AIA® Document G710™ – 2017

Architect's Supplemental Instructions

PROJECT: *(name and address)*
17-13 OSU College of Med. at Hastings
Tahlequah, OK

CONTRACT INFORMATION:
Contract For: General Construction
Date: 4/22/19

ASI INFORMATION:
ASI Number: Bid Pack 03 - ASI 01
Date: 4/22/19

OWNER: *(name and address)*
Cherokee Nation Businesses
777 West Cherokee St.
Catoosa, OK 74015

ARCHITECT: *(name and address)*
Childers Architect
45 South 4th Street
Fort Smith, AR 72901

CONTRACTOR: *(name and address)*
Cooper / Flintco

The Contractor shall carry out the Work in accordance with the following supplemental instructions without change in Contract Sum or Contract Time. Proceeding with the Work in accordance with these instructions indicates your acknowledgment that there will be no change in the Contract Sum or Contract Time.
(Insert a detailed description of the Architect's supplemental instructions and, if applicable, attach or reference specific exhibits.)
Item 1 : See Exhibit A (Structural Revisions)
Item 2 : See additional specification sections
07 1352
07 2100
31 3116
33 4613

ISSUED BY THE ARCHITECT:

Childers Architect
ARCHITECT *(Firm name)*

J. Breck Childers
SIGNATURE

J. Breck Childers, Architect
PRINTED NAME AND TITLE

4-22-19
DATE

Bid Package 03- ASI 01 – OSU College of Osteopathic Medicine At The Cherokee Nation

Chavez-Grieves would like to incorporate the following revisions into the drawings for the above referenced project.

Sheet	Description
S0.02	Revised information based on the revised geotechnical report dated April 4, 2019.
S0.02	Revised deferred submittals content.
S0.03	Wall backfill and drainage schematic added (A1/S0.03).
S1.02	Site retaining wall added to plan, near Grid 4.
S1.02	Spread footing size at Grid J-2.5 revised.
S1.02	Spread footing size at Grid H-1, H-2, and H-3 revised.
S1.02	Spread footing size at Grid G-1, G-2, G-3, and G-4 revised.
S1.02	Spread footing size at Grid F-1, F-2, F-3, and F-4 revised.
S1.02	Spread footing size at Grid E-1, E-2, E-3, and E-4 revised.
S1.02	Location of continuous footing parallel to Grid E revised.
S1.02	Under slab French drain system information clarified, moved from general sheet note 9, to sheet keynote 8.
S1.02	Under slab French drain system schematically shown on plan.
S1.02	Location of continuous footing parallel to Grid E revised.
S1.02	Sheet keynote 9, for site retaining wall, added.
S1.02	Section at Grid F-4 revised from A1/S3.11 to D3/S3.11.
S1.02	Section A2/S3.11 near Grid H-2 added.
S1.02	Detail D1/S5.11 near Grid H-3 and H-4 added.
S1.11	Clarifying site wall information added along east side of building, near Grid 1.
S1.11	Site wall added near Grid A.
S1.11	Sheet keynotes 11 and 12 added.
S1.11	Spread footing size at Grid D-1 and D-4 revised.
S1.11	Top of pilaster elevation provided at Grid D-3.
S1.11	Spread footing size at Grid C-2 and C-3 revised.
S1.11	Spread footing size at Grid B-2 and B-3 revised.
S1.11	Spread footing size at Grid A.5-2 revised.
S1.11	Continuous footing sizes along Grid A, between Grid 2.5 and 4 revised.
S1.11	Continuous footing sizes along Grid 4, between Grid A and D revised.
S1.11	Section between Grid A-2.5 and 3 revised from A5/S3.11 to C5/S3.11.
S1.11	Section at Grid A.5-1 revised from A3/S3.11 to B1/S3.11.
S1.11	Section D5/S3.11 added near Grid D-1.
S1.11	Stud wall support clarification added at Grid D-1.
S1.12	Clarifying site wall information added along east side of building, near Grid 1.
S1.12	Detail D3/S3.12 (SIM) added along Grid H in three instances.

S3.11	Entire sheet revised.
S3.12	Detail D5 revised.
S4.02	Canopy support post size and quantity along Grid 1 revised.
S4.02	Canopy support beam sizes along Grid 1 revised.
S4.02	Thickened slab added to plan A4.
S5.11	Detail D1 added.
S6.01	Slab-on-grade schedule revised.
S6.01	Spot footing and continuous footing schedules revised.
S6.01	Detail A1 revised.

Bid Package 03- ASI 01 – OSU College of Osteopathic Medicine At The Cherokee Nation

Chavez-Grieves would like to incorporate the following revisions into the specifications for the above referenced project.

Specification / Section	Description
31 2311 / 2.1: Structural Fill Material	Update to permissible fill materials and placement of said fill materials per revised geotechnical report dated April 4, 2019.

COLLEGE OF
Osteopathic Medicine
AT THE CHEROKEE NATION

BID PACKAGE 03
(FOUNDATION)

INDEX OF DRAWINGS - BID PACAKGE 03

Sheet Number	SHEET NAME	03-20-19 - BID PACKAGE 03	04-22-19 - BID PACKAGE 03 - ASI 01
GENERAL			
G0.03	COVER / INDEX	■	■
STRUCTURAL			
S0.01	ABBREVIATIONS AND LEGEND	■	
S0.02	GENERAL STRUCTURAL NOTES	■	■
S0.03	GENERAL STRUCTURAL NOTES AND SPECIAL INSPECTION TABLES	■	■
S1.02	BASEMENT FOUNDATION PLAN - SECTOR 2	■	■
S1.10	FIRST FLOOR FRAMING PLAN - OVERALL PLAN	■	
S1.11	FIRST FLOOR FOUNDATION AND FRAMING PLAN - SECTOR 1	■	■
S1.12	FIRST FLOOR FRAMING PLAN - SECTOR 2	■	■
S1.13	FIRST FLOOR SLAB REINFORCEMENT PLAN - SECTOR 2	■	
S2.01	MOMENT FRAME ELEVATIONS	■	
S2.02	MOMENT FRAME AND BRACED FRAME ELEVATIONS	■	
S2.03	FOUNDATION PILASTER ELEVATIONS	■	
S3.01	BUILDING SECTIONS	■	
S3.02	BUILDING SECTIONS	■	
S3.11	FOUNDATION SECTIONS	■	■
S3.12	FOUNDATION SECTIONS	■	■
S4.01	ENLARGED STAIR PLANS	■	
S4.02	ENLARGED PLANS	■	■
S5.11	CONCRETE DETAILS	■	■
S5.61	TYPICAL VERTICAL CIRCULATION FOUNDATION DETAILS	■	
S6.01	SCHEDULES	■	■
S7.11	TYPICAL CONCRETE DETAILS	■	
Grand total: 22			

CARDINAL ENGINEERING
525 CENTRAL PARK DRIVE, SUITE 402
OKLAHOMA CITY, OK 73105
(405) 842-1066
CIVIL ENGINEERING

CHAVEZ-GRIEVES
4700 LINCOLN ROAD NE, SUITE 102
ALBUQUERQUE, NM 87109
(505) 344-4080
STRUCTURAL ENGINEER

HP ENGINEERING, INC.
1836 SOUTH BALTIMORE AVE.
TULSA, OK 74119
(539) 664-4618
MECHANICAL & ELECTRICAL ENGINEER

1316 E 35TH PLACE, SUITE 100
TULSA, OK 74105
(918) 382-9120
EQUIPMENT PLANNER

HKS
1000 MACON ST, SUITE 150
FORT WORTH, TX 76102
(817) 348-0330
ARCHITECTURAL HEALTHCARE PLANNING

WSP
808 TRAVIS STREET, SUITE 200
HOUSTON, TX 77002
(281) 589-5900
FIRE PROTECTION / LIFE SAFETY

James R. Childers Architect, Inc.
45 South 4th Street
Fort Smith, AR 72901
479-783-2480
www.childersarchitect.com

PROFESSIONAL SEAL:

CONSULTANT LOGO:

CLIENT:

COLLEGE OF Osteopathic Medicine AT THE CHEROKEE NATION
TAHLEQUAH, OKLAHOMA

KEY PLAN:

PROJECT PHASE:
BID PACKAGE 03

REVISIONS		
#	DATE	DESCRIPTION
1	4/22/19	BID PACKAGE 03 ASI 01

DATE: 03/20/19
JOB NUMBER: 17-13
SHEET NUMBER: G0.03
COVER / INDEX

1/8" = 1'-0"
3/4" = 1'-0"
1/2" = 1'-0"
3/8" = 1'-0"
1/4" = 1'-0"
1" = 1'-0"
3" = 1'-0"
1 1/2" = 1'-0"

GENERAL STRUCTURAL NOTES

FOUNDATION NOTES

GENERAL:

A SUBSURFACE SOIL INVESTIGATION HAS BEEN MADE BY PALMERTON AND PARRISH, INC., PROJECT NO. 255932.

A REPORT OF THAT INVESTIGATION DATED 1/30/19 AND REV 1 DATED APRIL 4, 2019 IS AVAILABLE FOR VIEWING IN THE PROJECT MANUAL.

THE FOUNDATION SYSTEM FOR THIS PROJECT IS SPREAD FOOTINGS OVER ENGINEERED FILL AND/OR STIFF OR DENSE NATURAL SOILS.

1

ADDITIONAL INFORMATION CONCERNING SPECIFIC SOIL CONDITIONS TO BE ENCOUNTERED IS AVAILABLE IN THE SOILS REPORTS AND SHALL BE REVIEWED BY THE CONTRACTOR.

FIELD OBSERVATION AND TESTS:

THE OWNER SHALL EMPLOY THE SERVICES OF A REGISTERED, LICENSED GEOTECHNICAL ENGINEER TO OBSERVE ALL CONTROLLED EARTHWORK. THE GEOTECHNICAL ENGINEER SHALL PROVIDE CONTINUOUS ON-SITE OBSERVATION BY EXPERIENCED PERSONNEL DURING CONSTRUCTION OF CONTROLLED EARTHWORK. THE CONTRACTOR SHALL NOTIFY THE GEOTECHNICAL ENGINEER AT LEAST TWO WORKING DAYS IN ADVANCE OF ANY FIELD OPERATIONS OF THE CONTROLLED EARTHWORK.

TESTS OF MATERIALS SHALL BE MADE AT THE FOLLOWING MINIMUM RATES. THE ON-SITE GEOTECHNICAL ENGINEER SHALL DETERMINE THE ACTUAL TESTING RATES:

ONE FIELD DENSITY TEST PER 2500 SQUARE FEET OF COMPACTED SUBGRADE, PRIOR TO PLACING STRUCTURAL FILL OR SLAB-ON-GRADE, WITH A MINIMUM OF 3 TESTS.

ONE FIELD DENSITY TEST PER 2500 SQUARE FEET OF STRUCTURAL FILL PLACED OR EACH HORIZONTAL LAYER OF STRUCTURAL FILL, WHICHEVER IS GREATER.

ONE MOISTURE-DENSITY CURVE FOR EACH TYPE OF MATERIAL USED, AS INDICATED BY THE SIEVE ANALYSIS AND THE PLASTICITY INDEX.

THE GEOTECHNICAL ENGINEER SHALL SUBMIT THE RESULTS OF ALL REQUIRED TESTS.

CLEARING AND GRUBBING:

REMOVE ALL EXISTING FOUNDATIONS, CONCRETE SLAB,VEGETATIVE MATTER, STUMPS, MATTED ROOTS AND ROOTS BRUSH, AND RUBBISH FROM THE CONSTRUCTION AREA.

REMOVE ALL TOPSOIL FROM THE CONSTRUCTION AREA. TOPSOIL/VEGETATIVE MATTER STRIPPING ON THE ORDER OF 3-INCHES SHOULD BE ANTICIPATED IN GRASS COVERED AREAS. THIS MATERIAL SHALL NOT BE USED AS FILL MATERIAL, BUT MAY BE STOCKPILED AND LATER USED IN LAWN AND LANDSCAPE AREAS ONLY.

SITE, SUBFLOOR AND BEARING SURFACE PREPARATION:

A REPRESENTATIVE OF THE GEOTECHNICAL ENGINEER SHALL BE PRESENT TO CONFIRM COMPLETE EXCAVATION OF ANY UNCONTROLLED FILL, PROOF ROLLING AND CONSTRUCTION MATERIALS TESTING..

OVEREXCAVATE ALL SOILS UNDERLYING FOOTINGS AND FLOOR SLAB AND ALL UNCONTROLLED FILL TO A MINIMUM DEPTH OF 2 FEET OR TO GRAVELLY CLAYS/CLAYEY GRAVELS, WHICHEVER IS SHALLOWER, AND REPLACE WITH LVC FILL MATERIAL.

OVEREXCAVATE ALL SOILS UNDERLYING SITE RETAINING WALL FOOTINGS TO A MINIMUM DEPTH OF 2 FEET OR TO GRAVELLY CLAYS/CLAYEY GRAVELS, WHICHEVER IS SHALLOWER, AND REPLACE WITH LVC FILL MATERIAL.

ONCE THE BASEMENT EXCAVATION AND UNDERCUTTING OF EXISTING SOILS HAS BEEN COMPLETED THE BUILDING FOOTPRINT SOILS SHALL BE SCANNED WITH GROUND PENETRATING RADAR, GPR, TO SEARCH FOR ANY LARGE SHALLOW SURFACE VOIDS. THE GPR SCANNING SHALL BE PERFORMED BY THE GEOTECHNICAL ENGINEER OF RECORD FOR THE PROJECT.

AFTER GPR SCANNING AND WRITTEN APPROVAL THAT NO VOIDS EXIST, ALL UNDERCUT SUBGRADE SOILS SHALL BE PROOF ROLLED WITH A FULLY LOADED TANDEM AXLE DUMP TRUCK OR SIMILAR RUBBER TIRED CONSTRUCTION EQUIPMENT AND ANY AREAS WHICH RUT OR DEFLECT DURING ROLLING NOTED. ALL SOFT AREAS, IF ANY, IDENTIFIED DURING PROOF-ROLLING SHALL BE UNDERCUT AND REPLACED WITH COMPACTED FILL.

AFTER EVALUATION OF PROOF-ROLLING AND APPROVAL, SCARIFY ALL EXPOSED SUBGRADE SOILS TO A DEPTH OF 8 INCHES, MOISTEN TO OPTIMUM MOISTURE CONTENT (+/- 2%, OR 0 TO 4% FOR CH SOIL TYPES) AND COMPACT TO THE DENSITY SPECIFIED HEREINAFTER.

PLACE ALL STRUCTURAL FILL IN APPROXIMATELY HORIZONTAL LAYERS NOT GREATER THAN 12 INCHES IN LOOSE THICKNESS, MOISTEN TO OPTIMUM MOISTURE CONTENT (+/- 2% OR 0 TO 4% FOR CH SOIL TYPES) AND COMPACT TO DENSITY SPECIFIED HEREINAFTER.

ALL EARTHWORK FOR THE BUILDING PAD SHALL EXTEND A MINIMUM OF 5 FEET BEYOND THE PERIMETER FOOTINGS.

ENGINEERED FILL REQUIREMENTS:

LOW PLASTICITY COHESIVE SOIL OR GRANULAR FILL PER GEOTECHNICAL REPORT:

GRADATION (ASTM D422):

SIEVE SIZE	PERCENT PASSING BY WEIGHT
12"	100
NO. 200	</= 85%

LIQUID LIMIT (ASTM D4318): 50 MAXIMUM

MATERIAL LARGER THAN 12 INCHES SHALL NOT BE PLACED IN THE STRUCTURAL FILL, AND MATERIAL LARGER THAN 4 INCHES SHALL NOT BE PLACED WITHIN TWELVE INCHES OF THE BEARING SURFACES OF SLABS OR FOUNDATIONS.

NO BRUSH, SOD, FROZEN MATERIAL OR OTHER UNSUITABLE MATERIAL SHALL BE PLACED IN THE STRUCTURAL FILL. MATERIAL SHALL BE PLACED IN SUCH A MANNER AS TO RESULT IN A UNIFORMLY COMPACTED FILL.

BASED ON THE REQUIREMENTS FOR THE STRUCTURAL FILL AND THE DESCRIPTION OF THE EXISTING SITE SOILS IN THE PROJECT GEOTECHNICAL REPORT, THE CONTRACTOR IS RESPONSIBLE FOR DETERMINING THE MOST APPROPRIATE METHOD FOR PROVIDING THE REQUIRED STRUCTURAL FILL. DEPENDING ON THE SITE CONDITIONS, APPROPRIATE METHODS COULD INCLUDE REBLENDING OF THE EXISTING SITE SOILS, MIXING THE EXISTING SITE SOILS WITH IMPORTED FILL, OR REMOVING THE EXISTING SITE SOILS ENTIRELY AND REPLACING WITH IMPORTED FILL. SEE SPECIFIC REQUIREMENTS IN GEOTECHNICAL REPORT FOR USE OF ON-SITE SOILS/FILL.

ALL CONTINUOUS FOUNDATION WALLS (E.G., STEM WALLS AND BASEMENT WALLS) SHALL BE BACKFILLED WITH CLEAN CRUSHED STONE OR ROCK FILL IN THE MANNER SPECIFIED IN THE REVISED GEOTECHNICAL REPORT DATED APRIL 4, 2019. SEE A1/S0.03 FOR BACKFILL AND WALL DRAINAGE SCHEMATIC. CONTINUOUS WALL DRAINAGE IS NOT REQUIRED FOR FOUNDATION WALLS RETAINING SOIL NOT EXPOSED TO EXTERIOR CONDITIONS.

EXTERIOR SITE RETAINING WALLS ARE PERMITTED TO BE BACKFILLED WITH SUITABLE FILL MATERIAL OTHER THAN CLEAN CRUSHED STONE OR ROCK FILL.

NOTE: THESE BACKFILL REQUIREMENTS ALSO APPLY TO THE INSIDE FACE OF STEMWALLS ALONG GRID 4 BETWEEN A AND D.5, ALONG GRID A, AND ALONG GRID 1 BETWEEN D.5 AND A.

1

GRANULAR BASE COURSE REQUIREMENTS:

GRADATION (ASTM C136):

SIEVE SIZE	PERCENT PASSING BY WEIGHT
1"	100
3/4"	85-100
NO. 4	45-95
NO. 200	0-8

PLASTICITY INDEX (ASTM D4318): 3 MAXIMUM

THE COURSE AGGREGATE SHALL HAVE A PERCENT WEAR OF 50 OR LESS WHEN TESTED IN ACCORDANCE WITH ASTM C131.

THE FINISHED TOP SURFACE SHOULD BE FLAT AND LEVEL WITH SUFFICIENT FINES TO FILL BETWEEN COARSE AGGREGATE. IF THIS IS NOT THE CASE, PROVIDE UP TO 1/2 INCH BLOTTER LAYER OF SAND.

COMPACTION REQUIREMENTS:

IN ACCORDANCE WITH ASTM D698 (STANDARD PROCTOR), SUBGRADE SOILS AND STRUCTURAL FILL MATERIALS SHALL BE COMPACTED TO THE FOLLOWING PERCENTAGES OF THE MAXIMUM DRY DENSITY AT +/- 2% (OR 0 TO 4% FOR CH SOIL TYPES) OPTIMUM MOISTURE CONTENT:

MATERIAL	MINIMUM PERCENT COMPACTION
STRUCTURAL FILL IN THE BUILDING AREA	95
SUBBASE FOR SLAB SUPPORT	95
SUBGRADE BELOW STRUCTURAL FILL	95
MISCELLANEOUS BACKFILL	90

GENERAL STRUCTURAL NOTES

SITE RETAINING WALL DESIGN CRITERIA:

LOADING CONDITION	EQUIVALENT FLUID PRESSURE
ACTIVE EARTH PRESSURE FOR ON SITE SOIL BACKFILL	35 PCF UNRESTRAINED
ULTIMATE PASSIVE EARTH PRESSURE	460 PCF
EARTH PRESSURE AT REST	50 PCF UNRESTRAINED
ULTIMATE SOIL FRICTION FACTOR	0.52
SOIL BEARING CAPACITY	4500 PSF

1

CODES AND MANUALS:

IBC-15 INTERNATIONAL BUILDING CODE 2015
ASCE/SEI 3-91 STRUCTURAL DESIGN OF COMPOSITE SLABS
ASCE/SEI 7-10 MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES
AISC 360-10 SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS
AISC 341-10 SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS
AISC MANUAL OF STEEL CONSTRUCTION 14TH EDITION
SDI DIAPHRAGM DESIGN MANUAL, 3RD EDITION
ANSI/SDI NC1.0-06 STANDARD FOR NONCOMPOSITE STEEL FLOOR DECK
ANSI/SDI RD1.0-06 STANDARD FOR STEEL ROOF DECK
ANSI/SDI C1.0-06 STANDARD FOR COMPOSITE STEEL FLOOR DECK
AISI S100-12 NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS
AISI S200-12 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING – GENERAL PROVISIONS
AISI S210-12 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING – FLOOR AND ROOF SYSTEM DESIGN
AISI S211-07 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING – WALL STUD DESIGN WITH 2012 SUPPLEMENT
AISI S212-12 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING – HEADER DESIGN
AISI S213-12 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING – LATERAL DESIGN WITH 2010 SUPPLEMENT
AISI S214-12 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING – TRUSS DESIGN, WITH SUPPLEMENT 2, DATED 2008
ACI 318-14 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE
ACI 530-13 BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES
ACI 530.1-13 SPECIFICATIONS FOR MASONRY STRUCTURES
AWS D1.1-04 STRUCTURAL WELDING CODE – STEEL
AWS D1.3-98 STRUCTURAL WELDING CODE – SHEET STEEL
AWS D1.4-11 STRUCTURAL WELDING CODE – REINFORCING STEEL

DESIGN CRITERIA:

VERTICAL:

LIVE LOAD	
FLOOR	80 PSF
STAIRS AND EXIT-WAYS*	100 PSF
*MINIMUM CONCENTRATED LOAD	300 LBS
ASSEMBLY AREAS	100 PSF
SAFER ROOM	100 PSF
STORAGE	150 PSF
LIBRARY/ BOOKSTORE	150 PSF
GYM	150 PSF
ADDITIONAL SUPERIMPOSED LOADS	
PARTITIONS	15 PSF
SUSPENDED EQUIPMENT	10 PSF
CONCENTRATED LOAD (PER IBC 1607.4)	2000 LBS
ROOF LIVE LOAD: LR = 20*R1*R2	20 PSF
REDUCTION FACTOR BASED ON TRIB AREA	R1 = 1.0
REDUCTION FACTOR BASED ON ROOF SLOPE	R2 = 1.0
SNOW LOAD	
GROUND SNOW LOAD	PG = 10 PSF
FLAT ROOF SNOW LOAD**	PF = 16 PSF
SNOW EXPOSURE FACTOR	CE = 0.9
SNOW LOAD IMPORTANCE FACTOR	IS = 1.10
THERMAL FACTOR	CT = 1.0
**INCLUDES 5 PSF RAIN-ON SNOW SURCHARGE LOAD	

HORIZONTAL:

WIND	
ULTIMATE DESIGN WIND SPEED	120 MPH
RISK CATEGORY	III
EXPOSURE	C
INTERNAL PRESSURE COEFFICIENT	GCPI = 0.18
NATURAL FREQUENCY	0.685
STRUCTURE IS FLEXIBLE	
SEISMIC	
SEISMIC IMPORTANCE FACTOR	IS = 1.25
MAPPED SPECTRAL RESPONSE ACCELERATIONS	
SHORT PERIOD	SS = 0.152G
1 SECOND PERIOD	S1 = 0.081G
SITE CLASS	C
SPECTRAL RESPONSE COEFFICIENTS	
SHORT PERIOD	SDS = 0.122G
1 SECOND PERIOD	SD1 = 0.092G
SEISMIC DESIGN CATEGORY	B
BASIC SEISMIC FORCE RESISTING SYSTEM:	
STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE	
SEISMIC RESPONSE COEFFICIENT	CS = 0.043
RESPONSE MODIFICATION FACTOR	R = 3
DESIGN BASE SHEAR	V = 0.043W
ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE	
ALLOWABLE SOIL BEARING PRESSURE =	SPREAD FOOTINGS: 5000 PSF CONTINUOUS FOOTINGS: 4500 PSF
FROST DEPTH =	24 INCHES
FUTURE BUILDING EXPANSION:	NONE

1

GENERAL:

STRUCTURAL DRAWINGS ARE NOT STAND-ALONE DOCUMENTS AND ARE INTENDED TO BE USED IN CONJUNCTION WITH CIVIL, ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND DRAWINGS FROM OTHER DISCIPLINES. THE CONTRACTOR SHALL COORDINATE ALL REQUIREMENTS OF THE CONTRACT DOCUMENTS INTO THE SHOP DRAWINGS AND FIELD WORK.

COORDINATE DIMENSIONS OF ALL OPENINGS, DEPRESSIONS, BLOCKOUTS, ETC. WITH ARCHITECTURAL DRAWINGS, DRAWINGS FROM OTHER DISCIPLINES, PROJECT SHOP DRAWINGS, AND FIELD CONDITIONS PRIOR TO SHOP DRAWING SUBMITTAL. THE STRUCTURAL DRAWINGS ONLY REPRESENT A PORTION OF THE REQUIREMENTS FOR THE PROJECT.

SEE ARCHITECTURAL PLANS FOR INTERIOR NON-BEARING PARTITION WALLS. PARTITION FRAMING SHALL BE CONNECTED TO THE PRIMARY STRUCTURE TO ALLOW FOR VERTICAL LIVE LOAD DEFLECTIONS OF SPAN/360 FOR FLOOR FRAMING AND SPAN/240 FOR ROOF FRAMING.

CONTRACTOR'S CONSTRUCTION AND/OR ERECTION SEQUENCES SHALL RECOGNIZE AND CONSIDER THE EFFECTS OF THERMAL MOVEMENTS OF STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PERIOD.

THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS IN THE FIELD.

SHOP DRAWINGS SHALL BE FURNISHED AND REVIEWED BEFORE ANY FABRICATION OR ERECTION IS STARTED. THE CONTRACTOR SHALL REVIEW AND APPROVE SHOP DRAWINGS PRIOR TO SUBMITTAL TO THE ARCHITECT FOR REVIEW. POORLY EXECUTED SHOP DRAWINGS WILL BE REJECTED AND SHALL BE RESUBMITTED.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING SAFE AND ADEQUATE SHORING FOR ALL PARTS OF THE STRUCTURE DURING CONSTRUCTION.

TEMPORARY PROVISIONS SHALL BE MADE FOR STRUCTURAL STABILITY DURING CONSTRUCTION. THE STRUCTURE SHOWN ON THE DRAWINGS HAS BEEN DESIGNED FOR STABILITY UNDER FINAL CONFIGURATION.

NOTCHING OR CUTTING ANY STRUCTURAL MEMBER IN THE FIELD IS PROHIBITED.

THE CONTRACTOR SHALL VERIFY THE SIZE AND LOCATION OF FOUNDATIONS UNDER MECHANICAL AND ELECTRICAL EQUIPMENT AS REQUIRED. NO CONCRETE PADS SHALL BE LOCATED ON ROOF UNLESS SHOWN ON THE STRUCTURAL DRAWINGS.

BACKFILL SHALL NOT BE PLACED BEHIND RETAINING WALLS UNTIL CONCRETE HAS ATTAINED 100 PERCENT OF DESIGN STRENGTH.

BACKFILL SHALL NOT BE PLACED BEHIND BASEMENT WALLS UNTIL THE CONCRETE HAS ATTAINED 100 PERCENT OF DESIGN STRENGTH AND THE ELEVATED FLOOR PROVIDING LATERAL SUPPORT AT THE TOP OF THE WALL IS COMPLETELY CONSTRUCTED, OR TEMPORARY BRACING/SHORING OF THE WALL IS PROVIDED. DESIGN OF ANY TEMPORARY WALL BRACING/SHORING IS THE RESPONSIBILITY OF THE CONTRACTOR.

REMOVAL OF FORMS AND SHORING SHALL BE IN ACCORDANCE WITH ACI 347. WHERE CONCRETE MUST SUPPORT SUPERIMPOSED LOADS PRIOR TO ATTAINING THE SPECIFIED DESIGN STRENGTH, RESHORE CONCRETE IN ACCORDANCE WITH ACI 347. RESHORING SHALL NOT BE REMOVED SOONER THAN 28 DAYS FROM THE DATE OF POUR OR UNTIL CONCRETE HAS ATTAINED THE SPECIFIED DESIGN STRENGTH.

THE CONTRACTOR SHALL SUBMIT FOR PRIOR APPROVAL THE END OF POUR LOCATIONS FOR CONCRETE GRADE BEAMS, CONCRETE COLUMNS, AND CONCRETE BEAMS.

GENERAL STRUCTURAL NOTES

THE CONTRACTOR SHALL BE RESPONSIBLE FOR ADHERING TO ALL APPLICABLE STANDARDS SET FORTH BY OSHA, INCLUDING THE FOLLOWING REQUIREMENTS FROM STANDARDS - 29 CFR, SECTION 1926, SUBPART R:

A. THE STEEL ERECTION CONTRACTOR SHALL NOT ERECT STEEL UNLESS THEY HAVE RECEIVED WRITTEN NOTIFICATION FROM THE CONTRACTOR THAT THE CONCRETE IN THE FOOTINGS, PIERS AND WALLS OR THE MORTAR IN THE MASONRY PIERS AND WALLS HAS ATTAINED, ON THE BASIS OF AN APPROPRIATE ASTM STANDARD TEST METHOD OF FIELD-CURED SAMPLES, EITHER 75 PERCENT OF THE INTENDED MINIMUM COMPRESSIVE DESIGN STRENGTH OR SUFFICIENT STRENGTH TO SUPPORT THE LOADS IMPOSED DURING STEEL ERECTION.

   PROVIDE STRUCTURAL ENGINEER A COPY OF WRITTEN NOTIFICATION WHEN IT IS PROVIDED TO THE STEEL ERECTOR.

B. ANCHOR RODS (ANCHOR BOLTS) SHALL NOT BE REPAIRED, REPLACED OR FIELD-MODIFIED WITHOUT THE APPROVAL OF THE PROJECT STRUCTURAL ENGINEER OF RECORD.

   PRIOR TO ERECTION OF COLUMNS, THE CONTRACTOR SHALL PROVIDE WRITTEN NOTIFICATION TO THE STEEL ERECTOR IF THERE HAS BEEN ANY REPAIR, REPLACEMENT OR MODIFICATION OF THE ANCHOR RODS (ANCHOR BOLTS).

   PROVIDE STRUCTURAL ENGINEER A COPY OF WRITTEN NOTIFICATION WHEN IT IS PROVIDED TO THE STEEL ERECTOR.

C. NO MODIFICATION THAT AFFECTS THE STRENGTH OF A STEEL JOIST OR STEEL JOIST GIRDER SHALL BE MADE WITHOUT THE APPROVAL OF THE PROJECT STRUCTURAL ENGINEER OF RECORD.

D. METAL DECKING HOLES AND OPENINGS SHALL NOT BE CUT UNTIL IMMEDIATELY PRIOR TO BEING PERMANENTLY FILLED WITH THE EQUIPMENT OR STRUCTURE, OR SHALL BE IMMEDIATELY COVERED.

PROTECTION: PROPER PRECAUTIONS SHALL BE TAKEN AT ALL TIMES TO PROTECT VEHICULAR AND PEDESTRIAN TRAFFIC FROM ANY DAMAGE OR INJURY WHICH MAY BE CAUSED, EITHER DIRECTLY OR INDIRECTLY, BY THE WORK INCLUDED ON THESE DRAWINGS. SUCH PRECAUTIONS SHALL INCLUDE THE ERECTION AND MAINTENANCE OF FENCES, BARRICADES, RAILINGS, GUARDS, SIGNS, COVERINGS, LIGHTS, AND OTHER PRECAUTIONS AS MAY BE REQUIRED. IF AT ANY TIME, IN THE OPINION OF THE OWNER OR THE OWNER'S REPRESENTATIVE, PROPER PRECAUTIONS ARE NOT BEING TAKEN TO SECURE THIS PROTECTION, THE CONTRACTOR SHALL AT NO ADDITIONAL COST TO THE OWNER, INSTALL AND MAINTAIN SUCH ADDITIONAL PROTECTION AS MAY BE DIRECTED BY THE OWNER.

POLLUTION CONTROLS: USE WATER SPRINKLING, TEMPORARY ENCLOSURES, AND OTHER SUITABLE METHODS TO LIMIT DUST AND DIRT RISING AND SCATTERING IN THE AIR TO LOWEST PRACTICAL LEVEL. COMPLY WITH GOVERNING REGULATIONS PERTAINING TO ENVIRONMENTAL PROTECTION.

TYPICAL DETAIL SHEETS:

THE S7.00 SERIES SHEETS IN THESE DRAWINGS CONTAIN TYPICAL STRUCTURAL DETAILS FOR VARIOUS BUILDING MATERIALS. SOME OF THESE DETAILS MAY NOT BE PART OF THIS PROJECT.

DRAWINGS:

DO NOT SCALE DRAWINGS.

WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL NOTES, AND SPECIFICATIONS, THE MORE STRINGENT REQUIREMENTS SHALL GOVERN. DETAILS ON DRAWINGS TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS. DETAILS NOTED "TYPICAL" APPLY TO ALL SIMILAR CONDITIONS. WHERE NO SPECIFIC DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ELSEWHERE ON THE PROJECT.

FAST-TRACK/PHASED CONSTRUCTION:

THE STRUCTURAL PORTION OF THIS PROJECT IS BEING DESIGNED, BID, PERMITTED, AND CONSTRUCTED PRIOR TO THE COMPLETION OF ARCHITECTURAL, ENGINEERING, AND OTHER DESIGN TEAM CONSTRUCTION DOCUMENTS. THE OWNER, ARCHITECT, AND CONTRACTOR SHALL BE AWARE THAT THIS ACCELERATED STRUCTURAL SCHEDULE CREATES INHERENT RISK OF FUTURE CHANGES DUE TO DESIGN COORDINATION WITH OTHER DISCIPLINES. WHILE EVERY EFFORT HAS BEEN MADE TO MINIMIZE THESE CHANGES, THE RISK OF ADDED COSTS DUE TO THESE CHANGES SHALL BE UNDERSTOOD AND ACCEPTED BY ALL PARTIES.

DRAWINGS THAT DO NOT HAVE AN ENGINEERING SEAL BY THE STRUCTURAL ENGINEER OF RECORD OR NOT LABELED AS CONSTRUCTION DRAWINGS ARE PRELIMINARY AND SUBJECT TO CHANGE. IF THESE DOCUMENTS ARE BEING USED FOR PRICING, BIDDING, STEEL MILL ORDER, OR PREPARATION OF SHOP DRAWINGS, THE CONTRACTOR SHALL ANTICIPATE FUTURE DRAWING REVISIONS THAT MAY AFFECT THIS WORK OR INCREASE CONSTRUCTION COSTS. THE STRUCTURAL ENGINEER SHALL NOT BE RESPONSIBLE FOR ANY CHANGE ORDER COSTS INCURRED DUE TO THESE DRAWING REVISIONS, AND THE CONTRACTOR SHALL CONSIDER THESE ANTICIPATED COSTS IN ANY BIDS OR PRICE GUARANTEES TO THE OWNER.

USE THE MOST CURRENT SET OF DRAWINGS IN PREPARATION OF ALL SUBMITTALS. ALL SUBMITTALS SHALL LIST THE DATE OF THE DRAWINGS USED TO PREPARE THE SUBMITTAL. SUBMITTALS PREPARED FROM OUTDATED DRAWINGS MAY BE REJECTED. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ACQUIRING THE LATEST SET OF CONSTRUCTION DRAWINGS AND DISTRIBUTING TO THE APPROPRIATE PARTIES.

CAST-IN-PLACE CONCRETE:

ALL CONCRETE SHALL CONFORM TO THE SPECIFICATIONS FOR STRUCTURAL CONCRETE, ACI 301-10.

ALL EXPOSED EDGES OF CONCRETE SHALL HAVE A 3/4" CHAMFER UNLESS NOTED OTHERWISE.

NORMALWEIGHT CONCRETE:

A. F'C = 4500 PSI @ 28 DAYS – ALL CONCRETE EXPOSED TO FREEZE/THAW CYCLES AND OCCASIONAL MOISTURE, INCLUDING CONCRETE FLAT WORK, EXPOSED BUILDING STEM WALLS, SITE WALLS, ETC… EXTERIOR CONCRETE SHALL MEET EXPOSURE CATEGORY AND CLASS F1 ACCORDING TO ACI 318 TABLE 19.3.1.1.
B. F'C = 3000 PSI @ 28 DAYS - ALL FOOTINGS, TIE BEAMS, GRADE BEAMS.
C. F'C = 3000 PSI @ 28 DAYS - ALL INTERIOR SLABS ON GRADE, UNLESS NOTED OTHERWISE.
D. F'C = 3500 PSI @ 28 DAYS - ALL CONCRETE FILL OVER METAL DECK, UNLESS NOTED OTHERWISE.
E. F'C = 4000 PSI @ 28 DAYS – ALL CAST-IN-PLACE CONCRETE COLUMNS, PEDESTALS, RETAINING WALLS, AND ELEVATED BEAMS.
F. F'C = 4000 PSI @ 28 DAYS – ALL ELEVATED CAST-IN-PLACE SLABS.
G. F'C = 4000 PSI @ 28 DAYS – ALL SLABS ON GRADE AND ELEVATED SLABS TO RECEIVE POLISHED CONCRETE FINISH.

FIRE RATED SLABS: COORDINATE AIR CONTENT REQUIREMENTS WITH ARCHITECTURAL DRAWINGS.

CONCRETE MIX DESIGNS (INCLUDING AIR CONTENT, WATER TO CEMENT RATIOS, AND OTHER CRITERIA) SHALL CONFORM TO THE REQUIREMENTS SET FORTH IN ACI 318 TABLE 19.3.2.1, BASED ON THE EXPOSURE CATEGORIES AND CLASSES DEFINED IN ACI 318 TABLE 19.3.1.1. USE AIR ENTRAINING ADMIXTURE IN ALL EXTERIOR CONCRETE. AIR CONTENT IN FIRE RATED SLABS SHALL ALSO COMPLY WITH THE REQUIREMENTS IN THE SPECIFIED UL LISTING.

CRYSTALLINE WATERPROOFING SHALL BE BASF, W.R. MEADOWS ADMIXTURE OR APPROVED EQUAL.

CRYSTALLINE WATERPROOFING SHALL BE ADDED TO THE CONCRETE MIX PER THE MANUFACTURER'S RECOMMENDATIONS FOR ALL BASEMENT WALL CONCRETE.

1

COLD WEATHER CONCRETING: PROTECT CONCRETE WORK FROM PHYSICAL DAMAGE OR REDUCED STRENGTH CAUSED BY FROST, FREEZING OR LOW TEMPERATURES. COMPLY WITH ACI 306.1.

HOT WEATHER CONCRETING: WHEN HOT WEATHER CONDITIONS EXIST THAT WOULD IMPAIR THE QUALITY AND STRENGTH OF THE CONCRETE, REDUCE DELIVERY TIME OF READY MIX CONCRETE, LOWER THE TEMPERATURE OF MATERIALS, OR ADD RETARDER TO ENSURE THAT THE CONCRETE IS PLASTIC. RETEMPERING WITH WATER IS NOT ALLOWED. COMPLY WITH ACI 305R.

SLAB CURING: ALL INTERIOR CONCRETE SLABS, EXCEPT EXPOSED INTEGRALLY COLORED SLABS, ARE TO BE CURED WITH A MOISTURE RETAINING COVER FOR THE FIRST 7 DAYS (MINIMUM) AFTER PLACEMENT.

THE CONTRACTOR IS ALLOWED TO CAST FOUNDATIONS AGAINST EXCAVATED SOIL SURFACES, PROVIDED THE FOLLOWING IS ADHERED TO:

A. THE SIDE SLOPES OF THE EXCAVATION SHALL BE ABLE TO MAINTAIN VERTICAL SLOPE WITHOUT SOIL SLOUGHAGE.
B. THE BOTTOM WIDTH OF THE EXCAVATION SHALL BE ONE INCH WIDER MINIMUM ON EACH SIDE THAN THE SPECIFIED FOOTING WIDTH.
C. THE SIDE WALLS OF THE EXCAVATION SHALL BE BATTERED A MINIMUM OF ONE INCH HORIZONTAL TO TWELVE INCHES VERTICAL.
D. IF SANDY OR LOOSE MATERIALS ARE ENCOUNTERED, THE FOOTING MUST BE FORMED.
E. THE CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVAL OF ANY SOIL SLOUGHAGE FROM THE WET CONCRETE DURING THE CASTING OPERATION.
F. THE CONTRACTOR AGREES TO REMOVE AND RECAST ANY FOOTING WHERE THE ABOVE CONDITIONS ARE NOT MET.

EXPOSED SITE WALLS, RETAINING WALLS, AND STEM WALLS GREATER THAN 30 FEET IN LENGTH SHALL HAVE CONTROL JOINTS INSTALLED AT THE FOLLOWING MAXIMUM SPACING:

12'-0" ON CENTER FOR WALLS 6'-0" MAXIMUM HEIGHT
18'-0" ON CENTER FOR WALLS 10'-0" MAXIMUM HEIGHT
20'-0" ON CENTER FOR WALLS GREATER THAN 10'-0" IN HEIGHT

ALL CONCRETE EXPOSED TO GROUND SHALL BE MANUFACTURED WITH PORTLAND CEMENT TYPE I OR TYPE II.

SEE SHEET S7.11 FOR TYPICAL CONCRETE DETAILS.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE FABRICATED AND PLACED IN ACCORDANCE WITH THE BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI 318-14), AND DETAILS AND DETAILING OF CONCRETE REINFORCEMENT (ACI 315-99).

ALL REINFORCING STEEL SHALL CONFORM TO ASTM A615 GRADE 60; EXCEPT STIRRUPS, TIES AND INDICATED FIELD-BENT BARS, WHICH SHALL CONFORM TO ASTM A615 GRADE 40.

GENERAL STRUCTURAL NOTES

ALL WELDED WIRE FABRIC SHALL BE DEFORMED AND SHALL CONFORM TO ASTM A479. PROVIDE IN FLAT SHEETS ONLY.

TENSION AND COMPRESSION LAPS IN REINFORCING SHALL CONFORM TO THE LAP SPLICE SCHEDULE ON SHEET S6.01 AND BE IN ACCORDANCE WITH ACI 318, CHAPTER 12, UNLESS NOTED OTHERWISE.

ALL HORIZONTAL REINFORCING IN FOOTINGS, WALLS AND BEAMS SHALL BE CONTINUOUS AROUND CORNERS OR HAVE BENT (CORNER) BARS OF THE SAME SIZE AND SPACING AS THE HORIZONTAL BARS AND LAP 30 BAR DIAMETERS (24" MINIMUM).

CONCRETE COVER FOR REINFORCING SHALL BE AS FOLLOWS UNLESS OTHERWISE NOTED:

A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH:	3"
B. CONCRETE CAST AGAINST FORMS BUT EXPOSED TO EARTH OR WEATHER:	
1. BARS LARGER THAN NO. 5:	2"
2. BARS NO. 5 OR SMALLER:	1 1/2"
C. CONCRETE NOT EXPOSED TO WEATHER OR NOT IN CONTACT WITH GROUND:	
1. COLUMNS, GIRDERS AND BEAMS:	1 1/2"
2. STRUCTURAL SLABS, WALLS AND JOISTS (NO. 11 AND SMALLER):	3/4"
D. SLAB ON GRADE:	1 1/2" FROM TOP OF SLAB
E. STRUCTURAL SLABS ON METAL DECK:	1" FROM TOP OF SLAB

FORM TIES SHALL BE EITHER OF THE THREADED OR SNAP-OFF TYPE SO THAT NO METAL WILL BE LEFT WITHIN 1 INCH OF THE SURFACE OF THE WALL. FOLLOWING REMOVAL OF FORM TIES, RECESSES ARE TO BE CAREFULLY FILLED AND POINTED WITH MORTAR.

REINFORCING SHALL NOT BE TACK WELDED OR WELDED IN ANY MANNER UNLESS SPECIFICALLY DETAILED ON THE STRUCTURAL PLANS.

BAR SUPPORTS AND SPACERS FOR REINFORCING SHALL BE PROVIDED IN ACCORDANCE WITH ACI 315-99. REINFORCING SHALL BE SECURELY TIED TO SUPPORTS.

CHAIRS WITH 22 GAGE SAND PLATES OR PRECAST BLOCKS SHALL BE PROVIDED FOR ALL REINFORCING OF CONCRETE IN CONTACT WITH GRADE.

DECK CHAIRS SHALL BE PROVIDED FOR ALL WELDED WIRE FABRIC IN SLABS OVER METAL DECK.

POST INSTALLED ANCHORS:

THE STRUCTURAL DESIGN IS BASED ON THE POST INSTALLED ANCHORING SYSTEMS NOTED BELOW. SINCE ANCHOR CAPACITIES VARY BY MANUFACTURER, THE CONTRACTOR SHALL USE ONLY THE SYSTEMS NOTED BELOW UNLESS AN ALTERNATE IS APPROVED BY THE ENGINEER OF RECORD. ALTERNATE ANCHORING SYSTEMS MAY REQUIRE RE-DESIGN TO VERIFY ANCHOR QUANTITIES, SPACING, AND EMBED DEPTHS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY ADDITIONAL CONSTRUCTION AND RE-DESIGN COSTS ASSOCIATED WITH THE ALTERNATE ANCHORING SYSTEM.

ALL ADHESIVE (EPOXY) FOR POST INSTALLED ANCHORS AND/OR REBAR INTO GROUT FILLED MASONRY SHALL BE HILTI HIT HY 70 ADHESIVE ANCHORING SYSTEM. INSTALLATION SHALL BE PER THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.

ALL ADHESIVE (EPOXY) FOR POST INSTALLED ANCHORS AND/OR REBAR INTO HOLLOW MASONRY AND/OR BRICK SHALL BE HILTI HIT HY 270 ADHESIVE ANCHORING SYSTEM. INSTALLATION SHALL BE PER THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.

ALL POST INSTALLED MECHANICAL ANCHORS INTO CONCRETE SHALL BE HILTI KWIK HUS EZ (KH-EZ) SCREW ANCHOR. INSTALLATION SHALL BE PER THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.

ANCHOR LENGTHS SHOWN FOR ATTACHMENT TO CONCRETE AND/OR MASONRY ARE REQUIRED EMBEDMENT LENGTHS. THE CONTRACTOR SHALL PROVIDE ANCHORS WITH ADDITIONAL LENGTH TO FACILITATE THE REQUIRED CONNECTION.

SUBMIT ALL PROPOSED ANCHORING SYSTEMS INCLUDING ICC-ES REPORTS TO STRUCTURAL ENGINEER FOR REVIEW PRIOR TO INSTALLATION. THE ICC–ES FORMS SHALL MEET THE REQUIREMENTS OF THE IBC REFERENCED IN THESE NOTES.

ALL POST-INSTALLED ANCHORS SHALL BE INSTALLED WITH SPECIAL INSPECTION AS DICTATED BY THE RESPECTIVE PRODUCT'S ICC-ES EVALUATION SERVICE REPORT

THE CONTRACTOR SHALL ARRANGE AN ANCHOR MANUFACTURER'S REPRESENTATIVE TO PROVIDE ONSITE INSTALLATION TRAINING, UNLESS ALL PERSONNEL INSTALLING ANCHORS ARE CERTIFIED IN ACCORDANCE WITH ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM, OR EQUIVALENT APPROVED BY THE ENGINEER OF RECORD.

INSTALLATION OF ADHESIVE ANCHORS IN HORIZONTAL TO VERTICALLY OVERHEAD ORIENTATION SHALL BE CONTINUOUSLY INSPECTED DURING INSTALLATION BY AN INSPECTOR SPECIALLY APPROVED FOR THAT PURPOSE BY THE BUILDING OFFICIAL. INSTALLATION SHALL BE DONE BY A CERTIFIED ADHESIVE ANCHOR INSTALLER (AAI) AS CERTFIEID THROUGH ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM, OR EQUIVALENT. PROOF OF CURRENT CERTIFICATION SHALL BE SUBMITTED TO THE ENGINEER PRIOR TO COMMENCEMENT OF INSTALLATION, AND INSPECTION REPORTS SHALL BE PROVIDED TO THE ENGINEER OF RECORD AND THE BUILDING OFFICIAL.

STRUCTURAL AND MISCELLANEOUS STEEL:

ALL STRUCTURAL STEEL SHALL BE DETAILED AND FABRICATED IN ACCORDANCE WITH THE AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS".

ALL WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A992, GRADE 50, UNLESS NOTED OTHERWISE.

ALL MISCELLANEOUS STEEL MEMBERS, SUCH AS CHANNELS, ANGLES, FLAT BARS, AND PLATES SHALL CONFORM TO ASTM A36 UNLESS NOTED OTHERWISE.

ALL RECTANGULAR AND SQUARE STRUCTURAL TUBING SHALL CONFORM TO ASTM A500, GRADE B, FY = 46 KSI OR ASTM 1085, GRADE B, FY = 50 KSI.

ALL ROUND STRUCTURAL TUBING SHALL CONFORM TO ASTM A500, GRADE B, FY = 42 KSI OR ASTM 1085, GRADE B, FY = 50 KSI.

ALL STRUCTURAL PIPE SHALL CONFORM TO ASTM A53, TYPE E OR S, GRADE B, FY = 35 KSI.

BOLTS SHALL CONFORM TO ASTM A325N TENSION CONTROL BOLTS UNLESS NOTED OTHERWISE, WITH SIZES AS SHOWN ON THE DRAWINGS. WHERE CLEARANCE WITHIN A CONNECTION DOES NOT PERMIT THE USE OF TENSION CONTROL BOLTS, STANDARD A325N BOLTS SHALL BE USED AND INSPECTED IN ACCORDANCE WITH THE AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS".

ALL BOLTS SHALL BE INSTALLED IN A SNUG TIGHT CONDITION EXCEPT AT MOMENT CONNECTIONS, BRACED FRAME CONNECTIONS, AND AT CONNECTIONS DETAILED WITH A325SC BOLTS. AT THESE LOCATIONS, THE BOLTS SHALL BE TIGHTENED SO AS TO SHEAR THE SPLINE OFF THE BOLT.

ANCHOR BOLTS EMBEDDED IN CONCRETE SHALL BE ASTM F1554 GRADE 36 THREADED RODS WITH DOUBLE NUTS. PROVIDE FLAT WASHERS BETWEEN NUTS AND BASEPLATE SURFACES. ANCHOR BOLT LENGTHS SHOWN FOR ATTACHMENT TO CONCRETE AND/OR MASONRY ARE REQUIRED EMBEDMENT LENGTHS. THE CONTRACTOR SHALL PROVIDE ANCHOR BOLTS WITH ADDITIONAL BOLT LENGTH TO FACILITATE THE REQUIRED CONNECTION.

ANCHOR BOLT FLAT WASHERS SHALL BE PROVIDED IN ACCORDANCE WITH TABLE 14-2 OF AISC 360, AISC MANUAL OF STEEL CONSTRUCTION LATEST EDITION.

ALL WELDING SHALL BE DONE IN ACCORDANCE WITH THE LATEST STANDARDS OF THE AWS STRUCTURAL WELDING CODE.

ALL BOLT HOLES THAT ARE REQUIRED TO BE FIELD DRILLED SHALL BE DRILLED WITH A MAG DRILL. FLAME CUTTING OF HOLES OR ENLARGING OF MISALIGNED HOLES WILL NOT BE ALLOWED.

HEADED CONCRETE ANCHORS AND SHEAR CONNECTORS SHALL BE MADE FROM STEEL CONFORMING TO ASTM A108 AND MEET THE MECHANICAL PROPERTIES OF TYPE B, AS REQUIRED BY CHAPTER 7 OF AWS D1.1 "STRUCTURAL WELDING CODE-STEEL", LATEST EDITION. STRUCTURAL STEEL TO RECEIVE SHEAR CONNECTORS SHALL BE FREE OF PAINT. WELDING PREQUALIFICATION REQUIRED.

COMPOSITE FLOORS:

THE METAL DECK FOR COMPOSITE FLOORS SHALL BE UNSHORED UNLESS NOTED OTHERWISE.

THE SHEAR CONNECTORS SHALL BE 3/4" DIAMETER X 3" AT 1 1/2" DEEP DECK AND 3/4" DIAMETER X 4 1/2" AT 3" DEEP DECK UNLESS NOTED OTHERWISE. THE SHEAR CONNECTORS SHALL BE MADE FROM STEEL CONFORMING TO ASTM A108 AND MEET THE MECHANICAL PROPERTIES OF TYPE B, AS REQUIRED BY CHAPTER 7 OF AWS D1.1 "STRUCTURAL WELDING CODE STEEL", LATEST EDITION. STRUCTURAL STEEL TO RECEIVE SHEAR CONNECTIONS SHALL BE FREE OF PAINT. WELDING PREQUALIFICATION REQUIRED.

THE SHEAR CONNECTIONS SHALL NOT BE ADDED UNTIL THE METAL FLOOR DECK IS INSTALLED.

WHERE SHEAR CONNECTIONS AND PUDDLE WELDS COINCIDE, THE SHEAR CONNECTOR MAY REPLACE THE PUDDLE WELD.

CAMBERED BEAMS SHALL HAVE THE CAMBER PUT IN AT 1/3 POINTS OR ALONG A PARABOLIC CURVE.

THE CONTRACTOR SHALL SURVEY THE CAMBER OF THE BEAMS AFTER THE BEAMS HAVE BEEN ERECTED. THE CONTRACTOR SHALL SUBMIT THE SURVEY TO THE ENGINEER FOR REVIEW. THE CONTRACTOR SHALL NOT POUR THE SLAB UNTIL THE ENGINEER HAS REVIEWED AND APPROVED THE BEAM CAMBERS.

CONTRACTOR SHALL SHORE BEAMS WITH A CAMBER MORE THAN 1/2" LOWER THAN SPECIFIED. THE BEAM SHALL BE ALLOWED TO DEFLECT TO LEVEL.

GENERAL STRUCTURAL NOTES

THE TOPPING SLAB SHALL BE POURED AND PLACED TO THE ELEVATION INDICATED ON THE DRAWINGS.

THE CONCRETE FOR THE SLAB SHALL BE POURED AND PLACED TO THE ELEVATION INDICATED ON THE DRAWINGS WHILE MAINTAINING THE MINIMUM THICKNESS. SPREAD CONCRETE OVER AREA OF INFLUENCE TO ROUGH DEPTH IN ORDER TO LOAD BEAMS AND GIRDERS PRIOR TO SETTING SCREED ELEVATIONS.

THE WEIGHT OF THE WET CONCRETE WILL CAUSE DEFLECTIONS OF THE STEEL FRAMING. THEREFORE, CONCRETE OVERRRUNS ARE TO BE ANTICIPATED BY THE CONTRACTOR.

CONTRACTOR SHALL CONTINUOUSLY MONITOR THE THICKNESS AND ELEVATIONS DURING CONCRETE PLACING OPERATIONS.

PROVIDE #4 X 6'-0" AT 12" ON CENTER OVER ALL GIRDERS OF COMPOSITE FLOORS.

PROVIDE #4 X 6'-0" AT 12" ON CENTER OVER SHORED BEAMS THAT ARE NOT ALLOWED TO DEFLECT TO LEVEL.

PROVIDE WELDED WIRE FABRIC AS INDICATED ON DRAWINGS IN FLAT SHEETS ONLY.

PROVIDE DECK CHAIRS FOR ALL WELDED WIRE FABRIC IN SLABS OVER METAL DECK.

STEEL DECK:

ALL STEEL DECK SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE STEEL DECK INSTITUTE SPECIFICATIONS.

SEE PLANS FOR STEEL DECK TYPE, GAGE, FINISH AND CONNECTIONS.

PROVIDE A MINIMUM OF 1 1/2" BEARING FOR ALL STEEL DECK.

ALL SPLICES AND LAPS SHALL BE A MINIMUM OF 2" IN LENGTH AND SHALL BE LOCATED DIRECTLY ABOVE SUPPORTS.

ALL DECKING SHALL BE CONTINUOUS OVER TWO OR MORE SPANS.

COLD-FORMED METAL FRAMING (43 MILS OR HEAVIER):

ALL COLD-FORMED METAL FRAMING SHALL CONFORM TO THE LATEST EDITION OF AISI STANDARD S100 "NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS".

WALLS SHALL BE PROVIDED WITH MANUFACTURER'S STANDARD BRIDGING: (EITHER WELDED 2 1/2" x 43 MILS STUD OR CLIPPED COLD-ROLLED CHANNEL 1 1/2" x 54 MILS). PROVIDE BRIDGING AT 4'-0" ON CENTER MAXIMUM FOR LOAD BEARING WALLS AND EXTERIOR WALLS.

PROVIDE ALL MISCELLANEOUS ACCESSORIES AND FOLLOW ERECTION PROCEDURES AS PER MANUFACTURER'S SPECIFICATIONS AND RECOMMENDATIONS UNLESS NOTED OTHERWISE.

COLD-FORMED METAL FRAMING SHALL MEET THE MINIMUM PROPERTIES AS SHOWN IN THE STEEL STUD MANUFACTURERS ASSOCIATION (SSMA) SPECIFICATIONS.

ALL TRACK SHALL BE ANCHORED TO CONCRETE NOT MORE THAN 4' - 0" ON CENTER. ATTACHMENT BY OTHERS.

SECURE STUDS TO TOP AND BOTTOM TRACKS, ATTACHMENT BY OTHERS.

WELD CLIPS DESIGNED BY OTHERS. FASTEN WELD CLIPS TO STUDS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS AND LOAD DATA. WELD CLIPS SHALL BE DESIGNED FOR LATERAL LOADS AS SPECIFIED ON S0.03 AND VERTICAL LOADS AS DICTATED BY ARCHITECTURAL FINISHES AND SUPPORT REQUIREMENTS.

SLIDE CLIPS DESIGNED BY OTHERS. FASTEN SLIDE CLIPS TO STUDS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS AND LOAD DATA. SLIDE CLIPS SHALL BE DESIGNED FOR LATERAL LOADS AS SPECIFIED ON S0.03.

SEE SHEET S7.31 FOR TYPICAL COLD-FORMED DETAILS.

VENEER:

FOR CMU OR BRICK VENEER (4" MAXIMUM, 2" MINIMUM THICKNESS) ATTACHMENT TO STRUCTURAL MASONRY, PROVIDE ADJUSTABLE INTEGRAL ANCHOR TIES. ADJUSTABLE INTEGRAL ANCHOR TIES SHALL BE CORROSION RESISTANT AND HAVE TWO PINTLE LEGS MINIMUM WITH W2.8 (3/16") WIRE OR APPROVED EQUAL. PROVIDE DUR-O-WALL DA370 ADJUSTABLE INTEGRAL ANCHOR TIES OR APPOVED EQUAL.

FOR CMU OR BRICK VENEER (4" MAXIMUM, 2" MINIMUM THICKNESS) ATTACHMENT TO STRUCTURAL CONCRETE, PROVIDE ADJUSTABLE ANCHOR TIES. ADJUSTABLE ANCHOR TIES SHALL BE CORROSION RESISTANT AND HAVE A TWO PINTLE LEGS MINIMUM WITH A MINIMUM W2.8 (3/16") WIRE. ATTACH TO CONCRETE WITH 2-1/4" DIAMETER CONCRETE SCREWS, HILTI KWIKCON 11 x 1 1/2" OR APPROVED EQUAL.

FOR CMU OR BRICK VENEER (4" MAXIMUM, 2" MINIMUM THICKNESS) ATTACHMENT TO STRUCTURAL COLD FORMED METAL STUDS, PROVIDE ADJUSTABLE ANCHOR TIES. ADJUSTABLE ANCHOR TIES SHALL BE CORROSION RESISTANT AND HAVE TWO PINTLE LEGS MINIMUM W2.8 (3/16") WIRE. PROVIDE DUR-O-WALL DA213 ADJUSTABLE ANCHOR TIE OR APPROVED EQUAL. ATTACH THROUGH SHEATHING TO STUDS WITH 2-1/4" x 1 1/2" CORROSION RESISTANT TEK SCREWS.

SEE TYPICAL DETAILS ON SHEET S7.11, S7.21, AND S7.31 FOR VENEER TIE SPACING.

PROVIDE ADDITIONAL ANCHORS AROUND ALL OPENINGS LARGER THAN 16" IN EITHER DIMENSION. SPACE ANCHORS WITHIN 12" OF OPENING PERIMETER AND MATCH HORIZONTAL OR VERTICAL ANCHOR TIE SPACING.

COORDINATE VENEER LOCATION, TYPE, BOND PATTERN, ETC. WITH ARCHITECTURAL DRAWINGS.

GLASS CURTAIN WALL SYSTEM:

ALL LATERAL AND GRAVITY SUPPORT FOR THE GLASS CURTAIN WALL SYSTEM SHALL BE PER THE MANUFACTURER'S RECOMMENDATIONS. SHOP DRAWINGS AND STAMPED CALCULATIONS SHALL BE SUBMITTED FOR APPROVAL BY THE ENGINEER OF RECORD AND THE ARCHITECT PRIOR TO INSTALLATION.

THE ENGINEER STAMPING THE SHOP DRAWINGS SHALL BE REGISTERED IN THE STATE THAT THE PROJECT IS LOCATED.

THE GLASS CURTAIN WALL SYSTEM SHALL BE LATERALLY SUPPORTED AT ALL FLOORS AND ROOF LEVEL.

SEISMIC BRACING OF NON-STRUCTURAL COMPONENTS:

SEISMIC BRACING AND RESTRAINTS FOR MECHANICAL/ELECTRICAL EQUIPMENT AND SYSTEMS SHALL BE PROVIDED BY THE CONTRACTOR PER THE 2015 INTERNATIONAL BUILDING CODE (2015 IBC) AND THE ASCE 7-10, MINIMUM DESIGN LOADS FOR BUILDINGS AND STRUCTURES.

SEE THESE GENERAL STRUCTURAL NOTES FOR THE SITE-SPECIFIC SEISMIC DESIGN CRITERIA.

THE CONTRACTOR IS RESPONSIBLE FOR DESIGNING AND INSTALLING THE SEISMIC BRACING AND RESTRAINTS. STAMPED SHOP DRAWINGS, INCLUDING CALCULATIONS, SHALL BE SUBMITTED FOR APPROVAL PRIOR TO ANY BRACING INSTALLATION.

THE ENGINEER STAMPING THE SHOP DRAWINGS SHALL BE REGISTERED IN THE STATE THAT THE PROJECT IS LOCATED.

ELEVATORS:

THE STRUCTURE HAS BEEN DESIGNED FOR A KONE MONOSPACE 500: 2500/4000 (SUBJECT TO CHANGE) ELEVATOR.

ALL STRUCTURAL SUPPORTS, FLOOR PENETRATION SIZES AND PIT DIMENSIONS HAVE BEEN DESIGNED BASED ON THE ABOVE INFORMATION. SHOULD THE ACTUAL ELEVATOR(S) SELECTED DIFFER FROM THE INFORMATION PROVIDED ABOVE, THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY ADDITIONAL CONSTRUCTION AND REDESIGN COSTS ASSOCIATED WITH THE ALTERNATE ELEVATOR(S).

THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING ALL ELEVATOR PIT AND FLOOR PENETRATION LOCATIONS AND DIMENSIONS

DEFERRED SUBMITTALS:

THE DEFERRED SUBMITTALS LISTED BELOW ARE THOSE PORTIONS OF THE DESIGN THAT ARE NOT COMPLETED AT THE TIME OF APPLICATION AND ARE TO BE SUBMITTED TO THE BUILDING OFFICIAL AND APPROVED PRIOR TO THE INSTALLATION OF THOSE ITEMS. THE MANUFACTURER, CONSULTANT, OR CONTRACTOR, AS APPROPRIATE, SHALL PROVIDE SUBMITTALS TO THE ENGINEER OF RECORD FOR REVIEW FOR THE FOLLOWING ITEMS.

- PRECAST PRESTRESSED CONCRETE
- COLD-FORMED METAL (LIGHTGAGE) FRAMING
- METAL STAIRS, RAMPS, LADDERS, AND GUARDRAILS
- DEWATERING/DRAINAGE SYSTEM BEHIND BASEMENT WALLS, INCLUDING ITS TIE IN TO THE STORM WATER DRAINAGE SYSTEM
- SEISMIC BRACING OF ALL ARCHITECTURAL, LIGHTING, THEMING, FIRE SUPPRESSION, AND MEP COMPONENTS
- CURTAINWALL SYSTEMS
- TEMPORARY AND PERMANENT SHORING OF EXISTING STRUCTURES AND SOILS WHERE REQUIRED FOR NEW CONSTRUCTION
- UNDERSLAB FRENCH DRAIN SYSTEM LAYOUT AND TIE-IN TO STORM WATER DRAINAGE SYSTEM.

1

NOTE: THIS STRUCTURAL PACKAGE IS FOR FOUNDATIONS ONLY. ANY CHANGES TO THE PROJECT, INCLUDING, BUT NOT LIMITED TO: LOADING REQUIREMENTS, GEOMETRY CHANGES IN PLAN OR ELEVATION, SPACE USAGE REVISIONS, OR VALUE ENGINEERING MAY AFFECT THE STRUCTURAL STEEL MEMBER REQUIREMENTS SHOWN IN THESE DRAWINGS.

James R. Childers Architect, Inc.
45 South 4th Street
Fort Smith, AR 72901
479-783-2480
www.childersarchitect.com

PROFESSIONAL SEAL:

LICENSED PROFESSIONAL ENGINEER CHRIS S. ROMERO 23578 OKLAHOMA 3.20.19

CONSULTANT LOGO:

Chavez-Grieves consulting engineers, inc.
4700 Lincoln Road NE, Suite 102 · Albuquerque, NM 87109
505-344-4080 · 505-343-8759 (fax)

CLIENT:

SEAL OF THE CHEROKEE NATION SEPT. 6, 1839

COLLEGE OF Osteopathic Medicine AT THE CHEROKEE NATION

OKLAHOMA STATE UNIVERSITY

TAHLEQUAH, OKLAHOMA

KEY PLAN:

PROJECT PHASE: BID PACKAGE 03

REVISIONS		
#	DATE	DESCRIPTION
1	4/22/19	BID PACKAGE 03 ASI 01

DATE:	JOB NUMBER:
03-20-19	17-13

SHEET NUMBER: S0.02

GENERAL STRUCTURAL NOTES

SCHEDULE OF STRUCTURAL SPECIAL INSPECTIONS

1. SPECIAL INSPECTIONS / TESTING - "SPECIAL STRUCTURAL INSPECTION" SHALL NOT RELIEVE THE OWNER OR THEIR AGENT FROM HAVING THE INSPECTIONS OF THE JURISDICTION BUILDING DEPARTMENT PER SECTION 110 OF THE IBC PERFORMED. BOTH THE JURISDICTION BUILDING DEPARTMENT INSPECTIONS AND "SPECIAL STRUCTURAL INSPECTION" SHALL BE PERFORMED.

2. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR NOTIFYING THE JURISDICTION BUILDING OFFICIAL AND SPECIAL INSPECTOR WHEN WORK IS READY FOR INSPECTION.

3. REPORTING FOR SPECIAL INSPECTION - SPECIAL INSPECTION AND TESTING REPORTS SHALL BE COMPLETED AND DISTRIBUTED AT THE COMPLETION OF EACH TASK. IF A TASK IS TO TAKE LONGER THAN THREE (3) DAYS, PROVIDE REPORTS FOR EACH DAY. PROVIDE COPIES OF REPORTS TO CONTRACTOR, OWNER, ARCHITECT AND STRUCTURAL ENGINEER OF RECORD. SPECIAL INSPECTOR TO KEEP A NON-COMPLIANCE LIST DOCUMENTING ITEMS INSPECTED NOT MEETING APPROVED CONSTRUCTION DOCUMENTS AND WHEN / HOW RESOLVED.

4. SEE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING CONSTRUCTION DOCUMENTS FOR ADDITIONAL NON-STRUCTURAL SPECIAL INSPECTION ITEMS.

5. SPECIAL INSPECTION OF SHOP FABRICATED MEMBERS AND ASSEMBLIES SHALL BE IN ACCORDANCE WITH SECTION 1704.2, UNLESS FABRICATOR IS APPROVED TO PERFORM WORK WITHOUT SPECIAL INSPECTION.

6. IN ACCORDANCE WITH IBC CHAPTER 17, THE OWNER OR THE OWNER'S AGENT, OTHER THAN THE CONTRACTOR, SHALL EMPLOY ONE OR MORE APPROVED AGENCIES TO PROVIDE SPECIAL INSPECTIONS AND TESTS, DURING CONSTRUCTION FOR THE TYPES OF WORK LISTED BELOW THESE SPECIAL INSPECTIONS AND TESTS ARE IN ADDITION TO THE INSPECTIONS BY THE BUILDING OFFICIAL IDENTIFIED IN IBC SECTION 110

7. DEFINITIONS:
   * **SPECIAL INSPECTION:** INSPECTION AS HEREIN REQUIRED BY A QUALIFIED SPECIAL INSPECTOR COMPETENT WITH THE MATERIALS, INSTALLATION, FABRICATION, ERECTION OR PLACEMENT OF COMPONENTS AND CONNECTIONS REQUIRING SPECIAL EXPERTISE TO ENSURE COMPLIANCE WITH APPROVED CONSTRUCTION DOCUMENTS AND REFERENCED STANDARDS (SEE SECTION 1704).
   * **CONTINUOUS SPECIAL INSPECTION:** FULL-TIME OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED.
   * **PERIODIC SPECIAL INSPECTION:** THE PART-TIME OR INTERMITTENT OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK HAS BEEN OR IS BEING PERFORMED AND AT THE COMPLETION OF THE WORK.

ITEM	DESCRIPTION OF REQUIREMENTS	REQUIRED (YES/NO)
SPECIAL INSPECTION OF STRUCTURAL STEEL	TO BE PERFORMED IN ACCORDANCE WITH CHAPTER N OF AISC 360-10	YES
SPECIAL INSPECTION AND VERIFICATION OF STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.2	YES
SPECIAL INSPECTIONS AND VERIFICATIONS FOR CONCRETE CONSTRUCTION	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.3	YES
SPECIAL INSPECTIONS AND VERIFICATIONS FOR WOOD CONSTRUCTION	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.5	NO
SPECIAL INSPECTIONS AND VERIFICATIONS OF SOILS	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.6, THE GEOTECHNICAL REPORT LISTED IN THE GENERAL FOUNDATION NOTES, AND ANY OTHER REQUIREMENTS LISTED IN THE GENERAL FOUNDATION NOTES	YES

GENERAL STRUCTURAL NOTES

TEMPORARY SHORING OF EXCAVATIONS:

THE TEMPORARY SHORING OF EXCAVATIONS SHALL BE SOIL NAIL/SHOTCRETE SYSTEM, SHEET PILING, OR APPROVED EQUAL.

THE SHORING SHALL NOT BE DRIVEN OR INSTALLED IN ANY MANNER THAT COULD POTENTIALLY DAMAGE EXISTING STRUCTURES OR CAUSE HUMAN DISCOMFORT.

THE CONTRACTOR SHALL LOCATE ALL EXISTING UNDERGROUND UTILITIES PRIOR TO INSTALLING SHORING. PROVISIONS SHALL BE MADE TO AVOID EXISTING UTILITIES.

THE SHORING AS SHOWN ON THE PLANS IS FOR GRAPHICAL REPRESENTATION ONLY. THE CONTRACTOR SHALL VERIFY THE EXACT LOCATION AND CONFIGURATION OF THE SHORING.

ANY SHORING THAT REMAINS IN PLACE SHALL NOT HAMPER FUTURE CONSTRUCTION.

THE CONTRACTOR IS RESPONSIBLE FOR DESIGNING AND INSTALLING THE TEMPORARY SHORING. STAMPED SHOP DRAWINGS, INCLUDING CALCULATIONS, SHALL BE SUBMITTED FOR APPROVAL PRIOR TO ANY SHORING INSTALLATION.

THE ENGINEER STAMPING THE SHOP DRAWINGS SHALL BE REGISTERED IN THE STATE THAT THE PROJECT IS LOCATED.

DEMOLITION:

NOTCHING OR CUTTING ANY STRUCTURAL MEMBER IN THE FIELD IS PROHIBITED, UNLESS DETAILED OTHERWISE ON THE STRUCTURAL PLANS.

CONTRACTOR SHALL BE RESPONSIBLE FOR ADHERING TO ALL APPLICABLE STANDARDS SET FORTH BY OSHA.

PRIOR TO STARTING DEMOLITION WORK, THE CONTRACTOR SHALL MAKE AN INSPECTION OF ALL SURROUNDING IMPROVEMENTS TO REMAIN, TO DETERMINE AND RECORD THEIR EXISTING PHYSICAL CONDITION.

SHORING AND BRACING: THE CONTRACTOR SHALL FURNISH ALL SHORING, BRACING, AND INCIDENTALS NECESSARY AND REQUIRED FOR THE PROPER SUPPORT AND SAFETY OF ALL MEMBERS AFFECTED BY DEMOLITION WORK.

WHERE DEMOLITION WOULD AFFECT THE STRUCTURAL INTEGRITY OF THE REMAINING STRUCTURE, THE CONTRACTOR SHALL PROVIDE TEMPORARY SUPPORTS. THE CONTRACTOR SHALL NOTIFY THE ENGINEER OF ANY FIELD CONDITION WHICH WOULD PRESENT A HAZARDOUS CONDITION TO THE STRUCTURE BEFORE PROCEEDING.

PROTECTION: PROPER PRECAUTIONS SHALL BE TAKEN AT ALL TIMES TO PROTECT VEHICULAR AND PEDESTRIAN TRAFFIC FROM ANY DAMAGE OR INJURY WHICH MAY BE CAUSED, EITHER DIRECTLY OR INDIRECTLY, BY THE WORK INCLUDED ON THESE DRAWINGS. SUCH PRECAUTIONS SHALL INCLUDE THE ERECTION AND MAINTENANCE OF FENCES, BARRICADES, RAILINGS, GUARDS, SIGNS, COVERINGS, LIGHTS, AND OTHER PRECAUTIONS AS MAY BE REQUIRED. IF AT ANY TIME, IN THE OPINION OF THE OWNER OR THE OWNER'S REPRESENTATIVE, PROPER PRECAUTIONS ARE NOT BEING TAKEN TO SECURE THIS PROTECTION, THE CONTRACTOR SHALL AT NO ADDITIONAL COST TO THE OWNER, INSTALL AND MAINTAIN SUCH ADDITIONAL PROTECTION AS MAY BE DIRECTED BY THE OWNER.

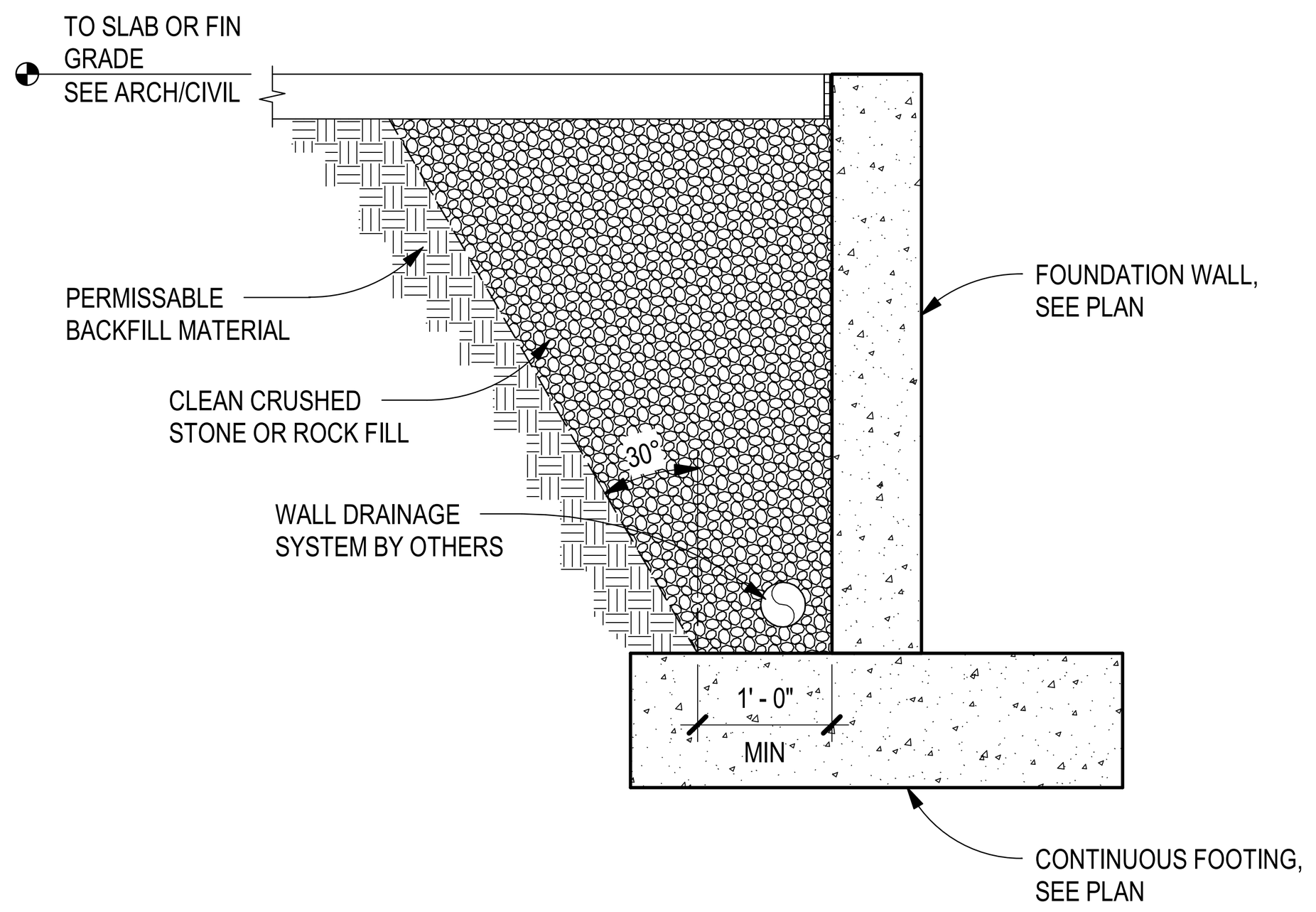
POLLUTION CONTROLS: USE WATER SPRINKLING, TEMPORARY ENCLOSURES, AND OTHER SUITABLE METHODS TO LIMIT DUST AND DIRT RISING AND SCATTERING IN THE AIR TO LOWEST PRACTICAL LEVEL. COMPLY WITH GOVERNING REGULATIONS PERTAINING TO ENVIRONMENTAL PROTECTION.

REMOVE DEBRIS FROM THE SITE AS IT ACCUMULATES. UNLESS OTHERWISE NOTED, DO NOT STORE, SELL, BURN, OR OTHERWISE DISPOSE OF DEBRIS ON THE SITE. REMOVAL OF DEBRIS INCLUDES CLEARING OF ALL LOWER LEVELS AND SIMILAR BELOW GRADE STRUCTURES. REMOVE ALL DEBRIS IN SUCH A MANNER AS TO PREVENT SPILLAGE. KEEP ALL PAVEMENTS AND AREAS ADJACENT TO THE SITE CLEAN AND FREE FROM MUD, DIRT, AND DEBRIS AT ALL TIMES.

USE OF EXPLOSIVES: THE CONTRACTOR IS ABSOLUTELY PROHIBITED FROM USING DYNAMITE OR ANY OTHER EXPLOSIVES IN ANY OF THE WORK OR OPERATIONS SHOWN ON THESE PLANS AT THE PROJECT SITE.

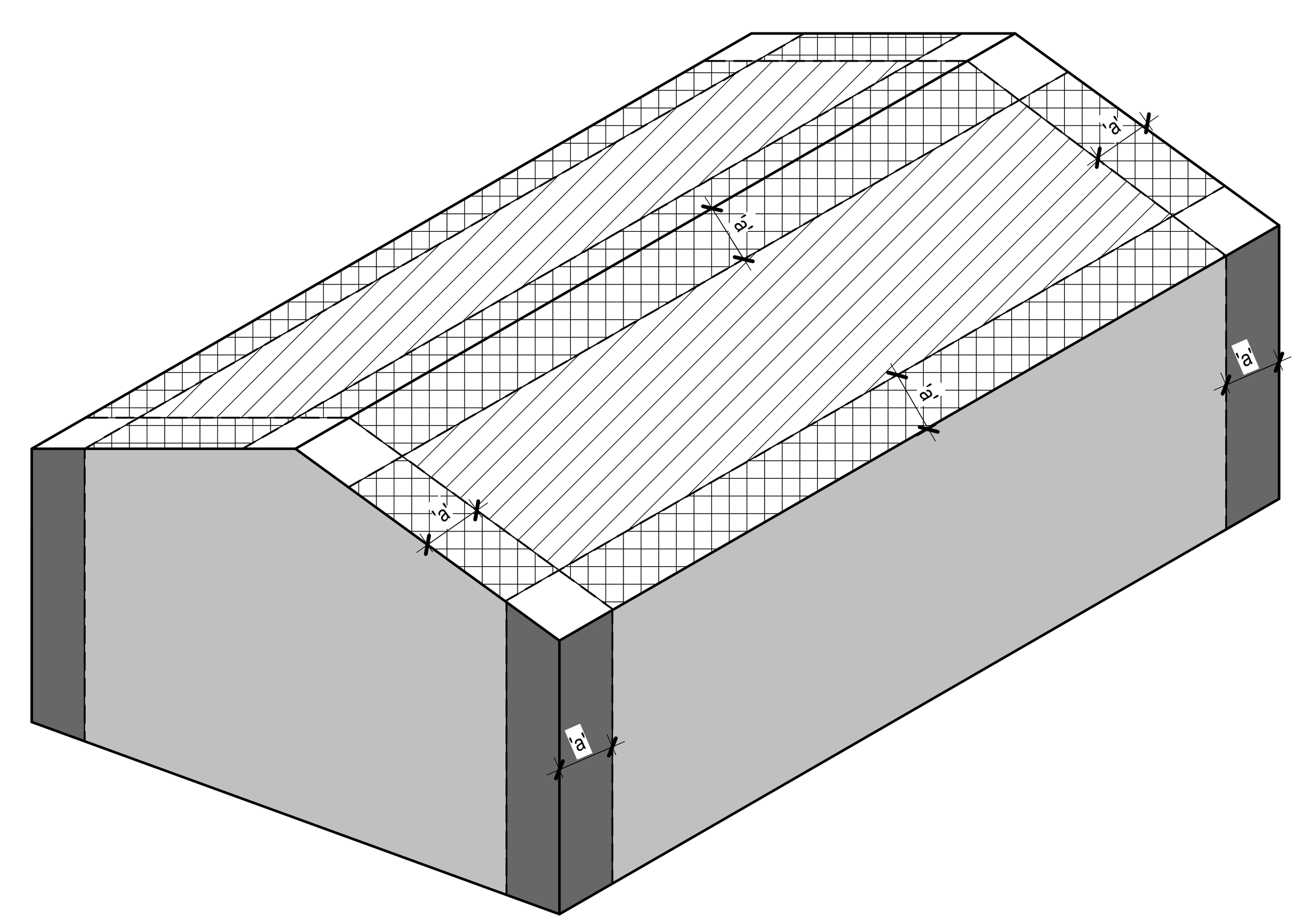
DEMOLITION SHALL BE PERFORMED IN A MANNER THAT WILL NOT DAMAGE ADJOINING SURFACES INDICATED TO REMAIN. SURFACES SHALL BE PATCHED, IF REQUIRED, TO PROVIDE A SUITABLE SUBSTRATE FOR NEW CONSTRUCTION.

SPECIFIC DEMOLITION NOTES ARE NOT TO BE CONSIDERED ALL INCLUSIVE OR COMPLETE IN THEMSELVES. CONTRACTOR SHALL PROVIDE ALL DEMOLITION INCIDENTAL TO OR REQUIRED FOR CONSTRUCTION WHETHER SPECIFICALLY NOTED OR NOT.



NOTES:
1. SEE GEOTECHNICAL REPORT FOR RETAINING WALL BACKFILL AND DRAINAGE REQUIREMENTS.
2. SEE APPROPRIATE SECTIONS AND DETAILS SPECIFIED ON PLANS FOR INFORMATION NOT SHOWN.

A1 **WALL BACKFILL AND DRAINAGE SCHEMATIC**
SCALE: NTS



COMPONENTS AND CLADDING WIND PRESSURES (PSF) CALCULATED AT MEAN ROOF HEIGHT = 68 FEET			
a = 9 FT	EFFECTIVE WIND AREA (FT^2)		
ZONE	10	100	500
1	-58.1	-48.2	-40.7
2	-91.3	-75.8	-63.9
1 AND 2 OVERHANGS	-91.3	-91.3	-91.3
3	-124.4	-103.2	-87.1
3 OVERHANGS	-143.0	-134.4	-128.7
4	39.7	35.0	31.8
4 PARAPETS	131.0	115.3	104.8
5	72.8	55.6	43.7
5 PARAPETS	164.1	125.2	98.5

ZONE 1
ZONE 2
ZONE 3
ZONE 4
ZONE 5

A4 **COMPONENT AND CLADDING WIND LOADING DIAGRAM**
SCALE: NTS

NOTE: THIS STRUCTURAL PACKAGE IS FOR FOUNDATIONS ONLY. ANY CHANGES TO THE PROJECT, INCLUDING, BUT NOT LIMITED TO: LOADING REQUIREMENTS, GEOMETRY CHANGES IN PLAN OR ELEVATION, SPACE USAGE REVISIONS, OR VALUE ENGINEERING MAY AFFECT THE STRUCTURAL STEEL MEMBER REQUIREMENTS SHOWN IN THESE DRAWINGS.

James R. Childers Architect, Inc.
45 South 4th Street
Fort Smith, AR 72901
479-783-2480
www.childersarchitect.com

PROFESSIONAL SEAL:
LICENSED PROFESSIONAL ENGINEER CHRIS S. ROMERO 23578 OKLAHOMA

CONSULTANT LOGO:
Chavez-Grieves consulting engineers, inc.

CLIENT:

COLLEGE OF Osteopathic Medicine AT THE CHEROKEE NATION
TAHLEQUAH, OKLAHOMA

KEY PLAN:

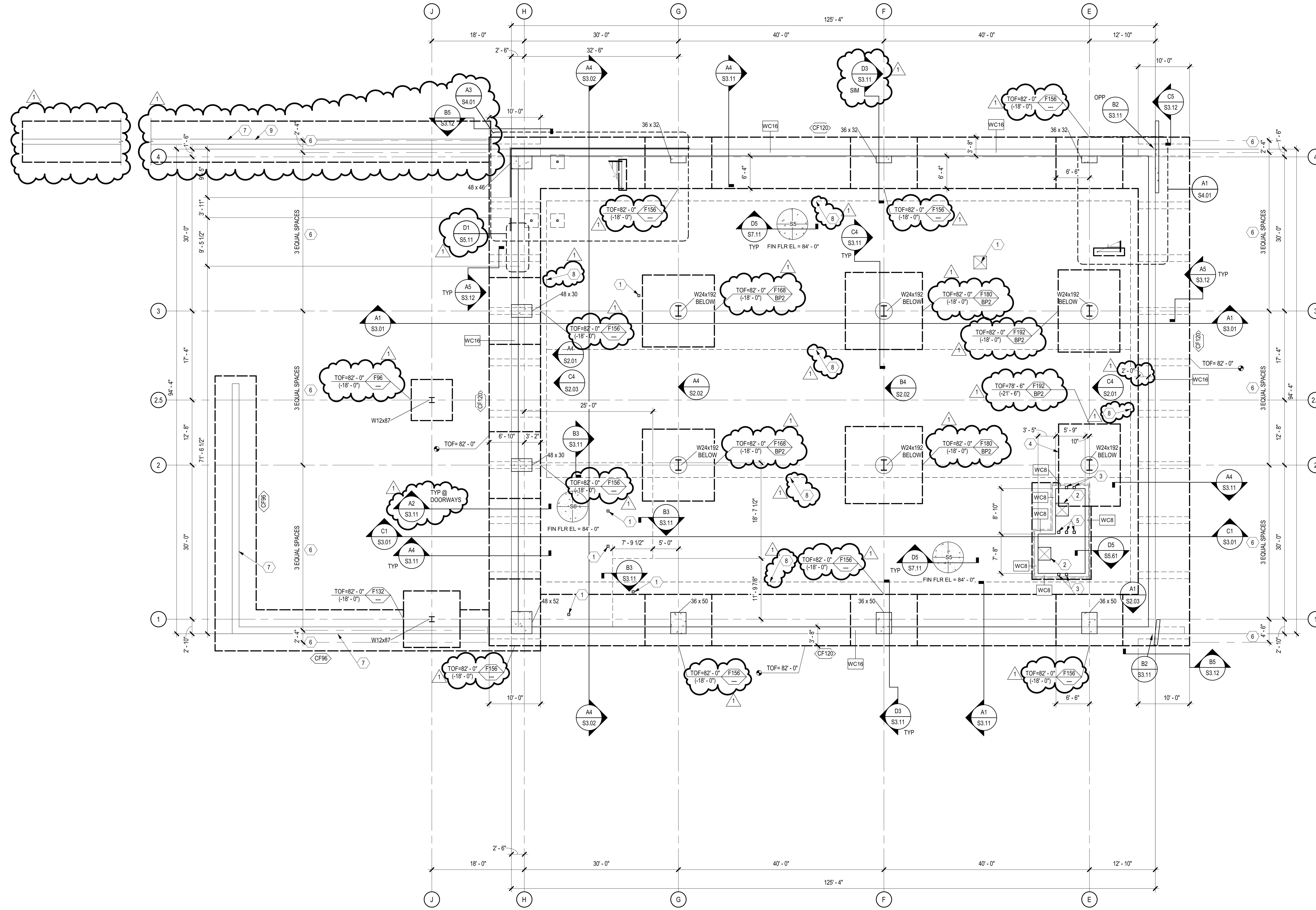
PROJECT PHASE:
BID PACKAGE 03

REVISIONS		
#	DATE	DESCRIPTION
1	4/22/19	BID PACKAGE 03 ASI 01

DATE:	JOB NUMBER:
03-20-19	17-13

SHEET NUMBER:
S0.03

GENERAL STRUCTURAL NOTES AND SPECIAL INSPECTION TABLES



A1 BASEMENT FOUNDATION PLAN - SECTOR 2

SCALE: 1/8" = 1'-0"

GENERAL SHEET NOTES

1. SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
2. REFERENCE FINISH FLOOR ELEVATION 100'-0" = MEAN SEA FINISH FLOOR ELEVATION. SEE CIVIL DRAWINGS.
3. NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE CONCRETE WALLS AND MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
4. DIMENSIONS ARE TO THE FACE OF CONCRETE UNLESS NOTED OTHERWISE.
5. PROVIDE SLAB JOINTS AT 10'-0" ON CENTER MAXIMUM. THE AREA OF THE CONTROL JOINT SHALL NOT EXCEED A 2.1 RATIO. CONTROL JOINTS SHALL BE LOCATED AT COLUMN LINES WHERE THE LAYOUT PERMITS. AT RE-ENTRANT CORNERS THAT DO NOT HAVE CONTROL JOINTS, PROVIDE 2-#4 x 3'-0" DIAGONAL TO THE RE-ENTRANT CORNER.
6. STRUCTURAL COLD FORMED METAL STUDS SHALL BE 8" WIDE UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING BY OTHERS.
7. SEE S7.00 SERIES SHEETS FOR TYPICAL FOUNDATION SECTIONS AND DETAILS.
8. SEE SHEET S6.01 FOR SCHEDULES.

SHEET KEYNOTE

1. FLOOR DRAIN / MOP SINK, SLOPE SLAB TO DRAIN 1/8" PER FOOT. COORDINATE EXACT SIZE AND LOCATION WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
2. ELEVATOR SUMP PIT, COORDINATE EXACT SIZE AND LOCATION WITH ELEVATOR MANUFACTURER, SEE A4 / S5.61
3. HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE B4 / S5.61
4. TOP OF FOOTING ELEVATION = 79'-6". ELEVATION SHALL BE COORDINATED WITH TOP OF ELEVATOR PIT FOOTING. CONTRACTOR TO COORDINATE.
5. HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE D4 / S5.61
6. TRANSVERSE SHEAR KEY AT LOCATIONS/SPACING SHOWN.
7. SITE RETAINING WALL. COORDINATE EXACT SIZE AND EXTENT WITH ARCHITECTURAL AND CIVIL DRAWINGS. SEE RETAINING WALL SCHEDULE A1 / S6.01
8. UNDER SLAB FRENCH DRAIN SYSTEM REQUIRED BELOW THE SLAB ON GRADE IN THE BASEMENT. SEE GEOTECHNICAL REPORT SECTION 12 FOR ADDITIONAL INFORMATION. FRENCH DRAIN PIPING SHALL BE SLOPED TO DRAIN WATER QUICKLY FROM BELOW THE SLAB TO A STORM DRAIN SYSTEM OUTSIDE THE BUILDING FOUNDATION PERIMETER.
9. PROVIDE WALL TYPE 'B', 12" STEM, SITE WALL AT PRIVACY FENCE LOCATIONS. SEE A1 / S6.01 FOR SITE RETAINING WALL SCHEDULE. SEE ARCHITECTURAL AND CIVIL DRAWINGS FOR INFORMATION NOT GIVEN.

NOTE: THIS STRUCTURAL PACKAGE IS FOR FOUNDATIONS ONLY. ANY CHANGES TO THE PROJECT, INCLUDING, BUT NOT LIMITED TO: LOADING REQUIREMENTS, GEOMETRY CHANGES IN PLAN OR ELEVATION, SPACE USAGE REVISIONS, OR VALUE ENGINEERING MAY AFFECT THE STRUCTURAL STEEL MEMBER REQUIREMENTS SHOWN IN THESE DRAWINGS.

James R. Childers Architect, Inc.
45 South 4th Street
Fort Smith, AR 72901
479-783-2480
www.childersarchitect.com

PROFESSIONAL SEAL:

LICENSED PROFESSIONAL ENGINEER
CHRIS S. ROMERO
23578
OKLAHOMA

CONSULTANT LOGO:

Chavez-Grieves
consulting engineers, inc.
4700 Lincoln Road NE, Suite 102 Albuquerque, NM 87109
505-344-4080 505-343-8759 (fax)

CLIENT:

THE CHEROKEE NATION
SEPT. 6, 1839

COLLEGE OF
Osteopathic Medicine
AT THE CHEROKEE NATION
TAHLEQUAH, OKLAHOMA

KEY PLAN:

02 01

PROJECT PHASE:

BID PACKAGE 03

REVISIONS

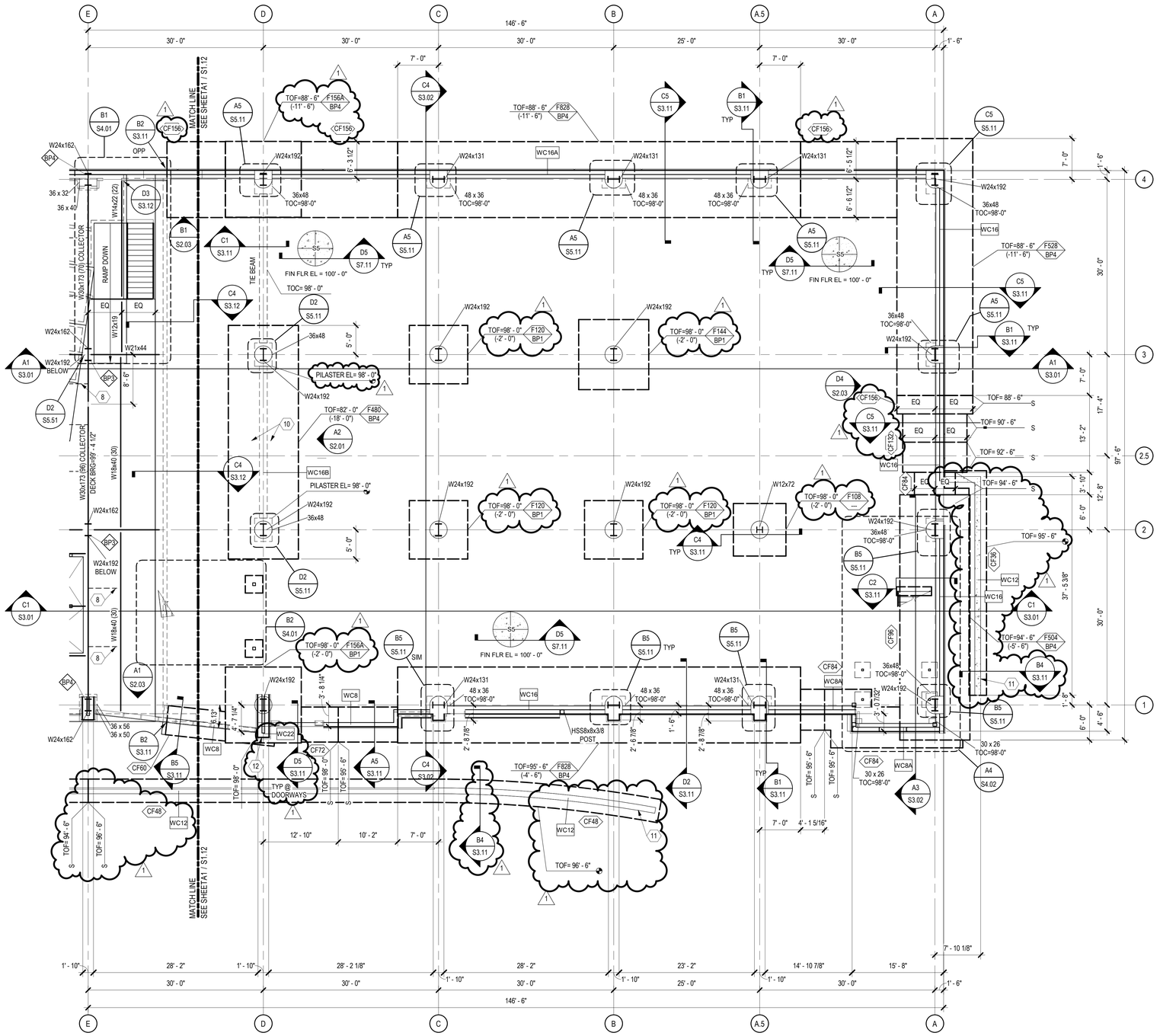
#	DATE	DESCRIPTION
1	4/22/19	BID PACKAGE 03 ASI 01

DATE: 03-20-19

JOB NUMBER: 17-13

SHEET NUMBER: S1.02

BASEMENT FOUNDATION PLAN - SECTOR 2

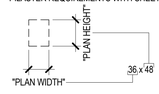


A1 FIRST FLOOR FOUNDATION AND FRAMING PLAN - SECTOR 1

SCALE: 1/8" = 1'-0"

GENERAL SHEET NOTES

1. SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
2. NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE CONCRETE WALLS AND MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
3. DIMENSIONS ARE TO THE FACE OF CONCRETE OR STUDS UNLESS NOTED OTHERWISE.
4. SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
5. BEAMS ARE SPACED EQUALLY BETWEEN GRIDS UNLESS NOTED OTHERWISE.
6. STRUCTURAL COLD FORMED METAL STUDS SHALL BE 8" WIDE UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING BY OTHERS.
7. SEE S7.00 SERIES SHEETS FOR TYPICAL DETAILS.
8. SEE SHEET S6.01 FOR SCHEDULES.
9. ALL MOMENT FRAMES LABELED ON PLAN UTILIZE SIDEPLATE PROPRIETARY MOMENT CONNECTIONS. SEE S8.00 SERIES SHEETS
10. ▶ DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
11. ■ DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE DRAWINGS.
12. DIMENSIONS SHOWN ON PLAN AS FOLLOWS ARE CONCRETE PILASTER DIMENSIONS IN INCHES: 38x36, 50x36, ETC. DIMENSIONS ARE "PLAN WIDTH" x "PLAN HEIGHT". COORDINATE PILASTER REQUIREMENTS WITH SHEET S2.03

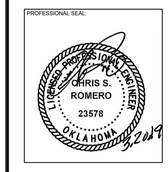


SHEET KEYNOTE

1. HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST, COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE B4 / S5.62 , C4 / S5.62 , D4 / S5.62
2. OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A5 / S5.52 FOR SUPPORT.
3. BEAM SPLICE LOCATION. SEE B4 / S5.52 FOR SPLICE DETAIL.
4. HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER. SEE A4 / S5.62, B4 / S5.62 , C4 / S5.62 , D4 / S5.62 , AND C3 / S5.62
5. W8x31 OUTRIGGER.
6. TOTAL NUMBER OF CHORD REINFORCEMENT BARS AT EXTENTS SHOWN. CHORD REINFORCEMENT SHALL BE LOCATED AS INDICATED ON PLAN. PROVIDE 130% LAP SPLICES WHEN REQUIRED.
7. 3-#7 SLAB REINFORCING BARS. EXTEND BARS 130% OF A LAP SPLICE LENGTH BEYOND OPENING, OR PROVIDE STD 90 DEGREE HOOK WHERE REQUIRED.
8. BOTTOM FLANGE BRACING AT EQUAL SPACING, UNLESS NOTED OTHERWISE. SEE B1 / S5.52
9. BOTTOM FLANGE BRACING SPACED AT 10' - 0" ON CENTER MAXIMUM, UNLESS NOTED OTHERWISE. SEE A1 / S5.52
10. BACKFILL PLACED AGAINST WALL SHALL BE DONE IN EQUAL LIFTS, ALTERNATING EACH SIDE OF WALL TO PREVENT UNINTENDED RETAINAGE OF SOIL.
11. SITE WALL. COORDINATE EXACT SIZE, EXTENT, AND RADIAL DIMENSIONS WITH ARCHITECTURAL AND CIVIL DRAWINGS. SEE B4 / S3.11
12. PROVIDE STEMWALL FOR SUPPORT OF EXTERIOR STUDS AND VENEER, SEE D1 / S3.11

James R. Childers Architect, Inc.
45 South 4th Street
Fort Smith, AR 72901
479-783-2480
www.childersarchitect.com

PROFESSIONAL SEAL:



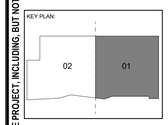
CONSULTANT LOGO:

Chavez-Grieves consulting engineers, inc.
4700 Lincoln Road NE, Suite 102 Albuquerque, NM 87109
505-344-4080 505-343-8759 (fax)

CLIENT:

COLLEGE OF Osteopathic Medicine AT THE CHEROKEE NATION
TAHLEQUAH, OKLAHOMA

KEY PLAN:



PROJECT PHASE:

BID PACKAGE 03

REVISIONS		
#	DATE	DESCRIPTION
1	4/22/19	BID PACKAGE 03 ASI 01

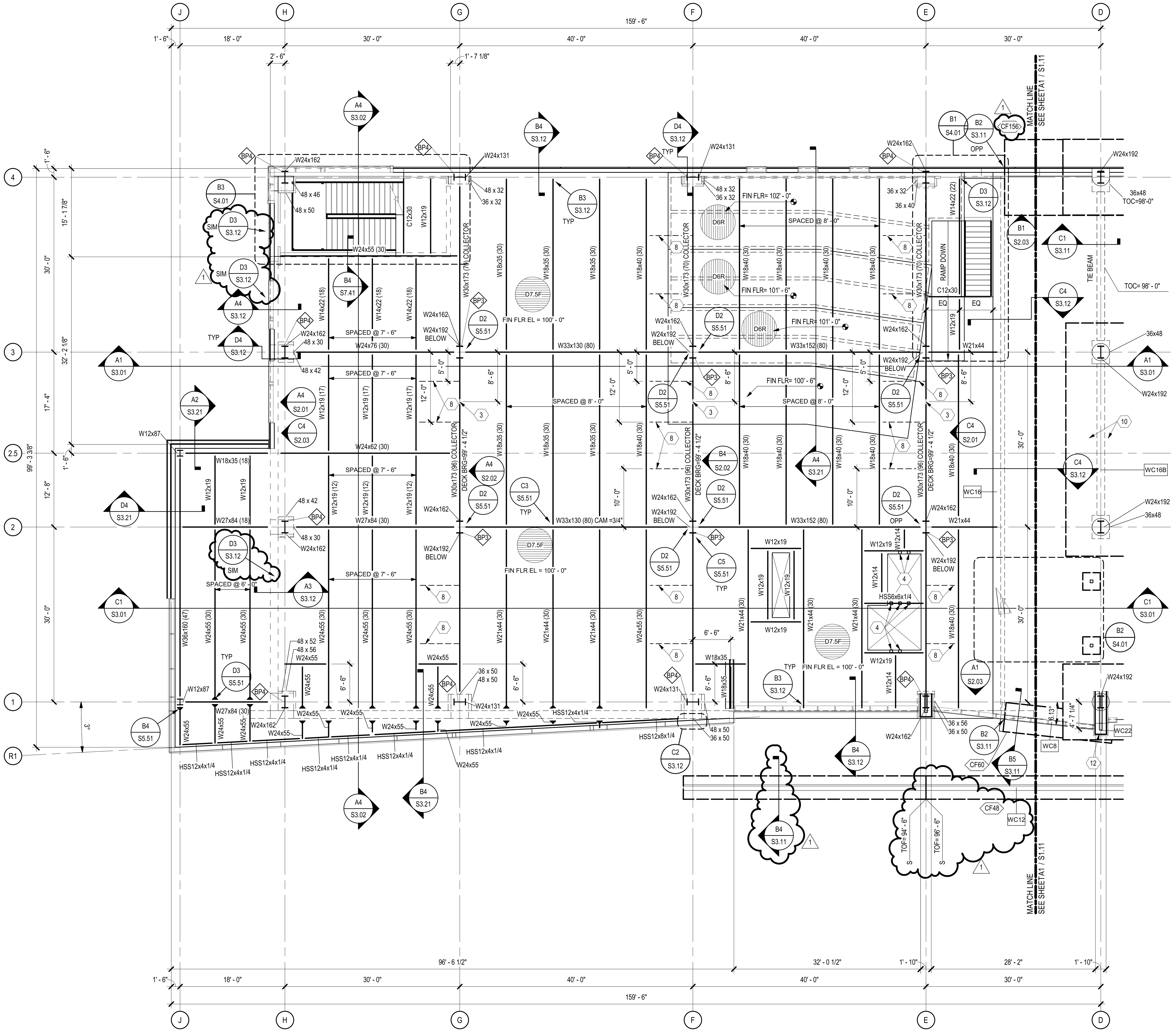
DATE: 03-20-19

JOB NUMBER: 17-13

SHEET NUMBER: S1.11

FIRST FLOOR FOUNDATION AND FRAMING PLAN - SECTOR 1

NOTE: THIS STRUCTURAL PACKAGE IS FOR FOUNDATIONS ONLY. ANY CHANGES TO THE PROJECT, INCLUDING, BUT NOT LIMITED TO: LOADING REQUIREMENTS, GEOMETRY CHANGES IN PLAN OR ELEVATION, SPACE USAGE REVISIONS, OR VALUE ENGINEERING MAY AFFECT THE STRUCTURAL STEEL MEMBER REQUIREMENTS SHOWN IN THESE DRAWINGS.

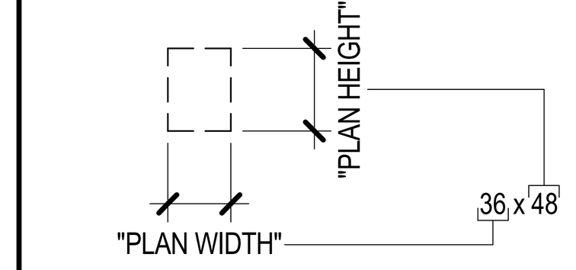


A1 FIRST FLOOR FRAMING PLAN - SECTOR 2

SCALE: 1/8" = 1'-0"

GENERAL SHEET NOTES

1. SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
2. NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE CONCRETE WALLS AND MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
3. DIMENSIONS ARE TO THE FACE OF CONCRETE OR STUDS UNLESS NOTED OTHERWISE.
4. SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
5. BEAMS ARE SPACED EQUALLY BETWEEN GRIDS UNLESS NOTED OTHERWISE.
6. STRUCTURAL COLD FORMED METAL STUDS SHALL BE 8" WIDE UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING BY OTHERS.
7. SEE S7.00 SERIES SHEETS FOR TYPICAL DETAILS.
8. SEE SHEET S6.01 FOR SCHEDULES.
9. ALL MOMENT FRAMES LABELED ON PLAN UTILIZE SIDEPLATE PROPRIETARY MOMENT CONNECTIONS. SEE S8.00 SERIES SHEETS
10. DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
11. DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE DRAWINGS.
12. DIMENSIONS SHOWN ON PLAN AS FOLLOWS ARE CONCRETE PILASTER DIMENSIONS IN INCHES: 38x36, 50x36, ETC. DIMENSIONS ARE "PLAN WIDTH" x "PLAN HEIGHT". COORDINATE PILASTER REQUIREMENTS WITH SHEET S2.03



SHEET KEYNOTE

1. HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST, COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE B4 / S5.62 , C4 / S5.62 , D4 / S5.62
2. OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A5 / S5.52 FOR SUPPORT.
3. BEAM SPLICE LOCATION. SEE B4 / S5.52 FOR SPLICE DETAIL.
4. HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE A4 / S5.62, B4 / S5.62 , C4 / S5.62 , D4 / S5.62 , AND C3 / S5.62
5. W8x31 OUTRIGGER.
6. TOTAL NUMBER OF CHORD REINFORCEMENT BARS AT EXTENTS SHOWN. CHORD REINFORCEMENT SHALL BE LOCATED AS INDICATED ON PLAN. PROVIDE 130% LAP SPLICES WHEN REQUIRED.
7. 3-#7 SLAB REINFORCING BARS. EXTEND BARS 130% OF A LAP SPLICE LENGTH BEYOND OPENING, OR PROVIDE STD 90 DEGREE HOOK WHERE REQUIRED.
8. BOTTOM FLANGE BRACING AT EQUAL SPACING, UNLESS NOTED OTHERWISE. SEE B1 / S5.52
9. BOTTOM FLANGE BRACING SPACED AT 10' - 0" ON CENTER MAXIMUM, UNLESS NOTED OTHERWISE. SEE A1 / S5.52
10. BACKFILL PLACED AGAINST WALL SHALL BE DONE IN EQUAL LIFTS, ALTERNATING EACH SIDE OF WALL TO PREVENT UNINTENDED RETAINAGE OF SOIL.
11. SITE WALL. COORDINATE EXACT SIZE, EXTENT, AND RADIAL DIMENSIONS WITH ARCHITECTURAL AND CIVIL DRAWINGS. SEE B4 / S3.11
12. PROVIDE STEMWALL FOR SUPPORT OF EXTERIOR STUDS AND VENEER, SEE D1 / S3.11

NOTE: THIS STRUCTURAL PACKAGE IS FOR FOUNDATIONS ONLY. ANY CHANGES TO THE PROJECT, INCLUDING, BUT NOT LIMITED TO: LOADING REQUIREMENTS, GEOMETRY CHANGES IN PLAN OR ELEVATION, SPACE USAGE REVISIONS, OR VALUE ENGINEERING MAY AFFECT THE STRUCTURAL STEEL MEMBER REQUIREMENTS SHOWN IN THESE DRAWINGS.

James R. Childers Architect, Inc.
45 South 4th Street
Fort Smith, AR 72901
479-783-2480
www.childersarchitect.com

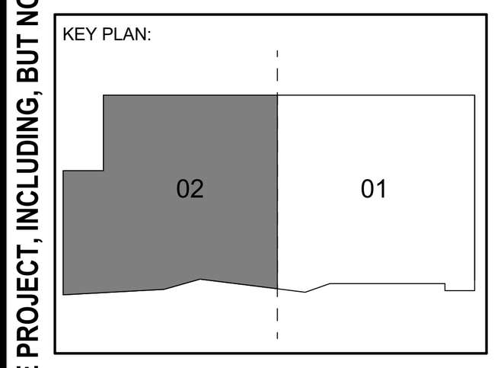
PROFESSIONAL SEAL:
LICENSED PROFESSIONAL ENGINEER
CHRIS S. ROMERO
23578
OKLAHOMA

CONSULTANT LOGO:
Chavez-Grieves
consulting engineers, inc.
4700 Lincoln Road NE, Suite 102 Albuquerque, NM 87109
505-344-4080 505-343-8759 (fax)

CLIENT:
THE CHEROKEE NATION
SEPT. 6, 1839

COLLEGE OF
Osteopathic Medicine
AT THE CHEROKEE NATION
TAHLEQUAH, OKLAHOMA

KEY PLAN:



PROJECT PHASE:
BID PACKAGE 03

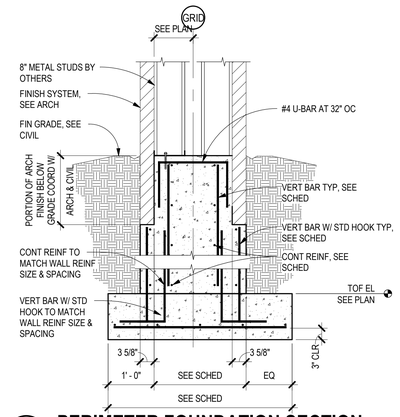
REVISIONS		
#	DATE	DESCRIPTION
1	4/22/19	BID PACKAGE 03 ASI 01

DATE: 03-20-19

JOB NUMBER: 17-13

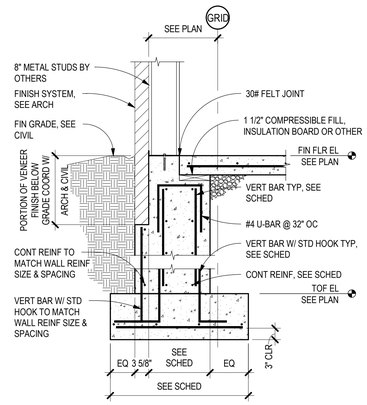
SHEET NUMBER: S1.12

FIRST FLOOR FRAMING PLAN - SECTOR 2



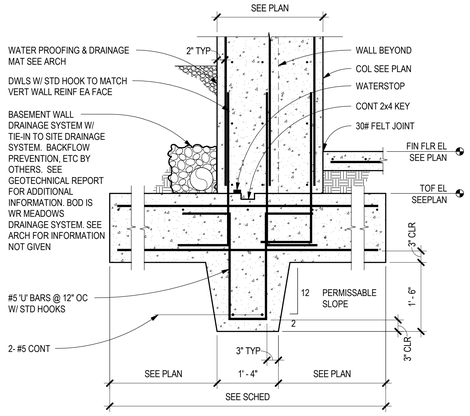
D1 PERIMETER FOUNDATION SECTION

SCALE: 3/4" = 1'-0"



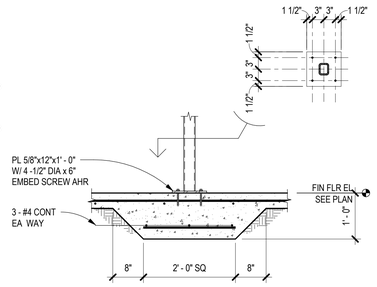
D2 PERIMETER FOUNDATION SECTION

SCALE: 3/4" = 1'-0"



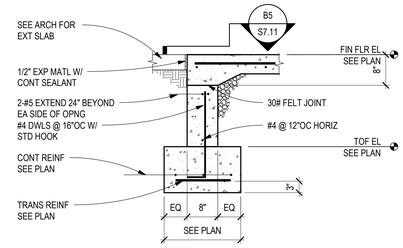
D3 BASEMENT COLUMN SECTION

SCALE: 3/4" = 1'-0"



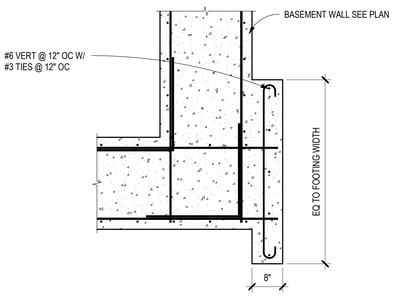
D4 INT COLUMN AT THICKENED SLAB

SCALE: 3/4" = 1'-0"



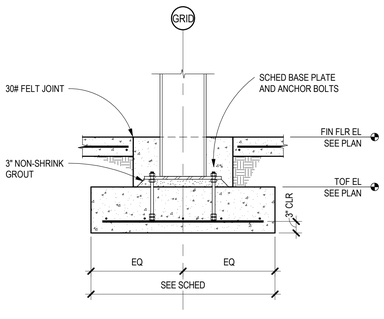
D5 CONC STEMWALL AT OPNG

SCALE: 3/4" = 1'-0"



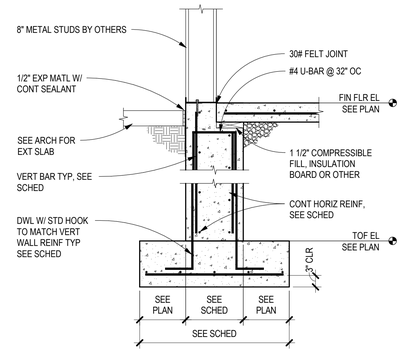
C3 FOUNDATION SECTION

SCALE: 3/4" = 1'-0"



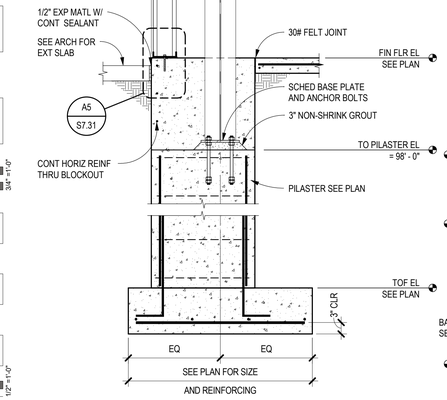
C4 INTERIOR COLUMN SECTION

SCALE: 3/4" = 1'-0"



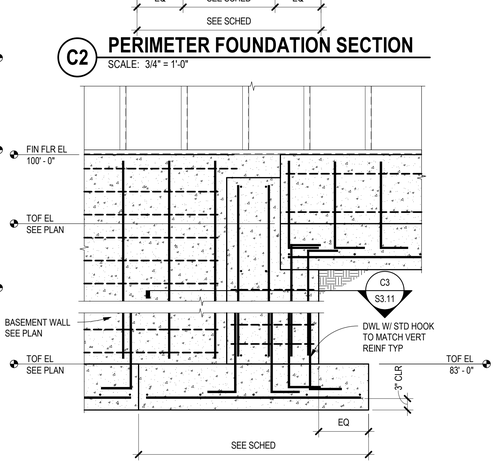
C5 PERIMETER FOUNDATION SECTION

SCALE: 3/4" = 1'-0"



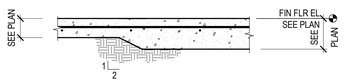
B1 FOUNDATION SECTION

SCALE: 3/4" = 1'-0"



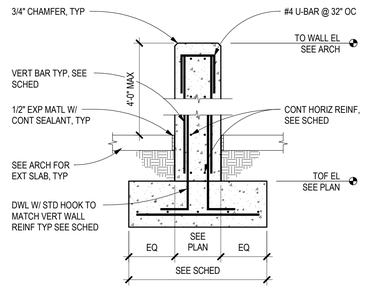
B2 FOUNDATION SECTION

SCALE: 3/4" = 1'-0"



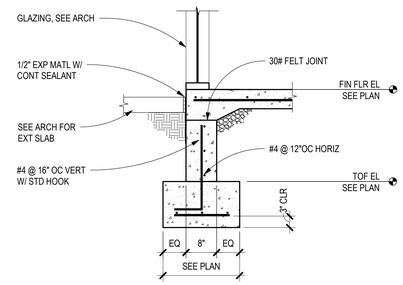
B3 SLAB THICKNESS TRANSITION

SCALE: 3/4" = 1'-0"



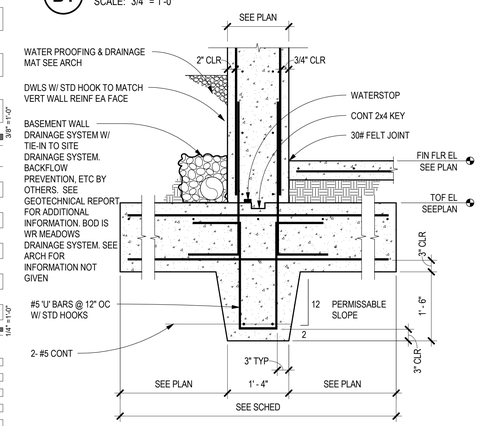
B4 EXTERIOR SITE WALL SECTION

SCALE: 3/4" = 1'-0"



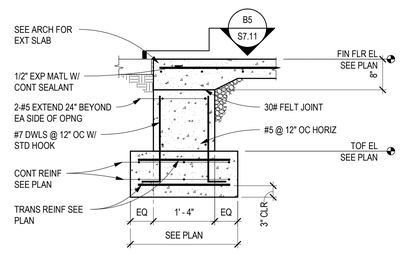
B5 PERIMETER FOUNDATION SECTION

SCALE: 3/4" = 1'-0"



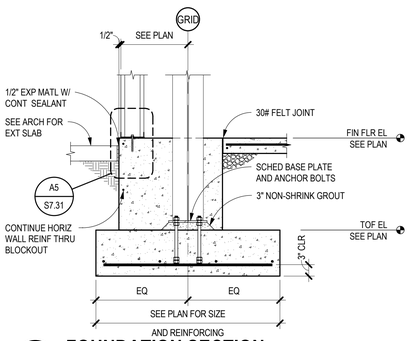
A1 BASEMENT WALL SECTION

SCALE: 3/4" = 1'-0"



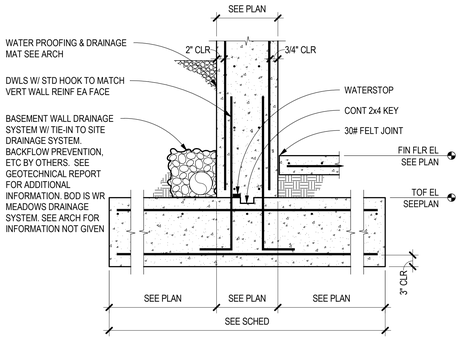
A2 BASEMENT WALL AT OPNG

SCALE: 3/4" = 1'-0"



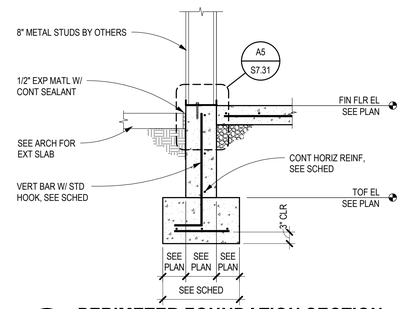
A3 FOUNDATION SECTION

SCALE: 3/4" = 1'-0"



A4 BASEMENT WALL SECTION

SCALE: 3/4" = 1'-0"



A5 PERIMETER FOUNDATION SECTION

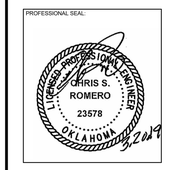
SCALE: 3/4" = 1'-0"

1 ENTIRE SHEET REVISED

NOTE: THIS STRUCTURAL PACKAGE IS FOR FOUNDATIONS ONLY. ANY CHANGES TO THE PROJECT, INCLUDING, BUT NOT LIMITED TO: LOADING REQUIREMENTS, GEOMETRY CHANGES IN PLAN OR ELEVATION, SPACE USAGE REVISIONS, OR VALUE ENGINEERING MAY AFFECT THE STRUCTURAL STEEL MEMBER REQUIREMENTS SHOWN IN THESE DRAWINGS.

James R. Childers Architect, Inc.
45 South 4th Street
Fort Smith, AR 72901
479-783-2480
www.childersarchitect.com

PROFESSIONAL SEAL:



CONSULTANT LOGO:

Chavez-Grieves consulting engineers, inc.
4700 Lincoln Road NE, Suite 102 Albuquerque, NM 87109
505-344-4080 505-343-8759 (fax)

CLIENT:

COLLEGE OF Osteopathic Medicine
AT THE CHEROKEE NATION
TAHLEQUAH, OKLAHOMA

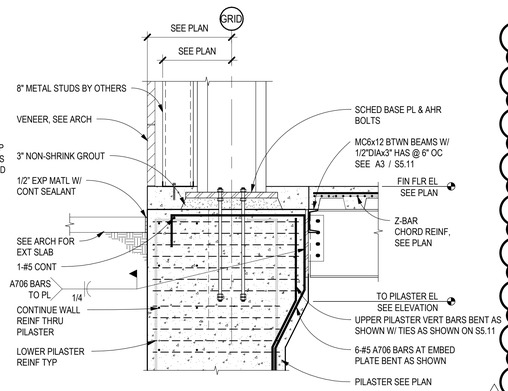
KEY PLAN:

PROJECT PHASE:
BID PACKAGE 03

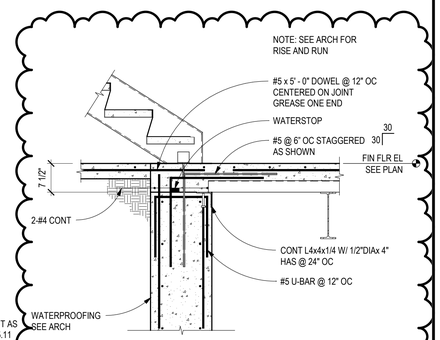
REVISIONS:		
#	DATE	DESCRIPTION
1	4/2/219	BID PACKAGE 03 ASI 01

DATE: 03-20-19
JOB NUMBER: 17-13
SHEET NUMBER: S3.11
FOUNDATION SECTIONS

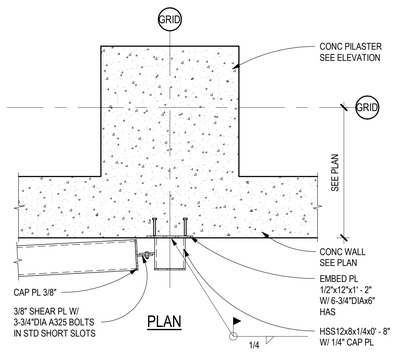
D3 BEAM TO CONC LINTEL DETAIL
SCALE: 3/4" = 1'-0"



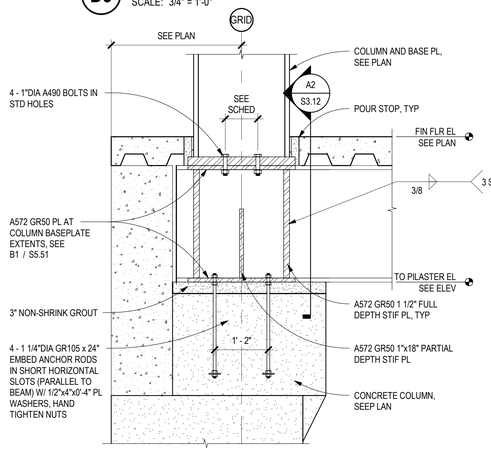
D4 BASEMENT WALL SECTION
SCALE: 3/4" = 1'-0"



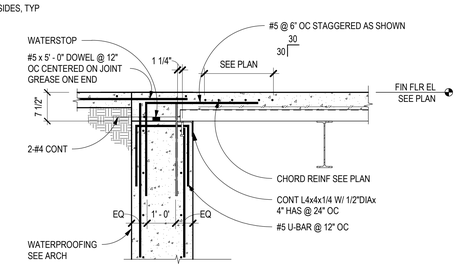
D5 BASEMENT WALL SECTION
SCALE: 3/4" = 1'-0"



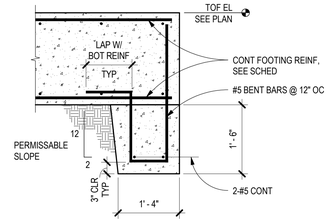
C2 HSS TO CONC WALL DETAIL
SCALE: 3/4" = 1'-0"



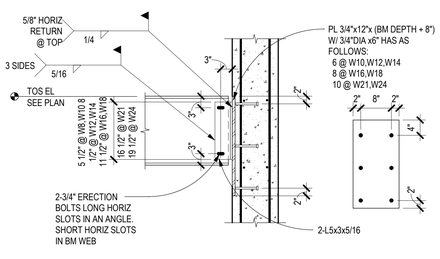
C3 PERIM COLUMN SECT @ E-1 & E-4 ONLY
SCALE: 3/4" = 1'-0"



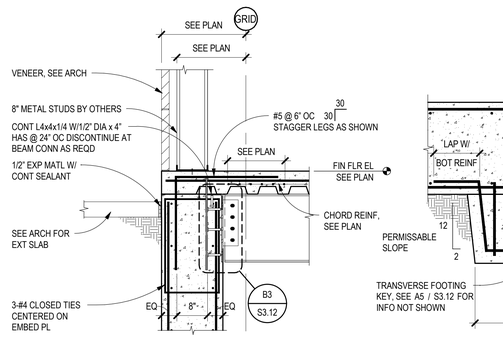
C4 BASEMENT WALL SECTION
SCALE: 3/4" = 1'-0"



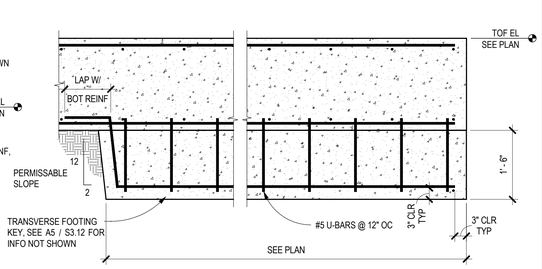
C5 TRANSVERSE FOOTING KEY SECTION
SCALE: 3/4" = 1'-0"



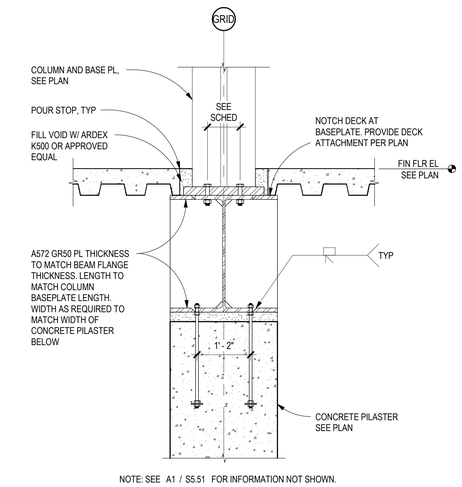
B3 BEAM TO CONC WALL DETAIL
SCALE: 3/4" = 1'-0"



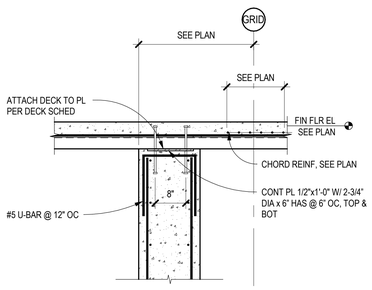
B4 BASEMENT WALL SECTION
SCALE: 3/4" = 1'-0"



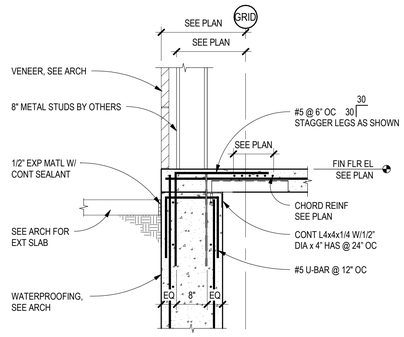
B5 TRANSVERSE FOOTING KEY SECTION
SCALE: 3/4" = 1'-0"



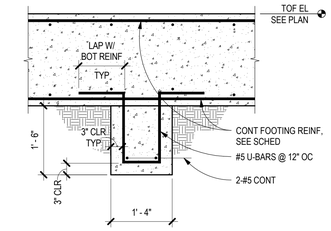
A2 MF COLUMN ON BEAM DETAIL
SCALE: 3/4" = 1'-0"



A3 BASEMENT WALL SECTION
SCALE: 3/4" = 1'-0"



A4 BASEMENT WALL SECTION
SCALE: 3/4" = 1'-0"



A5 TRANSVERSE FOOTING KEY SECTION
SCALE: 3/4" = 1'-0"

James R. Childers Architect, Inc.
45 South 4th Street
Fort Smith, AR 72901
479-783-2480
www.childersarchitect.com

PROFESSIONAL SEAL:

CONSULTANT LOGO:

Chavez-Grieves consulting engineers, inc.
4700 Lincoln Road NE, Suite 102 Albuquerque, NM 87109
505-344-4080 · 505-343-8759 (fax)

CLIENT:

COLLEGE OF Osteopathic Medicine
AT THE CHEROKEE NATION
TAHLEQUAH, OKLAHOMA

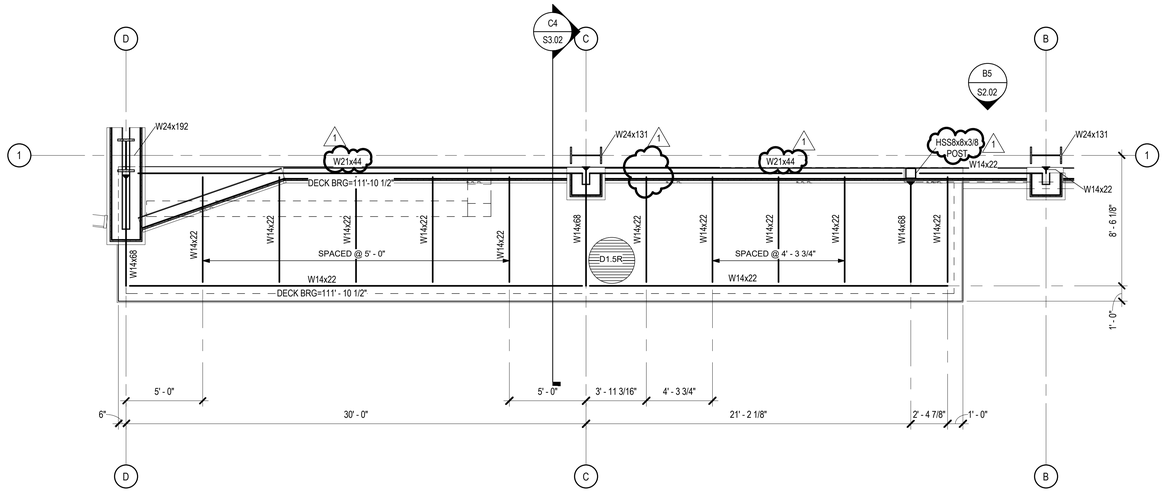
NOTE: THIS STRUCTURAL PACKAGE IS FOR FOUNDATIONS ONLY. ANY CHANGES TO THE PROJECT, INCLUDING, BUT NOT LIMITED TO: LOADING REQUIREMENTS, GEOMETRY CHANGES IN PLAN OR ELEVATION, SPACE USAGE REVISIONS, OR VALUE ENGINEERING MAY AFFECT THE STRUCTURAL STEEL MEMBER REQUIREMENTS SHOWN IN THESE DRAWINGS.

KEY PLAN:

PROJECT PHASE:
BID PACKAGE 03

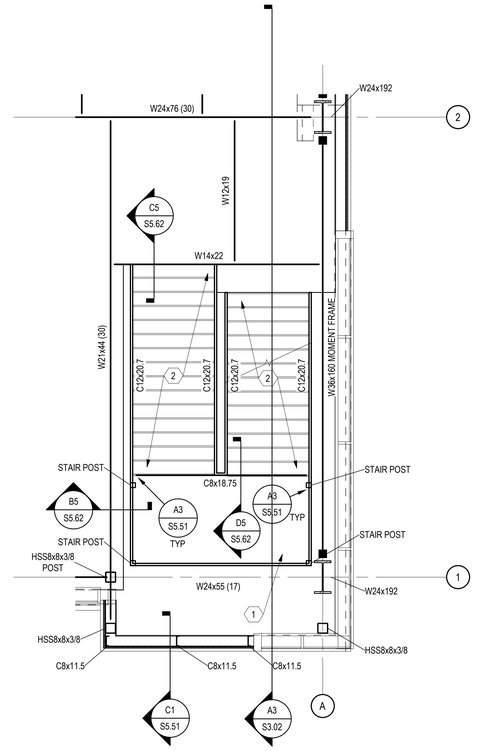
REVISIONS		
#	DATE	DESCRIPTION
1	4/2/19	BID PACKAGE 03 AS 01

DATE: 03-20-19
JOB NUMBER: 17-13
SHEET NUMBER: S3.12
FOUNDATION SECTIONS



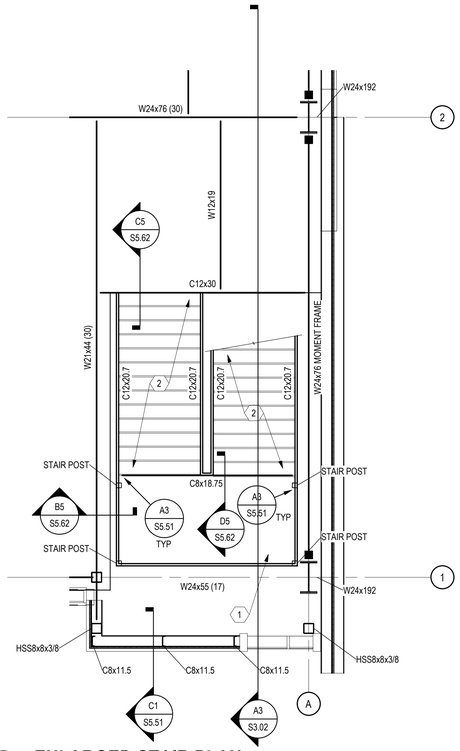
C1 ENTRY CANOPY FRAMING PLAN

SCALE: 1/4" = 1'-0"



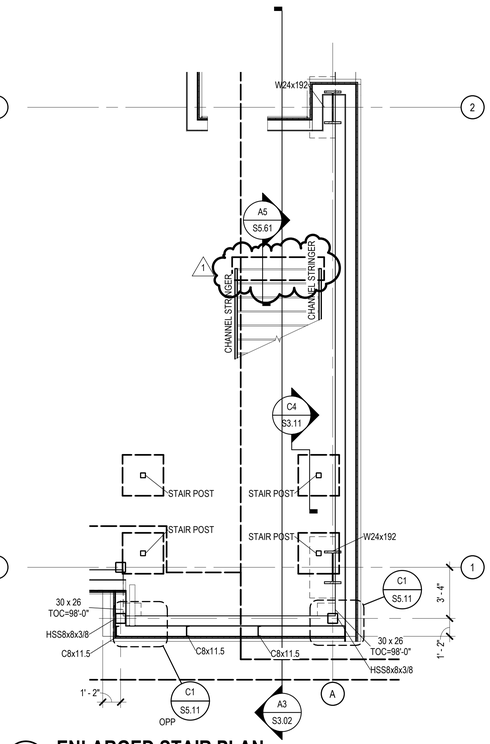
C4 ENLARGED STAIR PLAN

SCALE: 1/4" = 1'-0"



A3 ENLARGED STAIR PLAN

SCALE: 1/4" = 1'-0"



A4 ENLARGED STAIR PLAN

SCALE: 1/4" = 1'-0"

GENERAL SHEET NOTES

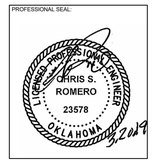
1. SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
2. DIMENSIONS ARE TO THE FACE OF CONCRETE, OR STUD UNLESS NOTED OTHERWISE. COORDINATE ALL STAIR DIMENSIONS WITH THE ARCHITECTURAL DRAWINGS.
3. SEE ARCHITECTURAL DRAWINGS FOR INTERMEDIATE LANDING ELEVATIONS.
4. SEE ARCHITECTURAL DRAWINGS FOR STAIR RISE AND RUN.
5. STRUCTURAL COLD FORMED METAL STUDS SHALL BE 600S162-43 AT 16" ON CENTER UNLESS NOTED OTHERWISE.

SHEET KEYNOTE

1. 4" NORMAL WEIGHT CONCRETE LANDING SLAB REINFORCED WITH 6x6-W2.1xW2.1 WELDED WIRE FABRIC IN FLAT SHEETS ONLY OVER METAL PAN. SEE ARCHITECTURAL DRAWINGS FOR LANDING ELEVATIONS.
2. 2" CONCRETE FILLED METAL PANS, REINFORCED WITH 6x6-W2.1xW2.1 WELDED WIRE FABRIC.

James R. Childers Architect, Inc.
45 South 4th Street
Fort Smith, AR 72901
479-783-2480
www.childersarchitect.com

PROFESSIONAL SEAL:



CONSULTANT LOGO:

Chavez-Grieves
consulting engineers, inc.
4700 Lincoln Road NE, Suite 102 Albuquerque, NM 87109
505-344-4080 505-343-8759 (fax)

CLIENT:

COLLEGE OF
Osteopathic Medicine
AT THE CHEROKEE NATION
TAHLEQUAH, OKLAHOMA

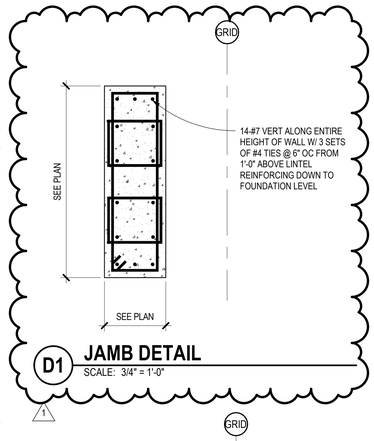
NOTE: THIS STRUCTURAL PACKAGE IS FOR FOUNDATIONS ONLY. ANY CHANGES TO THE PROJECT, INCLUDING, BUT NOT LIMITED TO: LOADING REQUIREMENTS, GEOMETRY CHANGES IN PLAN OR ELEVATION, SPACE USAGE REVISIONS, OR VALUE ENGINEERING MAY AFFECT THE STRUCTURAL STEEL MEMBER REQUIREMENTS SHOWN IN THESE DRAWINGS.

KEY PLAN:

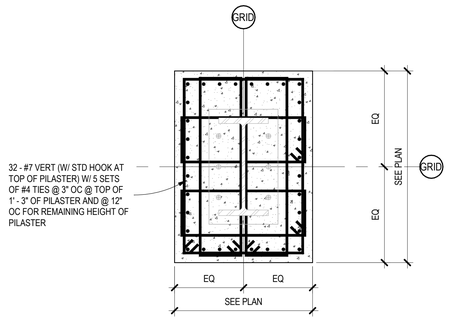
PROJECT PHASE:
BID PACKAGE 03

REVISIONS		
#	DATE	DESCRIPTION
1	4/22/19	BID PACKAGE 03 ADD 01

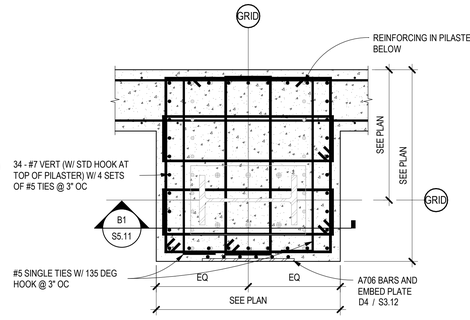
DATE: 03-20-19
JOB NUMBER: 17-13
SHEET NUMBER: S4.02
ENLARGED PLANS



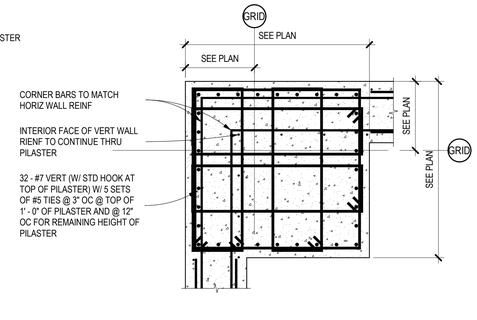
D1 JAMB DETAIL
SCALE: 3/4" = 1'-0"



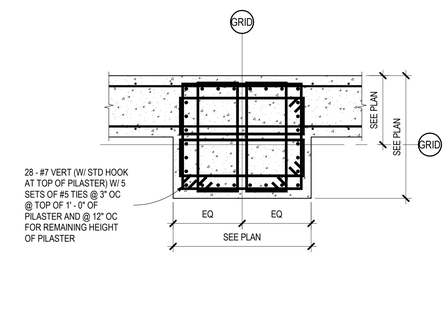
D2 PILASTER DETAIL
SCALE: 3/4" = 1'-0"



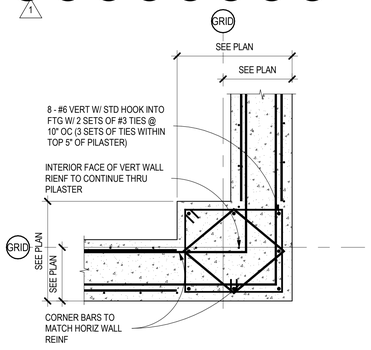
D3 PILASTER DETAIL - UPPER PILASTER
SCALE: 3/4" = 1'-0"



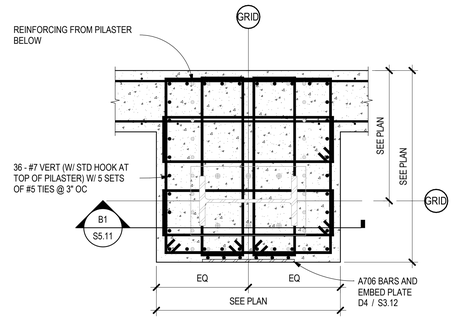
D4 PILASTER DETAIL - LOWER PILASTER
SCALE: 3/4" = 1'-0"



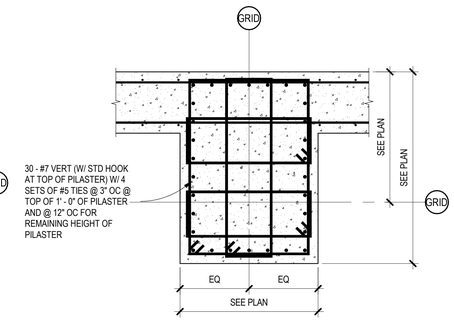
D5 PILASTER DETAIL - LOWER PILASTER
SCALE: 3/4" = 1'-0"



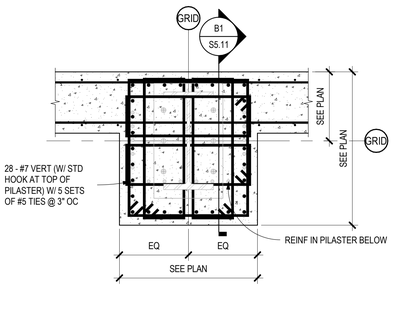
C1 PILASTER DETAIL - STAIR SHAFT
SCALE: 3/4" = 1'-0"



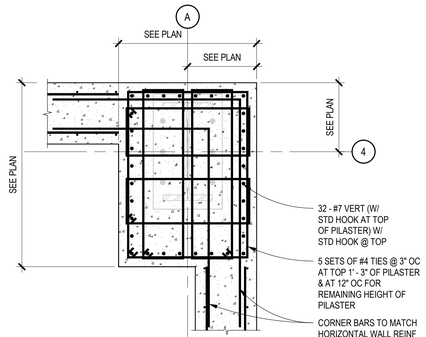
C2 PILASTER DETAIL - UPPER PILASTER
SCALE: 3/4" = 1'-0"



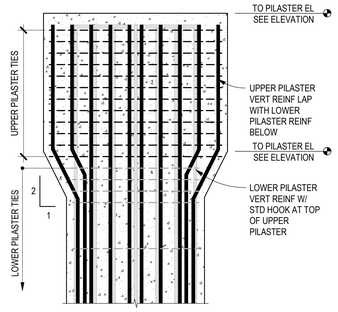
C3 PILASTER DETAIL - LOWER PILASTER
SCALE: 3/4" = 1'-0"



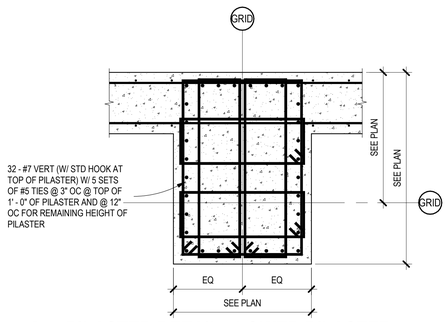
C4 PILASTER DETAIL - UPPER PILASTER
SCALE: 3/4" = 1'-0"



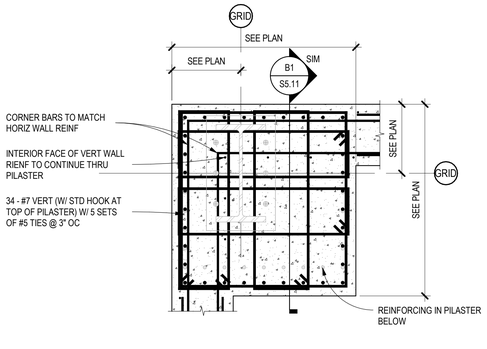
C5 PILASTER DETAIL
SCALE: 3/4" = 1'-0"



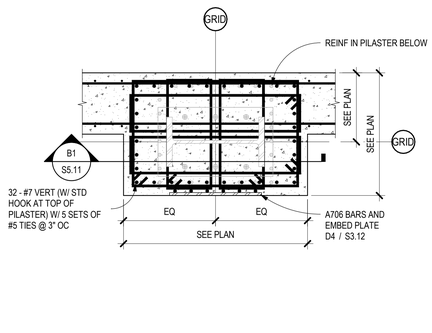
B1 PILASTER SPLICE SECTION
SCALE: 3/4" = 1'-0"



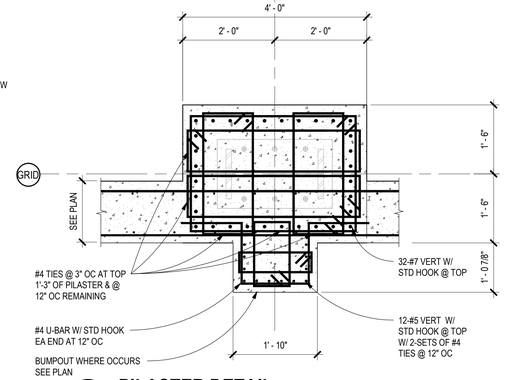
B2 PILASTER DETAIL - LOWER PILASTER
SCALE: 3/4" = 1'-0"



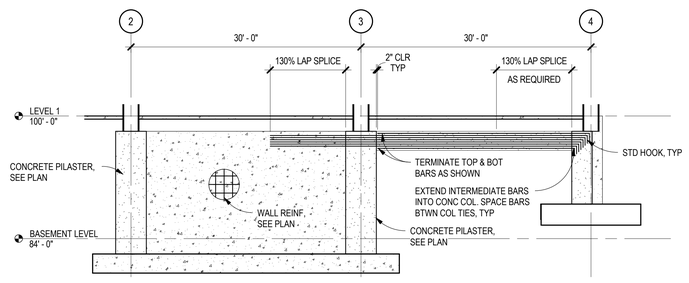
B3 PILASTER DETAIL - UPPER PILASTER
SCALE: 3/4" = 1'-0"



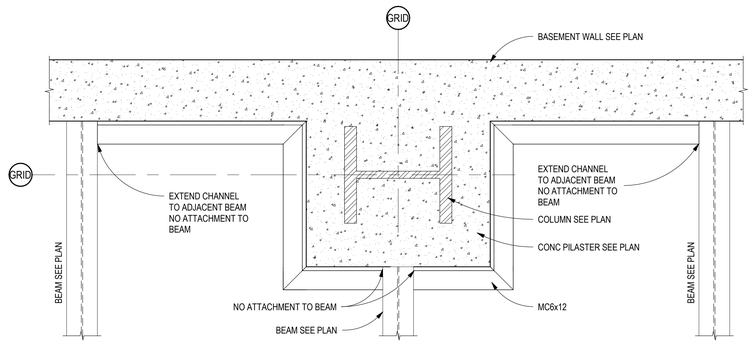
B4 PILASTER DETAIL - UPPER PILASTER
SCALE: 3/4" = 1'-0"



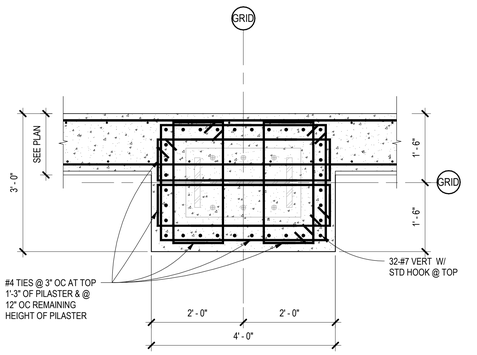
B5 PILASTER DETAIL
SCALE: 3/4" = 1'-0"



A1 SHEAR WALL ELEVATION DETAIL
SCALE: 1/8" = 1'-0"



A3 TYPICAL PERIMETER CHANNEL AT BASEMENT LID PLAN
SCALE: 3/4" = 1'-0"



A5 PILASTER DETAIL
SCALE: 3/4" = 1'-0"

NOTE: THIS STRUCTURAL PACKAGE IS FOR FOUNDATIONS ONLY. ANY CHANGES TO THE PROJECT, INCLUDING, BUT NOT LIMITED TO: LOADING REQUIREMENTS, GEOMETRY CHANGES IN PLAN OR ELEVATION, SPACE USAGE REVISIONS, OR VALUE ENGINEERING MAY AFFECT THE STRUCTURAL STEEL MEMBER REQUIREMENTS SHOWN IN THESE DRAWINGS.

James R. Childers Architect, Inc.
45 South 4th Street
Fort Smith, AR 72901
479-783-2480
www.childersarchitect.com

PROFESSIONAL SEAL:
CHRIS S. ROMERO 23578 OKLAHOMA 2019

CONSULTANT LOGO:
Chavez-Grieves consulting engineers, inc.
4700 Lincoln Road NE, Suite 102 Albuquerque, NM 87109
505-344-4080 505-343-8759 (fax)

CLIENT:
COLLEGE OF Osteopathic Medicine
AT THE CHEROKEE NATION
TAHLEQUAH, OKLAHOMA

KEY PLAN:

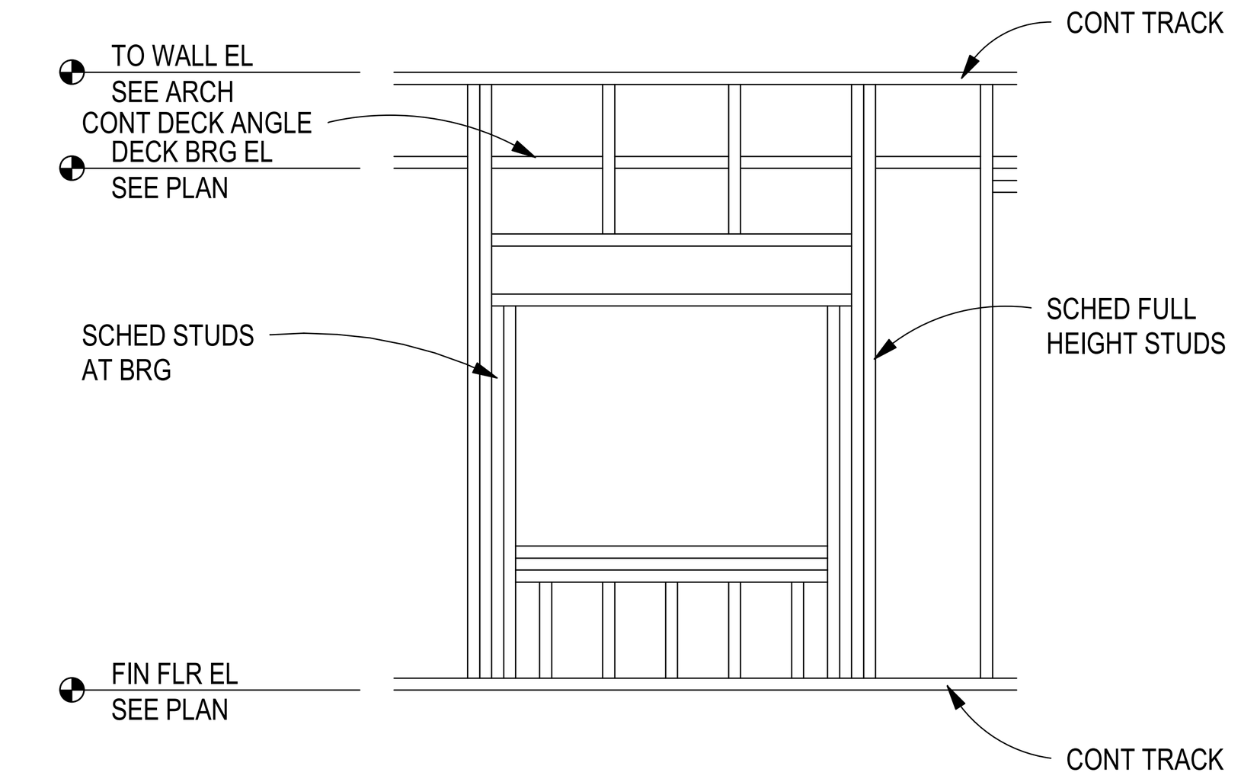
PROJECT PHASE:
BID PACKAGE 03

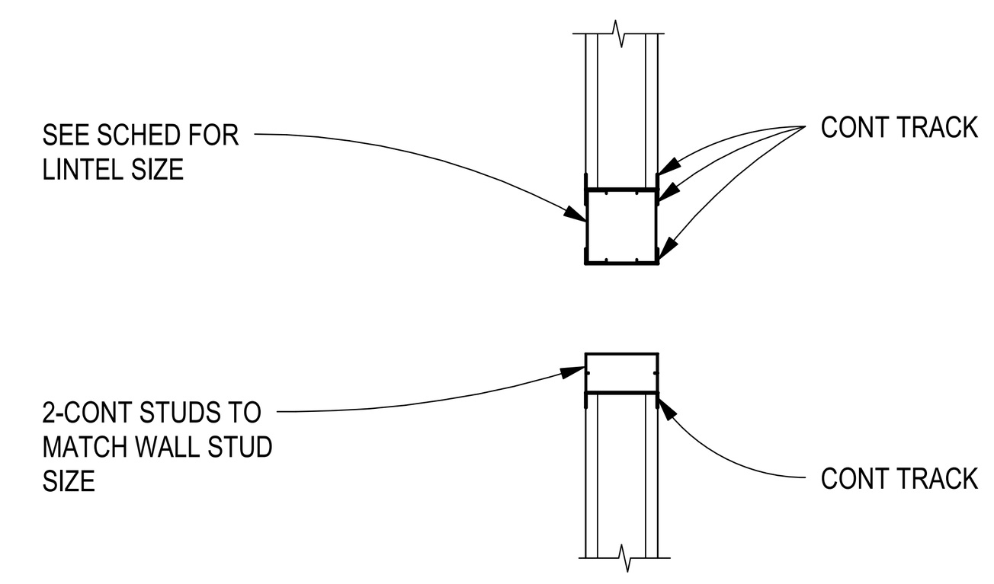
REVISIONS		
#	DATE	DESCRIPTION
1	4/2/19	BID PACKAGE 03 ADD 01

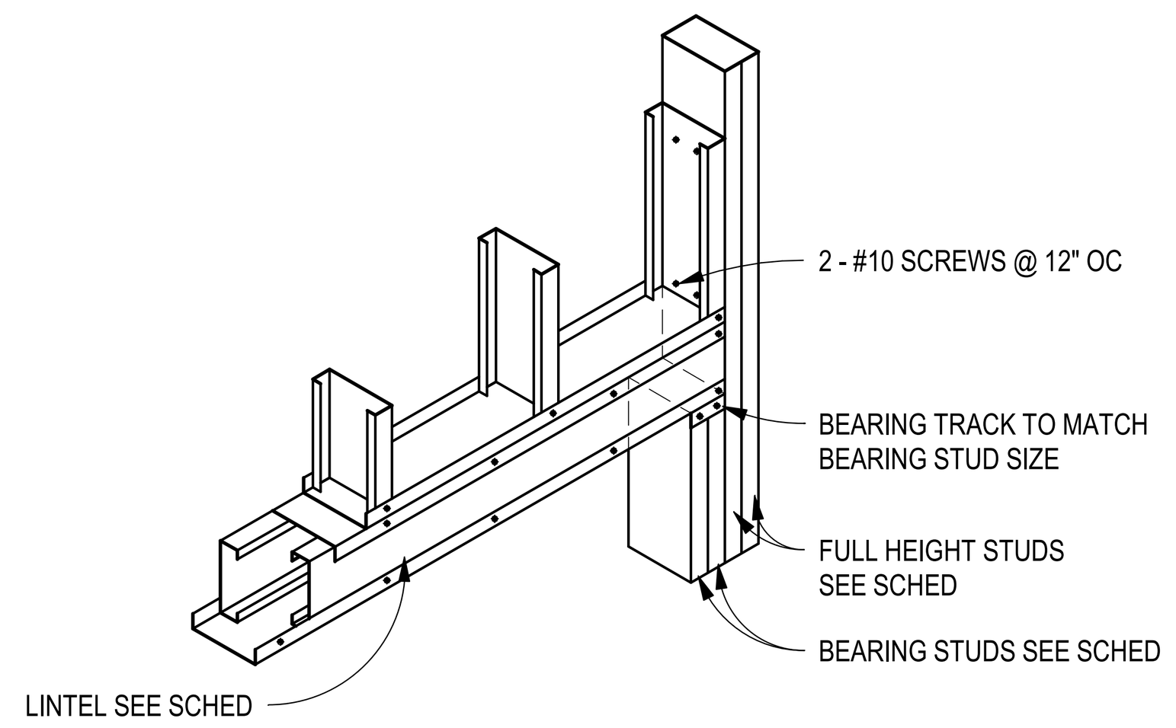
DATE:	JOB NUMBER:
03-20-19	17-13

SHEET NUMBER:
S5.11

CONCRETE DETAILS







DECK SCHEDULE

MARK	COMPOSITE SLAB	SLAB			METAL DECK				DECK ATTACHMENTS			TOTAL SLAB / DECK THICKNESS	COMMENTS
		THICK	MATL	REINF	THICK	TYPE	GAGE	FINISH	ATTACH PERP TO RIBS	ATTACH PARALLEL TO RIBS	ATTACH SIDELAPS		
D1.5R		- -	- -	- -	1 1/2"	B	20	PAINTED	4-5/8 "DIA PUDDLE WELDS PER 36 "WIDE SHEET	5/8" DIA PUDDLE WELDS @ 12" OC	#10 SCREWS @ 12" OC	1 1/2"	
D6R	X	3"	NW CONC	6x6 - W2.1xW2.1 WELD WIRE FABRIC IN FLAT SHEETS	3"	VLI	18	GALVANIZED	[4-5/8 "DIA PUDDLE WELDS] PER 36 "WIDE SHEET	[5/8" DIA PUDDLE WELDS] @ [12]" OC	[#10} SCREWS @ [12]" OC	6"	
D7.5F	X	4 1/2"	NW CONC	#5 @ 12" OC EA WAY	3"	VLI	18	GALVANIZED	[4-5/8 "DIA PUDDLE WELDS] PER 36 "WIDE SHEET	[5/8" DIA PUDDLE WELDS] @ [12]" OC	[#10} SCREWS @ [12]" OC	7 1/2"	PROVIDE 3/4" DIAMETER x 6" LONG HEADED ANCHOR STUDS WHEN SPECIFIED ON PLAN

SLAB-ON-GRADE SCHEDULE

MARK	SLAB		REINFORCING	BEARING STRATA	COMMENTS
	THICKNESS	MATL			
S5	5"	CONC	#4 @ 18" OC EA WAY	15 MIL VAPOR RETARDER OVER LASER LEVELED 4" MINIMUM GRANULAR BASE OVER COMPACTED SUBGRADE PER GEN STRUCT NOTES	THE FINISHED TOP SURFACE SHOULD BE FLAT AND LEVEL WITH SUFFICIENT FINES TO FILL BETWEEN COARSE AGGREGATE. IF THIS IS NOT THE CASE, PROVIDE UP TO 1/2 INCH BLOTTER LAYER OF SAND
S6	5"	CONC	#4 @ 12" OC EA WAY	15 MIL VAPOR RETARDER OVER LASER LEVELED 4" MINIMUM GRANULAR BASE OVER COMPACTED SUBGRADE PER GEN STRUCT NOTES	THE FINISHED TOP SURFACE SHOULD BE FLAT AND LEVEL WITH SUFFICIENT FINES TO FILL BETWEEN COARSE AGGREGATE. IF THIS IS NOT THE CASE, PROVIDE UP TO 1/2 INCH BLOTTER LAYER OF SAND

1

GRAVITY BASE PLATE SCHEDULE

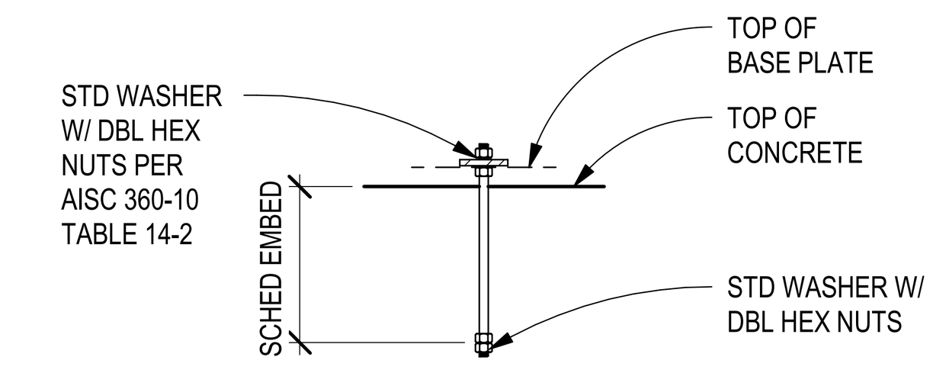
BASE PLATE			ANCHOR BOLTS
COLUMN SIZE	TYPE	SIZE "T"x"A"x"B"	QTY x SIZE x GRADE
W12	A	PL 2"x21"x1' - 9"	4 - 3/4" DIA x 9" F1554 GR 36
HSS6x6	A	PL 3/4"x14"x1' - 2"	4 - 3/4" DIA x 9" F1554 GR 36
HSS8x8	A	PL 3/4"x16"x1' - 4"	4 - 3/4" DIA x 9" F1554 GR 36

NOTE: FOR GRAVITY BASE PLATES, PROVIDE MAXIMUM OVERSIZED HOLES AND PLATE WASHERS WITH SIZE AND THICKNESS PER AISC 360-10 TABLE 14-2.

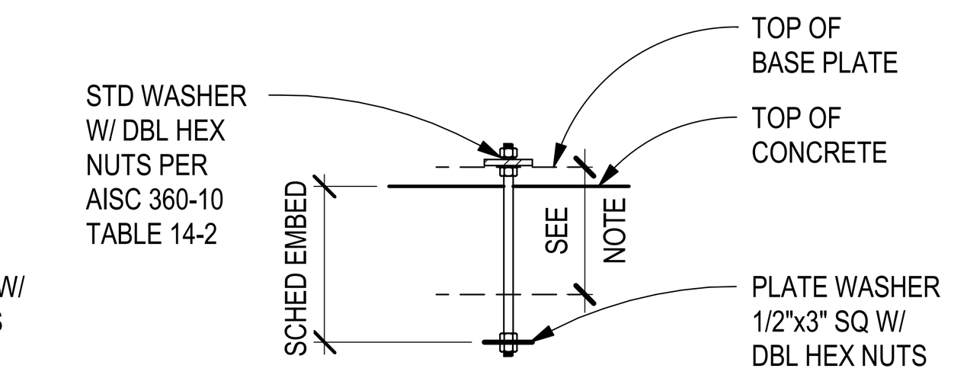
LATERAL BASE PLATE SCHEDULE

BASE PLATE			ANCHOR BOLTS
MARK	TYPE	SIZE "T"x"A"x"B"	QTY x SIZE x GRADE
BP1	B	PL 2 1/4"x30"x1'-6"	6 - 1 1/4" DIA x 18" F1554 GR 36
BP2	B	PL 2 1/2"x30"x1'-6"	6 - 1 1/4" DIA x 18" F1554 GR 36
BP3	C	PL 2 1/4"x30"x1'-6"	SEE A5 / S5.51
BP4	B	PL 2 1/4"x30"x1'-6"	6 - 1 1/4" DIA x 24" F1554 GR 36

NOTE: FOR LATERAL BASE PLATES, PROVIDE PLATE WASHERS WITH SIZE AND THICKNESS PER AISC 360-10 TABLE 14-2.

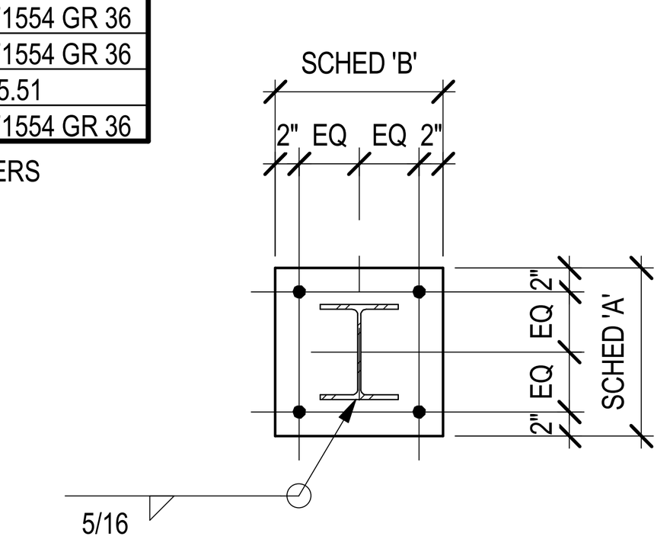


GRAVITY ANCHOR BOLT

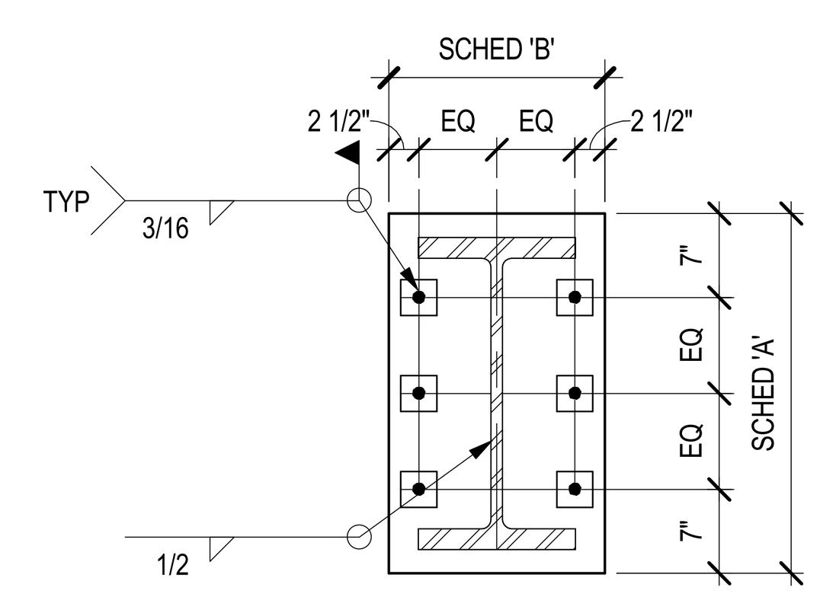


NOTE: FIRST 1'-0" OF EMBEDDED PORTION OF ANCHOR BOLT SHALL BE UNTHREADED

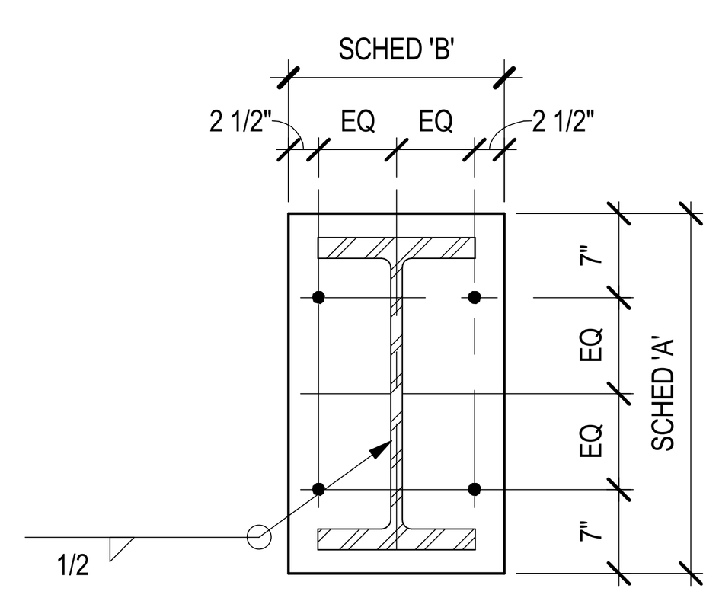
LATERAL ANCHOR BOLT



TYPE 'A'



TYPE 'B'



TYPE 'C'

WALL SCHEDULE

MARK	VENEER	WALL	REINFORCING			COMMENTS
			VERTICAL	HORIZONTAL	GRADE	
WC8	- -	8" CONC	#4 @ 12" OC	#4 @ 12" OC	A615	
WC8A	- -	8" CONC	#5 @ 9" OC	#4 @ 12" OC	A615	SEE D2/S3.11
WC12	- -	12" CONC	#5 @ 12" OC EA FACE	#4 @ 12" OC EA FACE	A615	
WC16	- -	16" CONC	#7 @ 12" OC EA FACE	#5 @ 12" OC EA FACE	A615	
WC16A	- -	16" CONC	#7 @ 12" OC EA FACE	#5 @ 12" OC EA FACE	A615	
WC16B	- -	16" CONC	#5 @ 12" OC EA FACE	#5 @ 12" OC EA FACE	A615	
WC22	- -	22" CONC	#5 @ 12" OC EA FACE	#5 @ 12" OC EA FACE	A615	

REINFORCING LAP SCHEDULE

REINFORCEMENT TYPE	#6 AND SMALLER (#db)			#7 AND LARGER (#db)			MINIMUM LENGTH (IN)	COMMENTS
	3000 PSI	4000 PSI	5000 PSI	3000 PSI	4000 PSI	5000 PSI		
CONTINUOUS WALL FOOTINGS AND STEMWALLS	30	30	30	30	30	30	18	
RETAINING WALLS AND BASEMENT WALL VERTICAL REINFORCING	57	50	45	72	62	56	12	
RETAINING WALLS AND BASEMENT WALL HORIZONTAL REINFORCING	57	50	45	72	62	56	12	
CONCRETE COLUMNS NOT SUPPORTING LATERAL FORCES	30	30	30	30	30	30	12	
CONCRETE COLUMNS SUPPORTING LATERAL FORCES	57	50	45	72	62	56	12	
TOP FLEXURAL REINFORCEMENT, INCLUDING BEAMS, GRADE BEAMS, AND COMBINED COLUMN FOOTING AT BRACED FRAME AND MOMENT FRAMES	57	50	45	72	62	56	12	
BOTTOM FLEXURAL REINFORCEMENT, INCLUDING BEAMS, GRADE BEAMS, AND COMBINED COLUMN FOOTING AT BRACED FRAME AND MOMENT FRAMES	57	50	45	55	62	56	12	
SLABS-ON-GRADE	30	30	30	30	30	30	12	
MINIMUM EMBEDMENT OF STANDARD HOOKS INTO CONCRETE BASE	16	14	12	16	14	14	6	INCREASE LENGTH FOR # 11 BARS AND LARGER BY A FACTOR OF 1.4
ALL REBAR LAPS IN CMU	72			72			12	

SPOT FOOTING SCHEDULE

MARK	SIZE			REINFORCING		COMMENTS
	WIDTH	LENGTH	DEPTH	BOTTOM	TOP	
F36	3' - 0"	3' - 0"	1' - 0"	4 - #4 EA WAY	---	STD HOOK EACH END OF EACH BAR
F96	8' - 0"	8' - 0"	2' - 0"	10 - #7 EA WAY	10 - #7 EA WAY	
F108	9' - 0"	9' - 0"	2' - 3"	12 - #7 EA WAY	12 - #7 EA WAY	
F120	10' - 0"	10' - 0"	2' - 6"	14 - #7 EA WAY	14 - #7 EA WAY	
F132	11' - 0"	11' - 0"	2' - 9"	12 - #8 EA WAY	12 - #8 EA WAY	
F144	12' - 0"	12' - 0"	2' - 9"	15 - #8 EA WAY	15 - #8 EA WAY	
F156	13' - 0"	10' - 0"	2' - 0"	15 - #7 EA WAY	---	CONTINUOUS FOOTING REINFORCEMENT SHALL BE CONTINUOUS THROUGH FOOTING
F156A	13' - 0"	13' - 0"	2' - 9"	18 - #8 EA WAY	18 - #8 EA WAY	CONTINUOUS FOOTING REINFORCEMENT SHALL BE CONTINUOUS THROUGH FOOTING
F168	14' - 0"	14' - 0"	3' - 0"	18 - #9 EA WAY	18 - #9 EA WAY	
F180	15' - 0"	15' - 0"	3' - 3"	22 - #9 EA WAY	22 - #9 EA WAY	
F192	16' - 0"	12' - 0"	3' - 3"	20 - #9 EA WAY	20 - #9 EA WAY	
F480	12' - 0"	40' - 0"	2' - 6"	#8 @ 9" OC EA WAY	#8 @ 9" OC EA WAY	
F504	12' - 0"	42' - 0"	2' - 6"	#8 @ 6" OC EA WAY	#8 @ 6" OC EA WAY	
F528	13' - 0"	44' - 0"	2' - 6"	#8 @ 6" OC EA WAY	#8 @ 6" OC EA WAY	
F828	13' - 0"	69' - 0"	2' - 6"	#8 @ 6" OC EA WAY	#8 @ 6" OC EA WAY	

CONTINUOUS FOOTING SCHEDULE

MARK	SIZE		REINFORCING		COMMENTS
	WIDTH	DEPTH	CONTINUOUS	TRANSVERSE	
CF36	3' - 0"	1' - 0"	4 - #4 BOT	#4 @ 12" OC BOT	
CF48	4' - 0"	1' - 0"	5 - #5 BOT	#5 @ 12" OC BOT	
CF60	5' - 0"	1' - 3"	8 - #5 BOT	#5 @ 12" OC BOT	
CF72	6' - 0"	1' - 3"	10 - #5 BOT	#5 @ 12" OC BOT	
CF84	7' - 0"	1' - 3"	12 - #5 TOP & BOT	#5 @ 12" OC TOP & BOT	
CF96	8' - 0"	1' - 6"	10 - #7 TOP & BOT	#7 @ 12" OC TOP & BOT	
CF120	10' - 0"	1' - 6"	12 - #7 TOP & BOT	#7 @ 12" OC TOP & BOT	
CF132	11' - 0"	1' - 6"	12 - #7 TOP & BOT	#7 @ 12" OC TOP & BOT	
CF156	13' - 0"	2' - 0"	18 - #7 TOP & BOT	#7 @ 12" OC TOP & BOT	

1

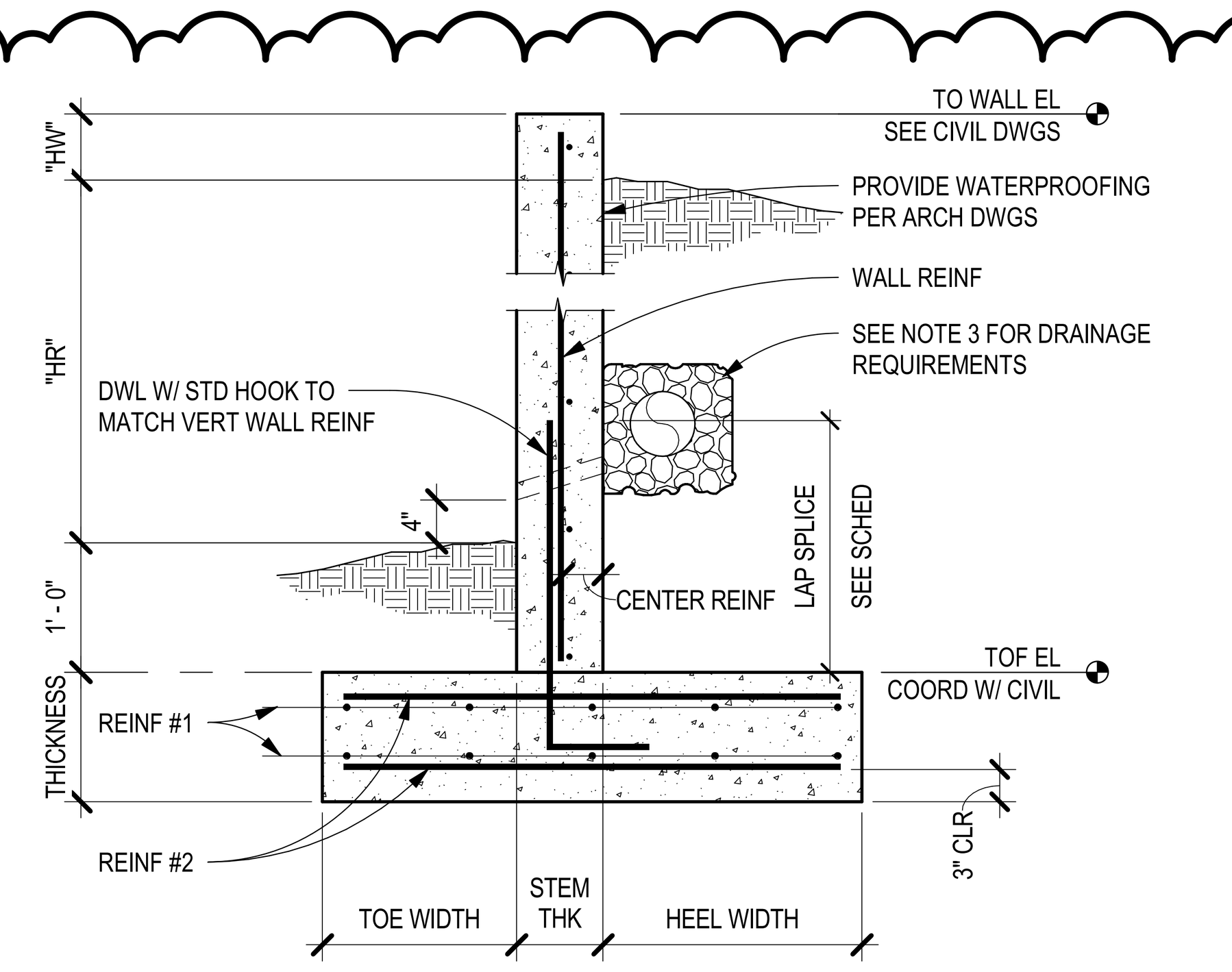
CONCRETE SITE RETAINING WALL SCHEDULE

WALL TYPE	STEM THICKNESS	WALL HEIGHT ABOVE GRADE "HW"	HEIGHT RETAINED "HR"	FOOTING SIZE			FOOTING REINFORCING		WALL REINFORCING	
				TOE WIDTH	HEEL WIDTH	THICKNESS	REINF #1	REINF #2		
	IN	FT-IN	FT-IN	FT-IN	FT-IN	FT-IN	QTY - SIZE	SIZE - SPACING	VERT SIZE - SPACING	HORIZ SIZE - SPACING
A	8"	0'-0" - 0'-6"	0'-0" - 4'-0"	1'-6"	3'-4"	1'-3"	6 - #5 CONT T&B	#5 @ 18" OC T&B	#5 @ 12" OC	#4 @ 12" OC
			4'-1" - 6'-0"	2'-0"	5'-4"	1'-6"	9 - #5 CONT T&B	#5 @ 12" OC T&B	#7 @ 12" OC	#4 @ 12" OC
B	10"	0'-0" - 0'-6"	6'-1" - 8'-0"	2'-6"	5'-8"	1'-6"	10 - #5 CONT T&B	#5 @ 12" OC T&B	#6 @ 12" OC EA FACE	#4 @ 12" OC EA FACE
	12"	0'-0" - 0'-6"	8'-1" - 10'-0"	3'-0"	6'-0"	2'-0"	12 - #5 CONT T&B	#5 @ 12" OC T&B	#7 @ 12" OC EA FACE	#4 @ 12" OC EA FACE

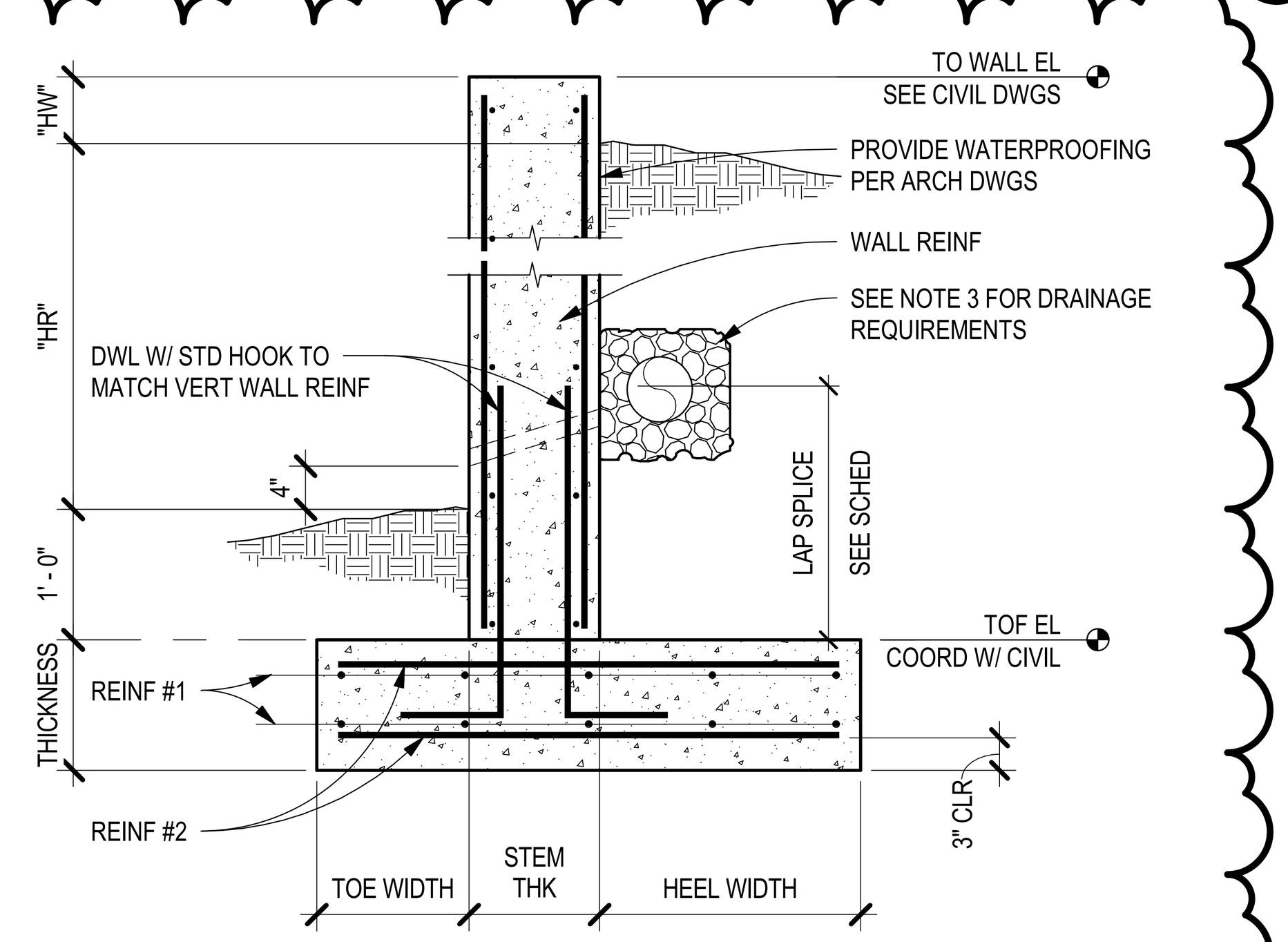
NOTES:

1. COORDINATE EXACT LOCATION AND EXTENT OF WALL WITH ARCHITECTURAL AND CIVIL DWGS.
2. PROVIDE CONCRETE WALL VERTICAL CONTROL JOINTS AT (2) TIMES THE WALL HEIGHT AND AT ALL STEPS IN TOP OF WALL. SEE DETAIL C2/S7.11 FOR CONTROL JOINT INFORMATION.
3. WALL DRAINAGE SYSTEM SHALL CONSIST OF ONE OF THE FOLLOWING:
   A. CONTINUOUS 4" DIAMETER SLOTTED SCHEDULE 40 DRAIN PIPE WITH SLOTS ON BOTTOM HALF OF PIPE.
   B. 1 1/2" (MIN) DIA PVC WEEP PIPE @ 8'-0" OC MAX W/ 12"x12"x12" GRAVEL & MIRAFI 140N GEOTEXTILE FILTER FABRIC BETWEEN SOIL & GRAVEL @ EA WEEP HOLE OR WEEP HOLES.
   C. TIE-IN TO SITE DRAINAGE SYSTEM. BACKFLOW PREVENTION, ETC. TO BE PROVIDED BY OTHERS.

SEE GEOTECHNICAL REPORT FOR ADDITIONAL INFORMATION.



WALL TYPE "A"



WALL TYPE "B"

1

A1 **TYPICAL CONCRETE SITE RETAINING WALL SCHEDULE AND DETAIL**

SCALE: 3/4" = 1'-0"

NOTE: THIS STRUCTURAL PACKAGE IS FOR FOUNDATIONS ONLY. ANY CHANGES TO THE PROJECT, INCLUDING, BUT NOT LIMITED TO: LOADING REQUIREMENTS, GEOMETRY CHANGES IN PLAN OR ELEVATION, SPACE USAGE REVISIONS, OR VALUE ENGINEERING MAY AFFECT THE STRUCTURAL STEEL MEMBER REQUIREMENTS SHOWN IN THESE DRAWINGS.

James R. Childers
Architect, Inc.
45 South 4th Street
Fort Smith, AR 72901
479-783-2480
www.childersarchitect.com

PROFESSIONAL SEAL:

LICENSED PROFESSIONAL ENGINEER
CHRIS S.
ROMERO
23578
OKLAHOMA
3.2019

CONSULTANT LOGO:

Chavez-Grieves
consulting engineers, inc.
4700 Lincoln Road NE, Suite 102 · Albuquerque, NM 87109
505-344-4080 · 505-343-8759 (fax)

CLIENT:

SEAL OF THE CHEROKEE NATION
SEPT. 6, 1839

COLLEGE OF
Osteopathic Medicine
AT THE CHEROKEE NATION
TAHLEQUAH, OKLAHOMA

KEY PLAN:

PROJECT PHASE:
BID PACKAGE 03

REVISIONS

#	DATE	DESCRIPTION
1	4/22/19	BID PACKAGE 03 ASI 01

DATE: 03-20-19

JOB NUMBER: 17-13

SHEET NUMBER: S6.01

SCHEDULES