 1/2 INCH BLOTTER LAYER OF SAND





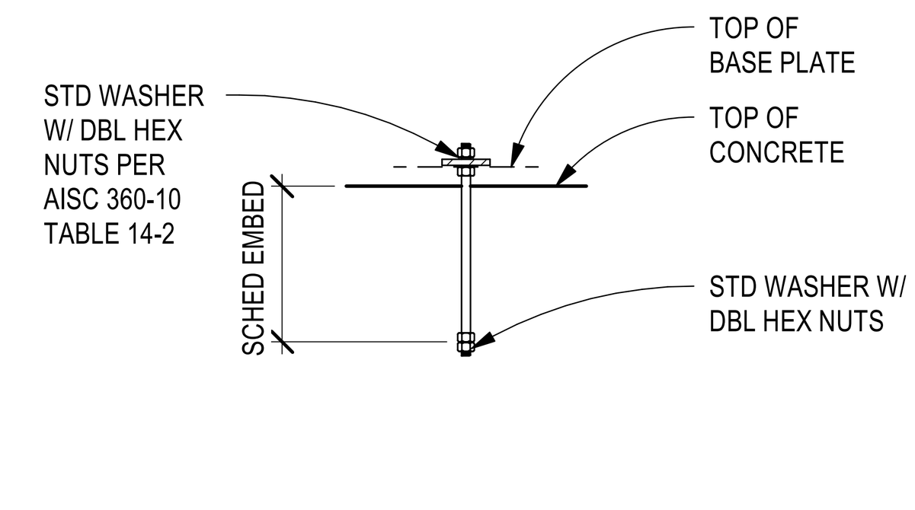


GRAVITY BASE PLATE SCHEDULE			
BASE PLATE			ANCHOR BOLTS
COLUMN SIZE	TYPE	SIZE "T"x"A"x"B"	QTY x SIZE x GRADE
W12	A	PL 2"x21"x1' - 9"	4 - 3/4" DIA x 9" F1554 GR 36
HSS6x6	A	PL 3/4"x14"x1' - 2"	4 - 3/4" DIA x 9" F1554 GR 36
HSS8x8	A	PL 3/4"x16"x1' - 4"	4 - 3/4" DIA x 9" F1554 GR 36

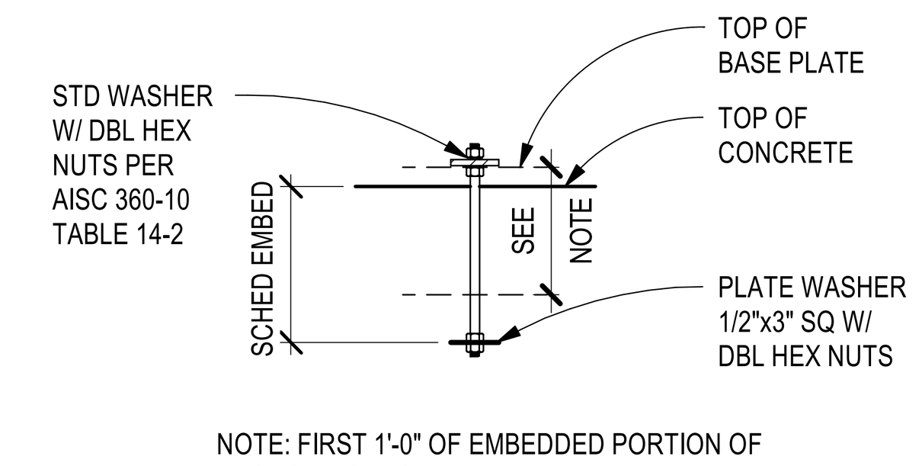
NOTE: FOR GRAVITY BASE PLATES, PROVIDE MAXIMUM OVERSIZED HOLES AND PLATE WASHERS WITH SIZE AND THICKNESS PER AISC 360-10 TABLE 14-2.

LATERAL BASE PLATE SCHEDULE			
BASE PLATE			ANCHOR BOLTS
MARK	TYPE	SIZE "T"x"A"x"B"	QTY x SIZE x GRADE
BP1	B	PL 2 1/4"x30"x1'-6"	6 - 1 1/4" DIA x 18" F1554 GR 36
BP2	B	PL 2 1/2"x30"x1'-6"	6 - 1 1/4" DIA x 18" F1554 GR 36
BP3	C	PL 2 1/4"x30"x1'-6"	SEE A5 / S5.51
BP4	B	PL 2 1/4"x30"x1'-6"	6 - 1 1/4" DIA x 24" F1554 GR 36

NOTE: FOR LATERAL BASE PLATES, PROVIDE PLATE WASHERS WITH SIZE AND THICKNESS PER AISC 360-10 TABLE 14-2.

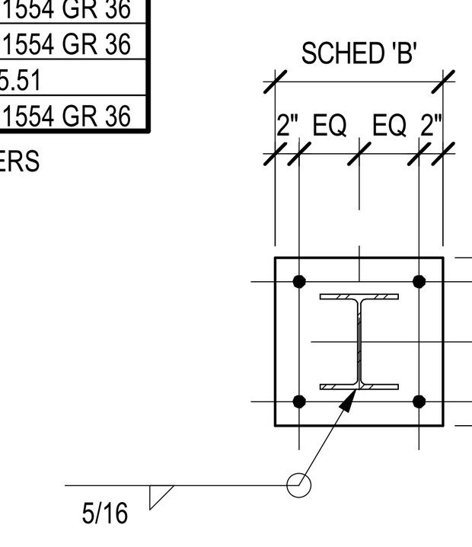


GRAVITY ANCHOR BOLT

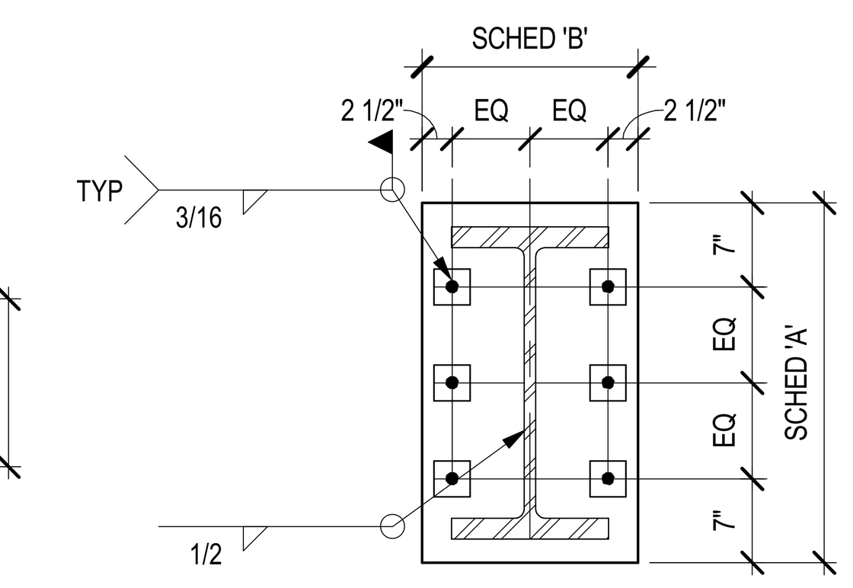


NOTE: FIRST 1'-0" OF EMBEDDED PORTION OF ANCHOR BOLT SHALL BE UNTHREADED

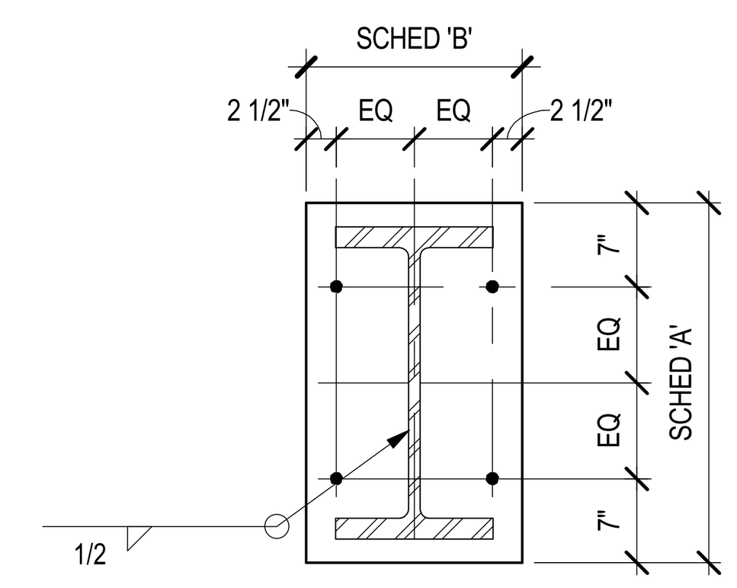
LATERAL ANCHOR BOLT



TYPE 'A'



TYPE 'B'



TYPE 'C'

REINFORCING LAP SCHEDULE								
REINFORCEMENT TYPE	#6 AND SMALLER (#db)			#7 AND LARGER (#db)			MINIMUM LENGTH (IN)	COMMENTS
	3000 PSI	4000 PSI	5000 PSI	3000 PSI	4000 PSI	5000 PSI		
CONTINUOUS WALL FOOTINGS AND STEMWALLS	30	30	30	30	30	30	18	
RETAINING WALLS AND BASEMENT WALL VERTICAL REINFORCING	57	50	45	72	62	56	12	
RETAINING WALLS AND BASEMENT WALL HORIZONTAL REINFORCING	57	50	45	72	62	56	12	
CONCRETE COLUMNS NOT SUPPORTING LATERAL FORCES	30	30	30	30	30	30	12	
CONCRETE COLUMNS SUPPORTING LATERAL FORCES	57	50	45	72	62	56	12	
TOP FLEXURAL REINFORCEMENT, INCLUDING BEAMS, GRADE BEAMS, AND COMBINED COLUMN FOOTING AT BRACED FRAME AND MOMENT FRAMES	57	50	45	72	62	56	12	
BOTTOM FLEXURAL REINFORCEMENT, INCLUDING BEAMS, GRADE BEAMS, AND COMBINED COLUMN FOOTING AT BRACED FRAME AND MOMENT FRAMES	57	50	45	55	62	56	12	
SLABS-ON-GRADE	30	30	30	30	30	30	12	
MINIMUM EMBEDMENT OF STANDARD HOOKS INTO CONCRETE BASE	16	14	12	16	14	14	6	INCREASE LENGTH FOR # 11 BARS AND LARGER BY A FACTOR OF 1.4
ALL REBAR LAPS IN CMU		72			72		12	

WALL SCHEDULE						
			REINFORCING			
MARK	VENEER	WALL	VERTICAL	HORIZONTAL	GRADE	COMMENTS
WC8	- -	8" CONC	#4 @ 12" OC	#4 @ 12" OC	A615	
WC8A	- -	8" CONC	#5 @ 9" OC	#4 @ 12" OC	A615	SEE D2/S3.11
WC16	- -	16" CONC	#7 @ 12" OC EA FACE	#5 @ 12" OC EA FACE	A615	
WC16A	- -	16" CONC	#7 @ 12" OC EA FACE	#5 @ 12" OC EA FACE	A615	
WC16B	- -	16" CONC	#5 @ 12" OC EA FACE	#5 @ 12" OC EA FACE	A615	

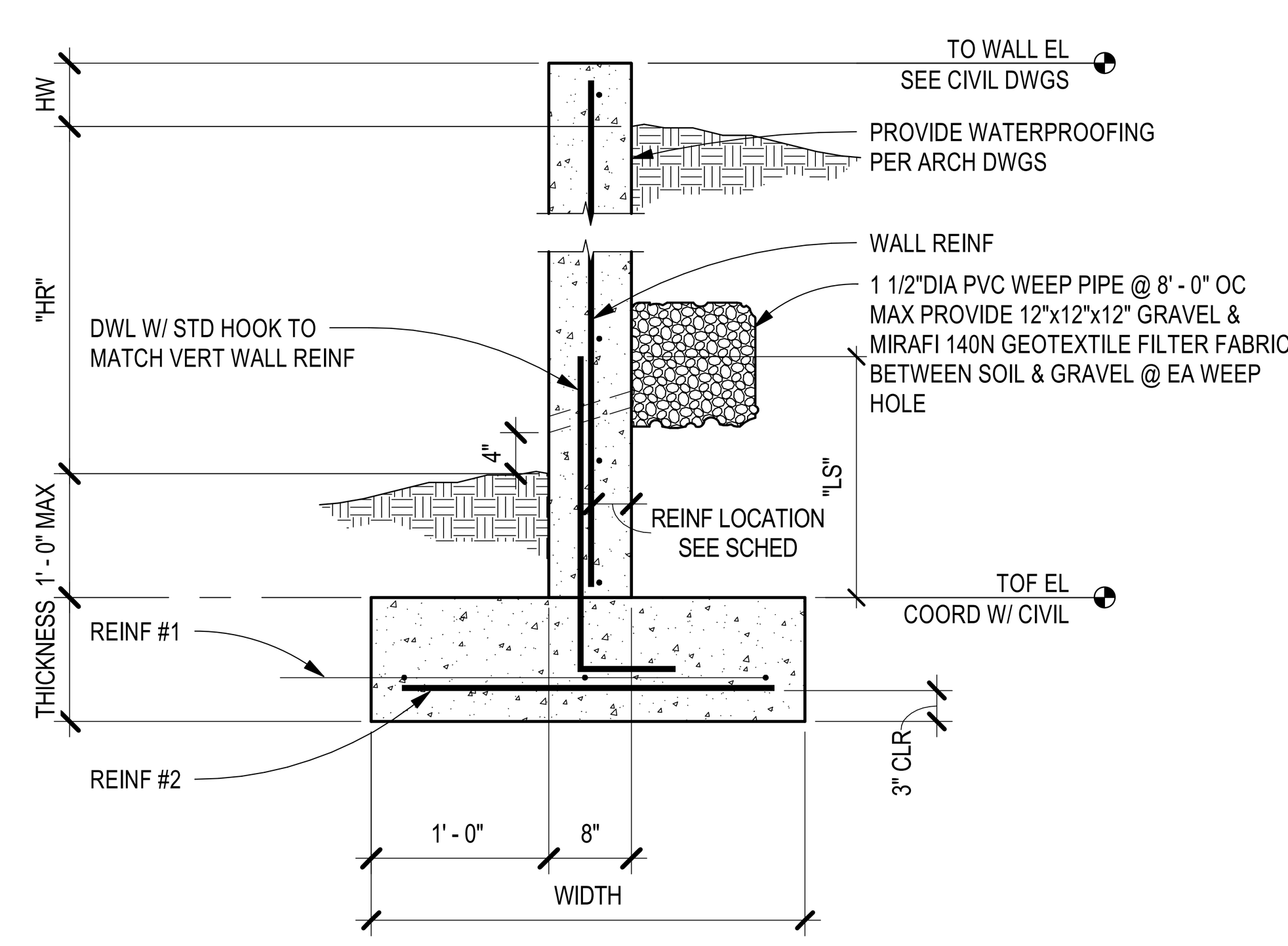
SPOT FOOTING SCHEDULE						
	SIZE			REINFORCING		
MARK	WIDTH	LENGTH	DEPTH	BOTTOM	TOP	COMMENTS
F36	3' - 0"	3' - 0"	1' - 0"	4 - #4 EA WAY	---	STD HOOK EACH END OF EACH BAR
F132	11' - 0"	11' - 0"	2' - 9"	12 - #8 EA WAY	12 - #8 EA WAY	
F144	12' - 0"	12' - 0"	2' - 9"	15 - #8 EA WAY	15 - #8 EA WAY	
F168	14' - 0"	14' - 0"	3' - 0"	18 - #9 EA WAY	18 - #9 EA WAY	
F186	15' - 6"	13' - 0"	1' - 6"	16 - #7 EA WAY	16 - #7 EA WAY	
F192	16' - 0"	16' - 0"	3' - 0"	21 - #9 EA WAY	21 - #9 EA WAY	
F216	18' - 0"	18' - 0"	3' - 6"	28 - #9 EA WAY	28 - #9 EA WAY	
F240	16' - 0"	20' - 0"	3' - 3"	38 - #9 EA WAY	38 - #9 EA WAY	
F480	12' - 0"	40' - 0"	2' - 6"	#8 @ 9" OC EA WAY	#8 @ 9" OC EA WAY	
F504	12' - 0"	42' - 0"	2' - 6"	#8 @ 6" OC EA WAY	#8 @ 6" OC EA WAY	
F528	14' - 0"	44' - 0"	2' - 9"	#8 @ 6" OC EA WAY	#8 @ 6" OC EA WAY	
F828	14' - 0"	69' - 0"	2' - 9"	#8 @ 6" OC EA WAY	#8 @ 6" OC EA WAY	

CONCRETE SITE RETAINING WALL SCHEDULE									
WALL TYPE	WALL HEIGHT ABOVE GRADE "HW"	HEIGHT RETAINED "HR"	FOOTING SIZE		FOOTING REINFORCING		WALL		
			WIDTH	THICKNESS	REINF #1	REINF #2	REINFORCING		REINF LOCATION
	FT-IN	FT-IN	FT-IN	FT-IN	NUMBER - SIZE	SIZE - SPACING	VERT SIZE - SPACING	HORIZ SIZE - SPACING	
A	0' - 0" - 0' - 6"	0' - 0" - 4' - 0"	4'-6"	1'-6"	3 - #5 CONT	#4 @ 48" OC	#5 @ 18" OC	#4 @ 12" OC	4"
B	0' - 0" - 0' - 6"	0' - 0" - 6' - 0"	7'-6"	1'-6"	9 - #5 CONT TOP & BOT	#4 @ 48" OC	#5 @ 12" OC	#4 @ 12" OC	4"
C	0' - 0" - 0 - 6"	6' - 1" - 8' - 0"	10'-6"	1'-6"	12 - #5 CONT TOP & BOT	#5 @ 18" OC	#6 @ 12" OC	#4 @ 12" OC	5.625"
D	0' - 0" - 4' - 0"	0' - 0" - 4' - 0"	3'-6"	1'-0"	3 - #5 CONT	#4 @ 48" OC T&B	#5 @ 18" OC	#4 @ 12" OC	4"
E	0' - 0" - 4' - 0"	4' - 1" - 7' - 1"	5'-0"	1'-0"	5 - #5 CONT	#5 @ 18" OC T&B	#5 @ 10" OC	#4 @ 12" OC	4"

NOTE:

COORDINATE EXACT LOCATION AND EXTENT OF WALL WITH ARCHITECTURAL AND CIVIL DWGS.

PROVIDE CONCRETE WALL VERTICAL CONTROL JOINTS AT (2) TIMES THE WALL HEIGHT AND AT ALL STEPS IN TOP OF WALL. SEE DETAIL C2/S7.11 FOR CONTROL JOINT INFORMATION.



WALL TYPE "A, B & C"

A1 TYPICAL CONCRETE SITE RETAINING WALL SCHEDULE AND DETAIL

SCALE: 3/4" = 1'-0"

James R. Childers Architect, Inc.
45 South 4th Street
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479-783-2480
www.childersarchitect.com

PROFESSIONAL SEAL:
LICENSED PROFESSIONAL ENGINEER CHRIS S. ROMERO 23578 OKLAHOMA 3.20.19

CONSULTANT LOGO:
Chavez-Grieves consulting engineers, inc.
4700 Lincoln Road NE, Suite 102 Albuquerque, NM 87109
505-344-4080 · 505-343-8759 (fax)

CLIENT: SEAL OF THE CHEROKEE NATION SEPT. 6, 1839

COLLEGE OF Osteopathic Medicine AT THE CHEROKEE NATION

TAHLEQUAH, OKLAHOMA

KEY PLAN:

PROJECT PHASE: BID PACKAGE 03

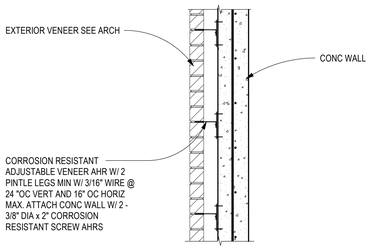
REVISIONS		
#	DATE	DESCRIPTION

DATE: 03-20-19

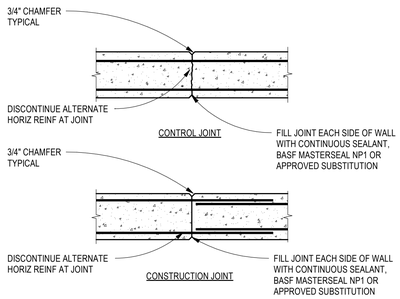
JOB NUMBER: 17-13

SHEET NUMBER: S6.01

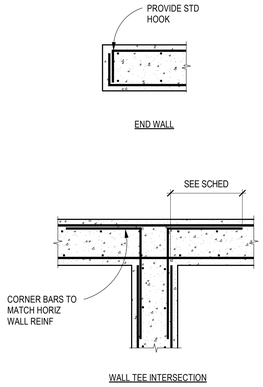
SCHEDULES

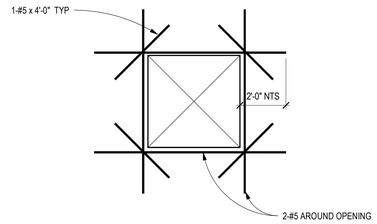


D1 TYPICAL VENEER TO CONC WALL
SCALE: NTS

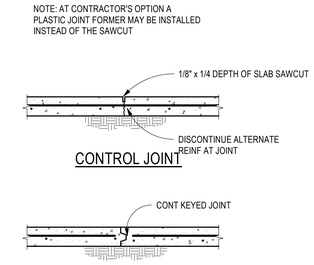


D2 TYPICAL WALL JOINT DETAIL
SCALE: NTS

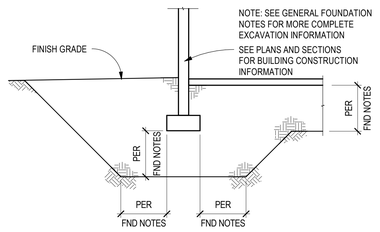




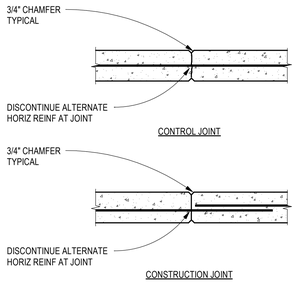
D4 TYPICAL OPNG IN CONC WALL DETAIL
SCALE: NTS



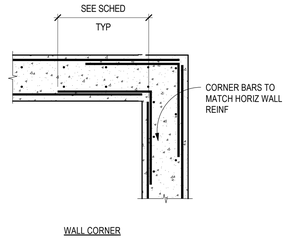
D5 TYPICAL SLAB JOINT
SCALE: NTS



C1 TYPICAL FND EXCAVATION DETAIL
SCALE: NTS

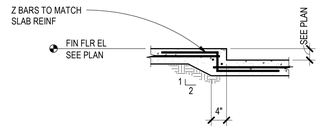


C2 TYPICAL WALL JOINT DETAIL
SCALE: NTS

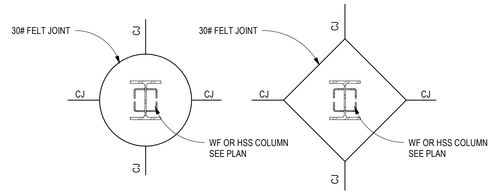


NOTE: REINFORCING SHOWN ON FOUNDATION SECTIONS AND SPECIFICALLY REFERENCED DETAILS SHALL TAKE GOVERN OVER REINFORCING IN STANDARD DETAILS.

C3 TYPICAL DOUBLE MAT WALL REINF
SCALE: NTS



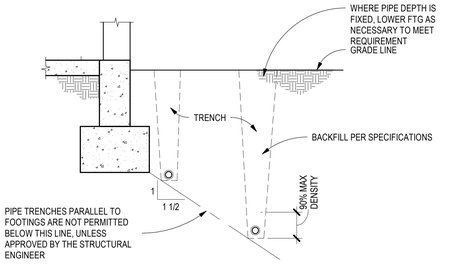
C4 TYPICAL DEPRESSED SLAB
SCALE: 3/4" = 1'-0"



NOTE : CONTRACTOR'S OPTION FOR TYPE TO BE USED. PLACE BLOCKOUTS AT ALL COLUMNS .

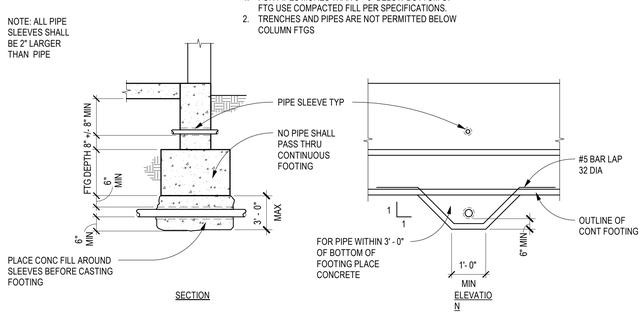
NOTE: ENLARGED BLOCKOUTS MAY BE REQUIRED AT BRACED FRAME LOCATIONS FOR GUSSET PLATE CLEARANCE.

C5 TYPICAL COLUMN BLOCKOUT
SCALE: NTS



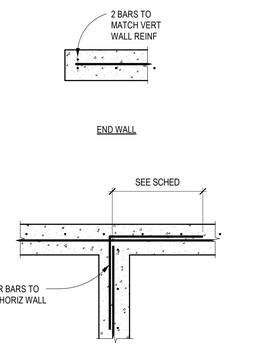
PIPE AND TRENCHES PARALLEL TO FOOTINGS

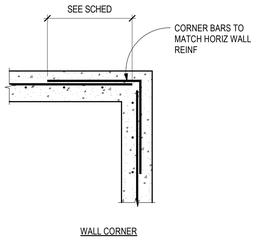
NOTES:
1. FOR PIPES MORES THAN 3' - 0" BELOW BOTTOM OF FTG USE COMPACTED FILL PER SPECIFICATIONS.
2. TRENCHES AND PIPES ARE NOT PERMITTED BELOW COLUMN FTGS



PIPE AND TRENCHES TRANSVERSE TO FOOTINGS

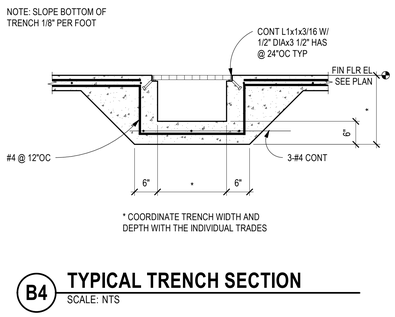
A1 TYPICAL PIPE PENETRATION AND TRENCH DETAILS
SCALE: NTS



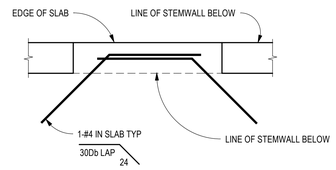


NOTE: REINFORCING SHOWN ON FOUNDATION SECTIONS AND SPECIFICALLY REFERENCED DETAILS SHALL TAKE GOVERN OVER REINFORCING IN STANDARD DETAILS.

A3 TYPICAL SINGLE MAT WALL REINF
SCALE: NTS

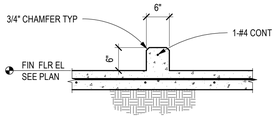


B4 TYPICAL TRENCH SECTION
SCALE: NTS

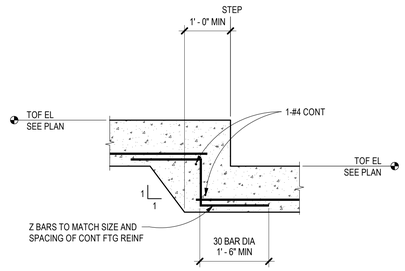


PLAN

B5 TYPICAL SLAB REINF AT OPNG
SCALE: NTS



A4 TYPICAL CURB SECTION
SCALE: NTS



A5 TYPICAL STEPPED FOOTING DETAIL
SCALE: NTS

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CONSULTANT LOGO:
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505-344-4080 505-343-8759 (fax)

CLIENT:
THE CHEROKEE NATION FIRST CAPITAL TAHLEQUAH, OK SEPT. 6, 1839

COLLEGE OF Osteopathic Medicine
AT THE CHEROKEE NATION
TAHLEQUAH, OKLAHOMA

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KEY PLAN:

PROJECT PHASE:
BID PACKAGE 03

REVISIONS:		
#	DATE	DESCRIPTION

DATE:	JOB NUMBER:
03-20-19	17-13

SHEET NUMBER:
S7.11

TYPICAL CONCRETE DETAILS