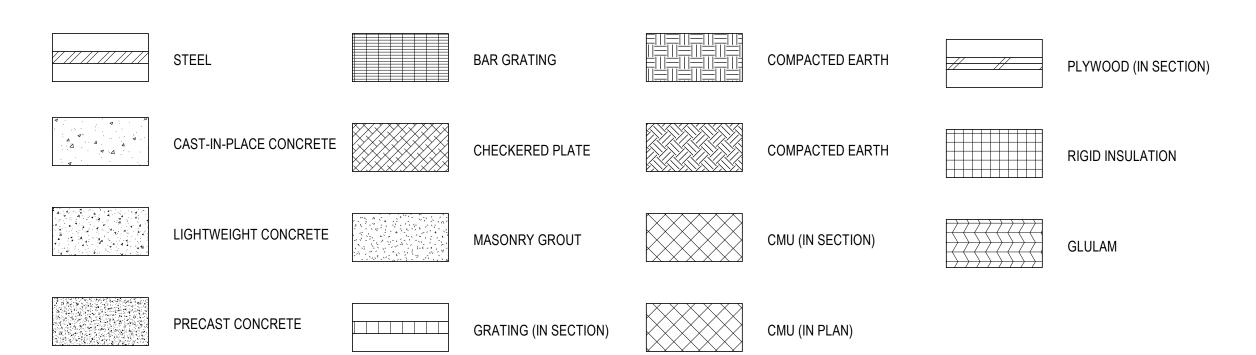


STRUCTURAL MATERIALS LEGEND

SEE SHEET XX/S-XXX





GRADATION (ASTM C136)

SIEVE SIZE PE	RCENT PASSING BY WEIGHT	
1" 3/4" NO. 4 NO. 200	100 85-100 45-95 0-8	
PLASTICITY INDEX (ASTM D4318):	3 MAXIMUM	
THE COLIDGE ACCDECATE SHALL	JAVE A DEDCENT WEAD OF 50 OD LESS WHEN TESTED IN ACCORDANCE I	۱۸/

THE COURSE AGGREGATE SHALL HAVE A PERCENT WEAR OF 50 OR LESS WHEN TESTED IN ACCORDANCE WITH **ASTM C131.**

THE FINISHED TOP SURFACE SHOULD BE FLAT AND LEVEL WITH SUFFICIENT FINES TO FILL BETWEEN COARSE AGGREGATE. IF THIS IS NOT THE CASE, PROVIDE UP TO 1/2 INCH BLOTTER LAYER OF SAND.

COMPACTION REQUIREMENTS:

IN ACCORDANCE WITH ASTM D698 (STANDARD PROCTOR), SUBGRADE SOILS AND STRUCTURAL FILL MATERIALS SHALL BE COMPACTED TO THE FOLLOWING PERCENTAGES OF THE MAXIMUM DRY DENSITY AT +/- 2% (OR 0 TO 4%) FOR CH SOIL TYPES) OPTIMUM MOISTURE CONTENT:

<u>MATERIAL</u>	MINIMUM PERCENT COMPACTION
STRUCTURAL FILL IN THE BUILDING AREA SUBBASE FOR SLAB SUPPORT SUBGRADE BELOW STRUCTURAL FILL MISCELLANEOUS BACKFILL	95 95 95 90

GENERAL STRUCTURAL NOTES

SITE RETAINING WALL DESIGN CRITERIA: **EQUIVALENT FLUID PRESSURE** ACTIVE EARTH PRESSURE FOR ON SITE SOIL BACKFILL 35 PCF UNRESTRAINED ULTIMATE PASSIVE EARTH PRESSURE 460 PCF

50 PCF UNRESTRAINED

PG = 10 PSF

PF = 16 PSF

CE = 0.9

IS = 1.10

CT = 1.0

120 MPH

GCPI = 0.18

IS = 1.25

SS = 0.152G

S1 = 0.081G

SDS = 0.122G

SD1 = 0.092G

CS = 0.043

SPREAD FOOTINGS: 5000 I

R = 3

BC-15 INTERNATIONAL BUILDING CODE 2015 ASCE/SEI 3-91 STRUCTURAL DESIGN OF COMPOSITE SLABS

ASCE/SEI 7-10 MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES AISC 360-10 SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS AISC 341-10 SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS

AISC MANUAL OF STEEL CONSTRUCTION 14TH EDITION SDI DIAPHRAGM DESIGN MANUAL, 3RD EDITION ANSI/SDI NC1.0-06 STANDARD FOR NONCOMPOSITE STEEL FLOOR DECK

ANSI/SDI C1.0-06 STANDARD FOR COMPOSITE STEEL FLOOR DECK AISI S100-12 NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS AISI S200-12 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING – GENERAL PROVISIONS

AISI S211-07 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - WALL STUD DESIGN WITH 2012

AISI S212-12 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - HEADER DESIGN AISI S213-12 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - LATERAL DESIGN WITH 2010

AISI S214-12 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - TRUSS DESIGN, WITH

ACI 318-14 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE ACI 530-13 BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES ACI 530.1-13 SPECIFICATIONS FOR MASONRY STRUCTURES

AWS D1.3-98 STRUCTURAL WELDING CODE - SHEET STEEL AWS D1.4-11 STRUCTURAL WELDING CODE – REINFORCING STEEL

LIVE LOAD	
FLOOR	80 PSF
STAIRS AND EXIT-WAYS*	100 PSF
*MINIMUM CONCENTRATED LOAD	300 LBS
ASSEMBLY AREAS	100 PSF
SAFER ROOM	100 PSF
STORAGE	150 PSF
LIBRARY/ BOOKSTORE	150 PSF
GYM	150 PSF
ADDITIONAL SUPERIMPOSED LOADS	
PARTITIONS	15 PSF
SUSPENDED EQUIPMENT	10 PSF
CONCENTRATED LOAD	2000 LBS
(PER IBC 1607.4)	
ROOF LIVE LOAD: LR = 20*R1*R2	20 PSF

REDUCTION FACTOR BASED ON TRIB AREA REDUCTION FACTOR BASED ON ROOF SLOPE R2 = 1.0

3/4"	85-100	
NO. 4	45-95	THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS IN THE FIELD.
NO. 200	0-8	
	,	SHOP DRAWINGS SHALL BE FURNISHED AND REVIEWED BEFORE ANY FABRICATION OR ERECTION IS STARTED
ICITY INDEX (ASTM D4318):	3 MAXIMUM	CONTRACTOR SHALL REVIEW AND APPROVE SHOP DRAWINGS PRIOR TO SUBMITTAL TO THE ARCHITECT FOR
,	,	REVIEW. POORLY EXECUTED SHOP DRAWINGS WILL BE REJECTED AND SHALL BE RESUBMITTED.
OLIDGE ACCDECATE SHALL HAV	WE A DEDCENT WEAD OF 50 OD LESS WHEN TESTED IN ACCORDANCE WITH	

BEE ARCHITECTURAL PLANS FOR INTERIOR NON-BEARING PARTITION WALLS. PARTITION FRAMING SHALL BE

CONNECTED TO THE PRIMARY STRUCTURE TO ALLOW FOR VERTICAL LIVE LOAD DEFLECTIONS OF SPAN/360 FOR

CONTRACTOR'S CONSTRUCTION AND/OR ERECTION SEQUENCES SHALL RECOGNIZE AND CONSIDER THE EFFECTS

THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING SAFE AND ADEQUATE SHORING FOR ALL PARTS OF THE STRUCTURE DURING CONSTRUCTION.

TEMPORARY PROVISIONS SHALL BE MADE FOR STRUCTURAL STABILITY DURING CONSTRUCTION. THE STRUCTURE 📗 E. THE CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVAL OF ANY SOIL SLOUGHAGE FROM THE WET SHOWN ON THE DRAWINGS HAS BEEN DESIGNED FOR STABILITY UNDER FINAL CONFIGURATION.

NOTCHING OR CUTTING ANY STRUCTURAL MEMBER IN THE FIELD IS PROHIBITED.

OF THERMAL MOVEMENTS OF STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PERIOD.

FLOOR FRAMING AND SPAN/240 FOR ROOF FRAMING.

THE CONTRACTOR SHALL VERIFY THE SIZE AND LOCATION OF FOUNDATIONS UNDER MECHANICAL AND ELECTRICAL EQUIPMENT AS REQUIRED. NO CONCRETE PADS SHALL BE LOCATED ON ROOF UNLESS SHOWN ON THE STRUCTURAL DRAWINGS.

BACKFILL SHALL NOT BE PLACED BEHIND RETAINING WALLS UNTIL CONCRETE HAS ATTAINED 100 PERCENT OF DESIGN STRENGTH.

BACKFILL SHALL NOT BE PLACED BEHIND BASEMENT WALLS UNTIL THE CONCRETE HAS ATTAINED 100 PERCENT OF DESIGN STRENGTH AND THE ELEVATED FLOOR PROVIDING LATERAL SUPPORT AT THE TOP OF THE WALL IS COMPLETELY CONSTRUCTED, OR TEMPORARY BRACING/SHORING OF THE WALL IS PROVIDED. DESIGN OF ANY TEMPORARY WALL BRACING/SHORING IS THE RESPONSIBILITY OF THE CONTRACTOR.

REMOVAL OF FORMS AND SHORING SHALL BE IN ACCORDANCE WITH ACI 347. WHERE CONCRETE MUST SUPPORT SUPERIMPOSED LOADS PRIOR TO ATTAINING THE SPECIFIED DESIGN STRENGTH. RESHORE CONCRETE IN ACCORDANCE WITH ACI 347. RESHORING SHALL NOT BE REMOVED SOONER THAN 28 DAYS FROM THE DATE OF POUR OR UNTIL CONCRETE HAS ATTAINED THE SPECIFIED DESIGN STRENGTH.

THE CONTRACTOR SHALL SUBMIT FOR PRIOR APPROVAL THE END OF POUR LOCATIONS FOR CONCRETE GRADE BEAMS, CONCRETE COLUMNS, AND CONCRETE BEAMS.

GENERAL STRUCTURAL NOTES

THE CONTRACTOR SHALL BE RESPONSIBLE FOR ADHERING TO ALL APPLICABLE STANDARDS SET FORTH BY OSHA INCLUDING THE FOLLOWING REQUIREMENTS FROM STANDARDS - 29 CFR, SECTION 1926, SUBPART R:

A. THE STEEL ERECTION CONTRACTOR SHALL NOT ERECT STEEL UNLESS THEY HAVE RECEIVED WRITTEN NOTIFICATION FROM THE CONTRACTOR THAT THE CONCRETE IN THE FOOTINGS, PIERS AND WALLS OR THE MORTAR IN THE MASONRY PIERS AND WALLS HAS ATTAINED, ON THE BASIS OF AN APPROPRIATE ASTM STANDARD TEST METHOD OF FIELD-CURED SAMPLES, EITHER 75 PERCENT OF THE INTENDED MINIMUM COMPRESSIVE DESIGN STRENGTH OR SUFFICIENT STRENGTH TO SUPPORT THE LOADS IMPOSED DURING STEEL ERECTION.

PROVIDE STRUCTURAL ENGINEER A COPY OF WRITTEN NOTIFICATION WHEN IT IS PROVIDED TO THE STEEL ERECTOR.

ANCHOR RODS (ANCHOR BOLTS) SHALL NOT BE REPAIRED, REPLACED OR FIELD-MODIFIED WITHOUT THE APPROVAL OF THE PROJECT STRUCTURAL ENGINEER OF RECORD.

PRIOR TO ERECTION OF COLUMNS, THE CONTRACTOR SHALL PROVIDE WRITTEN NOTIFICATION TO THE STEEL ERECTOR IF THERE HAS BEEN ANY REPAIR, REPLACEMENT OR MODIFICATION OF THE ANCHOR RODS (ANCHOR BOLTS).

PROVIDE STRUCTURAL ENGINEER A COPY OF WRITTEN NOTIFICATION WHEN IT IS PROVIDED TO THE STEEL ERECTOR.

C. NO MODIFICATION THAT AFFECTS THE STRENGTH OF A STEEL JOIST OR STEEL JOIST GIRDER SHALL BE MADE WITHOUT THE APPROVAL OF THE PROJECT STRUCTURAL ENGINEER OF RECORD.

. METAL DECKING HOLES AND OPENINGS SHALL NOT BE CUT UNTIL IMMEDIATELY PRIOR TO BEING PERMANENTLY FILLED WITH THE EQUIPMENT OR STRUCTURE, OR SHALL BE IMMEDIATELY COVERED.

PROTECTION: PROPER PRECAUTIONS SHALL BE TAKEN AT ALL TIMES TO PROTECT VEHICULAR AND PEDESTRIAN TRAFFIC FROM ANY DAMAGE OR INJURY WHICH MAY BE CAUSED, EITHER DIRECTLY OR INDIRECTLY, BY THE WORK INCLUDED ON THESE DRAWINGS. SUCH PRECAUTIONS SHALL INCLUDE THE ERECTION AND MAINTENANCE OF FENCES, BARRICADES, RAILINGS, GUARDS, SIGNS, COVERINGS, LIGHTS, AND OTHER PRECAUTIONS AS MAY BE REQUIRED. IF AT ANY TIME, IN THE OPINION OF THE OWNER OR THE OWNER'S REPRESENTATIVE, PROPER PRECAUTIONS ARE NOT BEING TAKEN TO SECURE THIS PROTECTION, THE CONTRACTOR SHALL AT NO ADDITIONAL COST TO THE OWNER, INSTALL AND MAINTAIN SUCH ADDITIONAL PROTECTION AS MAY BE DIRECTED

POLLUTION CONTROLS: USE WATER SPRINKLING, TEMPORARY ENCLOSURES, AND OTHER SUITABLE METHODS TO LIMIT DUST AND DIRT RISING AND SCATTERING IN THE AIR TO LOWEST PRACTICAL LEVEL. COMPLY WITH GOVERNING REGULATIONS PERTAINING TO ENVIRONMENTAL PROTECTION.

TYPICAL DETAIL SHEETS

THE S7.00 SERIES SHEETS IN THESE DRAWINGS CONTAIN TYPICAL STRUCTURAL DETAILS FOR VARIOUS BUILDING MATERIALS. SOME OF THESE DETAILS MAY NOT BE PART OF THIS PROJECT

DRAWINGS:

DO NOT SCALE DRAWINGS

WHERE DISCREPANCIES OCCUR BETWEEN PLANS. DETAILS. GENERAL NOTES. AND SPECIFICATIONS. THE MORE STRINGENT REQUIREMENTS SHALL GOVERN. DETAILS ON DRAWINGS TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS. DETAILS NOTED "TYPICAL" APPLY TO ALL SIMILAR CONDITIONS. WHERE NO SPECIFIC DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ELSEWHERE ON THE PROJECT.

FAST-TRACK/PHASED CONSTRUCTION

THE STRUCTURAL PORTION OF THIS PROJECT IS BEING DESIGNED, BID, PERMITTED, AND CONSTRUCTED PRIOR TO THE COMPLETION OF ARCHITECTURAL, ENGINEERING, AND OTHER DESIGN TEAM CONSTRUCTION DOCUMENTS. THE OWNER, ARCHITECT, AND CONTRACTOR SHALL BE AWARE THAT THIS ACCELERATED STRUCTURAL SCHEDULE CREATES INHERENT RISK OF FUTURE CHANGES DUE TO DESIGN COORDINATION WITH OTHER DISCIPLINES. WHILE EVERY EFFORT HAS BEEN MADE TO MINIMIZE THESE CHANGES, THE RISK OF ADDED COSTS DUE TO THESE CHANGES SHALL BE UNDERSTOOD AND ACCEPTED BY ALL PARTIES.

DRAWINGS THAT DO NOT HAVE AN ENGINEERING SEAL BY THE STRUCTURAL ENGINEER OF RECORD OR NOT LABELED AS CONSTRUCTION DRAWINGS ARE PRELIMINARY AND SUBJECT TO CHANGE. IF THESE DOCUMENTS ARE BEING USED FOR PRICING, BIDDING, STEEL MILL ORDER, OR PREPARATION OF SHOP DRAWINGS, THE CONTRACTOR SHALL ANTICIPATE FUTURE DRAWING REVISIONS THAT MAY AFFECT THIS WORK OR INCREASE CONSTRUCTION COSTS. THE STRUCTURAL ENGINEER SHALL NOT BE RESPONSIBLE FOR ANY CHANGE ORDER COSTS INCURRED DUE TO THESE DRAWING REVISIONS. AND THE CONTRACTOR SHALL CONSIDER THESE ANTICIPATED COSTS IN ANY BIDS OR PRICE GUARANTEES TO THE OWNER.

USE THE MOST CURRENT SET OF DRAWINGS IN PREPARATION OF ALL SUBMITTALS. ALL SUBMITTALS SHALL LIST THE DATE OF THE DRAWINGS USED TO PREPARE THE SUBMITTAL. SUBMITTALS PREPARED FROM OUTDATED DRAWINGS MAY BE REJECTED. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ACQUIRING THE LATEST SET OF CONSTRUCTION DRAWINGS AND DISTRIBUTING TO THE APPROPRIATE PARTIES.

CAST-IN-PLACE CONCRETE

ALL CONCRETE SHALL CONFORM TO THE SPECIFICATIONS FOR STRUCTURAL CONCRETE, ACI 301-10.

ALL EXPOSED EDGES OF CONCRETE SHALL HAVE A 3/4" CHAMFER UNLESS NOTED OTHERWISE.

NORMALWEIGHT CONCRETE:

A. $\,$ F'C = 4500 PSI @ 28 DAYS – ALL CONCRETE EXPOSED TO FREEZE/THAW CYCLES AND OCCASIONAL MOISTURE. INCLUDING CONCRETE FLAT WORK, EXPOSED BUILDING STEM WALLS, SITE WALLS, ETC. EXTERIOR CONCRETE SHALL MEET EXPOSURE CATEGORY AND CLASS F1 ACCORDING TO ACI 318 TABLE

B. F'C = 3000 PSI @ 28 DAYS - ALL FOOTINGS, TIE BEAMS, GRADE BEAMS.

C. F'C = 3000 PSI @ 28 DAYS - ALL INTERIOR SLABS ON GRADE, UNLESS NOTED OTHERWISE D. F'C = 3500 PSI @ 28 DAYS - ALL CONCRETE FILL OVER METAL DECK, UNLESS NOTED OTHERWISE. E. F'C = 4000 PSI @ 28 DAYS - ALL CAST-IN-PLACE CONCRETE COLUMNS, PEDESTALS, RETAINING WALLS, AND

ELEVATED BEAMS. F'C = 4000 PSI @ 28 DAYS - ALL ELEVATED CAST-IN-PLACE SLABS. G. F'C = 4000 PSI @ 28 DAYS – ALL SLABS ON GRADE AND ELEVATED SLABS TO RECEIVE POLISHED CONCRETE

FIRE RATED SLABS: COORDINATE AIR CONTENT REQUIREMENTS WITH ARCHITECTURAL DRAWINGS.

CONCRETE MIX DESIGNS (INCLUDING AIR CONTENT, WATER TO CEMENT RATIOS, AND OTHER CRITERIA) SHALL CONFORM TO THE REQUIREMENTS SET FORTH IN ACI 318 TABLE 19.3.2.1, BASED ON THE EXPOSURE CATEGORIES AND CLASSES DEFINED IN ACI 318 TABLE 19.3.1.1. USE AIR ENTRAINING ADMIXTURE IN ALL EXTERIOR CONCRETE AIR CONTENT IN FIRE RATED SLABS SHALL ALSO COMPLY WITH THE REQUIREMENTS IN THE SPECIFIED UL LISTING

CRYSTALLINE WATERPROOFING SHALL BE BASF, W.R. MEADOWS ADMIXTURE OR APPROVED EQUAL. CRYSTALLINE WATERPROOFING SHALL BE ADDED TO THE CONCRETE MIX PER THE MANUFACTURER'S RECOMMENDATIONS FOR ALL BASEMENT WALL CONCRETE

CAUSED BY FROST, FREEZING OR LOW TEMPERATURES. COMPLY WITH ACI 306.1. HOT WEATHER CONCRETING: WHEN HOT WEATHER CONDITIONS EXIST THAT WOULD IMPAIR THE QUALITY AND

STRENGTH OF THE CONCRETE, REDUCE DELIVERY TIME OF READY MIX CONCRETE, LOWER THE TEMPERATURE

OF MATERIALS, OR ADD RETARDER TO ENSURE THAT THE CONCRETE IS PLASTIC. RETEMPERING WITH WATER IS

NOT ALLOWED. COMPLY WITH ACI 305R. SLAB CURING: ALL INTERIOR CONCRETE SLABS, EXCEPT EXPOSED INTEGRALLY COLORED SLABS, ARE TO BE CURED WITH A MOISTURE RETAINING COVER FOR THE FIRST 7 DAYS (MINIMUM) AFTER PLACEMENT.

THE CONTRACTOR IS ALLOWED TO CAST FOUNDATIONS AGAINST EXCAVATED SOIL SURFACES, PROVIDED THE FOLLOWING IS ADHERED TO:

A. THE SIDE SLOPES OF THE EXCAVATION SHALL BE ABLE TO MAINTAIN VERTICAL SLOPE WITHOUT SOIL

SLOUGHAGE.

THE BOTTOM WIDTH OF THE EXCAVATION SHALL BE ONE INCH WIDER MINIMUM ON EACH SIDE THAN THE SPECIFIED FOOTING WIDTH THE SIDE WALLS OF THE EXCAVATION SHALL BE BATTERED A MINIMUM OF ONE INCH HORIZONTAL TO TWELVE INCHES VERTICAL

 IF SANDY OR LOOSE MATERIALS ARE ENCOUNTERED, THE FOOTING MUST BE FORMED. CONCRETE DURING THE CASTING OPERATION.

EXPOSED SITE WALLS. RETAINING WALLS. AND STEM WALLS GREATER THAN 30 FEET IN LENGTH SHALL HAVE

THE CONTRACTOR AGREES TO REMOVE AND RECAST ANY FOOTING WHERE THE ABOVE CONDITIONS ARE NOT

CONTROL JOINTS INSTALLED AT THE FOLLOWING MAXIMUM SPACING: 12'-0" ON CENTER FOR WALLS 6'-0" MAXIMUM HEIGHT

ALL CONCRETE EXPOSED TO GROUND SHALL BE MANUFACTURED WITH PORTLAND CEMENT TYPE I OR TYPE II. SEE SHEET S7.11 FOR TYPICAL CONCRETE DETAILS.

REINFORCING STEEL:

18'-0" ON CENTER FOR WALLS 10'-0" MAXIMUM HEIGHT

20'-0" ON CENTER FOR WALLS GREATER THAN 10'-0" IN HEIGHT

ALL REINFORCING STEEL SHALL BE FABRICATED AND PLACED IN ACCORDANCE WITH THE BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI 318-14), AND DETAILS AND DETAILING OF CONCRETE REINFORCEMENT (ACI 315-99).

ALL REINFORCING STEEL SHALL CONFORM TO ASTM A615 GRADE 60; EXCEPT STIRRUPS, TIES AND INDICATED FIELD-BENT BARS, WHICH SHALL CONFORM TO ASTM A615 GRADE 40.

GENERAL STRUCTURAL NOTES

ALL WELDED WIRE FABRIC SHALL BE DEFORMED AND SHALL CONFORM TO ASTM A479. PROVIDE IN FLAT SHEETS

TENSION AND COMPRESSION LAPS IN REINFORCING SHALL CONFORM TO THE LAP SPLICE SCHEDULE ON SHEET

S6.01 AND BE IN ACCORDANCE WITH ACI 318, CHAPTER 12, UNLESS NOTED OTHERWISE.

ALL HORIZONTAL REINFORCING IN FOOTINGS, WALLS AND BEAMS SHALL BE CONTINUOUS AROUND CORNERS OR HAVE BENT (CORNER) BARS OF THE SAME SIZE AND SPACING AS THE HORIZONTAL BARS AND LAP 30 BAR DIAMETERS (24" MINIMUM).

CONCRETE COVER FOR REINFORCING SHALL BE AS FOLLOWS UNLESS OTHERWISE NOTED:

A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH:

E. STRUCTURAL SLABS ON METAL DECK:

REINFORCING SHALL BE SECURELY TIED TO SUPPORTS.

B. CONCRETE CAST AGAINST FORMS BUT EXPOSED TO EARTH OR WEATHER: BARS LARGER THAN NO. 5: 2. BARS NO. 5 OR SMALLER:

C. CONCRETE NOT EXPOSED TO WEATHER OR NOT IN CONTACT WITH GROUND: COLUMNS, GIRDERS AND BEAMS: 2. STRUCTURAL SLABS, WALLS AND JOISTS (NO. 11 AND SMALLER): 1 1/2" FROM TOP OF SLAB D. SLAB ON GRADE

FORM TIES SHALL BE EITHER OF THE THREADED OR SNAP-OFF TYPE SO THAT NO METAL WILL BE LEFT WITHIN 1 INCH OF THE SURFACE OF THE WALL. FOLLOWING REMOVAL OF FORM TIES. RECESSES ARE TO BE CAREFULLY

FILLED AND POINTED WITH MORTAR. REINFORCING SHALL NOT BE TACK WELDED OR WELDED IN ANY MANNER UNLESS SPECIFICALLY DETAILED ON THE

1" FROM TOP OF SLAB

STRUCTURAL PLANS. BAR SUPPORTS AND SPACERS FOR REINFORCING SHALL BE PROVIDED IN ACCORDANCE WITH ACI 315-99.

CHAIRS WITH 22 GAGE SAND PLATES OR PRECAST BLOCKS SHALL BE PROVIDED FOR ALL REINFORCING OF CONCRETE IN CONTACT WITH GRADE

DECK CHAIRS SHALL BE PROVIDED FOR ALL WELDED WIRE FABRIC IN SLABS OVER METAL DECK.

POST INSTALLED ANCHORS:

THE STRUCTURAL DESIGN IS BASED ON THE POST INSTALLED ANCHORING SYSTEMS NOTED BELOW. SINCE ANCHOR CAPACITIES VARY BY MANUFACTURER, THE CONTRACTOR SHALL USE ONLY THE SYSTEMS NOTED BELOW UNLESS AN ALTERNATE IS APPROVED BY THE ENGINEER OF RECORD. ALTERNATE ANCHORING SYSTEMS MAY REQUIRE RE-DESIGN TO VERIFY ANCHOR QUANTITIES, SPACING, AND EMBED DEPTHS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY ADDITIONAL CONSTRUCTION AND RE-DESIGN COSTS ASSOCIATED WITH THE ALTERNATE ANCHORING SYSTEM.

ALL ADHESIVE (EPOXY) FOR POST INSTALLED ANCHORS AND/OR REBAR INTO GROUT FILLED MASONRY SHALL BE HILTI HIT HY 70 ADHESIVE ANCHORING SYSTEM. INSTALLATION SHALL BE PER THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.

ALL ADHESIVE (EPOXY) FOR POST INSTALLED ANCHORS AND/OR REBAR INTO HOLLOW MASONRY AND/OR BRICK SHALL BE HILTI HIT HY 270 ADHESIVE ANCHORING SYSTEM. INSTALLATION SHALL BE PER THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.

ALL POST INSTALLED MECHANICAL ANCHORS INTO CONCRETE SHALL BE HILTI KWIK HUS EZ (KH-EZ) SCREW ANCHOR. INSTALLATION SHALL BE PER THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.

ANCHOR LENGTHS SHOWN FOR ATTACHMENT TO CONCRETE AND/OR MASONRY ARE REQUIRED EMBEDMENT LENGTHS. THE CONTRACTOR SHALL PROVIDE ANCHORS WITH ADDITIONAL LENGTH TO FACILITATE THE REQUIRED. CONNECTION.

SUBMIT ALL PROPOSED ANCHORING SYSTEMS INCLUDING ICC-ES REPORTS TO STRUCTURAL ENGINEER FOR REVIEW PRIOR TO INSTALLATION. THE ICC-ES FORMS SHALL MEET THE REQUIREMENTS OF THE IBC REFERENCED

ALL POST-INSTALLED ANCHORS SHALL BE INSTALLED WITH SPECIAL INSPECTION AS DICTATED BY THE RESPECTIVE PRODUCT'S ICC-ES EVALUATION SERVICE REPORT

THE CONTRACTOR SHALL ARRANGE AN ANCHOR MANUFACTURER'S REPRESENTATIVE TO PROVIDE ONSITE INSTALLATION TRAINING, UNLESS ALL PERSONNEL INSTALLING ANCHORS ARE CERTIFIED IN ACCORDANCE WITH ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM, OR EQUIVALENT APPROVED BY THE ENGINEER OF RECORD.

INSTALLATION OF ADHESIVE ANCHORS IN HORIZONTAL TO VERTICALLY OVERHEAD ORIENTATION SHALL BE CONTINUOUSLY INSPECTED DURING INSTALLATION BY AN INSPECTOR SPECIALLY APPROVED FOR THAT PURPOSE BY THE BUILDING OFFICIAL. INSTALLATION SHALL BE DONE BY A CERTIFIED ADHESIVE ANCHOR INSTALLER (AAI) AS CERTFIEID THROUGH ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM, OR EQUIVALENT. PROOF OF CURRENT CERTIFICATION SHALL BE SUBMITTED TO THE ENGINEER PRIOR TO COMMENCEMENT OF INSTALLATION, AND INSPECTION REPORTS SHALL BE PROVIDED TO THE ENGINEER OF RECORD AND THE BUILDING

STRUCTURAL AND MISCELLANEOUS STEEL:

ALL STRUCTURAL STEEL SHALL BE DETAILED AND FABRICATED IN ACCORDANCE WITH THE AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS".

ALL WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A992, GRADE 50, UNLESS NOTED OTHERWISE.

TO ASTM A36 UNLESS NOTED OTHERWISE. ALL RECTANGULAR AND SQUARE STRUCTURAL TUBING SHALL CONFORM TO ASTM A500, GRADE B, FY = 46 KSI OR

ALL MISCELLANEOUS STEEL MEMBERS, SUCH AS CHANNELS, ANGLES, FLAT BARS, AND PLATES SHALL CONFORM

ASTM 1085, GRADE B, FY = 50 KSI. ALL ROUND STRUCTURAL TUBING SHALL CONFORM TO ASTM A500, GRADE B, FY = 42 KSI OR ASTM 1085, GRADE B,

ALL STRUCTURAL PIPE SHALL CONFORM TO ASTM A53, TYPE E OR S, GRADE B, FY = 35 KSI.

BOLTS SHALL CONFORM TO ASTM A325N TENSION CONTROL BOLTS UNLESS NOTED OTHERWISE, WITH SIZES AS SHOWN ON THE DRAWINGS. WHERE CLEARANCE WITHIN A CONNECTION DOES NOT PERMIT THE USE OF TENSION CONTROL BOLTS, STANDARD A325N BOLTS SHALL BE USED AND INSPECTED IN ACCORDANCE WITH THE AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS".

ALL BOLTS SHALL BE INSTALLED IN A SNUG TIGHT CONDITION EXCEPT AT MOMENT CONNECTIONS, BRACED FRAME CONNECTIONS, AND AT CONNECTIONS DETAILED WITH A325SC BOLTS. AT THESE LOCATIONS, THE BOLTS SHALL BE TIGHTENED SO AS TO SHEAR THE SPLINE OFF THE BOLT.

ANCHOR BOLTS EMBEDDED IN CONCRETE SHALL BE ASTM F1554 GRADE 36 THREADED RODS WITH DOUBLE NUTS. PROVIDE FLAT WASHERS BETWEEN NUTS AND BASEPLATE SURFACES. ANCHOR BOLT LENGTHS SHOWN FOR ATTACHMENT TO CONCRETE AND/OR MASONRY ARE REQUIRED EMBEDMENT LENGTHS. THE CONTRACTOR SHALL PROVIDE ANCHOR BOLTS WITH ADDITIONAL BOLT LENGTH TO FACILITATE THE REQUIRED CONNECTION.

ANCHOR BOLT FLAT WASHERS SHALL BE PROVIDED IN ACCORDANCE WITH TABLE 14-2 OF AISC 360. AISC MANUAL OF STEEL CONSTRUCTION LATEST EDITION.

ALL WELDING SHALL BE DONE IN ACCORDANCE WITH THE LATEST STANDARDS OF THE AWS STRUCTURAL WELDING CODE.

ALL BOLT HOLES THAT ARE REQUIRED TO BE FIELD DRILLED SHALL BE DRILLED WITH A MAG DRILL. FLAME CUTTING OF HOLES OR ENLARGING OF MISALIGNED HOLES WILL NOT BE ALLOWED. HEADED CONCRETE ANCHORS AND SHEAR CONNECTORS SHALL BE MADE FROM STEEL CONFORMING TO ASTM

A108 AND MEET THE MECHANICAL PROPERTIES OF TYPE B, AS REQUIRED BY CHAPTER 7 OF AWS D1.1 "STRUCTURAL WELDING CODE-STEEL", LATEST EDITION. STRUCTURAL STEEL TO RECEIVE SHEAR CONNECTORS SHALL BE FREE OF PAINT. WELDING PREQUALIFICATION REQUIRED.

COMPOSITE FLOORS:

BE ALLOWED TO DEFLECT TO LEVEL.

FY = 50 KSI.

THE METAL DECK FOR COMPOSITE FLOORS SHALL BE UNSHORED UNLESS NOTED OTHERWISE.

THE SHEAR CONNECTORS SHALL BE 3/4" DIAMETER X 3" AT 1 1/2" DEEP DECK AND 3/4" DIAMETER X 4 1/2" AT 3" DEEP DECK UNLESS NOTED OTHERWISE. THE SHEAR CONNECTORS SHALL BE MADE FROM STEEL CONFORMING TO ASTM A108 AND MEET THE MECHANICAL PROPERTIES OF TYPE B, AS REQUIRED BY CHAPTER 7 OF AWS D1.1 "STRUCTURAL WELDING CODE STEEL", LATEST EDITION. STRUCTURAL STEEL TO RECEIVE SHEAR CONNECTIONS SHALL BE FREE OF PAINT. WELDING PREQUALIFICATION REQUIRED.

THE SHEAR CONNECTIONS SHALL NOT BE ADDED UNTIL THE METAL FLOOR DECK IS INSTALLED.

WHERE SHEAR CONNECTIONS AND PUDDLE WELDS COINCIDE, THE SHEAR CONNECTOR MAY REPLACE THE PUDDLE WELD.

CAMBERED BEAMS SHALL HAVE THE CAMBER PUT IN AT 1/3 POINTS OR ALONG A PARABOLIC CURVE. THE CONTRACTOR SHALL SURVEY THE CAMBER OF THE BEAMS AFTER THE BEAMS HAVE BEEN ERECTED. THE

CONTRACTOR SHALL SUBMIT THE SURVEY TO THE ENGINEER FOR REVIEW. THE CONTRACTOR SHALL NOT POUR THE SLAB UNTIL THE ENGINEER HAS REVIEWED AND APPROVED THE BEAM CAMBERS. CONTRACTOR SHALL SHORE BEAMS WITH A CAMBER MORE THAN 1/2" LOWER THAN SPECIFIED. THE BEAM SHALL

GENERAL STRUCTURAL NOTES

THE TOPPING SLAB SHALL BE POURED AND PLACED TO THE ELEVATION INDICATED ON THE DRAWINGS.

THE CONCRETE FOR THE SLAB SHALL BE POURED AND PLACED TO THE ELEVATION INDICATED ON THE DRAWINGS WHILE MAINTAINING THE MINIMUM THICKNESS. SPREAD CONCRETE OVER AREA OF INFLUENCE TO ROUGH DEPTH

IN ORDER TO LOAD BEAMS AND GIRDERS PRIOR TO SETTING SCREED ELEVATIONS.

THE WEIGHT OF THE WET CONCRETE WILL CAUSE DEFLECTIONS OF THE STEEL FRAMING. THEREFORE, CONCRETE OVERRRUNS ARE TO BE ANTICIPATED BY THE CONTRACTOR.

CONTRACTOR SHALL CONTINUOUSLY MONITOR THE THICKNESS AND ELEVATIONS DURING CONCRETE PLACING

PROVIDE #4 X 6'-0" AT 12" ON CENTER OVER ALL GIRDERS OF COMPOSITE FLOORS.

PROVIDE #4 X 6'-0" AT 12" ON CENTER OVER SHORED BEAMS THAT ARE NOT ALLOWED TO DEFLECT TO LEVEL

PROVIDE WELDED WIRE FABRIC AS INDICATED ON DRAWINGS IN FLAT SHEETS ONLY.

PROVIDE DECK CHAIRS FOR ALL WELDED WIRE FABRIC IN SLABS OVER METAL DECK.

STEEL DECK:

ALL STEEL DECK SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE STEEL DECK INSTITUTE SPECIFICATIONS.

SEE PLANS FOR STEEL DECK TYPE. GAGE. FINISH AND CONNECTIONS.

PROVIDE A MINIMUM OF 1 1/2" BEARING FOR ALL STEEL DECK

ALL SPLICES AND LAPS SHALL BE A MINIMUM OF 2" IN LENGTH AND SHALL BE LOCATED DIRECTLY ABOVE SUPPORTS.

ALL DECKING SHALL BE CONTINUOUS OVER TWO OR MORE SPANS.

COLD-FORMED METAL FRAMING (43 MILS OR HEAVIER):

ALL COLD-FORMED METAL FRAMING SHALL CONFORM TO THE LATEST EDITION OF AISI STANDARD S100 "NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS".

WALLS SHALL BE PROVIDED WITH MANUFACTURER'S STANDARD BRIDGING: (EITHER WELDED 2 1/2" x 43 MILS STUD OR CLIPPED COLD-ROLLED CHANNEL 1 1/2" x 54 MILS). PROVIDE BRIDGING AT 4'-0" ON CENTER MAXIMUM FOR LOAD BEARING WALLS AND EXTERIOR WALLS.

PROVIDE ALL MISCELLANEOUS ACCESSORIES AND FOLLOW ERECTION PROCEDURES AS PER MANUFACTURER'S SPECIFICATIONS AND RECOMMENDATIONS UNLESS NOTED OTHERWISE.

ALL TRACK SHALL BE ANCHORED TO CONCRETE NOT MORE THAN 4' - 0" ON CENTER. ATTACHMENT BY OTHERS.

COLD-FORMED METAL FRAMING SHALL MEET THE MINIMUM PROPERTIES AS SHOWN IN THE STEEL STUD MANUFACTURERS ASSOCIATION (SSMA) SPECIFICATIONS.

SECURE STUDS TO TOP AND BOTTOM TRACKS, ATTACHMENT BY OTHERS. WELD CLIPS DESIGNED BY OTHERS. FASTEN WELD CLIPS TO STUDS IN ACCORDANCE WITH MANUFACTURER'S

S0.03 AND VERTICAL LOADS AS DICTATED BY ARCHITECTURAL FINISHES AND SUPPORT REQUIREMENTS.

SLIDE CLIPS DESIGNED BY OTHERS. FASTEN SLIDE CLIPS TO STUDS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS AND LOAD DATA. SLIDE CLIPS SHALL BE DESIGNED FOR LATERAL LOADS AS SPECIFIED ON

RECOMMENDATIONS AND LOAD DATA. WELD CLIPS SHALL BE DESIGNED FOR LATERAL LOADS AS SPECIFIED ON

SEE SHEET S7.31 FOR TYPICAL COLD-FORMED DETAILS

FOR CMU OR BRICK VENEER (4" MAXIMUM, 2" MINIMUM THICKNESS) ATTACHMENT TO STRUCTURAL MASONRY, PROVIDE ADJUSTABLE INTEGRAL ANCHOR TIES. ADJUSTABLE INTEGRAL ANCHOR TIES SHALL BE CORROSION RESISTANT AND HAVE TWO PINTLE LEGS MINIMUM WITH W2.8 (3/16") WIRE OR APPROVED EQUAL. PROVIDE DUR-O-WALL DA370 ADJUSTABLE INTEGRAL ANCHOR TIES OR APPOVED EQUAL.

PROVIDE ADJUSTABLE ANCHOR TIES. ADJUSTABLE ANCHOR TIES SHALL BE CORROSION RESISTANT AND HAVE A TWO PINTLE LEGS MINIMUM WITH A MINIMUM W2.8 (3/16") WIRE. ATTACH TO CONCRETE WITH 2-1/4" DIAMETER CONCRETE SCREWS, HILTI KWIKCON 11 x 1 1/2" OR APPROVED EQUAL FOR CMU OR BRICK VENEER (4" MAXIMUM, 2" MINIMUM THICKNESS) ATTACHMENT TO STRUCTURAL COLD FORMED

METAL STUDS, PROVIDE ADJUSTABLE ANCHOR TIES. ADJUSTABLE ANCHOR TIES SHALL BE CORROSION RESISTANT

AND HAVE TWO PINTLE LEGS MINIMUM W2.8 (3/16") WIRE. PROVIDE DUR-O-WALL DA213 ADJUSTABLE ANCHOR TIE

OR APPROVED EQUAL. ATTACH THROUGH SHEATHING TO STUDS WITH 2-1/4" x 1 1/2" CORROSION RESISTANT TEK

FOR CMU OR BRICK VENEER (4" MAXIMUM, 2" MINIMUM THICKNESS) ATTACHMENT TO STRUCTURAL CONCRETE

SEE TYPICAL DETAILS ON SHEET S7.11, S7.21, AND S7.31 FOR VENEER TIE SPACING. PROVIDE ADDITIONAL ANCHORS AROUND ALL OPENINGS LARGER THAN 16" IN EITHER DIMENSION. SPACE

ANCHORS WITHIN 12" OF OPENING PERIMETER AND MATCH HORIZONTAL OR VERTICAL ANCHOR TIE SPACING.

COORDINATE VENEER LOCATION, TYPE, BOND PATTERN, ETC. WITH ARCHITECTURAL DRAWINGS.

GLASS CURTAIN WALL SYSTEM: ALL LATERAL AND GRAVITY SUPPORT FOR THE GLASS CURTAIN WALL SYSTEM SHALL BE PER THE MANUFACTURER'S RECOMMENDATIONS. SHOP DRAWINGS AND STAMPED CALCULATIONS SHALL BE SUBMITTED FOR APPROVAL BY THE ENGINEER OF RECORD AND THE ARCHITECT PRIOR TO INSTALLATION.

THE ENGINEER STAMPING THE SHOP DRAWINGS SHALL BE REGISTERED IN THE STATE THAT THE PROJECT IS

THE GLASS CURTAIN WALL SYSTEM SHALL BE LATERALLY SUPPORTED AT ALL FLOORS AND ROOF LEVEL.

SEISMIC BRACING OF NON-STRUCTURAL COMPONENTS: SEISMIC BRACING AND RESTRAINTS FOR MECHANICAL/ELECTRICAL EQUIPMENT AND SYSTEMS SHALL BE

MINIMUM DESIGN LOADS FOR BUILDINGS AND STRUCTURES. SEE THESE GENERAL STRUCTURAL NOTES FOR THE SITE-SPECIFIC SEISMIC DESIGN CRITERIA

THE CONTRACTOR IS RESPONSIBLE FOR DESIGNING AND INSTALLING THE SEISMIC BRACING AND RESTRAINTS.

STAMPED SHOP DRAWINGS, INCLUDING CALCULATIONS, SHALL BE SUBMITTED FOR APPROVAL PRIOR TO ANY

PROVIDED BY THE CONTRACTOR PER THE 2015 INTERNATIONAL BUILDING CODE (2015 IBC) AND THE ASCE 7-10,

BRACING INSTALLATION.

THE ENGINEER STAMPING THE SHOP DRAWINGS SHALL BE REGISTERED IN THE STATE THAT THE PROJECT IS LOCATED.

ELEVATORS:

LOCATED.

THE STRUCTURE HAS BEEN DESIGNED FOR A KONE MONOSPACE 500: 2500/4000 (SUBJECT TO CHANGE) ELEVATOR.

ALL STRUCTURAL SUPPORTS. FLOOR PENETRATION SIZES AND PIT DIMENSIONS HAVE BEEN DESIGNED BASED ON THE ABOVE INFORMATION. SHOULD THE ACTUAL ELEVATOR(S) SELECTED DIFFER FROM THE INFORMATION PROVIDED ABOVE, THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY ADDITIONAL CONSTRUCTION AND

THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING ALL ELEVATOR PIT AND FLOOR PENETRATION LOCATIONS AND DIMENSIONS

DEFERRED SUBMITTALS:

THE DEFERRED SUBMITTALS LISTED BELOW ARE THOSE PORTIONS OF THE DESIGN THAT ARE NOT COMPLETED A $^\circ$ THE TIME OF APPLICATION AND ARE TO BE SUBMITTED TO THE BUILDING OFFICIAL AND APPROVED PRIOR TO TH INSTALLATION OF THOSE ITEMS. THE MANUFACTURER, CONSULTANT, OR CONTRACTOR, AS APPROPRIATE, SHALL

PRECAST PRESTRESSED CONCRETE

COLD-FORMED METAL (LIGHTGAGE) FRAMING METAL STAIRS, RAMPS, LADDERS, AND GUARDRAILS

REDESIGN COSTS ASSOCIATED WITH THE ALTERNATE ELEVATOR(S).

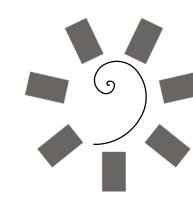
DEWATERING/DRAINAGE SYSTEM BEHIND BASEMENT WALLS, INCLUDING ITS TIE IN TO THE STORM WATER

SEISMIC BRACING OF ALL ARCHITECTURAL, LIGHTING, THEMING, FIRE SUPPRESSION, AND MEP COMPONENTS CURTAINWALL SYSTEMS TEMPORARY AND PERMANENT SHORING OF EXISTING STRUCTURES AND SOILS WHERE REQUIRED FOR NEW

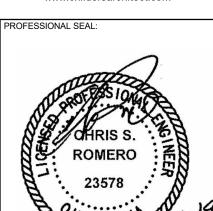
UNDERSLAB FRENCH DRAIN SYSTEM LAYOUT AND TIE-IN TO STORM WATER DRAINAGE SYSTEM.

PROVIDE SUBMITTALS TO THE ENGINEER OF RECORD FOR REVIEW FOR THE FOLLOWING ITEMS.

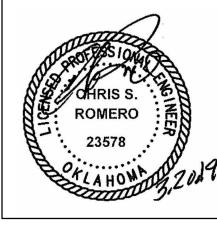
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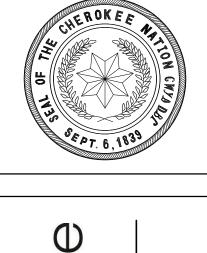
lames R. Childers Architect, Inc. 45 South 4th Street Fort Smith, AR 72901 479-783-2480

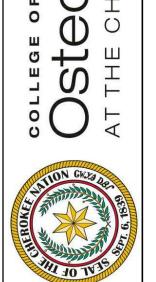


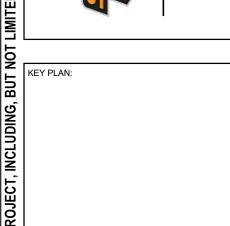
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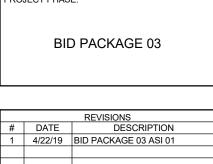












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GENERAL STRUCTURAL

NOTES

SCHEDULE OF STRUCTURAL SPECIAL INSPECTIONS

- 1. SPECIAL INSPECTIONS / TESTING "SPECIAL STRUCTURAL INSPECTION" SHALL NOT RELIEVE THE OWNER OR THEIR AGENT FROM HAVING THE INSPECTIONS OF THE JURISDICTION BUILDING DEPARTMENT PER SECTION 110 OF THE IBC PERFORMED. BOTH THE JURISDICTION BUILDING DEPARTMENT INSPECTIONS AND "SPECIAL STRUCTURAL INSPECTION" SHALL BE PERFORMED.
- 2. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR NOTIFYING THE JURISDICTION BUILDING OFFICIAL AND SPECIAL INSPECTOR WHEN WORK IS READY FOR INSPECTION.
- 3. REPORTING FOR SPECIAL INSPECTION SPECIAL INSPECTION AND TESTING REPORTS SHALL BE COMPLETED AND DISTRIBUTED AT THE COMPLETION OF EACH TASK. IF A TASK IS TO TAKE LONGER THAN THREE (3) DAYS, PROVIDE REPORTS FOR EACH DAY. PROVIDE COPIES OF REPORTS TO CONTRACTOR, OWNER, ARCHITECT AND STRUCTURAL ENGINEER OF RECORD. SPECIAL INSPECTOR TO KEEP A NON-COMPLIANCE LIST DOCUMENTING ITEMS INSPECTED NOT MEETING APPROVED CONSTRUCTION DOCUMENTS AND WHEN / HOW RESOLVED.
- 4. SEE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING CONSTRUCTION DOCUMENTS FOR ADDITIONAL NON-STRUCTURAL SPECIAL INSPECTION ITEMS.
- 5. SPECIAL INSPECTION OF SHOP FABRICATED MEMBERS AND ASSEMBLIES SHALL BE IN ACCORDANCE WITH SECTION 1704.2, UNLESS FABRICATOR IS APPROVED TO PERFORM WORK WITHOUT SPECIAL INSPECTION.
- 6. IN ACCORDANCE WITH IBC CHAPTER 17, THE OWNER OR THE OWNER'S AGENT, OTHER THAN THE CONTRACTOR, SHALL EMPLOY ONE OR MORE APPROVED AGENCIES TO PROVIDE SPECIAL INSPECTIONS AND TESTS, DURING CONSTRUCTION FOR THE TYPES OF WORK LISTED BELOW THESE SPECIAL INSPECTIONS AND TESTS ARE IN ADDITION TO THE INSPECTIONS BY THE BUILDING OFFICIAL IDENTIFIED IN IBC SECTION 110

7. DEFINITIONS:

- * SPECIAL INSPECTION: INSPECTION AS HEREIN REQUIRED BY A QUALIFIED SPECIAL INSPECTOR COMPETENT WITH THE MATERIALS, INSTALLATION, FABRICATION, ERECTION OR PLACEMENT OF COMPONENTS AND CONNECTIONS REQUIRING SPECIAL EXPERTISE TO ENSURE COMPLIANCE WITH APPROVED CONSTRUCTION DOCUMENTS AND REFERENCED STANDARDS (SEE SECTION 1704).
- * CONTINUOUS SPECIAL INSPECTION: FULL-TIME OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED. * PERIODIC SPECIAL INSPECTION: THE PART-TIME OR INTERMITTENT OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK HAS BEEN OR IS BEING PERFORMED AND AT THE COMPLETION OF THE WORK.

ITEM	DESCRIPTION OF REQUIREMENTS	REQUIRED (YES/NO)
SPECIAL INSPECTION OF STRUCTURAL STEEL	TO BE PERFORMED IN ACCORDANCE WITH CHAPTER N OF AISC 360-10	YES
SPECIAL INSPECTION AND VERIFICATION OF STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.2	YES
SPECIAL INSPECTIONS AND VERIFICATIONS FOR CONCRETE CONSTRUCTION	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.3	YES
SPECIAL INSPECTIONS AND VERIFICATIONS FOR WOOD CONSTRUCTION	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.5	NO
SPECIAL INSPECTIONS AND VERIFICATIONS OF SOILS	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.6, THE GEOTECHNICAL REPORT LISTED IN THE GENERAL FOUNDATION NOTES, AND ANY OTHER REQUIREMENTS LISTED IN THE GENERAL FOUNDATION NOTES	YES

GENERAL STRUCTURAL NOTES

TEMPORARY SHORING OF EXCAVATIONS:

THE TEMPORARY SHORING OF EXCAVATIONS SHALL BE SOIL NAIL/SHOTCRETE SYSTEM, SHEET PILING, OR APPROVED EQUAL.

THE SHORING SHALL NOT BE DRIVEN OR INSTALLED IN ANY MANNER THAT COULD POTENTIALLY DAMAGE EXISTING STRUCTURES OR CAUSE HUMAN DISCOMFORT.

THE CONTRACTOR SHALL LOCATE ALL EXISTING UNDERGROUND UTILITIES PRIOR TO INSTALLING SHORING. PROVISIONS SHALL BE MADE TO AVOID EXISTING UTILITIES.

THE SHORING AS SHOWN ON THE PLANS IS FOR GRAPHICAL REPRESENTATION ONLY. THE CONTRACTOR

SHALL VERIFY THE EXACT LOCATION AND CONFIGURATION OF THE SHORING.

ANY SHORING THAT REMAINS IN PLACE SHALL NOT HAMPER FUTURE CONSTRUCTION.

THE CONTRACTOR IS RESPONSIBLE FOR DESIGNING AND INSTALLING THE TEMPORARY SHORING. STAMPED SHOP DRAWINGS, INCLUDING CALCULATIONS, SHALL BE SUBMITTED FOR APPROVAL PRIOR TO ANY SHORING INSTALLATION.

THE ENGINEER STAMPING THE SHOP DRAWINGS SHALL BE REGISTERED IN THE STATE THAT THE PROJECT IS LOCATED.

DEMOLITION:

CONDITION.

NOTCHING OR CUTTING ANY STRUCTURAL MEMBER IN THE FIELD IS PROHIBITED, UNLESS DETAILED OTHERWISE ON THE STRUCTURAL PLANS.

CONTRACTOR SHALL BE RESPONSIBLE FOR ADHERING TO ALL APPLICABLE STANDARDS SET FORTH BY

PRIOR TO STARTING DEMOLITION WORK. THE CONTRACTOR SHALL MAKE AN INSPECTION OF ALL SURROUNDING IMPROVEMENTS TO REMAIN, TO DETERMINE AND RECORD THEIR EXISTING PHYSICAL

SHORING AND BRACING: THE CONTRACTOR SHALL FURNISH ALL SHORING, BRACING, AND INCIDENTALS NECESSARY AND REQUIRED FOR THE PROPER SUPPORT AND SAFETY OF ALL MEMBERS AFFECTED BY

DEMOLITION WORK. WHERE DEMOLITION WOULD AFFECT THE STRUCTURAL INTEGRITY OF THE REMAINING STRUCTURE, THE

CONTRACTOR SHALL PROVIDE TEMPORARY SUPPORTS. THE CONTRACTOR SHALL NOTIFY THE ENGINEER OF ANY FIELD CONDITION WHICH WOULD PRESENT A HAZARDOUS CONDITION TO THE STRUCTURE BEFORE PROCEEDING.

PROTECTION: PROPER PRECAUTIONS SHALL BE TAKEN AT ALL TIMES TO PROTECT VEHICULAR AND PEDESTRIAN TRAFFIC FROM ANY DAMAGE OR INJURY WHICH MAY BE CAUSED, EITHER DIRECTLY OR INDIRECTLY, BY THE WORK INCLUDED ON THESE DRAWINGS. SUCH PRECAUTIONS SHALL INCLUDE THE ERECTION AND MAINTENANCE OF FENCES, BARRICADES, RAILINGS, GUARDS, SIGNS, COVERINGS, LIGHTS, AND OTHER PRECAUTIONS AS MAY BE REQUIRED. IF AT ANY TIME, IN THE OPINION OF THE OWNER OR THE OWNER'S REPRESENTATIVE, PROPER PRECAUTIONS ARE NOT BEING TAKEN TO SECURE THIS PROTECTION, THE CONTRACTOR SHALL AT NO ADDITIONAL COST TO THE OWNER, INSTALL AND MAINTAIN SUCH ADDITIONAL PROTECTION AS MAY BE DIRECTED BY THE OWNER.

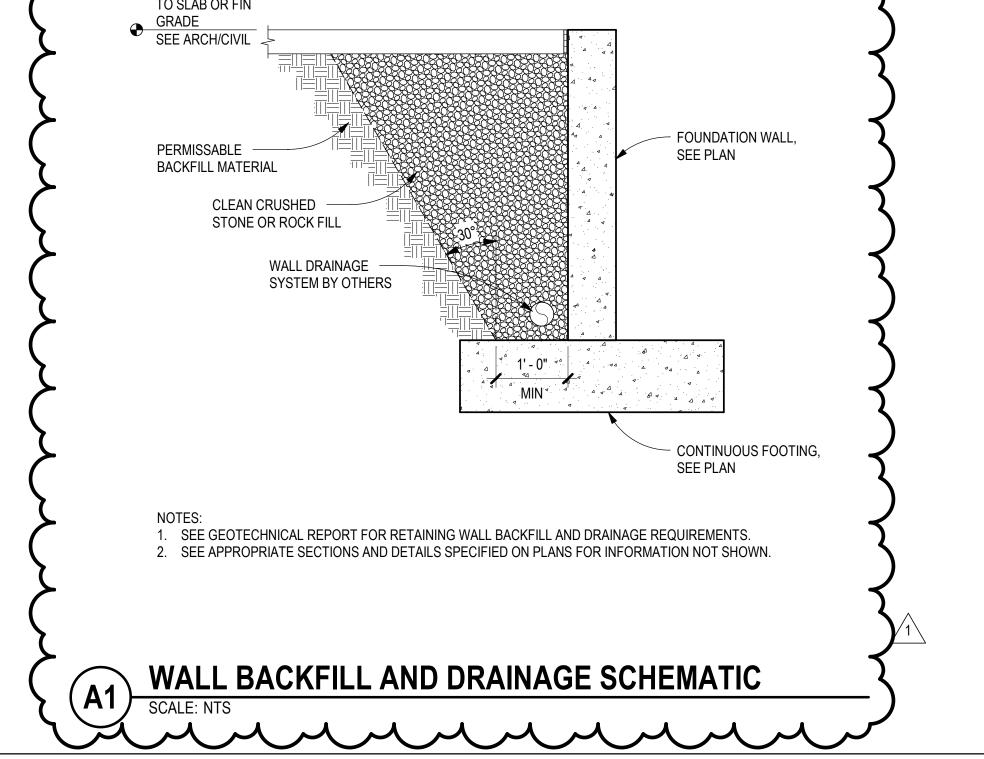
POLLUTION CONTROLS: USE WATER SPRINKLING, TEMPORARY ENCLOSURES, AND OTHER SUITABLE METHODS TO LIMIT DUST AND DIRT RISING AND SCATTERING IN THE AIR TO LOWEST PRACTICAL LEVEL. COMPLY WITH GOVERNING REGULATIONS PERTAINING TO ENVIRONMENTAL PROTECTION.

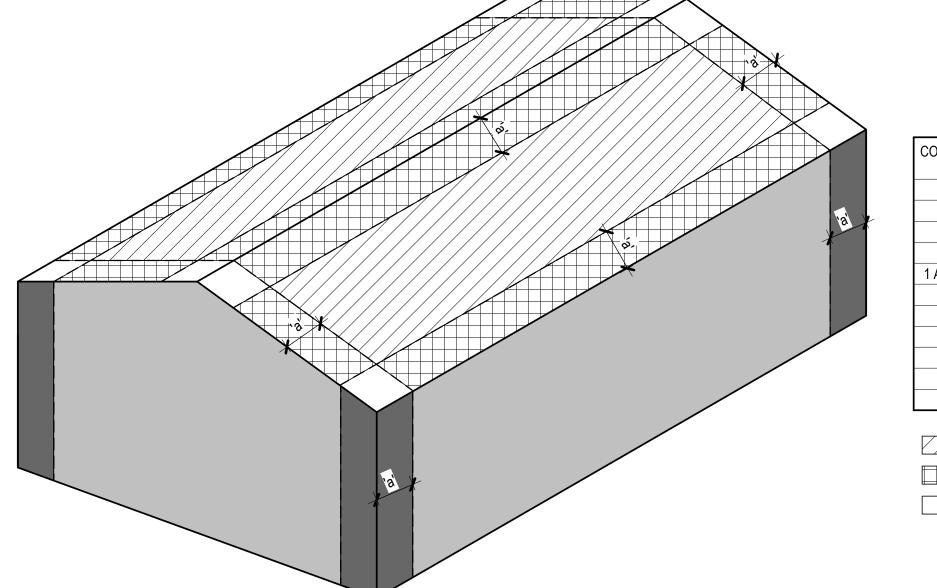
REMOVE DEBRIS FROM THE SITE AS IT ACCUMULATES. UNLESS OTHERWISE NOTED, DO NOT STORE, SELL, BURN, OR OTHERWISE DISPOSE OF DEBRIS ON THE SITE. REMOVAL OF DEBRIS INCLUDES CLEARING OF ALL LOWER LEVELS AND SIMILAR BELOW GRADE STRUCTURES. REMOVE ALL DEBRIS IN SUCH A MANNER AS TO PREVENT SPILLAGE. KEEP ALL PAVEMENTS AND AREAS ADJACENT TO THE SITE CLEAN AND FREE FROM MUD, DIRT, AND DEBRIS AT ALL TIMES.

USE OF EXPLOSIVES: THE CONTRACTOR IS ABSOLUTELY PROHIBITED FROM USING DYNAMITE OR ANY OTHER EXPLOSIVES IN ANY OF THE WORK OR OPERATIONS SHOWN ON THESE PLANS AT THE PROJECT

DEMOLITION SHALL BE PERFORMED IN A MANNER THAT WILL NOT DAMAGE ADJOINING SURFACES INDICATED TO REMAIN. SURFACES SHALL BE PATCHED, IF REQUIRED, TO PROVIDE A SUITABLE SUBSTRATE FOR NEW CONSTRUCTION.

SPECIFIC DEMOLITION NOTES ARE NOT TO BE CONSIDERED ALL INCLUSIVE OR COMPLETE IN THEMSELVES. CONTRACTOR SHALL PROVIDE ALL DEMOLITION INCIDENTAL TO OR REQUIRED FOR CONSTRUCTION WHETHER SPECIFICALLY NOTED OR NOT.



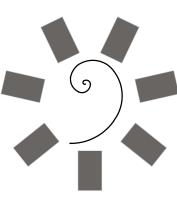


COMPONENTS AND CLA CALCULATED AT ME			•
a = 9 FT	EFFECTI	VE WIND AF	REA (FT ²)
ZONE	10	100	500
1	-58.1	-48.2	-40.7
2	-91.3	-75.8	-63.9
1 AND 2 OVERHANGS	-91.3	-91.3	-91.3
3	-124.4	-103.2	-87.1
3 OVERHANGS	-143.0	-134.4	-128.7
4	39.7	35.0	31.8
4 PARAPETS	131.0	115.3	104.8
5	72.8	55.6	43.7
5 PARAPETS	164.1	125.2	98.5

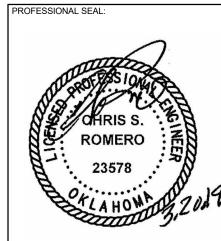
ZONE 1 ZONE 4 ZONE 2 ZONE 5 ZONE 3

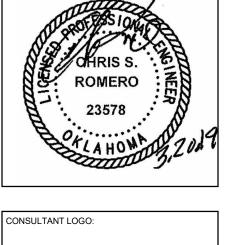
COMPONENT AND CLADDING WIND LOADING DIAGRAM

SCALE: NTS

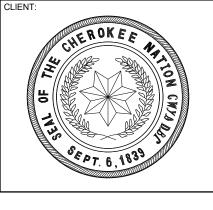


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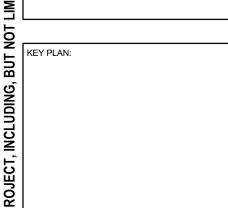












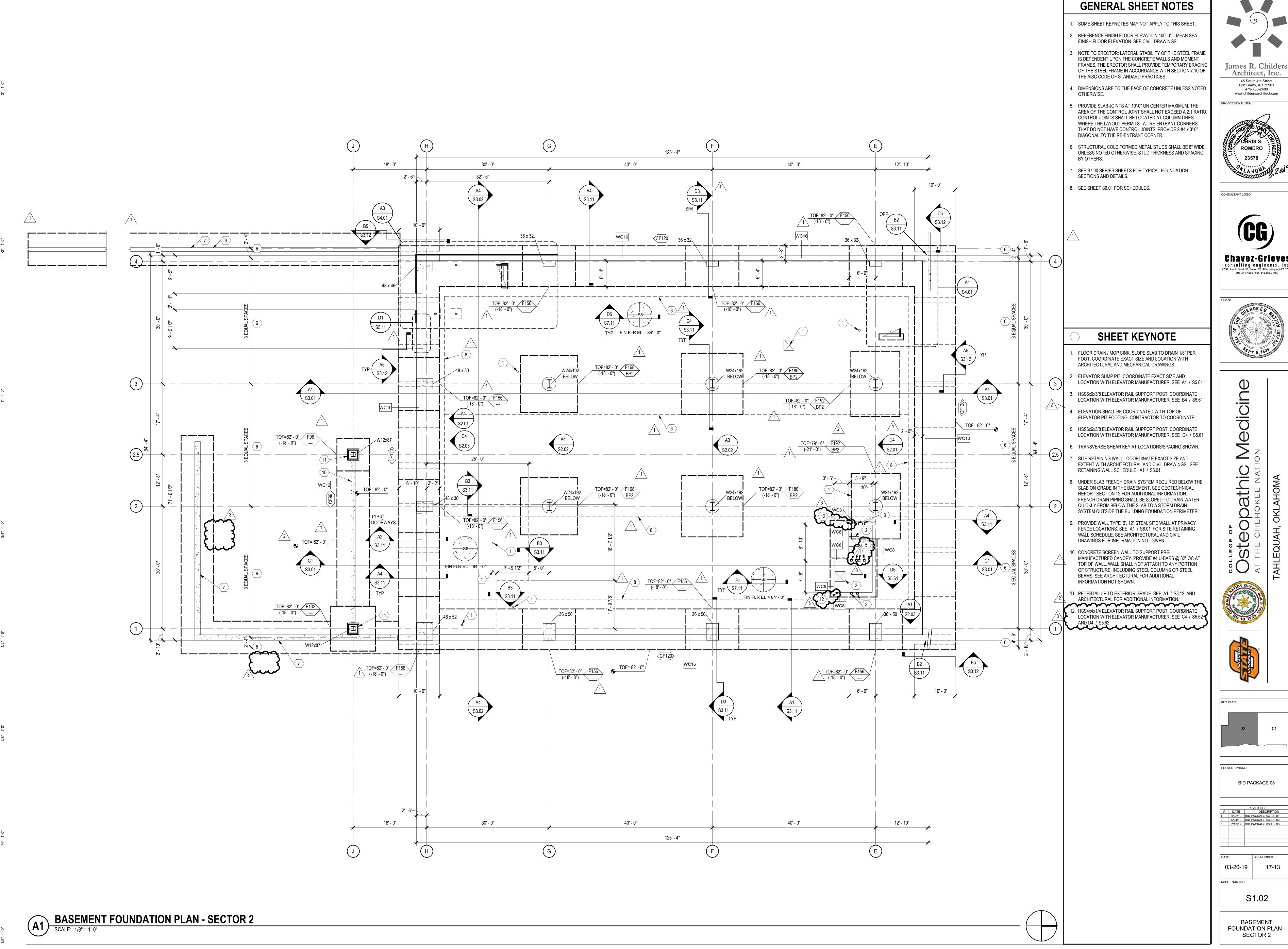
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BID PACKAGE 03

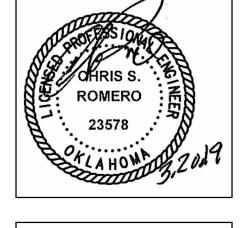
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GENERAL STRUCTURAL NOTES AND SPECIAL INSPECTION TABLES





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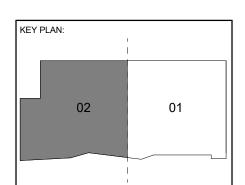
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PROJECT PHASE: **BID PACKAGE 03**

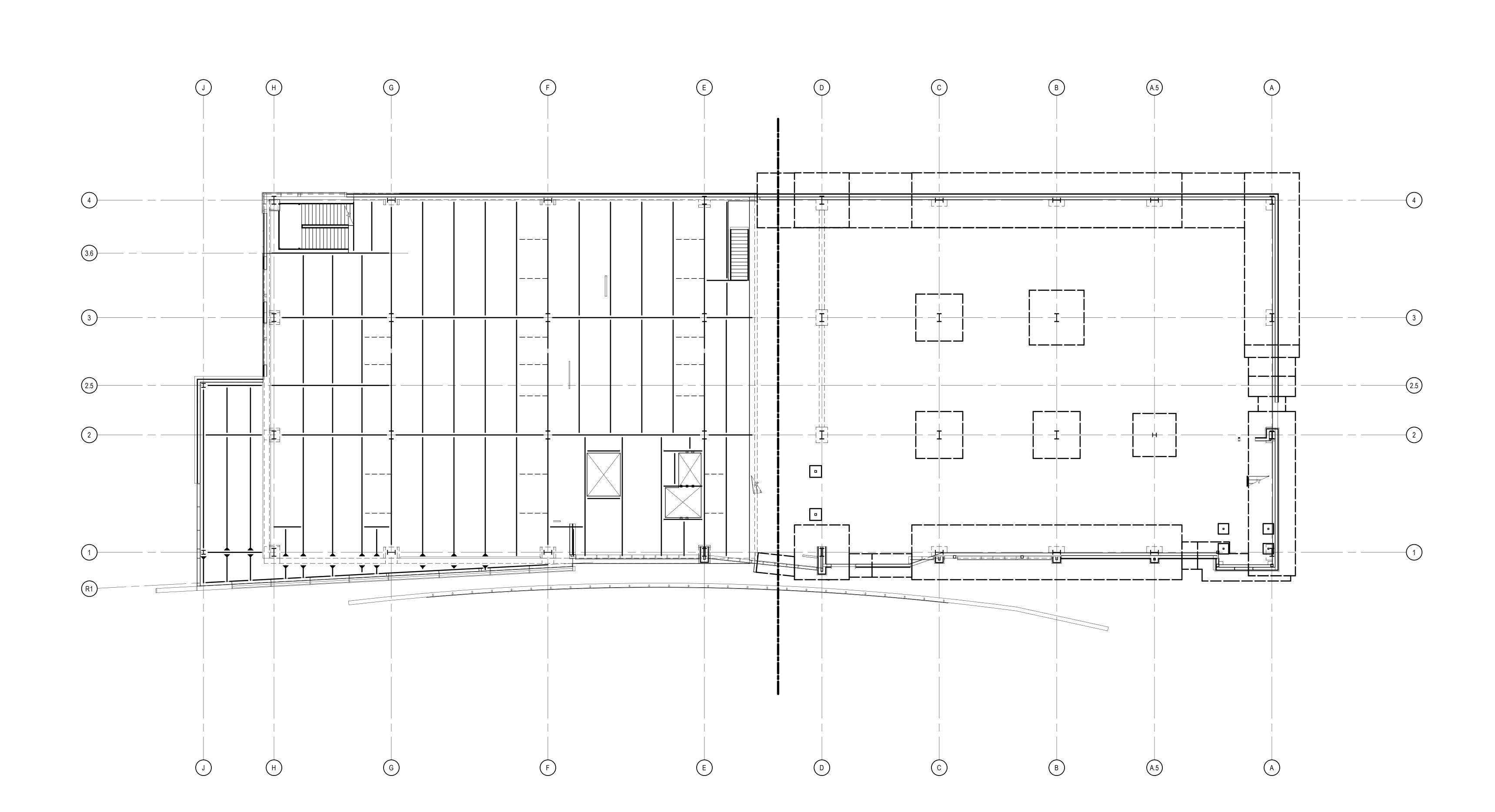
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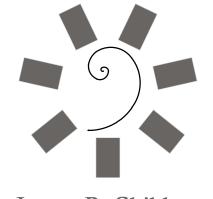
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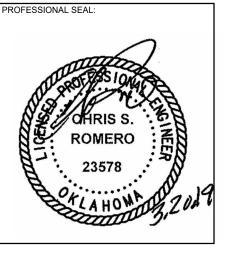
S1.02

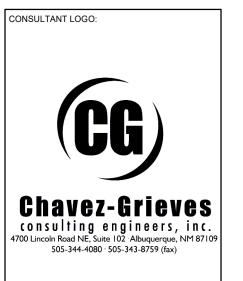
BASEMENT FOUNDATION PLAN -SECTOR 2

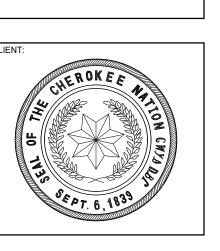


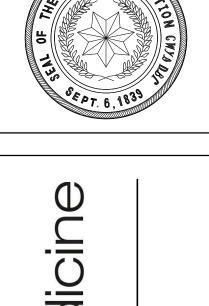


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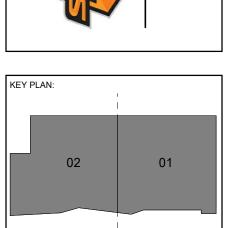


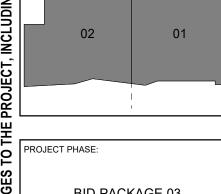












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본 FIRST FLOOR FRAMING 비 PLAN - OVERALL PLAN

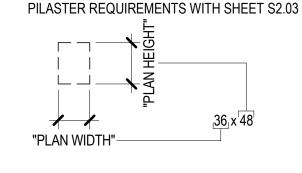
- SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE CONCRETE WALLS AND MOMENT
- OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES. DIMENSIONS ARE TO THE FACE OF CONCRETE OR STUDS
 - UNLESS NOTED OTHERWISE. . SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS

FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING

NOT SHOWN. BEAMS ARE SPACED EQUALLY BETWEEN GRIDS UNLESS NOTED

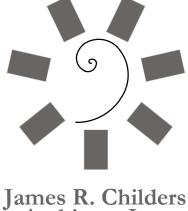
UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING

- OTHERWISE. . STRUCTURAL COLD FORMED METAL STUDS SHALL BE 8" WIDE
- BY OTHERS. SEE S7.00 SERIES SHEETS FOR TYPICAL DETAILS.
- . SEE SHEET S6.01 FOR SCHEDULES.
- ALL MOMENT FRAMES LABELED ON PLAN UTILIZE SIDEPLATE PROPRIETARY MOMENT CONNECTIONS. SEE SHEETS S8.01 -
- DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
- DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE
- DRAWINGS, SHEETS S8.01 S8.08. 2. DIMENSIONS SHOWN ON PLAN AS FOLLOWS ARE CONCRETE PILASTER DIMENSIONS IN INCHES: 38x36, 50x36, ETC. DIMENSIONS ARE "PLAN WIDTH" x "PLAN HEIGHT". COORDINATE



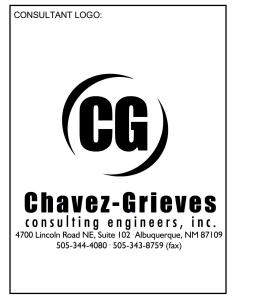
SHEET KEYNOTE

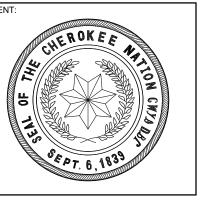
- HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST, COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE B4 / S5.62, C4 / S5.62, D4 / S5.62
- OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A5 / S5.52 FOR SUPPORT.
- BEAM SPLICE LOCATION. SEE B4 / S5.52 FOR SPLICE DETAIL.
- HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE A4 / S5.62, B4 / S5.62, C4 / S5.62, D4 / S5.62, AND C3 / S5.62
- . W8x31 OUTRIGGER.
- 6. TOTAL NUMBER OF CHORD REINFORCEMENT BARS AT EXTENTS SHOWN. CHORD REINFORCEMENT SHALL BE LOCATED AS INDICATED ON PLAN. PROVIDE 130% LAP SPLICES WHEN REQUIRED.
- 3-#7 SLAB REINFORCING BARS, EXTEND BARS 130% OF A LAP SPLICE LENGTH BEYOND OPENING, OR PROVIDE STD 90 DEGREE HOOK WHERE REQUIRED.
- BOTTOM FLANGE BRACING AT EQUAL SPACING, UNLESS NOTED OTHERWISE. SEE B1 / S5.52
- BOTTOM FLANGE BRACING SPACED AT 10' 0" ON CENTER MAXIMUM, UNLESS NOTED OTHERWISE. SEE A1 / S5.52
- 10. BACKFILL PLACED AGAINST WALL SHALL BE DONE IN EQUAL LIFTS, ALTERNATING EACH SIDE OF WALL TO PREVENT UNINTENDED RETAINAGE OF SOIL.
- 11. SITE WALL. COORDINATE EXACT SIZE, EXTENT, AND RADIAL DIMENSIONS WITH ARCHITECTURAL AND CIVIL DRAWINGS. SEE B4 / S3.11
- 12. PROVIDE STEMWALL FOR SUPPORT OF EXTERIOR STUDS AND VENEER, SEE D1 / S3.11
- 13. PODIUM FRAMING, COORDINATE EXACT SIZE AND LOCATION WITH ARCHITECTURAL DRAWINGS.
- 14. CONTRACTOR TO COORDINATE EXACT EXTENT OF SECTION FOR WALL FRAMING REQUIREMENTS WITH ARCHITECTURAL ELEVATIONS.
- 15. COVER PLATE BEAM AT EXTENTS SHOWN. DIMENSIONS GIVEN ARE TO FACE/EDGE OF FLANGE. SEE D5 / S3.23 FOR FRAMING DETAIL.
- 16. OUTRIGGER BEAM (STUB). SEE D5 / S3.23
- 17. CONTRACTOR TO COORDINATE FLOOR OPENING WITH ARCHITECTURAL AND MECHANICAL.
- 18. FLOOR DRAIN / MOP SINK, SLOPE SLAB TO DRAIN 1/8" PER FOOT. COORDINATE EXACT SIZE AND LOCATION WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
- 19. BOTTOM FLANGE CROSS BRACING AT EQUAL SPACING UNLESS NOTED OTHERWISE. NO CONNECTION AT BRACE INTERSECTION. SEE B1 / S5.52
- 20. BEAM SHALL BE CENTERED BETWEEN ELEVATOR GUIDERAIL SUPPORTS. CONTRACTOR TO COORDINATE BEAM LOCATION WITH SELECTED ELEVATOR.
- 21. HSS4x4x1/4 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE C4 / S5.62 AND D4 / S5.62

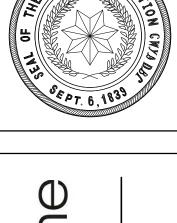


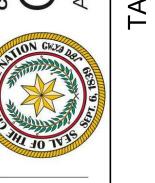


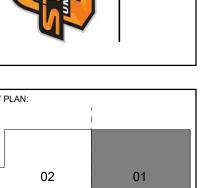


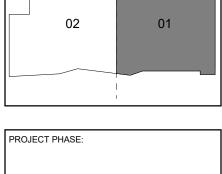




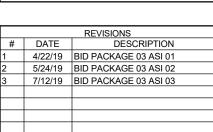








BID PACKAGE 03



03-20-19

S1.11

SHEET NUMBER:

FIRST FLOOR FOUNDATION AND FRAMING PLAN -SECTOR 1

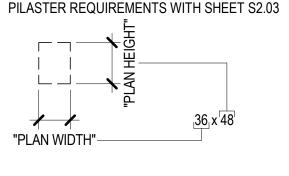
- SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE CONCRETE WALLS AND MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING

OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF

- THE AISC CODE OF STANDARD PRACTICES. DIMENSIONS ARE TO THE FACE OF CONCRETE OR STUDS UNLESS NOTED OTHERWISE.
- . SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
- BEAMS ARE SPACED EQUALLY BETWEEN GRIDS UNLESS NOTE OTHERWISE.
- STRUCTURAL COLD FORMED METAL STUDS SHALL BE 8" WIDE UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING BY OTHERS.
- . SEE S7.00 SERIES SHEETS FOR TYPICAL DETAILS.
- 8. SEE SHEET S6.01 FOR SCHEDULES.

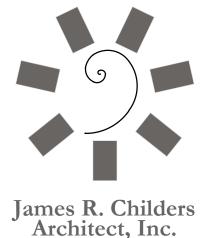
DRAWINGS, SHEETS S8.01 - S8.08.

- . ALL MOMENT FRAMES LABELED ON PLAN UTILIZE SIDEPLATE PROPRIETARY MOMENT CONNECTIONS. SEE SHEETS S8.01
- 10. DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
- DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE
- DIMENSIONS SHOWN ON PLAN AS FOLLOWS ARE CONCRETE PILASTER DIMENSIONS IN INCHES: 38x36, 50x36, ETC. DIMENSIONS ARE "PLAN WIDTH" x "PLAN HEIGHT". COORDINATE

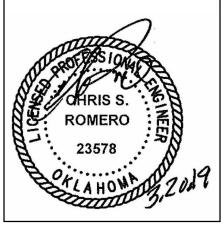




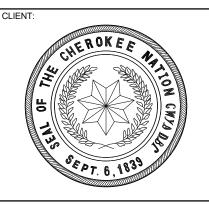
- HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST, COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE B4 / S5.62, C4 / S5.62, D4 / S5.62
- OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A5 / S5.52 FOR SUPPORT.
- BEAM SPLICE LOCATION. SEE B4 / S5.52 FOR SPLICE DETAIL.
- HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE A4 / S5.62, B4 / S5.62, C4 / S5.62, D4 / S5.62, AND C3 / S5.62
- 5. W8x31 OUTRIGGER.
- . TOTAL NUMBER OF CHORD REINFORCEMENT BARS AT EXTENTS SHOWN. CHORD REINFORCEMENT SHALL BE LOCATED AS INDICATED ON PLAN. PROVIDE 130% LAP SPLICES WHEN REQUIRED.
- 3-#7 SLAB REINFORCING BARS. EXTEND BARS 130% OF A LAP SPLICE LENGTH BEYOND OPENING, OR PROVIDE STD 90 DEGREE HOOK WHERE REQUIRED.
- BOTTOM FLANGE BRACING AT EQUAL SPACING, UNLESS NOTED OTHERWISE. SEE B1 / S5.52
- BOTTOM FLANGE BRACING SPACED AT 10' 0" ON CENTER MAXIMUM, UNLESS NOTED OTHERWISE. SEE A1 / S5.52
- 10. BACKFILL PLACED AGAINST WALL SHALL BE DONE IN EQUAL LIFTS, ALTERNATING EACH SIDE OF WALL TO PREVENT UNINTENDED RETAINAGE OF SOIL.
- 11. SITE WALL. COORDINATE EXACT SIZE, EXTENT, AND RADIAL DIMENSIONS WITH ARCHITECTURAL AND CIVIL DRAWINGS. SEE B4 / S3.11
- 12.1 PROVIDE STEMWALL FOR SUPPORT OF EXTERIOR STUDS AND VENEER, SEE D1 / S3.11
- . 13. PODIUM FRAMING, COORDINATE EXACT SIZE AND LOCATION WITH ARCHITECTURAL DRAWINGS.
- 14. CONTRACTOR TO COORDINATE EXACT EXTENT OF SECTION FOR WALL FRAMING REQUIREMENTS WITH ARCHITECTURAL ELEVATIONS.
- 15. COVER PLATE BEAM AT EXTENTS SHOWN. DIMENSIONS GIVEN ARE TO FACE/EDGE OF FLANGE. SEE D5 / S3.23 FOR FRAMING DETAIL.
- 16. OUTRIGGER BEAM (STUB). SEE D5 / S3.23
- 17. CONTRACTOR TO COORDINATE FLOOR OPENING WITH ARCHITECTURAL AND MECHANICAL.
- 18. FLOOR DRAIN / MOP SINK, SLOPE SLAB TO DRAIN 1/8" PER FOOT. COORDINATE EXACT SIZE AND LOCATION WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
- 19. BOTTOM FLANGE CROSS BRACING AT EQUAL SPACING UNLESS NOTED OTHERWISE. NO CONNECTION AT BRACE INTERSECTION. SEE B1 / S5.52
- 20. BEAM SHALL BE CENTERED BETWEEN ELEVATOR GUIDERAIL SUPPORTS. CONTRACTOR TO COORDINATE BEAM LOCATION WITH SELECTED ELEVATOR.
- \cdots . HSS4x4x1/4 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE C4 / S5.62 mmmmm

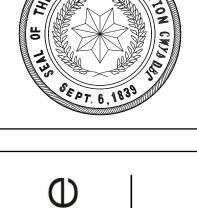






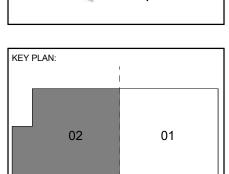


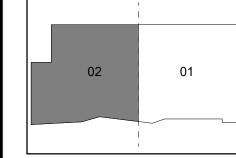












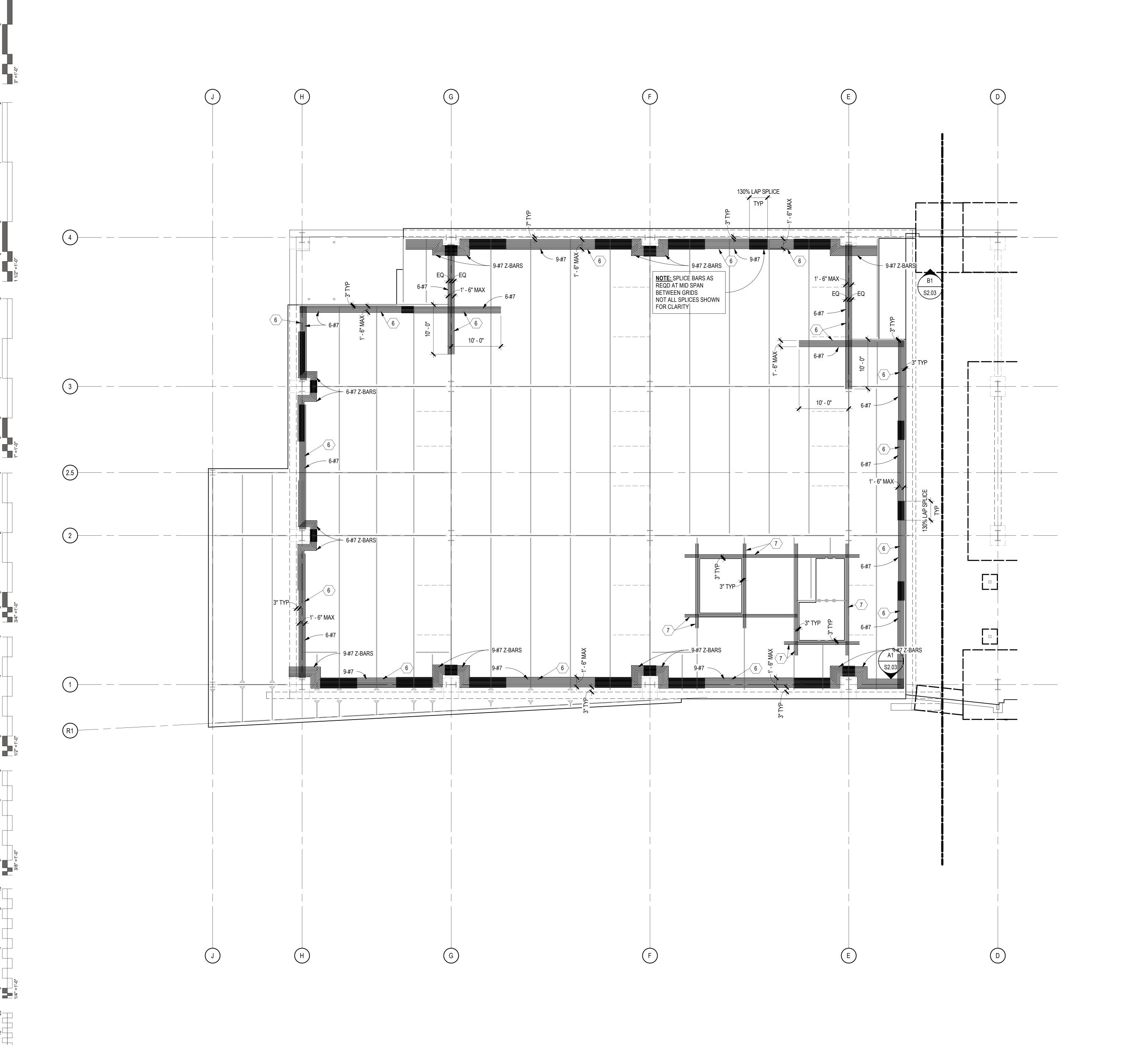
PROJECT PHASE: BID PACKAGE 03

REVISIONS
DESCRIPTION 4/22/19 BID PACKAGE 03 ASI 01 5/24/19 BID PACKAGE 03 ASI 02 1 6/17/19 BID PACKAGE 04 ASI 02 7/12/19 BID PACKAGE 03 ASI 03

03-20-19

S1.12

FIRST FLOOR FRAMING PLAN - SECTOR 2



- . SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- 2. NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE CONCRETE WALLS AND MOMENT

FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF

DIMENSIONS ARE TO THE FACE OF CONCRETE OR STUDS UNLESS NOTED OTHERWISE.

THE AISC CODE OF STANDARD PRACTICES.

- 4. SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
- . BEAMS ARE SPACED EQUALLY BETWEEN GRIDS UNLESS NOTED

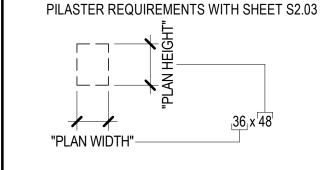
UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING

- OTHERWISE. 6. STRUCTURAL COLD FORMED METAL STUDS SHALL BE 8" WIDE
- . SEE S7.00 SERIES SHEETS FOR TYPICAL DETAILS.
- 8. SEE SHEET S6.01 FOR SCHEDULES.

BY OTHERS.

DRAWINGS.

- 9. ALL MOMENT FRAMES LABELED ON PLAN UTILIZE SIDEPLATE PROPRIETARY MOMENT CONNECTIONS. SEE S8.00 SERIES
- DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
- DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE
- 12. DIMENSIONS SHOWN ON PLAN AS FOLLOWS ARE CONCRETE PILASTER DIMENSIONS IN INCHES: 38x36, 50x36, ETC. DIMENSIONS ARE "PLAN WIDTH" x "PLAN HEIGHT". COORDINATE

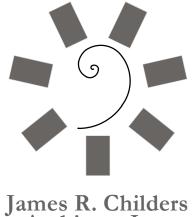


SHEET KEYNOTE

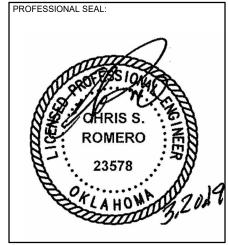
- 1. HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST, COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE B4 / S5.62, C4 / S5.62, D4 / S5.62
- 2. OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A5 / S5.52 FOR SUPPORT.
- 3. BEAM SPLICE LOCATION. SEE B4 / S5.52 FOR SPLICE DETAIL.
- 4. HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE A4 / S5.62,

B4 / S5.62, C4 / S5.62, D4 / S5.62, AND C3 / S5.62

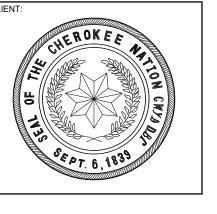
- 5. W8x31 OUTRIGGER.
- 6. TOTAL NUMBER OF CHORD REINFORCEMENT BARS AT EXTENTS SHOWN. CHORD REINFORCEMENT SHALL BE LOCATED AS INDICATED ON PLAN. PROVIDE 130% LAP SPLICES WHEN REQUIRED.
- 7. 3-#7 SLAB REINFORCING BARS. EXTEND BARS 130% OF A LAP SPLICE LENGTH BEYOND OPENING, OR PROVIDE STD 90 DEGREE HOOK WHERE REQUIRED.
- 8. BOTTOM FLANGE BRACING AT EQUAL SPACING, UNLESS NOTED OTHERWISE. SEE B1 / S5.52
- 9. BOTTOM FLANGE BRACING SPACED AT 10' 0" ON CENTER MAXIMUM, UNLESS NOTED OTHERWISE. SEE A1 / S5.52
- 10. BACKFILL PLACED AGAINST WALL SHALL BE DONE IN EQUAL LIFTS, ALTERNATING EACH SIDE OF WALL TO PREVENT UNINTENDED RETAINAGE OF SOIL.



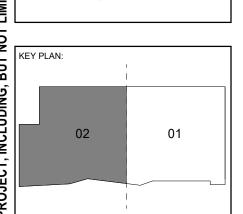
Architect, Inc. 45 South 4th Street Fort Smith, AR 72901 479-783-2480 www.childersarchitect.com











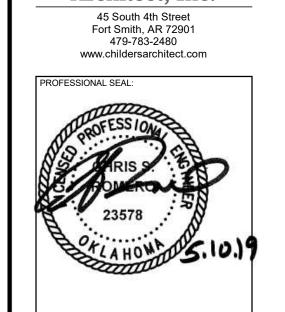
PROJECT PHASE: BID PACKAGE 03

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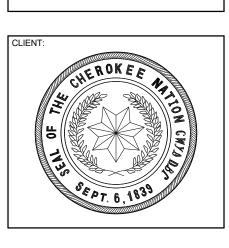
S1.13

FIRST FLOOR SLAB REINFORCING PLAN -SECTOR 2





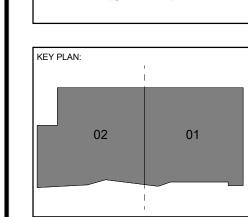


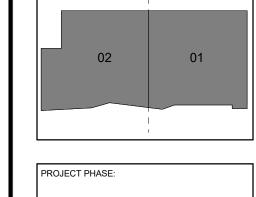












	PROJECT PHASE:
	BID PACKAGE 04

			REVISIONS
	#	DATE	DESCRIPTION

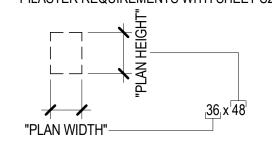
DATE:	JOB NUMBER:
05-10-19	17-13
SHEET NUMBER:	

- SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE CONCRETE WALLS AND MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
- DIMENSIONS ARE TO THE FACE OF CONCRETE OR STUDS UNLESS NOTED OTHERWISE.
- 4. SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
- BEAMS ARE SPACED EQUALLY BETWEEN GRIDS UNLESS NOTED
- 6. STRUCTURAL COLD FORMED METAL STUDS SHALL BE 8" WIDE UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING
- '. SEE S7.00 SERIES SHEETS FOR TYPICAL DETAILS.
- 8. SEE SHEET S6.01 FOR SCHEDULES.

OTHERWISE.

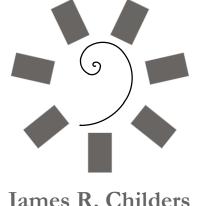
BY OTHERS.

- 9. ALL MOMENT FRAMES LABELED ON PLAN UTILIZE SIDEPLATE PROPRIETARY MOMENT CONNECTIONS. SEE SHEETS S8.01 -
- DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
- DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE DRAWINGS, SHEETS S8.01 - S8.08.
- 12. DIMENSIONS SHOWN ON PLAN AS FOLLOWS ARE CONCRETE PILASTER DIMENSIONS IN INCHES: 38x36, 50x36, ETC. DIMENSIONS ARE "PLAN WIDTH" x "PLAN HEIGHT". COORDINATE PILASTER REQUIREMENTS WITH SHEET S2.03

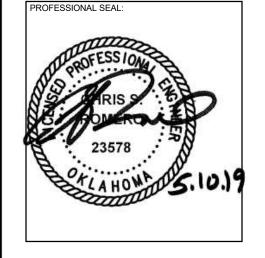


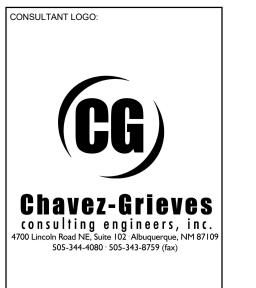
SHEET KEYNOTE

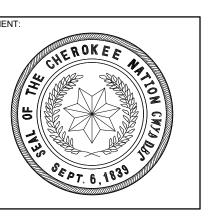
- HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST, COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE B4 / S5.62, C4 / S5.62, D4 / S5.62
- OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A5 / S5.52 FOR SUPPORT.
- . BEAM SPLICE LOCATION. SEE B4 / S5.52 FOR SPLICE DETAIL.
- . HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE A4 / S5.62, B4 / S5.62, C4 / S5.62, D4 / S5.62, AND C3 / S5.62
- 5. W8x31 OUTRIGGER.
- . TOTAL NUMBER OF CHORD REINFORCEMENT BARS AT EXTENTS SHOWN. CHORD REINFORCEMENT SHALL BE LOCATED AS INDICATED ON PLAN. PROVIDE 130% LAP SPLICES WHEN REQUIRED.
- 3-#7 SLAB REINFORCING BARS. EXTEND BARS 130% OF A LAP SPLICE LENGTH BEYOND OPENING, OR PROVIDE STD 90 DEGREE HOOK WHERE REQUIRED.
- BOTTOM FLANGE BRACING AT EQUAL SPACING, UNLESS NOTED OTHERWISE. SEE B1 / S5.52
- . BOTTOM FLANGE BRACING SPACED AT 10' 0" ON CENTER MAXIMUM, UNLESS NOTED OTHERWISE. SEE A1 / S5.52
- 10. BACKFILL PLACED AGAINST WALL SHALL BE DONE IN EQUAL LIFTS, ALTERNATING EACH SIDE OF WALL TO PREVENT UNINTENDED RETAINAGE OF SOIL.
- 11. SITE WALL. COORDINATE EXACT SIZE, EXTENT, AND RADIAL DIMENSIONS WITH ARCHITECTURAL AND CIVIL DRAWINGS. SEE B4 / S3.11
- 12. PROVIDE STEMWALL FOR SUPPORT OF EXTERIOR STUDS AND VENEER, SEE D1 / S3.11
- 13. PODIUM FRAMING, COORDINATE EXACT SIZE AND LOCATION WITH ARCHITECTURAL DRAWINGS.
- 14. CONTRACTOR TO COORDINATE EXACT EXTENT OF SECTION FOR WALL FRAMING REQUIREMENTS WITH ARCHITECTURAL ELEVATIONS.
- 15. COVER PLATE BEAM AT EXTENTS SHOWN. DIMENSIONS GIVEN ARE TO FACE/EDGE OF FLANGE. SEE D5 / S3.23 FOR FRAMING DETAIL.
- 16. OUTRIGGER BEAM (STUB). SEE D5 / S3.23
- 17. CONTRACTOR TO COORDINATE FLOOR OPENING WITH ARCHITECTURAL AND MECHANICAL.
- 18. FLOOR DRAIN / MOP SINK, SLOPE SLAB TO DRAIN 1/8" PER FOOT. COORDINATE EXACT SIZE AND LOCATION WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
- 19. BOTTOM FLANGE CROSS BRACING AT EQUAL SPACING UNLESS NOTED OTHERWISE. NO CONNECTION AT BRACE INTERSECTION. SEE B1 / S5.52
- 20. BEAM SHALL BE CENTERED BETWEEN ELEVATOR GUIDERAIL SUPPORTS. CONTRACTOR TO COORDINATE BEAM LOCATION WITH SELECTED ELEVATOR.
- auI. HSS4x4x1/4 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE C4 / S5.62 AND D4 / S5.62
- 22. STRUCTURAL MEMBER SUPPORTING EXIT STAIRS. SEE LIFE 🚽 SAFETY DRAWINGS FOR FIREPROOFING REQUIREMENTS.



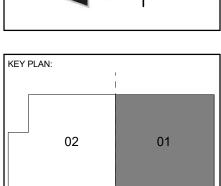
James R. Childers Architect, Inc. 45 South 4th Street Fort Smith, AR 72901 479-783-2480 www.childersarchitect.com

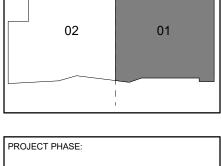


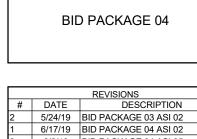












8/8/19 BID PACKAGE 04 ASI 05

05-10-19 SHEET NUMBER:

S1.21

SECOND FLOOR FRAMING PLAN -SECTOR 1

- SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE CONCRETE WALLS AND MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING

OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF

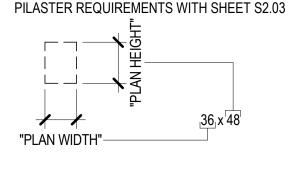
DIMENSIONS ARE TO THE FACE OF CONCRETE OR STUDS UNLESS NOTED OTHERWISE.

THE AISC CODE OF STANDARD PRACTICES.

- . SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
- BEAMS ARE SPACED EQUALLY BETWEEN GRIDS UNLESS NOTED
- OTHERWISE. STRUCTURAL COLD FORMED METAL STUDS SHALL BE 8" WIDE UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING
- SEE S7.00 SERIES SHEETS FOR TYPICAL DETAILS.
- 8. SEE SHEET S6.01 FOR SCHEDULES.

BY OTHERS.

- . ALL MOMENT FRAMES LABELED ON PLAN UTILIZE SIDEPLATE PROPRIETARY MOMENT CONNECTIONS. SEE SHEETS S8.01 -
- DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
- DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE DRAWINGS, SHEETS S8.01 - S8.08.
- 12. DIMENSIONS SHOWN ON PLAN AS FOLLOWS ARE CONCRETE PILASTER DIMENSIONS IN INCHES: 38x36, 50x36, ETC. DIMENSIONS ARE "PLAN WIDTH" x "PLAN HEIGHT". COORDINATE



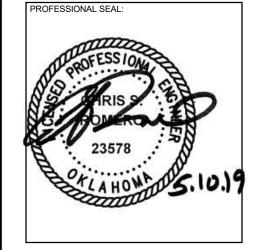
SHEET KEYNOTE

- HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST, COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE B4 / S5.62, C4 / S5.62, D4 / S5.62
- OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A5 / S5.52 FOR SUPPORT.
- BEAM SPLICE LOCATION. SEE B4 / S5.52 FOR SPLICE DETAIL.
- 4. HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE A4 / S5.62, B4 / S5.62, C4 / S5.62, D4 / S5.62, AND C3 / S5.62
- 5. W8x31 OUTRIGGER.
- 6. TOTAL NUMBER OF CHORD REINFORCEMENT BARS AT EXTENTS SHOWN. CHORD REINFORCEMENT SHALL BE LOCATED AS INDICATED ON PLAN. PROVIDE 130% LAP SPLICES WHEN REQUIRED.
- 3-#7 SLAB REINFORCING BARS. EXTEND BARS 130% OF A LAP SPLICE LENGTH BEYOND OPENING, OR PROVIDE STD 90 DEGREE HOOK WHERE REQUIRED.
- BOTTOM FLANGE BRACING AT EQUAL SPACING, UNLESS NOTED OTHERWISE. SEE B1 / S5.52
- BOTTOM FLANGE BRACING SPACED AT 10' 0" ON CENTER MAXIMUM, UNLESS NOTED OTHERWISE. SEE A1 / S5.52
- 10. BACKFILL PLACED AGAINST WALL SHALL BE DONE IN EQUAL LIFTS, ALTERNATING EACH SIDE OF WALL TO PREVENT UNINTENDED RETAINAGE OF SOIL.
- 11. SITE WALL. COORDINATE EXACT SIZE, EXTENT, AND RADIAL DIMENSIONS WITH ARCHITECTURAL AND CIVIL DRAWINGS. SEE B4 / S3.11
- 12. PROVIDE STEMWALL FOR SUPPORT OF EXTERIOR STUDS AND VENEER, SEE D1 / S3.11
- 13. PODIUM FRAMING, COORDINATE EXACT SIZE AND LOCATION WITH ARCHITECTURAL DRAWINGS.
- 14. CONTRACTOR TO COORDINATE EXACT EXTENT OF SECTION FOR WALL FRAMING REQUIREMENTS WITH ARCHITECTURAL ELEVATIONS.
- 15. COVER PLATE BEAM AT EXTENTS SHOWN. DIMENSIONS GIVEN ARE TO FACE/EDGE OF FLANGE. SEE D5 / S3.23 FOR FRAMING DETAIL.
- 16. OUTRIGGER BEAM (STUB). SEE D5 / S3.23
- 17. CONTRACTOR TO COORDINATE FLOOR OPENING WITH ARCHITECTURAL AND MECHANICAL.
- 18. FLOOR DRAIN / MOP SINK, SLOPE SLAB TO DRAIN 1/8" PER FOOT. COORDINATE EXACT SIZE AND LOCATION WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
- 19. BOTTOM FLANGE CROSS BRACING AT EQUAL SPACING UNLESS NOTED OTHERWISE. NO CONNECTION AT BRACE INTERSECTION. SEE B1 / S5.52
- 20. BEAM SHALL BE CENTERED BETWEEN ELEVATOR GUIDERAIL SUPPORTS. CONTRACTOR TO COORDINATE BEAM LOCATION WITH SELECTED ELEVATOR. \sim 21. HSS4x4x1/4 ELEVATOR RAIL SUPPORT POST, COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE C4 / S5.62°
- 22. STRUCTURAL MEMBER SUPPORTING EXIT STAIRS. SEE LIFE SAFETY DRAWINGS FOR FIREPROOFING REQUIREMENTS. munumum)

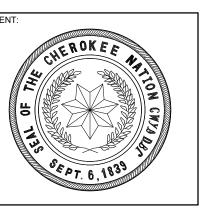
AND D4 / S5.62

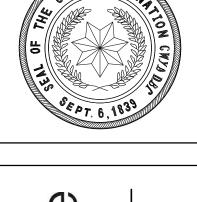


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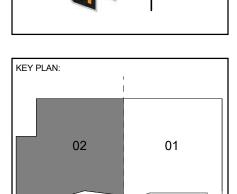


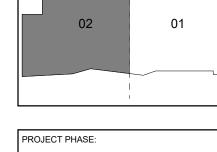












BID PACKAGE 04

| REVISIONS | | DATE | DESCRIPTION | | 5/24/19 | BID PACKAGE 03 ASI 02 | 6/17/19 | BID PACKAGE 04 ASI 02 | 8/8/19 BID PACKAGE 04 ASI 05

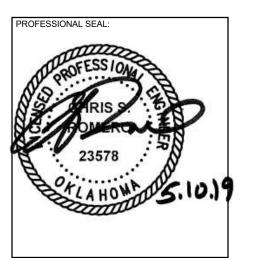
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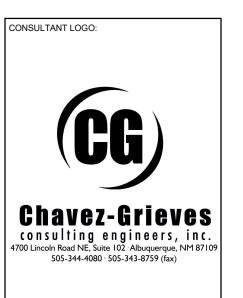
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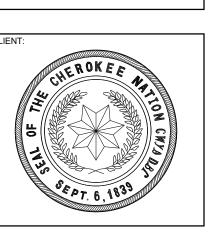
SECOND FLOOR FRAMING PLAN -SECTOR 2

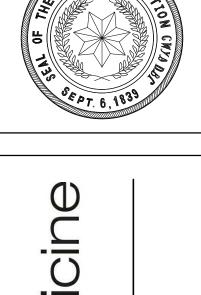


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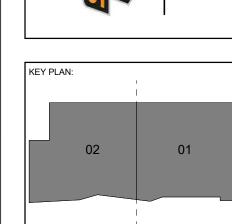


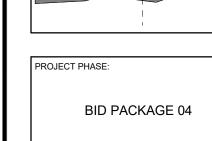












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05-10-19 17-13 S1.30

THIRD FLOOR FRAMING PLAN - OVERALL PLAN

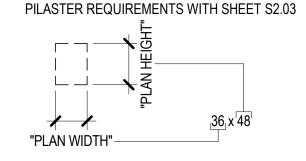
THIRD FLOOR FRAMING PLAN - OVERALL PLAN

SCALE: 3/32" = 1'-0"

- SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE CONCRETE WALLS AND MOMENT

FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF

- THE AISC CODE OF STANDARD PRACTICES.
 - DIMENSIONS ARE TO THE FACE OF CONCRETE OR STUDS UNLESS NOTED OTHERWISE.
 - . SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
 - BEAMS ARE SPACED EQUALLY BETWEEN GRIDS UNLESS NOTE OTHERWISE.
 - STRUCTURAL COLD FORMED METAL STUDS SHALL BE 8" WIDE UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING
 - SEE S7.00 SERIES SHEETS FOR TYPICAL DETAILS.
 - 8. SEE SHEET S6.01 FOR SCHEDULES.
 - 9. ALL MOMENT FRAMES LABELED ON PLAN UTILIZE SIDEPLATE PROPRIETARY MOMENT CONNECTIONS. SEE SHEETS \$8.01 -
 - DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
 - DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE DRAWINGS, SHEETS S8.01 - S8.08.
 - 12. DIMENSIONS SHOWN ON PLAN AS FOLLOWS ARE CONCRETE PILASTER DIMENSIONS IN INCHES: 38x36, 50x36, ETC. DIMENSIONS ARE "PLAN WIDTH" x "PLAN HEIGHT". COORDINATE



SHEET KEYNOTE

- HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST, COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE B4 / S5.62, C4 / S5.62, D4 / S5.62
- 2. OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A5 / S5.52 FOR SUPPORT.
- BEAM SPLICE LOCATION. SEE B4 / S5.52 FOR SPLICE DETAIL
- HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE A4 / S5.62,
- B4 / S5.62, C4 / S5.62, D4 / S5.62, AND C3 / S5.62
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- BOTTOM FLANGE BRACING AT EQUAL SPACING, UNLESS NOTED OTHERWISE. SEE B1 / S5.52
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- 12. PROVIDE STEMWALL FOR SUPPORT OF EXTERIOR STUDS AND VENEER, SEE D1 / S3.11
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- 1. HSS4x4x1/4 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE C4 / S5.62 AND D4 / S5.62
- 22. STRUCTURAL MEMBER SUPPORTING EXIT STAIRS. SEE LIFE 🜙

BID PACKAGE 04 SAFETY DRAWINGS FOR FIREPROOFING REQUIREMENTS.

	REVISIONS				
#	DATE	DESCRIPTION			
1	6/17/19	BID PACKAGE 04 ASI 02			
2	8/8/19	BID PACKAGE 04 ASI 05			

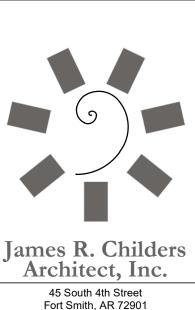
PROJECT PHASE:

05-10-19

THIRD FLOOR FRAMING PLAN - SECTOR 1

S1.31

THIRD FLOOR FRAMING PLAN - SECTOR 1



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PROFESSIONAL SEAL:

CONSULTANT LOGO:

Chavez-Grieves

CONSUITING ENGINEERS, INC. 4700 Lincoln Road NE, Suite 102 · Albuquerque, NM 87109 505-344-4080 · 505-343-8759 (fax)

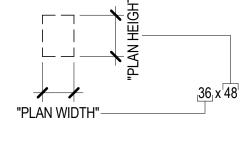
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- UNLESS NOTED OTHERWISE.

 . SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS

DIMENSIONS ARE TO THE FACE OF CONCRETE OR STUDS

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- BEAMS ARE SPACED EQUALLY BETWEEN GRIDS UNLESS NOTED OTHERWISE.
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- SEE S7.00 SERIES SHEETS FOR TYPICAL DETAILS.
- 8. SEE SHEET S6.01 FOR SCHEDULES.
- 9. ALL MOMENT FRAMES LABELED ON PLAN UTILIZE SIDEPLATE PROPRIETARY MOMENT CONNECTIONS. SEE SHEETS S8.01 S8.08.
- DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
- 11. DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE DRAWINGS, SHEETS S8.01 S8.08.
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 DIMENSIONS ARE "PLAN WIDTH" x "PLAN HEIGHT". COORDINATE
 PILASTER REQUIREMENTS WITH SHEET S2.03

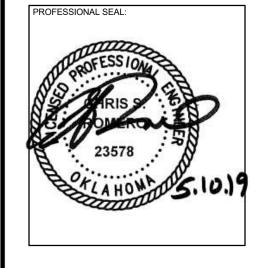


SHEET KEYNOTE

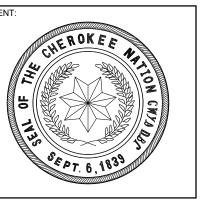
- 1. HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST, COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE B4 / S5.62, C4 / S5.62, D4 / S5.62
- OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A5 / S5.52 FOR SUPPORT.
- BEAM SPLICE LOCATION. SEE B4 / S5.52 FOR SPLICE DETAIL.
- 4. HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE A4 / S5.62, B4 / S5.62, C4 / S5.62, D4 / S5.62, AND C3 / S5.62
- 5. W8x31 OUTRIGGER.
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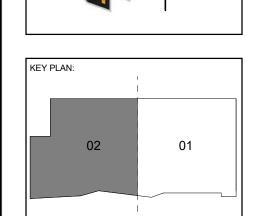




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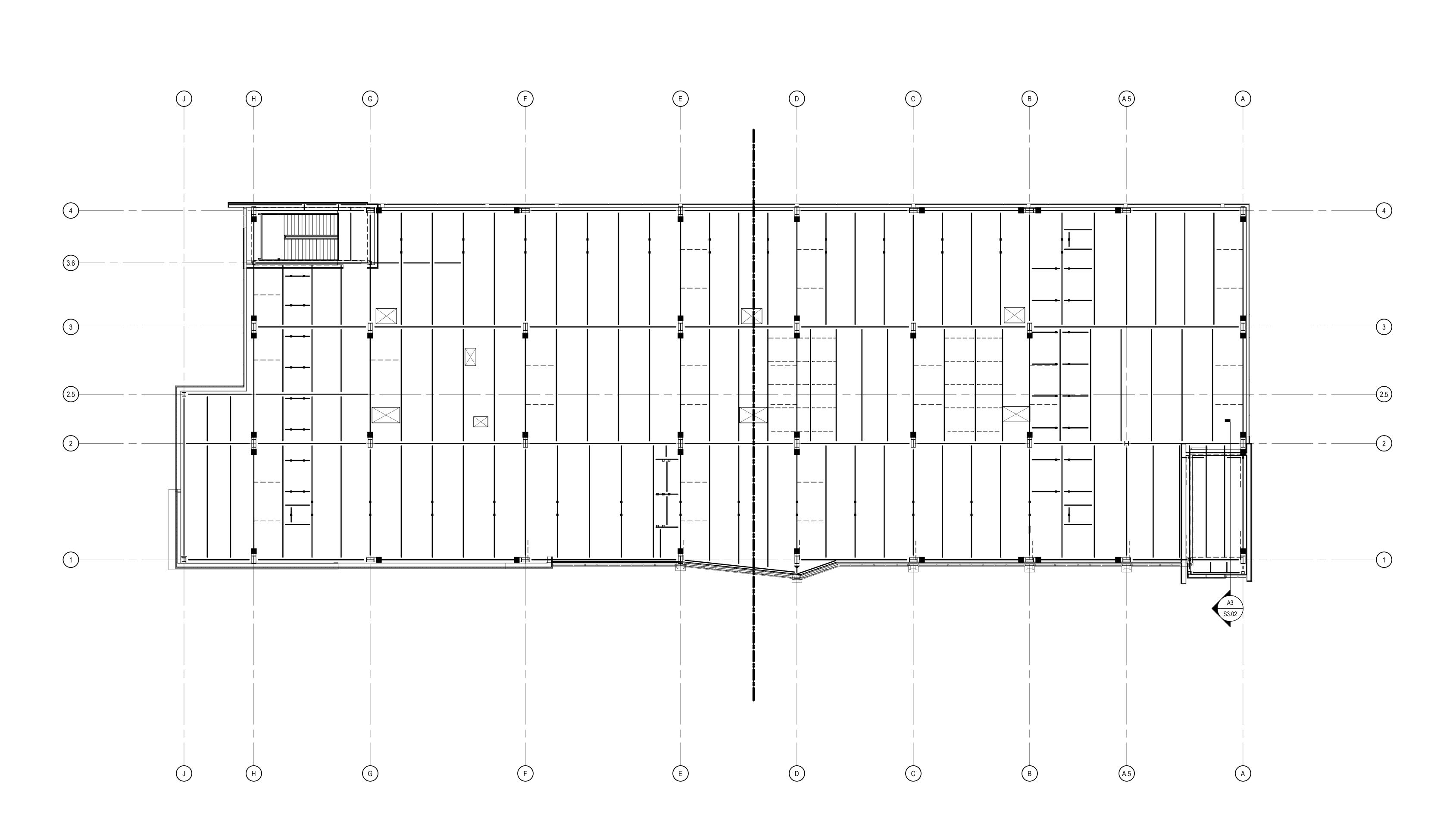
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1	6/17/19	BID PACKAGE 04 ASI 02
2	8/8/19	BID PACKAGE 04 ASI 05

DATE: JOB NUMBER: 17-13

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S1.32

THIRD FLOOR FRAMING PLAN - SECTOR 2

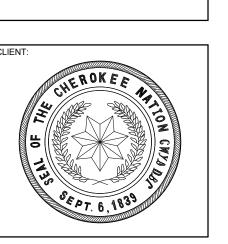


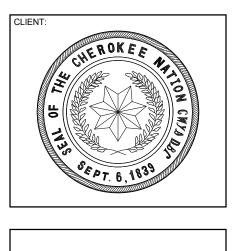


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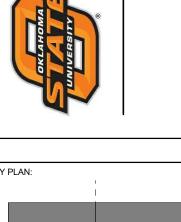


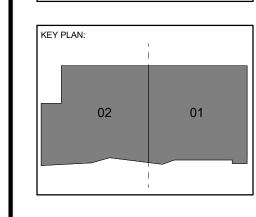












	PROJECT PHASE:
	BID PACKAGE 04
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DATE:	JOB NUMBER:
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- . SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- 2. NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE CONCRETE WALLS AND MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
- DIMENSIONS ARE TO THE FACE OF CONCRETE OR STUDS UNLESS NOTED OTHERWISE.
- SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
- 5. BEAMS ARE SPACED EQUALLY BETWEEN GRIDS UNLESS
- NOTED OTHERWISE.

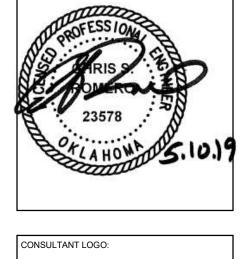
 STRUCTURAL COLD FORMED METAL STUDS SHALL BE 8" WIDE

UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING

- . SEE S7.00 SERIES SHEETS FOR TYPICAL DETAILS.
- 8. SEE SHEET S6.01 FOR SCHEDULES.

BY OTHERS.

- DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
- 10. DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE DRAWINGS



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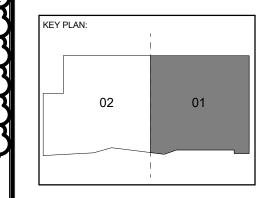




- . MECHANICAL UNIT, COORDINATE EXACT SIZE AND LOCATION WITH MECHANICAL DRAWINGS.
- MECHANICAL OPENING. COORDINATE EXACT SIZE AND LOCATION WITH MECHANICAL DRAWINGS. SEE B2 / S7.41 AND C2 / S7.41 FOR TYPICAL FRAMING.
- 3. HSS6x4x3/16 EXTERIOR CLADDING SUPPORT.
- 4. OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A4 / S5.62, B4 / S5.62, C4 / S5.62, AND D4 / S5.62 FOR SUPPORT.
- HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE A4 / S5.62, B4 / S5.62, C4 / S5.62, AND D4 / S5.62
- 6. HSS4x4x1/4 WEB VERTICAL BRACE, SEE C3 / S5.53
- 7. W12x26.
- 8. BOTTOM FLANGE BRACING AT EQUAL SPACING, UNLESS NOTED OTHERWISE ON PLAN. SEE B1 / S5.52
- 9. BOTTOM FLANGE BRACING AT EACH BEAM, UNLESS NOTED OTHERWISE ON PLAN A1 / S5.52
- 10. HSS4x4x1/4 FLANGE VERTICAL BRACE, SEE D3 / S5.53
- 11. HSS4x4x1/4 BRACE BELOW.
- 12. SEE B5 / S5.52 FOR BENT DBL HSS GIRT DETAIL
- 13. HSS3x3x1/4 OUTRIGGER. SEE A4 / S5.5214. CONTRACTOR TO COORDINATE EXACT EXTENT OF SECTION
- WITH ARCHITECTURAL ELEVATIONS.
- 15. HSS VERTICAL BRACE, SEE A2 / S5.53
- 16. COVER PLATE BEAM AT EXTENTS SHOWN. DIMENSIONS GIVEN ARE TO FACE/EDGE OF FLANGE. SEE D5 / S3.23 FOR FRAMING DETAIL.
- 17. OUTRIGGER BEAM (STUB). SEE D5 / S3.23
- 18. W8x31 ELEVATOR HOIST BEAM. BEAM SIZE CONTINGENT UPON
- DESIGN BASIS ELEVATOR. CONTRACTOR TO COORDINATE.

 19. HSS4x4x1/4 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE C4 / S5.62

 AND D4 / S5.62
- 20. W14x22 OUTRIGGER BEAM WITH L4x4x1/4 KNEE BRACE ATTACHMENT BELOW. SEE A2 / S5.54
- 21. HSS8x4x1/4 SPANNING PERPENDICULAR TO FRAMING BELOW.
- 22. RIPPED WT9x17.5 TO LEVEL STAIR LANDING AND MATCH SLOPE OF ROOF.
- 23. STRUCTURAL MEMBER SUPPORTING EXIT STAIRS. SEE LIFE SAFETY DRAWINGS FOR FIREPROOFING REQUIREMENTS.
- 24. WT4x33.5 CENTERED ON BOTTOM FLANGE OF NOTED BEAM. EXTENT OF WT: 7'-6" FROM GRID 3 AND STOPS 5'-0" FROM GRID 4. SEE A4 / S3.23 FOR ADDITIONAL INFORMATION.



PROJECT PHASE:					
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DATE: JOB NUMBER:

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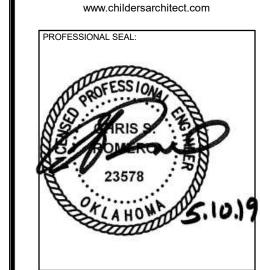
SECTOR 1

ROOF FRAMING PLAN

ROOF FRAMING PLAN - SECTOR 1

SCALE: 1/8" = 1'-0"

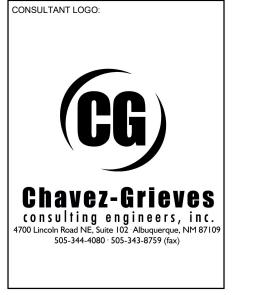
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- 8. SEE SHEET S6.01 FOR SCHEDULES.
- DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
-). DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE

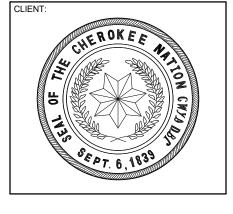


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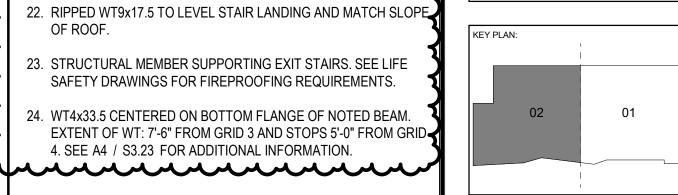
SHEET KEYNOTE

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- MECHANICAL OPENING. COORDINATE EXACT SIZE AND LOCATION WITH MECHANICAL DRAWINGS. SEE B2 / S7.41 AND C2 / S7.41 FOR TYPICAL FRAMING.
- HSS6x4x3/16 EXTERIOR CLADDING SUPPORT.
- . OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A4 / S5.62, B4 / S5.62 , C4 / S5.62 , AND D4 / S5.62 FOR SUPPORT.
- HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE A4 / S5.62, B4 / S5.62, C4 / S5.62, AND D4 / S5.62
- 6. HSS4x4x1/4 WEB VERTICAL BRACE, SEE C3 / S5.53
- . W12x26.
- BOTTOM FLANGE BRACING AT EQUAL SPACING, UNLESS NOTED OTHERWISE ON PLAN. SEE B1 / S5.52
- BOTTOM FLANGE BRACING AT EACH BEAM, UNLESS NOTED OTHERWISE ON PLAN A1 / S5.52
- 10. HSS4x4x1/4 FLANGE VERTICAL BRACE, SEE D3 / S5.53
- 11. HSS4x4x1/4 BRACE BELOW.
- 12. SEE B5 / S5.52 FOR BENT DBL HSS GIRT DETAIL 13. HSS3x3x1/4 OUTRIGGER. SEE A4 / S5.52
- 14. CONTRACTOR TO COORDINATE EXACT EXTENT OF SECTION WITH ARCHITECTURAL ELEVATIONS.
- 15. HSS VERTICAL BRACE, SEE A2 / S5.53
- 16. COVER PLATE BEAM AT EXTENTS SHOWN. DIMENSIONS GIVEN ARE TO FACE/EDGE OF FLANGE. SEE D5 / S3.23 FOR FRAMING DETAIL.

18. W8x31 ELEVATOR HOIST BEAM. BEAM SIZE CONTINGENT UPON

- 17. OUTRIGGER BEAM (STUB). SEE D5 / S3.23
- 19. HSS4x4x1/4 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE C4 / S5.62 AND D4 / S5.62
- 20. W14x22 OUTRIGGER BEAM WITH L4x4x1/4 KNEE BRACE ATTACHMENT BELOW. SEE A2 / S5.54
- 21. HSS8x4x1/4 SPANNING PERPENDICULAR TO FRAMING BELOW.
- 22. RIPPED WT9x17.5 TO LEVEL STAIR LANDING AND MATCH SLOPE OF ROOF.
- 23. STRUCTURAL MEMBER SUPPORTING EXIT STAIRS. SEE LIFE
- SAFETY DRAWINGS FOR FIREPROOFING REQUIREMENTS. 24. WT4x33.5 CENTERED ON BOTTOM FLANGE OF NOTED BEAM. EXTENT OF WT: 7'-6" FROM GRID 3 AND STOPS 5'-0" FROM GRID

4. SEE A4 / S3.23 FOR ADDITIONAL INFORMATION.



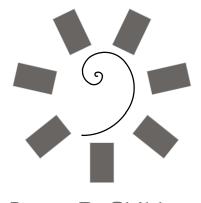
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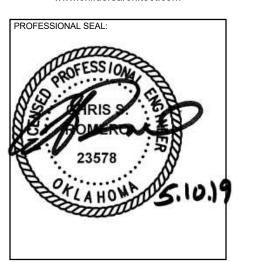
ROOF FRAMING PLAN SECTOR 2

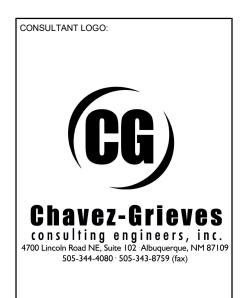
ROOF FRAMING PLAN - SECTOR 2

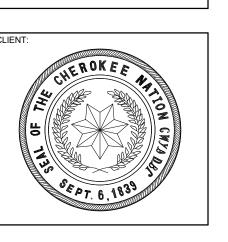


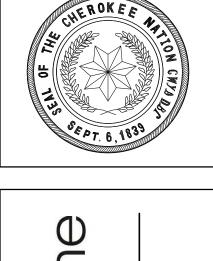
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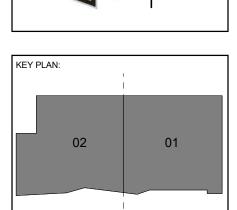


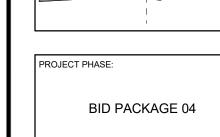




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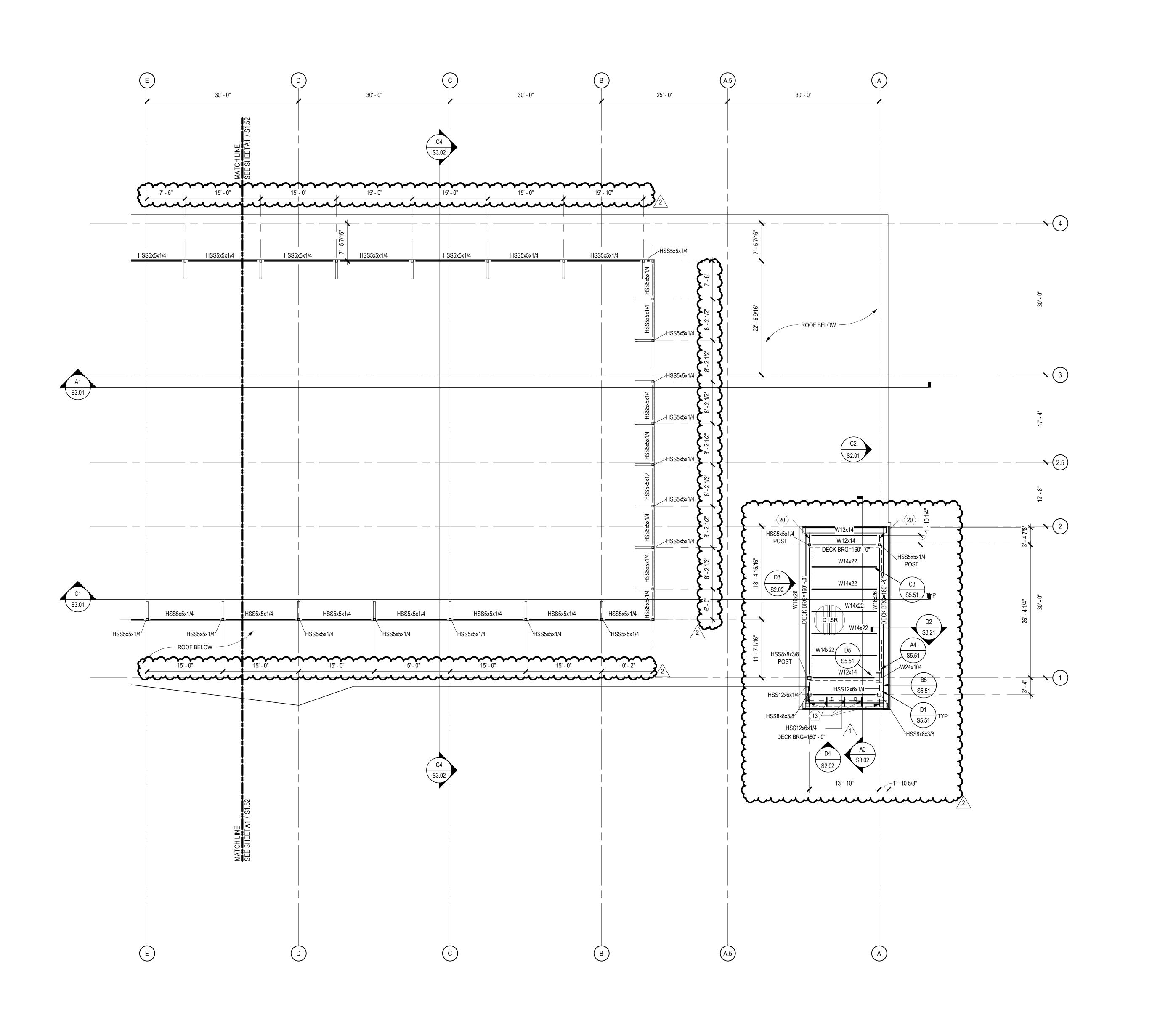
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HIGH ROOF FRAMING PLAN - OVERALL PLAN

HIGH ROOF FRAMING PLAN - OVERALL PLAN

SCALE: 3/32" = 1'-0"

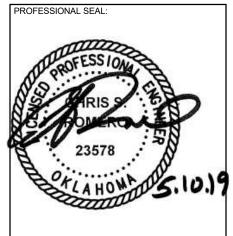


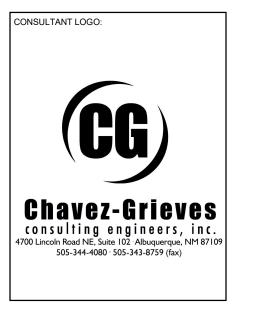
- SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- 2. NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE CONCRETE WALLS AND MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
- 3. DIMENSIONS ARE TO THE FACE OF CONCRETE OR STUDS UNLESS NOTED OTHERWISE.
- 4. SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
- 5. BEAMS ARE SPACED EQUALLY BETWEEN GRIDS UNLESS
- STRUCTURAL COLD FORMED METAL STUDS SHALL BE 8" WIDE UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING
- 7. SEE S7.00 SERIES SHEETS FOR TYPICAL DETAILS.
- 8. SEE SHEET S6.01 FOR SCHEDULES.

NOTED OTHERWISE.

- DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
- 10. DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE DRAWINGS.









- 1. MECHANICAL UNIT, COORDINATE EXACT SIZE AND LOCATION WITH MECHANICAL DRAWINGS.
- 2. MECHANICAL OPENING. COORDINATE EXACT SIZE AND LOCATION WITH MECHANICAL DRAWINGS. SEE B2 / S7.41 AND C2 / S7.41 FOR TYPICAL FRAMING.
- 3. HSS6x4x3/16 EXTERIOR CLADDING SUPPORT
- 4. OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A4 / S5.62, B4 / S5.62, C4 / S5.62, AND D4 / S5.62 FOR SUPPORT.
- HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE A4 / S5.62, B4 / S5.62, C4 / S5.62, AND D4 / S5.62
- 6. HSS4x4x1/4 WEB VERTICAL BRACE, SEE C3 / S5.53
- 7. W12x26.
- 8. BOTTOM FLANGE BRACING AT EQUAL SPACING, UNLESS NOTED OTHERWISE ON PLAN. SEE B1 / S5.52
- 9. BOTTOM FLANGE BRACING AT EACH BEAM, UNLESS NOTED OTHERWISE ON PLAN A1 / S5.52
- 10. HSS4x4x1/4 FLANGE VERTICAL BRACE, SEE D3 / S5.53
- 11. HSS4x4x1/4 BRACE BELOW.
- 12. SEE B5 / S5.52 FOR BENT DBL HSS GIRT DETAIL13. HSS3x3x1/4 OUTRIGGER. SEE A4 / S5.52
- 14. CONTRACTOR TO COORDINATE EXACT EXTENT OF SECTION
- 15. HSS VERTICAL BRACE, SEE A2 / S5.53

WITH ARCHITECTURAL ELEVATIONS.

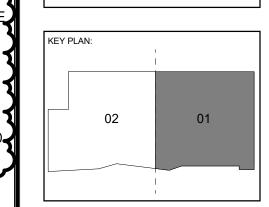
10. HOS VERTICAL BRACE, SEE AZ / 50.00

AND D4 / S5.62

- 16. COVER PLATE BEAM AT EXTENTS SHOWN. DIMENSIONS GIVEN ARE TO FACE/EDGE OF FLANGE. SEE D5 / S3.23 FOR FRAMING DETAIL.
- 17. OUTRIGGER BEAM (STUB). SEE D5 / S3.23
- 18. W8x31 ELEVATOR HOIST BEAM. BEAM SIZE CONTINGENT UPON DESIGN BASIS ELEVATOR. CONTRACTOR TO COORDINATE.
 19. HSS4x4x1/4 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE C4 / S5.62
- 20. W14x22 OUTRIGGER BEAM WITH L4x4x1/4 KNEE BRACE
- ATTACHMENT BELOW. SEE A2 / S5.54

21. HSS8x4x1/4 SPANNING PERPENDICULAR TO FRAMING BELOW.

- 22. RIPPED WT9x17.5 TO LEVEL STAIR LANDING AND MATCH SLOPE OF ROOF.
- 23. STRUCTURAL MEMBER SUPPORTING EXIT STAIRS. SEE LIFE
- SAFETY DRAWINGS FOR FIREPROOFING REQUIREMENTS.
- 24. WT4x33.5 CENTERED ON BOTTOM FLANGE OF NOTED BEAM.
 EXTENT OF WT: 7'-6" FROM GRID 3 AND STOPS 5'-0" FROM GRID
 4. SEE A4 / S3.23 FOR ADDITIONAL INFORMATION.



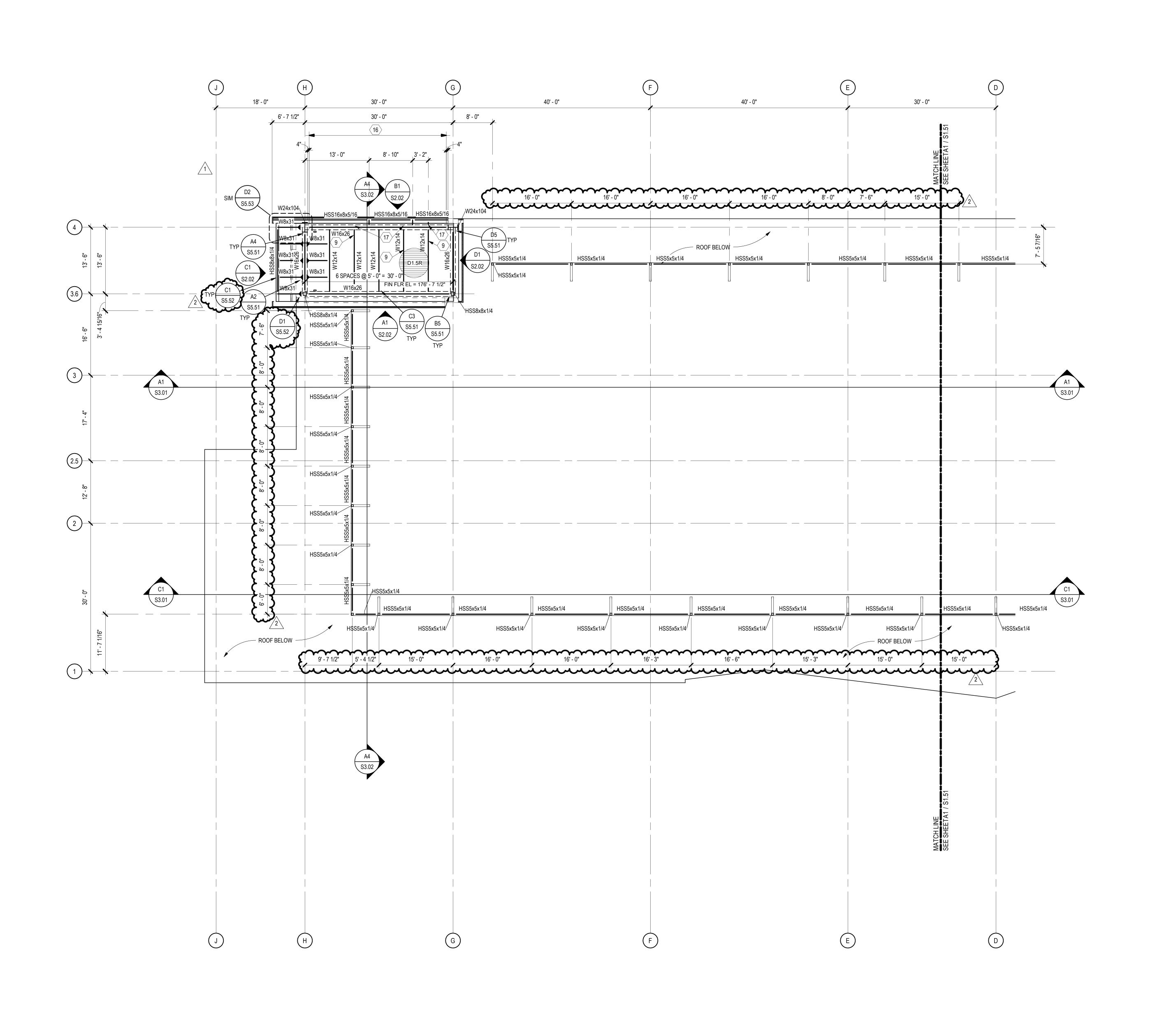
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2	8/8/19	BID PACKAGE 04 ASI 05

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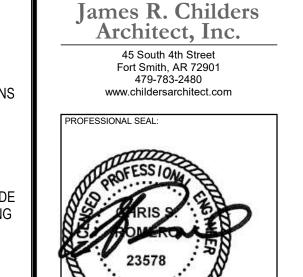
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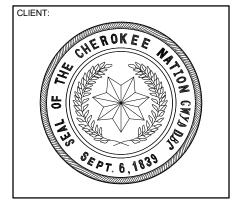
HIGH ROOF FRAMING PLAN - SECTOR 1



- SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE CONCRETE WALLS AND MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
- DIMENSIONS ARE TO THE FACE OF CONCRETE OR STUDS UNLESS NOTED OTHERWISE.
- 4. SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
- BEAMS ARE SPACED EQUALLY BETWEEN GRIDS UNLESS
- NOTED OTHERWISE. 6. STRUCTURAL COLD FORMED METAL STUDS SHALL BE 8" WIDE
- UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING
- 7. SEE S7.00 SERIES SHEETS FOR TYPICAL DETAILS.
- 8. SEE SHEET S6.01 FOR SCHEDULES.
- DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
- DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE DRAWINGS.







SHEET KEYNOTE

- MECHANICAL UNIT, COORDINATE EXACT SIZE AND LOCATION WITH MECHANICAL DRAWINGS.
- MECHANICAL OPENING. COORDINATE EXACT SIZE AND LOCATION WITH MECHANICAL DRAWINGS. SEE B2 / S7.41 AND C2 / S7.41 FOR TYPICAL FRAMING.
- HSS6x4x3/16 EXTERIOR CLADDING SUPPORT.
- 4. OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A4 / S5.62, B4 / S5.62 , C4 / S5.62 , AND D4 / S5.62 FOR SUPPORT.
- HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE A4 / S5.62, B4 / S5.62 , C4 / S5.62 , AND D4 / S5.62
- 6. HSS4x4x1/4 WEB VERTICAL BRACE, SEE C3 / S5.53
- . W12x26.
- B. BOTTOM FLANGE BRACING AT EQUAL SPACING, UNLESS NOTED OTHERWISE ON PLAN. SEE B1 / S5.52
- 9. BOTTOM FLANGE BRACING AT EACH BEAM, UNLESS NOTED OTHERWISE ON PLAN A1 / S5.52
- 10. HSS4x4x1/4 FLANGE VERTICAL BRACE, SEE D3 / S5.53
- 11. HSS4x4x1/4 BRACE BELOW.
- 12. SEE B5 / S5.52 FOR BENT DBL HSS GIRT DETAIL
- 13. HSS3x3x1/4 OUTRIGGER. SEE A4 / S5.52
- 14. CONTRACTOR TO COORDINATE EXACT EXTENT OF SECTION WITH ARCHITECTURAL ELEVATIONS.
- 15. HSS VERTICAL BRACE, SEE A2 / S5.53

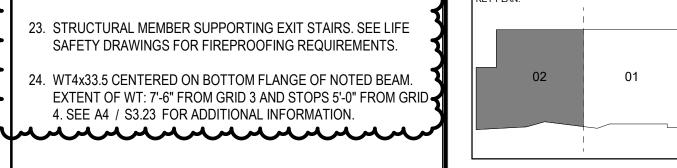
AND D4 / S5.62

- 16. COVER PLATE BEAM AT EXTENTS SHOWN. DIMENSIONS GIVEN ARE TO FACE/EDGE OF FLANGE. SEE D5 / S3.23 FOR FRAMING DETAIL.
- 17. OUTRIGGER BEAM (STUB). SEE D5 / S3.23
- 18. W8x31 ELEVATOR HOIST BEAM. BEAM SIZE CONTINGENT UPON 19. HSS4x4x1/4 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION WITH ELEVATOR MANUFACTURER, SEE C4 / S5.62
- 20. W14x22 OUTRIGGER BEAM WITH L4x4x1/4 KNEE BRACE
- ATTACHMENT BELOW. SEE A2 / S5.54
- 22. RIPPED WT9x17.5 TO LEVEL STAIR LANDING AND MATCH SLOPE OF ROOF.
- 23. STRUCTURAL MEMBER SUPPORTING EXIT STAIRS. SEE LIFE

21. HSS8x4x1/4 SPANNING PERPENDICULAR TO FRAMING BELOW.

SAFETY DRAWINGS FOR FIREPROOFING REQUIREMENTS. 24. WT4x33.5 CENTERED ON BOTTOM FLANGE OF NOTED BEAM. EXTENT OF WT: 7'-6" FROM GRID 3 AND STOPS 5'-0" FROM GRID ◀

4. SEE A4 / S3.23 FOR ADDITIONAL INFORMATION.



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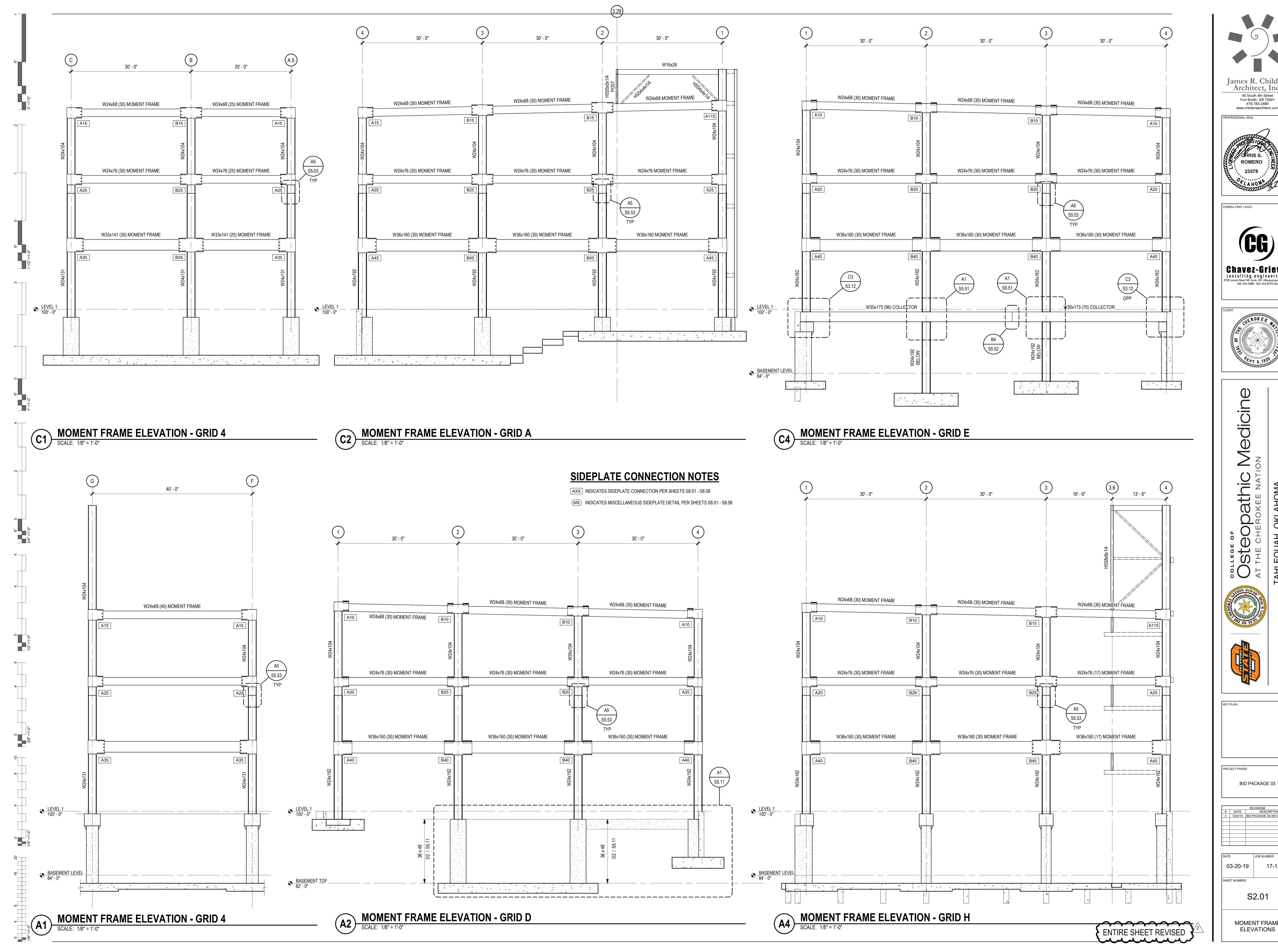
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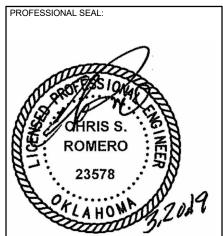
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HIGH ROOF FRAMING PLAN - SECTOR 2

HIGH ROOF FRAMING PLAN - SECTOR 2

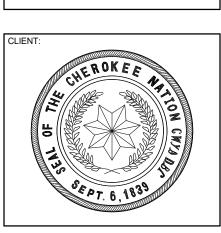


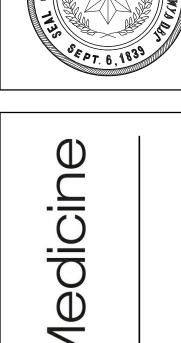
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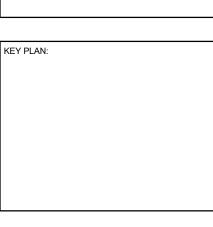








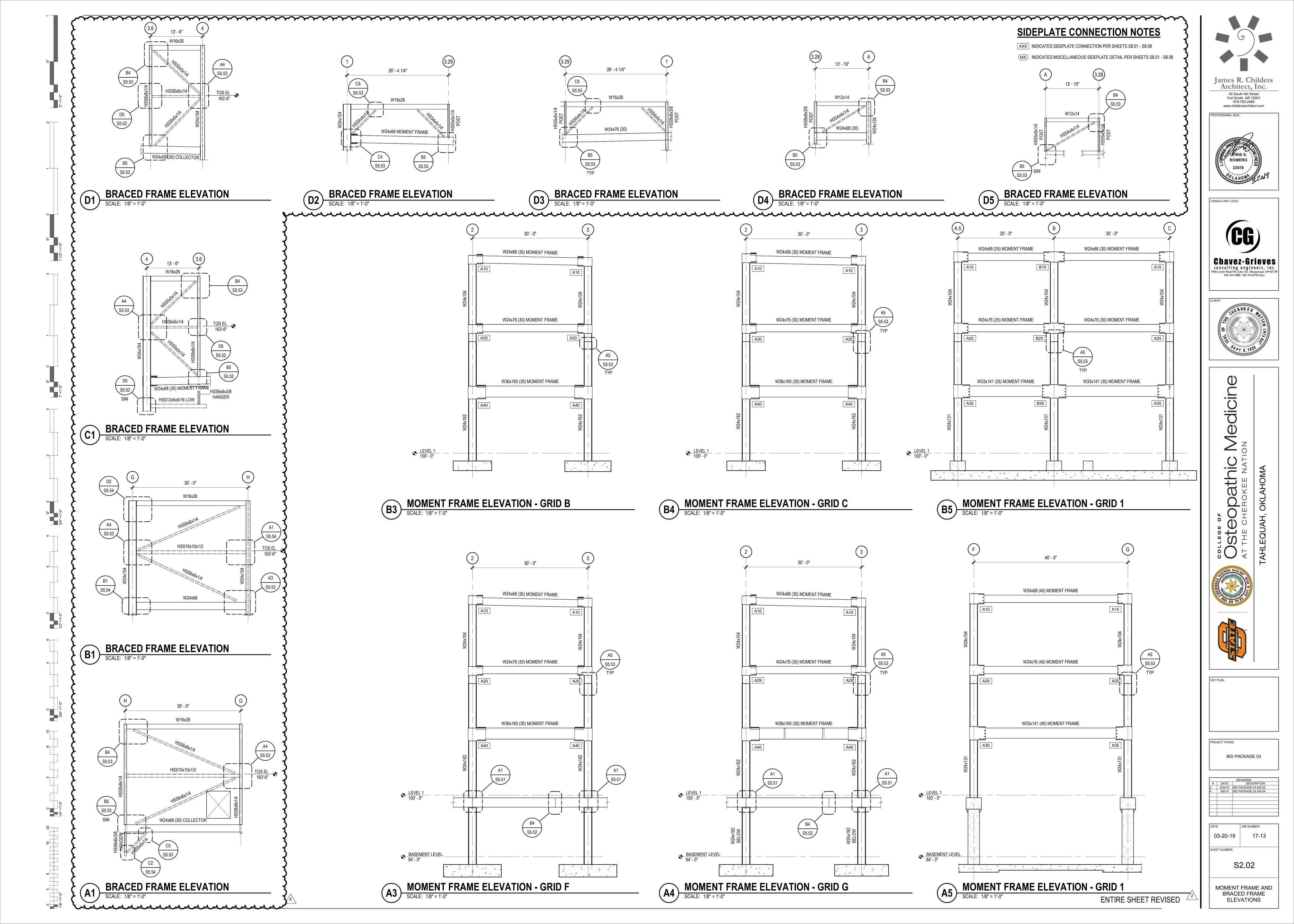


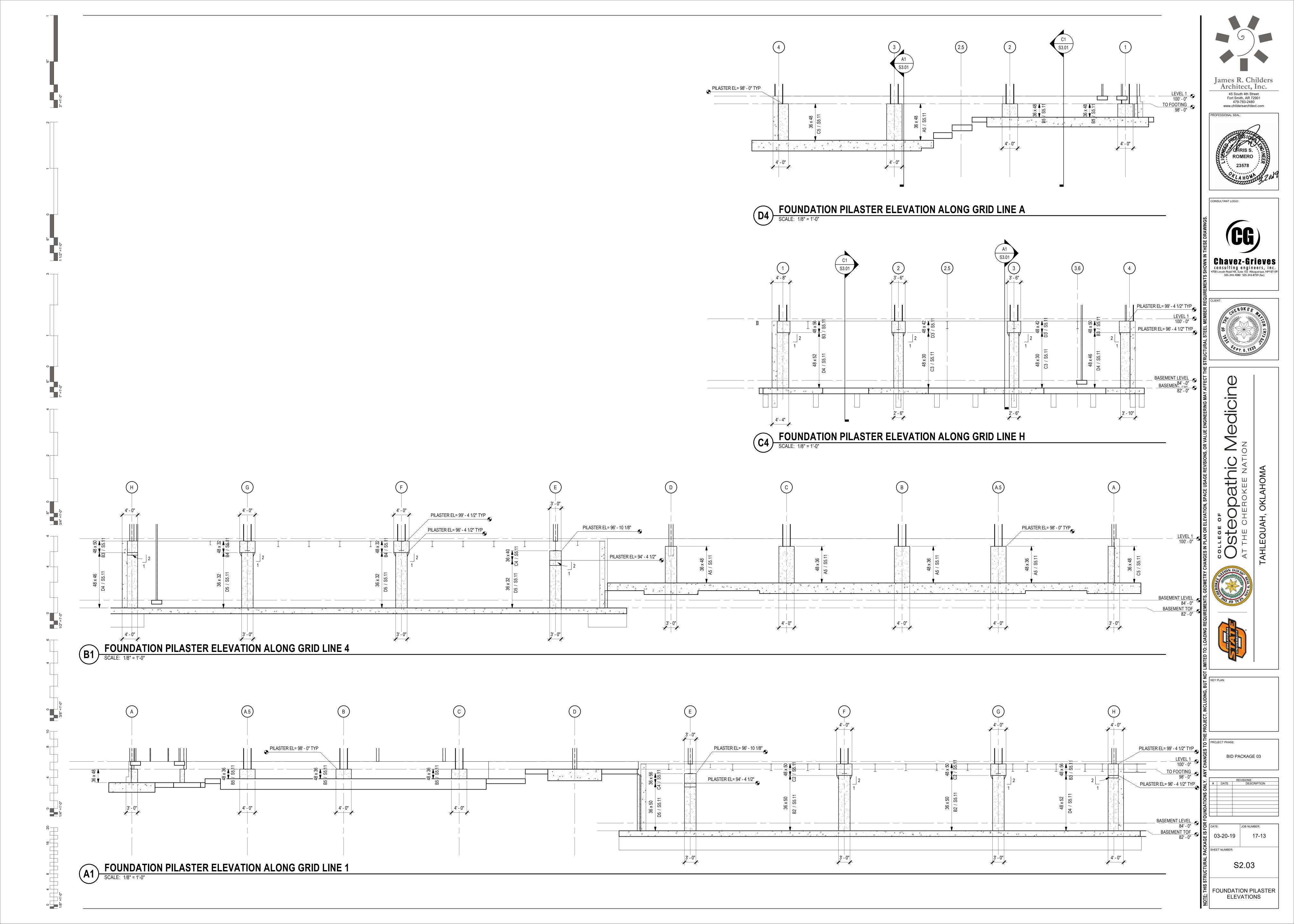


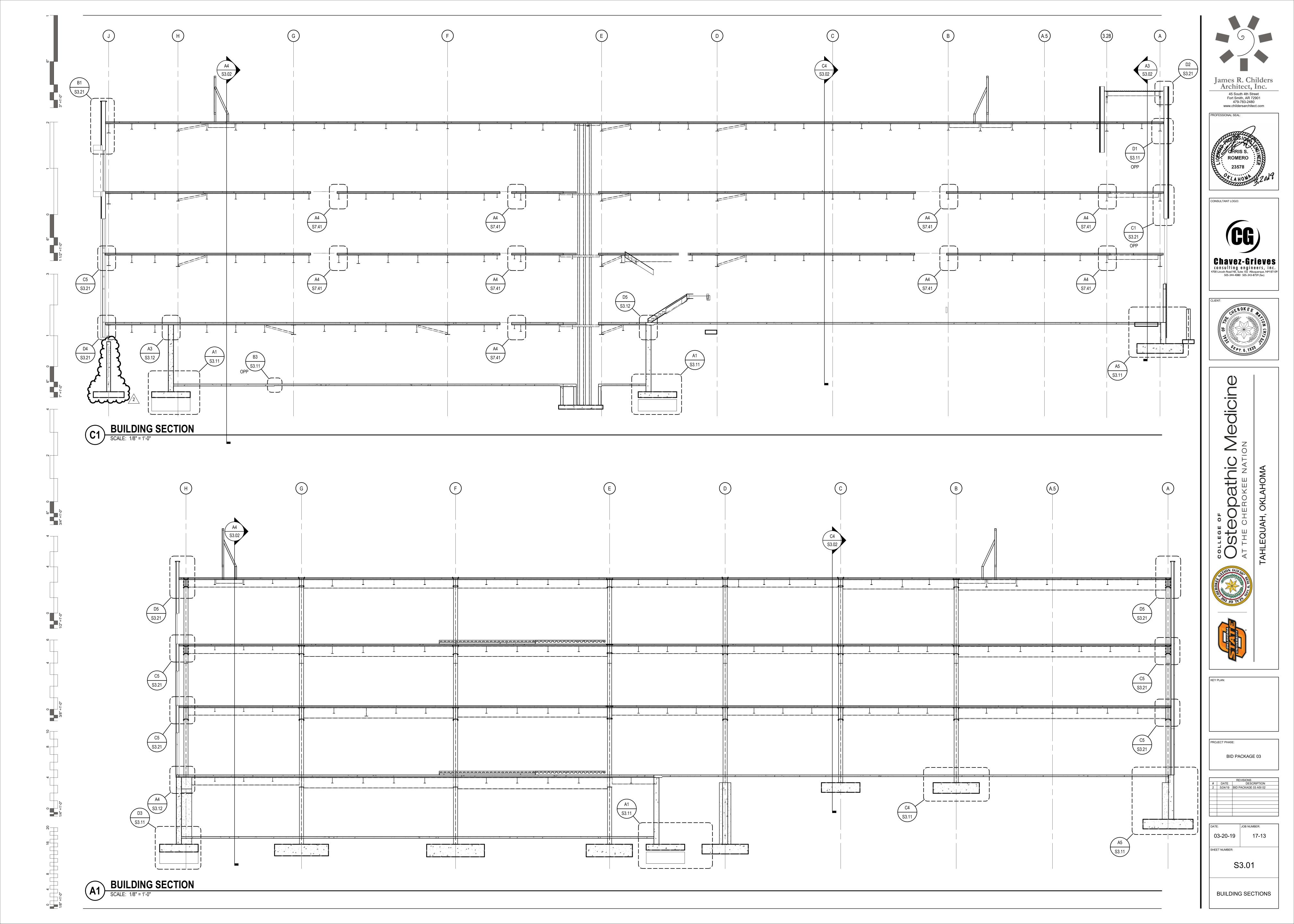
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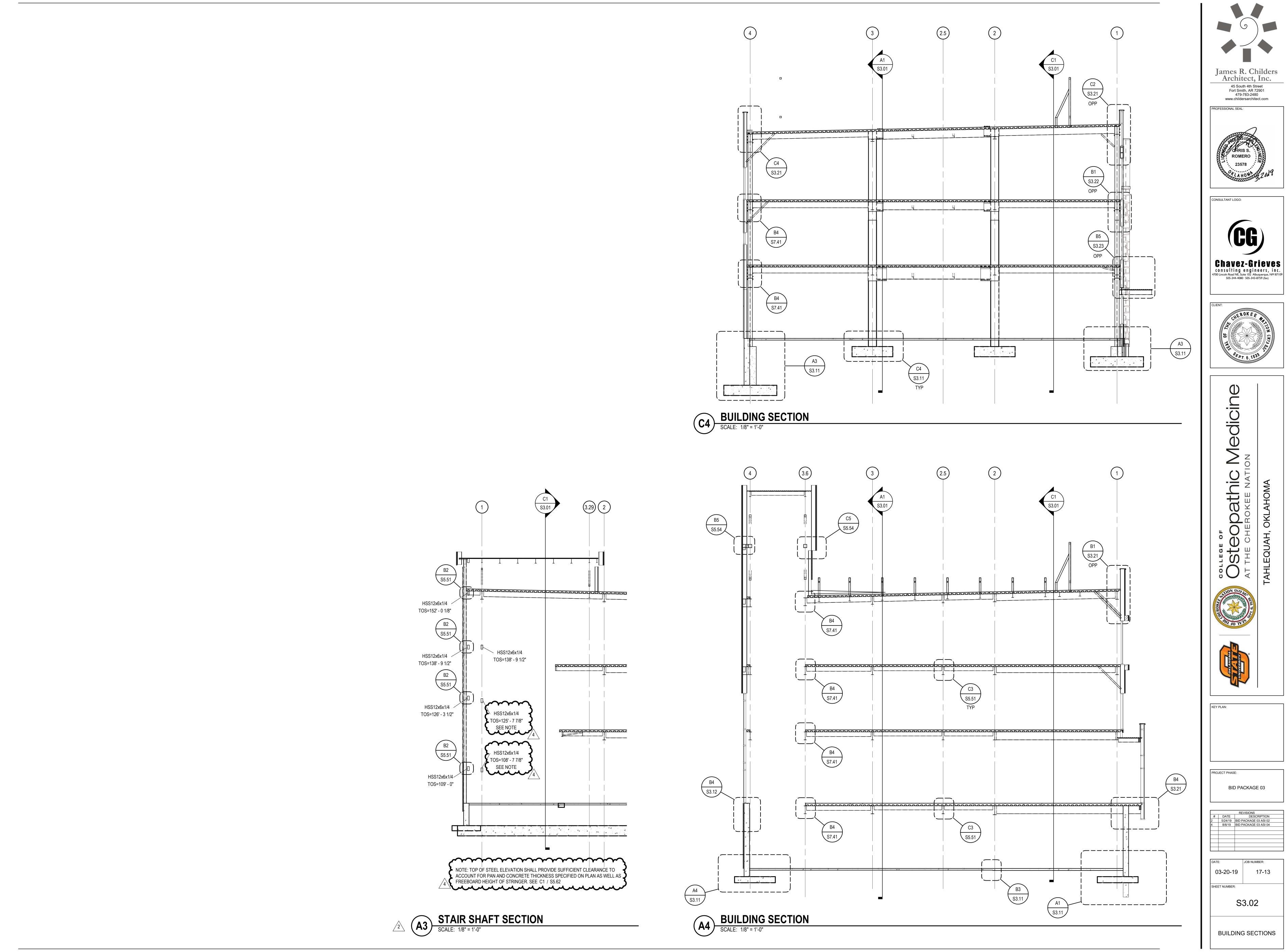
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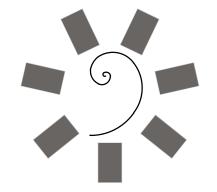
S2.01 MOMENT FRAME











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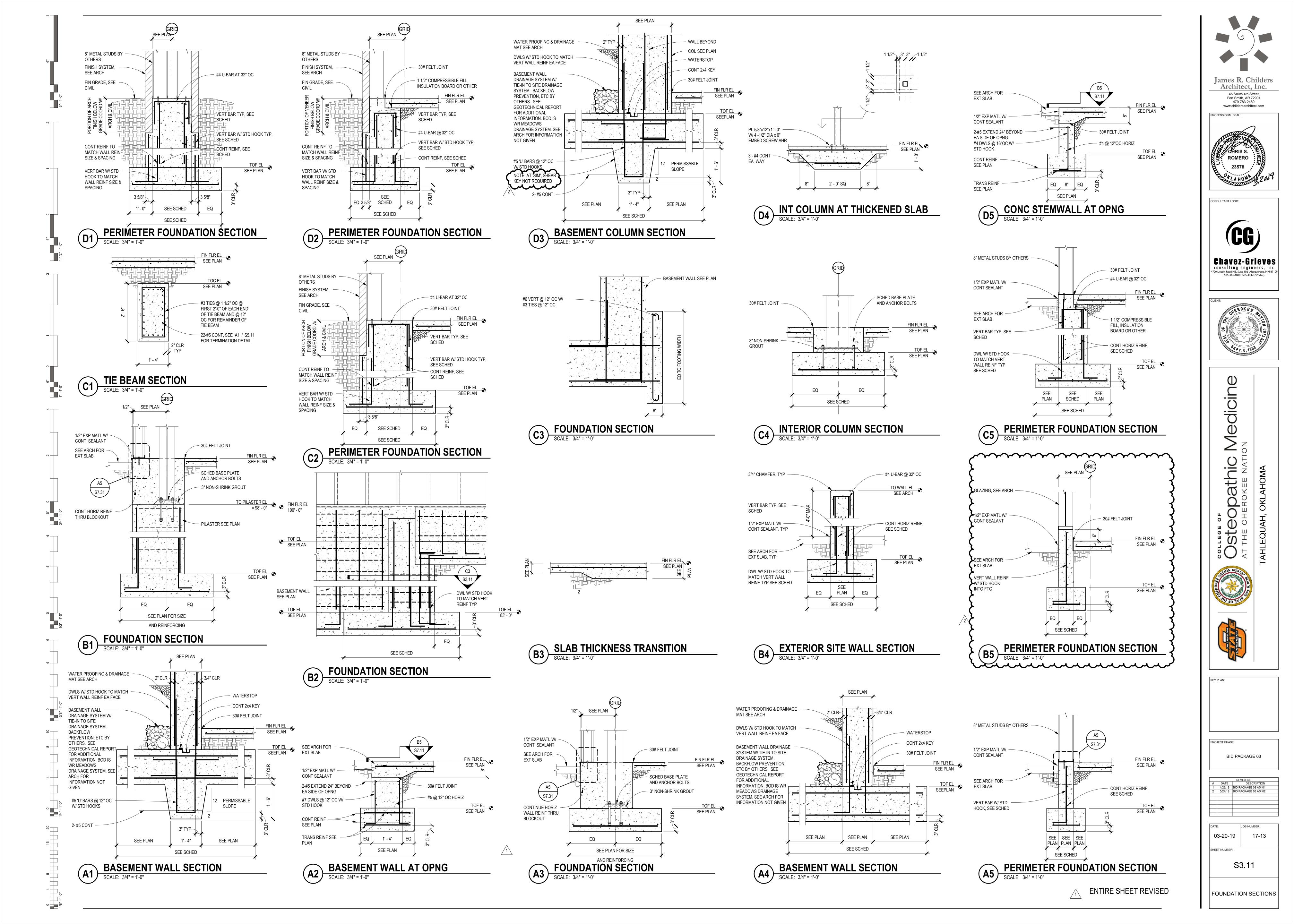
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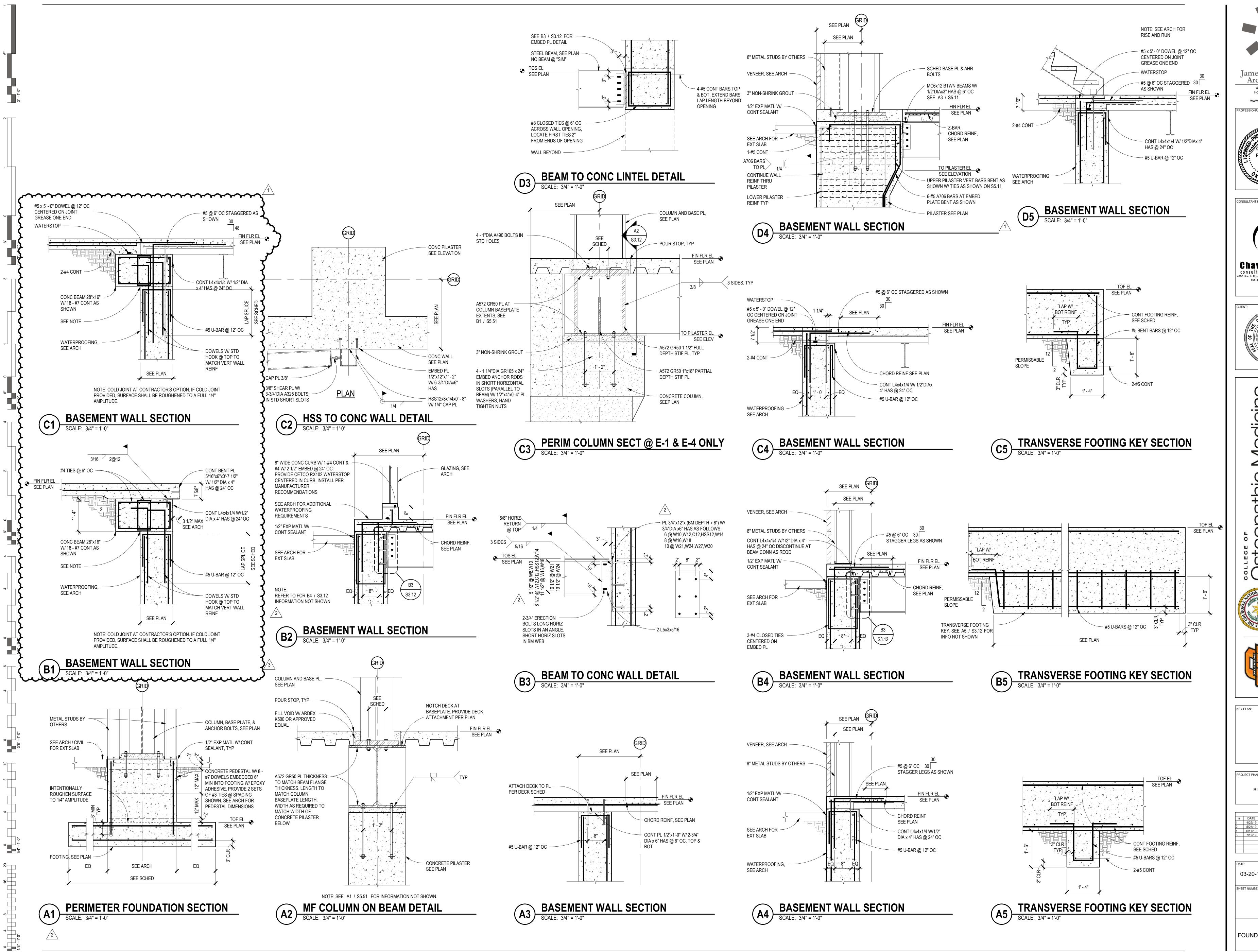
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03-20-19 17-13 SHEET NUMBER:

S3.02

BUILDING SECTIONS

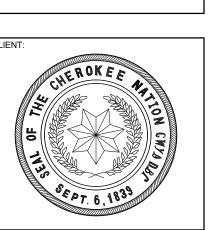


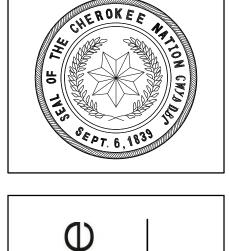


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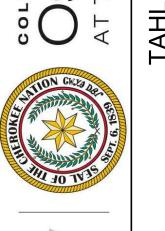




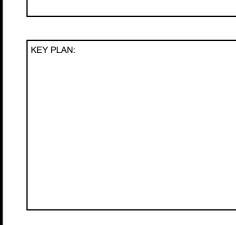


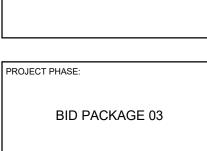


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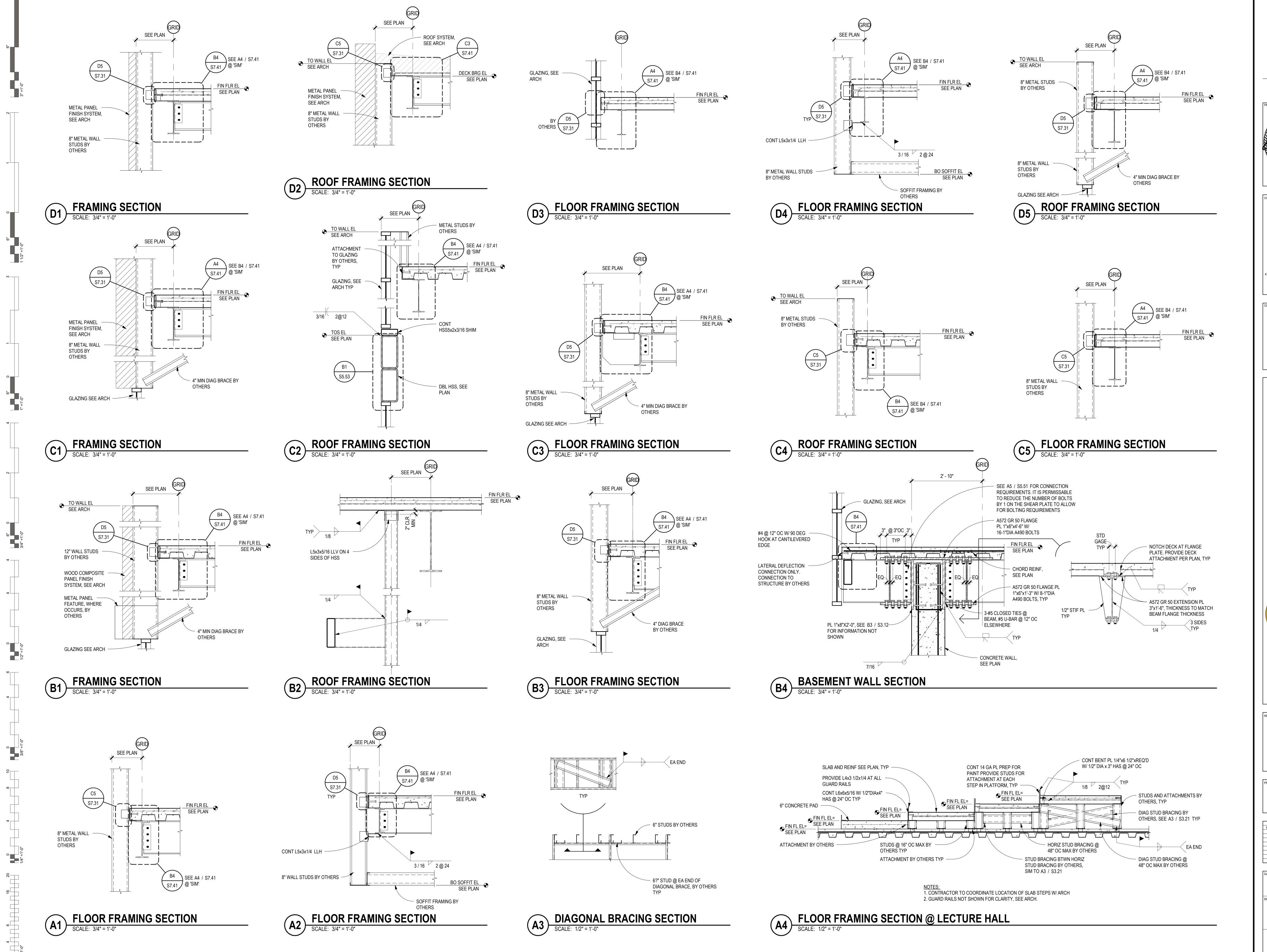


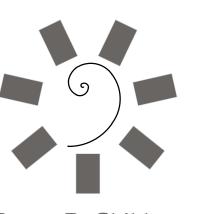


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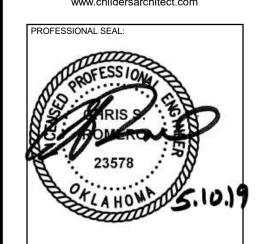
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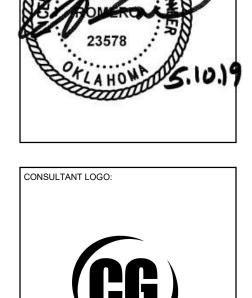




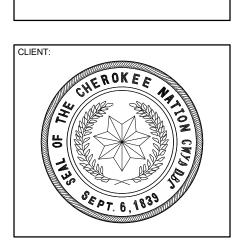
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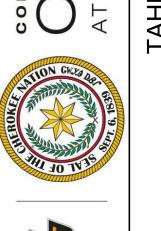


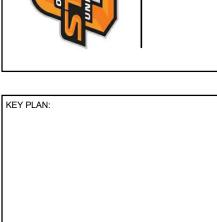


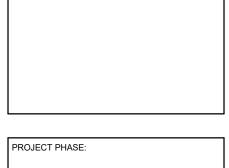


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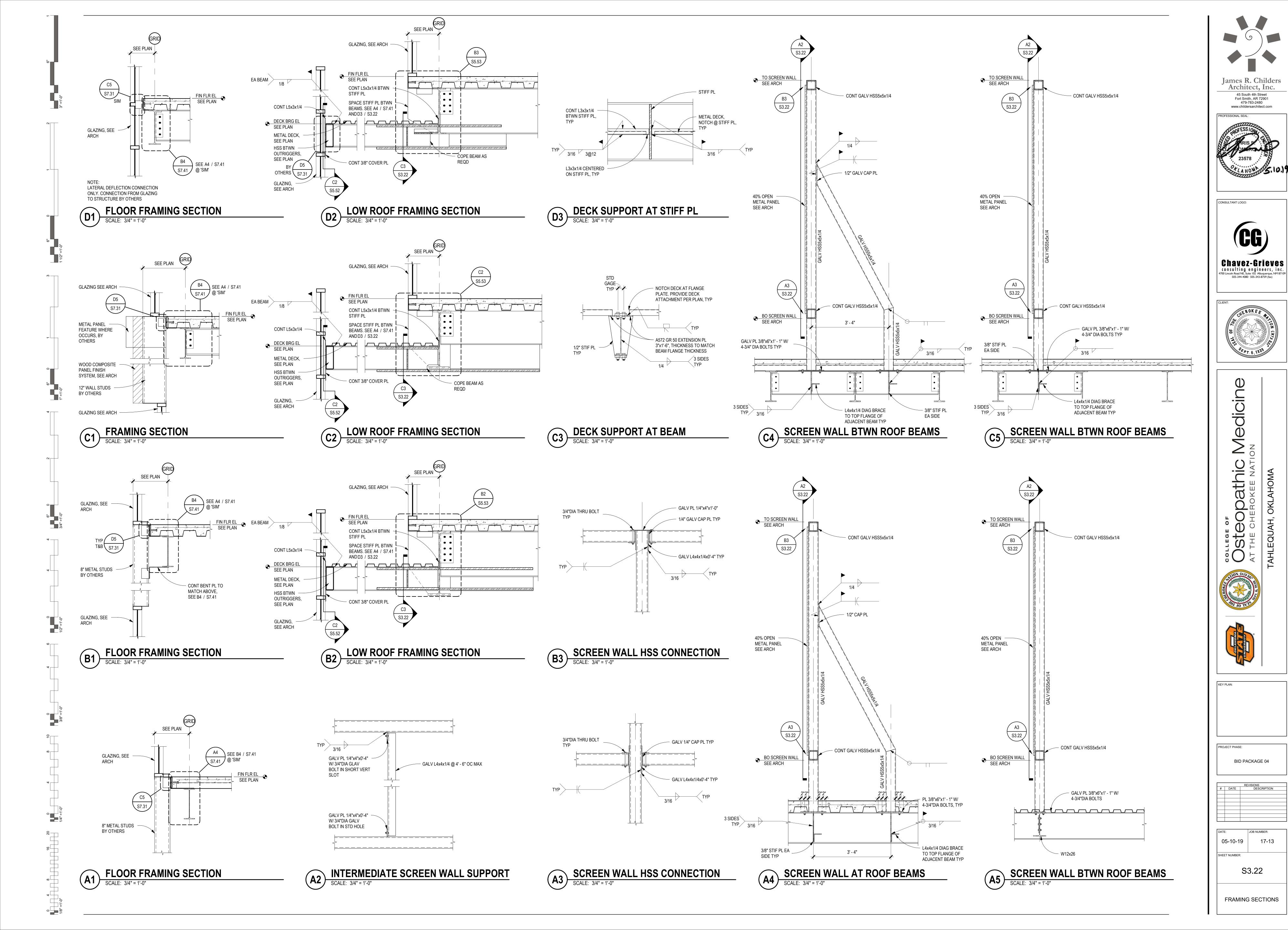
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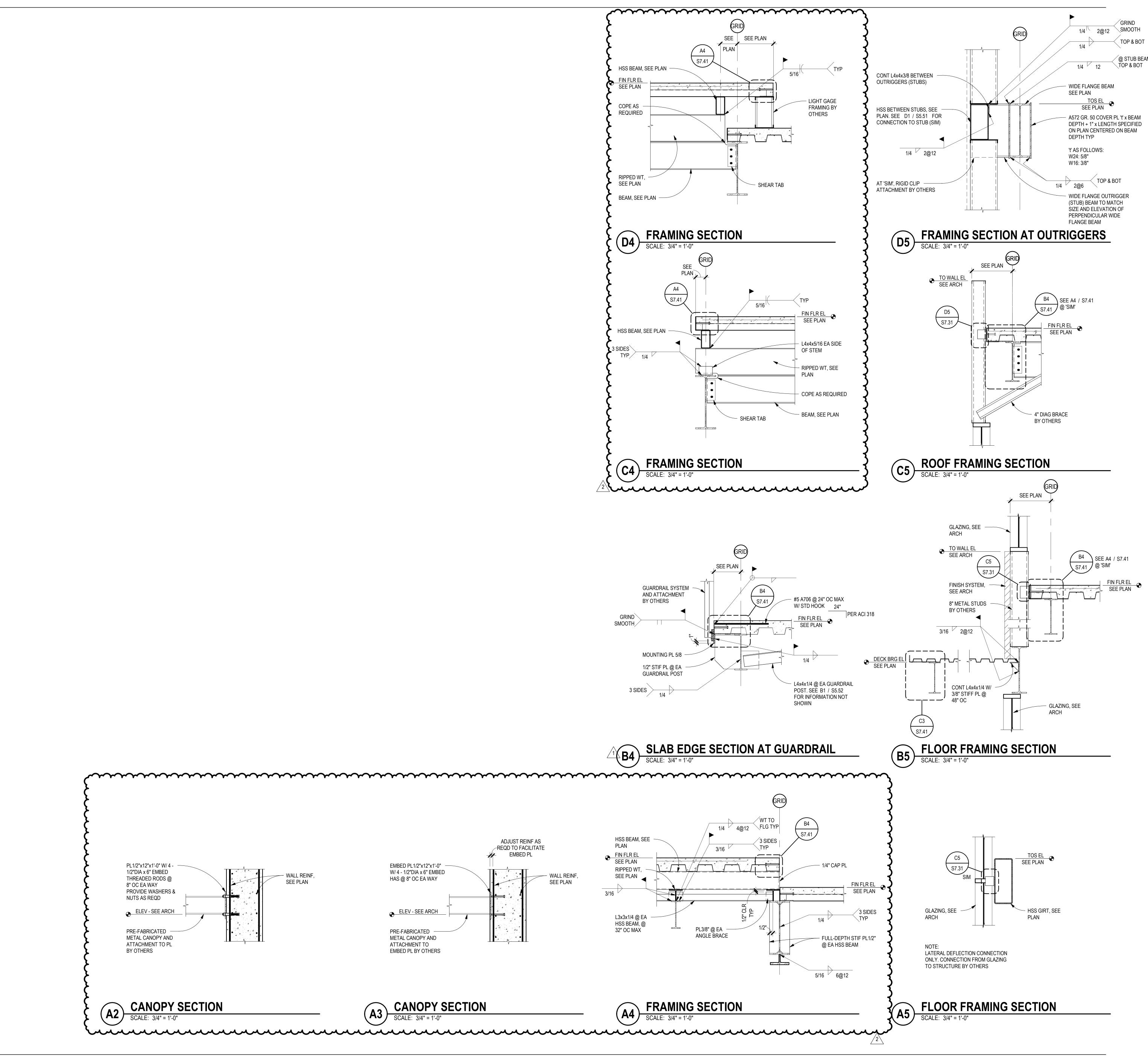
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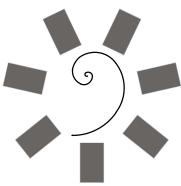
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FRAMING SECTIONS





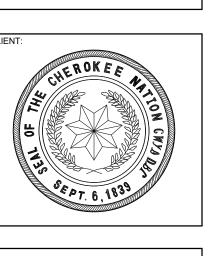


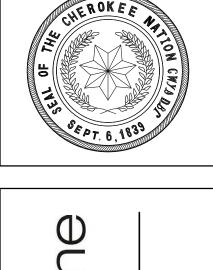
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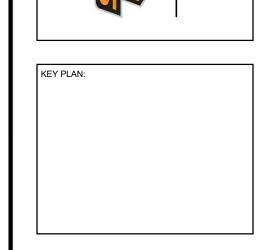


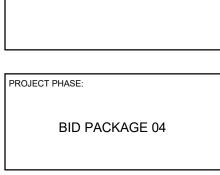




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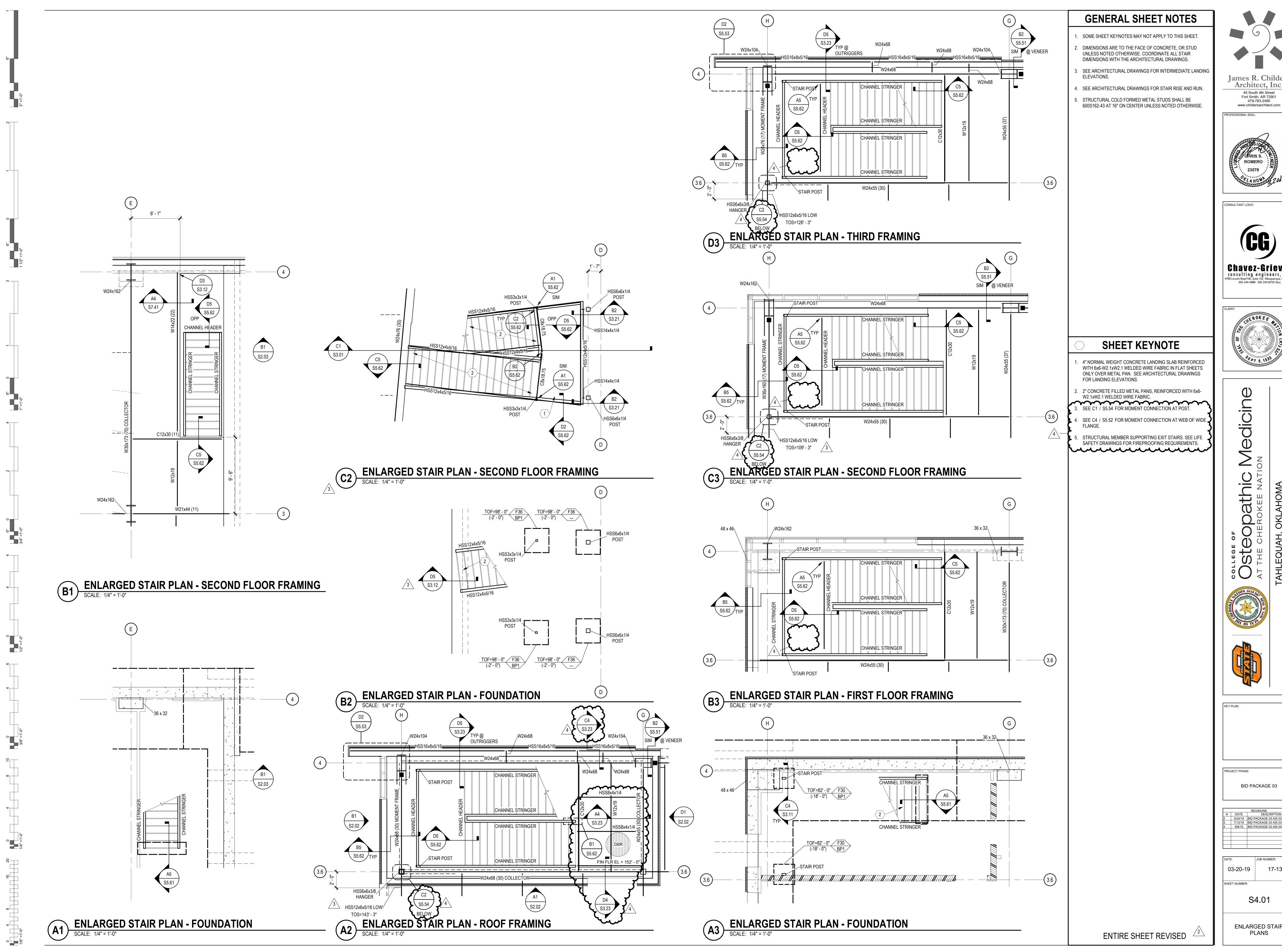




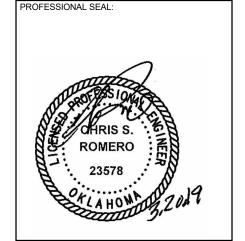
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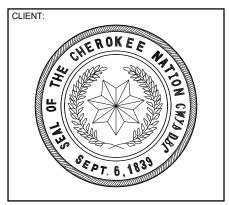
FRAMING SECTIONS



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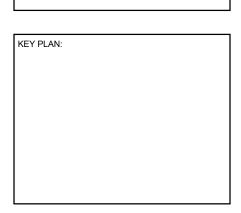










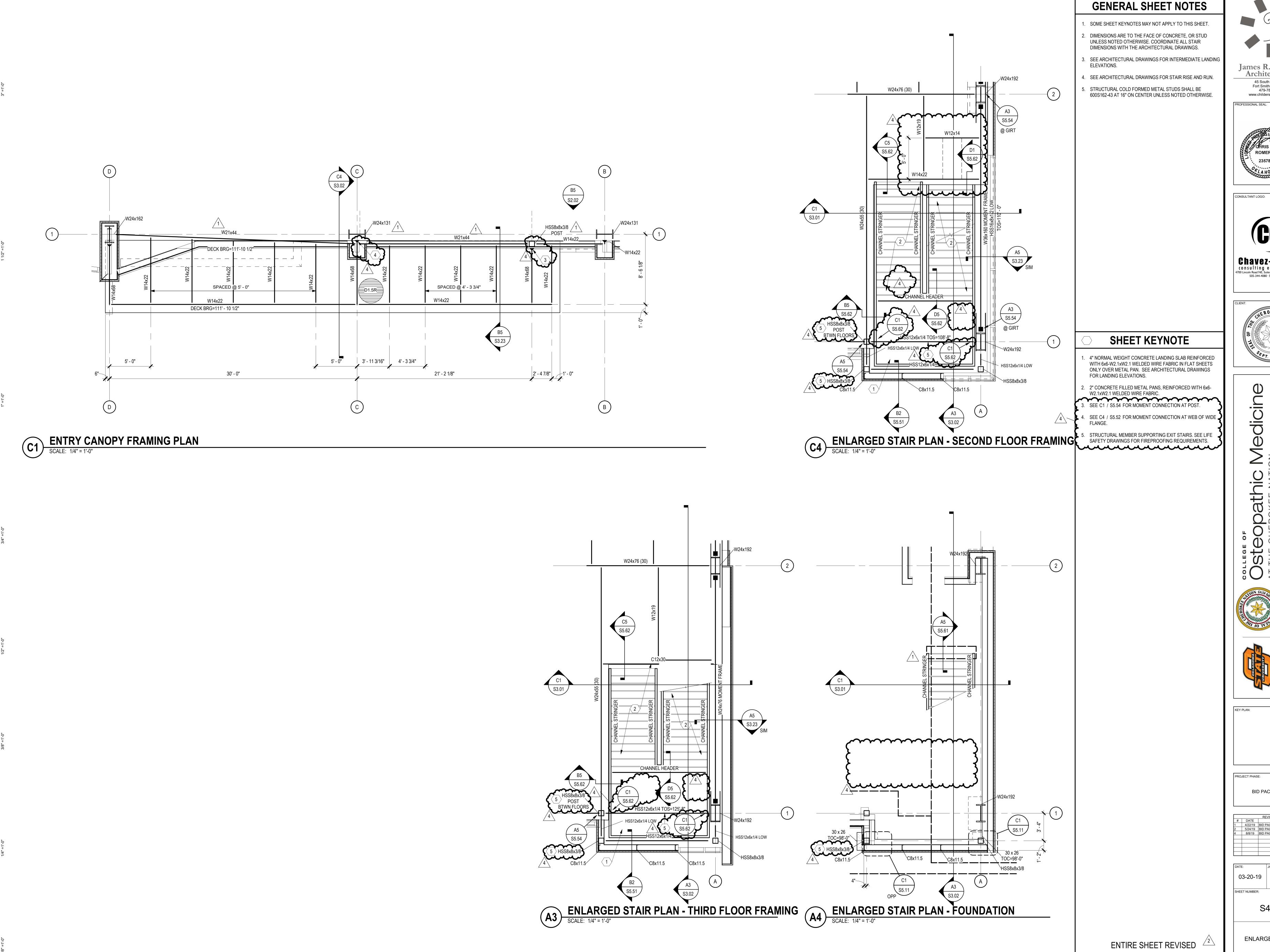


BID PACKAGE 03

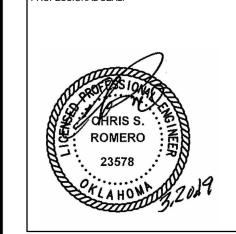
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S4.01

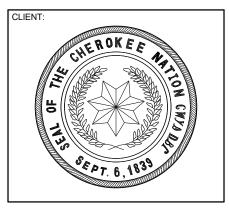
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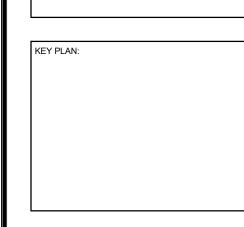












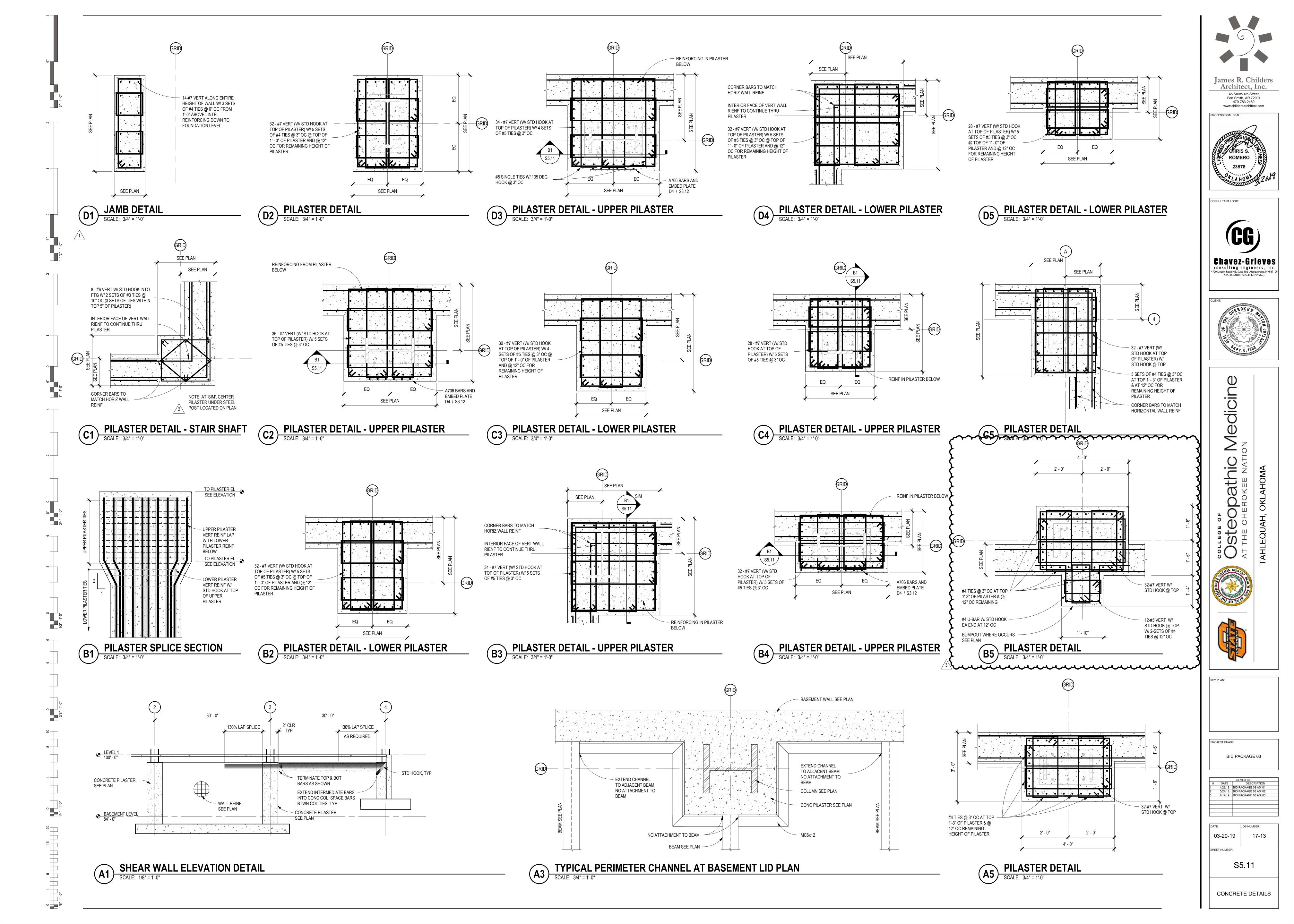
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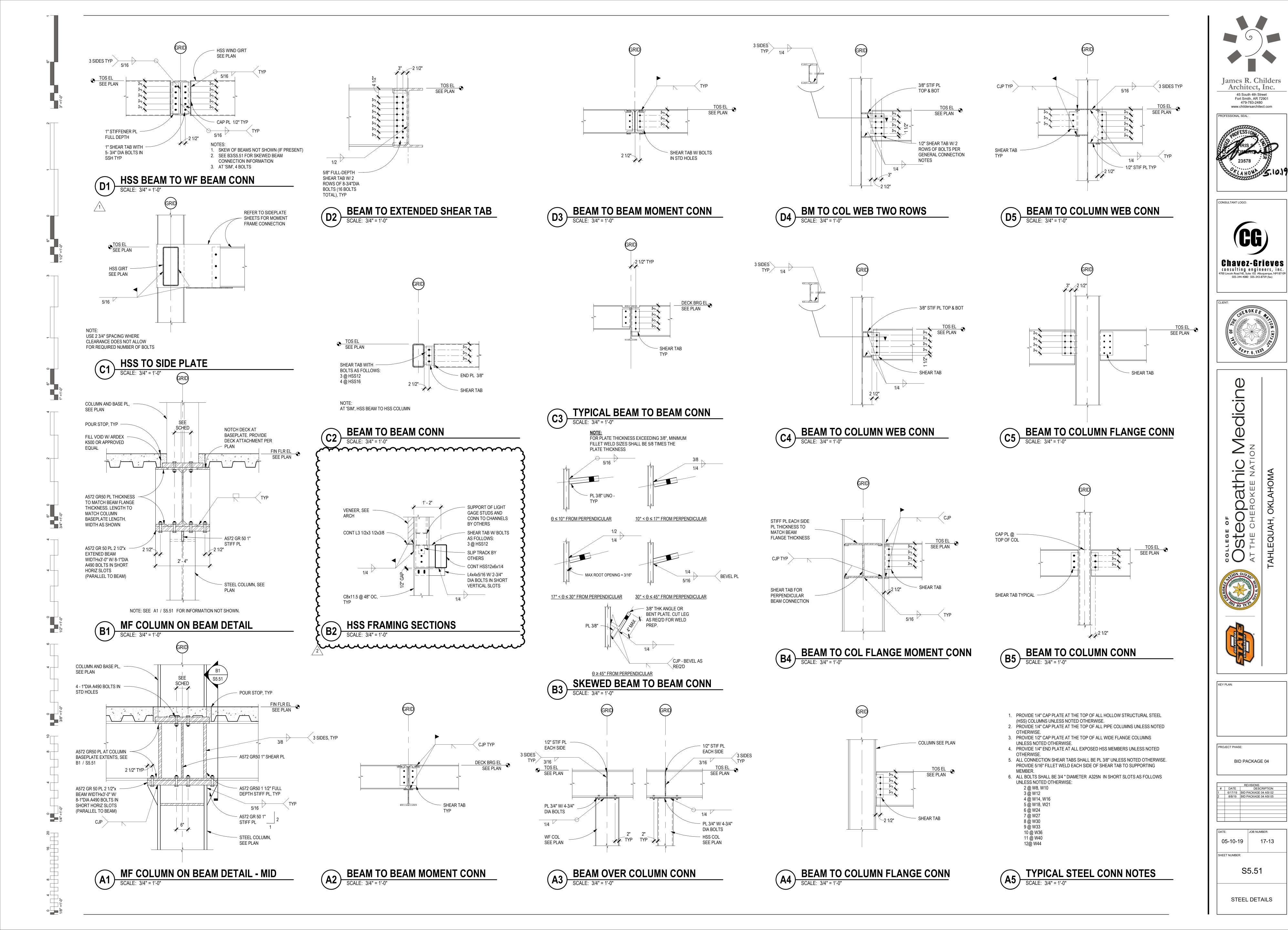
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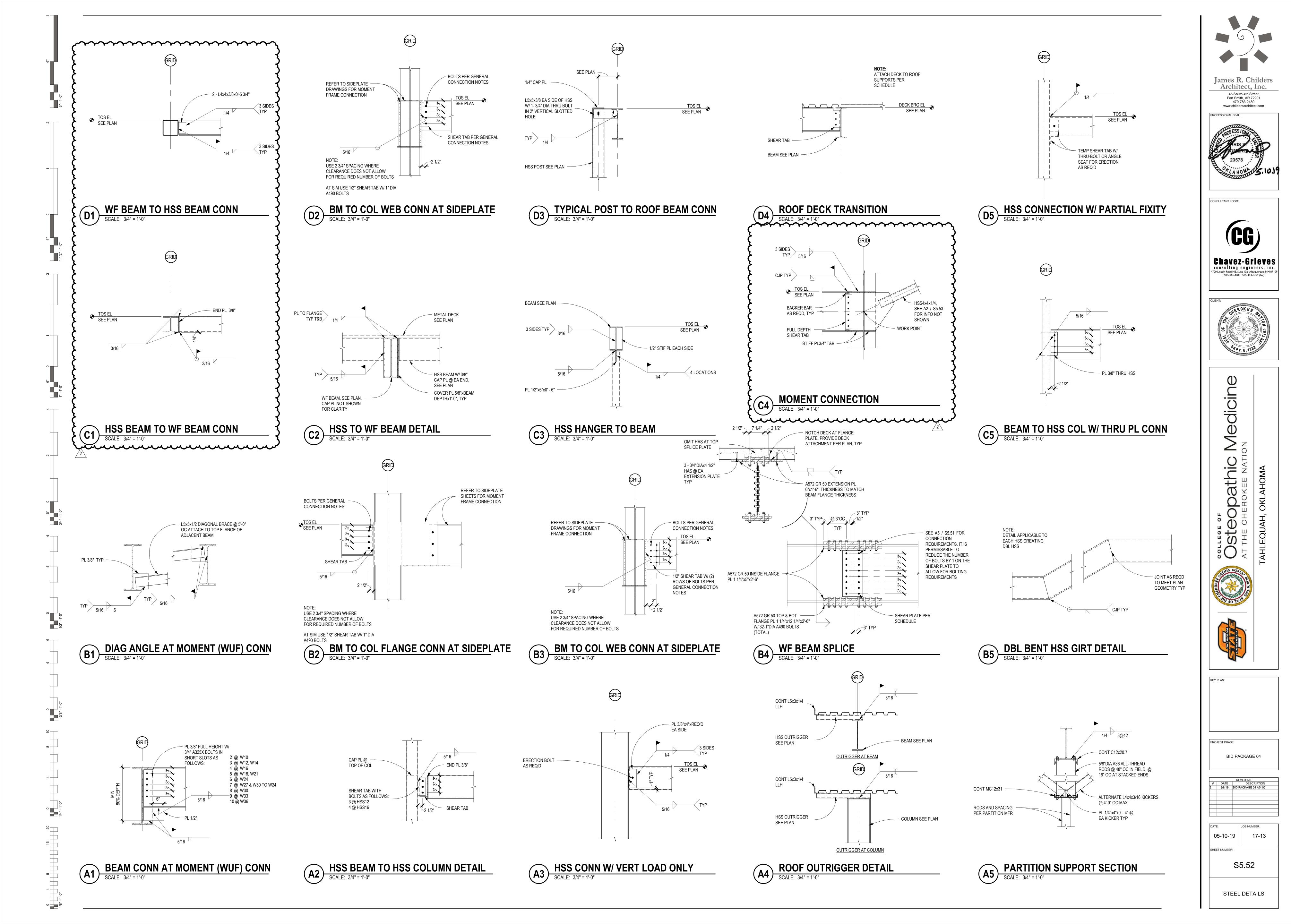
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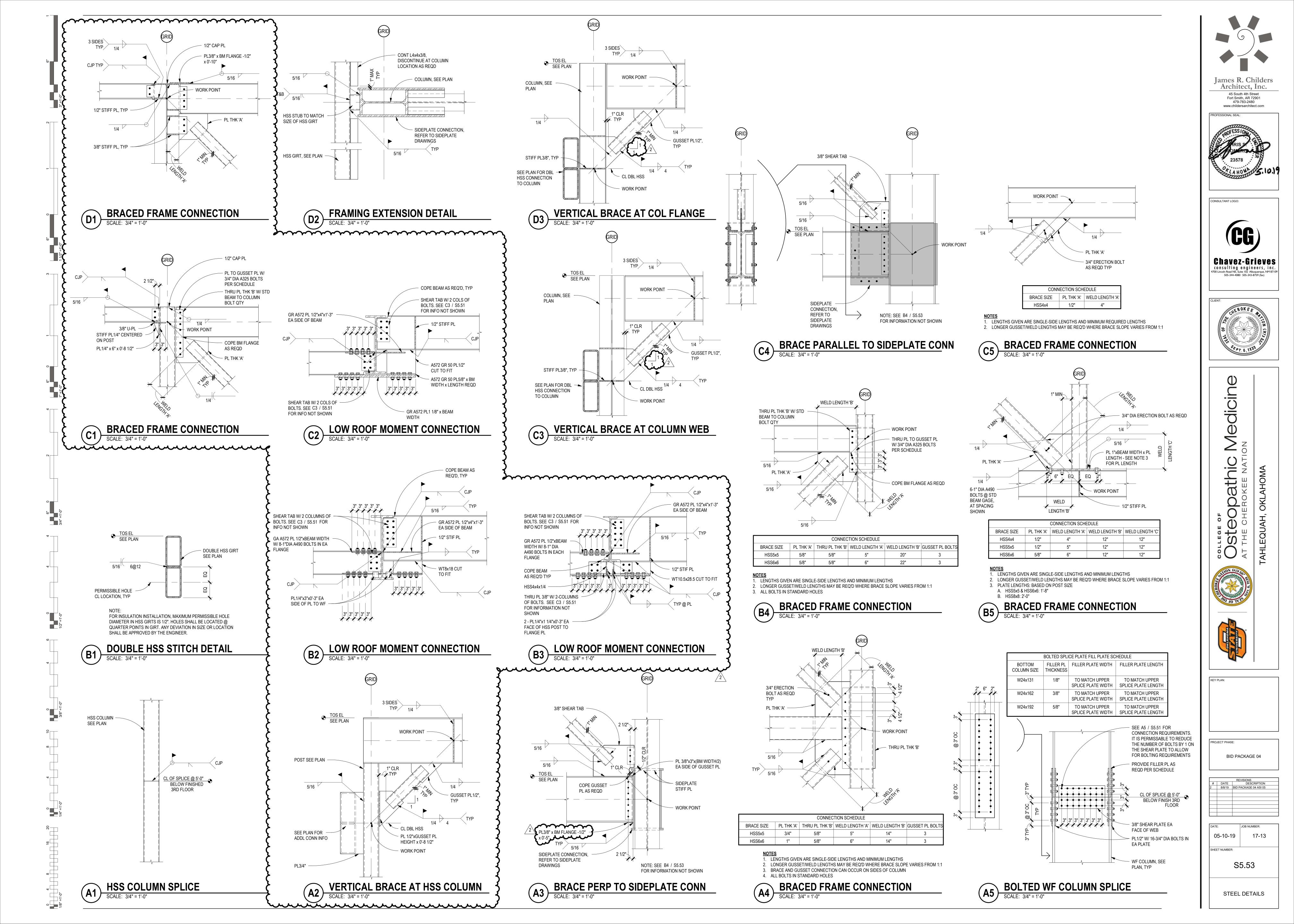
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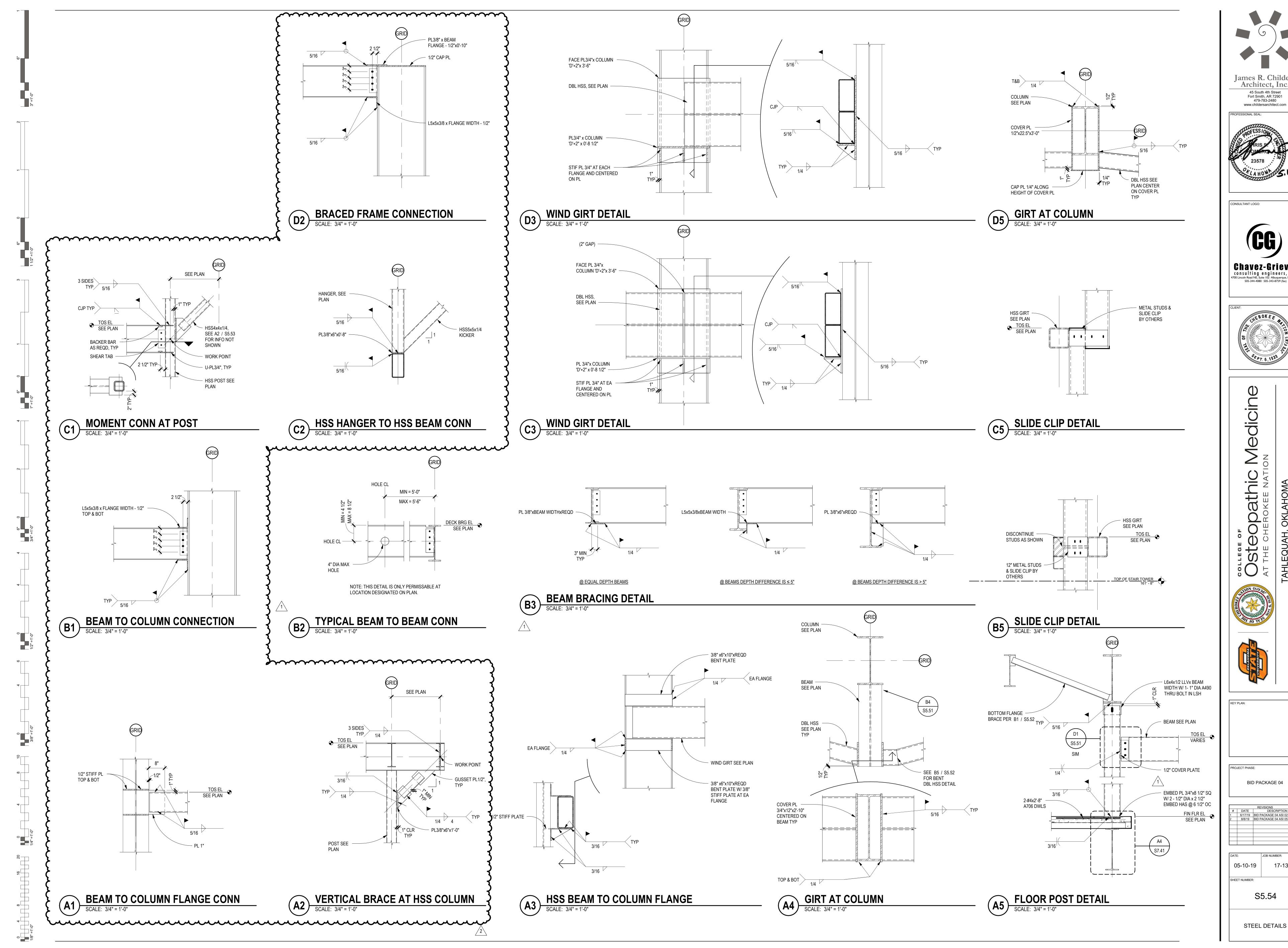
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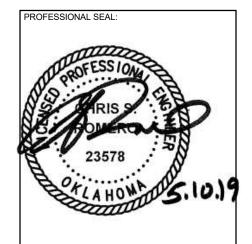


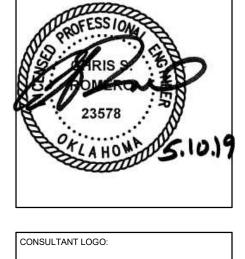




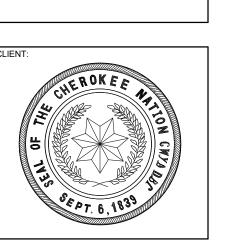


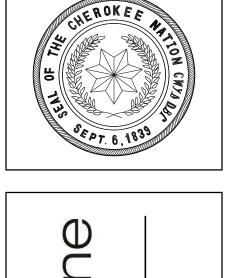
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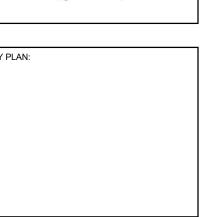


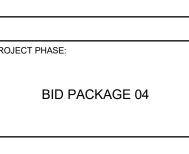






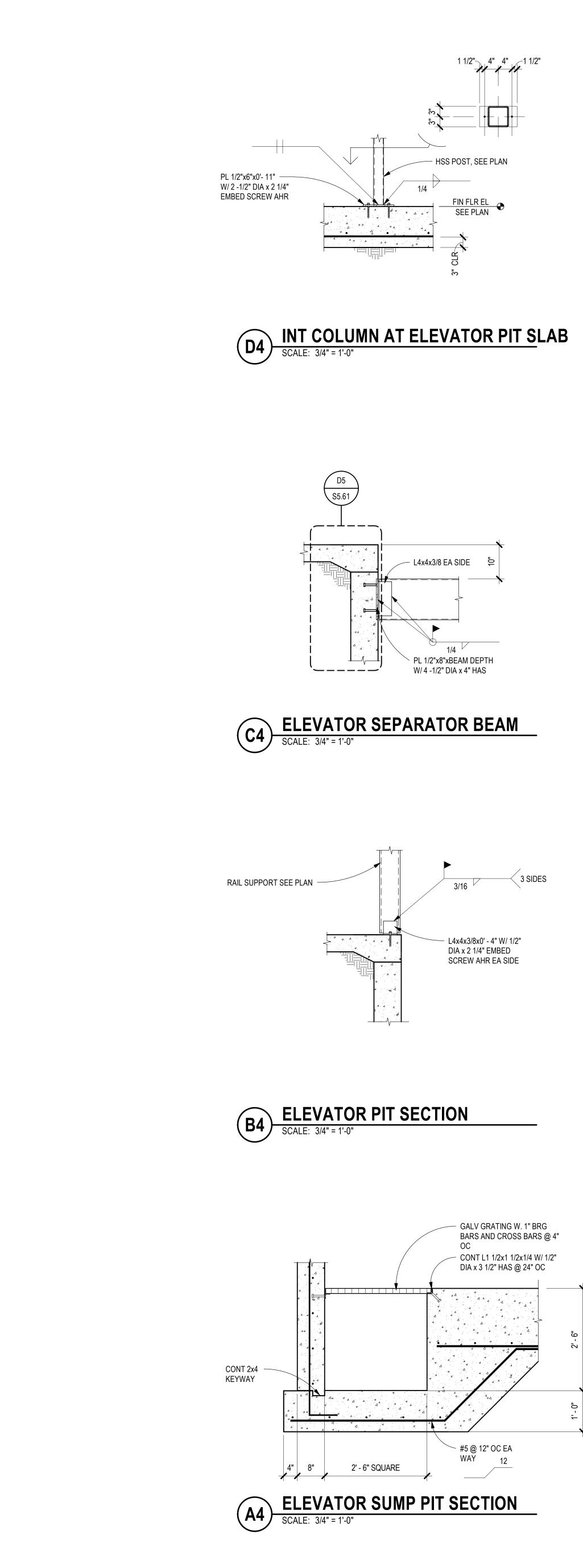


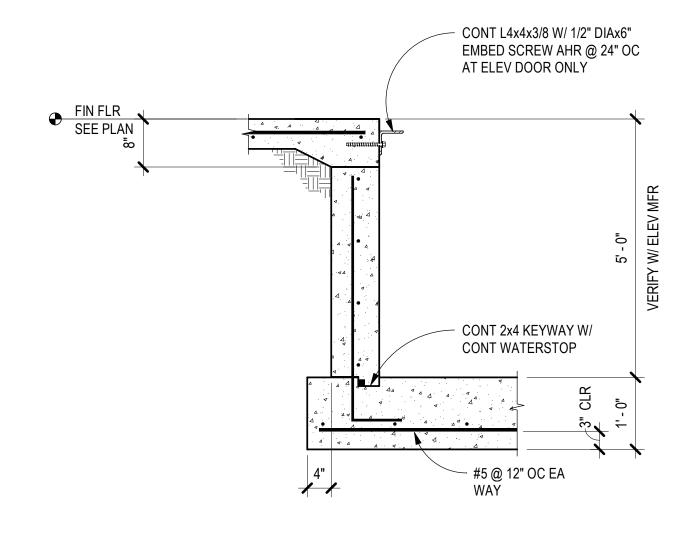




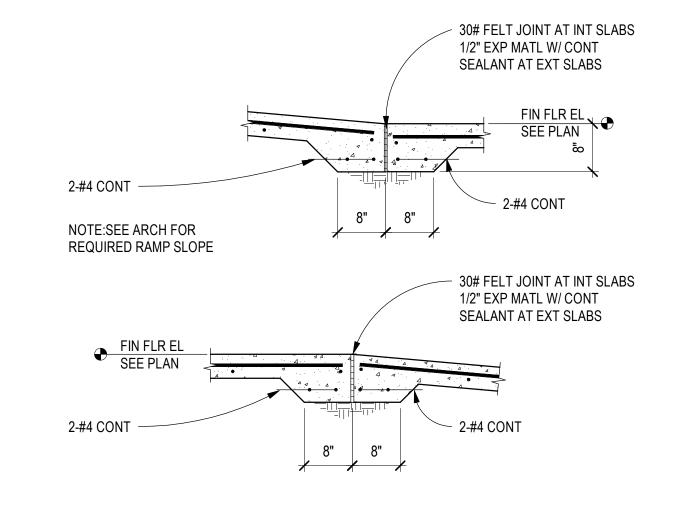
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S5.54

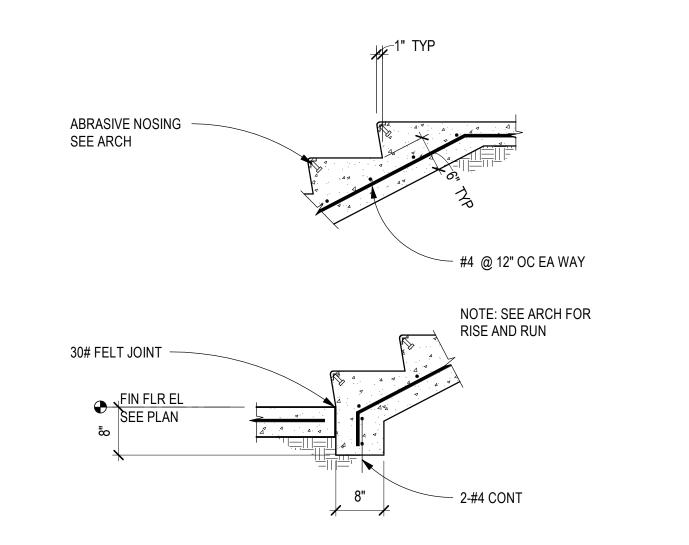




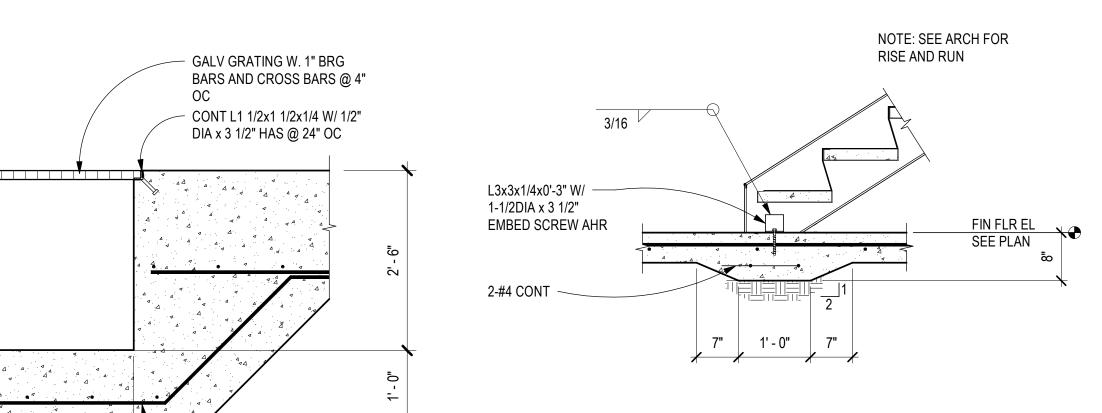




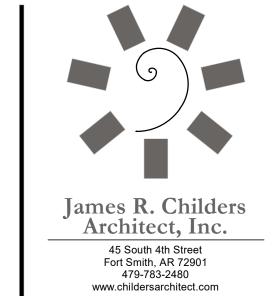


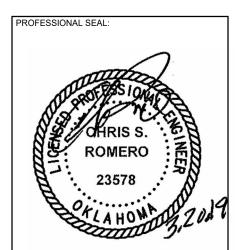


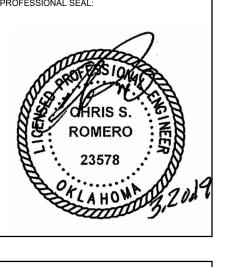




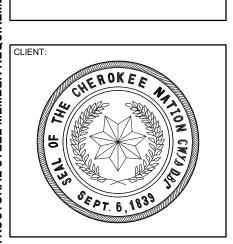








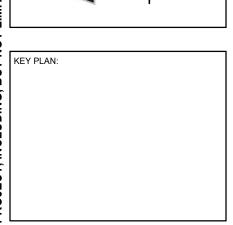


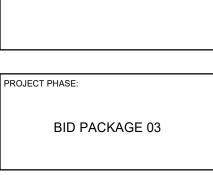










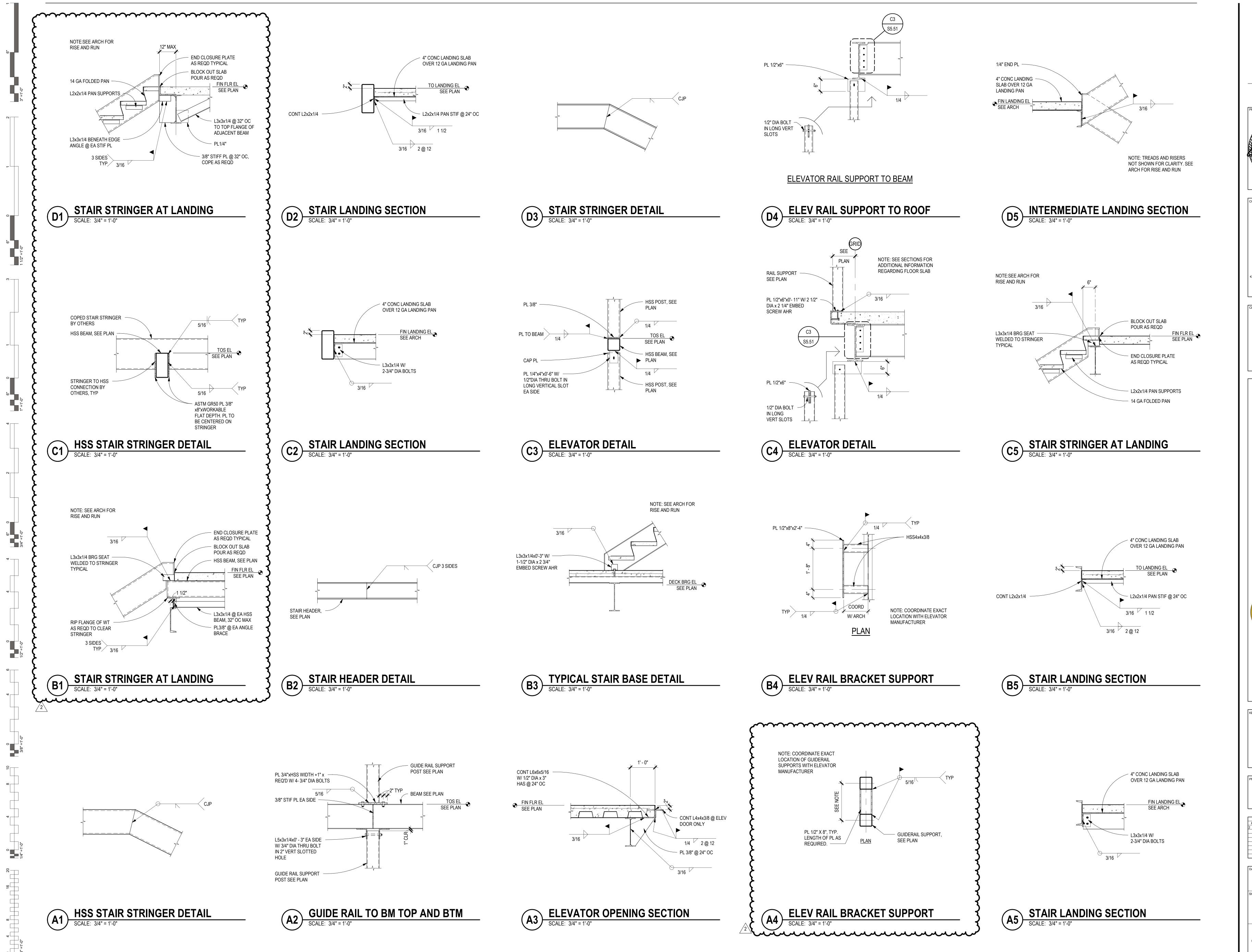


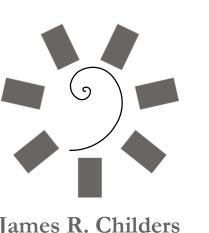
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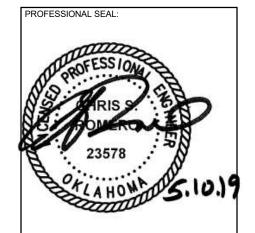
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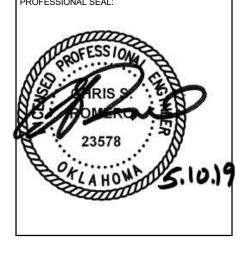
TYPICAL VERTICAL CIRCULATION FOUNDATION DETAILS

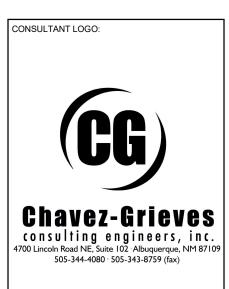


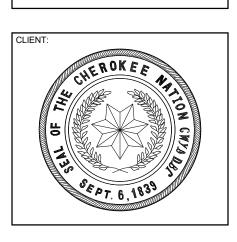


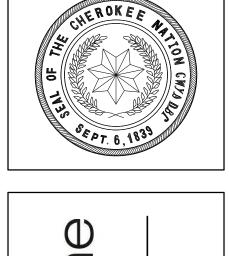
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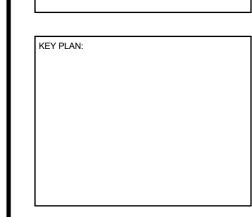


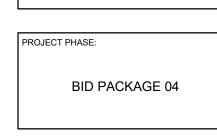


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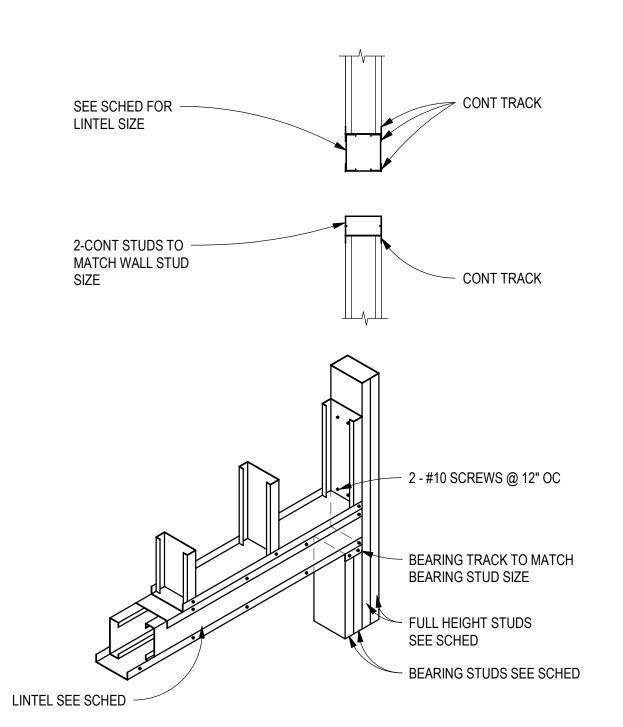




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#	DATE DESCRIPTION									
2	8/8/19	BID PACKAGE 04 ASI 05								
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05-10-19 SHEET NUMBER: S5.62

TYPICAL VERTICAL CIRCULATION FRAMING DETAILS



REINFORCEMENT TYPE	#6 ANF	SMALLE	R (#db)	#7 AND LARGER (#db)			MANAGE ENGINEER	
TELLI STOLEMENT IN E	3000 PSI	4000 PSI	5000 PSI	3000 PSI	4000 PSI	5000 PSI	MINIMUM LENGTH (IN)	COMMENTS
CONTINUOUS WALL FOOTINGS AND STEMWALLS	30	30	30	30	30	30	18	
RETAINING WALLS AND BASEMENT WALL VERTICAL REINFORCING	57	50	45	72	62	56	12	
RETAINING WALLS AND BASEMENT WALL HORIZONTAL REINFORCING	57	50	45	72	62	56	12	
CONCRETE COLUMNS NOT SUPPORTING LATERAL FORCES	30	30	30	30	30	30	12	
CONCRETE COLUMNS SUPPORTING LATERAL FORCES	57	50	45	72	62	56	12	
TOP FLEXURAL REINFORCEMENT, INCLUDING BEAMS, GRADE BEAMS, AND COMBINED COLUMN FOOTING AT BRACED FRAME AND MOMENT FRAMES	57	50	45	72	62	56	12	
BOTTOM FLEXURAL REINFORCEMENT, INCLUDING BEAMS, GRADE BEAMS, AND COMBINED COLUMN FOOTING AT BRACED FRAME AND MOMENT FRAMES	57	50	45	55	62	56	12	
SLABS-ON-GRADE	30	30	30	30	30	30	12	
MINIMUM EMBEDMENT OF STANDARD HOOKS INTO CONCRETE BASE	16	14	12	16	14	14	6	INCREASE LENGTH FOR # 11 BARS AND

	DECK SCHEDULE														
				SLAB		ME	TAL DE	CK		DECK ATTACHMENTS		TOTAL SLAB /	TOTAL SLAB /		
MARK	COMPOSITE SLAB	THICK	MATL	REINF	THICK	TYPE	GAGE	FINISH	ATTACH PERP TO RIBS	ATTACH PARALLEL TO RIBS	ATTACH SIDELAPS	DECK THICKNESS	COMMENTS		
D1.5R					1 1/2"	В	20	PAINTED	4-5/8 "DIA PUDDLE WELDS PER 36 "WIDE SHEET	5/8" DIA PUDDLE WELDS @ 12" OC	#10 SCREWS @ 12" OC	1 1/2"			
D6R	Х	3"	NW CONC	6x6 - W2.1xW2.1 WELD WIRE FABRIC IN FLAT SHEETS	3"	VLI	18	GALVANIZED	4-5/8 "DIA PUDDLE WELDS PER 36 "WIDE SHEET	5/8" DIA PUDDLE WELDS @ 12" OC	#10 SCREWS @ 12" OC	6"			
D7.5F	Х	4 1/2"	NW CONC	#5 @ 12" OC EA WAY	3"	VLI	18	GALVANIZED	4-5/8 "DIA PUDDLE WELDS PER 36 "WIDE SHEET	5/8" DIA PUDDLE WELDS @ 12" OC	#10 SCREWS @ 12" OC	7 1/2"	PROVIDE 3/4" DIAMETER x 6" LONG HEAD ANCHOR STUDS WHEN SPECIFIED ON P		

	SLAB-ON-GRADE SCHEDULE										
SLAB											
MARK	THICKNESS	MATL	REINFORCING	BEARING STRATA	COMMENTS						
S5	5"	CONC	#4 @ 18" OC EA WAY	15 MIL VAPOR RETARDER OVER LASER LEVELED 4" MINIMUM GRANULAR BASE OVER COMPACTED SUBGRADE PER GEN STRUCT NOTES	THE FINISHED TOP SURFACE SHOULD BE FLAT AND LEVEL WITH SUFFICIENT FINES TO FILL BETWEEN COARSE AGGREGATE. IF THIS IS NOT THE CASE, PROVIDE UP TO 1/2 INCH BLOTTE LAYER OF SAND						
S6	6"	CONC	#4 @ 12" OC EA WAY	15 MIL VAPOR RETARDER OVER LASER LEVELED 4" MINIMUM GRANULAR BASE OVER COMPACTED SUBGRADE PER GEN STRUCT NOTES	THE FINISHED TOP SURFACE SHOULD BE FLAT AND LEVEL WITH SUFFICIENT FINES TO FILL BETWEEN COARSE AGGREGATE. IF THIS IS NOT THE CASE, PROVIDE UP TO 1/2 INCH BLOTTE LAYER OF SAND						

	/ 2 \ RAVITY BASE PLATE				— TOP OF BASE PLATE				— TOP OF BASE PLATE
COLUMN SIZE TYPE	SIZE "T"x"A"x"B"	ANCHOR BOLTS QTY x SIZE x GRADE	STD WASHER W/ DBL HEX NUTS PER		 TOP OF CONCRETE	STD WASHEI W/ DBL HEX NUTS PER			TOP OF CONCRETE
	PL 2"x21"x1' - 9" PL 3/4"x14"x1' - 2" PL 3/4"x16"x1' - 4"	4 - 3/4" DIA x 9" F1554 GR 36 4 - 3/4" DIA x 9" F1554 GR 36 4 - 3/4" DIA x 9" F1554 GR 36	AISC 360-10 TABLE 14-2	IED EMBED	— STD WASHER W/	AISC 360-10 TABLE 14-2	HED EMBED	NOTE	— PLATE WASHEF
	•	OVIDE MAXIMUM OVERSIZED E AND THICKNESS PER AISC		SCHED	DBL HEX NUTS		SCH		1/2"x3" SQ W/ DBL HEX NUTS
360-10 TABLE 14-2.	VASHERS WITH SIZI	E AND THICKNESS PER AISC					NOTE: FIRST 1'-0" OF E	_	-

GRAVITY ANCHOR BOLT

	L/	ATERAL BASE PLAT	E SCHEDULE	GRAVIII ANG	HUK BULI		LATERAL ANCHUR
	BASE P	LATE	ANCHOR BOLTS				
MARK	TYPE	SIZE "T"x"A"x"B"	QTY x SIZE x GRADE				
BP1	В	PL 2 1/4"x30"x1'-6"	6 - 1 1/4" DIA x 18" F1554 GR 36			SCHED 'B'	
BP2	В	PL 2 1/2"x30"x1'-6"	6 - 1 1/4" DIA x 18" F1554 GR 36	SCHED 'B'			
BP3	С	PL 2 1/4"x30"x1'-6"	SEE A5 / S5.51	1		2 1/2" EQ EQ 2	1/2"
BP4	В	PL 2 1/4"x30"x1'-6"	6 - 1 1/4" DIA x 24" F1554 GR 36	2" EQ EQ 2"		7 1 1 1 1 1	
		. BASE PLATES, PRO KNESS PER AISC 36	OVIDE PLATE WASHERS 60-10 TABLE 14-2.		TYP 3/16		Ē-
				A EO Z'			A. E. E.

TYPE 'A' TYPE 'B'

2 1/2" EQ EQ 2 1/2" TYPE 'C'

NOTE: FIRST 1'-0" OF EMBEDDED PORTION OF ANCHOR BOLT SHALL BE UNTHREADED

LATERAL ANCHOR BOLT

		<u> </u>				
			WAL	L SCHEDULE		
MARK	VENEER	WALL	VERTICAL	HORIZONTAL	GRADE	COMMENTS
WC8		8" CONC	#4 @ 12" OC	#4 @ 12" OC	A615	
WC8A		8" CONC	#5 @ 9" OC	#4 @ 12" OC	A615	SEE D2/S3.11
WC12		12" CONC	#5 @ 12" OC EA FACE	#4 @ 12" OC EA FACE	A615	
WC16		16" CONC	#7 @ 12" OC EA FACE	#5 @ 12" OC EA FACE	A615	
VC16A		16" CONC	#7 @ 12" OC EA FACE	#5 @ 12" OC EA FACE	A615	
VC16B		16" CONC	#5 @ 12" OC EA FACE	#5 @ 12" OC EA FACE	A615	
\ <i>\</i> \(C22		22" CONC	#5 @ 12" OC EA EACE	#5 @ 12" OC EA EACE	Δ615	

	SPOT FOOTING SCHEDULE									
		SIZE		REINFO	PRCING					
MARK	WIDTH	LENGTH	DEPTH	BOTTOM	TOP	COMMENTS				
F30	2' - 6"	2' - 6"	1' - 0"							
F36	3' - 0"	3' - 0"	1' - 0"	4 - #4 EA WAY		STD HOOK EACH END OF EACH BAF				
F96	8' - 0"	8' - 0"	2' - 0"	10 - #7 EA WAY	10 - #7 EA WAY					
F108	9' - 0"	9' - 0"	2' - 3"	12 - #7 EA WAY	12 - #7 EA WAY					
F120	10' - 0"	10' - 0"	2' - 6"	14 - #7 EA WAY	14 - #7 EA WAY					
F132	11' - 0"	11' - 0"	2' - 9"	12 - #8 EA WAY	12 - #8 EA WAY					
F144	12' - 0"	12' - 0"	2' - 9"	15 - #8 EA WAY	15 - #8 EA WAY					
F156	13' - 0"	10' - 0"	2' - 0"	15 - #7 EA WAY		CONTINUOUS FOOTING REINFORCEMENT SHALL BE CONTINUOUS THROUGH FOOTING				
F156A	13' - 0"	13' - 0"	2' - 9"	18 - #8 EA WAY	18 - #8 EA WAY	CONTINUOUS FOOTING REINFORCEMENT SHALL BE CONTINUOUS THROUGH FOOTING				
F168	14' - 0"	14' - 0"	3' - 0"	18 - #9 EA WAY	18 - #9 EA WAY					
F180	15' - 0"	15' - 0"	3' - 3"	22 - #9 EA WAY	22 - #9 EA WAY					
F192	16' - 0"	12' - 0"	3' - 3"	20 - #9 EA WAY	20 - #9 EA WAY					
F480	12' - 0"	40' - 0"	2' - 6"	#8 @ 9" OC EA WAY	#8 @ 9" OC EA WAY					
F504	12' - 0"	42' - 0"	2' - 6"	#8 @ 6" OC EA WAY	#8 @ 6" OC EA WAY					
F528	13' - 0"	44' - 0"	2' - 6"	#8 @ 6" OC EA WAY	#8 @ 6" OC EA WAY					
F828	13' - 0"	69' - 0"	2' - 6"	#8 @ 6" OC EA WAY	#8 @ 6" OC EA WAY					

				C	ONCRETE SITE RET	AINING WALL SCH	HEDULE			
WALL	STEM	WALL HEIGHT	HEIGHT RETAINED		FOOTING SIZE		FOOTING RE	EINFORCING		
_	THICKNESS	ABOVE GRADE "HW"	"HR"	TOE WIDTH HEEL WIDTH		THICKNESS	REINF #1 REINF #2		WALL REINFORCING	
	IN	FT-IN	FT-IN	FT-IN	FT-IN	FT-IN	QTY - SIZE	SIZE - SPACING	VERT SIZE - SPACING	HORIZ SIZE - SPACIN
	8"	0'-0" - 0'-6"	0'-0" - 4'-0"	1'-6"	3'-4"	1'-3"	6 - #5 CONT T&B	#5 @ 18" OC T&B	#5 @ 12" OC	#4 @ 12" OC
A	8"	0-0 - 0-6	4'-1" - 6'-0"	2'-0"	5'-4"	1'-6"	9 - #5 CONT T&B	#5 @ 12" OC T&B	#7 @ 12" OC	#4 @ 12" OC
	10"	0'-0" - 0'-6"	6'-1" - 8'-0"	2'-6"	5'-8"	1'-6"	10 - #5 CONT T&B	#5 @ 12" OC T&B	#6 @ 12" OC EA FACE	#4 @ 12" OC EA FA
В	12"	01.011.01.011	8'-1" - 10'-0"	3'-0"	6'-0"	2'-0"	12 - #5 CONT T&B	#5 @ 12" OC T&B	#7 @ 12" OC EA FACE	#4 @ 12" OC EA FA
		12"	0'-0" - 0'-6"	10'-1" - 12-0"	3'-6"	7'-0"	2'-0"	14 - #5 CONT T&B	#5 @ 12" OC T&B	#7 @ 6" OC EA FACE

ALL REBAR LAPS IN CMU

- 1. COORDINATE EXACT LOCATION AND EXTENT OF WALL WITH ARCHITECTURAL AND CIVIL DWGS.
- 2. PROVIDE CONCRETE WALL VERTICAL CONTROL JOINTS AT (2) TIMES THE WALL HEIGHT AND AT ALL STEPS IN TOP OF WALL. SEE DETAIL C2/S7.11 FOR CONTROL JOINT INFORMATION.
- 3. WALL DRAINAGE SYSTEM SHALL CONSIST OF ONE OF THE FOLLOWING: A. CONTINUOUS 4" DIAMETER SLOTTED SCHEDULE 40 DRAIN PIPE WITH SLOTS ON BOTTOM HALF
- B. 1 1/2" (MIN) DIA PVC WEEP PIPE @ 8'-0" OC MAX W/ 12"x12"x12" GRAVEL & MIRAFI 140N GEOTEXTILE FILTER FABRIC BETWEEN SOIL & GRAVEL @ EA WEEP HOLE OR WEEP HOLES.

 C. TIE-IN TO SITE DRAINAGE SYSTEM. BACKFLOW PREVENTION, ETC. TO BE PROVIDED BY OTHERS.

SEE GEOTECHNICAL REPORT FOR ADDITIONAL INFORMATION.

, "HW"		TO WALL EL SEE CIVIL DWGS PROVIDE WATERPROOFING
))		PER ARCH DWGS WALL REINF
) "HR"	DWL W/ STD HOOK TO MATCH VERT WALL REINF	SEE NOTE 3 FOR DRAINAGE REQUIREMENTS
1' - 0"	CENTER REINF 2	TOE EI
THICKNESS	REINF #1	COORD W/ CIVIL
	REINF #2 TOE WIDTH THK HEEL WIDTH	3" CLR-
	WALL TYPE "A"	

MH 4 4	TO WALL EL SEE CIVIL DWGS
	PROVIDE WATERPROOFING PER ARCH DWGS
	WALL REINF
型 DWL W/ STD HOOK TO MATCH VERT WALL REINF	SEE NOTE 3 FOR DRAINAGE REQUIREMENTS
10-1-	LAP SPLICE SEE SCHED
REINF #1	COORD W/ CIVIL
REINF #2 TOE WIDTH THK	HEEL WIDTH
WALL TY	PE "B"

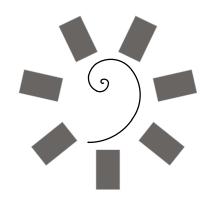
		CO	NTINUOUS FOOTING S	SCHEDULE	
	SI	ZE			
MARK	MARK WIDTH DEPTH		CONTINUOUS	TRANSVERSE	COMMENTS
CF36	3' - 0"	1' - 0"	4 - #4 BOT	#4 @ 12" OC BOT	
CF48	4' - 0"	1' - 0"	5 - #5 BOT	#5 @ 12" OC BOT	
CF60	5' - 0"	1' - 3"	8 - #5 BOT	#5 @ 12" OC BOT	
CF72	6' - 0"	1' - 3"	10 - #5 BOT	#5 @ 12" OC BOT	
CF84	7' - 0"	1' - 3"	12 - #5 TOP & BOT	#5 @ 12" OC TOP & BOT	
CF96	8' - 0"	1' - 6"	10 - #7 TOP & BOT	#7 @ 12" OC TOP & BOT	
CF120	10' - 0"	1' - 6"	12 - #7 TOP & BOT	#7 @ 12" OC TOP & BOT	
CF132	11' - 0"	1' - 6"	12 - #7 TOP & BOT	#7 @ 12" OC TOP & BOT	
CF156	13' - 0"	2' - 0"	18 - #7 TOP & BOT	#7 @ 12" OC TOP & BOT	

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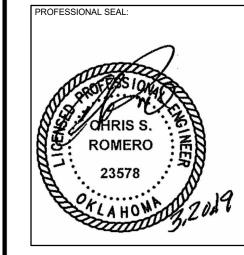
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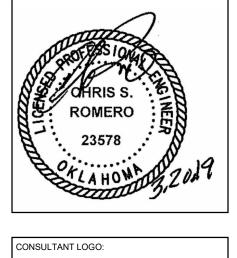
TYPICAL CONCRETE SITE RETAINING WALL SCHEDULE AND DETAIL

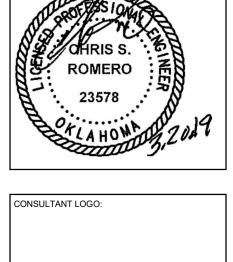
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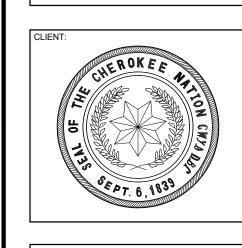
James R. Childers Architect, Inc. 45 South 4th Street Fort Smith, AR 72901 479-783-2480 www.childersarchitect.com









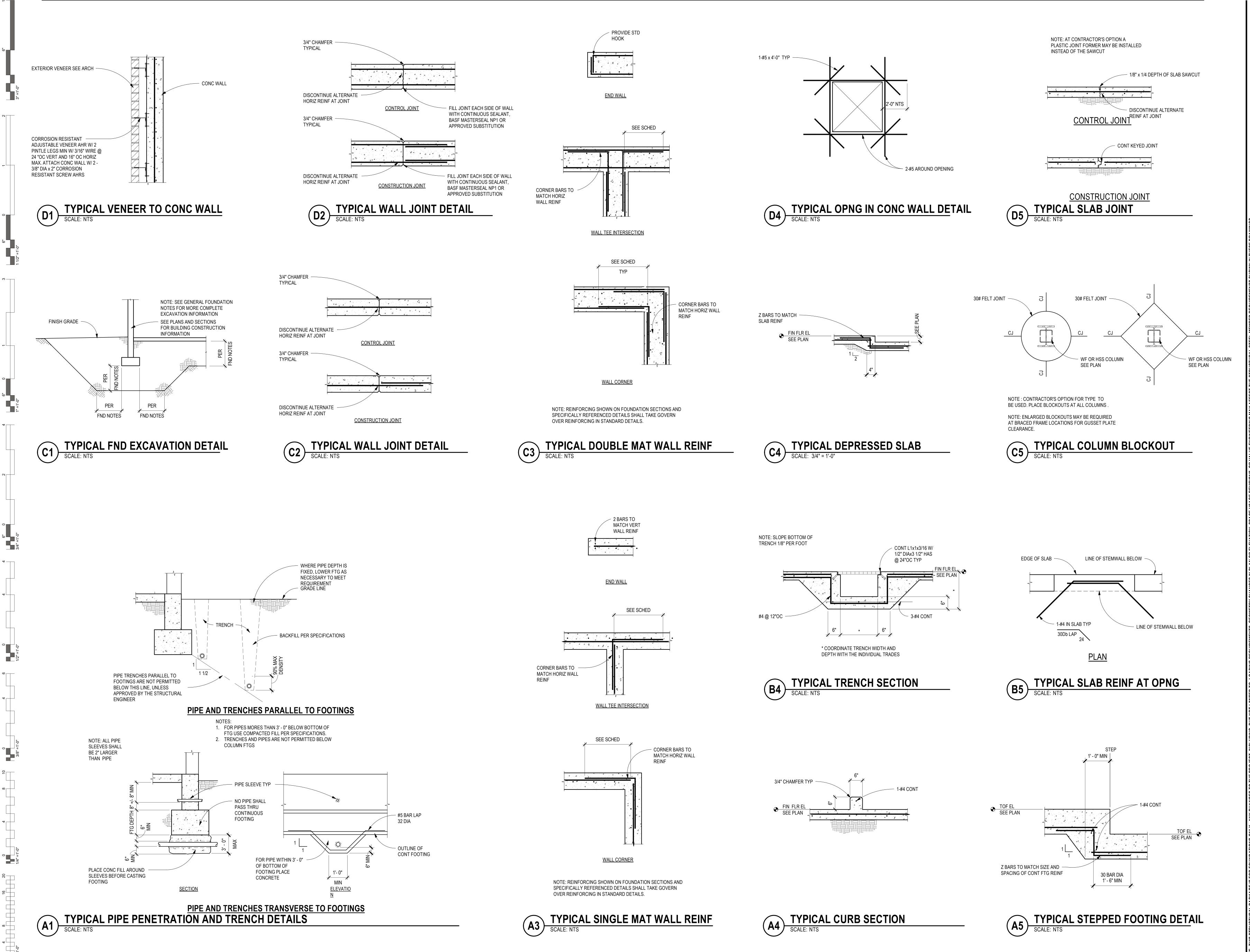


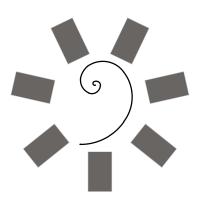




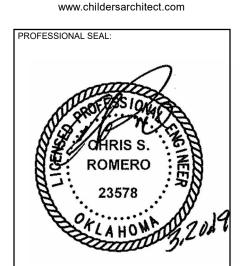


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2	5/24/19	BID PACKAGE 03 ASI 02

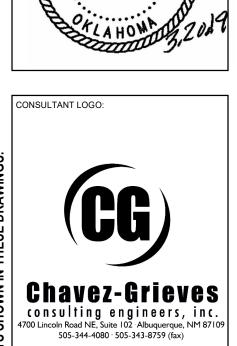


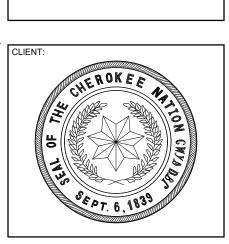


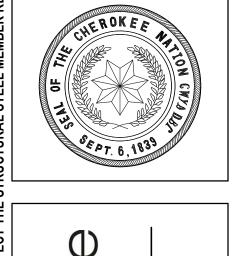
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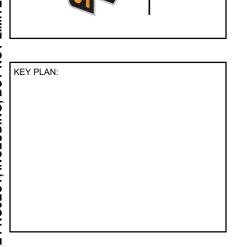




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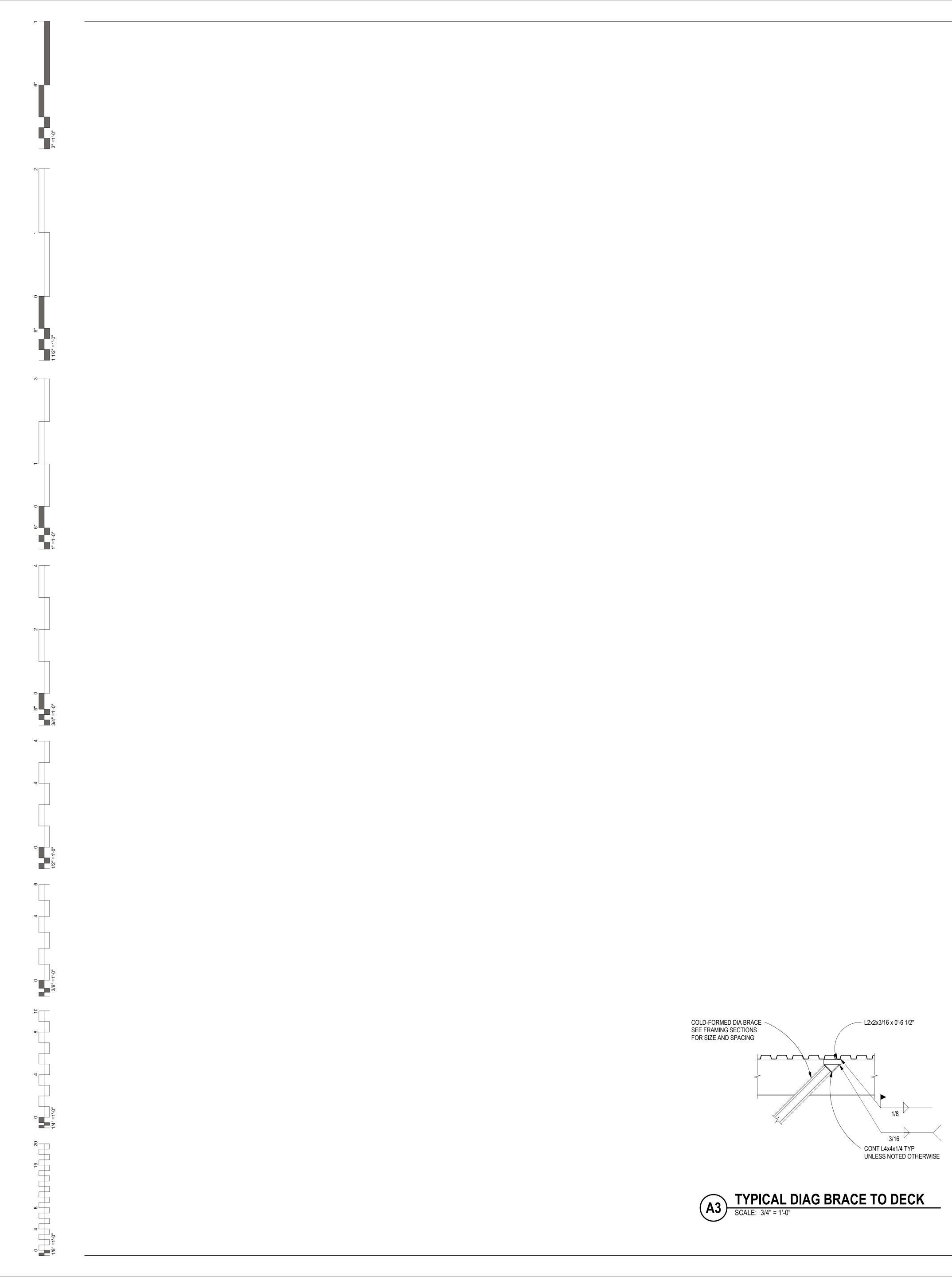


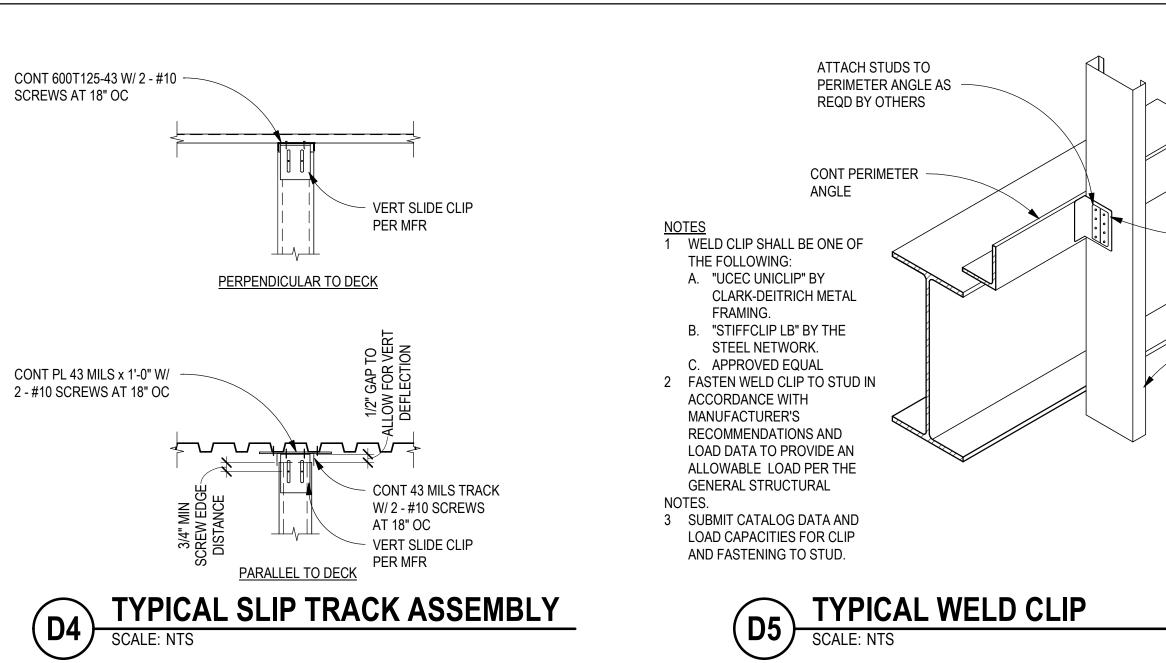
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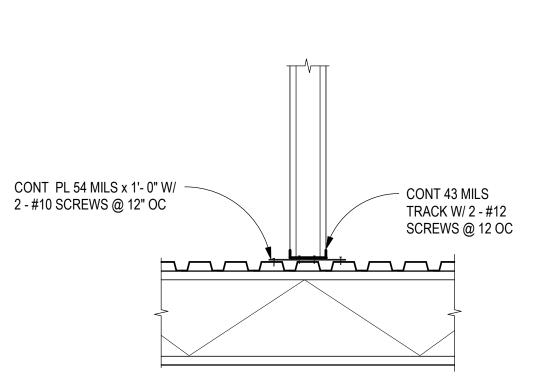
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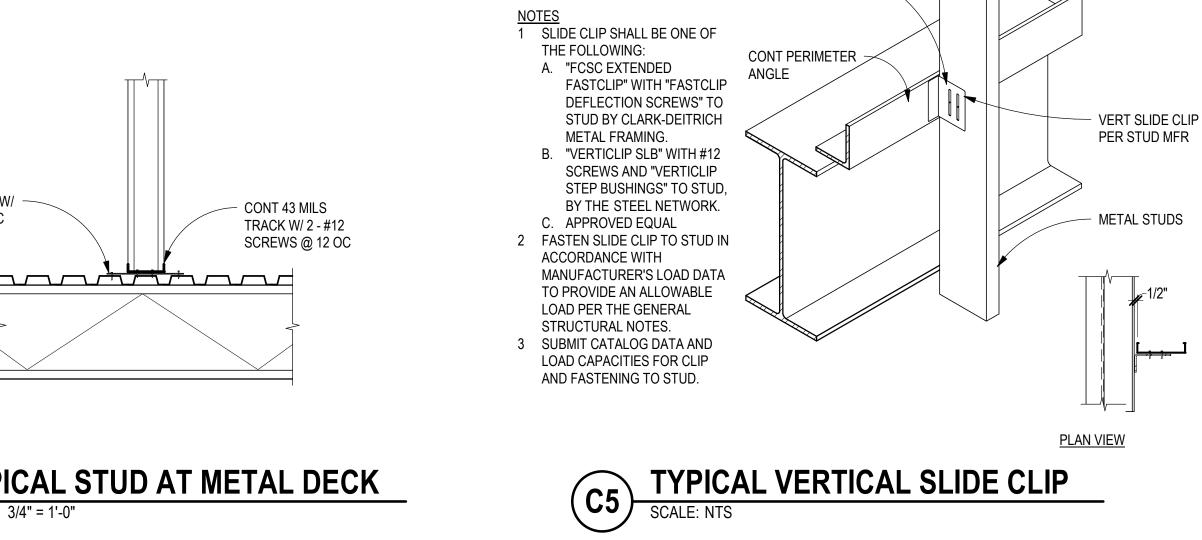
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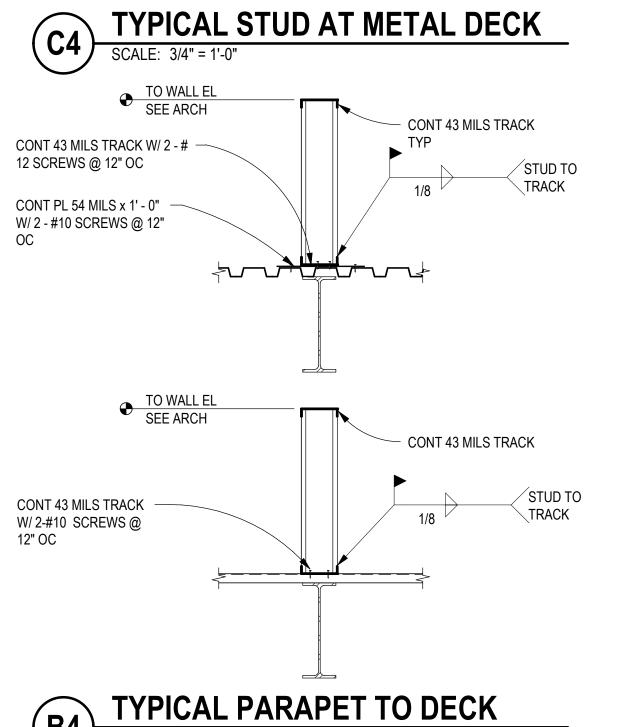
TYPICAL CONCRETE DETAILS

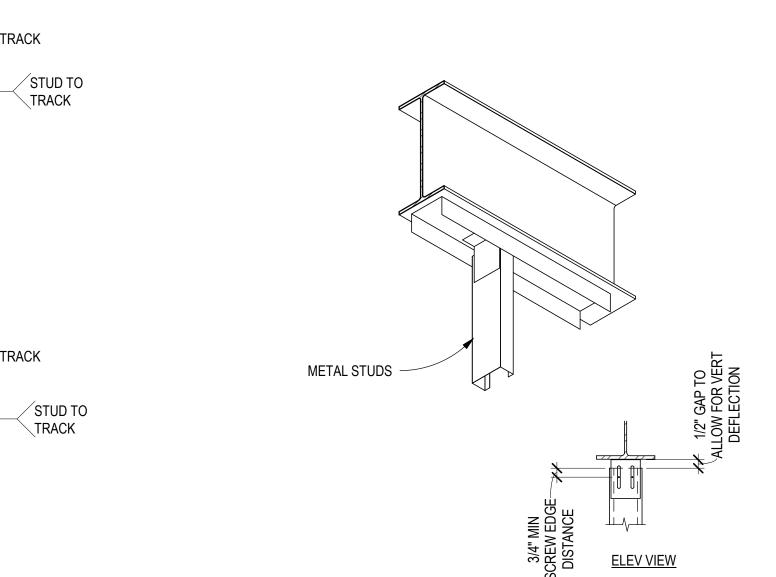








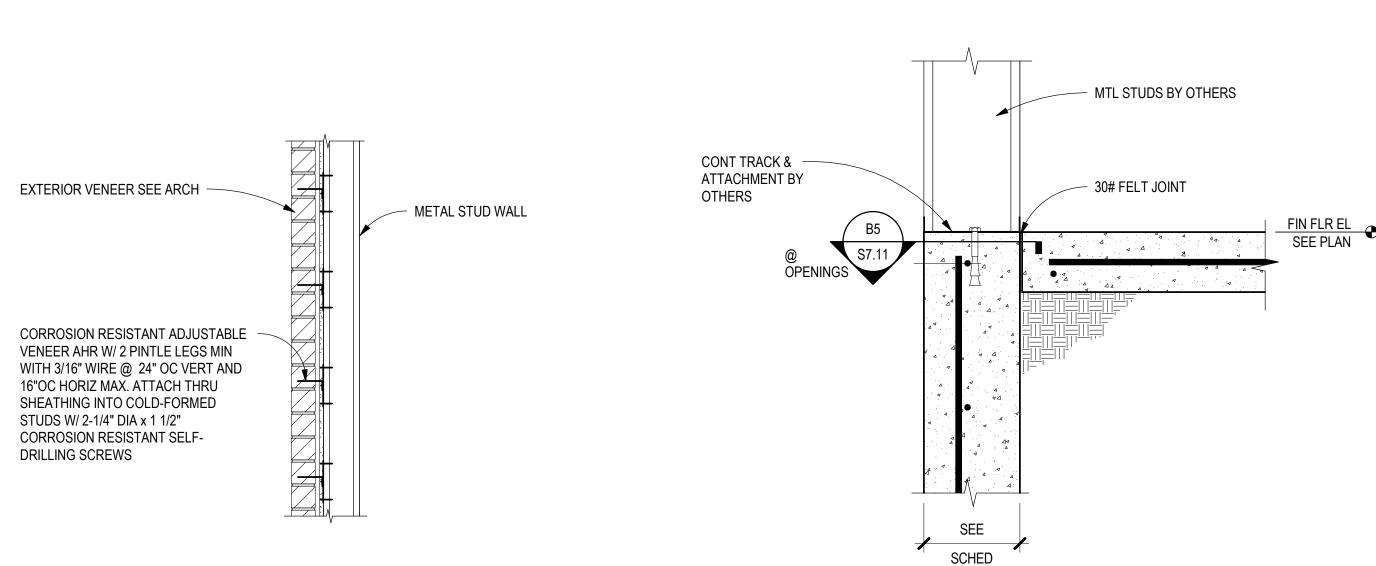




ATTACH STUDS TO

PERIMETER ANGLE AS REQD BY OTHERS



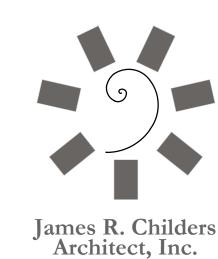


TYPICAL VENEER ON MTL STUDS

SCALE: 3/4" = 1'-0"

TYPICAL STUD @ CONC STEMWALL

SCALE: 1 1/2" = 1'-0"

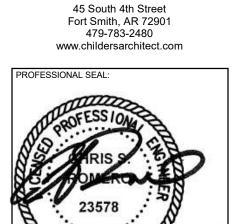


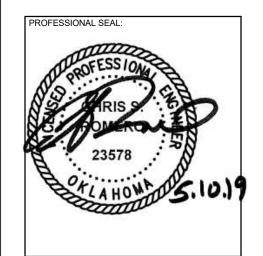
WELD CLIP

PER STUD MFR

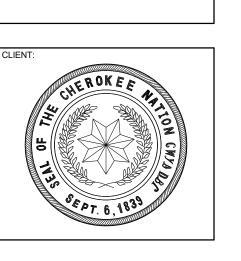
- METAL STUDS

PLAN VIEW



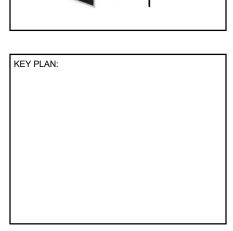


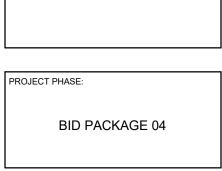








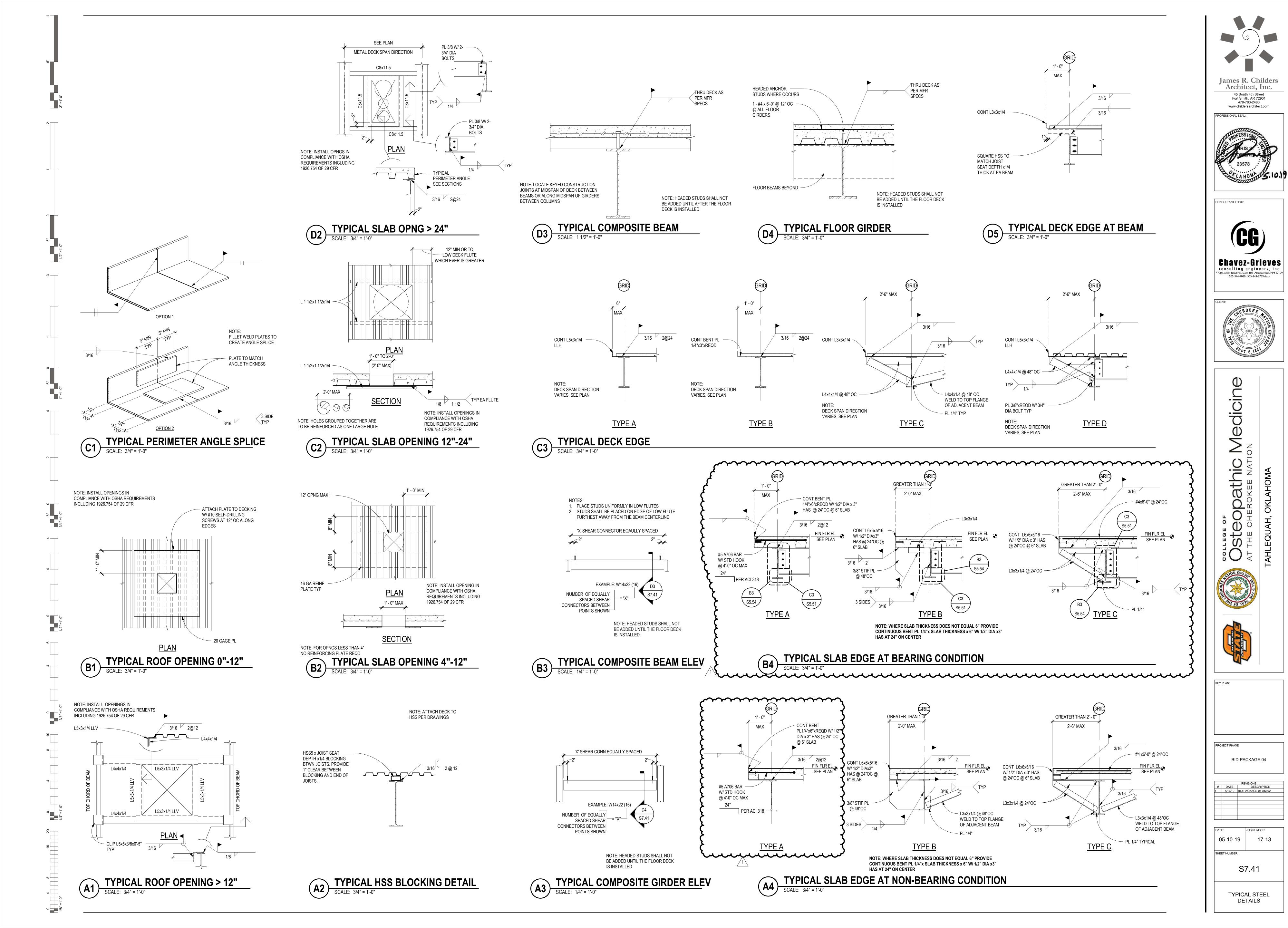




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TYPICAL COLD-FORMED DETAILS



c. ESTIMATED NUMBER OF SIDEPLATE JOINTS FOR THIS PROJECT THAT ARE **NOT** SUPPORTED BY eDATA = 0 d. MISCELLANEOUS DETAILS, TYPICALLY DESIGNATED BY M#, ARE NOT SUPPORTED.

INSTRUCTIONS TO STEEL FABRICATOR

- a. THE STEEL FABRICATOR'S BID PRICE FOR PROCUREMENT, FABRICATION AND ERECTION OF STRUCTURAL AND MISCELLANEOUS STEEL SHALL INCLUDE THE SIDEPLATE LICENSE FEE FOR THE PROJECT. EACH PROSPECTIVE STEEL FABRICATOR WHO BIDS THE PROJECT SHALL
- FORMALLY REQUEST THE SIDEPLATE LICENSE FEE BY ACCESSING THE SIDEPLATE WEBSITE (http://www.sideplate.com b. UPON THE SUCCESSFUL STEEL FABRICATOR SIGNING A CONTRACT TO FABRICATE STRUCTURAL STEEL FOR THIS PROJECT, THE STEEL FABRICATOR SHALL SUBMIT A PURCHASE ORDER (PO) TO SIDEPLATE SYSTEMS, INC. FOR THE TOTAL AMOUNT OF THE SIDEPLATE LICENSE
- FEE AND SHALL INCLUDE SAID FEE IN ITS FIRST CONSTRUCTION DRAW c. THE STEEL FABRICATOR SHALL MAKE PAYMENT OF THE SIDEPLATE LICENSE FEE DIRECTLY TO:

SIDEPLATE SYSTEMS, INC. 25909 PALA, SUITE 200 MISSION VIEJO, CA 92691

TEL: 949-238-8900

1. IN ADDITION TO THE REQUIRED SUBMITTALS SPECIFIED BY THE BALANCE OF THE CONTRACT DOCUMENTS, THE FOLLOWING SUBMITTALS SHALL BE SENT TO SIDEPLATE SYSTEMS, INC. ELECTRONICALLY VIA THE STRUCTURAL ENGINEER OF RECORD FOR THEIR REVIEW AND DISPOSITION: a. QUALITY CONTROL PROGRAM (REQUIRED IF NOT AISC CERTIFIED)

b. ONE ELECTRONIC COPY OF ALL STRUCTURAL STEEL DRAWINGS THAT EITHER DIRECTLY PERTAINS TO AND/OR AFFECTS THE SHOP FABRICATION OR FIELD ERECTION OF THE SIDEPLATE STEEL FRAME CONNECTION SYSTEM, INCLUDING THE INITIAL SUBMITTAL AND ALL CORRECTED RE-SUBMITTALS OF AFFECTED DRAWINGS. SIDEPLATE SYSTEMS, INC. SHALL BE GIVEN, AS A MINIMUM, THE SAME SPECIFIED REVIEW TIME (NOT LESS THAN SEVEN BUSINESS DAYS) AS THE ENGINEER OF RECORD.

PRE-DETAILING MEETING

a. PRIOR TO THE START OF DETAILING OF THE SHOP DRAWINGS, THE FABRICATION CONTRACTOR SHALL FORMALLY REQUEST A PRE-DETAILING MEETING FROM SIDEPLATE SYSTEMS, INC. THIS MEETING IS TYPICALLY A WEBINAR TO DISCUSS BEST PRACTICES FOR THE DETAILING OF THE SIDEPLATE CONNECTIONS, AND TO CREATE A PROACTIVE FORUM TO ANSWER ANY QUESTIONS.

a. PRIOR TO THE START OF FABRICATION, THE FABRICATION CONTRACTOR SHALL FORMALLY REQUEST A PRE-FABRICATION MEETING FROM SIDEPLATE SYSTEMS, INC. THIS MEETING IS TYPICALLY A WEBINAR TO DISCUSS BEST PRACTICES FOR THE FABRICATION OF THE SIDEPLATE CONNECTIONS, AND TO CREATE A PROACTIVE FORUM TO ANSWER ANY QUESTIONS.

a. PRIOR TO THE START OF STEEL ERECTION, THE ERECTION CONTRACTOR SHALL FORMALLY REQUEST A PRE-ERECTION MEETING FROM SIDEPLATE SYSTEMS, INC. THIS MEETING IS TYPICALLY A WEBINAR TO DISCUSS BEST PRACTICES FOR FIELD ERECTION OF THE SIDEPLATE BEAMS AND COLUMNS, AND TO CREATE A PROACTIVE FORUM TO ANSWER ANY QUESTIONS.

1. THE GOVERNING CODES SHALL CONSIST OF ANSI/AWS D1.1-2010 (AWS D1.1), AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES (APRIL 14, 2010), 2009 RCSC SPECIFICATIONS FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS, AND ALL APPLICABLE BUILDING AND JURISDICTIONAL CODES AND PROJECT STANDARDS SPECIFIED IN THE PROJECT SPECIFICATION STRUCTURAL STEEL SECTION. WHERE THE REQUIREMENTS DIFFER BETWEEN SIDEPLATE CONNECTION NOTES, THE GENERAL STRUCTURAL NOTES, AND THE GOVERNING CODES, THE MORE STRINGENT SECTION CRITERIA SHALL CONTROL.

2. ALPHA AND NUMERIC DESIGNATORS {X} & {#} USED HEREIN TO SIMPLIFY THE IDENTIFICATION OF PLATES, ANGLES, AND WELDS ARE DEFINED

SIDE PLATE FOR UNIAXIAL CONNECTIONS

BEAM FLANGE COVER PLATE, AS REQUIRED

VERTICAL SHEAR PLATE OR FLAT BAR, AS REQUIRED

{D} HORIZONTAL SHEAR PLATE OR FLAT BAR, AS REQUIRED

VERTICAL ANGLE WELDED TO THE VERTICAL SHEAR PLATE (C), AS REQUIRED

VERTICAL SHEAR ELEMENT (VSE) WHICH CONSISTS OF PLATE {C} AND ANGLE {E} MATERIAL, AS REQUIRED

{G} LONGITUDINAL ANGLE WELDED TO THE OUTSIDE FACE OF SIDE PLATE {A}, AS REQUIRED

LONGITUDINAL ANGLE WELDED TO THE BOTTOM BEAM FLANGE (OR TOP BEAM FLANGE AS REQUIRED)

{T} HORIZONTAL PLATE WELDED TO THE OUTSIDE FACE OF SIDE PLATE {A}, AS REQUIRED

{1} FILLET WELD CONNECTING SIDE PLATE {A} TO HORIZONTAL SHEAR PLATE {D} OR COLUMN

{2} FILLET (AND/OR FLARE BEVEL) WELD CONNECTING INSIDE FACE OF SIDE PLATE {A} TO COLUMN

{3} FILLET WELD CONNECTING HORIZONTAL SHEAR PLATE {D} TO COLUMN, AS REQUIRED

{4} FILLET WELD TO CONSTRUCT VSE {F} AND TO CONNECT IT TO THE WEB OF THE BEAM, AS REQUIRED

FILLET (AND/OR PJP) WELD CONNECTING BEAM FLANGE TIPS TO COVER PLATE {B} AND/OR LONGITUDINAL ANGLE {H}, AS REQUIRED

{5a} FILLET WELD CONNECTING OUTSIDE FACE OF BEAM FLANGE TO COVER PLATE {B} AND/OR LONGITUDINAL ANGLE {H}, AS REQUIRED

{5b} FILLET WELD CONNECTING COVER PLATE {B} EDGE TO TOP FACE OF BEAM FLANGE, ACROSS ITS WIDTH

({5p}) PJP WELD CONNECTING ANGLE {H} TO BEVELED BEAM FLANGE

{8} FILLET (AND/OR PJP) WELD CONNECTING LONGITUDINAL ANGLE {G} (AND/OR PLATE {T}) TO SIDE PLATE {A}, AS REQUIRED

({8p}) PJP WELD CONNECTING PLATE {T} TO SIDE PLATE {A} AND/OR CONNECTING BUILT UP ANGLE {H} PLATES TOGETHER, AS REQUIRED

{9} FILLET WELD CONNECTING SIDE PLATE {A} TO COLUMN FACE, WRAPPED AROUND THREE SIDES OF SIDE PLATE {A}

{10} FILLET WELD TO CONSTRUCT SIDE PLATE SLOTTED INTERLOCK ASSEMBLY

{10p} PJP WELD TO CONSTRUCT SIDE PLATE SLOTTED INTERLOCK ASSEMBLY

{10r} REINFORCING FILLET WELD TO CONSTRUCT SIDE PLATE SLOTTED INTERLOCK ASSEMBLY

3. ALPHA DESIGNATORS, USED HEREIN TO SIMPLIFY THE IDENTIFICATION OF DIMENSIONS OF THE SIDEPLATE CONNECTIONS, ARE DEFINED BELOW GAP PHYSICAL SEPARATION BETWEEN THE END OF THE MOMENT FRAME BEAM AND THE ADJOINING FACE OF THE COLUMN FLANGE

B DEPTH OF SIDE PLATE {A}

C LENGTH OF COVER PLATE {B} AND/OR LONGITUDINAL ANGLE {H}

LENGTH OF SLOT FROM THE TOE OF THE RADIUS IN THE COVER PLATE {B}, AS REQUIRED

EDGE DISTANCE OF BOLT HOLES IN COVER PLATE {B}, AS REQUIRED

GAGE DISTANCE TO CENTERLINE OF BOLT HOLES IN ANGLES (G) AND (H), AND PLATE (T), AS REQUIRED

ADDED DIMENSION TO COLUMN FLANGE WIDTH TO DEFINE TOTAL COVER PLATE {B} WIDTH

DISTANCE FROM END OF THE BEAM TO CENTERLINE OF VERTICAL BOLT HOLES IN VSE {F}, AS REQUIRED

R RADIUS OF SLOT DIMENSION IN COVER PLATE {B}

HORIZONTAL SPACING BETWEEN BOLT HOLES

ADDED DIMENSION TO COLUMN FLANGE WIDTH FOR ALLOWABLE SPREAD OF SIDE PLATES (A)

a. ALL PLATE MATERIAL SHALL HAVE A MINIMUM YIELD STRENGTH (F_V) OF 50 KSI.

b. ANGLE AND BAR MATERIAL SHALL HAVE A HIGH STRENGTH STEEL SPECIFICATION AND SHALL HAVE A MINIMUM YIELD STRENGTH (F_y) OF 50

HIGH STRENGTH BOLTS/FASTENERS:

a. BOLTS SHALL BE TYPE 1 OR TYPE 3 AND SHALL BE A490 HEAVY HEX, F2280 TWIST-OFF-TYPE TENSION-CONTROL BOLT ASSEMBLIES, OR F3148 FIXED SPLINE BOLT ASSEMBLIES. THE BOLT HEAD SHALL BE DISTINCTIVELY MARKED WITH A MINIMUM MARKING OF A490, A490TC, OR 144 RESPECTIVELY. AN ALTERNATIVE DESIGN THAT MEETS THE REQUIREMENTS OF RCSC SECTION 2.8 MAY BE USED, WITH THE WRITTEN APPROVAL FROM SIDEPLATE SYSTEMS, INC. b. WASHERS SHALL BE ORDINARY THICKNESS AND ASTM F436 TYPE 1 OR TYPE 3.

c. NUTS SHALL BE ASTM A563 GRADE DH OR DH3. d. THE BOLT ASSEMBLY SHALL BE COVERED IN A LIGHT PROTECTIVE OIL. F2280 AND F3148 ASSEMBLIES SHALL ONLY BE LUBRICATED BY THE

e. THE MILL TEST REPORT (MTR) MUST HAVE DOCUMENTED LOT TRACEABILITY, STATEMENT OF DIMENSIONAL RESULTS, FULL CHEMICAL AND MECHANICAL TEST RESULTS TO THE SPECIFICATIONS ABOVE. THE USE OF FINGER SHIMS ARE ACCEPTABLE PER BOLTING SECTION 8.

a. ALL ROLLED SHAPES USED FOR COLUMNS AND BEAMS IN CONSTRUCTING SIDEPLATE MOMENT FRAMES SHALL BE ASTM A992 GRADE 50

4. <u>HSS TUBE SHAPES</u>:
a. ALL HSS SHAPES USED FOR COLUMNS AND BEAMS IN CONSTRUCTING SIDEPLATE MOMENT FRAMES SHALL, AS A MINIMUM, BE ASTM A500 GRADE B OR GRADE C OR ASTM1085.

PREPARATION

1. THE STEEL FABRICATION AND ERECTION SUBCONTRACTORS SHALL EMPLOY A DISTORTION CONTROL PROGRAM PRIOR TO THE START OF SIDEPLATE MOMENT FRAME FABRICATION. THE DISTORTION CONTROL PROGRAM SHALL BE IN ACCORDANCE WITH THE PROVISIONS OF AWS D1.1 SECTION 5.21 AND 5.22 TO ENSURE THAT THE FOLLOWING ARE MAINTAINED:

 DIMENSIONAL ACCURACY FRAMING AND ALIGNMENT TOLERANCES COMPLIANCE WITH AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES, SECTION 7.0, ERECTION PROVISIONS CONTROL OF DISTORTION AND WELD SHRINKAGE

1. WELDER QUALIFICATION: THE PERFORMANCE OF ALL WELDERS, WELDING OPERATORS AND TACK WELDERS SHALL BE QUALIFIED IN CONFORMANCE WITH AWS D1.1, SECTION 4, PART C TO DEMONSTRATE ABILITY TO PRODUCE SOUND WELDS.

1. BOLTS/FASTENERS SHALL BE INSTALLED TO PRETENSIONED CONDITION USING ONE OF THE METHODS PRESCRIBED HERE: TURN-OF-NUT (A490), CALIBRATED WRENCH (A490), TWIST-OFF-TYPE TENSION-CONTROL BOLT (F2280), OR TORQUE AND ANGLE METHOD(F3148). 2. FOR ALL PRETENSIONING METHODOLOGIES, ALL FASTENER ASSEMBLIES WITHIN THE JOINT SHALL FIRST BE BROUGHT TO A SNUG TIGHT CONDITION, FOLLOWED BY A SYSTEMATIC PRETENSIONING PROCESS, PRETENSIONING SHALL BEGIN AT THE MOST RIGID PART OF THE JOINT

AND CONTINUE IN A MANNER THAT WILL MINIMIZE THE RELAXATION OF PREVIOUSLY PRETENSIONED FASTENERS, UNTIL THE CONNECTED PLIES ARE IN AS FIRM CONTACT AS POSSIBLE.

- 3. REUSE OF A490, F2280, AND F3148 BOLT ASSEMBLIES SHALL NOT BE ALLOWED. TOUCHING UP OR RE-TIGHTENING BOLTS THAT MAY HAVE BEEN LOOSENED BY THE INSTALLATION OF ADJACENT BOLTS SHALL NOT BE CONSIDERED TO BE A REUSE.
- 4. ALL BOLT HOLES SHALL BE ALIGNED TO PERMIT INSERTION OF THE BOLTS WITHOUT UNDUE DAMAGE TO THE THREADS. 5. THE BOLT LENGTH USED SHALL BE SUCH THAT THE BOLT THREAD EXTENDS BEYOND OR IS AT LEAST FLUSH WITH THE OUTER FACE OF THE NUT WHEN PROPERLY INSTALLED.
- 6. FASTENER COMPONENTS SHALL BE PROTECTED FROM DIRT AND MOISTURE IN CLOSED CONTAINERS AT THE SITE OF INSTALLATION. 7. F2280 OR F3148 ASSEMBLIES AND ALTERNATIVE DESIGN FASTENERS THAT MEET THE SPECIFIED REQUIREMENTS PREVIOUSLY MENTIONED SHALL NOT BE RE-LUBRICATED, EXCEPT BY THE MANUFACTURER.
- 8. FINGER SHIMS MAY BE USED UP TO 1/4 INCH WITHOUT RESTRICTION, SHIM REQUIREMENTS GREATER THAN 1/4 INCH SHALL BE SUBMITTED TO SIDEPLATE SYSTEMS INC FOR APPROVAL PRIOR TO USE 9. WASHERS SHALL BE ASTM F436 ORDINARY THICKNESS AND SHALL BE USED UNDER THE NUT OF THE FASTENER ASSEMBLY SO AS TO PROVIDE A HARDENED NON-GALLING SURFACE OF THE TURNED ELEMENT. WHEN USING THE TURN-OF-NUT OR CALIBRATED WRENCH METHOD, THE

TURNED ELEMENT MUST BE THE SAME AS WAS USED WHEN PERFORMING PREINSTALLATION VERIFICATION TESTING.

1. THE FABRICATOR AND ERECTOR SHALL BE RESPONSIBLE FOR QUALITY CONTROL BY PROVIDING, AS A MINIMUM, IN-PROCESS VISUAL INSPECTION OF ALL FABRICATION AND ERECTION ACTIVITIES TO ENSURE THAT MATERIALS AND WORKMANSHIP MEET THE REQUIREMENTS OF THE CONTRACT DOCUMENTS, AND SHALL INCLUDE WORK PERFORMED PRIOR TO ASSEMBLY, SUCH WORK SHALL INCLUDE, BUT NOT BE LIMITED TO, VERIFYING THAT EFFECTIVE PROCEDURES AND METHODS HAVE BEEN EMPLOYED IN THE FORM OF A DISTORTION CONTROL PROGRAM TO ACCOUNT FOR AND COUNTERACT THE EFFECTS OF WELD SHRINKAGE, EXISTING BEAM SWEEP AND CAMBER, AND CHANGES IN MOMENT FRAME

GEOMETRY DUE TO SKEWED AND CURVED DESIGN CONFIGURATIONS (AS OCCURS), TO ENSURE COMPLIANCE WITH SPECIFIED ERECTION AND ALIGNMENT TOLERANCES. QC INSPECTION SHALL INCLUDE HOLD POINTS FOR THE FOLLOWING: a. COLUMN TREE ASSEMBL' VERIFICATION THAT ACTUAL COLUMN FLANGE WIDTH IS AT LEAST NOMINAL COLUMN FLANGE WIDTH WHERE THE SIDE PLATES {A} ARE

TO BE INSTALLED. IN THE UNLIKELY EVENT ACTUAL COLUMN FLANGE WIDTH IS LESS THAN NOMINAL, BUT WITHIN AISC STANDARD MILL TOLERANCES (-3/16 INCH MAX), CONTACT SIDEPLATE SYSTEMS, INC FOR APPROPRIATE RECOMMENDATIONS. 2. MINIMUM CLEAR DIMENSION SHALL BE VERIFIED AFTER PLACEMENT OF WELD {2}, COOLING OF WELD {2}, AND REMOVAL OF TEMPORARY SHOP CONSTRUCTION AID(S). VERIFY THAT A MINIMUM ACTUAL COLUMN FLANGE WIDTH DIMENSION OCCURS ANYWHERE IN BETWEEN THE SIDE PLATES {A} FROM TOP TO BOTTOM. THE SIDE PLATES SHALL BE PARALLEL TO ONE ANOTHER. IN NO CASE SHALL THEY BE LESS THAN THE ACTUAL COLUMN FLANGE WIDTH.

3. MAXIMUM SPREAD DIMENSION OF SIDE PLATE {A} SHALL NOT EXCEED ACTUAL COLUMN FLANGE WIDTH PLUS THE SCHEDULED SPREAD DIMENSION Y. THE FIELD CONSTRUCTION AID SHALL BE PLACED AND HOLD THE SIDE PLATES IN THIS FLARED CONDITION UNTIL THE BEAM HAS BEEN SAFELY ERECTED. IN NO CASE SHALL THE SPREAD CAUSE PERMANENT DEFORMATION IN THE SIDE PLATES. 4. VERIFICATION OF BOLT HOLE ELEVATION AND SPACING FOR POSITION OF SIDE PLATE (A) AND PROPER POSITION AND ELEVATION OF

BEAM ASSEMBL 1. VERIFICATION OF PERPENDICULAR ALIGNMENT BETWEEN THE TOP COVER PLATE (B) AND BOTTOM ANGLES (H) TO THE WEB OF THE BEAM, TO MINIMIZE, IF NOT ELIMINATE, ANY MISALIGNMENT OF BOLT HOLES DUE TO BEAM FLANGE TILT WHEN THE BEAM HAS BEEN

LOWERED INTO PLACE. 2. VERIFICATION OF BOLT HOLE SPACING AND POSITION ON COVER PLATE {B} AND ANGLES {H}. CONSIDERATION SHALL BE GIVEN TO THE CUPPING EFFECT OF THE TOP COVER PLATE {B}, DUE TO WELD SHRINKAGE. 3. VERIFICATION OF THE DISTANCE BETWEEN EXTERIOR ANGLE {H} FACES AND THEIR RESPECTIVE BOLT HOLE PLACEMENT TO EACH

OTHER (VERTICALLY AND HORIZONTALLY). 4. VERIFICATION THAT IN NO CASE SHALL THE OUTSIDE FACE OF VSE {F} EXTEND BEYOND THE OUTSIDE FACES OF THE LONGITUDINAL 5. VERIFICATION THAT VERTICAL PLACEMENT OF VSE {F} IS IN THE CORRECT LOCATION.

a. THE PARTS TO BE JOINED BY FILLET WELDS SHALL BE BROUGHT INTO AS CLOSE CONTACT AS PRACTICABLE, USING AS NECESSARY SUITABLE CLAMPING MEANS. THE ROOT OPENING (I.E., THE FIT-UP GAP) SHALL NOT EXCEED 1/4 INCH. FOR FILLET WELD ROOT GAPS GREATER THAN 1/16 INCH, THE LEG SIZE (I.E., THE SPECIFIED SIZE) OF FILLET WELD SHALL BE INCREASED BY THE AMOUNT OF THE ROOT OPENING. THERMAL CUTTING

a. THE ROUGHNESS OF ALL THERMAL-CUT SURFACES SHALL BE NO GREATER THAN AN ANSI SURFACE ROUGHNESS VALUE OF 1000 MICRO-INCHES. ROUGHNESS EXCEEDING THIS VALUE AND NOTCHES OR GOUGES NOT MORE THAN 3/16 INCH DEEP SHALL BE REMOVED BY MACHINING OR GRINDING. NOTCHES OR GOUGES IN THE THERMALLY CUT EDGES DEEPER THAN 3/16 INCH SHALL BE REPAIRED PER AWS.

4. TENSION CALIBRATION FOR PRE-INSTALLATION:
a. TENSION CALIBRATION SHALL BE USED TO CONFIRM THE SUITABILITY OF THE COMPLETE FASTENER ASSEMBLY, AND THE PROCEDURE TO BE USED BY THE BOLTING CREW.

QUALITY ASSURANCE

IN ADDITION TO ALL OTHER QUALITY ASSURANCE INSPECTION ACTIVITIES, THE OWNER'S VERIFICATION INSPECTOR SHALL BE RESPONSIBLE FOR: a. TO ASSURE THE PROPER AMPERAGE AND VOLTAGE OF THE WELDING PROCESS, THE USE OF HAND HELD CALIBRATED AMP AND VOLT METERS SHALL BE USED. THIS EQUIPMENT SHALL BE USED BY THE FABRICATOR AND THE INSPECTOR. AMPERAGE AND VOLTAGE SHALL BE MEASURED NEAR THE ARC. TRAVEL SPEED AND ELECTRODE STICK OUT SHALL BE VERIFIED TO BE IN COMPLIANCE WITH THE APPROVED

VISUAL INSPECTION SHALL BE PERFORMED ON ALL SHOP WELDS. c. EACH WELDER EMPLOYED ON THE PROJECT SHALL UNDERSTAND ALL THE REQUIREMENTS OF THE WELDING PROCEDURE

SPECIFICATION(S) BEFORE WELDING ON THE PROJECT. d. AS-BUILT BEAM TO COLUMN GAP PER CONNECTION SCHEDULE IS ALLOWED TO BE INSTALLED WITH A TOLERANCE OF PLUS OR MINUS 1/2

a. THE SURFACES ADJACENT TO THE BOLT HEAD AND NUT SHALL BE FREE OF DIRT AND OTHER FOREIGN MATERIAL OTHER THAN THE b. FAYING SURFACES ARE PERMITTED TO BE UNCOATED AND COATED WITH ANY COATINGS OF ANY FORMULATION OR GALVANIZATION. c. AFTER THE CONNECTIONS HAVE BEEN ASSEMBLED, VISUALLY ENSURE THAT THE PLIES OF THE CONNECTED ELEMENTS HAVE BEEN BROUGHT INTO AS CLOSE OF CONTACT AS PRACTICABLE WITH ONE ANOTHER. GAPS UP TO 1/8 INCH BETWEEN THE SURFACES SHALL BE ALLOWED. GAPS GREATER THAN 1/8 INCH UP TO 1/4 INCH SHALL HAVE FINGER SHIMS INSTALLED BEFORE PRETENSIONING. FOR GAPS GREATER THAN 1/4 INCH, CONTACT SIDEPLATE SYSTEMS, INC

SIDEPLATE CONNECTIONS REQUIRING THIS TYPE OF FINISH SHALL FOLLOW THE SAME CONSTRUCTION SEQUENCING AS PREVIOUSLY OUTLINED WITH THE FOLLOWING MODIFICATIONS: a. HORIZONTAL SHEAR PLATES {D} SHALL HAVE AN INCREASED CLIP SIZE WHICH SHALL BE 1 5/8 INCH BY 1 5/8 INCH TO PROVIDE ADEQUATE

VENTILATION AND DRAINAGE. CONTACT SIDEPLATE SYSTEMS, INC. IN THE EVENT THAT THE GALVANIZING CONTRACTOR SPECIFICATIONS

REQUIRE A LARGER OPENING THAN THAT SPECIFIED HEREIN. SEAL WELDING SHALL BE ALLOWED ON THE PLATES (B) AND ANGLES. c. ANY DEVIATIONS TO THESE MODIFICATIONS SHALL BE COORDINATED WITH SIDEPLATE SYSTEMS, INC. AND THE SEOR.

1. WHEN REQUIRED BY THE GOVERNING CODE FOR CERTAIN TYPES OF CONSTRUCTION, SIDEPLATE CONNECTIONS SHALL HAVE A FIRE-RESISTANCE RATING LIKE THAT OF A STEEL "STRUCTURAL FRAME". 2. THE MINIMUM THICKNESS OF SPRAY-APPLIED FIRE-RESISTIVE MATERIAL (SFRM) FOR STEEL SIDEPLATE CONNECTIONS PLATES THAT ARE NOT ENCASED IN CONCRETE, SHALL BE DETERMINED JUST LIKE THAT OF A PIPE/TUBE COLUMN SECTION WITH A CONSTANT STEEL WALL THICKNESS USING THE THICKNESS OF SIDE PLATE {A} FOR EACH SIDEPLATE CONNECTION ID PER THE SIDEPLATE CONNECTION SCHEDULE, WHICH ARE UNIFORMLY HEATED AND PROTECTED (THE FIRE EXPOSURE OF A PIPE/TUBE COLUMN IS DIRECTLY ANALOGOUS TO A PLATE WITH A 1-SIDED

FIRE EXPOSURE AND PROTECTION). THE SFRM SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ASTM E119 AND LISTED FOR FIRE RESISTIVE

PIPE/TUBE COLUMN APPLICATIONS FOR NO LESS THAN THE REQUIRED RATED TIME. 3. AS REQUIRED, WHEN NO VERTICAL SHEAR ELEMENT $\{F\}$ EXISTS IN THE BEAM, SPRAY THE MINIMUM THICKNESS OF SFRM BETWEEN INSIDE OF SIDE PLATE (A) AND BEAM WEB COVERING ALL SURFACES INCLUDING COLUMN FLANGE. NOTE: THIS DOES NOT NECESSITATE FILLING THE

4. WHEN VERTICAL SHEAR ELEMENT {F} IS USED, THE CONTRACTOR SHALL PROVIDE THE MEANS, TYPICALLY DONE WITH A LAYERING TECHNIQUE, FOR FIREPROOFING ACROSS THE BOTTOM OF THE GAP. 5. SEE GRAPHIC NUMBER 10 IN FIELD ERECTION OF THE SIDEPLATE BOLTED SYSTEM FOR FIREPROOFING ACROSS THE BOTTOM OF THE GAP.

1. IN ORDER TO SAFEGUARD THE AUTHORIZED USE AND INTELLECTUAL PROPERTY OF THE PATENTED SIDEPLATE CONNECTION TECHNOLOGY, THE STEEL FABRICATION SUBCONTRACTOR SHALL SATISFY THE FOLLOWING REQUIREMENTS: a. A NOTICE OF INTELLECTUAL PROPERTY, IDENTICAL TO THAT PROVIDED ON THIS SHEET, SHALL BE AFFIXED ON EACH SHEET OF SHOP DETAIL AND FIELD ERECTION DRAWINGS CONTAINING SIDEPLATE SYSTEM INFORMATION WHICH DISCLOSES IN ANY WAY THE SIDEPLATE CONNECTION CONCEPT PRIOR TO RELEASING SUCH INFORMATION FOR ITS INTENDED USE. SUCH NOTICE SHALL BE PROVIDED TO THE STEEL FABRICATION SUBCONTRACTOR BY SIDEPLATE SYSTEMS, INC. IN A FORMAT (E.G. WORD OR AUTOCAD) SUITABLE TO THE NEEDS OF THE STEEL FABRICATION SUBCONTRACTOR'S DETAILER.

CONNECTION AND ON ONE END OF THE BEAM WEB IN COMPLIANCE WITH THE PATENT AND INTELLECTUAL PROPERTY LAWS. 1. THE CONTRACTOR SHALL ASSUME FULL AND COMPLETE RESPONSIBILITY FOR THE MEANS AND METHODS OF CONSTRUCTING THE STEEL FRAME USING THE SIDEPLATE BOLTED SYSTEM. CONSTRUCTION MEANS AND METHODS SHALL BE COMPLIANT WITH THE CURRENT PROVISIONS

b. PATENT LABELS SHALL BE APPLIED ON THE OUTSIDE FACE OF ONE OF THE TWO BOTTOM HORIZONTAL SHEAR PLATES {D} OF EACH MOMENT

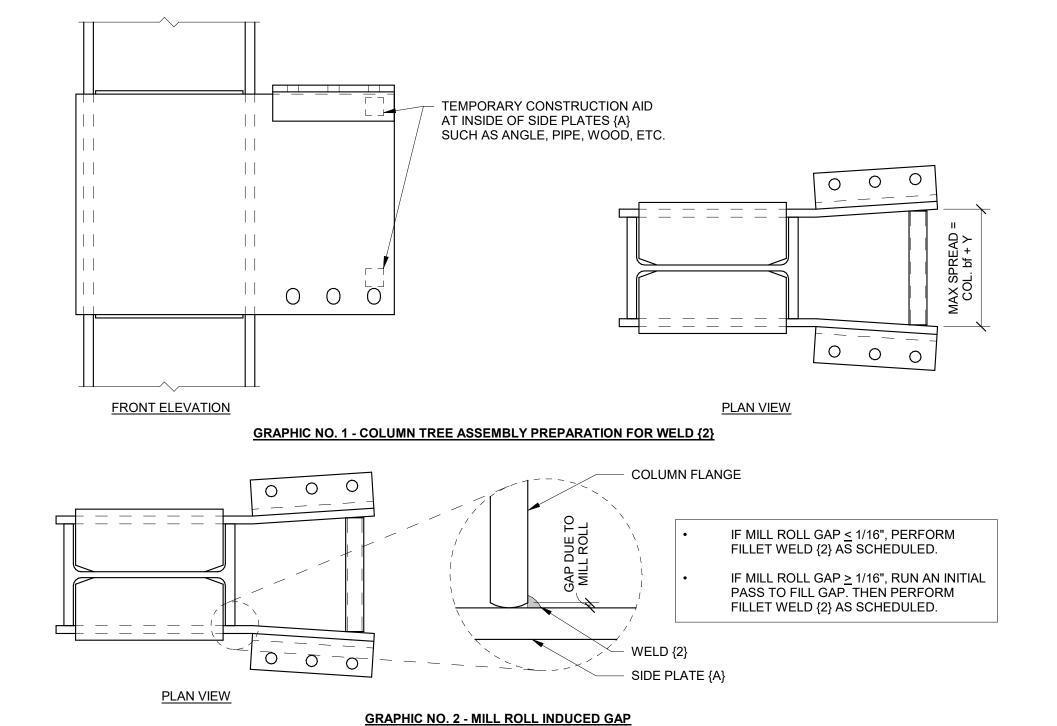
OF AWS D1.1, THE AISC 360 CODE OF STANDARD PRACTICE, THE RCSC HIGH-STRENGTH BOLTING SPECIFICATIONS, AND THE CONSTRUCTION GUIDELINES PROVIDED HEREIN AND SHALL INCLUDE, BUT ARE NOT LIMITED TO: a. DIMENSIONAL VERIFICATION AND CONTROL FABRICATION AND ERECTION PROCEDURES (INCLUDING METHODS FOR CONTROLLING DISTORTION DUE TO WELD SHRINKAGE, AND FOR CONTROLLING COMBINED MILL, FABRICATION AND ERECTION TOLERANCES)

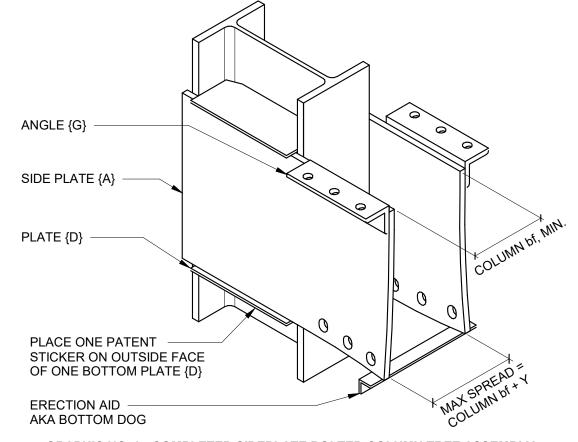
CONSTRUCTION AIDS SUCH AS ERECTION RIGGING AND SHORING d. PROPER BOLT HOLE ALIGNMENT PROPER PRETENSIONING OF BOLTS 2. THE SEQUENCE OF CONSTRUCTION OPTIONS PROVIDED BELOW IN THESE CONSTRUCTION GUIDELINES HAVE PROVEN TO BE SUCCESSFUL BY

STEEL FABRICATORS AND ERECTORS TO COST EFFICIENTLY CONSTRUCT THE BOLTED SIDEPLATE CONNECTION SYSTEM. VARIATIONS TO THESE CONSTRUCTION SEQUENCE OPTIONS PROVIDED BELOW SHALL BE SUBMITTED FOR REVIEW AND DISPOSITION TO SIDEPLATE SYSTEMS, INC. 3. A PRE-FABRICATION COORDINATION MEETING WITH A SIDEPLATE SYSTEMS, INC. REPRESENTATIVE IS REQUIRED FOR ALL PROJECTS. THE PRE-FABRICATION COORDINATION MEETING IS INTENDED TO SHARE BEST PRACTICES AND COMMON MISTAKES TO AVOID.

SHOP FABRICATION OF THE SIDEPLATE BOLTED SYSTEM

https://portal.sideplate.com/account/login

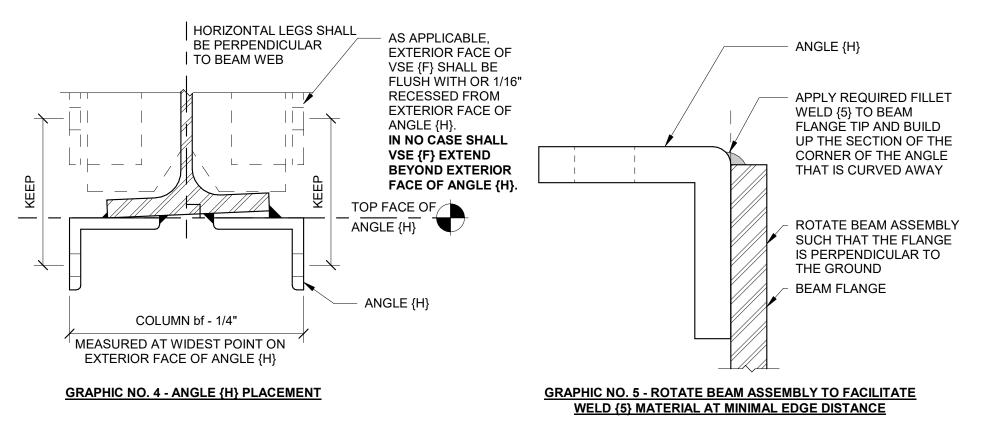




GRAPHIC NO. 3 - COMPLETED SIDEPLATE BOLTED COLUMN TREE ASSEMBLY

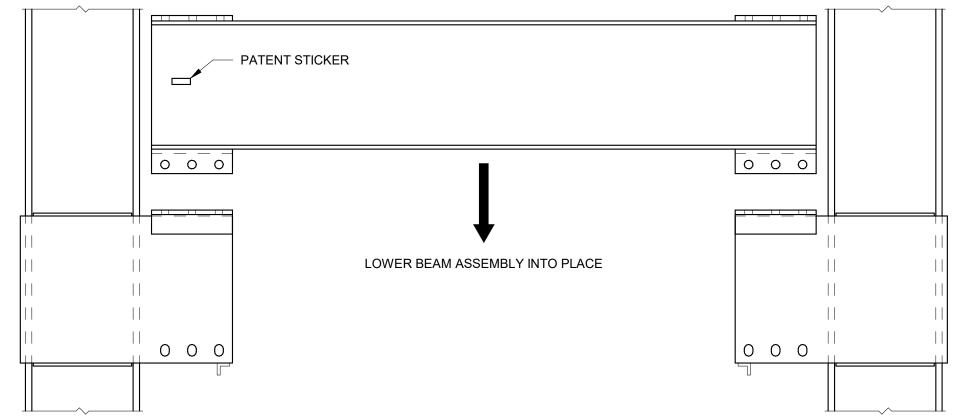
2. WATCH OUR SIDEPLATE BEAM ASSEMBLY VIDEO AT

https://portal.sideplate.com/account/login

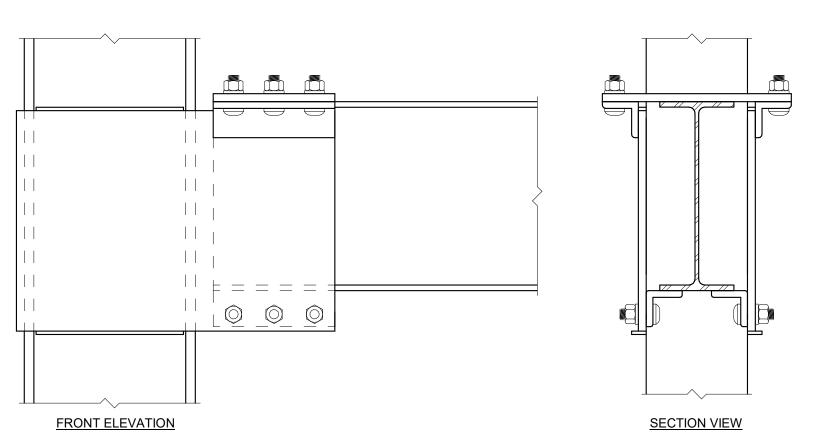


FIELD ERECTION OF SIDEPLATE BOLTED SYSTEM

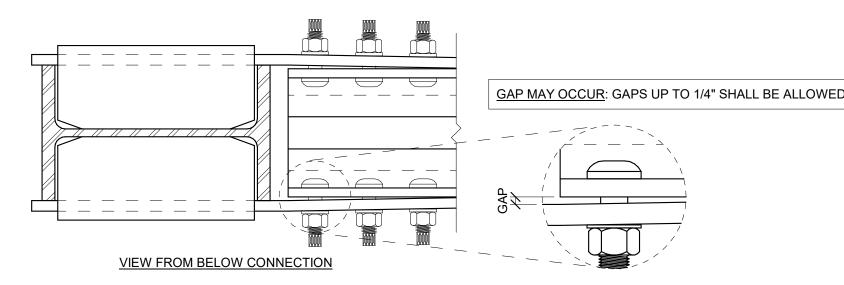
https://portal.sideplate.com/account/login



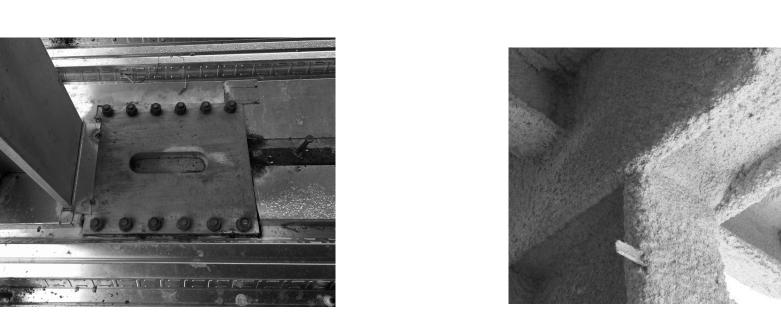
GRAPHIC NO. 6 - FIELD ERECTION OF SIDEPLATE BEAM ASSEMBLY



GRAPHIC NO. 7 - COMPLETED SIDEPLATE BOLTED CONNECTION



GRAPHIC NO. 8 - SNUG TIGHT CONDITION PRIOR TO PRETENSIONING BOLTS



GRAPHIC NO. 9 - TYPICAL GAP CLOSURE AT THE TOP OF THE GAP

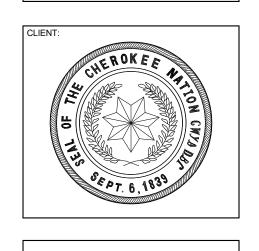
GRAPHIC NO. 10 - FIREPROOFING ACROSS THE BOTTOM OF THE GAP

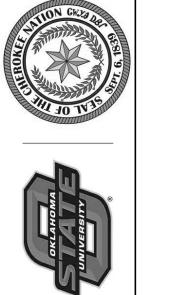


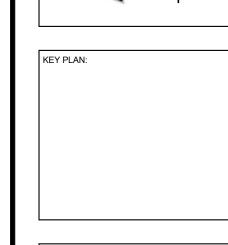


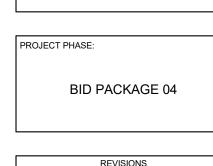


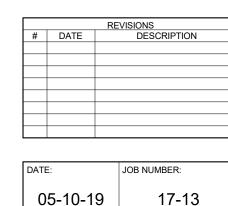
consulting engineers, inc. 4700 Lincoln Road NE, Suite 102 Albuquerque, NM 87109 505-344-4080 · 505-343-8759 (fax)









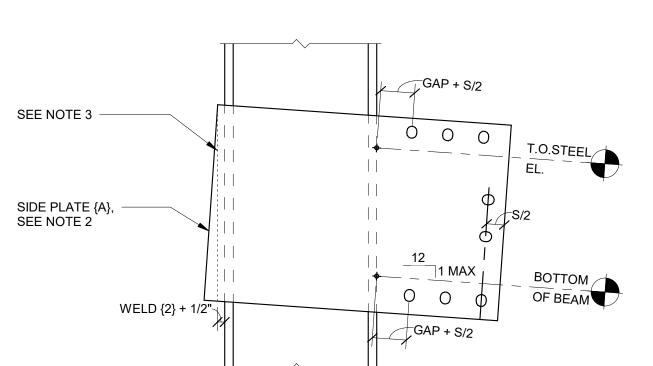


SHEET NUMBER: SIDEPLATE GENERAL

NOTES AND CONSTRUCTION **GUIDELINES**

INTELLECTUAL PROPERTY RIGHTS NOTICE The SIDEPLATE® steel frame connection system is covered by one or more of U.S. Pat. Nos. 6,138,427; 6,516,583; 6,591,573; 7,178,296; 8,122,671; 8,122,672; 8,146,322; 8,176,706; 8,205,408; and 9,091,065 and foreign counterparts. Other U.S. and foreign applications pending.

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BOLT

2 2 1/8 4 1/2

2 2 1/8 4 1/2 3 2 1/8 4 1/2 3 2 1/8 4 1/2

DIAMETER HORIZONTAL VERTICAL G S

SIDE PLATE {A} EXTENSION DESIGN (INCHES)

31 3/4 | 1 3/8 | 2 1/2 | 1 1/8

44 | 1 3/8 | 4 3/8 | 1 1/8

THICKNESS B E Y

WELD {2} + 1/2" 0 0 01 SEE NOTE 3 SIDE PLATE {A} 1 MAX OF BEAM GAP + S/2

FRONT ELEVATION NOTE(S):
1. FOR BEAM SLOPES > 1" PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC. 2. COORDINATE PLATES, ANGLES, AND DIMENSIONS WITH RESPECT TO THE SLOPE OF THE CONNECTION. 3. AT CONTRACTOR'S DISCRETION, SIDE PLATE (A) MAY BE CUT AS SHOWN.

8 SLOPED UP CONNECTION (AS APPLICABLE)
N.T.S.

-SEE NOTE 3 SIDE PLATE {A}, SEE NOTE 2 12 1 MAX 9 0 0 WELD {2} + 1. GAP + S/2

FRONT ELEVATION NOTE(S):
1. FOR BEAM SLOPES > 1" PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC. 2. COORDINATE PLATES, ANGLES, AND DIMENSIONS WITH RESPECT TO THE SLOPE OF THE CONNECTION. 3. AT CONTRACTOR'S DISCRETION, SIDE PLATE (A) MAY BE CUT AS SHOWN.

4 SLOPED DOWN CONNECTION (AS APPLICABLE)
N.T.S.

WELD {2} + 1/2"-SEE NOTE 3 SIDE PLATE {A} -GAP + S/2

FRONT ELEVATION

NOTE(S):
1. FOR BEAM SLOPES > 1" PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC. 2. COORDINATE PLATES, ANGLES, AND DIMENSIONS WITH RESPECT TO THE SLOPE OF THE CONNECTION. 3. AT CONTRACTOR'S DISCRETION, SIDE PLATE (A) MAY BE CUT AS SHOWN.

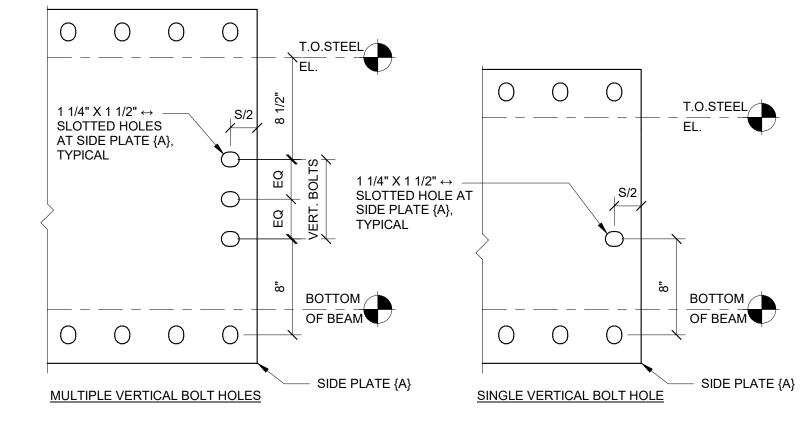
NOTE(S):
1. FOR BEAM SLOPES > 1" PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC.
2. COORDINATE PLATES, ANGLES, AND DIMENSIONS WITH RESPECT TO THE SLOPE OF THE CONNECTION. 3. AT CONTRACTOR'S DISCRETION, SIDE PLATE (A) MAY BE CUT AS SHOWN.

FRONT ELEVATION

9 SLOPED DOWN CONNECTION (AS APPLICABLE)
N.T.S.

COLUMN PANEL ZONE DESIGN (INCHES)

A25



NOTE(S):

1. SEE COLUMN SCHEDULE FOR BOLT QUANTITY.

	COLUMN PANEL ZO	NE DESI	GN (INCHE	S)				SIDE PLATE (A)	EXTENSION D	ESIGN (INC	HES)							
COLUMN ID SERIES	COLUMN	WELD	BEAM	1	PL	ATE			ANGLE		WELD		BOLT					
	{2}	SHAPE	GAP	{/	A}			{G}		{8}	DIAMETER	HORIZONTAL	G	s				
			OLIVIES	SIZE		SHAPE	GAP	THICKNESS	В	Y	SUGGESTED SIZE	HORIZONTAL LEG	VERTICAL LEG	SIZE	DIAIVIETER	#	G	3
A10	W24x	3/8	W24X68	2	5/8	27 1/4	2 1/2	L5X3-1/2X5/8	3-1/2 to 6	4 to 6	5/16	1 1/8	4	2 1/8	4 1/			
A20, A29	W24x	3/8	W24X76	2	5/8	27 3/8	2 1/2	L5X3-1/2X5/8	3-1/2 to 6	4 to 6	5/16	1 1/8	4	2 1/8	4 1/			
A40	W24x	3/8	W36X160	2	3/4	39 1/2	4 3/8	L5X3-1/2X5/8	3-1/2 to 6	4 to 6	5/16	1 1/8	6	2 1/8	4 1			

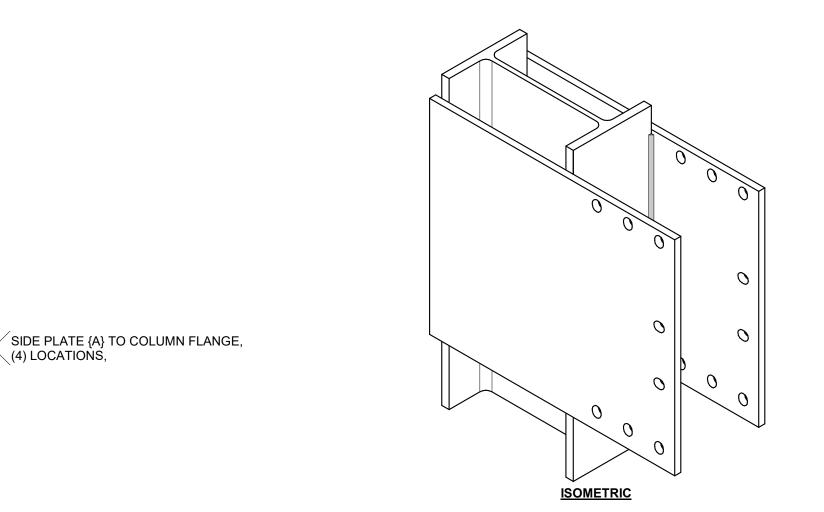
6 A TYPE NARROW COLUMN CONNECTION SCHEDULE N.T.S.

PLAN VIEW

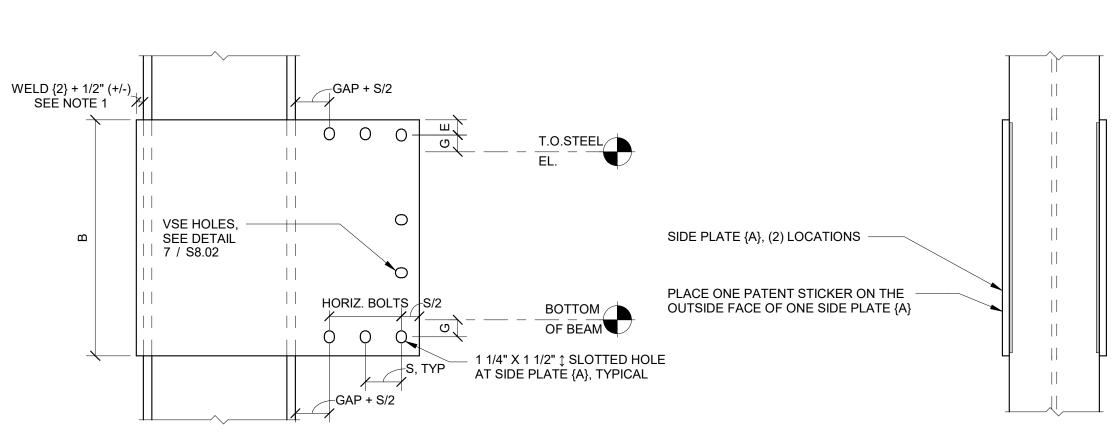
FRONT ELEVATION

7 SIDE PLATE {A} VSE BOLT HOLE DETAIL N.T.S.

2 A TYPE COLUMN CONNECTION SCHEDULE N.T.S.

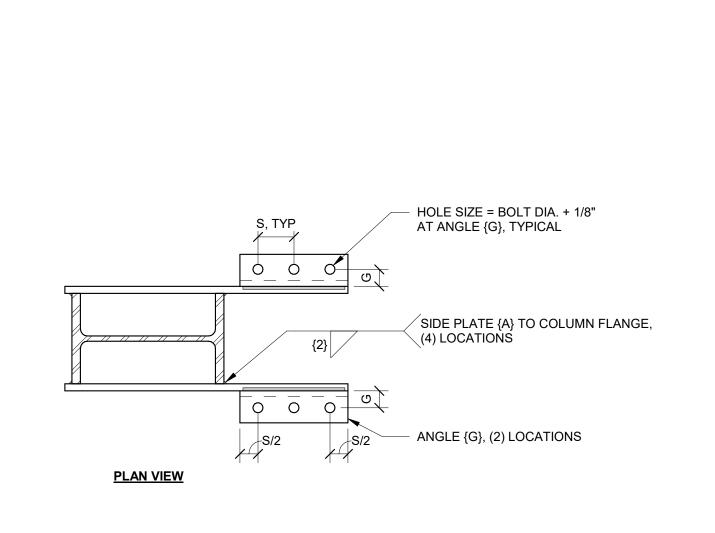


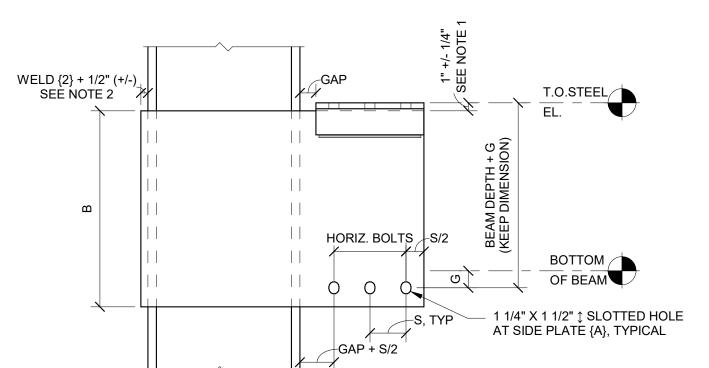
SIDE ELEVATION



NOTE(S):

1. THE 1/2 INCH OVERHANG ON THE SIDE PLATE {A} IS TO ENSURE SUFFICIENT ROOM FOR WELD {2}, THE +/- TOLERANCE IS APPLIED SO THAT IF DESIRED, THE DETAILER CAN MAKE THE SIDE PLATES {A} THE SAME LENGTH WITH SLIGHTLY VARYING COLUMN DEPTHS WITHIN A GROUP OF THE SAME CONNECTION ID'S.





FRONT ELEVATION

ANGLE (G) TO SIDE PLATE {A}, (2) LOCATIONS SIDE PLATE {A}, (2) LOCATIONS -

COORDINATE ANGLE {G}
ORIENTATION WITH DECK SUPPORT AS NEEDED PLACE ONE PATENT STICKER ON THE OUTSIDE FACE OF ONE SIDE PLATE {A}

SIDE ELEVATION

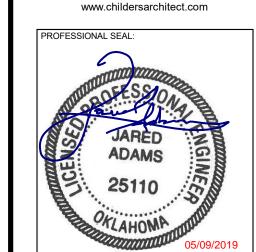
NOTE(S):

1. THE +/- 1/4 INCH TOLERANCE FOR PLACEMENT OF ANGLES {G} IS TO ENSURE CORRECT TOP OF STEEL PLACEMENT RELATIVE TO THE CENTERLINE OF THE BOTTOM HORIZONTAL ROW OF BOLT HOLES. THE PLACEMENT OF ANGLES {G} SHALL NEVER BE MEASURED FROM THE BOTTOM EDGE OF SIDE PLATE {A} TO ESTABLISH THE CORRECT TOP OF STEEL.

THE STATE OF THE SIDE BY ATE (A) IS TO ENSURE CUEFICIENT BOOM FOR WELD (2). THE +/- TOLERANCE IS APPLIED SO THAT IF DESIRED. THE DETAILER CAN MAKE THE SIDE PLATES {A} THE SAME LENGTH. 2. THE 1/2 INCH OVERHANG ON THE SIDE PLATE {A} IS TO ENSURE SUFFICIENT ROOM FOR WELD {2}, THE +/- TOLERANCE IS APPLIED SO THAT IF DESIRED, THE DETAILER CAN MAKE THE SIDE PLATES {A} THE SAME LENGTH WITH SLIGHTLY VARYING COLUMN DEPTHS WITHIN A GROUP OF THE SAME CONNECTION ID'S.

James R. Childers Architect, Inc.

> 45 South 4th Street Fort Smith, AR 72901 479-783-2480



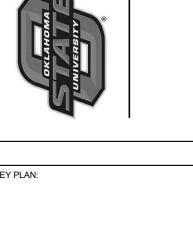


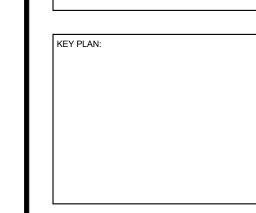












	PROJECT PHASE:
	BID PACKAGE 04

		REVISIONS
#	DATE	DESCRIPTION

DATE:	JOB NUMBER:
05-10-19	17-13
SHEET NUMBER:	
S	8.02

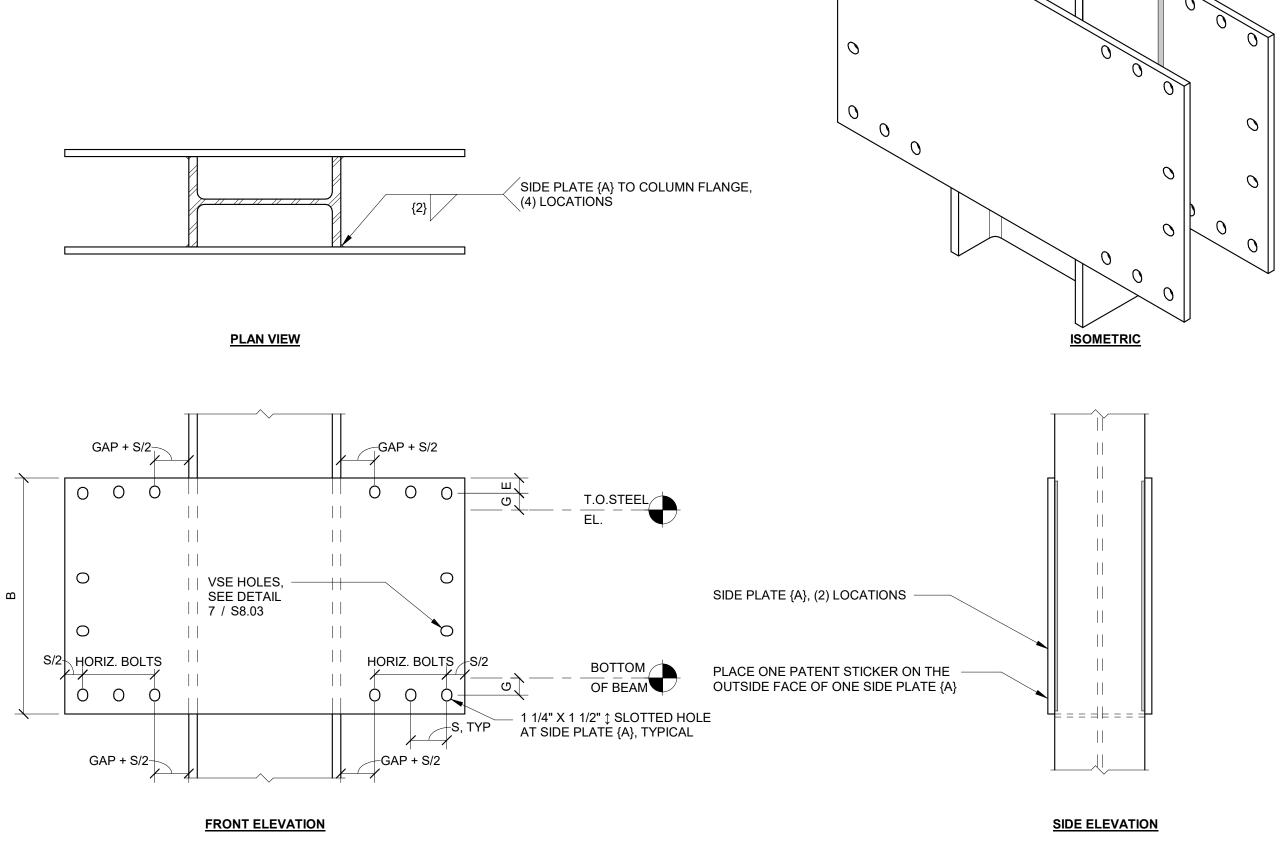
SIDEPLATE COLUMN DETAILS, A TYPE NO PLATE D



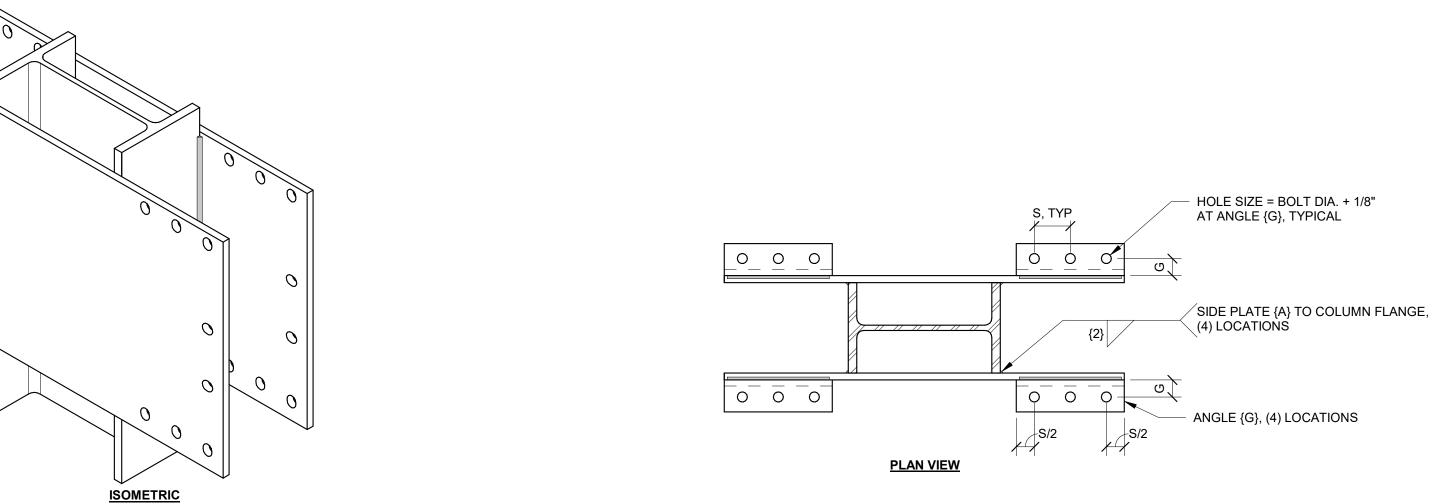
1 B TYPE BOLTED CONNECTION N.T.S.

NOTE(S):

1. THE +/- 1/4 INCH TOLERANCE FOR PLACEMENT OF ANGLES {G} IS TO ENSURE CORRECT TOP OF STEEL PLACEMENT RELATIVE TO THE CENTERLINE OF THE BOTTOM HORIZONTAL ROW OF BOLT HOLES. THE PLACEMENT OF ANGLES {G} SHALL NEVER BE MEASURED FROM THE BOTTOM EDGE OF SIDE PLATE {A} TO ESTABLISH THE CORRECT TOP OF STEEL.

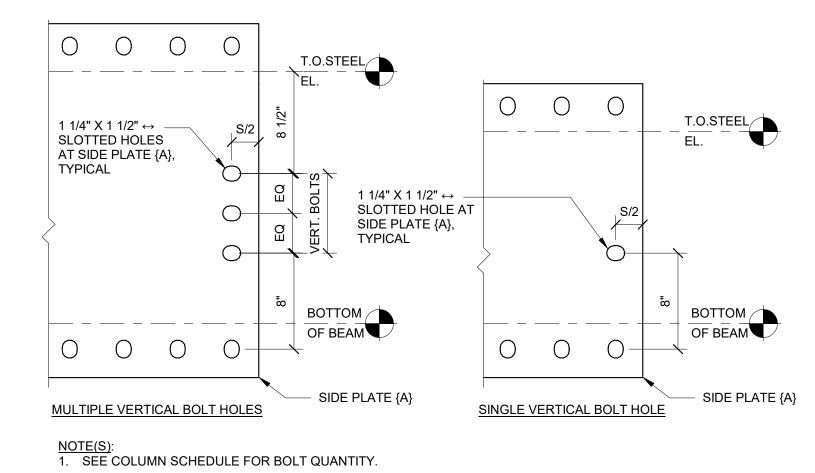


O O O PLA	ANGLE {G}, (4) LOCATIONS S/2 N VIEW	IS ISOMETRIC
GAP S/2 HORIZ. ROWS O O O	GAP T.O.STEEL FL. (NOISNAME AND	ANGLE (G) TO SIDE PLATE (A), (4) LOCATIONS SIDE PLATE (A), (2) LOCATIONS PLACE ONE PATENT STICKER ON THE OUTSIDE FACE OF ONE SIDE PLATE (A) HOLE CAL
FRONT I	ELEVATION	SIDE ELEVATION



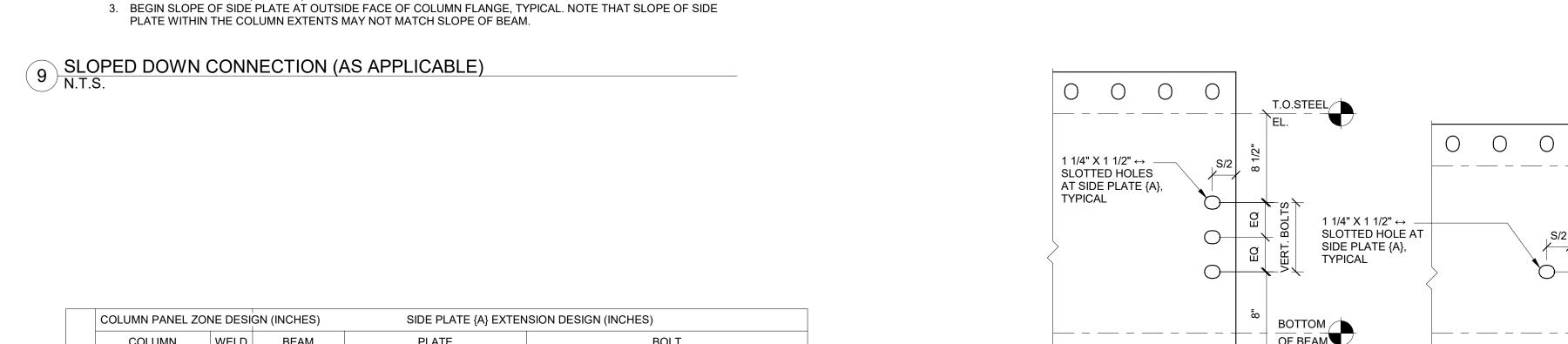
2 B TYPE COLUMN CONNECTION SCHEDULE N.T.S.

	COLUMN PANEL ZONE DESIGN (INCHES) SIDE PLATE {A} EXTENSION DESIGN (INCHES)												
	COLUMN	WELD	BEAN	1		PLATE BOLT							
ID	SERIES	SERIES {2}		GAP	{A}				DIAMETER	HORIZONTAL #	VERTICAL #	G	S
		SIZE			THICKNESS	В	Е	Υ		#	#		
B15	W24x	7/16	W24X68	2	3/4	31 3/4	1 3/8	2 1/8	1 1/8	4	2	2 1/8	4 1/2
B25	W24x	3/8	W24X76	2	5/8	31 7/8	1 3/8	2 1/2	1 1/8	4	2	2 1/8	4 1/2
B35	W24x	7/16	W33X141	2	3/4	41 1/4	1 3/8	3 1/8	1 1/8	5	3	2 1/8	4 1/2
B45	W24x	7/16	W36X160	2	3/4	44	1 3/8	4 3/8	1 1/8	6	3	2 1/8	4 1/2



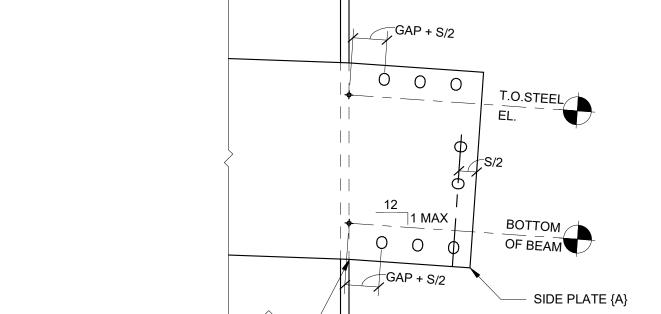
	COLUMN PANEL ZO	NE DESI	GN (INCHE	HES)		SIDE PLATE {A} EXTENSION DESIGN (INCHES)									
	COLUMN	WELD	BEAN	1	PLA	ATE		ANGLE		WELD		BOLT			
ID	SERIES	{2}	SHAPE	GAP	{/	4}			{G}		{8}	DIAMETER	HORIZONTAL	G	S
	SERIES	SIZE	SHAPE	GAP	THICKNESS	В	Y	SUGGESTED SIZE	HORIZONTAL LEG	VERTICAL LEG	SIZE	DIAIVIETER	#	G	3
B10	W24x	3/8	W24X68	2	5/8	27 1/4	2 1/2	L5X3-1/2X5/8	3-1/2 to 6	4 to 6	5/16	1 1/8	4	2 1/8	4 1/2
B20, B29	W24x	3/8	W24X76	2	5/8	27 3/8	2 1/2	L5X3-1/2X5/8	3-1/2 to 6	4 to 6	5/16	1 1/8	4	2 1/8	4 1/2
B40	W24x	7/16	W36X160	2	3/4	39 1/2	4 3/8	L5X3-1/2X5/8	3-1/2 to 6	4 to 6	5/16	1 1/8	6	2 1/8	4 1/2

COLUMN PANEL 2	ZONE DESIG	GN (INCHES)	SIDE PLATE {A} EXTEN	SION DESIGN (INCHES)		
COLUMN PANEL 2	ZONE DESIG	GN (INCHES) BEAM	SIDE PLATE {A} EXTEN PLATE	SION DESIGN (INCHES) BOL	.T	









FRONT ELEVATION

NOTE(S):

1. FOR BEAM SLOPES > 1" PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC.

2. COORDINATE PLATES, ANGLES, AND DIMENSIONS WITH RESPECT TO THE SLOPE OF THE CONNECTION.

SEE NOTE 3 -

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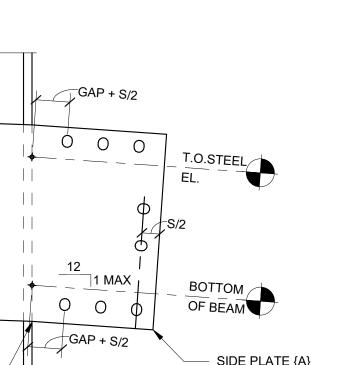
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v15.06.00 ₹





	FOR BEAM SLOPES > 1" PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC.
	COORDINATE PLATES, ANGLES, AND DIMENSIONS WITH RESPECT TO THE SLOPE OF
3.	BEGIN SLOPE OF SIDE PLATE AT OUTSIDE FACE OF COLUMN FLANGE, TYPICAL. NO
	PLATE WITHIN THE COLUMN EXTENTS MAY NOT MATCH SLOPE OF BEAM.
SI OPF	D UP CONNECTION (AS APPLICABLE)
(8) N.T.S.	B CT COTTICIT (7 to 7 ti 1 LIO7 tBLL)
₩. I . S.	

	FRONT ELEVATION
1. 2.	TE(S): FOR BEAM SLOPES > 1" PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC. COORDINATE PLATES, ANGLES, AND DIMENSIONS WITH RESPECT TO THE SLOPE OF THE CONNECTION. BEGIN SLOPE OF SIDE PLATE AT OUTSIDE FACE OF COLUMN FLANGE, TYPICAL. NOTE THAT SLOPE OF SIDE PLATE WITHIN THE COLUMN EXTENTS MAY NOT MATCH SLOPE OF BEAM.

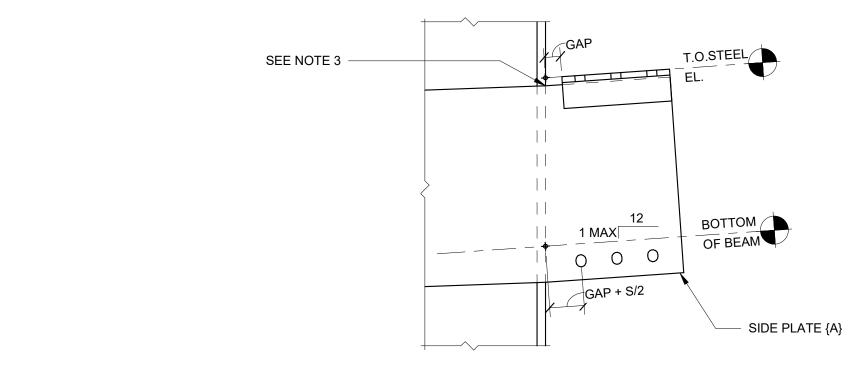
7 SIDE PLATE {A} VSE BOLT HOLE DETAIL N.T.S.

EE NOTE 3	GAP + S/2 O O T.O.STEEL EL.
	S/2 1 MAX
	GAP + S/2 SIDE PLATE {A}

	FRONT ELEVATION
1. 2.	TE(S): FOR BEAM SLOPES > 1" PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC. COORDINATE PLATES, ANGLES, AND DIMENSIONS WITH RESPECT TO THE SLOPE OF THE CONNECTION. BEGIN SLOPE OF SIDE PLATE AT OUTSIDE FACE OF COLUMN FLANGE, TYPICAL. NOTE THAT SLOPE OF SIDE PLATE WITHIN THE COLUMN EXTENTS MAY NOT MATCH SLOPE OF BEAM.

4 SLOPED DOWN CONNECTION (AS APPLICABLE)
N.T.S.

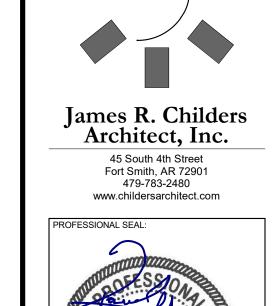
•	T.O.STEEL EL.
SEE NOTE 3	GAP + S/2 BOTTOM OF BEAM GAP + S/2 SIDE PLATE {A}



FRONT ELEVATION

NOTE(S):
1. FOR BEAM SLOPES > 1" PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC.
2. COORDINATE PLATES, ANGLES, AND DIMENSIONS WITH RESPECT TO THE SLOPE OF THE CONNECTION.

3. BEGIN SLOPE OF SIDE PLATE AT OUTSIDE FACE OF COLUMN FLANGE, TYPICAL. NOTE THAT SLOPE OF SIDE PLATE WITHIN THE COLUMN EXTENTS MAY NOT MATCH SLOPE OF BEAM.



CONSULTANT LOGO:

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505-344-4080 · 505-343-8759 (fax)

Medicir

Leopathic E CHEROKEE NATION

PROJECT PHASE:

05-10-19

SHEET NUMBER:

BID PACKAGE 04

S8.03

SIDEPLATE COLUMN

DETAILS, B TYPE NO PLATE D



5 SLOPED DOWN CONNECTION (AS APPLICABLE) N.T.S.

INTELLECTUAL PROPERTY RIGHTS NOTICE The SIDEPLATE® steel frame connection system is covered by one or more of U.S. Pat. Nos. 6,138,427; 6,516,583; 6,591,573; 7,178,296; 8,122,671; 8,122,672; 8,146,322; 8,176,706; 8,205,408; and 9,091,065 and foreign counterparts.

Other U.S. and foreign applications pending.

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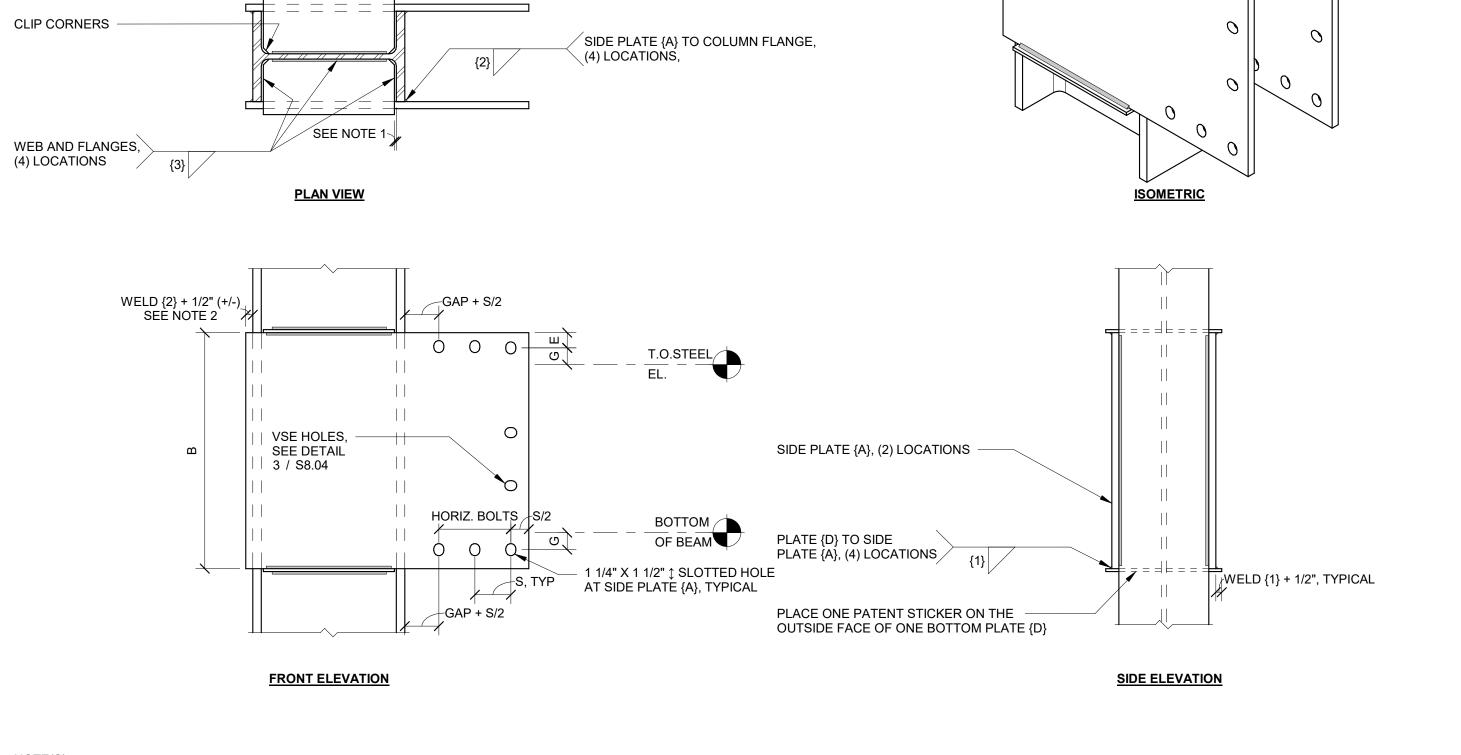
> NOTE(S):
>
> 1. FOR BEAM SLOPES > 1" PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC. 2. COORDINATE PLATES, ANGLES, AND DIMENSIONS WITH RESPECT TO THE SLOPE OF THE CONNECTION. 3. AT CONTRACTOR'S DISCRETION, SIDE PLATE (A) MAY BE CUT AS SHOWN.

SEE NOTE 3 -SIDE PLATE {A} PLATE {D} -| | <u>12</u> | 1 MAX , WELD {2} + 1/ GAP + S/2 **FRONT ELEVATION**

1 A TYPE NARROW BOLTED CONNECTION N.T.S.

NOTE(S):

1. DIMENSION BETWEEN PLATE {D} AND INSIDE FACE OF COLUMN FLANGE SHALL NOT EXCEED 1/4 INCH. THIS DIMENSION MAY VARY DEPENDING ON THE DETAILER'S PREFERENCE TO ACCOMMODATE MILL TOLERANCE AND/OR THE UNIFORMITY OF PIECE MARKS. 2. THE 1/2 INCH OVERHANG ON THE SIDE PLATE {A} IS TO ENSURE SUFFICIENT ROOM FOR WELD {2}, THE +/- TOLERANCE IS APPLIED SO THAT IF DESIRED, THE DETAILER CAN MAKE THE SIDE PLATES {A} THE SAME LENGTH WITH SLIGHTLY VARYING COLUMN DEPTHS WITHIN A GROUP OF THE SAME CONNECTION ID'S.



2 A TYPE NARROW COLUMN CONNECTION SCHEDULE N.T.S.

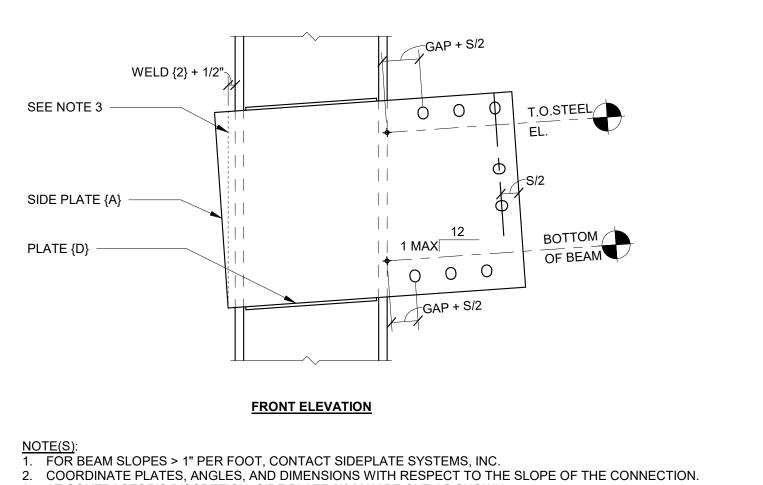
PLATE {D}, (4) LOCATIONS

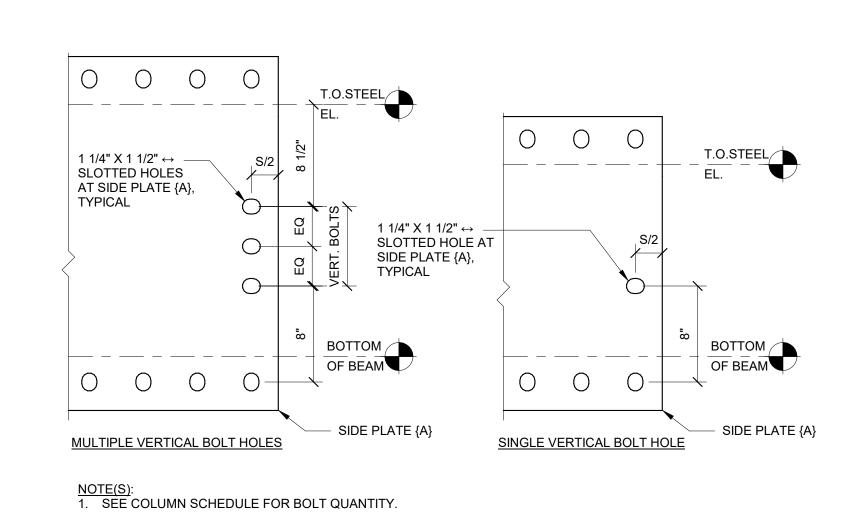
	COLUMN	PANEL ZONE	DESIG	N (INC	HES)			S	IDE PLA	ATE {A}	EXTE	NSION DESIG	SN (INCHES)			
	COLUMN	PLATE		WELD		BEA	M		PLATE				BOL	Т.		
ID	SERIES	{D}	{1}	{2}	{3}	SHAPE	GAP		{A}			DIAMETER	HORIZONTAL	VERTICAL	G	S
		THICKNESS	SIZE	SIZE	SIZE			THICKNESS	В	Е	Y		#	#		
A115	W24x	3/8	1/4	3/8	1/4	W24X68	2	5/8	31 3/4	1 3/8	2 1/2	1 1/8	4	2	2 1/8	4 1/2

4 SLOPED UP CONNECTION (AS APPLICABLE)
N.T.S.

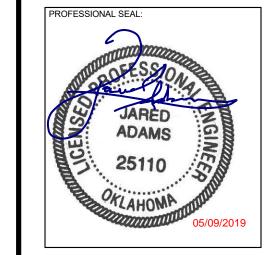
3. AT CONTRACTOR'S DISCRETION, SIDE PLATE (A) MAY BE CUT AS SHOWN.

3 SIDE PLATE {A} VSE BOLT HOLE DETAIL N.T.S.

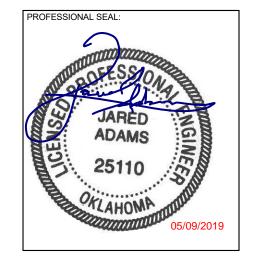




PROFESSIONAL SEAL:



CONSULTANT LOGO:



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PROJECT PHASE:

05-10-19

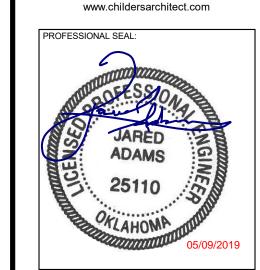
SHEET NUMBER:

BID PACKAGE 04

REVISIONS DESCRIPTION

S8.04

SIDEPLATE COLUMN DETAILS, A TYPE



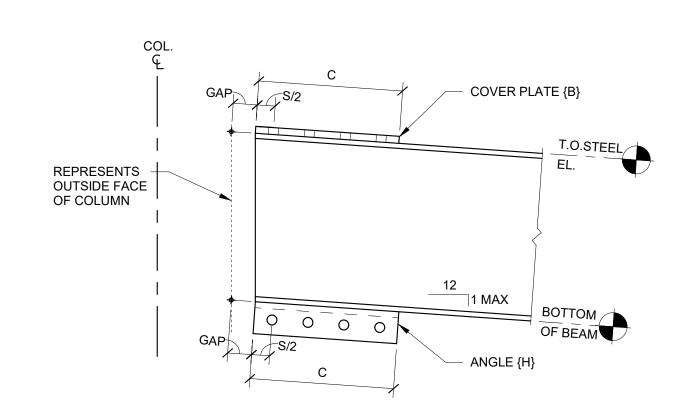






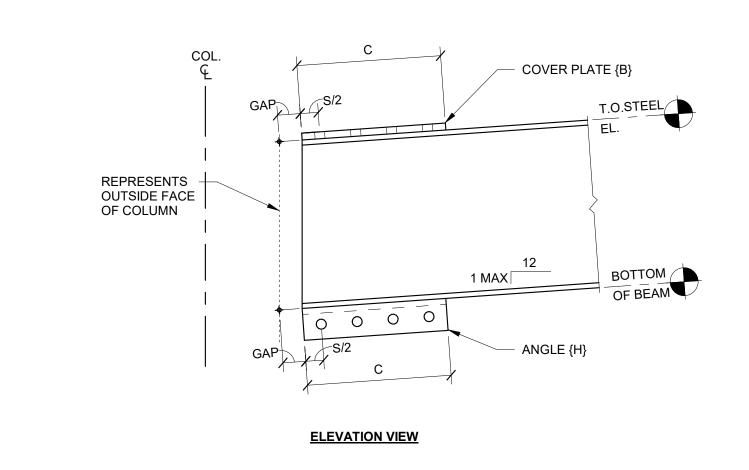
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NOTE(S):
1. FOR BEAM SLOPES > 1" PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC.

ELEVATION VIEW



NOTE(S):
1. FOR BEAM SLOPES > 1" PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC.

3 SLOPED UP BEAM END (AS APPLICABLE)
N.T.S.

2 1/8 | 4 1/2

2 1/8 | 4 1/2

2 1/8 | 4 1/2

4 SLOPED DOWN BEAM END (AS APPLICABLE)
N.T.S.

							BE	AM I	DESIGN (INCHE	S)							
	BEAM	1		PLATE					ANGLE			WELD			BOLT		
ID	SHAPE	GAP		{B}					{H}		{5}	{5a}	{5b}	DIAMETER	HORIZONTAL	G	S
	SHAFE	GAP	COVER PLATE TYPE	THICKNESS	Е	Н	SUGGESTED SIZE	С	HORIZONTAL LEG	VERTICAL LEG	SIZE	SIZE	SIZE	DIAMETER	#	G	3
A10	W24X68	2	Slotted	7/8	1 3/8	8 1/4	L5X3-1/2X5/8	18	5	3 1/2	5/16	5/16	5/16	1 1/8	4	2 1/8	4 1/2
A20, B20	W24X76	2	Slotted	3/4	1 3/8	8 1/4	L5X3-1/2X5/8	18	5	3 1/2	5/16	5/16	5/16	1 1/8	4	2 1/8	4 1/2
A29	W24X76	2	Slotted	1 3/8	1 3/8	8 1/4	L5X3-1/2X5/8	18	5	3 1/2	5/16	5/16	5/16	1 1/8	4	2 1/8	4 1/2

3 1/2 | 5/16 | 5/16 | 5/16 | 1 1/8

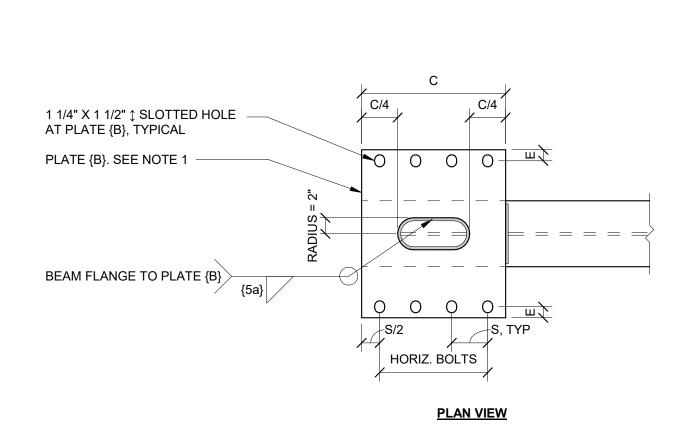
3 1/2 | 5/16 | 5/16 | 5/16 | 1 1/8

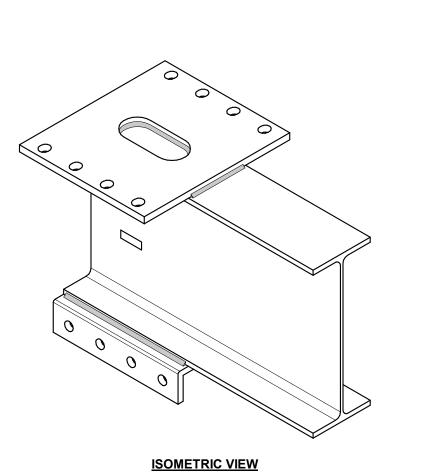
3/4 | 1 3/8 | 8 1/2 | L5X3-1/2X5/8 | 27

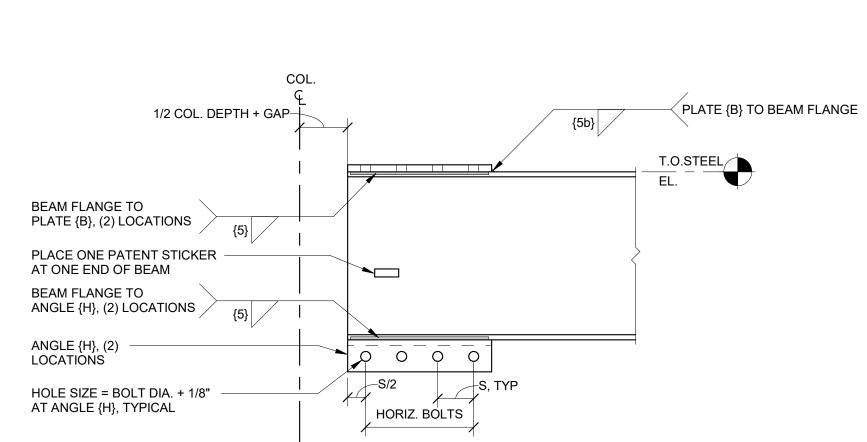
1 3/8 | 8 1/4 | L5X3-1/2X5/8 | 18 |

1 3/8 | 8 1/4 | L5X3-1/2X5/8 | 18

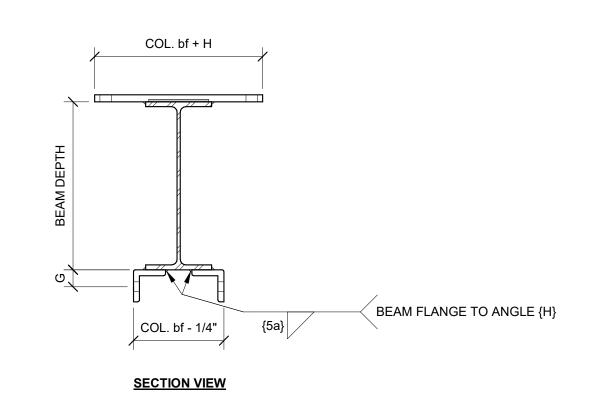
2 BEAM END SCHEDULE N.T.S.







FRONT ELEVATION



NOTE(S):	
1. USE SLOTTED OR RECTANGULAR COVER PLATE (B) PER SCHEDULE. FOR RECTANGULA	AR COVER PLATE, SEE DETAIL 5 / S8.05

PLATE {B} TO BEAM FLANGE

1 1/4" X 1 1/2"

\$\times\$ SLOTTED HOLE AT PLATE {B}, TYPICAL

m 0 0 0

S/2 S, TYP
HORIZ. BOLTS

PLAN VIEW

NOTE(S):
1. FOR ITEMS NOT NOTED, SEE DETAIL 1 / S8.05

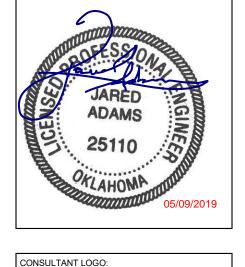
5 RECTANGULAR COVER PLATE {B}
N.T.S.

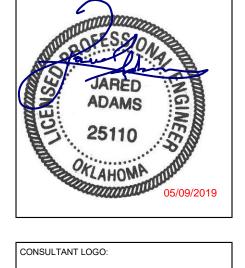
= = = = =

1 BEAM END DETAIL N.T.S.

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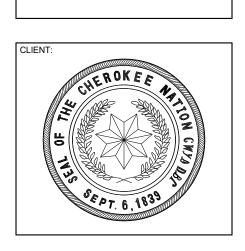


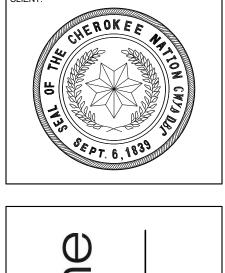




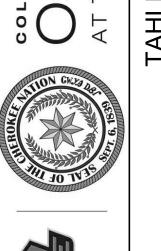




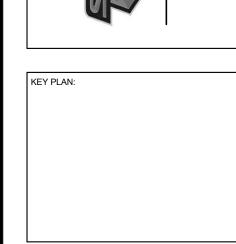


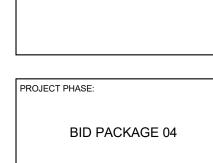












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# DATE	DESCRIPTION

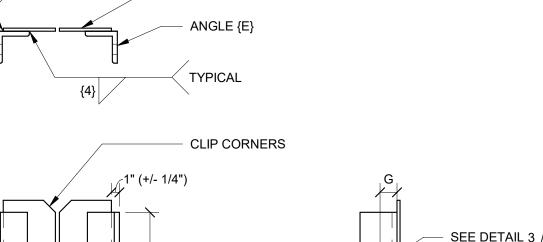
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SHEET NUMBER:		
S8.05		

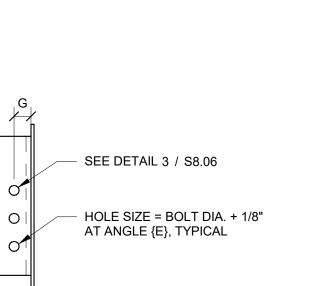
SIDEPLATE BEAM DETAILS

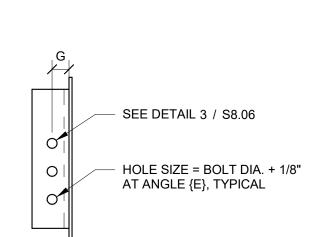
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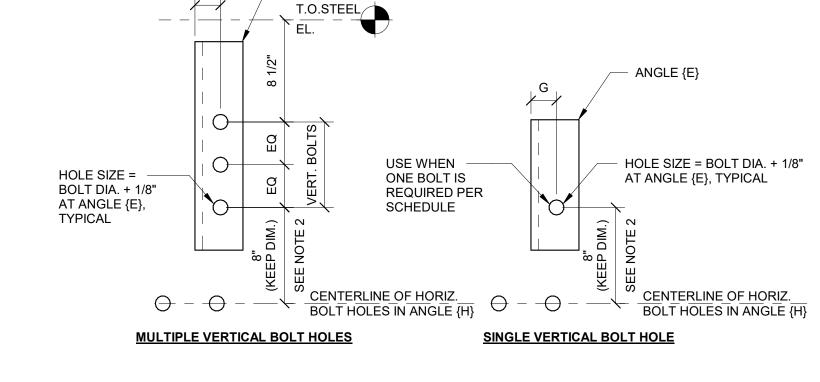
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Other U.S. and foreign applications pending.









NOTE(S):

1. SEE BEAM END SCHEDULE FOR BOLT QUANTITY.

2. EFFECTS OF MILL AND FABRICATION TOLERANCES ARE ACCOUNTED FOR BY MEASURING FROM THE CENTERLINE OF THE HORIZONTAL ROW OF BOLTS IN THE BOTTOM ANGLES {H}.

3 2 1/8 24 3/4 4 1/2

NOTE(S):

1. SEE SIDEPLATE SCHEDULE FOR BOLT QUANTITY.

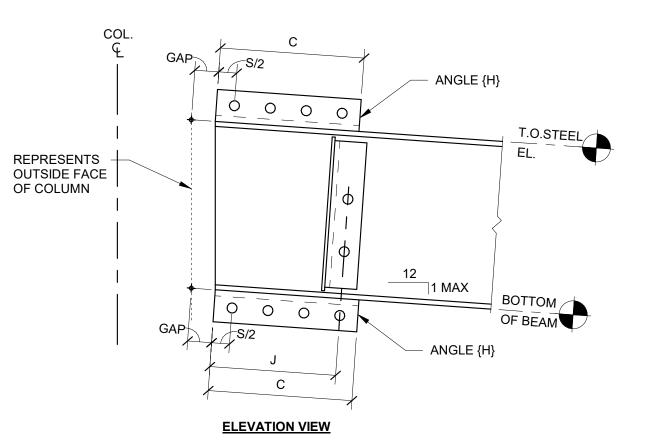
3 VSE {F} HOLE DETAIL N.T.S.

BEAM DESIGN (INCHES) **ANGLE** BOLT HORIZONTAL VERTICAL LEG LEG THICKNESS SUGGESTED SIZE 3/8 L5X3-1/2X5/8 18 A15, A115, B15 | W24X68 | 2 2 2 1/8 15 3/4 4 1/2 2 2 1/8 15 3/4 4 1/2 L5X3-1/2X5/8 18 3 2 1/8 20 1/4 4 1/2 L5X3-1/2X5/8 22 1/2

3 1/2 | L4X4X1/2 | 1/4 | 5/16 | 5/16 | 1 1/8

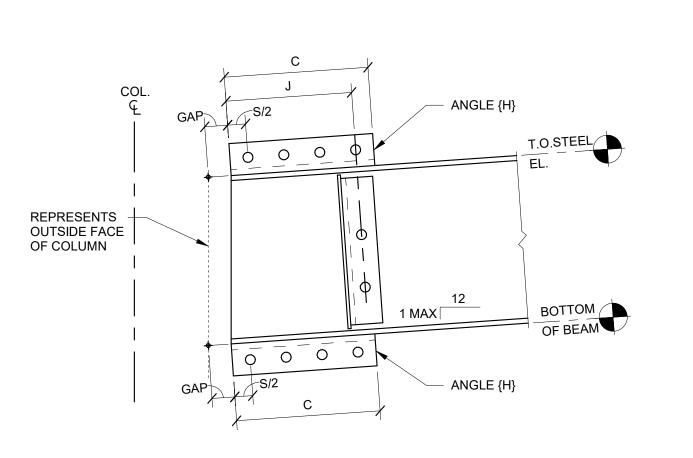
2 NARROW BEAM END SCHEDULE N.T.S.

3/8 L5X3-1/2X5/8 27



NOTE(S):
1. FOR BEAM SLOPES > 1" PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC.

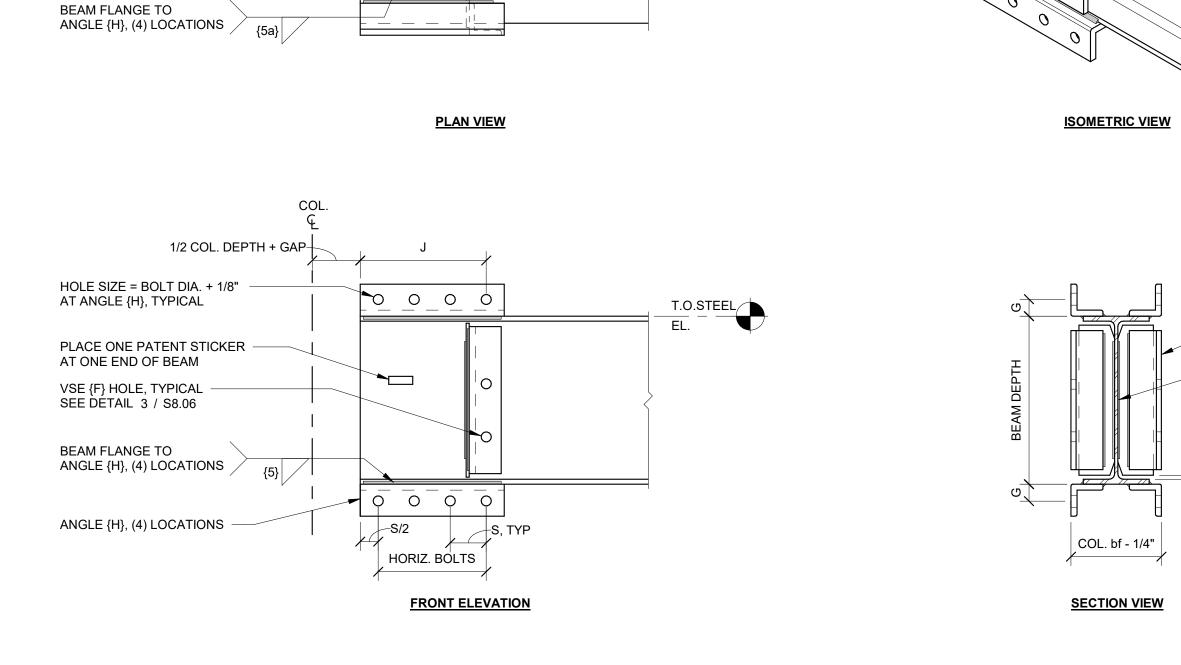
6 SLOPED DOWN BEAM END (AS APPLICABLE) N.T.S.



NOTE(S):
1. FOR BEAM SLOPES > 1" PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC.

ELEVATION VIEW

5 SLOPED UP BEAM END (AS APPLICABLE)
N.T.S.



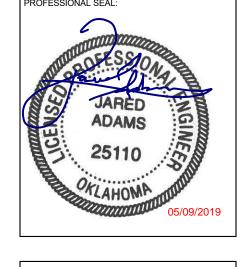
NOTE(S):

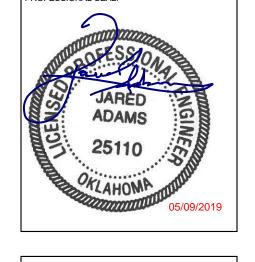
1. DIMENSION BETWEEN PLATE {C} AND INSIDE FACE OF BEAM FLANGE SHALL NOT EXCEED 1/4 INCH, AND MAY VARY DEPENDING ON BEAM MILL TOLERANCES. PLATE {C} SHALL BE CENTERED ON THE DEPTH OF THE BEAM.

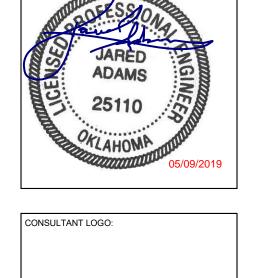
1 NARROW BEAM END DETAIL N.T.S.

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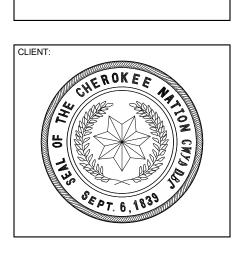


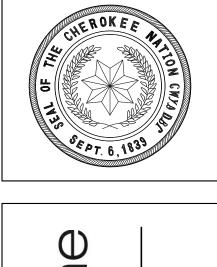










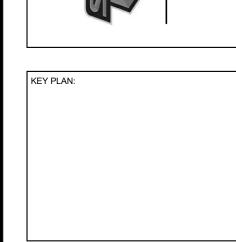


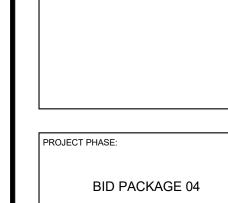












- VSE {F}, SEE DETAIL 4 / S8.06

PLATE {C} TO BEAM WEB, TYPICAL

		REVISIONS
#	DATE	DESCRIPTION

DATE:	JOB NUMBER:	
05-10-19	17-13	
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S8.06		

SIDEPLATE BEAM

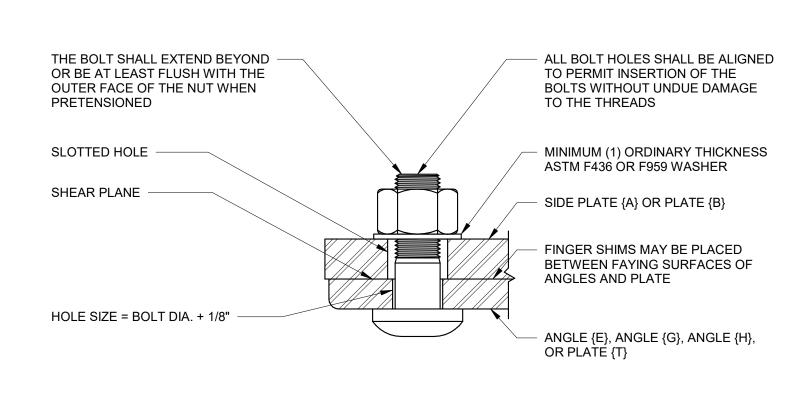
DETAILS, NARROW

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WHEN THE BEAM WEB IS NOT VERTICAL, IT MAY CAUSE THE ANGLES TO BIND MAKING IT DIFFICULT TO LOWER THE BEAM INTO PLACE SIDE PLATES HELD APART BY TEMORARY CONSTRUCTION AID **INCORRECTLY RIGGED BEAM** CORRECTLY RIGGED BEAM

4 BEAM INSTALLATION DETAIL N.T.S.



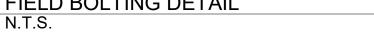
NOTE(S):

1. BOLTS SHALL BE INSTALLED AS SHOWN TO KEEP THREADS OUTSIDE OF SHEAR PLANE.

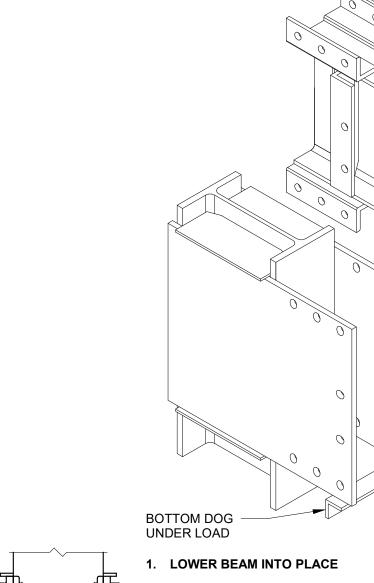
2. BOLTS SHALL BE SYSTEMATICALLY INSTALLED AS OUTLINED IN THE BOLTING SPECIFICATIONS. FIRST TO A SNUG TIGHT CONDITION, AND THEN PRETENSIONED. THE USE OF FINGER SHIMS ARE ALLOWED FOR GAPS GREATER THAN 1/8 INCH UP TO 1/4 INCH. CONTACT SIDEPLATE SYSTEMS, INC. IF GAPS ARE GREATER THAN 1/4 INCH.
 NUT SHALL BE ASTM A563.

5. THE BOLT/FASTENER ASSEMBLY SHALL BE COVERED IN A LIGHT PROTECTIVE OIL.
6. FOLLOW QUALITY CONTROL SECTION FOR EXPOSURE LIMITATION ON BOLTS/FASTENERS.

ID	ERECTION DESIGN (INCHES)				
	BEAM	BOLT			
	SHAPE	DIAMETER	HORIZONTAL #	TOTAL # PER BEAM END	
A10, B10	W24X68	1 1/8	4	16	
A20, A29, B20, B29	W24X76	1 1/8	4	16	
A40, B40	W36X160	1 1/8	6	24	



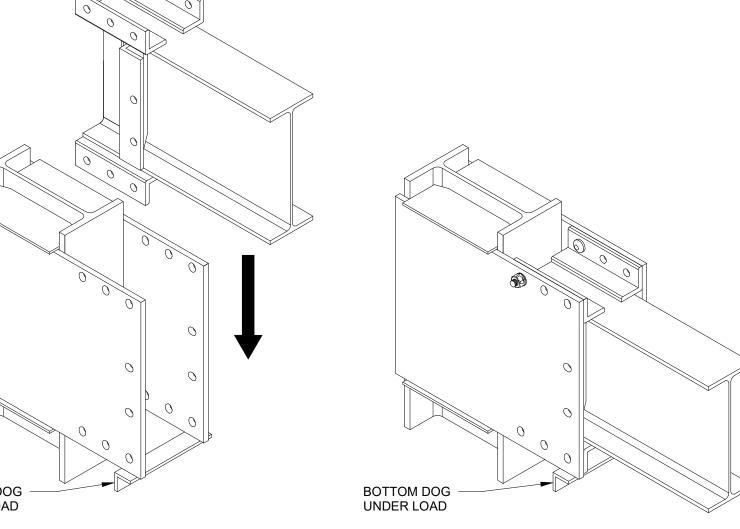
2 BEAM ERECTION SCHEDULE N.T.S.



SECTION VIEW

REMOVED

TOP ROW.



ERECTION DESIGN (INCHES)

A15, A115, B15 | W24X68 | 1 1/8

A35, B35 | W33X141 | 1 1/8

A45, B45 | W36X160 | 1 1/8

6 NARROW BEAM ERECTION SCHEDULE N.T.S.

A25, B25 W24X76

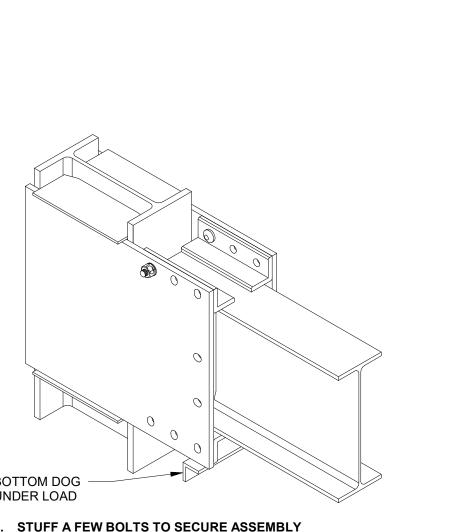
HORIZONTAL VERTICAL TOTAL # PER

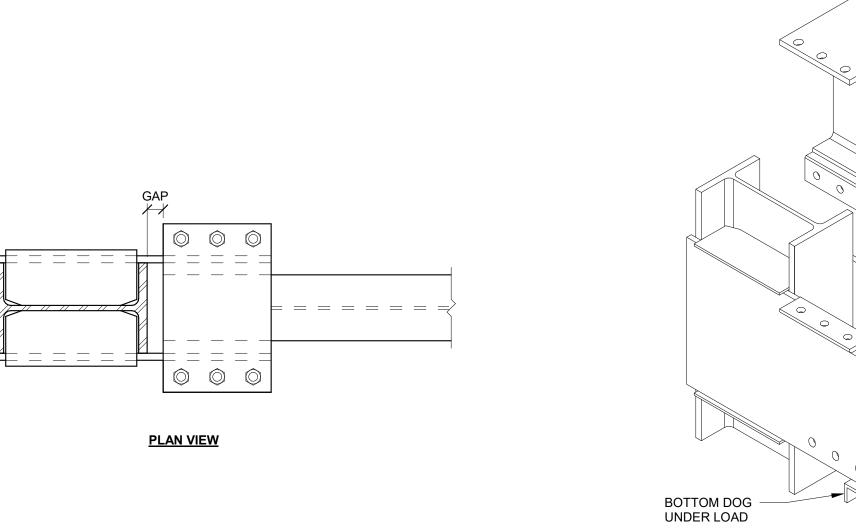
2 20

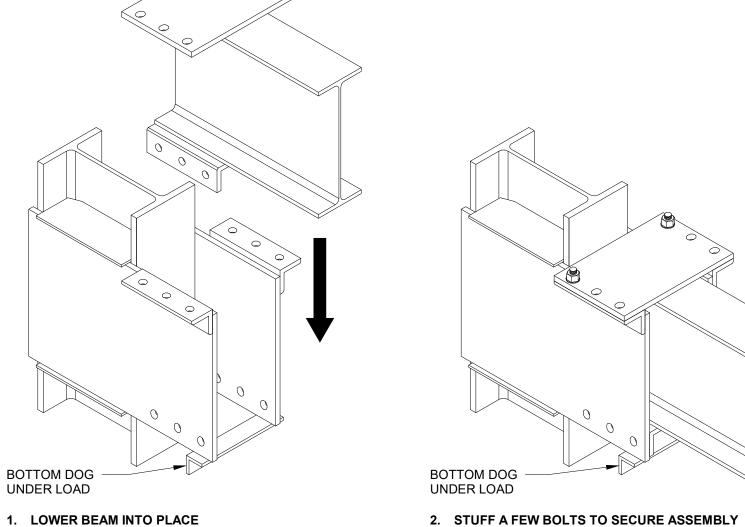
2 20

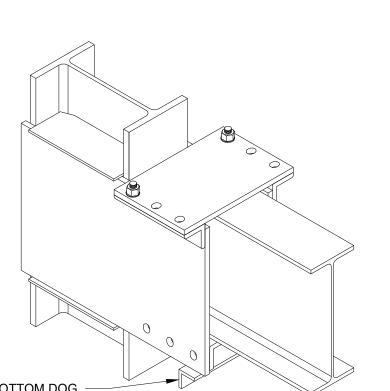
3 26

3 30

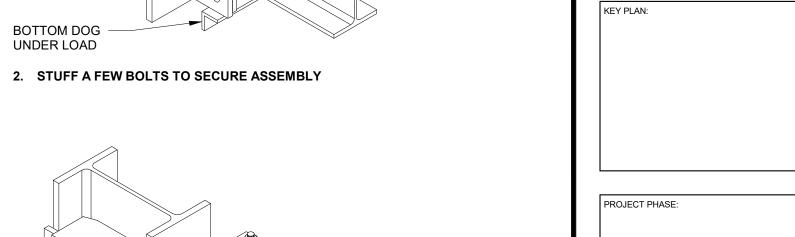












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			REVISIONS
	#	DATE	DESCRIPTION

James R. Childers

Architect, Inc.

45 South 4th Street

Fort Smith, AR 72901 479-783-2480 www.childersarchitect.com

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CONSULTANT LOGO:

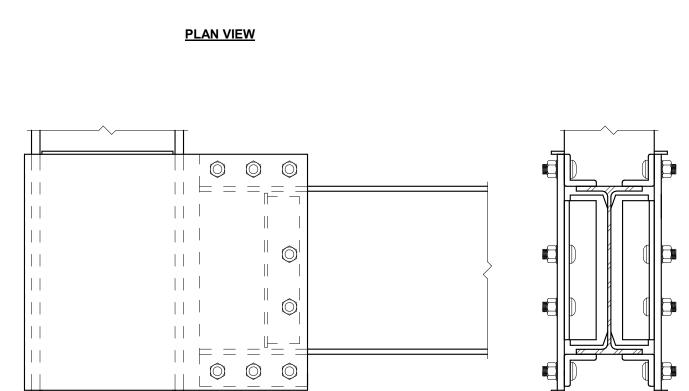
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SIDEPLATE FIELD **ERECTION DETAILS**



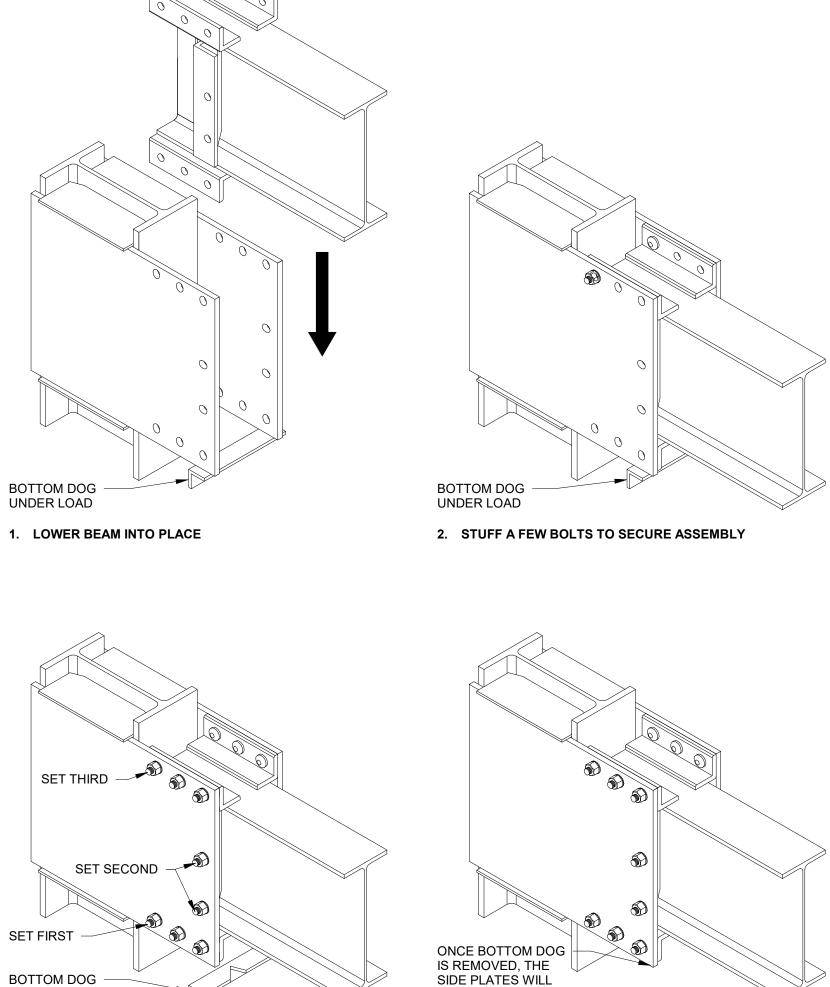
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TYPICAL SEQUENCE OF ERECTION:

1. LOWER THE BEAM INTO PLACE FROM ABOVE.

FRONT ELEVATION

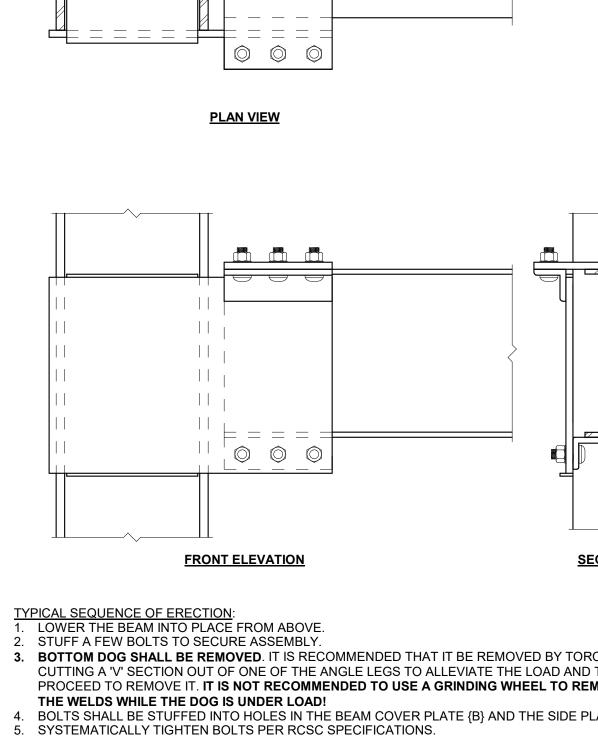
- 2. STUFF A FEW BOLTS TO SECURE ASSEMBLY. 3. BOTTOM DOG SHALL BE REMOVED. IT IS RECOMMENDED THAT IT BE REMOVED BY TORCH CUTTING A 'V' SECTION OUT OF ONE OF THE ANGLE LEGS TO ALLEVIATE THE LOAD AND THEN PROCEED TO REMOVE IT. IT IS NOT RECOMMENDED TO USE A GRINDING WHEEL TO REMOVE THE WELDS WHILE THE DOG IS UNDER LOAD!
- 4. BOLTS SHALL BE STUFFED INTO HOLES IN THE BEAM COVER PLATE {B} AND THE SIDE PLATES {A}. 5. SYSTEMATICALLY TIGHTEN BOLTS PER RCSC SPECIFICATIONS. 6. THE WELD REMNANTS OF THE BOTTOM DOG MAY REMAIN IN PLACE AND DO NOT NEED TO BE GROUND SMOOTH.

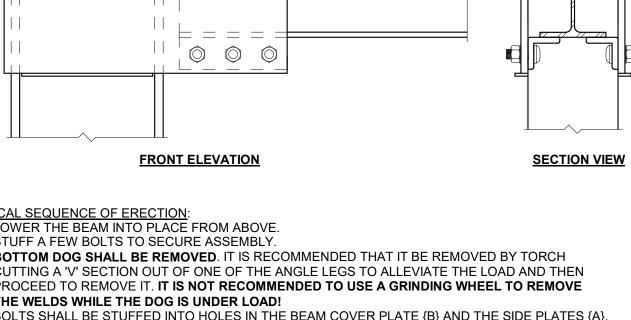


CLAMP TOGETHER

AND ELIMINATE GAPS

4. SYSTEMATICALLY TIGHTEN BOLTS PER SPECIFICATIONS.





3. BOTTOM DOG SHALL BE REMOVED. IT IS RECOMMENDED THAT IT BE REMOVED BY TORCH CUTTING A 'V' SECTION OUT OF ONE OF THE ANGLE LEGS TO ALLEVIATE THE LOAD AND THEN PROCEED TO REMOVE IT. IT IS NOT RECOMMENDED TO USE A GRINDING WHEEL TO REMOVE 4. BOLTS SHALL BE STUFFED INTO HOLES IN THE BEAM COVER PLATE (B) AND THE SIDE PLATES (A). 5. SYSTEMATICALLY TIGHTEN BOLTS PER RCSC SPECIFICATIONS. 6. THE WELD REMNANTS OF THE BOTTOM DOG MAY REMAIN IN PLACE AND DO NOT NEED TO BE

GROUND SMOOTH.

BEAM ERECTION DETAIL

REMOVED BOTTOM ROW AND THEN THE TOP ROW.

BOTTOM DOG

3. CAREFULLY REMOVE BOTTOM DOG AS IT IS UNDER LOAD. 4. SYSTEMATICALLY TIGHTEN BOLTS PER THEN STUFF ALL REMAINING BOLTS STARTING WITH THE

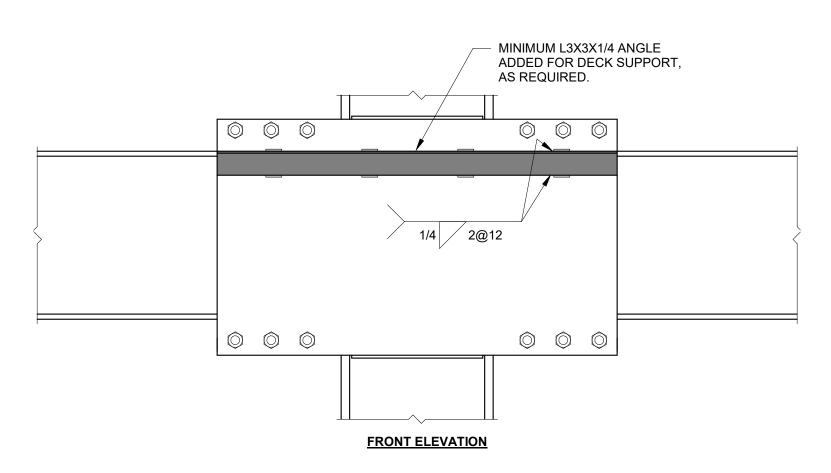
ONCE BOTTOM DOG
IS REMOVED. THE IS REMOVED, THE SIDE PLATES WILL CLAMP TOGETHER AND ELIMINATE GAPS SPECIFICATIONS.

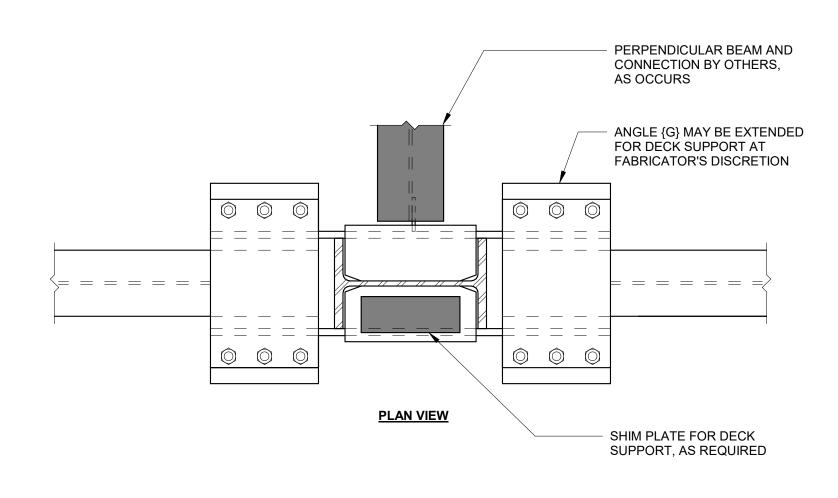
CAREFULLY REMOVE BOTTOM DOG AS IT IS UNDER LOAD.

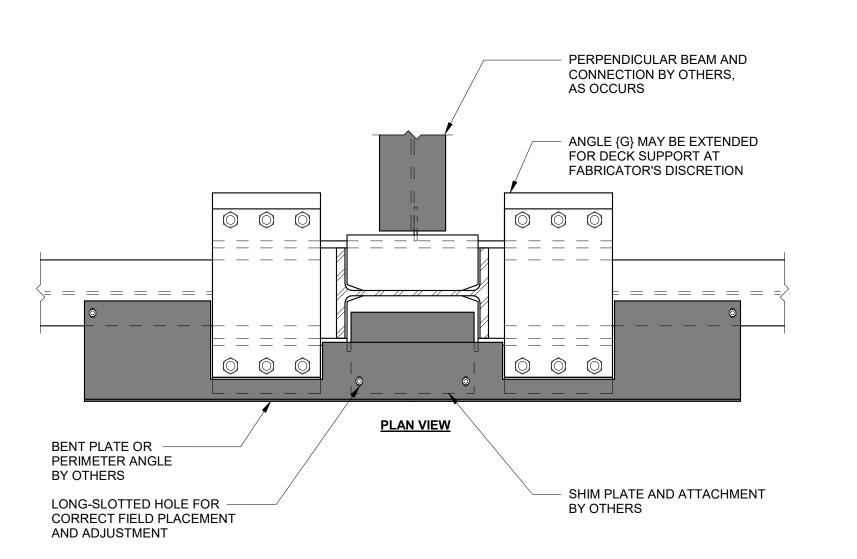
THEN STUFF ALL REMAINING BOLTS STARTING WITH THE

BOTTOM ROW, THEN THE VERTICAL ROW, AND LAST THE

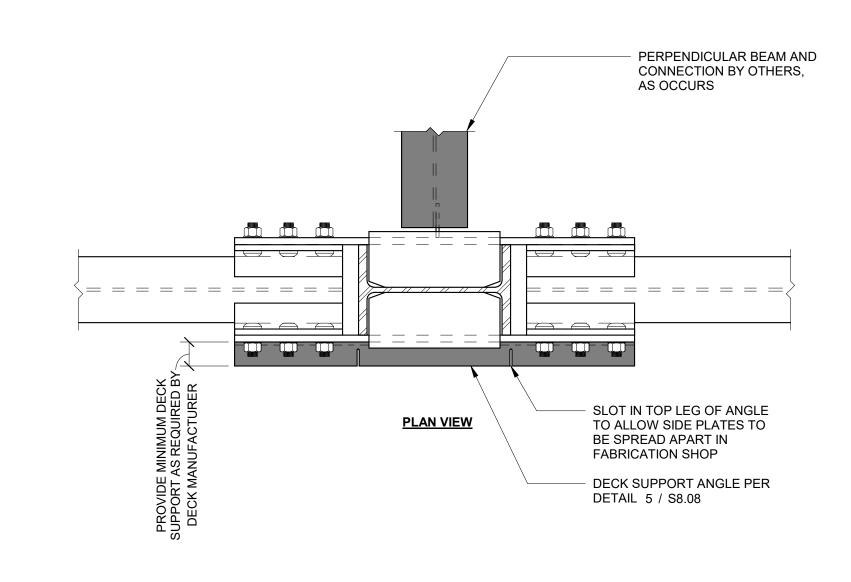
1 DECK SUPPORT DETAIL N.T.S.



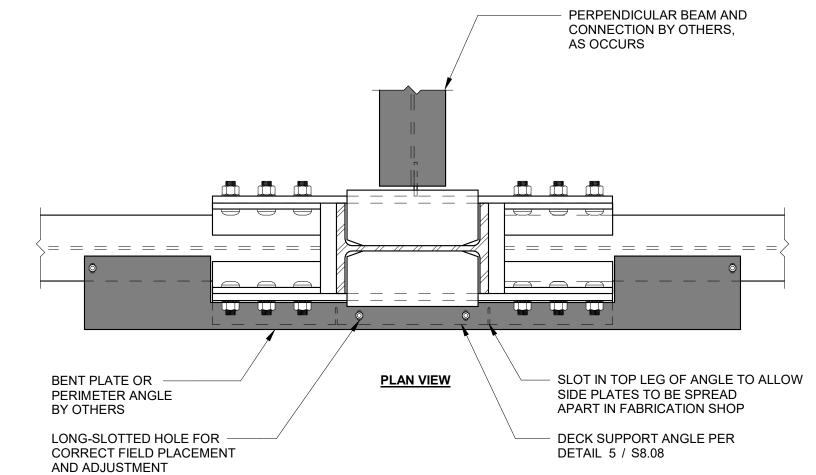


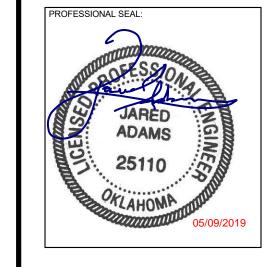


3 NARROW CONFIGURATION DECK SUPPORT DETAIL N.T.S.

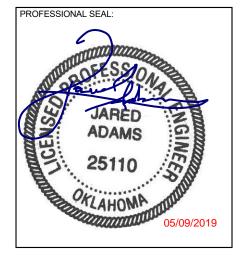


4 NARROW CONFIGURATION SLAB EDGE DETAIL N.T.S.





CONSULTANT LOGO:



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4700 Lincoln Road NE, Suite 102 · Albuquerque, NM 87109
505-344-4080 · 505-343-8759 (fax)

Osteopathic
AT THE CHEROKEE NATIO

PROJECT PHASE:

05-10-19

SHEET NUMBER:

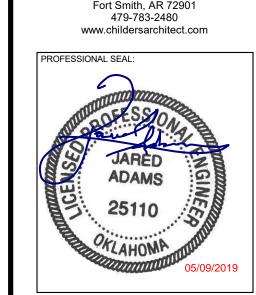
BID PACKAGE 04

S8.08

SIDEPLATE MISC DETAILS AND

COORDINATION ITEMS

REVISIONS
DATE DESCRIPTION

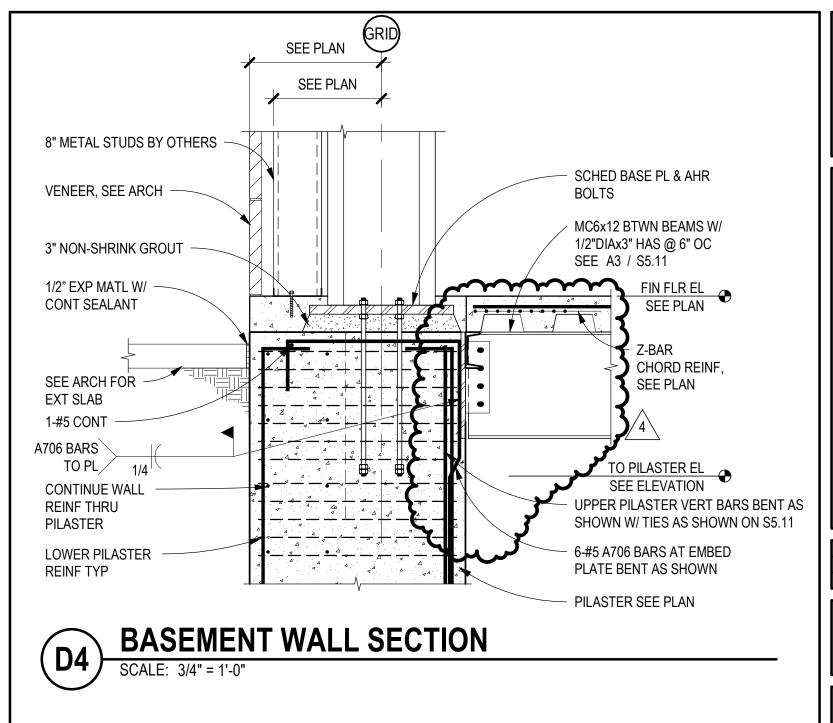








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OSU COLLEGE OF OSTEOPATHIC MEDICINE AT THE CHEROKEE NATION

DESIGNED BY:	CR	3/4" = 1'-0"
DRAWN BY:	CY	C124-144
SHEET REF:	S3.12	8/01/19

TAHLEQUAH, OK

FRAMING DETAIL

SKS-5