

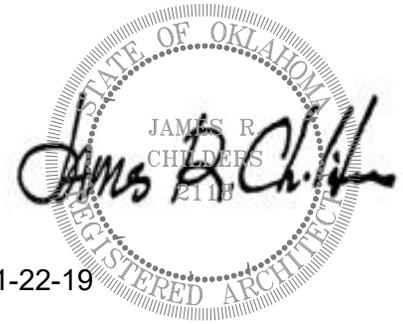


BID PACKAGE 01 – ADDENDUM 01

Date: November 22, 2019

Re: Wilma P Mankiller Health Center Expansion

From: James R Childers Architect, Inc.
45 South 4th Street
Fort Smith, Arkansas 72901



11-22-19

This addendum forms part of the Contract Documents, and modifies the documents as noted below. Acknowledge receipt of this addendum in the space provided on the bid form. Failure to do so may subject the bidder to disqualification.

Item 01 See attached revised structural drawings and narrative list from Chavez Grieves.

Bid Package 01- Addendum 01 – Wilma P. Mankiller Health Center Expansion

Chavez-Grieves would like to incorporate the following revisions into the drawings for the above referenced project.

<u>Sheet</u>	<u>Description</u>
S0.02	Revised floor loading information.
S0.03	Revised wind loading diagram.
S1.01	Sheet keynotes 6, 13, and 14 revised.
S1.01	Column size at Grid G/9 revised.
S1.01	Sections added at canopy near Grid A/7.
S1.01	Column and spread footing sizes updated at canopy near Grid A/7.
S1.02	Canopy footings and column sizes revised near Grid A/4.6.
S1.11	Column size at Grid G/9 revised.
S1.11	Beam along Grid 10, between Grids G and F updated.
S1.11	Beams between Grids 9 and 11, near Grid D updated.
S1.11	Pilaster tags added to masonry shaft wall.
S1.11	Canopy/lobby area top of steel elevations added near Grids A and C.
S1.11	Canopy/lobby area details and sections added near Grids A and C.
S1.11	Canopy/lobby area clarifying dimensions added near Grids A and C.
S1.12	Canopy/lobby area top of steel elevations added near Grids A and C.
S1.12	Canopy/lobby area details and sections added near Grids A and C.
S1.12	Canopy/lobby area clarifying dimensions added near Grids A and C.
S1.12	Operable partition support beam sizes updated near Grid D, between Grids 1 and 2.
S1.12	Detail D3/S5.53 added near Grid H/2.
S1.13	Sheet keynote 12 revised.
S1.13	Snapping point between Grid E and D corrected.
S1.13	Beams/girders supporting rooftop mechanical units revised.
S1.13	Framing between Grid G/8 and F/10 revised.
S1.13	Connection details at Grid G/4 and D/7 revised.
S1.13	Beam removed and bottom flange bracing (keynote 8) added to moment frame beam at Grid F/3.
S1.13	Connection details updated at Grids E/4, E/6, D/7, and F/5.
S1.13	Bottom flange bracing (keynote 9) added to girders along Grid F and E, between Grids 4/5 and 6/7, respectively.
S1.13	Beam/bottom flange bracing added at post near Grid G/7.
S1.21	Clarifying dimensions and sections added around interior and exterior roof perimeter.
S1.21	Location of enlarged framing plan tag D2/S4.01 adjusted.
S1.21	Beam sizes near Grid G/11 revised.
S1.21	Beams added near post at Grid G2/9.
S1.21	Beam sizes near Grid E/11 revised.

S1.21 Beam sizes near Grid D/11 revised.
S1.21 Beam details revised at Grid B/9 and D/6.
S1.22 Sheet keynote 7 added.
S1.22 Clarifying dimensions and sections added around interior and exterior roof perimeter.
S1.22 Bottom flange bracing (keynote 4) added along Grids D and G.
S1.22 Joist size updates made along Grid 1, between Grids G and E.
S1.22 Beam size updates made near Grid E/1.
S2.01 SidePlate sheet numbers added to elevation key.
S3.03 Section A1 revised.
S3.11 Sections C3 and D2 revised.
S3.12 Section C4 added.
S3.31 Sections A5, B3, B4, D5, and D2 revised.
S3.31 Sections A1 and B1 added.
S4.01 Plans A3, D2, and D4 revised.
S5.21 Detail D5 revised.
S5.52 Details A5 and B5 revised.
S5.53 Detail C3 revised.
S5.54 Sheet added.
S6.01 Footing F60A information revised.
S6.01 Baseplate schedule information revised.
S6.01 Deck schedule information revised.
S6.01 Masonry opening information revised.
S7.21 Details A3 and D4 revised.
S7.31 Detail C3 added.
S7.42 Detail C4 revised.

GENERAL STRUCTURAL NOTES

CODES AND MANUALS:
IBC-15 INTERNATIONAL BUILDING CODE 2015
ASCE/SEI 3-91 STRUCTURAL DESIGN OF COMPOSITE SLABS
ASCE/SEI 7-10 MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES
AISC 360-10 SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS
AISC 341-10 SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS
AISC MANUAL OF STEEL CONSTRUCTION 14TH EDITION
SJI-K-1.1-10 STANDARD SPECIFICATION FOR OPEN WEB STEEL JOISTS, K-SERIES
SJI-L-HDLH-1-10 STANDARD SPECIFICATION FOR LONGSPAN STEEL JOISTS, LH-SERIES AND DEEP LONGSPAN STEEL JOISTS, LH-SERIES
SJI-G-1.1-10 STANDARD SPECIFICATION FOR JOIST GIRDERS
SJI-CJ-1.0-10 STANDARD SPECIFICATION FOR COMPOSITE STEEL JOIST, CJ-SERIES
SDI DIAPHRAGM DESIGN MANUAL, 3RD EDITION
ANSI/SDDI NC1-0-06 STANDARD FOR NONCOMPOSITE STEEL FLOOR DECK
ANSI/SDDI R01-0-06 STANDARD FOR STEEL ROOF DECK
ANSI/SDDI C1-0-06 STANDARD FOR COMPOSITE STEEL FLOOR DECK
AISI S100-12 NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS
AISI S200-12 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - GENERAL PROVISIONS
AISI S210-12 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - FLOOR AND ROOF SYSTEM DESIGN
AISI S211-07 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - WALL STUD DESIGN WITH 2012 SUPPLEMENT
AISI S212-12 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - HEADER DESIGN
AISI S213-12 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - LATERAL DESIGN WITH 2010 SUPPLEMENT
AISI S214-12 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - TRUSS DESIGN, WITH SUPPLEMENT 2, DATED 2008
MBMA METAL BUILDING SYSTEMS MANUAL, 2007 EDITION
ACI 318-14 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE
PCI DESIGN HANDBOOK - MANUAL OF DESIGN, 2ND EDITION, 2010
ACI 530-13 BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES
ACI 530.1-13 SPECIFICATIONS FOR MASONRY STRUCTURES
AWS D1.1-04 STRUCTURAL WELDING CODE - STEEL
AWS D1.3-98 STRUCTURAL WELDING CODE - SHEET STEEL
AWS D1.4-11 STRUCTURAL WELDING CODE - REINFORCING STEEL

DESIGN CRITERIA:
VERTICAL:
LIVE LOAD
FLOOR
STAIRS AND EXIT-WAYS*
MINIMUM CONCENTRATED LOAD
ADDITIONAL SUPERIMPOSED LOADS
PARTITIONS
SUSPENDED EQUIPMENT
CONCENTRATED LOAD
ROOF LIVE LOAD: LR = 20'R1'R2
REDUCTION FACTOR BASED ON TRIB AREA
REDUCTION FACTOR BASED ON ROOF SLOPE
SNOW LOAD
GROUND SNOW LOAD
FLAT ROOF SNOW LOAD**
SNOW EXPOSURE FACTOR
SNOW LOAD IMPORTANCE FACTOR
THERMAL FACTOR
**INCLUDES 5 PSF RAIN-ON SNOW SURCHARGE LOAD
HORIZONTAL:
WIND
ULTIMATE DESIGN WIND SPEED
RISK CATEGORY
INTERNAL PRESSURE COEFFICIENT
NATURAL FREQUENCY
STRUCTURE IS FLEXIBLE
SEISMIC
SEISMIC IMPORTANCE FACTOR
MAPPED SPECTRAL RESPONSE ACCELERATIONS
SHORT PERIOD
1 SECOND PERIOD
SITE CLASS
SPECTRAL RESPONSE COEFFICIENTS
SHORT PERIOD
1 SECOND PERIOD
SEISMIC DESIGN CATEGORY
BASIC SEISMIC FORCE RESISTING SYSTEM: STRUCTURAL STEEL SYSTEMS
STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE
SEISMIC RESPONSE COEFFICIENT
RESPONSE MODIFICATION FACTOR
DESIGN BASE SHEAR
ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE
BASIC SEISMIC FORCE RESISTING SYSTEM: REINFORCED MASONRY SYSTEMS
INTERMEDIATE REINFORCED MASONRY SHEAR WALLS
SEISMIC RESPONSE COEFFICIENT
RESPONSE MODIFICATION FACTOR
DESIGN BASE SHEAR
ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE
ALLOWABLE SOIL BEARING PRESSURE =
(STONE COLUMNS REQUIRED TO ACHIEVE BEARING PRESSURE)
FROST DEPTH =
FUTURE BUILDING EXPANSION:
GENERAL:
STRUCTURAL DRAWINGS ARE NOT STAND-ALONE DOCUMENTS AND ARE INTENDED TO BE USED IN CONJUNCTION WITH CIVIL, ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND DRAWINGS FROM OTHER DISCIPLINES: THE CONTRACTOR SHALL COORDINATE ALL REQUIREMENTS OF THE CONTRACT DOCUMENTS INTO THE SHOP DRAWINGS AND FIELD WORK.
COORDINATE DIMENSIONS OF ALL OPENINGS, DEPRESSIONS, BLOCKOUTS, ETC. WITH ARCHITECTURAL DRAWINGS, DRAWINGS FROM OTHER DISCIPLINES, PROJECT SHOP DRAWINGS, AND FIELD CONDITIONS PRIOR TO SHOP DRAWING SUBMITTAL. THE STRUCTURAL DRAWINGS ONLY REPRESENT A PORTION OF THE REQUIREMENTS FOR THE PROJECT.
SEE ARCHITECTURAL PLANS FOR INTERIOR NON-BEARING PARTITION WALLS. PARTITION FRAMING SHALL BE CONNECTED TO THE PRIMARY STRUCTURE TO ALLOW FOR VERTICAL LIVE LOAD DEFLECTIONS OF SPAN/360 FOR FLOOR FRAMING AND SPAN/240 FOR ROOF FRAMING.
CONTRACTOR'S CONSTRUCTION AND/OR ERECTION SEQUENCES SHALL RECOGNIZE AND CONSIDER THE EFFECTS OF THERMAL MOVEMENTS OF STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PERIOD.
THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS IN THE FIELD.
SHOP DRAWINGS SHALL BE FURNISHED AND REVIEWED BEFORE ANY FABRICATION OR ERECTION IS STARTED. THE CONTRACTOR SHALL REVIEW AND APPROVE SHOP DRAWINGS PRIOR TO SUBMITTAL TO THE ARCHITECT FOR REVIEW. POORLY EXECUTED SHOP DRAWINGS WILL BE REJECTED AND SHALL BE RESUBMITTED.
THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING SAFE AND ADEQUATE SHORING FOR ALL PARTS OF THE STRUCTURE DURING CONSTRUCTION.
TEMPORARY PROVISIONS SHALL BE MADE FOR STRUCTURAL STABILITY DURING CONSTRUCTION. THE STRUCTURE SHOWN ON THE DRAWINGS HAS BEEN DESIGNED FOR STABILITY UNDER FINAL CONFIGURATION.
NOTCHING OR CUTTING ANY STRUCTURAL MEMBER IN THE FIELD IS PROHIBITED.
THE CONTRACTOR SHALL VERIFY THE SIZE AND LOCATION OF FOUNDATIONS UNDER MECHANICAL AND ELECTRICAL EQUIPMENT AS REQUIRED. NO CONCRETE PADS SHALL BE LOCATED ON ROOF UNLESS SHOWN ON THE STRUCTURAL DRAWINGS.
BACKFILL SHALL NOT BE PLACED BEHIND RETAINING WALLS UNTIL CONCRETE HAS ATTAINED 100 PERCENT OF DESIGN STRENGTH.
BACKFILL SHALL NOT BE PLACED BEHIND BASEMENT WALLS UNTIL THE CONCRETE HAS ATTAINED 100 PERCENT OF DESIGN STRENGTH AND THE ELEVATED FLOOR PROVIDING LATERAL SUPPORT AT THE TOP OF THE WALL IS COMPLETELY CONSTRUCTED, OR TEMPORARY BRACING/SHORING OF THE WALL IS PROVIDED. DESIGN OF ANY TEMPORARY WALL BRACING/SHORING IS THE RESPONSIBILITY OF THE CONTRACTOR.
REMOVAL OF FORMS AND SHORING SHALL BE IN ACCORDANCE WITH ACI 347. WHERE CONCRETE MUST SUPPORT SUPERIMPOSED LOADS PRIOR TO ATTAINING THE SPECIFIED DESIGN STRENGTH, RESHORE CONCRETE IN ACCORDANCE WITH ACI 347. RESHORING SHALL NOT BE REMOVED SOONER THAN 28 DAYS FROM THE DATE OF POUR OR UNTIL CONCRETE HAS ATTAINED THE SPECIFIED DESIGN STRENGTH.
THE CONTRACTOR SHALL SUBMIT FOR PRIOR APPROVAL THE END OF POUR LOCATIONS FOR CONCRETE GRADE BEAMS, CONCRETE COLUMNS, AND CONCRETE BEAMS.

GENERAL STRUCTURAL NOTES

THE CONTRACTOR SHALL BE RESPONSIBLE FOR ADHERING TO ALL APPLICABLE STANDARDS SET FORTH BY OSHA, INCLUDING THE FOLLOWING REQUIREMENTS FROM STANDARDS - 29 CFR, SECTION 1926, SUBPART R:
A. THE STEEL ERECTION CONTRACTOR SHALL NOT ERECT STEEL UNLESS THEY HAVE RECEIVED WRITTEN NOTIFICATION FROM THE CONTRACTOR THAT THE CONCRETE IN THE FOOTINGS, PIERS AND WALLS OR THE MORTAR IN THE MASONRY PIERS AND WALLS HAS ATTAINED, ON THE BASIS OF AN APPROPRIATE ASTM STANDARD TEST METHOD OF FIELD-CURED SAMPLES, EITHER 75 PERCENT OF THE INTENDED MINIMUM COMPRESSIVE DESIGN STRENGTH OR SUFFICIENT STRENGTH TO SUPPORT THE LOADS IMPOSED DURING STEEL ERECTION.
PROVIDE STRUCTURAL ENGINEER A COPY OF WRITTEN NOTIFICATION WHEN IT IS PROVIDED TO THE STEEL ERECTOR.
B. ANCHOR RODS (ANCHOR BOLTS) SHALL NOT BE REPAIRED, REPLACED OR FIELD-MODIFIED WITHOUT THE APPROVAL OF THE PROJECT STRUCTURAL ENGINEER OF RECORD.
PRIOR TO ERECTION OF COLUMNS, THE CONTRACTOR SHALL PROVIDE WRITTEN NOTIFICATION TO THE STEEL ERECTOR IF THERE HAS BEEN ANY REPAIR, REPLACEMENT OR MODIFICATION OF THE ANCHOR RODS (ANCHOR BOLTS).
PROVIDE STRUCTURAL ENGINEER A COPY OF WRITTEN NOTIFICATION WHEN IT IS PROVIDED TO THE STEEL ERECTOR.
C. NO MODIFICATION THAT AFFECTS THE STRENGTH OF A STEEL JOIST OR STEEL JOIST GIRDER SHALL BE MADE WITHOUT THE APPROVAL OF THE PROJECT STRUCTURAL ENGINEER OF RECORD.
D. METAL DECKING HOLES AND OPENINGS SHALL NOT BE CUT UNTIL IMMEDIATELY PRIOR TO BEING PERMANENTLY FILLED WITH THE EQUIPMENT OR STRUCTURE, OR SHALL BE IMMEDIATELY COVERED.
PROTECTION: PROPER PRECAUTIONS SHALL BE TAKEN AT ALL TIMES TO PROTECT VEHICULAR AND PEDESTRIAN TRAFFIC FROM ANY DAMAGE OR INJURY WHICH MAY BE CAUSED, EITHER DIRECTLY OR INDIRECTLY, BY THE WORK INCLUDED ON THESE DRAWINGS. SUCH PRECAUTIONS SHALL INCLUDE THE ERECTION AND MAINTENANCE OF FENCES, BARRICADES, RAILINGS, GUARDS, SIGNS, COVERINGS, LIGHTS, AND OTHER PRECAUTIONS AS MAY BE REQUIRED. IF AT ANY TIME THE OWNER OR THE OWNER'S REPRESENTATIVE, PROPER PRECAUTIONS ARE NOT BEING TAKEN TO SECURE THIS PROTECTION, THE CONTRACTOR SHALL AT NO ADDITIONAL COST TO THE OWNER, INSTALL AND MAINTAIN SUCH ADDITIONAL PROTECTION AS MAY BE DIRECTED BY THE OWNER.
POLLUTION CONTROLS: USE WATER SPRINKLING, TEMPORARY ENCLOSURES, AND OTHER SUITABLE METHODS TO LIMIT DUST AND DIRT RISING AND SCATTERING IN THE AIR TO LOWEST PRACTICAL LEVEL. COMPLY WITH GOVERNING REGULATIONS PERTAINING TO ENVIRONMENTAL PROTECTION.
TYPICAL DETAIL SHEETS:
THE S7.00 SERIES SHEETS IN THESE DRAWINGS CONTAIN TYPICAL STRUCTURAL DETAILS FOR VARIOUS BUILDING MATERIALS. SOME OF THESE DETAILS MAY NOT BE PART OF THIS PROJECT.
DRAWINGS:
DO NOT SCALE DRAWINGS.
WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL NOTES, AND SPECIFICATIONS, THE MORE STRINGENT REQUIREMENTS SHALL GOVERN. DETAILS ON DRAWINGS TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS. DETAILS NOTED "TYPICAL" APPLY TO ALL SIMILAR CONDITIONS. WHERE NO SPECIFIC DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ELSEWHERE ON THE PROJECT.
CAST-IN-PLACE CONCRETE:
ALL CONCRETE SHALL CONFORM TO THE SPECIFICATIONS FOR STRUCTURAL CONCRETE, ACI 301-10.
ALL EXPOSED EDGES OF CONCRETE SHALL HAVE A 3/4" CHAMFER UNLESS NOTED OTHERWISE.
NORMALWEIGHT CONCRETE:
A. FC = 4500 PSI @ 28 DAYS - ALL CONCRETE EXPOSED TO FREEZE/THAW CYCLES AND OCCASIONAL MOISTURE, INCLUDING CONCRETE FLAT WORK, EXPOSED BUILDING STEM WALLS, SITE WALLS, ETC...
EXTERIOR CONCRETE SHALL MEET EXPOSURE CATEGORY AND CLASS F1 ACCORDING TO ACI 318 TABLE 19.3.1.1.
B. FC = 3000 PSI @ 28 DAYS - ALL INTERIOR CONCRETE (I.E. FOOTINGS, PEDESTALS, THE BEAMS, GRADE BEAMS, RETAINING WALLS, ETC.).
C. FC = 3000 PSI @ 28 DAYS - ALL INTERIOR SLABS ON GRADE, UNLESS NOTED OTHERWISE.
D. FC = 3500 PSI @ 28 DAYS - ALL CONCRETE FILL OVER METAL DECK, UNLESS NOTED OTHERWISE.
CONCRETE MIX DESIGNS (INCLUDING AIR CONTENT, WATER TO CEMENT RATIOS, AND OTHER CRITERIA) SHALL CONFORM TO THE REQUIREMENTS SET FORTH IN ACI 318 TABLE 4.3.1, BASED ON THE EXPOSURE CATEGORIES AND CLASSES DEFINED IN ACI 318 TABLE 4.2.1. USE AIR ENTRAINING ADMIXTURE IN ALL EXTERIOR CONCRETE. AIR CONTENT IN FIRE RATED SLABS SHALL ALSO COMPLY WITH THE REQUIREMENTS IN THE SPECIFIED UL LISTING.
COLD WEATHER CONCRETING: PROTECT CONCRETE WORK FROM PHYSICAL DAMAGE OR REDUCED STRENGTH CAUSED BY FROST, FREEZING OR LOW TEMPERATURES. COMPLY WITH ACI 306.1.
HOT WEATHER CONCRETING: WHEN HOT WEATHER CONDITIONS EXIST THAT WOULD IMPAIR THE QUALITY AND STRENGTH OF THE CONCRETE, REDUCE DELIVERY TIME OF READY MIX CONCRETE, LOWER THE TEMPERATURE OF MATERIALS, OR ADD RETARDER TO ENSURE THAT THE CONCRETE IS PLASTIC. RETEMPERING WITH WATER IS NOT ALLOWED. COMPLY WITH ACI 305R.
SLAB CURING: ALL INTERIOR CONCRETE SLABS, EXCEPT EXPOSED INTEGRALLY COLORED SLABS, ARE TO BE CURED WITH A MOISTURE RETAINING COVER FOR THE FIRST 7 DAYS (MINIMUM) AFTER PLACEMENT.
THE CONTRACTOR IS ALLOWED TO CAST FOUNDATIONS AGAINST EXCAVATED SOIL SURFACES, PROVIDED THE FOLLOWING IS ADHERED TO:
A. THE SIDE SLOPES OF THE EXCAVATION SHALL BE ABLE TO MAINTAIN VERTICAL SOIL SURFACE WITHOUT SOIL SLOUGHAGE.
B. THE BOTTOM WIDTH OF THE EXCAVATION SHALL BE ONE INCH WIDER MINIMUM ON EACH SIDE THAN THE SPECIFIED FOOTING WIDTH.
C. THE SIDE WALLS OF THE EXCAVATION SHALL BE BATTERED A MINIMUM OF ONE INCH HORIZONTAL TO TWELVE INCHES VERTICAL.
D. IF SANDY OR LOOSE MATERIALS ARE ENCOUNTERED, THE FOOTING MUST BE FORMED.
E. THE CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVAL OF ANY SOIL SLOUGHAGE FROM THE WET CONCRETE DURING THE CASTING OPERATION.
F. THE CONTRACTOR AGREES TO REMOVE AND RECAST ANY FOOTING WHERE THE ABOVE CONDITIONS ARE NOT MET.
EXPOSED SITE WALLS, RETAINING WALLS, AND STEM WALLS GREATER THAN 30 FEET IN LENGTH SHALL HAVE CONTROL JOINTS INSTALLED AT THE FOLLOWING MAXIMUM SPACING:
12'-0" ON CENTER FOR WALLS 6'-0" MAXIMUM HEIGHT
18'-0" ON CENTER FOR WALLS 10'-0" MAXIMUM HEIGHT
20'-0" ON CENTER FOR WALLS GREATER THAN 10'-0" IN HEIGHT
ALL CONCRETE EXPOSED TO GROUND SHALL BE MANUFACTURED WITH PORTLAND CEMENT TYPE II OR TYPE V.
SEE THE S7.00 SERIES SHEETS FOR TYPICAL CONCRETE DETAILS.
REINFORCING STEEL:
ALL REINFORCING STEEL SHALL BE FABRICATED AND PLACED IN ACCORDANCE WITH THE BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI 318-14), AND DETAILS AND DETAILING OF CONCRETE REINFORCEMENT (ACI 315-99).
ALL REINFORCING STEEL SHALL CONFORM TO ASTM A615 GRADE 60, EXCEPT STIRRUPS, TIES AND INDICATED FIELD-BENT BARS, WHICH SHALL CONFORM TO ASTM A615 GRADE 40.
ALL WELDED WIRE FABRIC SHALL BE DEFORMED AND SHALL CONFORM TO ASTM A479. PROVIDE IN FLAT SHEETS ONLY.
TENSION AND COMPRESSION LAPS IN REINFORCING SHALL CONFORM TO THE LAP SPlice SCHEDULE ON SHEET S-601 AND BE IN ACCORDANCE WITH ACI 318, CHAPTER 12, UNLESS NOTED OTHERWISE.
ALL HORIZONTAL REINFORCING IN FOOTINGS, WALLS AND BEAMS SHALL BE CONTINUOUS AROUND CORNERS OR HAVE BENT (CORNER) BARS OF THE SAME SIZE AND SPACING AS THE HORIZONTAL BARS AND LAP 30 BAR DIAMETERS (24" MINIMUM).
CONCRETE COVER FOR REINFORCING SHALL BE AS FOLLOWS UNLESS OTHERWISE NOTED:
A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3"
B. CONCRETE CAST AGAINST FORMS BUT EXPOSED TO EARTH OR WEATHER:
1. BARS LARGER THAN NO. 5: 2"
2. BARS NO. 5 OR SMALLER: 1 1/2"
C. CONCRETE NOT EXPOSED TO WEATHER OR NOT IN CONTACT WITH GROUND:
1. COLUMNS, GIRDERS AND BEAMS: 1 1/2"
2. STRUCTURAL SLABS, WALLS AND JOISTS (NO. 11 AND SMALLER): 3/4"
D. SLAB ON GRADE: 1 1/2" FROM TOP OF SLAB
E. STRUCTURAL SLAB ON METAL DECK: 1" FROM TOP OF SLAB
FORM TIES SHALL BE EITHER OF THE THREADED OR SNAP-OFF TYPE SO THAT NO METAL WILL BE LEFT WITHIN 1 INCH OF THE SURFACE OF THE WALL. FOLLOWING REMOVAL OF FORM TIES, RECESSES ARE TO BE CAREFULLY FILLED AND POINTED WITH MORTAR.
REINFORCING SHALL NOT BE TACK WELDED OR WELDED IN ANY MANNER UNLESS SPECIFICALLY DETAILED ON THE STRUCTURAL PLANS.
BAR SUPPORTS AND SPACERS FOR REINFORCING SHALL BE PROVIDED IN ACCORDANCE WITH ACI 315-99. REINFORCING SHALL BE SECURELY TIED TO SUPPORTS.
CHAIRS WITH 22 GAGE SAND PLATES OR PRECAST BLOCKS SHALL BE PROVIDED FOR ALL REINFORCING OF CONCRETE IN CONTACT WITH GRADE.
DECK CHAIRS SHALL BE PROVIDED FOR ALL WELDED WIRE FABRIC IN SLABS OVER METAL DECK.

GENERAL STRUCTURAL NOTES

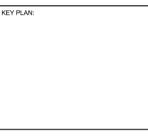
POST INSTALLED ANCHORS:
THE STRUCTURAL DESIGN IS BASED ON THE POST INSTALLED ANCHORING SYSTEMS NOTED BELOW. SINCE ANCHOR CAPACITIES VARY BY MANUFACTURER, THE CONTRACTOR SHALL USE ONLY THE SYSTEMS NOTED BELOW UNLESS AN ALTERNATE IS APPROVED BY THE ENGINEER OF RECORD. ALTERNATE ANCHORING SYSTEMS MAY REQUIRE RE-DESIGN TO VERIFY ANCHOR QUANTITIES, SPACING, AND EMBED DEPTHS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY ADDITIONAL CONSTRUCTION AND RE-DESIGN COSTS ASSOCIATED WITH THE ALTERNATE ANCHORING SYSTEM.
ALL ADHESIVE (EPOXY) FOR POST INSTALLED ANCHORS AND/OR REBAR INTO CONCRETE SHALL BE HILTI HIT-RE 500 V3 OR HIT-HY 200 EPOXY ADHESIVE ANCHORING SYSTEM, HILTI HIT-RE 100 OR HIT-HY 200 EPOXY ADHESIVE SYSTEM OR APPROVED EQUAL. INSTALLATION SHALL BE PER MANUFACTURER'S RECOMMENDATIONS.
ALL ADHESIVE (EPOXY) FOR POST INSTALLED ANCHORS AND/OR REBAR INTO GROUT FILLED MASONRY SHALL BE HILTI HIT HY 70 ADHESIVE ANCHORING SYSTEM OR APPROVED EQUAL. INSTALLATION SHALL BE PER MANUFACTURER'S RECOMMENDATIONS.
ALL ADHESIVE (EPOXY) FOR POST INSTALLED ANCHORS AND/OR REBAR INTO HOLLOW MASONRY AND/OR BRICK SHALL BE HILTI HIT HY 70 ADHESIVE ANCHORING SYSTEM OR APPROVED EQUAL. INSTALLATION SHALL BE PER MANUFACTURER'S RECOMMENDATIONS.
ALL POST INSTALLED MECHANICAL ANCHORS INTO CONCRETE SHALL BE HILTI KWIK BOLT TZ EXPANSION ANCHOR OR APPROVED EQUAL. INSTALLATION SHALL BE PER MANUFACTURER'S RECOMMENDATIONS.
ALL POST INSTALLED MECHANICAL ANCHORS INTO GROUT FILLED MASONRY SHALL BE HILTI KWIK BOLT 3 EXPANSION ANCHOR OR APPROVED EQUAL. INSTALLATION SHALL BE PER MANUFACTURER'S RECOMMENDATIONS.
ANCHOR LENGTHS SHOWN FOR ATTACHMENT TO CONCRETE AND/OR MASONRY ARE REQUIRED EMBEDMENT LENGTHS. THE CONTRACTOR SHALL PROVIDE ANCHORS WITH ADDITIONAL LENGTH TO FACILITATE THE REQUIRED CONNECTION.
SUBMIT ALL PROPOSED ANCHORING SYSTEMS INCLUDING ICC-ES REPORTS TO STRUCTURAL ENGINEER FOR REVIEW PRIOR TO INSTALLATION. THE ICC-ES FORMS SHALL MEET THE REQUIREMENTS OF THE IBC REFERENCED IN THESE NOTES.
STRUCTURAL AND MISCELLANEOUS STEEL:
ALL STRUCTURAL STEEL SHALL BE DETAILED AND FABRICATED IN ACCORDANCE WITH THE AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS".
ALL WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A992, GRADE 50, UNLESS NOTED OTHERWISE.
ALL MISCELLANEOUS STEEL MEMBERS, SUCH AS CHANNELS, ANGLES, FLAT BARS, AND PLATES SHALL CONFORM TO ASTM A36 UNLESS NOTED OTHERWISE.
ALL RECTANGULAR AND SQUARE STRUCTURAL TUBING SHALL CONFORM TO ASTM A500, GRADE B, FY = 46 KSI OR ASTM 1085, GRADE B, FY = 50 KSI.
ALL ROUND STRUCTURAL TUBING SHALL CONFORM TO ASTM A500, GRADE B, FY = 42 KSI OR ASTM 1085, GRADE B, FY = 50 KSI.
ALL STRUCTURAL PIPE SHALL CONFORM TO ASTM A53, TYPE E OR S, GRADE B, FY = 35 KSI.
BOLTS SHALL CONFORM TO ASTM A325N TENSION CONTROL BOLTS UNLESS NOTED OTHERWISE, WITH SIZES AS SHOWN ON THE DRAWINGS. WHERE CLEARANCE WITHIN A CONNECTION DOES NOT PERMIT THE USE OF TENSION CONTROL BOLTS, STANDARD A325N BOLTS SHALL BE USED AND INSPECTED IN ACCORDANCE WITH THE AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS".
ALL BOLTS SHALL BE INSTALLED IN A SNUG TIGHT CONDITION EXCEPT AT MOMENT CONNECTIONS, BRACED FRAME CONNECTIONS, AND AT CONNECTIONS DETAILED WITH A325SC BOLTS. AT THESE LOCATIONS, THE BOLTS SHALL BE TIGHTENED SO AS TO SHEAR THE SPLINE OFF THE BOLT.
ANCHOR BOLTS EMBEDDED IN CONCRETE SHALL BE ASTM F1554 GRADE 36 THREADED RODS WITH DOUBLE NUTS. PROVIDE FLAT WASHERS BETWEEN NUTS AND BASEPLATE SURFACES. ANCHOR BOLT LENGTHS SHOWN FOR ATTACHMENT TO CONCRETE AND/OR MASONRY ARE REQUIRED EMBEDMENT LENGTHS. THE CONTRACTOR SHALL PROVIDE ANCHOR BOLTS WITH ADDITIONAL BOLT LENGTH TO FACILITATE THE REQUIRED CONNECTION.
ANCHOR BOLT FLAT WASHERS SHALL BE PROVIDED IN ACCORDANCE WITH TABLE 14-2 OF AISC 360, AISC MANUAL OF STEEL CONSTRUCTION LATEST EDITION.
ALL WELDING SHALL BE DONE IN ACCORDANCE WITH THE LATEST STANDARDS OF THE AWS STRUCTURAL WELDING CODE.
ALL BOLT HOLES THAT ARE REQUIRED TO BE FIELD DRILLED SHALL BE DRILLED WITH A MAG DRILL. FLAME CUTTING OF HOLES OR ENLARGING OF MISALIGNED HOLES WILL NOT BE ALLOWED.
HEADED CONCRETE ANCHORS AND SHEAR CONNECTORS SHALL BE MADE FROM STEEL CONFORMING TO ASTM A108 AND MEET THE MECHANICAL PROPERTIES OF TYPE B, AS REQUIRED BY CHAPTER 7 OF AWS D1.1 "STRUCTURAL WELDING CODE-STEEL", LATEST EDITION. STRUCTURAL STEEL TO RECEIVE SHEAR CONNECTORS SHALL BE FREE OF PAINT. WELDING PREQUALIFICATION REQUIRED.
PROVIDE A SLIDE BEARING CONNECTION FOR STEEL BEAMS BEARING ON MASONRY WALLS UNLESS NOTED OTHERWISE. SEE SHEET S7.41 FOR TYPICAL CONNECTION DETAIL.
SEE S7.00 SERIES SHEETS FOR TYPICAL STEEL DETAILS.
COMPOSITE FLOORS:
THE METAL DECK FOR COMPOSITE FLOORS SHALL BE UNSHORED UNLESS NOTED OTHERWISE.
THE SHEAR CONNECTORS SHALL BE 3/4" DIAMETER X 4 1/2" AT 3" DEEP DECK UNLESS NOTED OTHERWISE. THE SHEAR CONNECTORS SHALL BE MADE FROM STEEL CONFORMING TO ASTM A108 AND MEET THE MECHANICAL PROPERTIES OF TYPE B, AS REQUIRED BY CHAPTER 7 OF AWS D1.1 "STRUCTURAL WELDING CODE-STEEL", LATEST EDITION. STRUCTURAL STEEL TO RECEIVE SHEAR CONNECTIONS SHALL BE FREE OF PAINT. WELDING PREQUALIFICATION REQUIRED.
THE SHEAR CONNECTIONS SHALL NOT BE ADDED UNTIL THE METAL FLOOR DECK IS INSTALLED.
WHERE SHEAR CONNECTIONS AND PUDDLE WELDS COINCIDE, THE SHEAR CONNECTOR MAY REPLACE THE PUDDLE WELD.
CAMBERED BEAMS SHALL HAVE THE CAMBER PUT IN AT 1/3 POINTS OR ALONG A PARABOLIC CURVE.
THE CONTRACTOR SHALL SURVEY THE CAMBER OF THE BEAMS AFTER THE BEAMS HAVE BEEN ERECTED. THE CONTRACTOR SHALL SUBMIT THE SURVEY TO THE ENGINEER FOR REVIEW. THE CONTRACTOR SHALL NOT POUR THE SLAB UNTIL THE ENGINEER HAS REVIEWED AND APPROVED THE BEAM CAMBERS.
CONTRACTOR SHALL SHORE BEAMS WITH A CAMBER MORE THAN 1/2" LOWER THAN SPECIFIED. THE BEAM SHALL BE ALLOWED TO DEFLECT TO LEVEL.
THE CONCRETE FOR THE SLAB SHALL BE POURED AND PLACED TO THE ELEVATION INDICATED ON THE DRAWINGS WHILE MAINTAINING THE MINIMUM THICKNESS. SPREAD CONCRETE OVER AREA OF INFLUENCE TO ROUGH DEPTH IN ORDER TO LOAD BEAMS AND GIRDERS PRIOR TO SETTING SCREED ELEVATIONS.
THE WEIGHT OF THE WET CONCRETE WILL CAUSE DEFLECTIONS OF THE STEEL FRAMING. THEREFORE, CONCRETE OVERRUNS ARE TO BE ANTICIPATED BY THE CONTRACTOR.
CONTRACTOR SHALL CONTINUOUSLY MONITOR THE THICKNESS AND ELEVATIONS DURING CONCRETE PLACING OPERATIONS.
PROVIDE #4 X 6'-0" AT 12" ON CENTER OVER ALL GIRDERS OF COMPOSITE FLOORS.
PROVIDE #4 X 6'-0" AT 12" ON CENTER OVER SHORED BEAMS THAT ARE NOT ALLOWED TO DEFLECT TO LEVEL.
PROVIDE WELDED WIRE FABRIC AS INDICATED ON DRAWINGS IN FLAT SHEETS ONLY.
PROVIDE DECK CHAIRS FOR ALL WELDED WIRE FABRIC IN SLABS OVER METAL DECK.
STEEL JOISTS:
STEEL JOISTS SHALL BE MANUFACTURED BY A MEMBER OF SJI.
STEEL JOISTS SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE STEEL JOIST INSTITUTE STANDARD SPECIFICATIONS.
NO CONSTRUCTION LOADS OF ANY KIND SHALL BE PLACED ON UNBRIDGED JOISTS.
WHERE COLUMNS ARE NOT FRAMED IN AT LEAST TWO DIRECTIONS WITH STRUCTURAL STEEL MEMBERS, JOISTS AT OR CLOSEST TO COLUMN LINES SHALL BE FIELD BOLTED TO ADD LATERAL STABILITY DURING CONSTRUCTION.
PROVIDE BRIDGING IN ACCORDANCE WITH THE LATEST EDITION OF THE SJI STANDARD SPECIFICATIONS AND OSHA REQUIREMENTS.
THE STRUCTURAL DRAWINGS ARE NOT STAND-ALONE DOCUMENTS. THE CONTRACTOR SHALL COORDINATE THE LOCATIONS AND WEIGHTS OF ALL MECHANICAL, PLUMBING, ELECTRICAL AND OTHER EQUIPMENT WITH THE APPLICABLE DRAWINGS. THE JOIST SUPPLIER SHALL ACCOUNT FOR THE LOADS IN THEIR DESIGN.
THE STEEL JOIST MANUFACTURER SHALL DESIGN ROOF JOISTS SUPPORTING MECHANICAL UNITS, INDICATED AS SP JOISTS ON THE DRAWINGS. FOR 1.2x MECHANICAL UNIT WEIGHTS SHOWN, USE 25 PSF DEAD LOAD AND 20 PSF LIVE LOAD (NON-REDUCIBLE) UNLESS NOTED OTHERWISE. CONTRACTOR SHALL VERIFY ACTUAL MECHANICAL LOADS. NOTIFY STEEL JOIST MANUFACTURER OF ANY DISCREPANCIES.
JOIST DEFLECTIONS SHALL BE LIMITED PER SJI REQUIREMENTS AND SHALL NOT BE LESS THAN THE FOLLOWING:
L/360 FOR ROOF LIVE LOAD WITH STANDARD SJI CAMBER WHERE PLASTER OR STUCCO CEILINGS ARE SUPPORTED.
L/240 FOR ROOF LIVE LOAD WITH STANDARD SJI CAMBER WHERE NONPLASTER CEILINGS ARE SUPPORTED.
L/180 FOR ROOF LIVE LOAD WITH STANDARD SJI CAMBER WHERE NO CEILINGS ARE SUPPORTED AND PROVISIONS FOR FUTURE CEILINGS ARE NOT REQUIRED.
STEEL ROOF JOISTS SHALL BE DESIGNED FOR A NET WIND UPLIFT LOAD OF 15 PSF UNLESS NOTED OTHERWISE. THE DEAD LOAD OF MISCELLANEOUS ROOFTOP ITEMS, INCLUDING SCREEN WALLS, SKYLIGHTS, FIRE SUPPRESSION SYSTEM, SOLAR PHOTOVOLTAIC SYSTEM, ETC., SHALL BE ACCOUNTED FOR IN THE DESIGN OF THE STEEL ROOF JOISTS. THE CONTRACTOR SHALL COORDINATE THE MISCELLANEOUS LOADS WITH THE STEEL JOIST MANUFACTURER.

GENERAL STRUCTURAL NOTES

JOIST BEARING SEATS SHALL BEAR ON STEEL SUPPORTS AND SHALL BE CONNECTED AS FOLLOWS UNLESS NOTED OTHERWISE:
KKGCS: TWO 1/8" x 2 1/2" LONG FILLET WELDS
LH02-06 (OR 2 1/2" AND SMALLER TOP CHORD ANGLE LEG): TWO 3/16" x 2 1/2" LONG FILLET WELDS
LH0LH07-17 (OR 3 1/2" AND SMALLER TOP CHORD ANGLE LEG): TWO 1/4" x 2 1/2" LONG FILLET WELDS
LH0LH 18-25 (OR 4" AND LARGER TOP CHORD ANGLE LEG): TWO 1/4" x 4" LONG FILLET WELDS
ALL WELDS SHALL MEET CURRENT MINIMUM SJI REQUIREMENTS.
STEEL DECK:
ALL STEEL DECK SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE STEEL DECK INSTITUTE SPECIFICATIONS.
SEE PLANS FOR STEEL DECK TYPE, GAGE, FINISH AND CONNECTIONS.
PROVIDE A MINIMUM OF 1 1/2" BEARING FOR ALL STEEL DECK.
ALL SPLICES AND LAPS SHALL BE A MINIMUM OF 2" IN LENGTH AND SHALL BE LOCATED DIRECTLY ABOVE SUPPORTS.
ALL DECKING SHALL BE CONTINUOUS OVER TWO OR MORE SPANS.
POWDER DRIVEN FASTENERS SHALL BE EQUIVALENT TO:
HILTI X-HSN 24 FOR STEEL BASE MATERIAL 1/4" UP TO 3/8".
HILTI ENP-19 FOR STEEL BASE MATERIAL 1/4" OR THICKER.
MASONRY:
ALL MASONRY UNITS SHALL COMPLY WITH ASTM C 90 WITH A COMPRESSIVE STRENGTH OF 2000 PSI (NET AREA).
FM = 1900 PSI
MORTAR SHALL BE TYPE S.
GROUT - FC = 2000 PSI, MINIMUM.
CELLS CONTAINING REBAR SHALL BE GROUTED SOLID FROM THE BOTTOM TO THE TOP OF THE WALL IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE.
ALL CELLS BELOW GRADE SHALL BE GROUTED SOLID UP TO GRADE.
CELLS CONTAINING EXPANSION ANCHORS SHALL BE GROUTED SOLID.
ALL VERTICAL REBAR SHALL BE IN PLACE AND SECURED WITH REBAR POSITIONERS PRIOR TO GROUTING.
COVER FOR REINFORCING SHALL BE AS FOLLOWS UNLESS OTHERWISE NOTED:
A. MASONRY FACE NOT EXPOSED TO EARTH OR WEATHER: 1 1/2"
B. MASONRY FACE EXPOSED TO EARTH OR WEATHER:
1. BARS LARGER THAN NO. 5: 2"
2. BARS NO. 5 OR SMALLER: 1 1/2"
UNLESS OTHERWISE NOTED MASONRY CELLS SHALL BE GROUTED IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE (MAXIMUM 5 FOOT GROUT LIFTS).
LAP REBAR PER THE SCHEDULE ON S6.01.
WHERE REBAR LAP SPLICES EXCEED 5 FT GROUT LIFTS, 8 FT GROUT LIFTS MAY BE USED WITH CLEANOUTS PROVIDED AT THE BOTTOM OF EACH VERTICALLY REINFORCED CELL. SOLID GROUTED WALLS SHALL HAVE CLEANOUTS AT 32" ON CENTER MAXIMUM.
ALL HORIZONTAL REINFORCING IN BOND BEAMS SHALL BE CONTINUOUS AROUND CORNERS OR HAVE BENT (CORNER) BARS OF THE SAME SIZE AND A LAP AS NOTED ABOVE. VERTICAL STEEL SHALL CONTINUE THROUGH BOND BEAMS.
PROVIDE STANDARD LADDER TYPE JOINT REINFORCING AT 16" ON CENTER (ALTERNATE COURSES) UNLESS NOTED OTHERWISE IN THE PROJECT DOCUMENTS. USE PREFABRICATED CORNERS AND TEES AT ALL WALL CORNERS AND INTERSECTIONS RESPECTIVELY.
PROVIDE A SLIDE BEARING CONNECTION FOR STEEL BEAMS BEARING ON MASONRY WALLS UNLESS NOTED OTHERWISE. SEE SHEET S7.41 FOR TYPICAL CONNECTION DETAIL.
SEE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR INFORMATION REGARDING MASONRY COLORS, FINISHES, BOND, ETC. AT ALL EXPOSED MASONRY WALLS.
ALL MASONRY WALL CONFIGURATIONS INCLUDING WALL OPENINGS SHALL BE COORDINATED WITH CIVIL, MECHANICAL, PLUMBING, ELECTRICAL AND DRAWINGS FROM ALL OTHER DISCIPLINES.
EXPOSED MASONRY SITE WALLS AND RETAINING WALLS GREATER THAN 16 FEET IN LENGTH SHALL HAVE MASONRY CONTROL JOINTS INSTALLED AT THE FOLLOWING MINIMUM SPACING:
12'-0" ON CENTER FOR WALLS 6'-0" MAXIMUM HEIGHT
18'-0" ON CENTER FOR WALLS 10'-0" MAXIMUM HEIGHT
20'-0" ON CENTER FOR WALLS GREATER THAN 10'-0" IN HEIGHT
SEE S7.00 SERIES SHEETS FOR TYPICAL MASONRY DETAILS.
VENEER:
FOR CMU OR BRICK VENEER (5" MAXIMUM, 3" MINIMUM THICKNESS) ATTACHMENT TO STRUCTURAL MASONRY, PROVIDE ADJUSTABLE INTEGRAL ANCHOR TIES. ADJUSTABLE INTEGRAL ANCHOR TIES SHALL BE CORROSION RESISTANT AND HAVE TWO PINTLE LEGS MINIMUM WITH W2.8 (3/16)" WIRE OR APPROVED EQUAL. PROVIDE DUR-O-WALL DA370 ADJUSTABLE INTEGRAL ANCHOR TIES OR APPROVED EQUAL.
FOR CMU OR BRICK VENEER (5" MAXIMUM, 3" MINIMUM THICKNESS) ATTACHMENT TO STRUCTURAL CONCRETE, PROVIDE ADJUSTABLE ANCHOR TIES. ADJUSTABLE ANCHOR TIES SHALL BE CORROSION RESISTANT AND HAVE A TWO PINTLE LEGS MINIMUM WITH A MINIMUM W2.8 (3/16)" WIRE. ATTACH TO CONCRETE WITH 2-1/4" DIAMETER CONCRETE SCREWS, HILTI KWIKCON 11 x 1 1/2" OR APPROVED EQUAL.
FOR CMU OR BRICK VENEER (5" MAXIMUM, 3" MINIMUM THICKNESS) ATTACHMENT TO STRUCTURAL COLD FORMED METAL STUDS, PROVIDE ADJUSTABLE ANCHOR TIES. ADJUSTABLE ANCHOR TIES SHALL BE CORROSION RESISTANT AND HAVE TWO PINTLE LEGS MINIMUM W2.8 (3/16)" WIRE. PROVIDE DUR-O-WALL DA213 ADJUSTABLE ANCHOR TIE OR APPROVED EQUAL. ATTACH THROUGH SHEATHING TO STUDS WITH 2-1/4" x 1 1/2" CORROSION RESISTANT TEK SCREWS.
SEE TYPICAL DETAILS ON SHEET S7.31 FOR VENEER TIE SPACING.
PROVIDE ADDITIONAL ANCHORS AROUND ALL OPENINGS LARGER THAN 16" IN EITHER DIMENSION. SPACE ANCHORS WITHIN 12" OF OPENING PERIMETER AND MATCH HORIZONTAL OR VERTICAL ANCHOR TIE SPACING.
COORDINATE VENEER LOCATION, TYPE, BOND PATTERN, ETC. WITH ARCHITECTURAL DRAWINGS.
PRE-ENGINEERED METAL BUILDING:
FOUNDATION CONFIGURATION AND SIZES SHOWN ON THESE DRAWINGS ARE BASED ON PRELIMINARY DESIGN CALCULATIONS. THESE SIZES MAY REQUIRE MODIFICATIONS PER THE METAL BUILDING MANUFACTURER'S FINAL GRAVITY AND LATERAL DESIGN CALCULATIONS.
THE METAL BUILDING MANUFACTURER SHALL PROVIDE FINAL GRAVITY AND LATERAL DESIGN CALCULATIONS FOR APPROVAL PRIOR TO THE COMMENCEMENT OF FOUNDATION EARTHWORK.
THE BUILDING SHALL BE A MANUFACTURER'S STANDARD PREFABRICATED METAL STRUCTURE OF THE APPROXIMATE INSIDE AREA SHOWN, EXCEPT AS NOTED. RIGID FRAMES SHALL BE SPACED AS SPECIFIED ON THE DRAWINGS, BUT OVERALL DIMENSIONS AND CONSTRUCTION DETAILS MAY VARY TO SUIT MANUFACTURER'S STANDARD DESIGN. MINIMUM WEB THICKNESS OF RIGID FRAMES SHALL BE 3/16".
THE BUILDING SHALL BE DESIGNED AND FABRICATED ACCORDING TO AISC, MBMA AND AISI SPECIFICATIONS. THE DIMENSIONAL TOLERANCES APPLICABLE TO ROLLED FORM STEEL UNDER THE LATEST EDITION OF THE AISC "STANDARD MILL PRACTICE" SECTION SHALL BE REQUIRED IN THE FABRICATION OF THE STEEL BUILDING FRAMES.
THE BUILDING FRAME SHALL BE DESIGNED TO LIMIT THE LATERAL DEFLECTION TO H/240 INCH AT THE BUILDING EAVE FOR THE SPECIFIED BASIC WIND SPEED.
THE BUILDING SHALL BE DESIGNED TO SUPPORT ALL MECHANICAL EQUIPMENT INCLUDING HEATERS, SPRINKLERS, EXHAUST SYSTEMS AND ALL OTHER DEVICES. ADDITIONAL GIRTS OR PURLINS SHALL BE PLACED IN CONVENIENT LOCATIONS FOR ATTACHMENT OF ALL MECHANICAL EQUIPMENT. THE CONTRACTOR SHALL COORDINATE THE MECHANICAL LOADS WITH THE METAL BUILDING MANUFACTURER AND THE MECHANICAL DRAWINGS.
DESIGN LOADS SHALL CONFORM WITH THESE GENERAL NOTES. LOAD COMBINATIONS SHALL COMPLY WITH MBMA SPECIFICATIONS.
ANCHOR BOLTS SHOWN ON THESE DRAWINGS ARE BASED ON PRELIMINARY DESIGN CALCULATIONS. THESE SIZES MAY REQUIRE MODIFICATIONS PER THE METAL BUILDING MANUFACTURER'S FINAL GRAVITY AND LATERAL DESIGN CALCULATIONS.
THE METAL BUILDING MANUFACTURER SHALL DESIGN THE SUPPORTS FOR ALL CONNECTIONS OF MASONRY AND/OR METAL STUD WALLS TO THE METAL BUILDING COMPONENTS AND PROVIDE CALCULATIONS FOR THE DESIGN OF THE SUPPORTS.
PREPARE THE SHOP DRAWINGS AND CALCULATIONS UNDER THE SEAL OF A PROFESSIONAL STRUCTURAL ENGINEER REGISTERED IN THE STATE THE PROJECT IS LOCATED.
COORDINATE WITH ARCHITECTURAL DRAWINGS.
GLASS CURTAIN WALL SYSTEM:
ALL LATERAL AND GRAVITY SUPPORT FOR THE GLASS CURTAIN WALL SYSTEM SHALL BE PER THE MANUFACTURER'S RECOMMENDATIONS. SHOP DRAWINGS AND STAMPED CALCULATIONS SHALL BE SUBMITTED FOR APPROVAL BY THE ENGINEER OF RECORD AND THE ARCHITECT PRIOR TO INSTALLATION.
THE ENGINEER STAMPING THE SHOP DRAWINGS SHALL BE REGISTERED IN THE STATE THAT THE PROJECT IS LOCATED.
THE BEAMS AT ALL FLOORS HAVE BEEN DESIGNED TO SUPPORT THE GRAVITY LOAD OF THE GLASS CURTAIN WALL SYSTEM. THE GLASS CURTAIN WALL SYSTEM SHALL BE LATERALLY SUPPORTED AT ALL FLOORS AND ROOF LEVEL.



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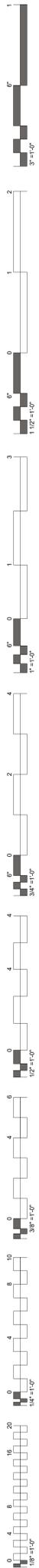
PROJECT PHASE:
BID PACKAGE 01

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DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER: S0.02

GENERAL STRUCTURAL NOTES



GENERAL STRUCTURAL NOTES

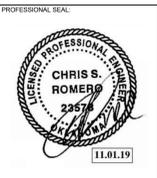
ELEVATORS:
 THE STRUCTURE HAS BEEN DESIGNED FOR A KONE ELEVATOR.
 ALL STRUCTURAL SUPPORTS, FLOOR PENETRATION SIZES AND PIT DIMENSIONS HAVE BEEN DESIGNED BASED ON THE ABOVE INFORMATION. SHOULD THE ACTUAL ELEVATOR(S) SELECTED DIFFER FROM THE INFORMATION PROVIDED ABOVE, THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY ADDITIONAL CONSTRUCTION AND REDESIGN COSTS ASSOCIATED WITH THE ALTERNATE ELEVATOR(S).
 THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING ALL ELEVATOR PIT AND FLOOR PENETRATION LOCATIONS AND DIMENSIONS.
 THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING ALL ELEVATOR OVERRUN REQUIREMENTS.
 ELEVATOR CAR RAIL AND COUNTERWEIGHT RAIL SUPPORTS SHALL BE PROVIDED BY THE CONTRACTOR. CAR RAIL AND COUNTERWEIGHT RAIL SUPPORTS SHALL BE PROVIDED AT AND BETWEEN ALL FLOOR LEVELS SERVICED BY THE ELEVATOR, ABOVE THE LAST STOP OF THE ELEVATOR, AND BETWEEN THE BASEMENT AND THE GROUND FLOOR AS REQUIRED BY THE ELEVATOR MANUFACTURER. IF THE ELEVATOR MANUFACTURER REQUIRES RAIL SUPPORTS THAT DIFFER FROM THOSE PROVIDED, THE ELEVATOR MANUFACTURER SHALL BE RESPONSIBLE FOR ADDITIONAL CONSTRUCTION COST AND DESIGN COST.
 THE CONTRACTOR SHALL VERIFY THE DESIGN OF THE HOISTS/SAFETY BEAM AND CONNECTIONS AS REQUIRED PER THE ELEVATOR MANUFACTURER.
 STRUCTURAL ELEMENTS AFFECTED BY THE ELEVATOR LAYOUT SHALL NOT BE FABRICATED PRIOR TO APPROVAL OF ELEVATOR SHOP DRAWINGS.
SPECIAL INSPECTION:
 THE OWNER SHALL PROVIDE FOR SERVICES OF A CERTIFIED INSPECTOR (APPROVED BY THE BUILDING OFFICIAL OR THE ENGINEER OF RECORD) IN ACCORDANCE WITH CHAPTER 17 OF THE INTERNATIONAL BUILDING CODE FOR THE SPECIAL INSPECTION ITEMS NOTED ON SHEET S0.03.
DEFERRED SUBMITTALS:
 THE DEFERRED SUBMITTALS LISTED BELOW ARE THOSE PORTIONS OF THE DESIGN THAT ARE NOT COMPLETED AT THE TIME OF APPLICATION AND ARE TO BE SUBMITTED TO THE BUILDING OFFICIAL AND APPROVED PRIOR TO THE INSTALLATION OF THOSE ITEMS. THE MANUFACTURER, CONSULTANT, OR CONTRACTOR, AS APPROPRIATE, SHALL PROVIDE SUBMITTALS TO THE ENGINEER OF RECORD FOR REVIEW FOR THE FOLLOWING ITEMS:
 SPECIAL STEEL JOISTS
 METAL STAIRS
 EXTERIOR COLD-FORMED METAL FRAMING
 INTERIOR COLD-FORMED METAL FRAMING
 TEMPORARY SHORING
 HANDRAILS
 CURTAIN WALL AND STOREFRONT
 AGGREGATE PIERS / STONE COLUMN GROUND IMPROVEMENT
 PRECAST CONCRETE COLUMNS AND ASSOCIATED ATTACHMENTS AND ANCHORAGE

SCHEDULE OF STRUCTURAL SPECIAL INSPECTIONS

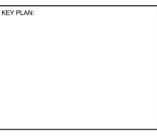
- SPECIAL INSPECTIONS / TESTING - "SPECIAL STRUCTURAL INSPECTION" SHALL NOT RELIEVE THE OWNER OR THEIR AGENT FROM HAVING THE INSPECTIONS OF THE JURISDICTION BUILDING DEPARTMENT PER SECTION 110 OF THE IBC PERFORMED, BOTH THE JURISDICTION BUILDING DEPARTMENT INSPECTIONS AND "SPECIAL STRUCTURAL INSPECTION" SHALL BE PERFORMED.
- THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR NOTIFYING THE JURISDICTION BUILDING OFFICIAL AND SPECIAL INSPECTOR WHEN WORK IS READY FOR INSPECTION.
- REPORTING FOR SPECIAL INSPECTION - SPECIAL INSPECTION AND TESTING REPORTS SHALL BE COMPLETED AND DISTRIBUTED AT THE COMPLETION OF EACH TASK. IF A TASK IS TO TAKE LONGER THAN THREE (3) DAYS, PROVIDE REPORTS FOR EACH DAY. PROVIDE COPIES OF REPORTS TO CONTRACTOR, OWNER, ARCHITECT AND STRUCTURAL ENGINEER OF RECORD. SPECIAL INSPECTOR TO KEEP A NON-COMPLIANCE LIST DOCUMENTING ITEMS INSPECTED NOT MEETING APPROVED CONSTRUCTION DOCUMENTS AND WHEN / HOW RESOLVED.
- SEE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING CONSTRUCTION DOCUMENTS FOR ADDITIONAL NON-STRUCTURAL SPECIAL INSPECTION ITEMS.
- SPECIAL INSPECTION OF SHOP FABRICATED MEMBERS AND ASSEMBLIES SHALL BE IN ACCORDANCE WITH SECTION 1704.2, UNLESS FABRICATOR IS APPROVED TO PERFORM WORK WITHOUT SPECIAL INSPECTION.
- IN ACCORDANCE WITH IBC CHAPTER 17, THE OWNER OR THE OWNER'S AGENT, OTHER THAN THE CONTRACTOR, SHALL EMPLOY ONE OR MORE APPROVED AGENCIES TO PROVIDE SPECIAL INSPECTIONS AND TESTS, DURING CONSTRUCTION FOR THE TYPES OF WORK LISTED BELOW THESE SPECIAL INSPECTIONS AND TESTS ARE IN ADDITION TO THE INSPECTIONS BY THE BUILDING OFFICIAL IDENTIFIED IN IBC SECTION 110.
- DEFINITIONS:
 * **SPECIAL INSPECTION:** INSPECTION AS HEREIN REQUIRED BY A QUALIFIED SPECIAL INSPECTOR COMPETENT WITH THE MATERIALS, INSTALLATION, FABRICATION, ERECTION OR PLACEMENT OF COMPONENTS AND CONNECTIONS REQUIRING SPECIAL EXPERTISE TO ENSURE COMPLIANCE WITH APPROVED CONSTRUCTION DOCUMENTS AND REFERENCED STANDARDS (SEE SECTION 1704).
 * **CONTINUOUS SPECIAL INSPECTION:** FULL-TIME OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED.
 * **PERIODIC SPECIAL INSPECTION:** THE PART-TIME OR INTERMITTENT OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK HAS BEEN OR IS BEING PERFORMED AND AT THE COMPLETION OF THE WORK.

ITEM	DESCRIPTION OF REQUIREMENTS	REQUIRED (YES/NO)
SPECIAL INSPECTION OF STRUCTURAL STEEL	TO BE PERFORMED IN ACCORDANCE WITH CHAPTER N OF AISC 360-10	YES
SPECIAL INSPECTION AND VERIFICATION OF STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.2	YES
SPECIAL INSPECTIONS AND VERIFICATIONS FOR CONCRETE CONSTRUCTION	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.3	YES
SPECIAL INSPECTIONS AND VERIFICATIONS FOR MASONRY CONSTRUCTION	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.4 AND REFERENCED STANDARDS	YES
SPECIAL INSPECTIONS AND VERIFICATIONS FOR WOOD CONSTRUCTION	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.5	NO
SPECIAL INSPECTIONS AND VERIFICATIONS OF SOILS	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.6, THE GEOTECHNICAL REPORT LISTED IN THE GENERAL FOUNDATION NOTES, AND ANY OTHER REQUIREMENTS LISTED IN THE GENERAL FOUNDATION NOTES	YES
SPECIAL INSPECTIONS AND VERIFICATIONS FOR DEEP FOUNDATIONS (DRIVEN PILES, CAST-IN-PLACE, OR HELICAL PILES AS APPLICABLE)	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTIONS 1705.7-1705.9 AS APPLICABLE, THE GEOTECHNICAL REPORT LISTED IN THE GENERAL FOUNDATION NOTES, AND ANY OTHER REQUIREMENTS LISTED IN THE CONSTRUCTION DOCUMENTS	NO
SPECIAL INSPECTIONS FOR WIND RESISTANCE (REQUIRED ONLY FOR V _W = 155MPH OR GREATER IN EXPOSURE CATEGORY B, OR V _W = 142MPH OR GREATER IN EXPOSURE CATEGORY C OR D)	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.11	NO
SPECIAL INSPECTIONS AND VERIFICATIONS FOR SEISMIC RESISTANCE (REQUIRED FOR STRUCTURES ASSIGNED TO CATEGORIES C, D, E, OR F)	TO BE PERFORMED IN ACCORDANCE WITH ALL APPLICABLE PORTIONS OF IBC SECTIONS 1705.12 AND 1705.13	NO

ADDITIONAL INSPECTIONS REQUIRED PER SIDEPLATE SYSTEMS ON SHEET S8.01



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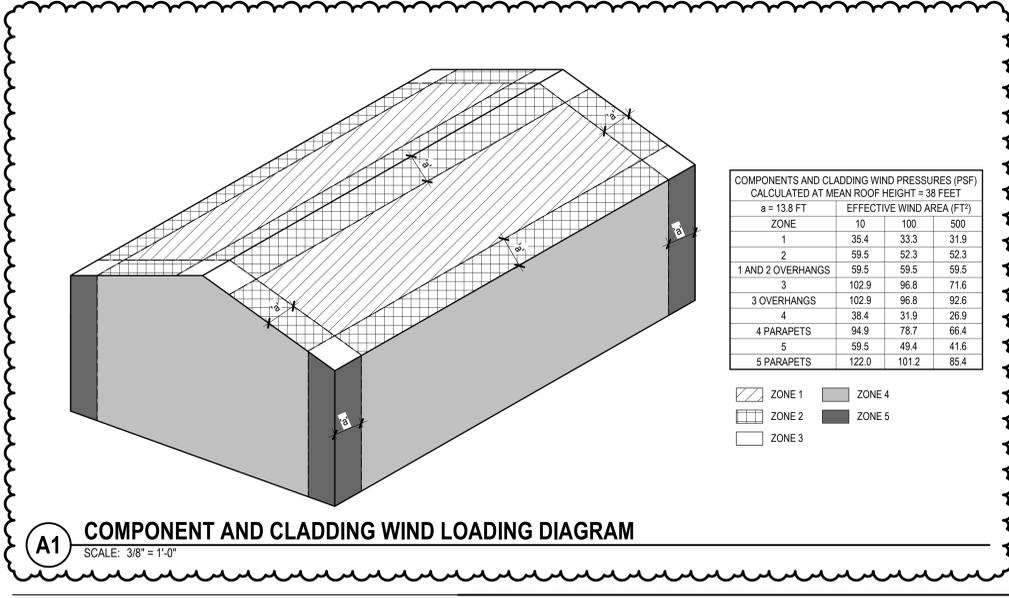
PROJECT PHASE:
 BID PACKAGE 01

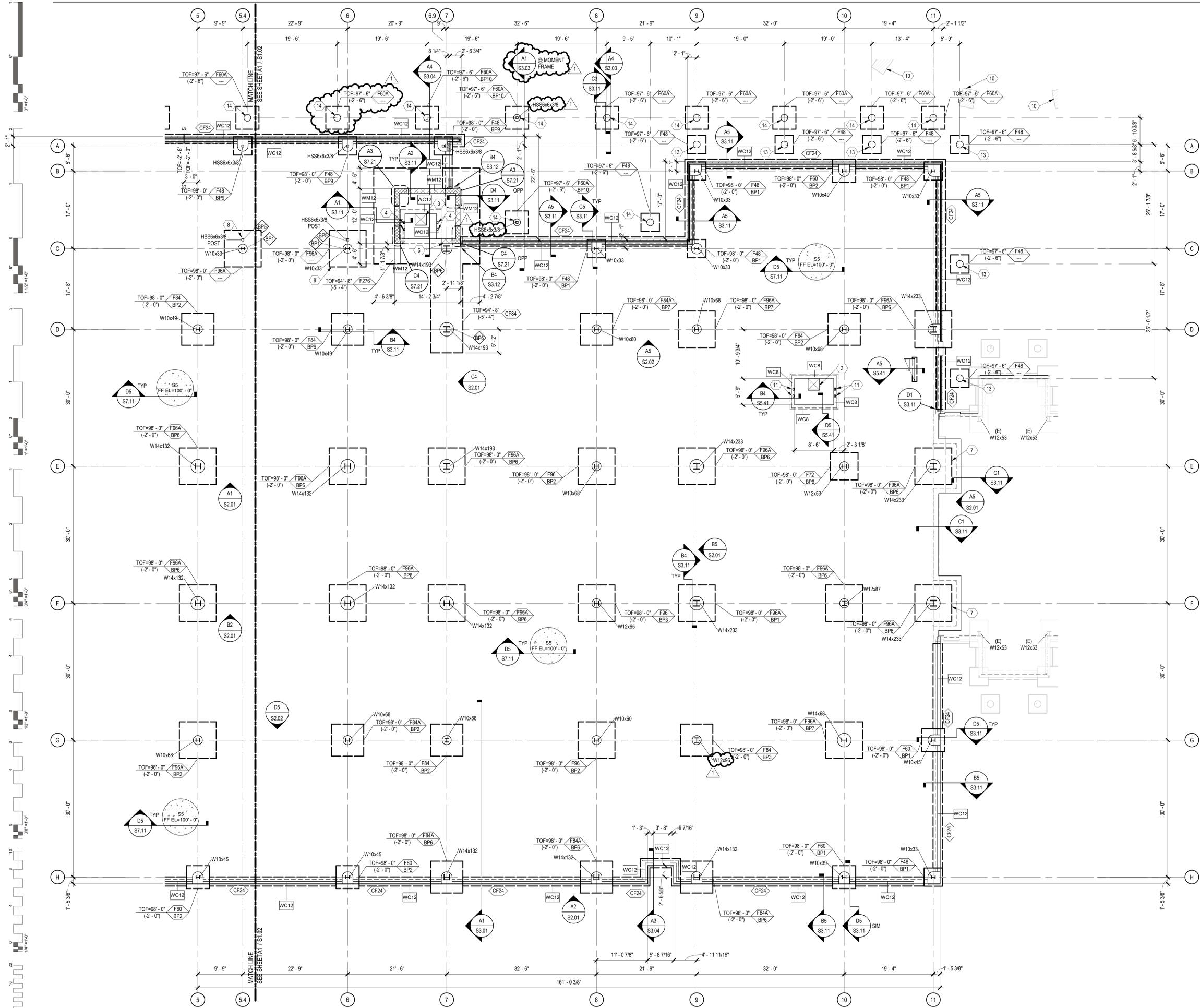
#	DATE	REVISIONS / DESCRIPTION
1	11/02/19	BID PACKAGE 01 - ADD 01

DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER:
 S0.03

GENERAL STRUCTURAL NOTES AND SPECIAL INSPECTIONS





GENERAL SHEET NOTES

- SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- REFERENCE FINISH FLOOR ELEVATION 100'-0" = MEAN SEA FINISH FLOOR ELEVATION. SEE CIVIL DRAWINGS.
- TOP OF FOOTING ELEVATION = 98'-0" (-2'-0"), UNLESS NOTED OTHERWISE ON PLAN.
- NOTE TO CONTRACTOR: ENLARGED SLAB BLOCKOUTS MAY BE REQUIRED AT FRAME COLUMNS FOR MOMENT FRAME BASE PLATE CLEARANCE.
- NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
- DIMENSIONS ARE TO THE FACE OF STUD UNLESS NOTED OTHERWISE.
- SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
- EXISTING CONSTRUCTION IS PER AVAILABLE EXISTING DRAWINGS. ALL EXISTING CONSTRUCTION AND DIMENSIONS SHALL BE VERIFIED PRIOR TO CONSTRUCTION. SHOULD CONDITIONS VARY FROM THOSE SHOWN, CONTACT ENGINEER BEFORE PROCEEDING.
- PROVIDE SLAB JOINTS AT 10'-0" ON CENTER MAXIMUM. THE AREA OF THE CONTROL JOINT SHALL NOT EXCEED A 2:1 RATIO. CONTROL JOINTS SHALL BE LOCATED AT COLUMN LINES WHERE THE LAYOUT PERMITS. AT RE-ENTRANT CORNERS THAT DO NOT HAVE CONTROL JOINTS, PROVIDE 2#4 x 3'-0" DIAGONAL TO THE RE-ENTRANT CORNER.
- STRUCTURAL COLD FORMED METAL STUDS SHALL BE 6" WIDE UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING BY OTHERS.
- SEE SHEET S7.00 SERIES SHEETS FOR TYPICAL FOUNDATION SECTIONS AND DETAILS.
- SEE SHEET S6.01 FOR SCHEDULES.

SHEET KEYNOTE

- FLOOR DRAIN: SLOPE SLAB TO DRAIN 1/8" PER FOOT. COORDINATE EXACT SIZE AND LOCATION WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
- POST AND FOUNDATION AS REQUIRED FOR STAIR SUPPORT. STAIR ENGINEER TO PROVIDE REQUIRED LOADS AND LOCATIONS.
- ELEVATOR SUMP PIT. COORDINATE EXACT SIZE AND LOCATION WITH ELEVATOR MANUFACTURER. SEE A4 / S5.41
- HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION AND SPACING WITH ELEVATOR MANUFACTURER. SEE B4 / S5.41
- PRE-ENGINEERED METAL BUILDING STEEL AND ANCHORAGE BY OTHERS. CONTRACTOR TO CONFIRM LOCATIONS OF FOUNDATIONS WITH FINAL PRE-ENGINEERED METAL BUILDING SHOP DRAWINGS.
- NOTCH MASONRY AS REQUIRED TO FACILITATE BASEPLATE INSTALLATION. STEP BOND BEAM AT THIS LOCATION. FILL VOID FROM NOTCH WITH NON-SHRINK GROUT.
- CUT AND REMOVE EXISTING SLAB AS REQUIRED TO PLACE NEW FOOTING. NEW SLAB TO POUR UP TO REMAINING SLAB.
- CENTER FOOTING ON GRID C.
- F60A PRE-MANUFACTURED SUNSHADE CONCRETE FOOTING. TOP OF FOOTING = 99'-0" (+1'-0"). SEE SHEET S6.01 FOR FOOTING SCHEDULE. COORDINATE FINAL LOCATION WITH SUNSHADE MANUFACTURER.
- EXISTING CANOPY. SEE ARCHITECTURAL DEMOLITION PLANS FOR EXTENT OF DEMOLITION.
- HSS6x4x1/2 ELEVATOR SUPPORT POST. COORDINATE EXACT LOCATION AND SPACING WITH ELEVATOR MANUFACTURER. SEE B4 / S5.41, D3 / S5.41, A2 / S5.41, B2 / S5.41, AND C2 / S5.41
- 1 1/2" RECESSED SLAB AT ADA SHOWER. COORDINATE EXACT SIZE, LOCATION, AND SLOPE REQUIREMENTS WITH ARCHITECTURAL DRAWINGS. SEE C4 / S7.11
- 18" DIAMETER PRECAST CONCRETE COLUMN BY OTHERS. SEE C3 / S3.11 AND B1 / S3.31
- 18" DIAMETER PRECAST CONCRETE CANOPY COLUMN BY OTHERS. SEE C3 / S3.11, C4 / S3.12, A1 / S3.31, AND A5 / S3.31

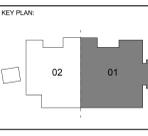
A1 FOUNDATION PLAN - SECTOR 1
SCALE: 1/8" = 1'-0"



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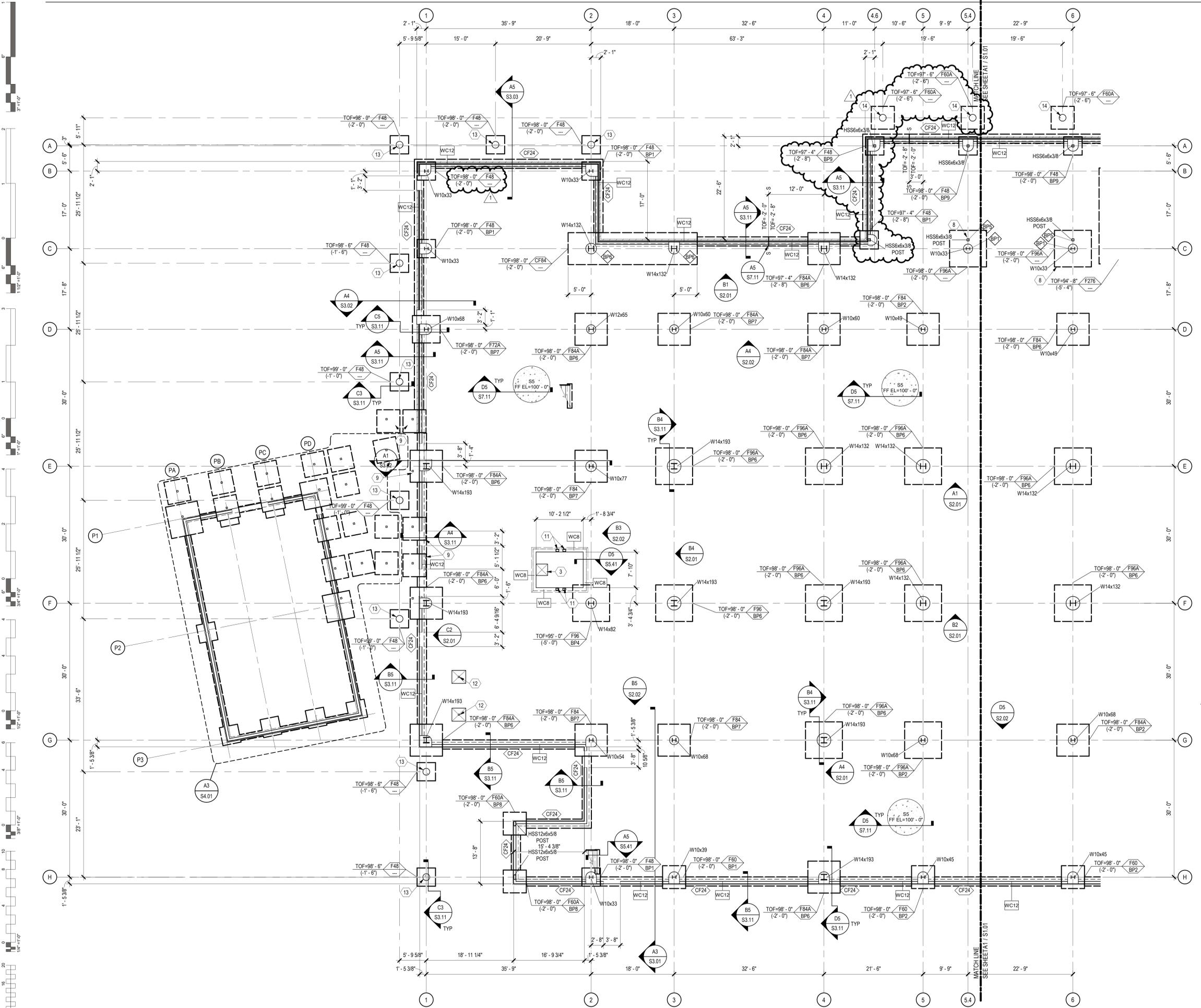
PROJECT PHASE:
BID PACKAGE 01

#	DATE	REVISION / DESCRIPTION
1	11/02/19	BID PACKAGE 01 - ADD 01

DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER: S1.01

FOUNDATION PLAN SECTOR 1



GENERAL SHEET NOTES

- SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- REFERENCE FINISH FLOOR ELEVATION 100'-0" = MEAN SEA FINISH FLOOR ELEVATION. SEE CIVIL DRAWINGS.
- TOP OF FOOTING ELEVATION = 98'-0" (-2'-0"), UNLESS NOTED OTHERWISE ON PLAN.
- NOTE TO CONTRACTOR: ENLARGED SLAB BLOCKOUTS MAY BE REQUIRED AT FRAME COLUMNS FOR MOMENT FRAME BASE PLATE CLEARANCE.
- NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
- DIMENSIONS ARE TO THE FACE OF STUD UNLESS NOTED OTHERWISE.
- SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
- EXISTING CONSTRUCTION IS PER AVAILABLE EXISTING DRAWINGS. ALL EXISTING CONSTRUCTION AND DIMENSIONS SHALL BE VERIFIED PRIOR TO CONSTRUCTION. SHOULD CONDITIONS VARY FROM THOSE SHOWN, CONTACT ENGINEER BEFORE PROCEEDING.
- PROVIDE SLAB JOINTS AT 10'-0" ON CENTER MAXIMUM. THE AREA OF THE CONTROL JOINT SHALL NOT EXCEED A 2:1 RATIO. CONTROL JOINTS SHALL BE LOCATED AT COLUMN LINES WHERE THE LAYOUT PERMITS. AT RE-ENTRANT CORNERS THAT DO NOT HAVE CONTROL JOINTS, PROVIDE 2#4 x 3'-0" DIAGONAL TO THE RE-ENTRANT CORNER.
- STRUCTURAL COLD FORMED METAL STUDS SHALL BE 6" WIDE UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING BY OTHERS.
- SEE SHEET S7.00 SERIES SHEETS FOR TYPICAL FOUNDATION SECTIONS AND DETAILS.
- SEE SHEET S6.01 FOR SCHEDULES.

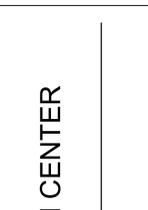
SHEET KEYNOTE

- FLOOR DRAIN, SLOPE SLAB TO DRAIN 1/8" PER FOOT. COORDINATE EXACT SIZE AND LOCATION WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
- POST AND FOUNDATION AS REQUIRED FOR STAIR SUPPORT. STAIR ENGINEER TO PROVIDE REQUIRED LOADS AND LOCATIONS.
- ELEVATOR SUMP PIT. COORDINATE EXACT SIZE AND LOCATION WITH ELEVATOR MANUFACTURER. SEE A4 / S5.41
- HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION AND SPACING WITH ELEVATOR MANUFACTURER. SEE B4 / S5.41
- PRE-ENGINEERED METAL BUILDING STEEL AND ANCHORAGE BY OTHERS. CONTRACTOR TO CONFIRM LOCATIONS OF FOUNDATIONS WITH FINAL PRE-ENGINEERED METAL BUILDING SHOP DRAWINGS.
- NOTCH MASONRY AS REQUIRED TO FACILITATE BASEPLATE INSTALLATION. STEP BOND BEAM AT THIS LOCATION. FILL VOID FROM NOTCH WITH NON-SHRINK GROUT.
- CUT AND REMOVE EXISTING SLAB AS REQUIRED TO PLACE NEW FOOTING. NEW SLAB TO POUR UP TO REMAINING SLAB.
- CENTER FOOTING ON GRID C.
- F60A PRE-MANUFACTURED SUNSHADE CONCRETE FOOTING. TOP OF FOOTING = 99'-0" (-1'-0"). SEE SHEET S6.01 FOR FOOTING SCHEDULE. COORDINATE FINAL LOCATION WITH SUNSHADE MANUFACTURER.
- EXISTING CANOPY. SEE ARCHITECTURAL DEMOLITION PLANS FOR EXTENT OF DEMOLITION.
- HSS6x6x1/2 ELEVATOR SUPPORT POST. COORDINATE EXACT LOCATION AND SPACING WITH ELEVATOR MANUFACTURER. SEE B4 / S5.41, D3 / S5.41, A2 / S5.41, B2 / S5.41, AND C2 / S5.41
- 1 1/2" RECESSED SLAB AT ADA SHOWER. COORDINATE EXACT SIZE, LOCATION, AND SLOPE REQUIREMENTS WITH ARCHITECTURAL DRAWINGS. SEE C4 / S7.11
- 18" DIAMETER PRECAST CONCRETE COLUMN BY OTHERS. SEE C3 / S3.11 AND B1 / S3.31
- 18" DIAMETER PRECAST CONCRETE CANOPY COLUMN BY OTHERS. SEE C3 / S3.11, C4 / S3.12, A1 / S3.31, AND A5 / S3.31

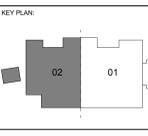
A1 FOUNDATION PLAN - SECTOR 2
SCALE: 1/8" = 1'-0"



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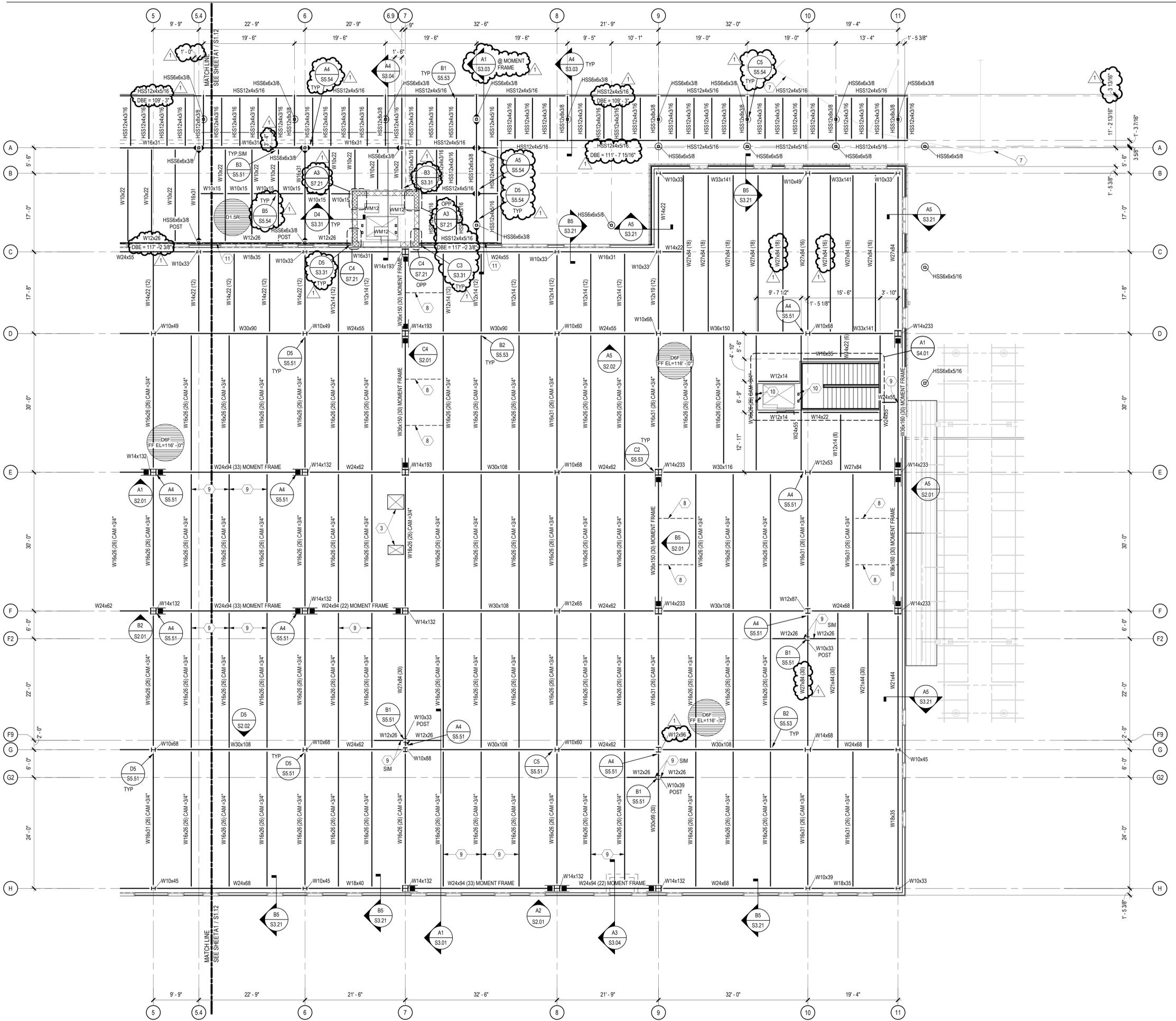
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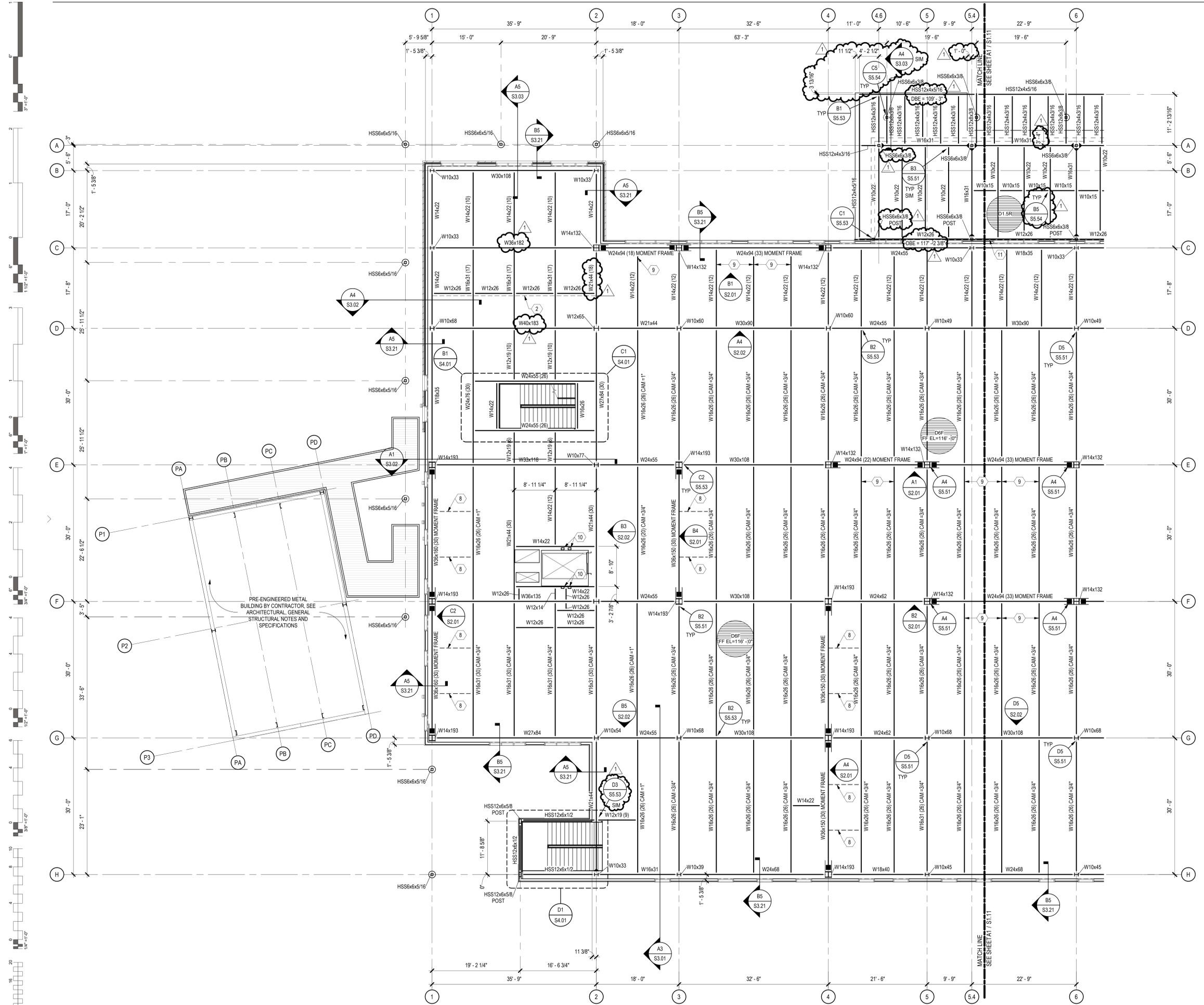
DATE: 11-01-19
JOB NUMBER: 18-01.01

SHEET NUMBER:
S1.02

FOUNDATION PLAN
SECTOR 2







A1 FLOOR FRAMING PLAN - SECTOR 2
SCALE: 1/8" = 1'-0"

GENERAL SHEET NOTES

- SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
- DIMENSIONS ARE TO THE FACE OF STUD UNLESS NOTED OTHERWISE.
- SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
- EXISTING CONSTRUCTION IS PER AVAILABLE EXISTING DRAWINGS. ALL EXISTING CONSTRUCTION AND DIMENSIONS SHALL BE VERIFIED PRIOR TO CONSTRUCTION. SHOULD CONDITIONS VARY FROM THOSE SHOWN, CONTACT ENGINEER BEFORE PROCEEDING.
- STRUCTURAL COLD FORMED METAL STUDS SHALL BE 60S162-43 AT 16" ON CENTER UNLESS NOTED OTHERWISE.
- BEAMS AND JOISTS ARE SPACED EQUALLY BETWEEN GRIDS AND COLUMNS UNLESS NOTED OTHERWISE.
- SEE SHEET S7.00 SERIES SHEETS FOR TYPICAL FLOOR FRAMING SECTIONS.
- SEE SHEET S6.01 FOR SCHEDULES.
- ⊕ DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
- ⊕ DENOTES SIDEPATE MOMENT CONNECTION. SEE SIDEPATE DRAWINGS.

SHEET KEYNOTE

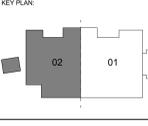
- MECHANICAL UNIT. COORDINATE EXACT SIZE AND LOCATION WITH MECHANICAL DRAWINGS.
- OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A5 / S5.52 AND B5 / S5.52 FOR SUPPORT.
- MECHANICAL OPENING. COORDINATE EXACT SIZE AND LOCATION WITH MECHANICAL DRAWINGS. SEE C5 / S7.42
- HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST. COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE A2 / S5.41, B2 / S5.41, C2 / S5.41, AND D3 / S5.41
- HSS6x4x1/4 COLLECTOR BLOCKING BETWEEN BEAMS. SEE D4 / S5.52. ATTACH BLOCKING TO DECK VALLEYS PER DECK SCHEDULE. PROVIDE 20 GAGE PLATE AS REQUIRED TO MAKE ATTACHMENT.
- 4" HOUSEKEEPING PAD REINFORCED WITH #4 @ 18" ON CENTER EACH WAY AND #4 VERT DOWNELS DRILLED AND EPOXIED 2" INTO CONCRETE SLAB BELOW @ 48" ON CENTER EACH WAY (12" FROM EDGES AND CORNERS). PAD SHALL EXTEND 6" BEYOND FACE OF MECHANICAL UNIT ALL AROUND. COORDINATE EXACT SIZE AND LOCATION OF PAD WITH MECHANICAL DRAWINGS.
- EXISTING CANOPY. SEE ARCHITECTURAL DEMOLITION PLANS FOR EXTENT OF DEMOLITION.
- BOTTOM FLANGE BRACING AT EQUAL SPACING. UNLESS NOTED OTHERWISE, BRACE TO BE ATTACHED TO BOTTOM FLANGE OF BEAM NOTED AS MOMENT FRAME OR BRACED FRAME TO TOP FLANGE OF ADJACENT BEAM. SEE B3 / S5.52
- BOTTOM FLANGE BRACING. SEE A3 / S5.52
- HSS6x4x1/2 ELEVATOR RAIL SUPPORT BEAM. COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE A1 / S5.41 AND B1 / S5.41 FOR TYPICAL DETAILS.
- 2" BUILDING EXPANSION JOINT. SEE ARCHITECTURAL DRAWINGS.
- SLAB EDGE TO BE LOCATED 6" FROM GRID. SEE S7.41 FOR SLAB EDGE DETAILS.



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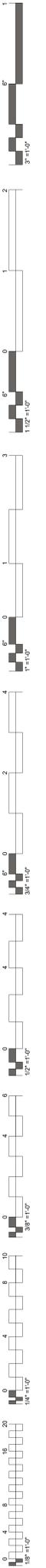
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BID PACKAGE 01

#	DATE	REVISIONS	DESCRIPTION
1	11/02/19	BID PACKAGE 01 - ADD 01	

DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER:
S1.12

FLOOR FRAMING PLAN - SECTOR 2



GENERAL SHEET NOTES

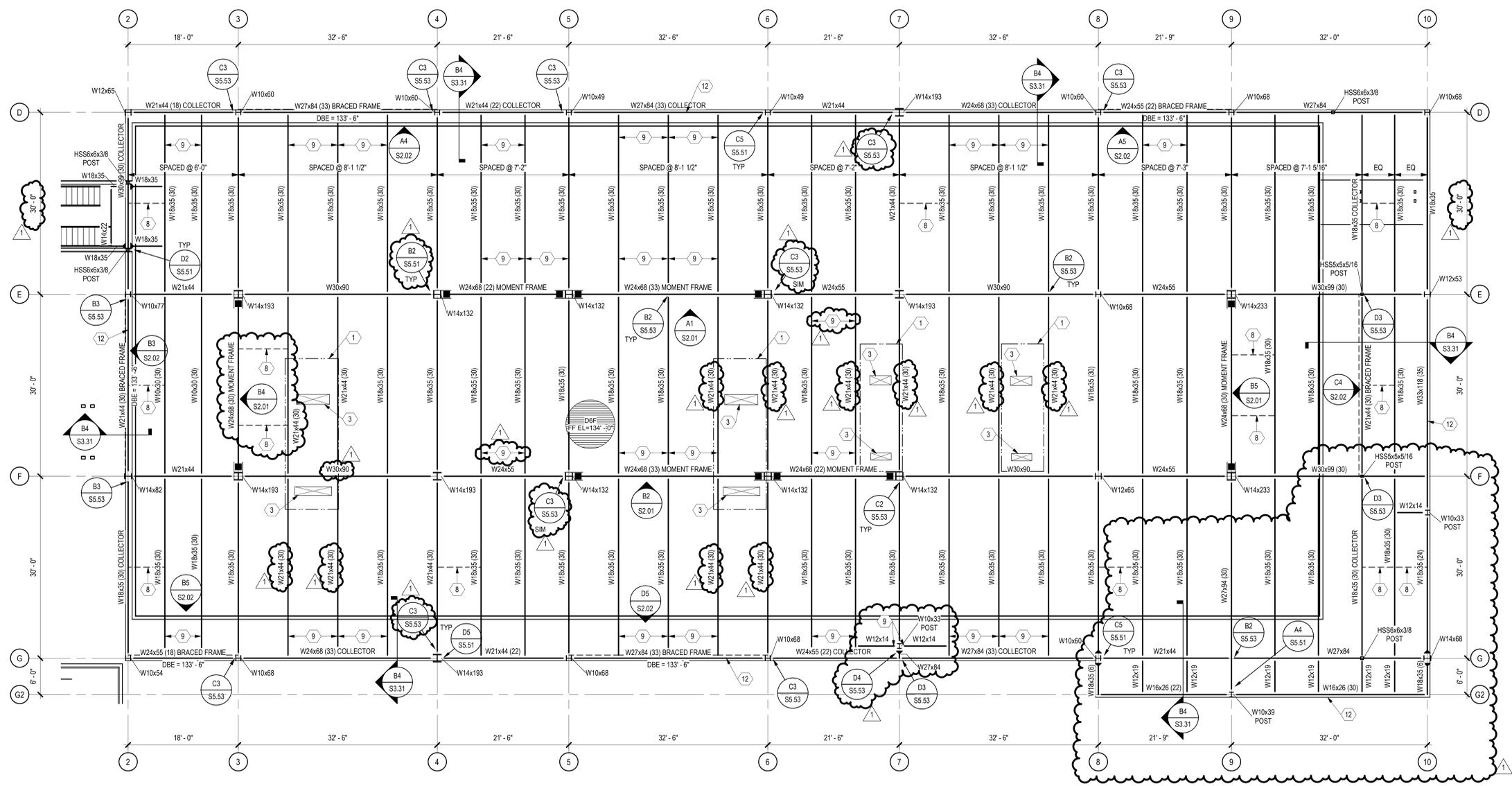
- SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
- DIMENSIONS ARE TO THE FACE OF STUD UNLESS NOTED OTHERWISE.
- SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
- EXISTING CONSTRUCTION IS PER AVAILABLE EXISTING DRAWINGS. ALL EXISTING CONSTRUCTION AND DIMENSIONS SHALL BE VERIFIED PRIOR TO CONSTRUCTION. SHOULD CONDITIONS VARY FROM THOSE SHOWN, CONTACT ENGINEER BEFORE PROCEEDING.
- STRUCTURAL COLD FORMED METAL STUDS SHALL BE 60S162-43 AT 16" ON CENTER UNLESS NOTED OTHERWISE.
- BEAMS AND JOISTS ARE SPACED EQUALLY BETWEEN GRIDS AND COLUMNS UNLESS NOTED OTHERWISE.
- SEE SHEET S7.00 SERIES SHEETS FOR TYPICAL FLOOR FRAMING SECTIONS.
- SEE SHEET S6.01 FOR SCHEDULES.
- ▬ DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
- ▬ DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE DRAWINGS.



SHEET KEYNOTE

- MECHANICAL UNIT. COORDINATE EXACT SIZE AND LOCATION WITH MECHANICAL DRAWINGS.
- OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A5 / S5.52 AND B5 / S5.52 FOR SUPPORT.
- MECHANICAL OPENING. COORDINATE EXACT SIZE AND LOCATION WITH MECHANICAL DRAWINGS. SEE C5 / S7.42
- HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST. COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE A2 / S5.41, B2 / S5.41, C2 / S5.41, AND D3 / S5.41
- HSS6x4x1/4 COLLECTOR BRACING BETWEEN BEAMS. SEE D4 / S5.52. ATTACH BRACING TO DECK VALLEYS PER DECK SCHEDULE. PROVIDE 20 GAGE PLATE AS REQUIRED TO MAKE ATTACHMENT.
- 4" HOUSEKEEPING PAD REINFORCED WITH #4 @ 18" ON CENTER EACH WAY AND #4 VERT BOWELS DRILLED AND EPOXYED 2" INTO CONCRETE SLAB BELOW @ 48" ON CENTER EACH WAY (12" FROM EDGES AND CORNERS). PAD SHALL EXTEND 6" BEYOND FACE OF MECHANICAL UNIT ALL AROUND. COORDINATE EXACT SIZE AND LOCATION OF PAD WITH MECHANICAL DRAWINGS.
- EXISTING CANOPY. SEE ARCHITECTURAL DEMOLITION PLANS FOR EXTENT OF DEMOLITION.
- BOTTOM FLANGE BRACING AT EQUAL SPACING. UNLESS NOTED OTHERWISE, BRACE TO BE ATTACHED TO BOTTOM FLANGE OF BEAM NOTED AS MOMENT FRAME OR BRACED FRAME TO TOP FLANGE OF ADJACENT BEAM. SEE B3 / S5.52
- BOTTOM FLANGE BRACING. SEE A3 / S5.52
- HSS6x4x1/2 ELEVATOR RAIL SUPPORT BEAM. COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE A1 / S5.41 AND B1 / S5.41 FOR TYPICAL DETAILS.
- 2" BUILDING EXPANSION JOINT. SEE ARCHITECTURAL DRAWINGS.
- SLAB EDGE TO BE LOCATED 6" FROM GRID. SEE S7.41 FOR SLAB EDGE DETAILS.

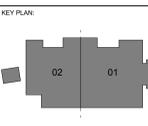
1. 2. 3. 4. 5. 6. 7. 8. 9. 10. 11. 12.



A1 LOW ROOF FRAMING PLAN
SCALE: 1/8" = 1'-0"



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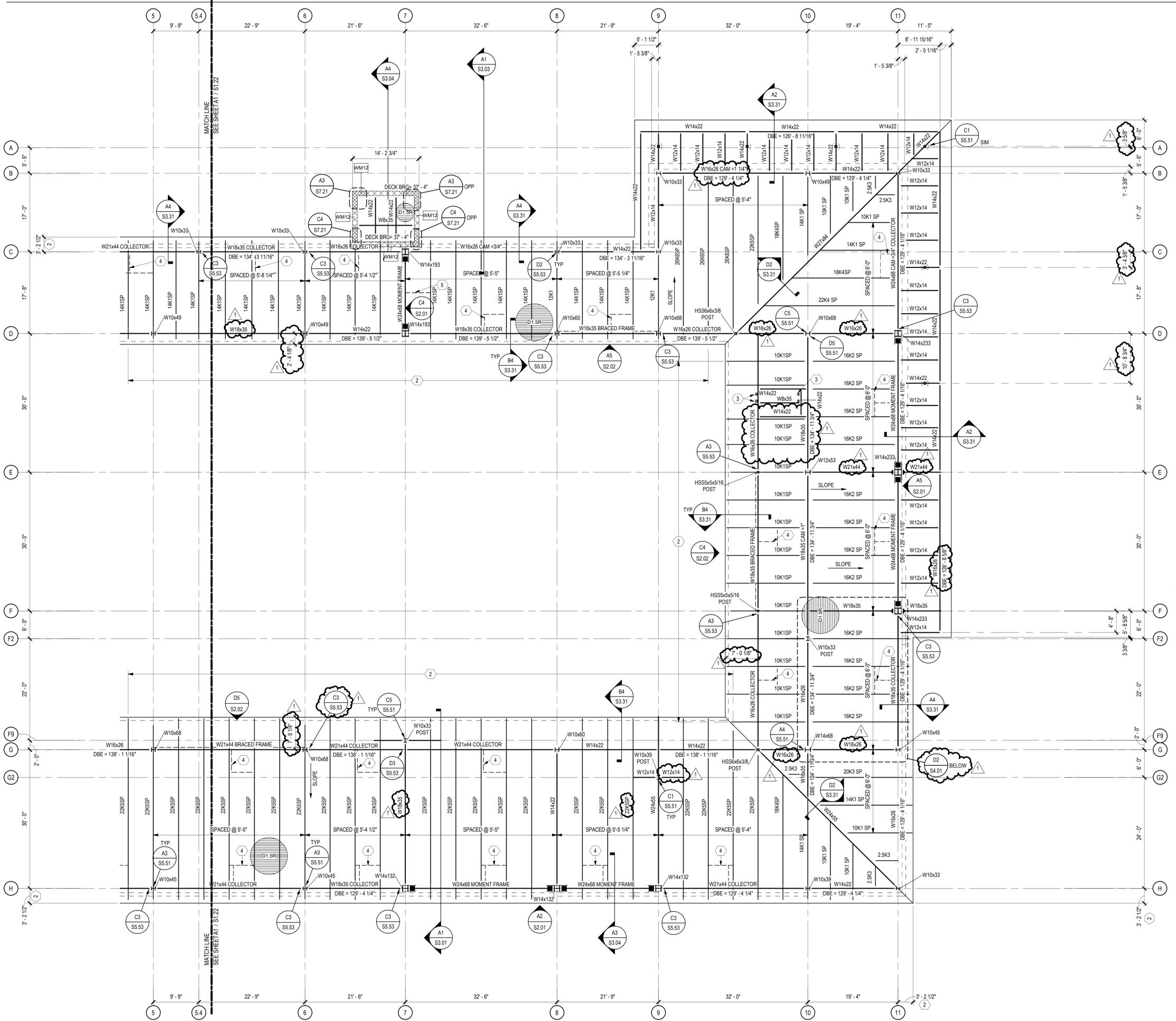
PROJECT PHASE:
BID PACKAGE 01

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| 1 | 11/02/19 | BID PACKAGE 01 | ADD 01 |

DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER: S1.13

LOW ROOF FRAMING PLAN



GENERAL SHEET NOTES

- SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
- DIMENSIONS ARE TO THE FACE OF STUD UNLESS NOTED OTHERWISE.
- SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
- EXISTING CONSTRUCTION IS PER AVAILABLE EXISTING DRAWINGS. ALL EXISTING CONSTRUCTION AND DIMENSIONS SHALL BE VERIFIED PRIOR TO CONSTRUCTION. SHOULD CONDITIONS VARY FROM THOSE SHOWN, CONTACT ENGINEER BEFORE PROCEEDING.
- BEAMS AND JOISTS ARE SPACED EQUALLY BETWEEN GRIDS AND COLUMNS UNLESS NOTED OTHERWISE.
- PROVIDE JOIST BRIDGING PER THE 42ND EDITION OF THE SJI SPECIFICATIONS AND OSHA REQUIREMENTS.
- STEEL JOIST MANUFACTURER SHALL DESIGN ROOF JOISTS AND ROOF JOIST GIRDERS SUPPORTING MECHANICAL UNITS FOR 1.2x MECHANICAL UNIT WEIGHTS SHOWN. USE 28 PSF DEAD LOAD AND 20 PSF LIVE LOAD UNLESS NOTED OTHERWISE. CONTRACTOR SHALL VERIFY ACTUAL MECHANICAL LOADS. NOTIFY STEEL JOIST MANUFACTURER OF ANY DISCREPANCIES.
- STRUCTURAL COLD FORMED METAL STUDS SHALL BE 6" IN WIDTH, UNLESS NOTED OTHERWISE.
- SEE SHEET S6.01 FOR SCHEDULES.
- NOTES DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
- NOTES DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE DRAWINGS.
- PROVIDE HSSxJOIST SEAT DEPTHx1/4 BETWEEN JOISTS AT ALL BEAMS LABELED AS : MOMENT FRAME, BRACED FRAME, AND COLLECTOR. SEE C2 / S7.41

SHEET KEYNOTE

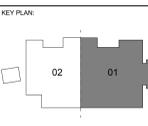
- MECHANICAL UNIT. COORDINATE EXACT SIZE AND LOCATION WITH MECHANICAL DRAWINGS.
- JOIST EXTENDED END. DESIGN EXTENDED END FOR 20 PSF DEAD LOAD, 20 PSF LIVE LOAD, AND ANY POSITIVE OR NEGATIVE WIND PRESSURES PER ROOF WIND LOADING DIAGRAM ON S0.03. DEPTH OF EXTENDED END PER JOIST MANUFACTURER.
- HSS8x12 ELEVATOR RAIL SUPPORT POST. COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE A2 / S5.41, B2 / S5.41, C2 / S5.41, AND D3 / S5.41
- BOTTOM FLANGE BRACE AT EQUAL SPACING, UNLESS NOTED OTHERWISE. BRACE TO BE ATTACHED TO BOTTOM FLANGE OF BEAM NOTED AS MOMENT FRAME OR BRACED FRAME AND TO TOP FLANGE OF ADJACENT BEAM OR JOIST. SEE B3 / S5.52. JOISTS TO BE DESIGNED FOR 1,500# VERTICAL, (REVERSIBLE) WIND AND SEISMIC LOAD FROM BRACE.
- BOTTOM FLANGE BRACING AT EQUAL SPACING, UNLESS NOTED OTHERWISE. SEE D1 / S5.51. JOISTS TO BE DESIGNED FOR 1,500# VERTICAL, (REVERSIBLE) WIND AND SEISMIC LOAD FROM BRACE.
- BOTTOM FLANGE BRACING ANGLE. SEE A3 / S5.52
- R1 JOIST EXTENDED END.



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PROJECT PHASE:
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| 1 | 11/22/19 | BID PACKAGE 01 - ADD 01 |

DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER: S1.21

ROOF FRAMING PLAN - SECTOR 1

A1 ROOF FRAMING PLAN - SECTOR 1
SCALE: 1/8" = 1'-0"



SIDEPLATE CONNECTION NOTES

(AXX) INDICATES SIDEPLATE CONNECTION PER SHEETS S8.01 - S8.07.
 (MX) INDICATES MISCELLANEOUS SIDEPLATE DETAIL PER SHEETS S8.01 - S8.07.



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KEY PLAN:

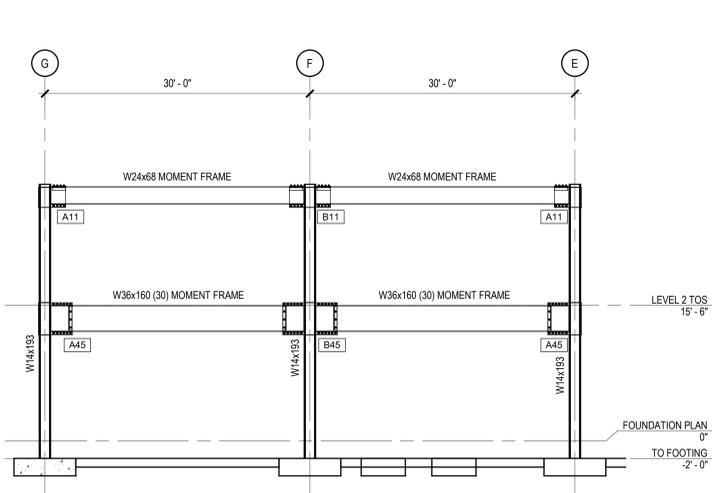
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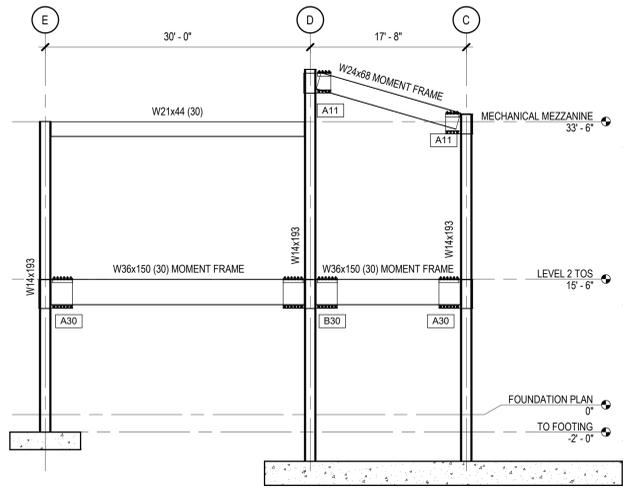
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 JOB NUMBER: 18-01.01

SHEET NUMBER:
S2.01

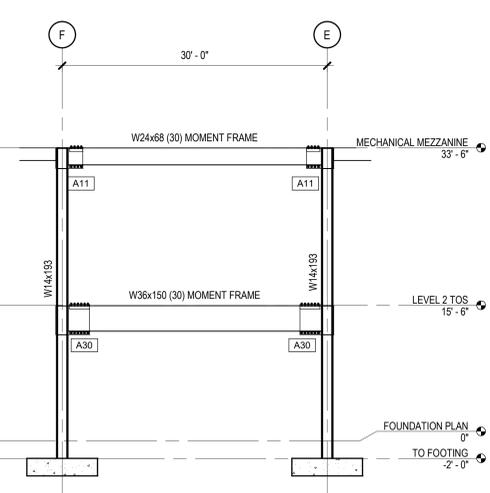
MOMENT FRAME ELEVATIONS



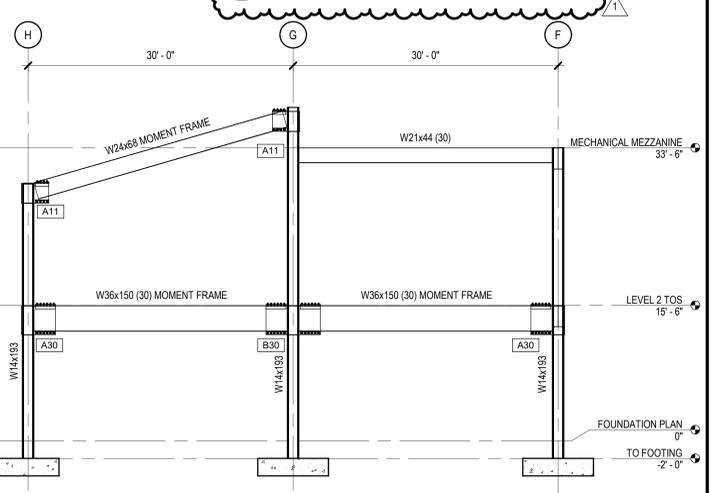
C2 MOMENT FRAME ELEVATION
 SCALE: 1/8" = 1'-0"



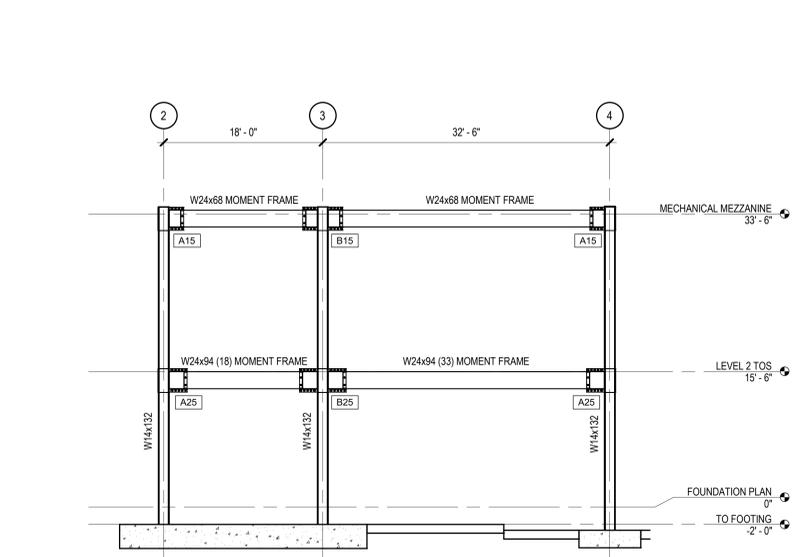
C4 MOMENT FRAME ELEVATION
 SCALE: 1/8" = 1'-0"



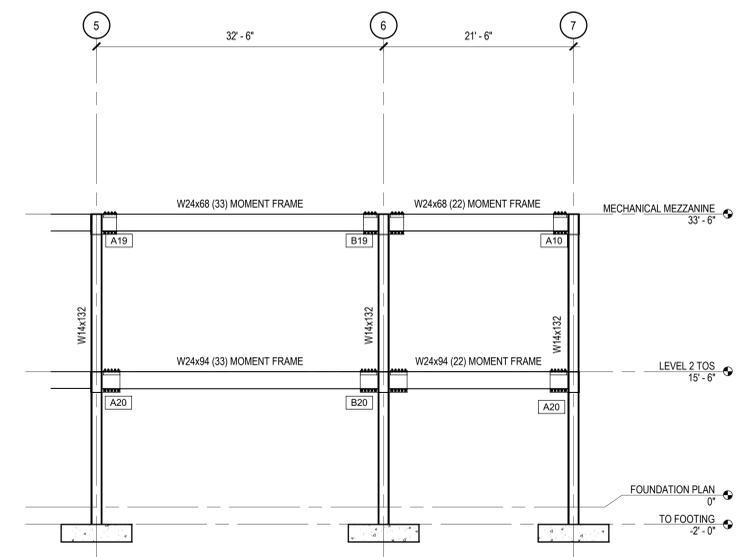
B4 MOMENT FRAME ELEVATION
 SCALE: 1/8" = 1'-0"



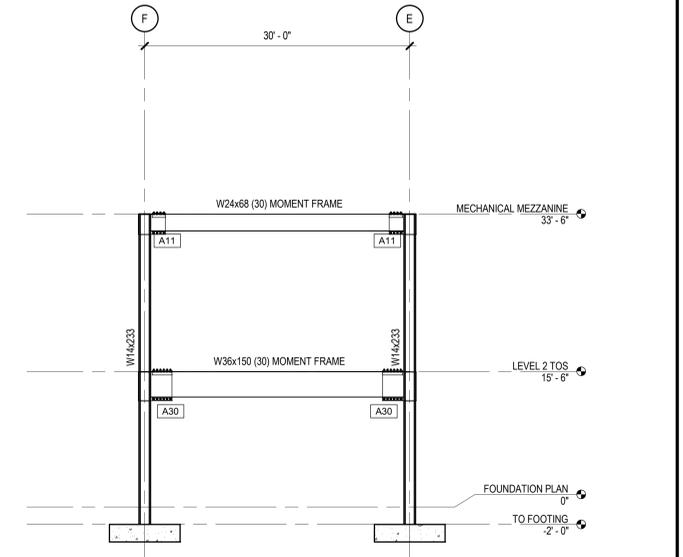
A4 MOMENT FRAME ELEVATION
 SCALE: 1/8" = 1'-0"



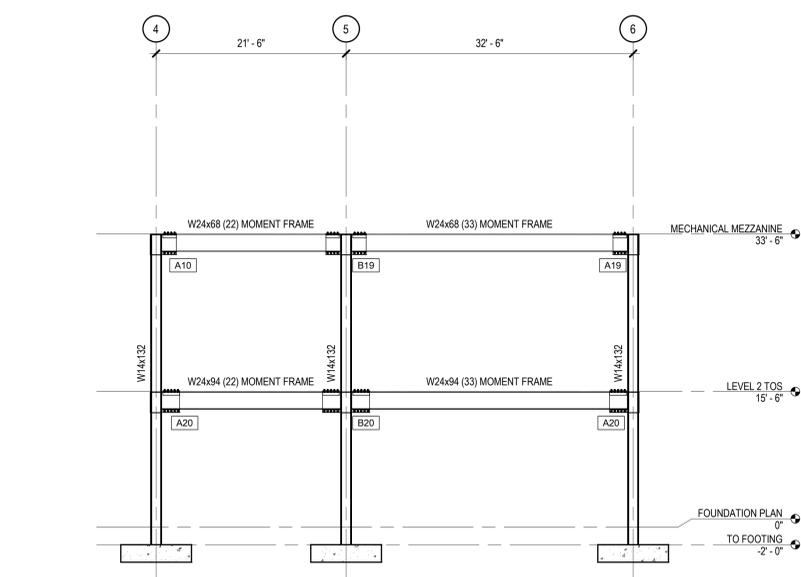
B1 MOMENT FRAME ELEVATION
 SCALE: 1/8" = 1'-0"



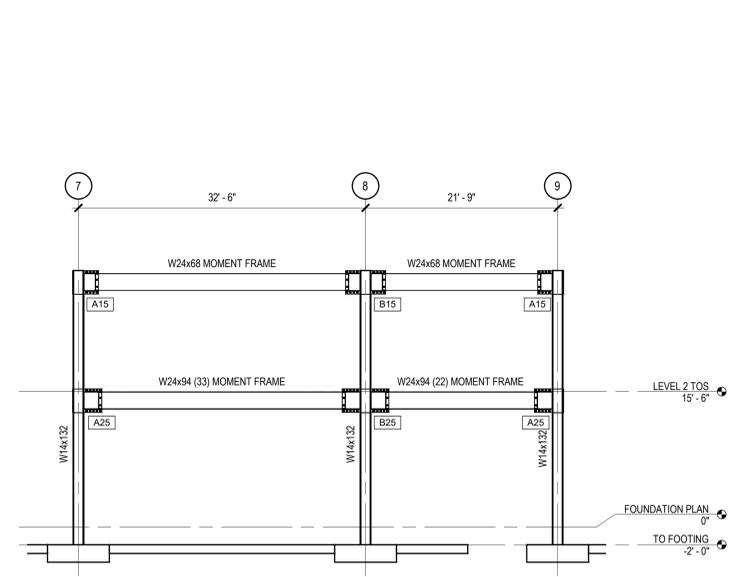
B2 MOMENT FRAME ELEVATION
 SCALE: 1/8" = 1'-0"



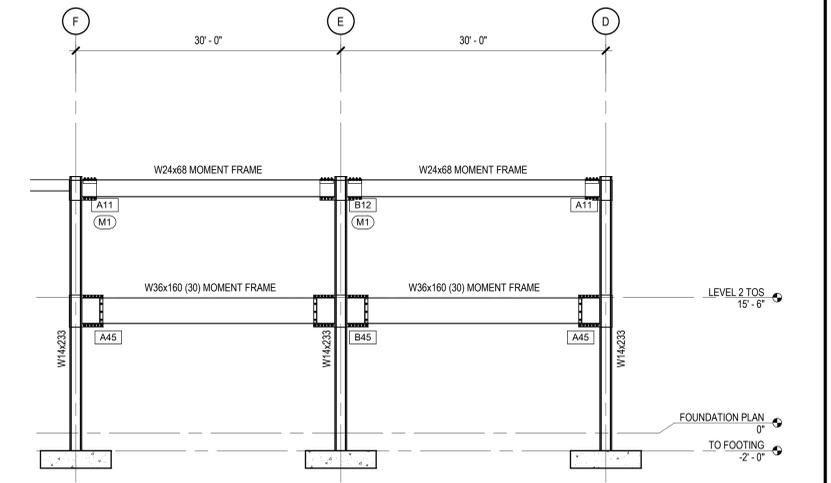
B5 MOMENT FRAME ELEVATION
 SCALE: 1/8" = 1'-0"



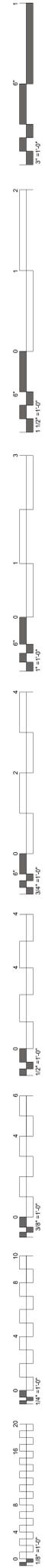
A1 MOMENT FRAME ELEVATION
 SCALE: 1/8" = 1'-0"

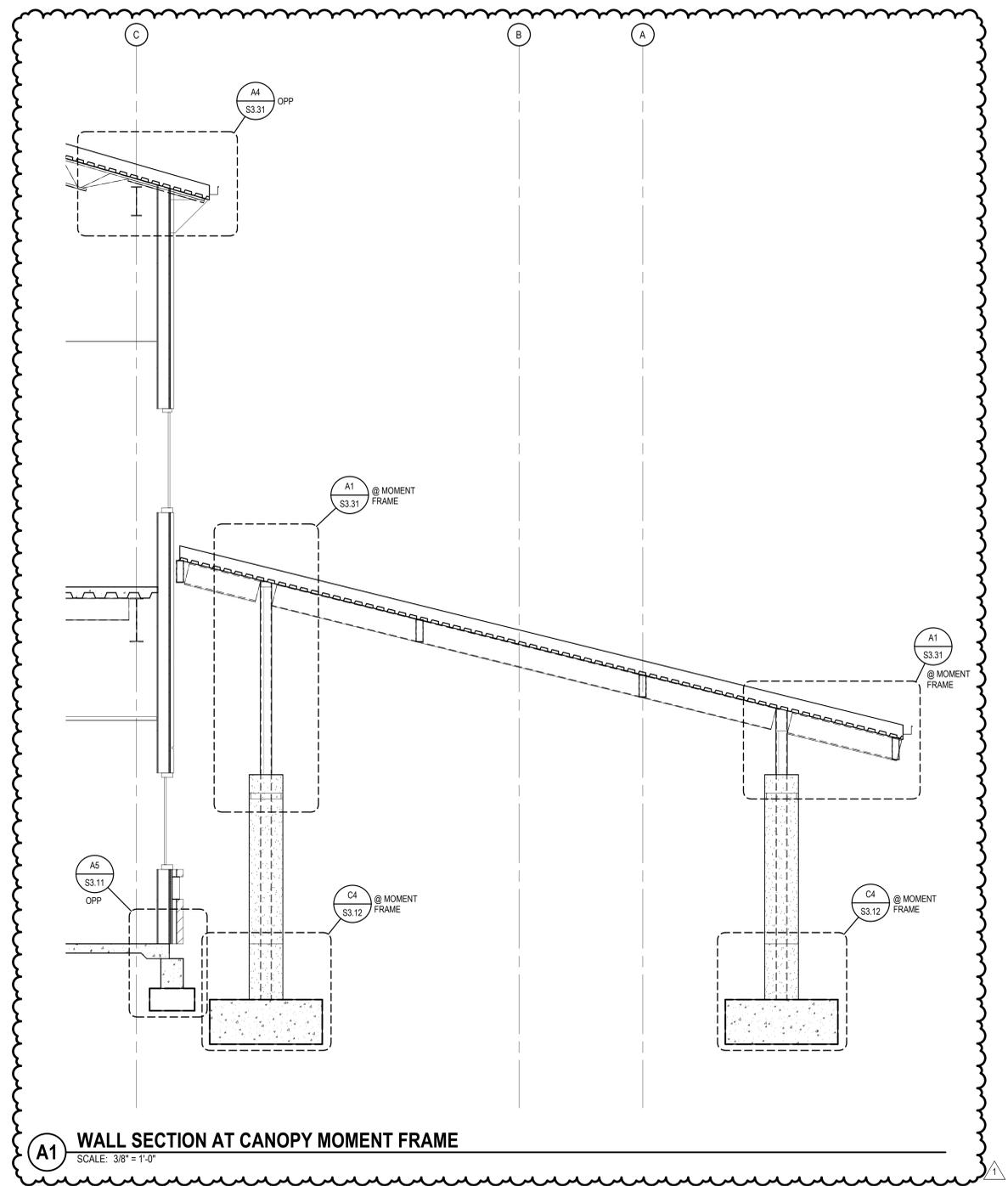
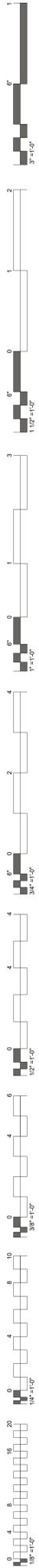


A2 MOMENT FRAME ELEVATION
 SCALE: 1/8" = 1'-0"

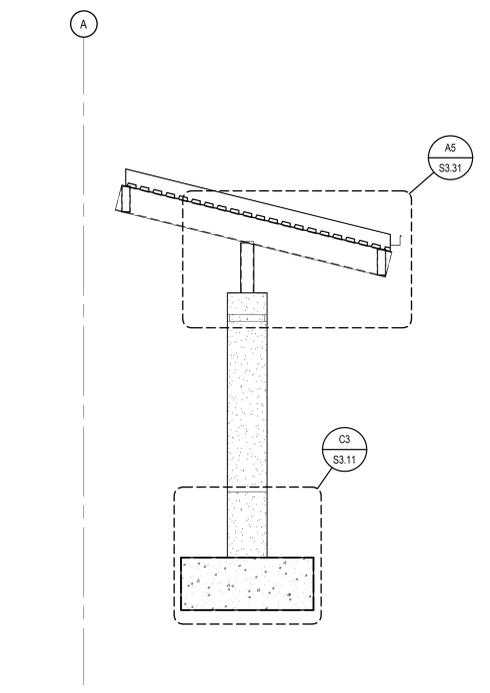


A5 MOMENT FRAME ELEVATION
 SCALE: 1/8" = 1'-0"

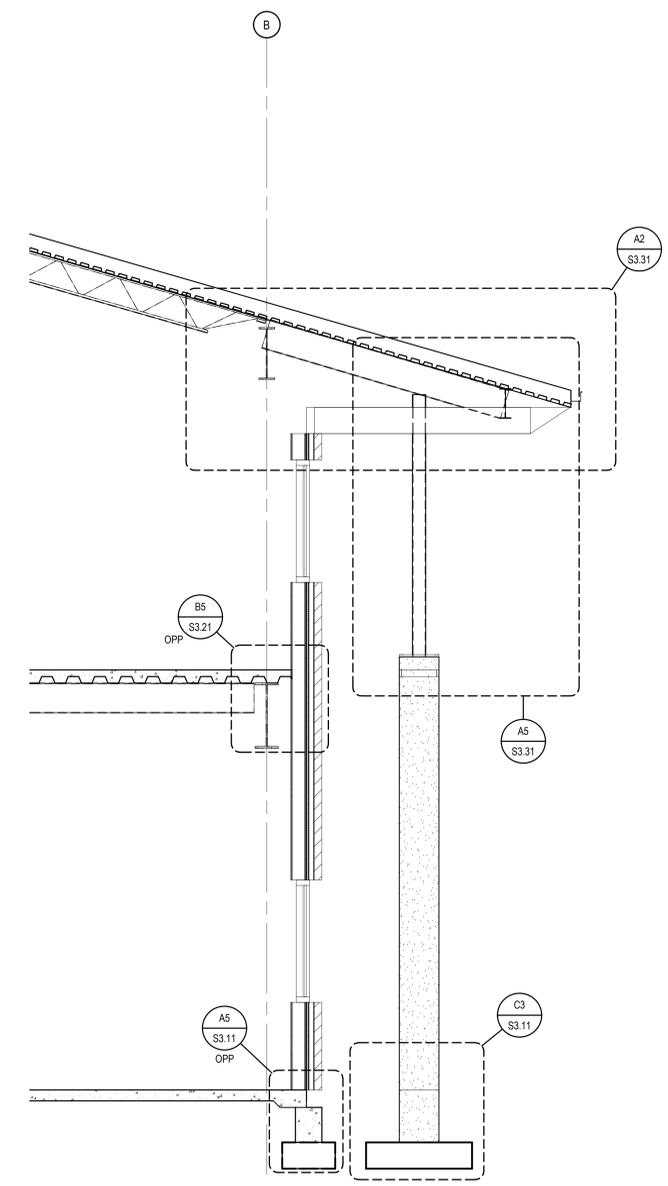




A1 WALL SECTION AT CANOPY MOMENT FRAME
SCALE: 3/8" = 1'-0"

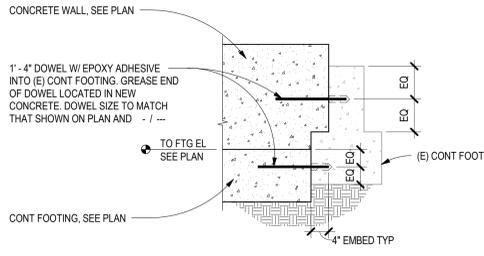


A4 WALL SECTION
SCALE: 3/8" = 1'-0"

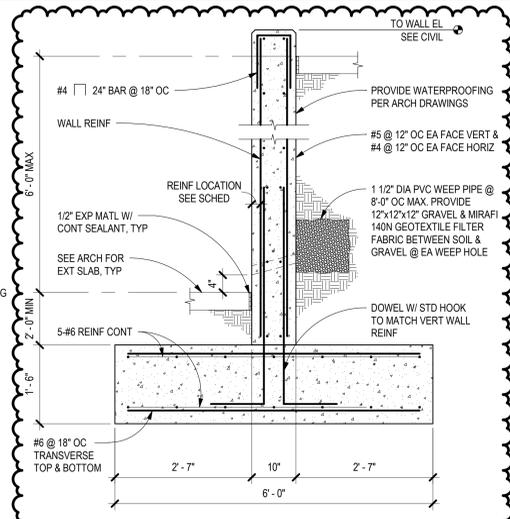


A5 WALL SECTION
SCALE: 3/8" = 1'-0"

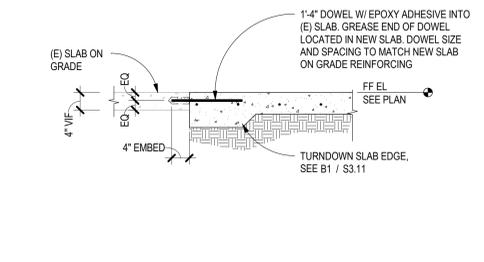
| # | DATE | REVISIONS / DESCRIPTION |
|---|----------|-------------------------|
| 1 | 11/02/19 | BID PACKAGE 01 - ADD 01 |



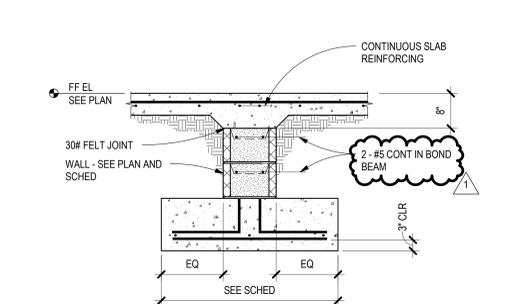
D1 DOWEL INTO (E) CONTINUOUS FOOTING
SCALE: 3/4" = 1'-0"



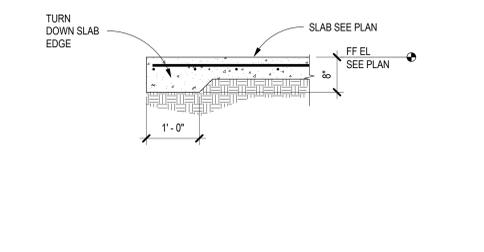
D2 RETAINING WALL SECTION
SCALE: 3/4" = 1'-0"



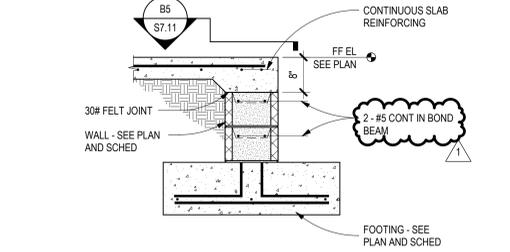
C1 DOWEL INTO (E) SLAB ON GRADE
SCALE: 3/4" = 1'-0"



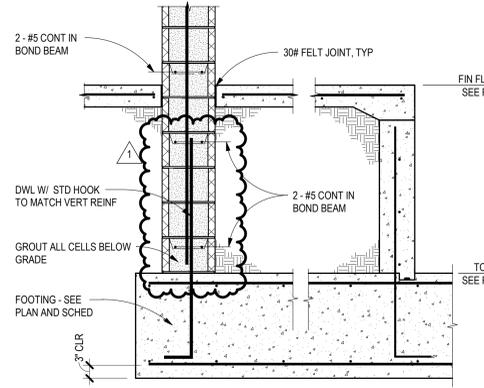
C2 INT SECTION @ DOORWAYS CMU
SCALE: 3/4" = 1'-0"



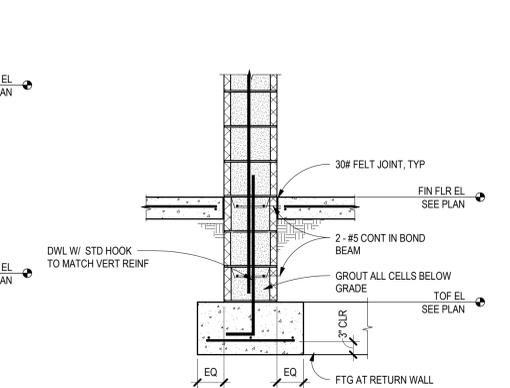
B1 TYP TURN DOWN SLAB EDGE
SCALE: 3/4" = 1'-0"



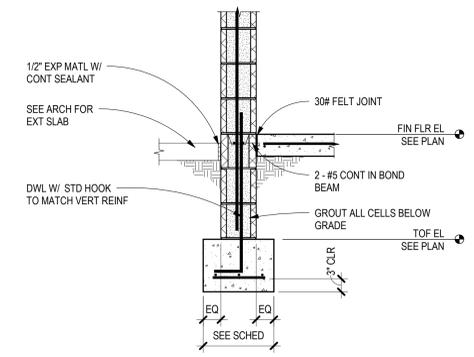
B2 PERIM SECTION AT DOORWAYS CMU
SCALE: 3/4" = 1'-0"



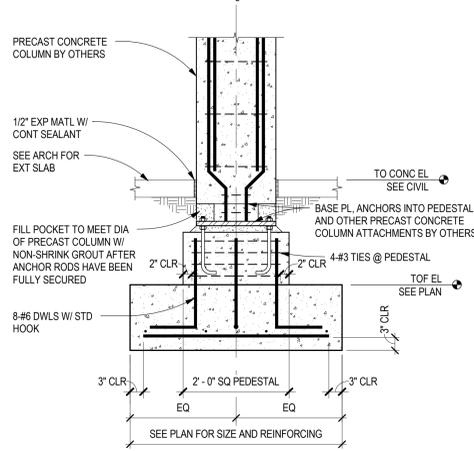
A1 FOUNDATION SECTION @ ELEVATOR
SCALE: 3/4" = 1'-0"



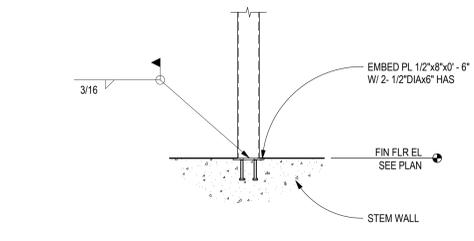
A2 PERIMETER FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



D3 PERIMETER FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



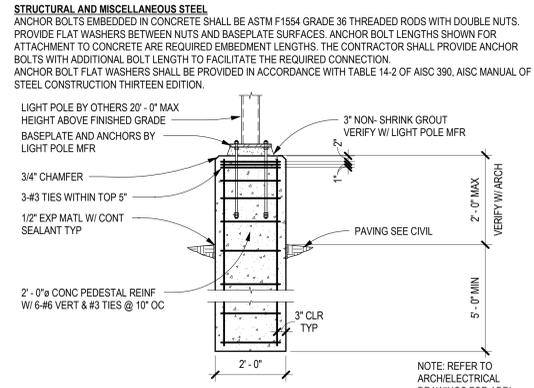
C3 EXT CONC COLUMN SECTION
SCALE: 3/4" = 1'-0"



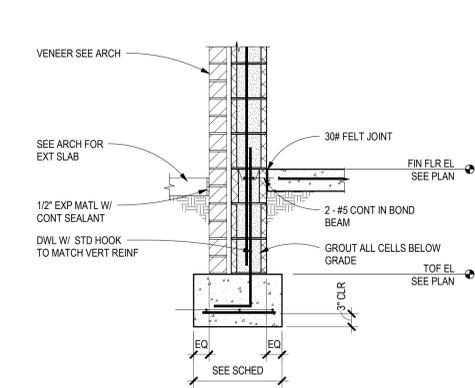
B3 HSS TO STEM WALL SECTION
SCALE: 3/4" = 1'-0"

CAST-IN-PLACE CONCRETE:
ALL CONCRETE SHALL CONFORM TO THE SPECIFICATIONS FOR STRUCTURAL CONCRETE, ACI 301-05. ALL EXPOSED EDGES OF CONCRETE SHALL HAVE A 3/4" CHAMFER UNLESS NOTED OTHERWISE. NORMAL WEIGHT CONCRETE: FC = 4000 PSI @ 28 DAYS

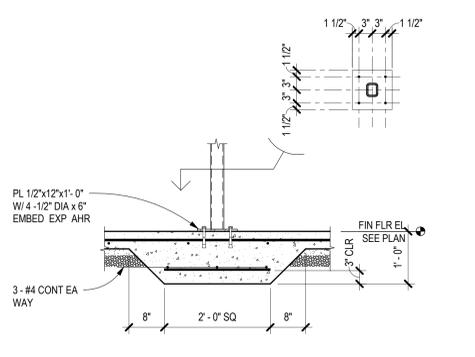
REINFORCING STEEL:
ALL REINFORCING STEEL SHALL BE FABRICATED AND PLACED IN ACCORDANCE WITH THE BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI 318-08), AND DETAILS AND DETAILING OF CONCRETE REINFORCEMENT (ACI 315-99). ALL REINFORCING STEEL SHALL CONFORM TO ASTM A615 GRADE 60; EXCEPT STIRRUPS, TIES AND INDICATED FIELD-BENT BARS, WHICH SHALL CONFORM TO ASTM A615 GRADE 40. REINFORCING SHALL NOT BE TACK WELDED OR WELDED IN ANY MANNER UNLESS SPECIFICALLY DETAILED ON THE STRUCTURAL PLANS.



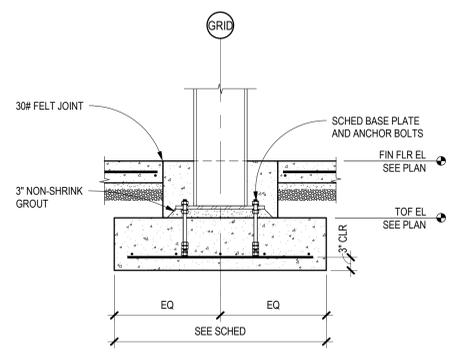
A3 LIGHT POLE BASE
SCALE: 1/2" = 1'-0"



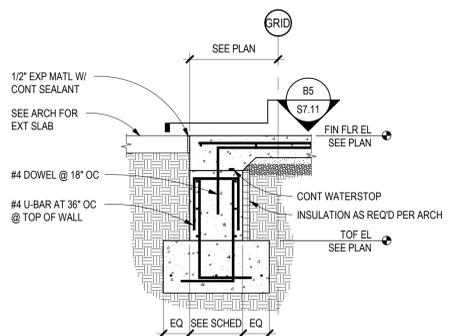
D4 PERIMETER FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



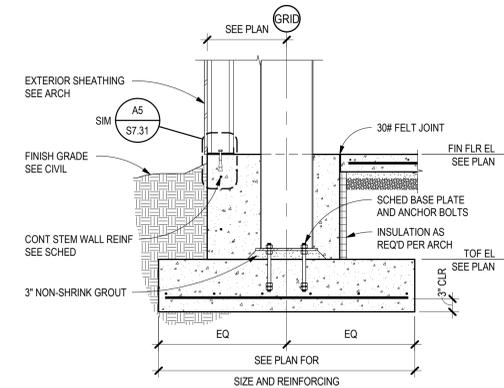
C4 INT COLUMN AT THICKENED SLAB
SCALE: 3/4" = 1'-0"



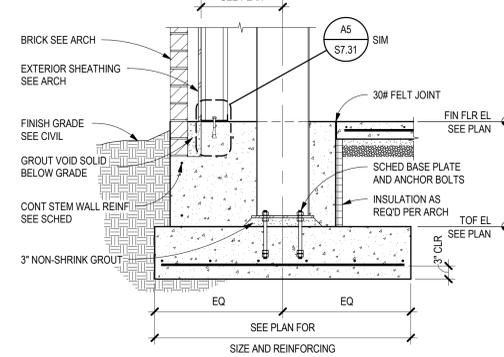
B4 INTERIOR COLUMN SECTION
SCALE: 3/4" = 1'-0"



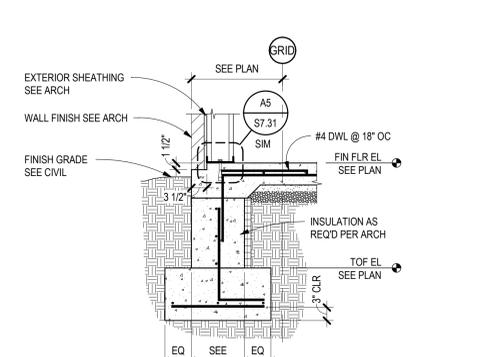
A4 FOUNDATION SECTION AT DOORWAY
SCALE: 3/4" = 1'-0"



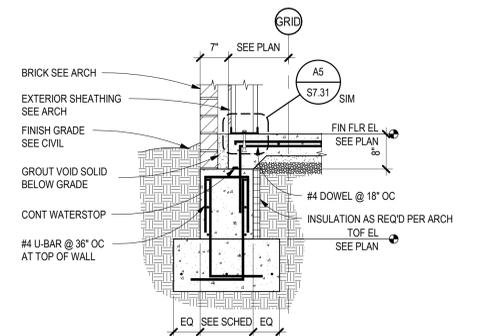
D5 FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



C5 FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



B5 FOUNDATION SECTION NO VENEER
SCALE: 3/4" = 1'-0"



A5 TYPICAL FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



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KEY PLAN:

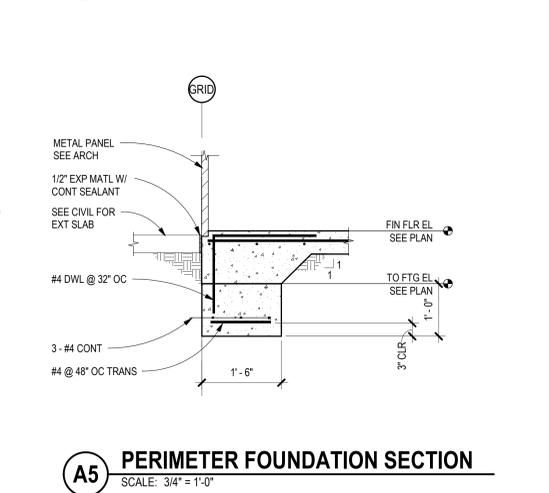
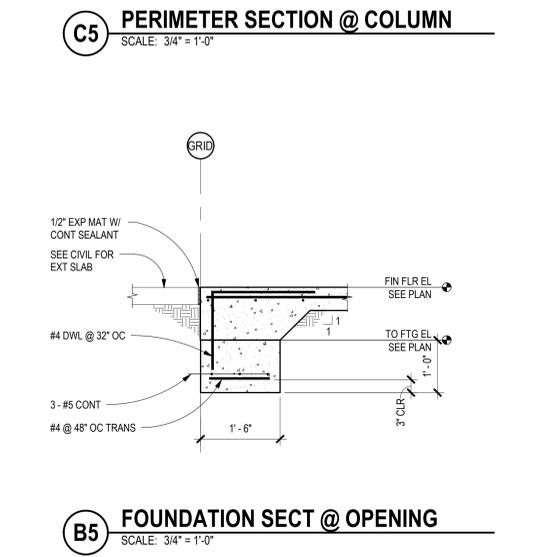
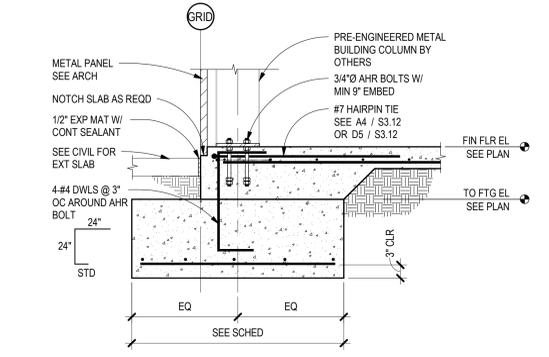
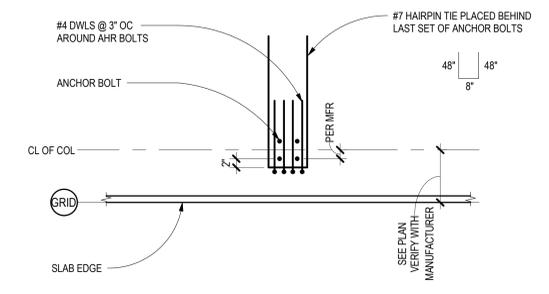
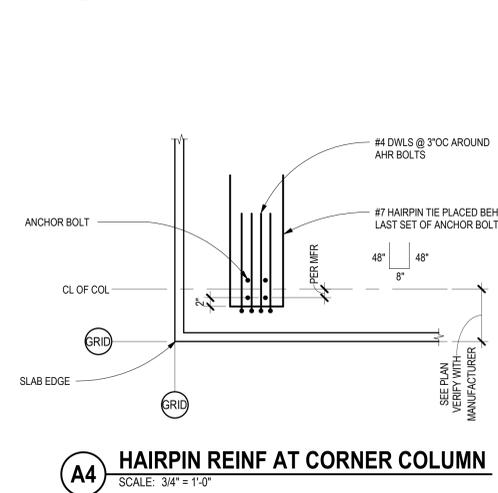
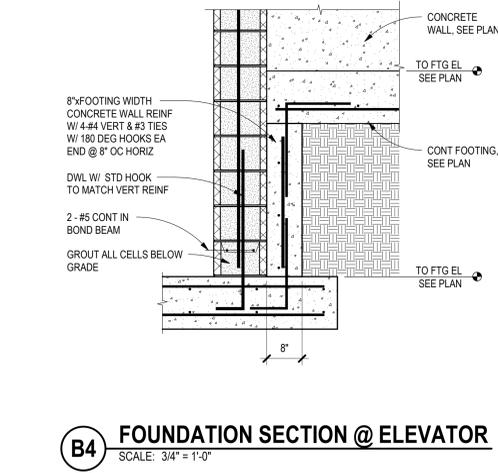
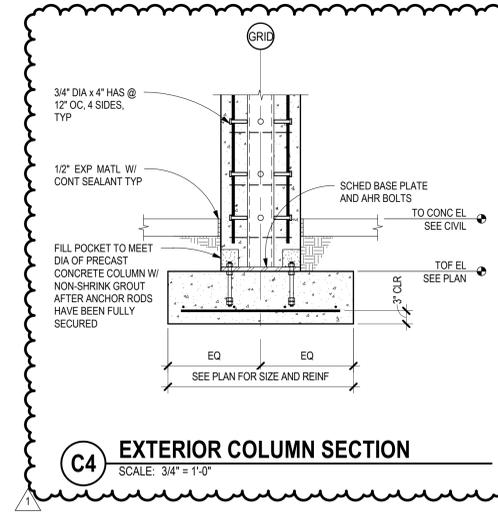
PROJECT PHASE:
BID PACKAGE 01

| # | DATE | REVISIONS | DESCRIPTION |
|---|----------|-------------------------|-------------|
| 1 | 11/02/19 | BID PACKAGE 01 - ADD 01 | |

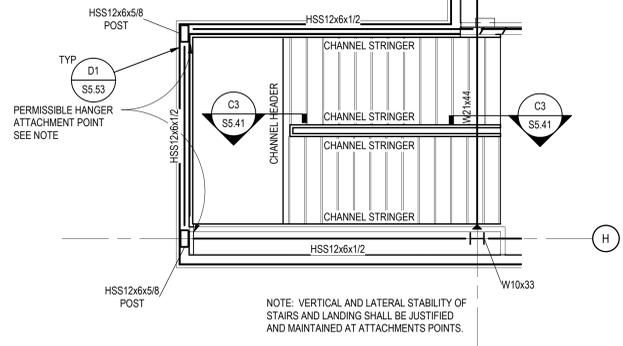
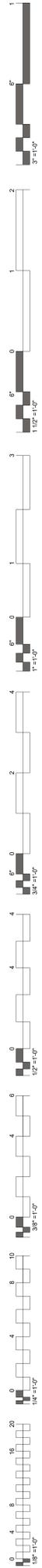
DATE: 11-01-19
JOB NUMBER: 18-01.01

SHEET NUMBER:
S3.11

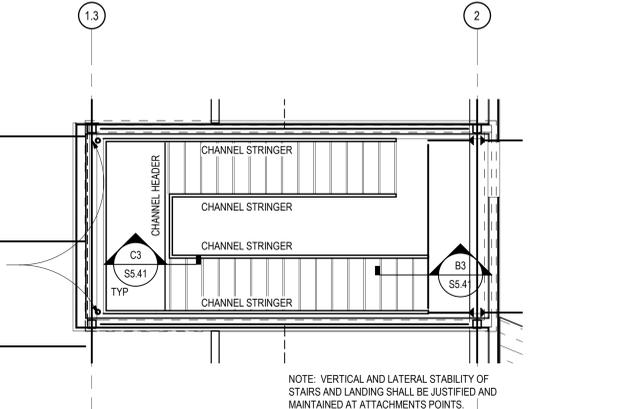
FOUNDATION SECTIONS



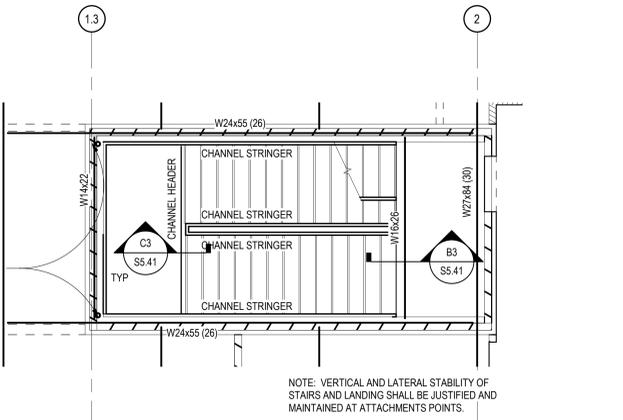
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|---|----------|-------------------------|
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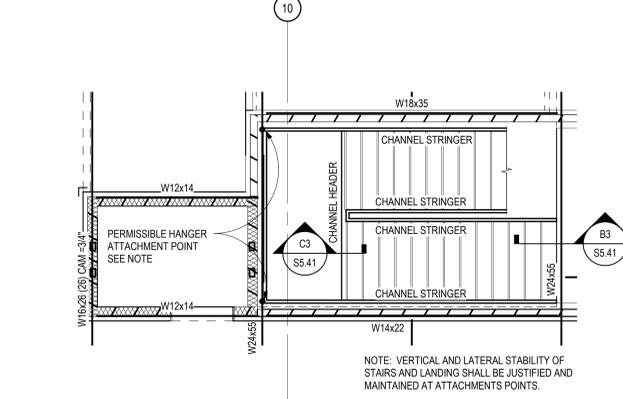
D1 ENLARGED SOUTHWEST STAIR FRAMING PLAN
SCALE: 1/4" = 1'-0"



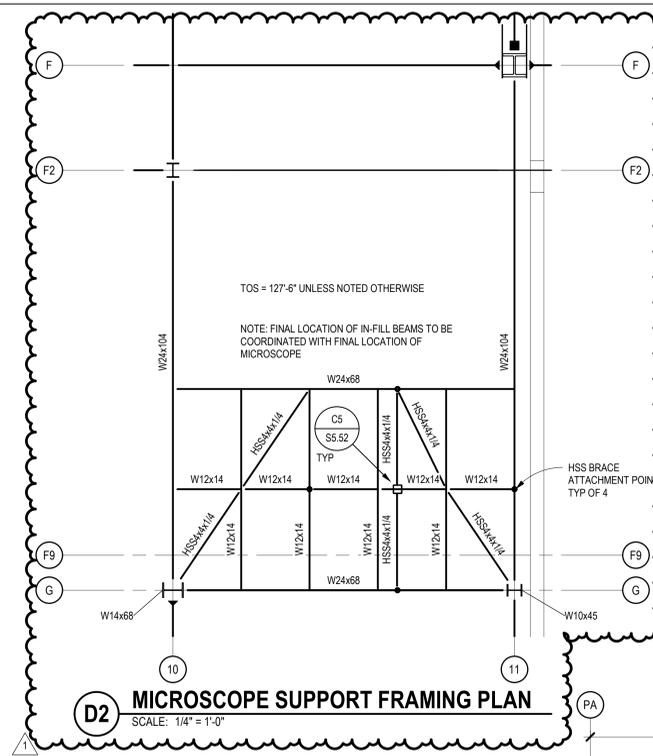
C1 ENLARGED WEST STAIR FRAMING PLAN TO MEZZANINE
SCALE: 1/4" = 1'-0"



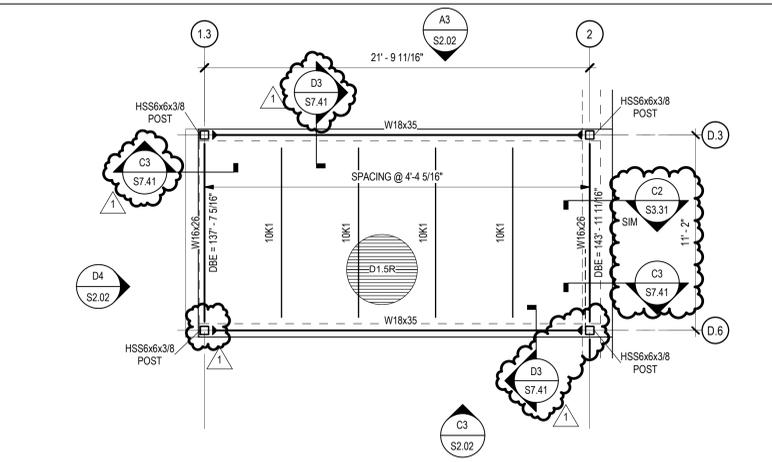
B1 ENLARGED WEST STAIR FRAMING PLAN
SCALE: 1/4" = 1'-0"



A1 ENLARGED EAST STAIR FRAMING PLAN
SCALE: 1/4" = 1'-0"

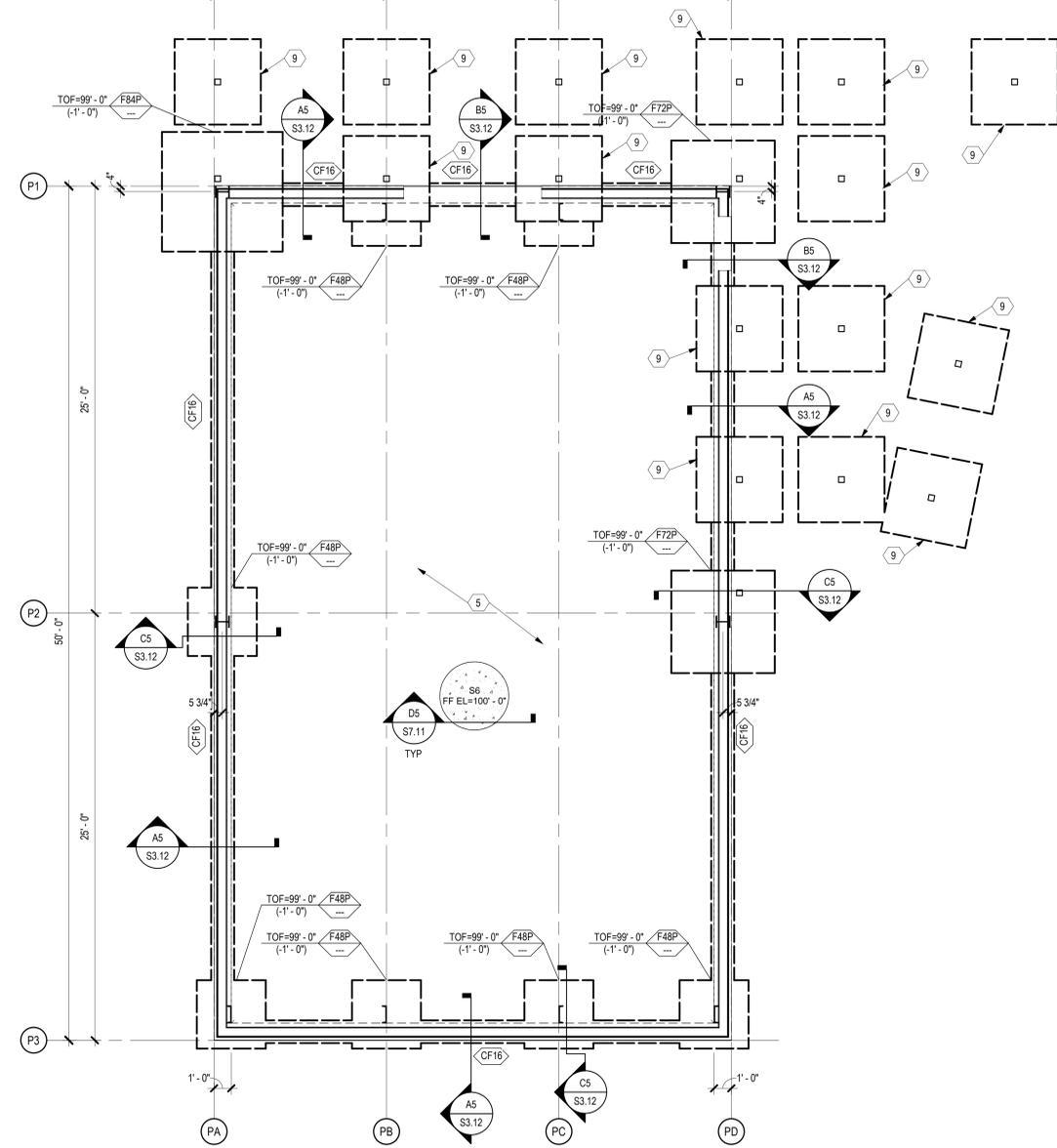


D2 MICROSCOPE SUPPORT FRAMING PLAN
SCALE: 1/4" = 1'-0"



D4 ENLARGED PLAN - WEST ROOF POP-UP
SCALE: 1/4" = 1'-0"

NOTE:
1. STONE COLUMN GROUND IMPROVEMENTS ARE NOT REQUIRED FOR PRE-ENGINEERED METAL BUILDING OR PRE-ENGINEERED CANOPY FOUNDATIONS.
2. FOUNDATION AND SLAB DESIGN FOR PRE-ENGINEERED METAL BUILDING AND PRE-ENGINEERED CANOPY ARE PRELIMINARY. SIZES SHALL BE VERIFIED UPON RECEIVING FINAL DESIGNS FROM THE MANUFACTURERS.



A3 ENLARGED PLAN - PEMB FOUNDATION
SCALE: 1/4" = 1'-0"

GENERAL SHEET NOTES

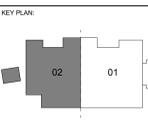
- SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- REFERENCE FINISH FLOOR ELEVATION 100'-0" = MEAN SEA FINISH FLOOR ELEVATION. SEE CIVIL DRAWINGS.
- TOP OF FOOTING ELEVATION = 98'-0" (-2'-0"), UNLESS NOTED OTHERWISE ON PLAN.
- NOTE TO CONTRACTOR: ENLARGED SLAB BLOCKOUTS MAY BE REQUIRED AT FRAME COLUMNS FOR MOMENT FRAME BASE PLATE CLEARANCE.
- NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACINGS OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
- DIMENSIONS ARE TO THE FACE OF STUD UNLESS NOTED OTHERWISE.
- SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
- EXISTING CONSTRUCTION IS PER AVAILABLE EXISTING DRAWINGS. ALL EXISTING CONSTRUCTION AND DIMENSIONS SHALL BE VERIFIED PRIOR TO CONSTRUCTION. SHOULD CONDITIONS VARY FROM THOSE SHOWN, CONTACT ENGINEER BEFORE PROCEEDING.
- PROVIDE SLAB JOINTS AT 10'-0" ON CENTER MAXIMUM. THE AREA OF THE CONTROL JOINT SHALL NOT EXCEED A 2:1 RATIO. CONTROL JOINTS SHALL BE LOCATED AT COLUMN LINES WHERE THE LAYOUT PERMITS. AT RE-ENTRANT CORNERS THAT DO NOT HAVE CONTROL JOINTS, PROVIDE 2#4 x 3'-0" DIAGONAL TO THE RE-ENTRANT CORNER.
- STRUCTURAL COLD FORMED METAL STUDS SHALL BE 6" WIDE UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING BY OTHERS.
- SEE SHEET S7.00 SERIES SHEETS FOR TYPICAL FOUNDATION SECTIONS AND DETAILS.
- SEE SHEET S6.01 FOR SCHEDULES.

SHEET KEYNOTE

- FLOOR DRAIN, SLOPE SLAB TO DRAIN 1/8" PER FOOT. COORDINATE EXACT SIZE AND LOCATION WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
- POST AND FOUNDATION AS REQUIRED FOR STAIR SUPPORT. STAIR ENGINEER TO PROVIDE REQUIRED LOADS AND LOCATIONS.
- ELEVATOR SUMP PIT. COORDINATE EXACT SIZE AND LOCATION WITH ELEVATOR MANUFACTURER. SEE A4 / S5.41
- HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION AND SPACING WITH ELEVATOR MANUFACTURER. SEE B4 / S5.41
- PRE-ENGINEERED METAL BUILDING STEEL AND ANCHORAGE BY OTHERS. CONTRACTOR TO CONFIRM LOCATIONS OF FOUNDATIONS WITH FINAL PRE-ENGINEERED METAL BUILDING SHOP DRAWINGS.
- NOTCH MASONRY AS REQUIRED TO FACILITATE BASEPLATE INSTALLATION. STEP BOND BEAM AT THIS LOCATION. FILL VOID FROM NOTCH WITH NON-SHRINK GROUT.
- CUT AND REMOVE EXISTING SLAB AS REQUIRED TO PLACE NEW FOOTING. NEW SLAB TO POUR UP TO REMAINING SLAB.
- CENTER FOOTING ON GRID C.
- F8A PRE-MANUFACTURED SUNSHADE CONCRETE FOOTING. TOP OF FOOTING = 99'-0" (-1'-0"). SEE SHEET S6.01 FOR FOOTING SCHEDULE. COORDINATE FINAL LOCATION WITH SUNSHADE MANUFACTURER.
- EXISTING CANOPY. SEE ARCHITECTURAL DEMOLITION PLANS FOR EXTENT OF DEMOLITION.
- HSS6x6x1/2 ELEVATOR SUPPORT POST. COORDINATE EXACT LOCATION AND SPACING WITH ELEVATOR MANUFACTURER. SEE B4 / S5.41, D3 / S5.41, A2 / S5.41, B2 / S5.41, AND C2 / S5.41
- 1 1/2" RECESSED SLAB AT ADA SHOWER. COORDINATE EXACT SIZE, LOCATION, AND SLOPE REQUIREMENTS WITH ARCHITECTURAL DRAWINGS. SEE C4 / S7.11
- 18" DIAMETER PRECAST CONCRETE COLUMN BY OTHERS. SEE C3 / S3.11 AND B1 / S3.31
- 18" DIAMETER PRECAST CONCRETE CANOPY COLUMN BY OTHERS. SEE C3 / S3.11, C4 / S3.12, A1 / S3.31, AND A5 / S3.31



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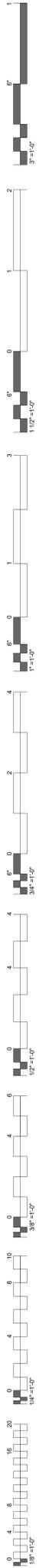
PROJECT PHASE:
BID PACKAGE 01

| # | DATE | REVISIONS / DESCRIPTION |
|---|----------|-------------------------|
| 1 | 11/22/19 | BID PACKAGE 01 - ADD 01 |

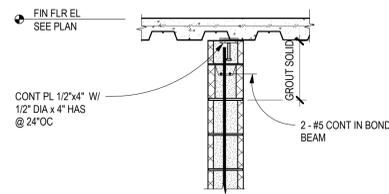
DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER: S4.01

ENLARGED PLANS

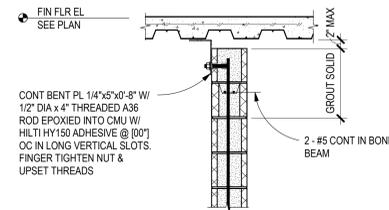


NOTE: ATTACH DECK TO SUPPORT ANGLE PER NOTE ON PLAN

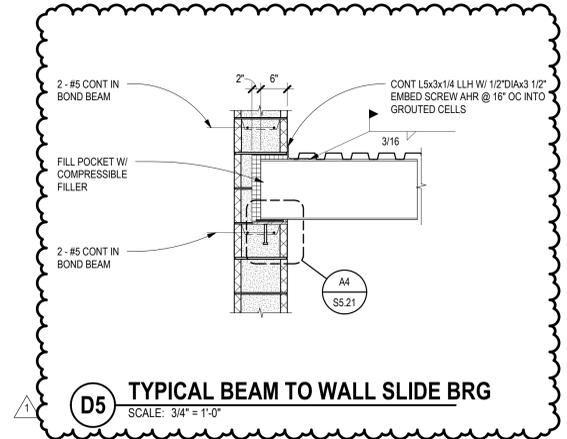


D3 CMU BEARING WALL TO DECK
SCALE: 3/4" = 1'-0"

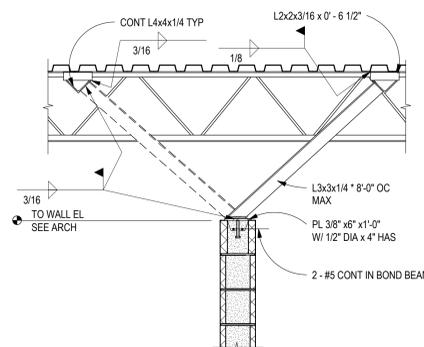
NOTE: ATTACH DECK TO SUPPORT ANGLE PER NOTE ON PLAN



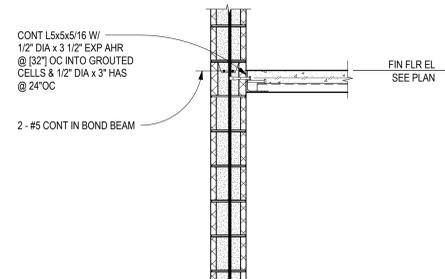
D4 CMU NON-BRG WALL TO DECK
SCALE: 3/4" = 1'-0"



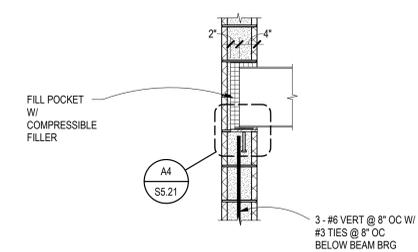
D5 TYPICAL BEAM TO WALL SLIDE BRG
SCALE: 3/4" = 1'-0"



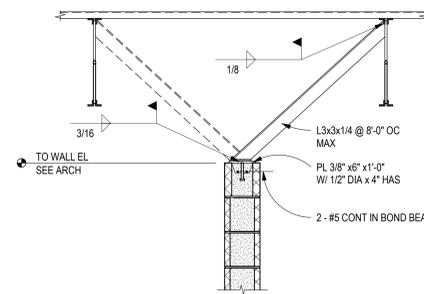
C3 CMU WALL BRACING SECTION
SCALE: 3/4" = 1'-0"



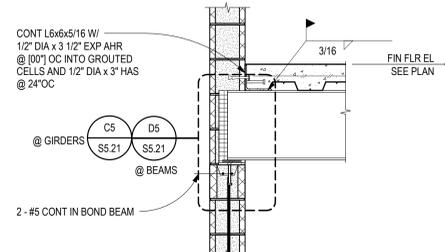
C4 FLOOR FRAMING SECTION
SCALE: 3/4" = 1'-0"



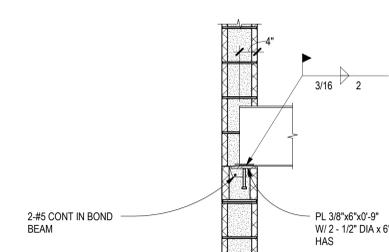
C5 TYPICAL GIRDER TO WALL SLIDE BRG
SCALE: 3/4" = 1'-0"



B3 CMU WALL BRACING SECTION
SCALE: 3/4" = 1'-0"

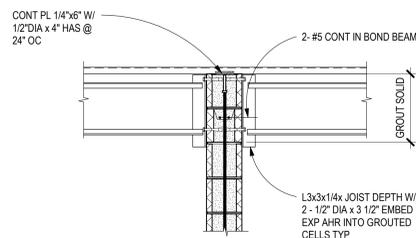


B4 FLOOR FRAMING SECTION
SCALE: 3/4" = 1'-0"



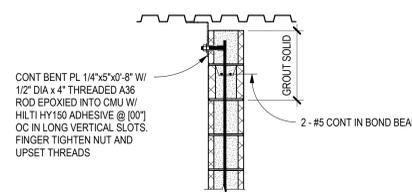
B5 TYPICAL BEAM TO CMU WALL
SCALE: 3/4" = 1'-0"

NOTE: ATTACH DECK TO SUPPORT ANGLE PER NOTE ON PLAN



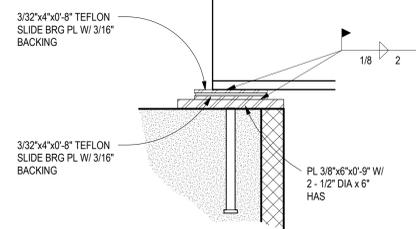
A2 INT CMU BEARING WALL TO DECK
SCALE: 3/4" = 1'-0"

NOTE: ATTACH DECK TO SUPPORT ANGLE PER NOTE ON PLAN

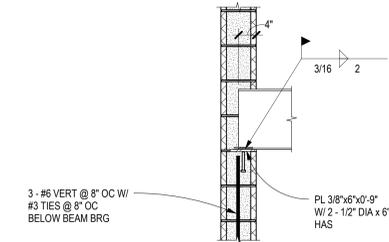


A3 CMU NON-BRG WALL TO DECK
SCALE: 3/4" = 1'-0"

NOTE: INSTALL PLATES TEFLON TO TEFLON



A4 TYPICAL SLIDE BRG CONN
SCALE: 1 1/2" = 1'-0"



A5 TYPICAL GIRDER TO CMU WALL
SCALE: 3/4" = 1'-0"



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CONSULTANT LOGO



**WILMA P. MANKILLER HEALTH CENTER
EXPANSION**
STILWELL, OKLAHOMA

KEY PLAN

PROJECT PHASE:
BID PACKAGE 01

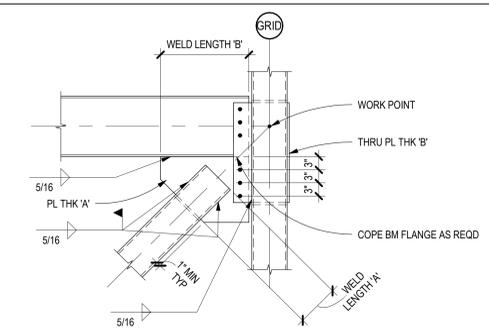
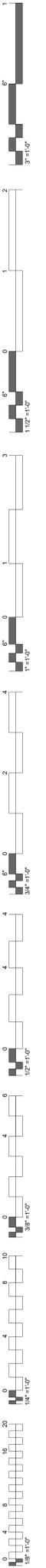
| # | DATE | REVISIONS |
|---|----------|-------------------------|
| 1 | 11/22/19 | BID PACKAGE 01 - ADD 01 |

DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER:

S5.21

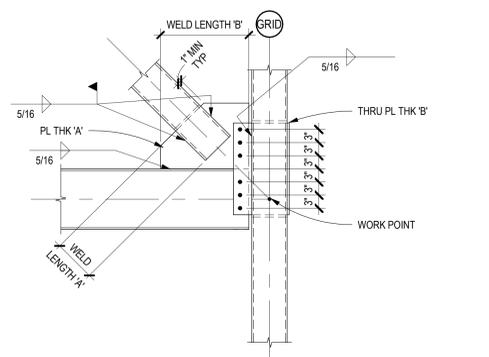
MASONRY FRAMING SECTIONS AND DETAILS



| CONNECTION SCHEDULE | | | | | |
|---------------------|------------|-----------------|-----------------|-----------------|-----------------|
| BRACE SIZE | PL THK 'A' | THRU PL THK 'B' | WELD LENGTH 'A' | WELD LENGTH 'B' | GUSSET PL BOLTS |
| HSS4x4 | 5/8" | 5/8" | 4" | 20" | 3 |
| HSS5x5 | 5/8" | 5/8" | 5" | 20" | 3 |
| HSS6x6 | 5/8" | 5/8" | 6" | 22" | 3 |
| HSS8x8 | 3/4" | 3/4" | 8" | 24" | 4 |
| HSS12x8 | 1" | 1" | 12" | 28" | 5 |

NOTES
 1. LENGTHS GIVEN ARE SINGLE-SIDE LENGTHS AND MINIMUM LENGTHS
 2. LONGER GUSSET/WELD LENGTHS MAY BE REQD WHERE BRACE SLOPE VARIES FROM 1:1
 3. ALL BOLTS IN STANDARD HOLES

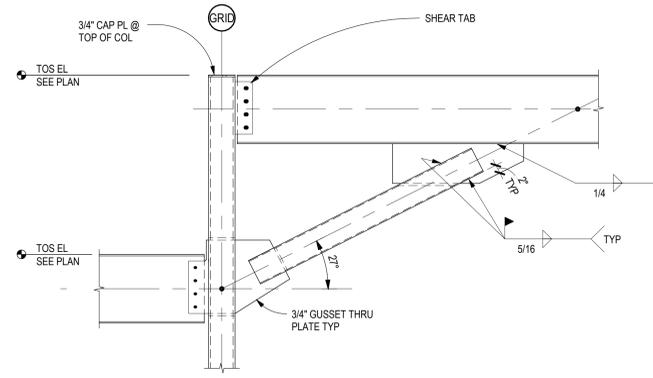
D2 BRACED FRAME CONNECTION
 SCALE: 3/4" = 1'-0"



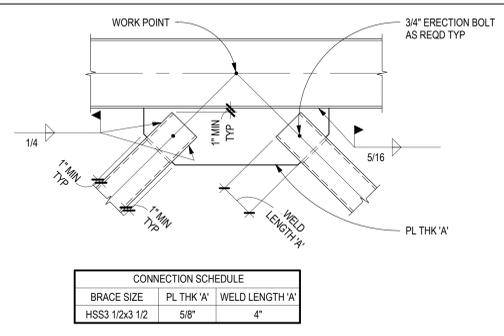
| CONNECTION SCHEDULE | | | | | |
|---------------------|------------|-----------------|-----------------|-----------------|-----------------|
| BRACE SIZE | PL THK 'A' | THRU PL THK 'B' | WELD LENGTH 'A' | WELD LENGTH 'B' | GUSSET PL BOLTS |
| HSS4x4 | 5/8" | 5/8" | 4" | 20" | 3 |
| HSS5x5 | 5/8" | 5/8" | 5" | 20" | 3 |
| HSS6x6 | 5/8" | 5/8" | 6" | 22" | 3 |
| HSS8x8 | 3/4" | 3/4" | 8" | 24" | 4 |
| HSS12x8 | 1" | 1" | 12" | 28" | 5 |

NOTES
 1. LENGTHS GIVEN ARE SINGLE-SIDE LENGTHS AND MINIMUM LENGTHS
 2. LONGER GUSSET/WELD LENGTHS MAY BE REQD WHERE BRACE SLOPE VARIES FROM 1:1
 3. ALL BOLTS IN STANDARD HOLES

B2 BRACED FRAME CONNECTION
 SCALE: 3/4" = 1'-0"



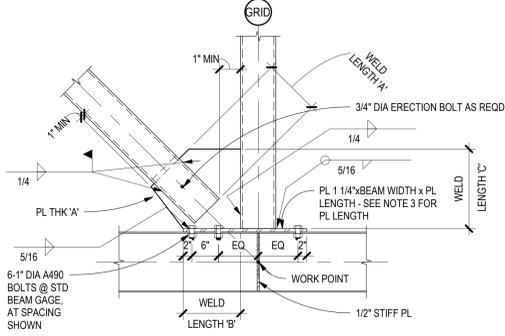
A1 COLLECTOR FRAMING DETAIL
 SCALE: 3/4" = 1'-0"



| CONNECTION SCHEDULE | | |
|---------------------|------------|-----------------|
| BRACE SIZE | PL THK 'A' | WELD LENGTH 'A' |
| HSS3 1/2x3 1/2 | 5/8" | 4" |

NOTES
 1. LENGTHS GIVEN ARE SINGLE-SIDE LENGTHS AND MINIMUM REQUIRED LENGTHS
 2. LONGER GUSSET/WELD LENGTHS MAY BE REQD WHERE BRACE SLOPE VARIES FROM 1:1
 3. BRACE AND GUSSET CONNECTION CAN OCCUR ON TOP AND BOTTOM FLANGES

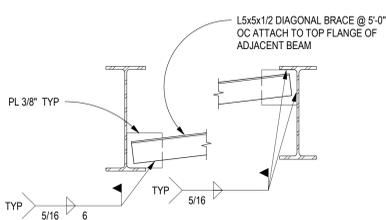
D3 BRACED FRAME CONNECTION
 SCALE: 3/4" = 1'-0"



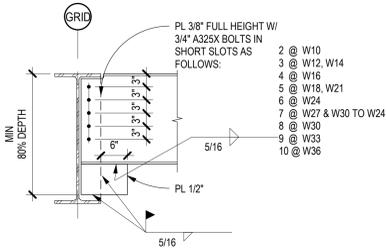
| CONNECTION SCHEDULE | | | |
|---------------------|------------|-----------------|-----------------|
| BRACE SIZE | PL THK 'A' | WELD LENGTH 'A' | WELD LENGTH 'C' |
| HSS3 1/2x3 1/2 | 5/8" | 4" | 12" |
| HSS5x5 | 1/2" | 5" | 12" |

NOTES
 1. LENGTHS GIVEN ARE SINGLE-SIDE LENGTHS AND MINIMUM LENGTHS
 2. LONGER GUSSET/WELD LENGTHS MAY BE REQD WHERE BRACE SLOPE VARIES FROM 1:1
 3. PLATE LENGTHS: BASED ON POST SIZE
 A. HSS5x5 & HSS6x6: 2 - 0"

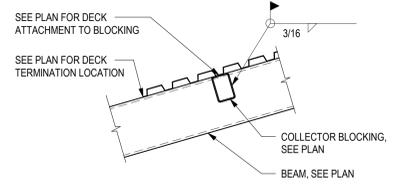
C3 BRACED FRAME CONNECTION
 SCALE: 3/4" = 1'-0"



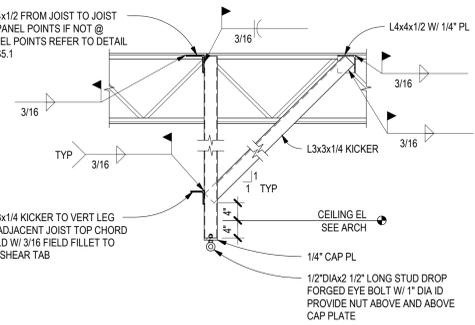
B3 DIAG ANGLE AT MOMENT CONN
 SCALE: 3/4" = 1'-0"



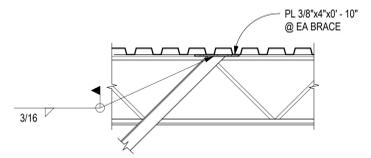
A3 BEAM CONN AT MOMENT (WUF) CONN
 SCALE: 3/4" = 1'-0"



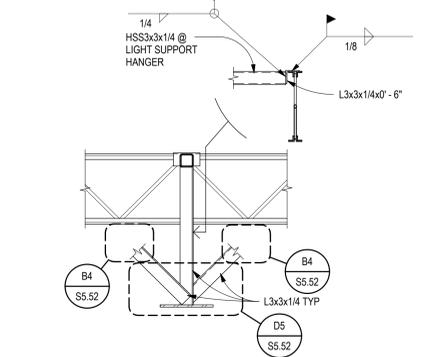
D4 CANOPY COLLECTOR BLOCKING
 SCALE: 3/4" = 1'-0"



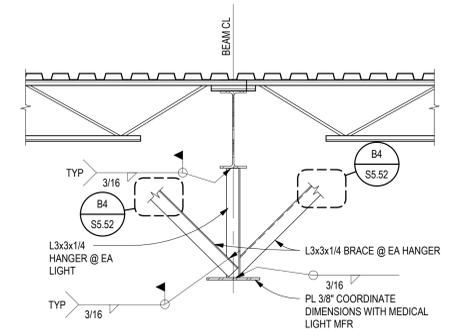
C4 SWING SUPPORT BETWEEN JOISTS
 SCALE: 3/4" = 1'-0"



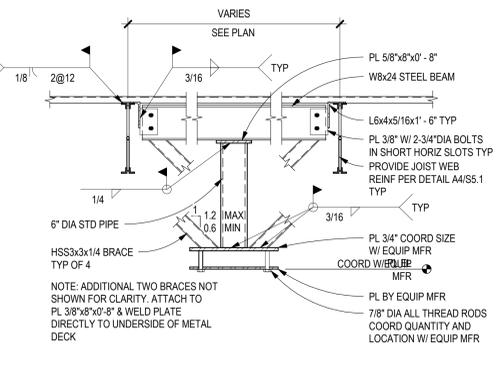
B4 LIGHT SUPPORT BETWEEN JOISTS
 SCALE: 3/4" = 1'-0"



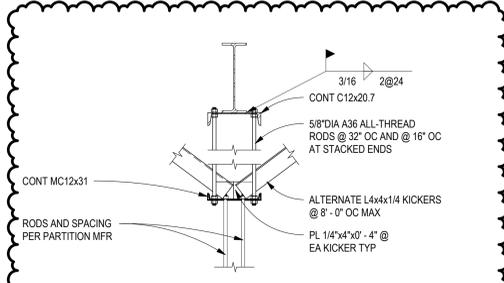
A4 LIGHT SUPPORT BETWEEN JOISTS
 SCALE: 3/4" = 1'-0"



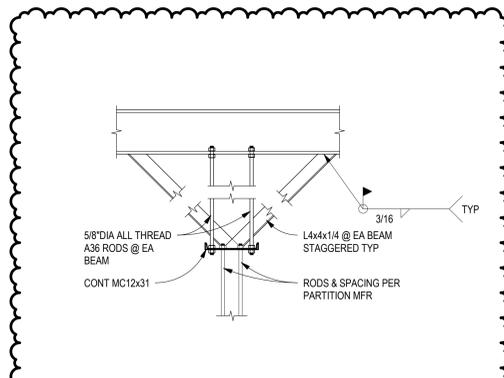
D5 MEDICAL LIGHT SUPPORT
 SCALE: 3/4" = 1'-0"



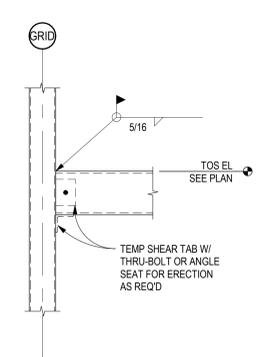
C5 SURGICAL LIGHT SUPPORT
 SCALE: 3/4" = 1'-0"



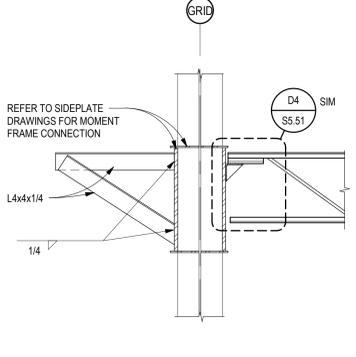
B5 PARTITION SUPPORT SECTION
 SCALE: 3/4" = 1'-0"



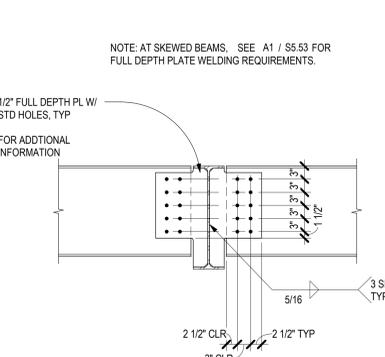
A5 PARTITION FRAMING SECTION
 SCALE: 3/4" = 1'-0"



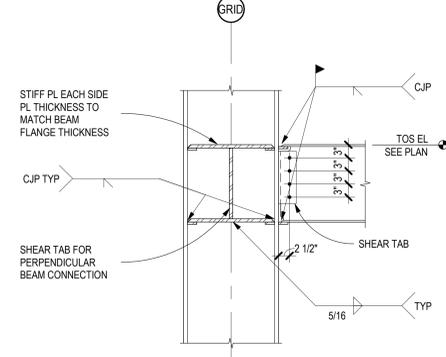
D1 HSS TO COLUMN WITH PARTIAL FIXITY
SCALE: 3/4" = 1'-0"



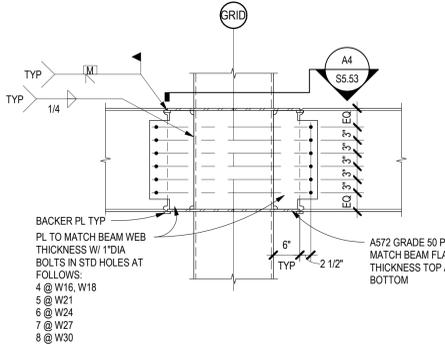
D2 BM TO COL WEB CONN AT SIDEPLATE
SCALE: 3/4" = 1'-0"



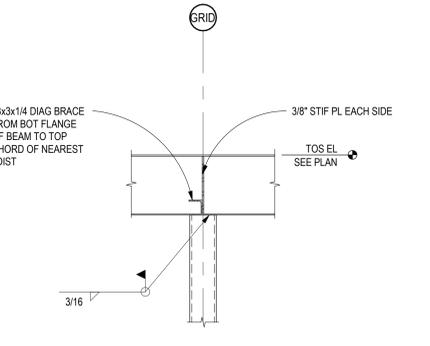
D3 BEAM TO BEAM COLLECTOR DETAIL
SCALE: 3/4" = 1'-0"



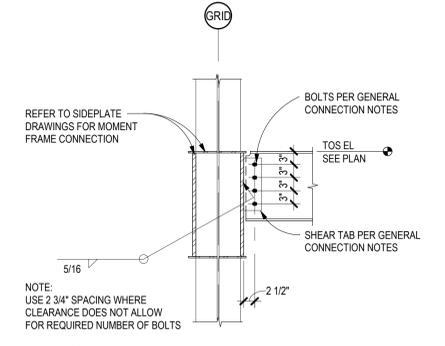
D4 BEAM TO COL FLANGE MOMENT CONN
SCALE: 3/4" = 1'-0"



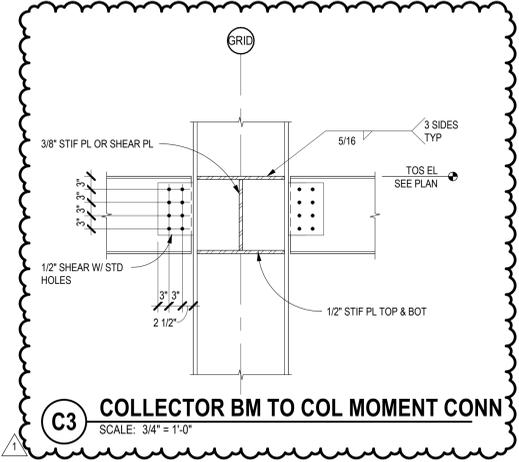
D5 TYPICAL FLANGE PLATE TO COLUMN
SCALE: 3/4" = 1'-0"



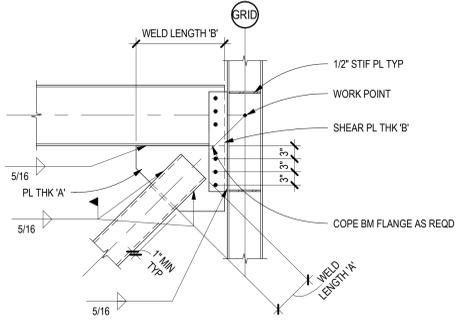
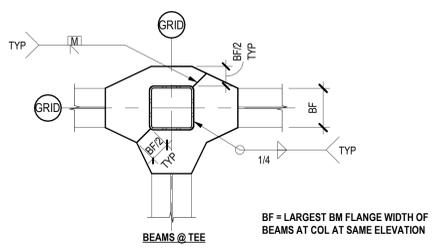
C1 BEAM OVER COLUMN CONN
SCALE: 3/4" = 1'-0"



C2 BM TO COL WEB CONN AT SIDEPLATE
SCALE: 3/4" = 1'-0"



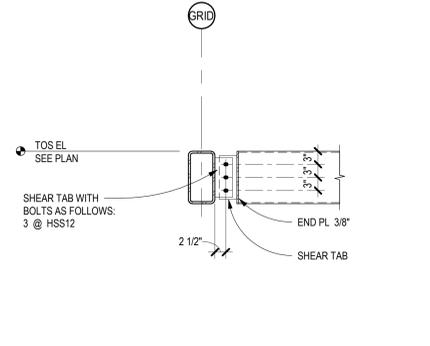
C3 COLLECTOR BM TO COL MOMENT CONN
SCALE: 3/4" = 1'-0"



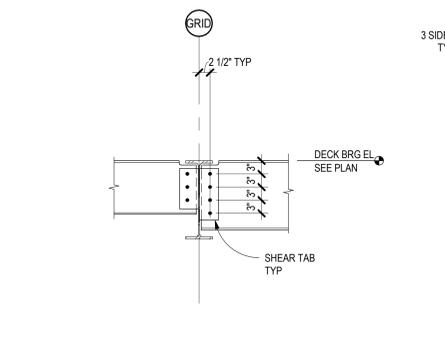
| CONNECTION SCHEDULE | | | | | |
|---------------------|------------|-----------------|-----------------|-----------------|-----------------|
| BRACE SIZE | PL THK 'A' | THRU PL THK 'B' | WELD LENGTH 'A' | WELD LENGTH 'B' | GUSSET PL BOLTS |
| HSS4x4 | 5/8" | 5/8" | 4" | 20" | 3 |
| HSS5x5 | 5/8" | 5/8" | 5" | 20" | 3 |
| HSS6x6 | 5/8" | 5/8" | 6" | 22" | 3 |
| HSS8x8 | 3/4" | 3/4" | 8" | 24" | 4 |
| HSS12x8 | 1" | 1" | 12" | 28" | 5 |

NOTES:
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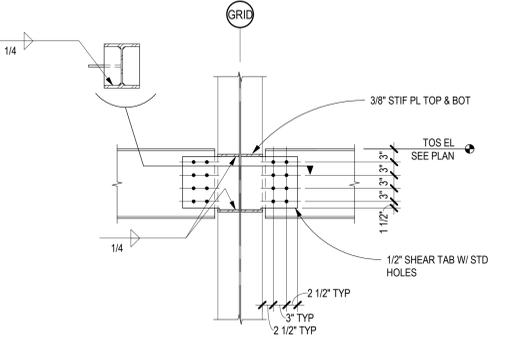
B5 BRACED FRAME CONNECTION
SCALE: 3/4" = 1'-0"



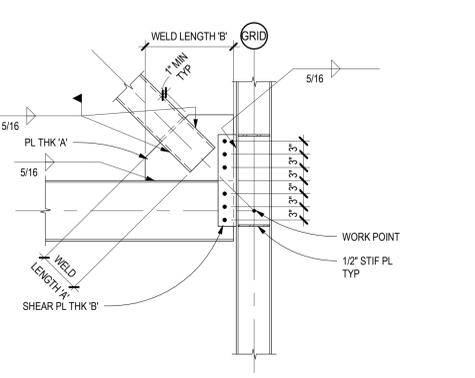
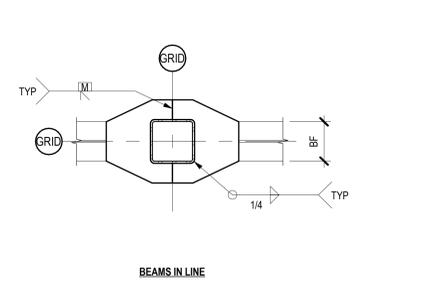
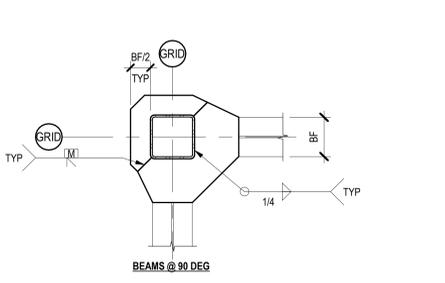
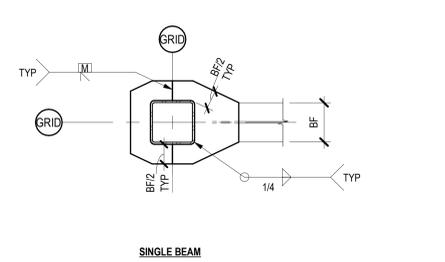
B1 BEAM TO BEAM CONN
SCALE: 3/4" = 1'-0"



B2 TYPICAL BEAM TO BEAM CONN
SCALE: 3/4" = 1'-0"



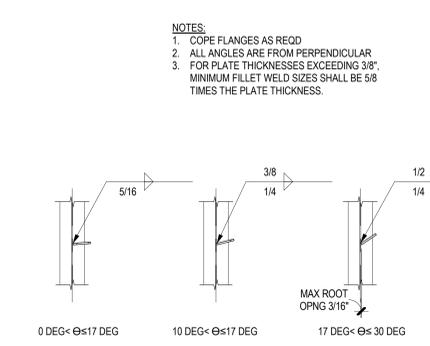
B3 COLLECTOR BM TO COL WEB CONN
SCALE: 3/4" = 1'-0"



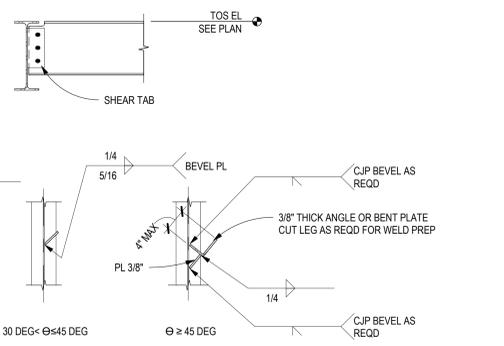
| CONNECTION SCHEDULE | | | | | |
|---------------------|------------|-----------------|-----------------|-----------------|-----------------|
| BRACE SIZE | PL THK 'A' | THRU PL THK 'B' | WELD LENGTH 'A' | WELD LENGTH 'B' | GUSSET PL BOLTS |
| HSS4x4 | 5/8" | 5/8" | 4" | 20" | 3 |
| HSS5x5 | 5/8" | 5/8" | 5" | 20" | 3 |
| HSS6x6 | 5/8" | 5/8" | 6" | 22" | 3 |
| HSS8x8 | 3/4" | 3/4" | 8" | 24" | 4 |
| HSS12x8 | 1" | 1" | 12" | 28" | 5 |

NOTES:
1. LENGTHS GIVEN ARE SINGLE-SIDE LENGTHS AND MINIMUM LENGTHS
2. LONGER GUSSET/WELD LENGTHS MAY BE REQ'D WHERE BRACE SLOPE VARIES FROM 1:1
3. ALL BOLTS IN STANDARD HOLES

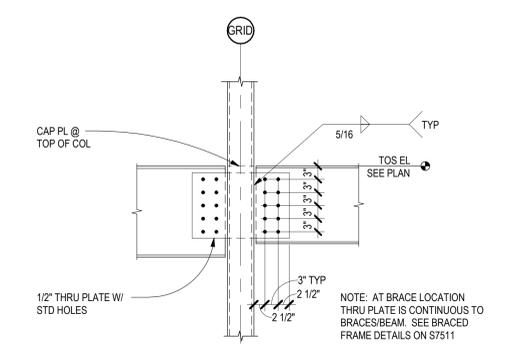
A5 BRACED FRAME CONNECTION
SCALE: 3/4" = 1'-0"



A1 SKEWED BEAM TO BEAM CONN
SCALE: 3/4" = 1'-0"



A3 COLLECTOR BEAM TO COLUMN CONN
SCALE: 3/4" = 1'-0"



A3 COLLECTOR BEAM TO COLUMN CONN
SCALE: 3/4" = 1'-0"

A4 FLANGE PLATE CONN AT HSS COL
SCALE: 3/4" = 1'-0"

A5 BRACED FRAME CONNECTION
SCALE: 3/4" = 1'-0"



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**WILMA P. MANKILLER HEALTH CENTER
EXPANSION**
STILWELL, OKLAHOMA

PROJECT PHASE:
BID PACKAGE 01

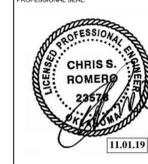
| # | DATE | REVISIONS |
|---|----------|-------------------------|
| 1 | 11/02/19 | BID PACKAGE 01 - ADD 01 |

DATE: 11-01-19 JOB NUMBER: 18-01.01
SHEET NUMBER: S5.53
STEEL DETAILS



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PROFESSIONAL SEAL:



CONSULTANT LOGO



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consulting engineers, inc.
4700 Lincoln Road, Suite 102, Abbeville, LA 71411
504-344-4000 504-343-8759 (fax)

CLIENT:



**WILMA P. MANKILLER HEALTH CENTER
EXPANSION**
STILWELL, OKLAHOMA

KEY PLAN:

PROJECT PHASE:

BID PACKAGE 01

REVISIONS:

| # | DATE | DESCRIPTION |
|---|----------|-------------------------|
| 1 | 11/22/19 | BID PACKAGE 01 - ADD 01 |

DATE:

11-01-19

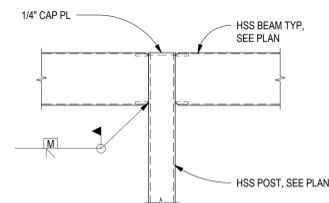
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18-01.01

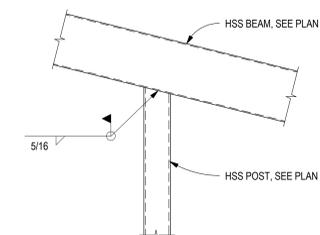
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S5.54

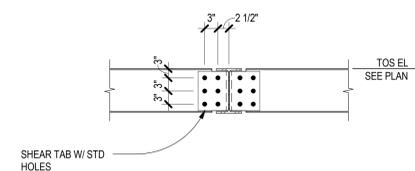
STEEL DETAILS



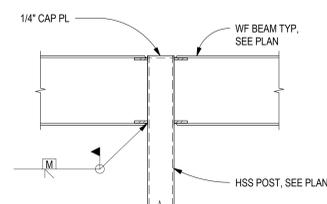
D5 BEAM TO POST MOMENT CONN
SCALE: 3/4" = 1'-0"



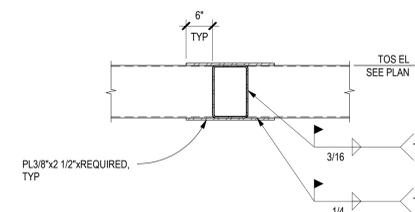
C5 BEAM OVER POST CONNECTION
SCALE: 3/4" = 1'-0"



B5 BEAM TO BEAM CONNECTION
SCALE: 3/4" = 1'-0"



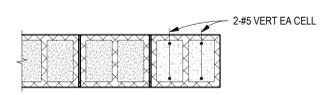
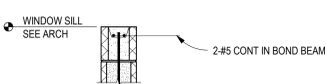
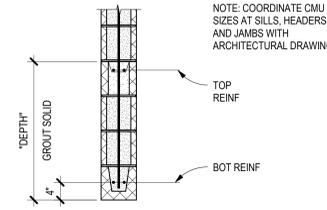
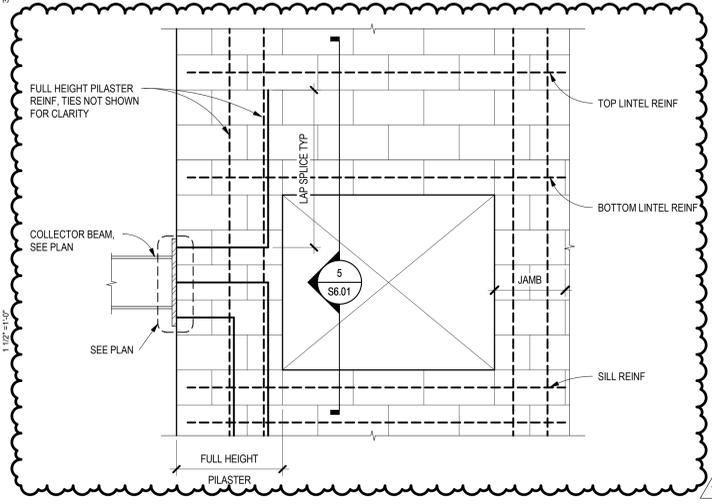
A4 BEAM TO POST MOMENT CONN
SCALE: 3/4" = 1'-0"



A5 HSS TO HSS MOMENT CONNECTION
SCALE: 3/4" = 1'-0"

ENTIRE SHEET MODIFIED

| CMU LINTEL SCHEDULE | | | | | | |
|---------------------|-------|-------|--------------------|--------|-------|-------------|
| OPENING WIDTH | WIDTH | DEPTH | LINTEL REINFORCING | | SILL | |
| | | | TOP | BOTTOM | DEPTH | REINFORCING |
| 0'-0" - 8'-0" | 12" | 32" | 2-#5 | 2-#5 | 8" | 2-#5 |
| | | | | | | TYPE A |

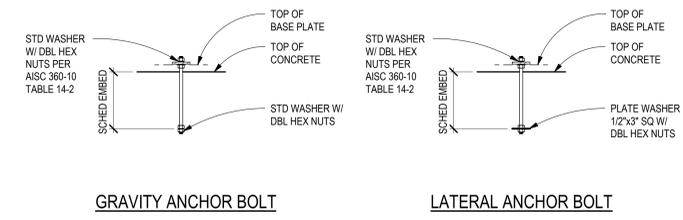
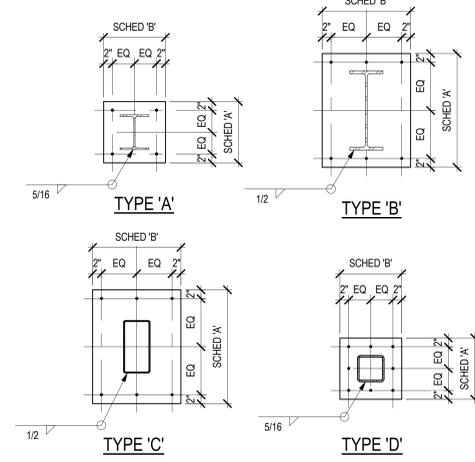


NOTE: SEE TYPICAL CMU PLAN DETAILS SHEET S7.21 FOR TYPICAL HORIZONTAL REINFORCING REQUIREMENTS.

| MARK | COMPOSITE | SLAB | | | | METAL DECK | | | | DECK ATTACHMENTS | | | TOTAL SLAB / DECK THICKNESS | COMMENTS |
|-------|-----------|-------|---------|---|-------|------------|------|------------|--|--------------------------------|--------------------|-------|-----------------------------|----------|
| | | THICK | MATL | REINF | THICK | TYPE | GAGE | FINISH | ATTACH PERP TO RIBS | ATTACH PARALLEL TO RIBS | ATTACH SIDELAPS | | | |
| D1.5R | X | 3" | NW CONC | 6x6 - W2.1xW2.1 WELD WIRE FABRIC IN FLAT SHEETS | 3" | VLI | 18 | GALVANIZED | 7-5/8" DIA PUDDLE WELDS PER 36" WIDE SHEET | 5/8" DIA PUDDLE WELDS @ 12" OC | #10 SCREWS @ 9" OC | 11/2" | | |

| MARK | SLAB | | | BEARING STRATA | COMMENTS |
|------|-----------|------|--------------------|---|--|
| | THICKNESS | MATL | REINFORCING | | |
| S5 | 5" | CONC | #4 @ 18" OC EA WAY | 15 MIL VAPOR RETARDER OVER 1/2" SAND BLOTTER LAYER OVER 4" COMPACTED GRANULAR FILL OVER 14" OF COMPACTED STRUCTURAL FILL OVER COMPACTED SUBGRADE. SUBGRADE WILL BE PLACED WITH LASER LEVEL. | PREPARE SUBGRADE AND STRUCTURAL FILL PER GEOTECHNICAL REPORT |
| S6 | 6" | CONC | #4 @ 12" OC EA WAY | 15 MIL VAPOR RETARDER OVER 1/2" SAND BLOTTER LAYER OVER 4" COMPACTED GRANULAR FILL OVER 14" OF COMPACTED STRUCTURAL FILL OVER COMPACTED SUBGRADE. SUBGRADE WILL BE PLACED WITH LASER LEVEL. | PREPARE SUBGRADE AND STRUCTURAL FILL PER GEOTECHNICAL REPORT |

| MARK | BASE PLATE | | ANCHOR BOLTS | |
|------|------------|---------------------|--------------------|---------|
| | TYPE | SIZE | F1554 ANCHOR BOLTS | TYPE |
| BP1 | A | PL 1 1/4"x18"x1-6" | 4-3/4" DIA x 9" | GRAVITY |
| BP2 | A | PL 1 3/4"x18"x1-6" | 4-3/4" DIA x 9" | GRAVITY |
| BP3 | A | PL 1 3/4"x20"x1-8" | 4-3/4" DIA x 9" | GRAVITY |
| BP4 | A | PL 1 3/4"x22"x1-10" | 6-1" DIA x 18" | GRAVITY |
| BP5 | A | PL 3/4"x14"x1-2" | 4-3/4" DIA x 9" | GRAVITY |
| BP6 | B | PL 1 3/4"x22"x1-8" | 6-1" DIA x 18" | LATERAL |
| BP7 | B | PL 1 3/4"x22"x1-8" | 6-1" DIA x 18" | LATERAL |
| BP8 | C | PL 1 1/2"x20"x1-4" | 6-1" DIA x 18" | LATERAL |
| BP9 | D | PL 3/4"x14"x1-2" | 4-3/4" DIA x 9" | LATERAL |
| BP10 | D | PL 1"x18"x1-6" | 8-3/4" DIA x 9" | LATERAL |



| MARK | VENEER | WALL | REINFORCING | | | COMMENTS |
|------|----------|----------|---------------------|--|-------|---|
| | | | VERTICAL | HORIZONTAL | GRADE | |
| WC8 | -- | 8" CONC | #4 @ 12" OC | #4 @ 12" OC | A615 | |
| WC12 | SEE ARCH | 12" CONC | #5 @ 12" OC EA FACE | #5 @ 12" OC EA FACE | A615 | |
| WM12 | SEE ARCH | 12" CMU | #7 @ 16" OC EA FACE | #5 @ 24" OC EA FACE & STD LADDER TYPE JOINT REINF @ 16" OC | A615 | GROUT ALL CELLS SOLID. SEE 7.21 FOR MASONRY DETAILS. D4/S7.21 FOR HORIZONTAL REINFORCEMENT LOCATION |

| REINFORCEMENT TYPE | REQUIRED LAP SPLICES ACI 318-14/IBC 2015 | | | | | | MINIMUM LENGTH (IN) | COMMENTS |
|---|--|----------------------|---------|---------|---------|---------|---------------------|----------------------------------|
| | #6 AND SMALLER (#6) | #7 THROUGH #11 (#6b) | 3000PSI | 4000PSI | 5000PSI | 3000PSI | | |
| CONTINUOUS WALL FOOTINGS AND HORIZONTAL REINFORCEMENT IN SITE WALLS | 30 | 30 | 30 | 30 | 30 | 30 | 18 | |
| CONCRETE WALLS: ALL VERTICAL REINFORCEMENT | 44 | 38 | 34 | 55 | 48 | 43 | 12 | |
| CONCRETE WALLS: ALL HORIZONTAL REINFORCEMENT, EXCLUDING SITE WALLS AND STEM WALLS | 57 | 50 | 45 | 72 | 62 | 56 | 12 | |
| CONCRETE COLUMNS | 44 | 38 | 34 | 55 | 48 | 43 | 12 | |
| TOP FLEXURAL REINFORCEMENT, INCLUDING BEAMS, GRADE BEAMS, AND COMBINED COLUMN FOOTINGS | 57 | 50 | 45 | 72 | 62 | 56 | 12 | |
| BOTTOM FLEXURAL REINFORCEMENT, INCLUDING BEAMS, GRADE BEAMS, AND COMBINED COLUMN FOOTINGS | 44 | 38 | 34 | 55 | 48 | 43 | 12 | |
| MINIMUM EMBEDMENT OF STANDARD HOOKS INTO CONCRETE BASE | 22 | 19 | 17 | 22 | 19 | 17 | 6 | ALLOWED FOR BARS LARGER THAN #11 |
| SLABS ON GRADE | 30 | 30 | 30 | 30 | 30 | 30 | 12 | |
| SLABS ON METAL DECK | 30 | 30 | 30 | 30 | 30 | 30 | 12 | WWF MINIMUM LAP LENGTH = 6 IN |

NOTES:

- LAP SPLICES SHALL NOT BE PERMITTED FOR BARS LARGER THAN #11.
- LAP SPLICES FOR BUNDLED BARS SHALL BE IN ACCORDANCE WITH ACI 318-14 SECTION 25.5.1.4
- LAP LENGTHS FOR LIGHTWEIGHT CONCRETE SHALL BE INCREASED BY 33%
- LAP LENGTHS FOR EPOXY COATED BARS SHALL BE INCREASED BY 50%
- FOR INTERMEDIATE OR LARGER VALUES OF F_{CD}, USE THE CLOSEST LOWER VALUE IN THE TABLE. DO NOT INTERPOLATE

| MASONRY LAP SPLICES (#6) ACI 530-13/IBC 2015 | | | | | | | | |
|--|----|----|----|----|-----|-----|-----|--|
| | #3 | #4 | #5 | #6 | #7 | #8 | #9 | |
| 8" BLOCK WITH 1-LAYER OF REINFORCEMENT | 32 | 40 | 51 | 72 | N/A | N/A | N/A | |
| 8" BLOCK WITH 2-LAYERS OF REINFORCEMENT | 51 | 68 | 72 | 72 | N/A | N/A | N/A | |
| 12" BLOCK WITH 1-LAYER OF REINFORCEMENT | 32 | 24 | 23 | 37 | 43 | 57 | 65 | |
| 12" BLOCK WITH 2-LAYERS OF REINFORCEMENT | 51 | 68 | 72 | 72 | 72 | N/A | N/A | |
| 16" BLOCK WITH 1-LAYER OF REINFORCEMENT | 32 | 24 | 23 | 30 | 32 | 42 | 48 | |
| 16" BLOCK WITH 2-LAYERS OF REINFORCEMENT | 51 | 68 | 72 | 72 | 72 | 72 | 72 | |

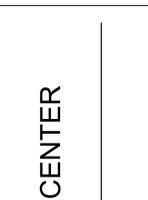
BARS LARGER THAN #9 SHALL BE SPLICED USING MECHANICAL CONNECTIONS

| MARK | SIZE | | | REINFORCING | | COMMENTS |
|------|--------|--------|-------|-----------------------------|-------|------------------------------------|
| | WIDTH | LENGTH | DEPTH | REINFORCING | GRADE | |
| F48 | 4'-0" | 4'-0" | 1'-0" | 4-#5 EA WAY BOT | A615 | |
| F48P | 4'-0" | 4'-0" | 1'-0" | 4-#5 EA WAY BOT | A615 | |
| F60 | 5'-0" | 5'-0" | 1'-6" | 5-#6 EA WAY BOT | A615 | |
| F60A | 5'-0" | 5'-0" | 2'-0" | 6-#6 EA WAY TOP & BOT | A615 | TOP BARS TO HAVE STD HOOKS AT ENDS |
| F72 | 6'-0" | 6'-0" | 1'-6" | 6-#6 EA WAY BOT | A615 | |
| F72A | 6'-0" | 6'-0" | 2'-0" | 8-#6 EA WAY TOP & BOT | A615 | TOP BARS TO HAVE STD HOOKS AT ENDS |
| F72P | 6'-0" | 6'-0" | 1'-6" | 6-#6 EA WAY TOP & BOT | A615 | TOP BARS TO HAVE STD HOOKS AT ENDS |
| F84 | 7'-0" | 7'-0" | 2'-0" | 9-#6 EA WAY BOT | A615 | |
| F84A | 7'-0" | 7'-0" | 2'-0" | 9-#6 EA WAY TOP & BOT | A615 | TOP BARS TO HAVE STD HOOKS AT ENDS |
| F84P | 7'-0" | 7'-0" | 2'-0" | 9-#6 EA WAY TOP & BOT | A615 | TOP BARS TO HAVE STD HOOKS AT ENDS |
| F96 | 8'-0" | 8'-0" | 2'-0" | 7-#7 EA WAY BOT | A615 | |
| F96A | 8'-0" | 8'-0" | 2'-0" | 7-#7 EA WAY TOP & BOT | A615 | TOP BARS TO HAVE STD HOOKS AT ENDS |
| F276 | 23'-0" | 21'-0" | 2'-9" | #8 @ 9" OC EA WAY TOP & BOT | A615 | TOP BARS TO HAVE STD HOOKS AT ENDS |

| MARK | SIZE | | REINFORCING | | COMMENTS |
|------|-------|-------|-------------|-------------|------------------------------------|
| | WIDTH | DEPTH | CONTINUOUS | TRANSVERSE | |
| CF16 | 1'-4" | 1'-0" | 3-#4 | #4 @ 48" OC | |
| CF24 | 2'-0" | 1'-0" | 3-#4 | #4 @ 48" OC | |
| CF84 | 7'-0" | 2'-9" | 8-#8 | #8 @ 9" OC | TOP BARS TO HAVE STD HOOKS AT ENDS |



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EXPANSION
STILWELL, OKLAHOMA

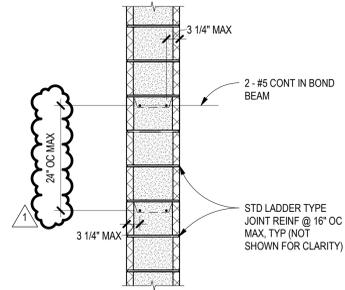
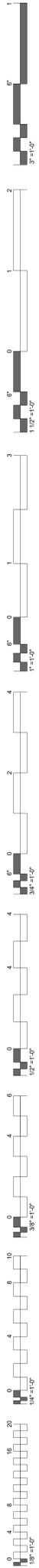
PROJECT PHASE:
BID PACKAGE 01

| # | DATE | REVISIONS / DESCRIPTION |
|---|----------|-------------------------|
| 1 | 11/02/19 | BID PACKAGE 01 - ADD 01 |

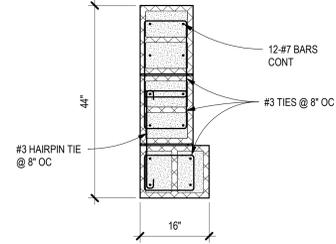
DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER: S6.01

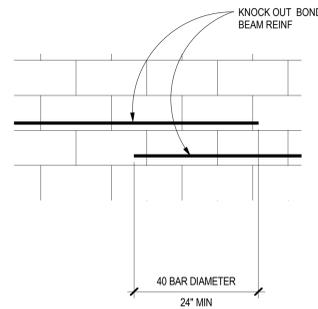
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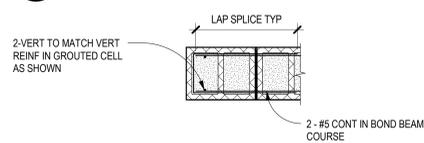
D4 TYPICAL BOND BEAM DETAIL
SCALE: 3/4" = 1'-0"



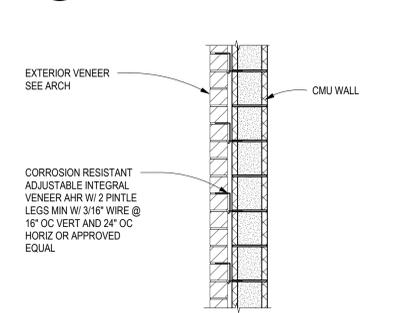
C4 PILASTER DETAIL
SCALE: 3/4" = 1'-0"



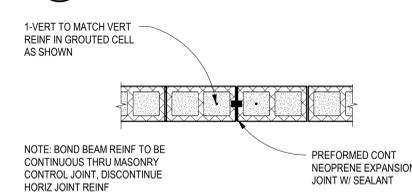
B4 TYPICAL STEP IN BOND BEAM
SCALE: 3/4" = 1'-0"



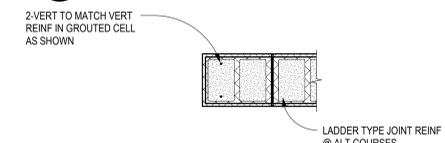
D5 TYPICAL JOIST THRU CMU WALL
SCALE: 3/4" = 1'-0"



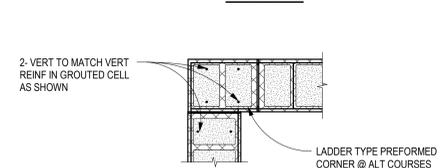
C5 TYPICAL VENEER TO CMU WALL DETAIL
SCALE: 3/4" = 1'-0"



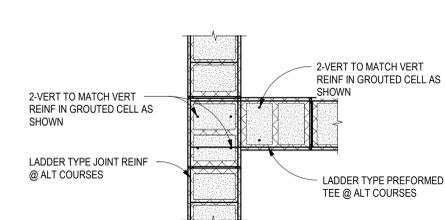
B5 TYPICAL CMU CNRTL JOINT (MCJ)
SCALE: 3/4" = 1'-0"



WALL END



WALL CORNER

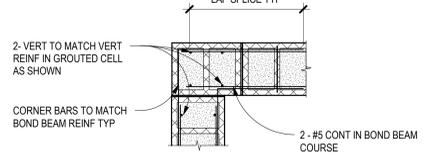


WALL TEE

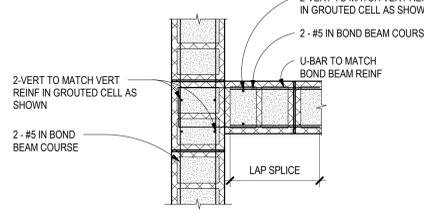
A5 TYPICAL 12" CMU PLAN DETAILS
SCALE: 3/4" = 1'-0"

A4 TYPICAL 12" CMU PLAN DETAILS
SCALE: 3/4" = 1'-0"

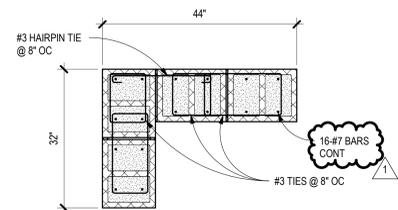
BOND BEAM AT WALL END



BOND BEAM AT WALL CORNER

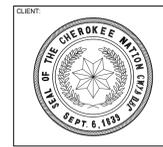
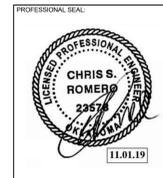


BOND BEAM AT WALL TEE



A3 PILASTER DETAIL
SCALE: 3/4" = 1'-0"

NOTES:
1. ALL PILASTER REINFORCING IS IN ADDITION TO REINFORCING SPECIFIED IN WALL SCHEDULE.
2. ALL REINFORCING SHALL BE CONTINUOUS ALONG ENTIRE HEIGHT OF WALL, UNLESS NOTED OTHERWISE.



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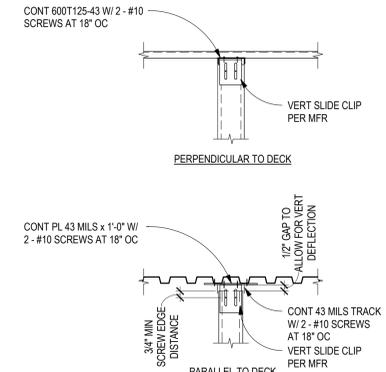
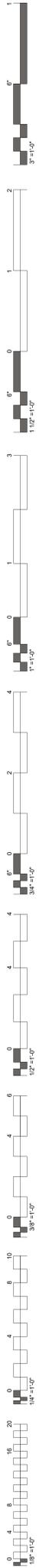
KEY PLAN:

PROJECT PHASE:
BID PACKAGE 01

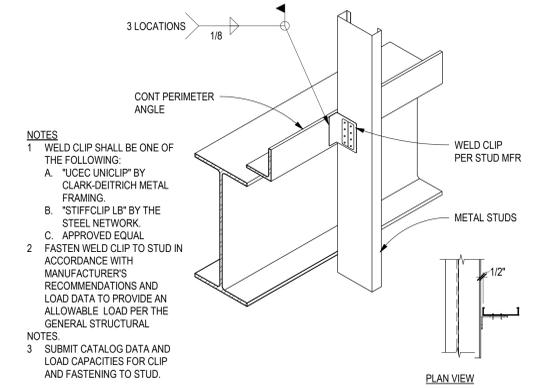
| # | DATE | REVISIONS | DESCRIPTION |
|---|----------|-------------------------|-------------|
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DATE: 11-01-19 JOB NUMBER: 18-01.01

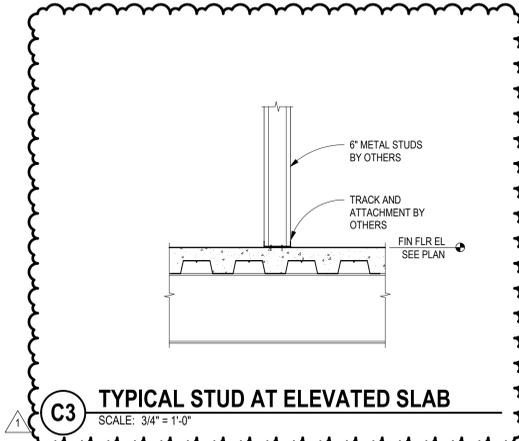
SHEET NUMBER:
S7.21
TYPICAL MASONRY DETAILS



D4 TYPICAL SLIP TRACK ASSEMBLY
SCALE: NTS

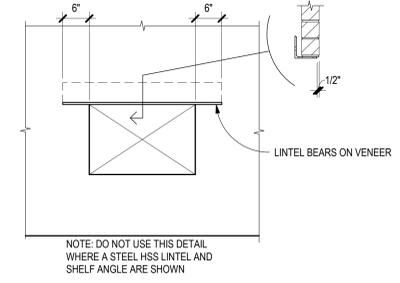


D5 TYPICAL WELD CLIP
SCALE: NTS

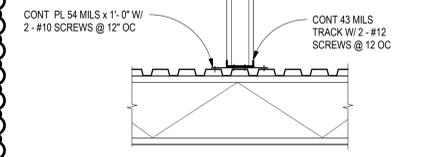


C3 TYPICAL STUD AT ELEVATED SLAB
SCALE: 3/4\"/>

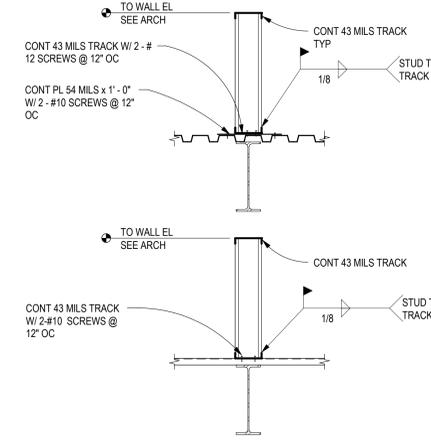
| VENEER LINTEL | |
|--------------------|---------------|
| OPENING WIDTH | ANGLE |
| 0' - 0" TO 2' - 0" | L5x3x1/4 LLH |
| 2' - 1" TO 3' - 4" | L5x3x5/16 LLH |
| 3' - 5" TO 4' - 0" | L5x3x3/8 LLH |
| 4' - 1" TO 6' - 4" | L6x6x1/2 |
| 6' - 5" TO 8' - 0" | L6x6x1/2 |



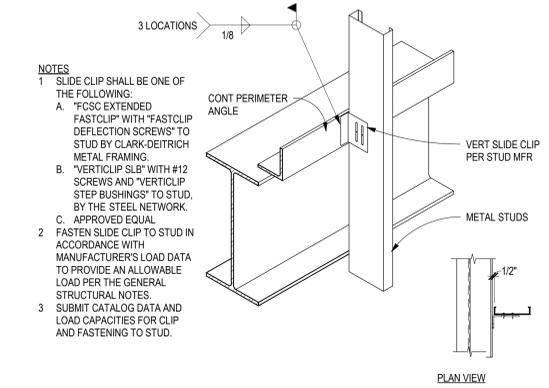
B3 COLD-FORMED LINTEL SECTION @ VENEER
SCALE: 3/4\"/>



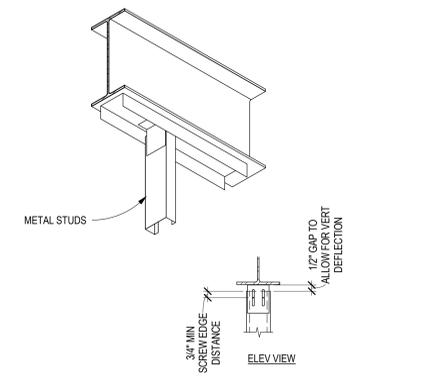
C4 TYPICAL STUD AT METAL DECK
SCALE: 3/4\"/>



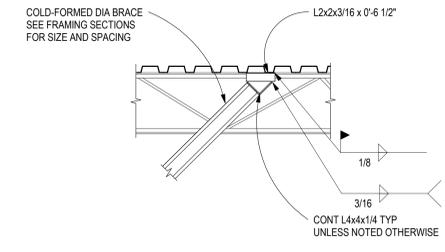
B4 TYPICAL PARAPET TO DECK
SCALE: 3/4\"/>



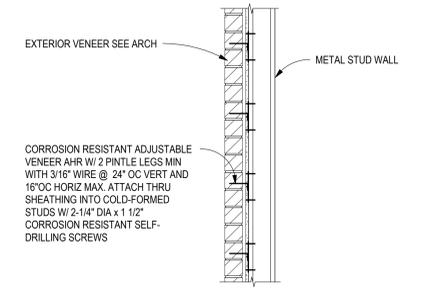
C5 TYPICAL VERTICAL SLIDE CLIP
SCALE: NTS



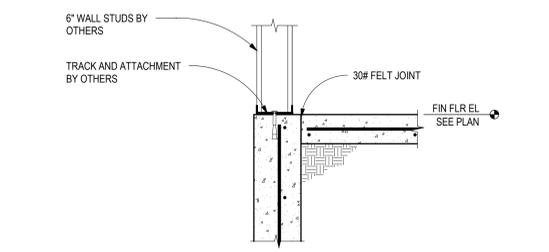
B5 TYPICAL SLIP TRACK ASSEMBLY
SCALE: NTS



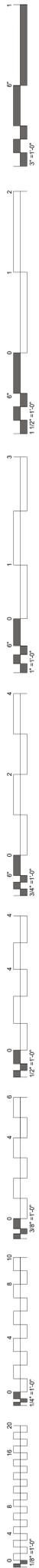
A3 TYPICAL DIAG BRACE TO DECK
SCALE: 3/4\"/>



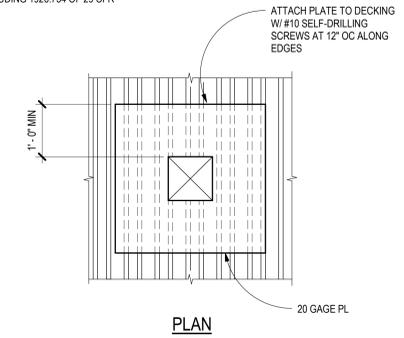
A4 TYPICAL VENEER ON MTL STUDS
SCALE: 3/4\"/>



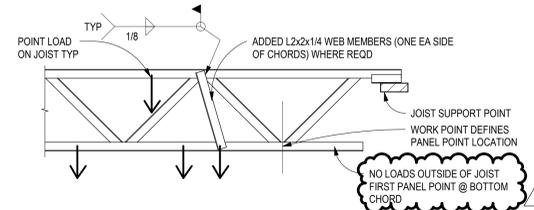
A5 TYPICAL STUDS AT STEMWALL
SCALE: 1\"/>



NOTE: INSTALL OPENINGS IN COMPLIANCE WITH OSHA REQUIREMENTS INCLUDING 1926.754 OF 29 CFR



D5 TYPICAL ROOF OPENING 0''-12''
SCALE: 3/4" = 1'-0"

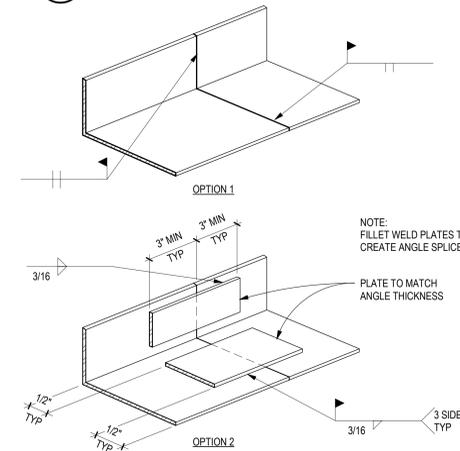


POINT LOADS ON K-SERIES OR LH-SERIES (NON SP JOISTS) SHALL MEET ALL OF THE FOLLOWING REQUIREMENTS:

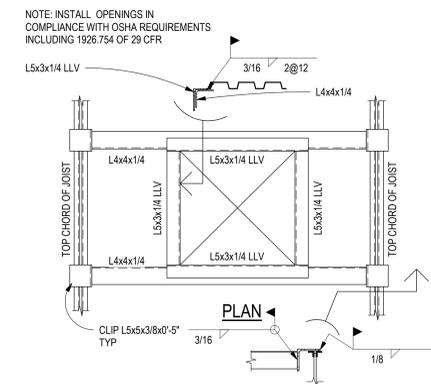
- POINT LOADS ON THE JOIST TOP CHORD OR BOTTOM CHORD THAT EXCEED 100LBS SHALL BE PLACED WITHIN 4" OF A JOIST PANEL POINT, OR ADDITIONAL WEB MEMBERS SHALL BE ADDED AT THE POINT OF LOAD APPLICATION.
- WHERE MULTIPLE POINT LOADS ARE PLACED BETWEEN THE SAME TWO PANEL POINTS, THE SUM OF THOSE LOADS THAT ARE NOT REINFORCED WITH ADDITIONAL WEB MEMBERS SHALL NOT EXCEED 100LBS.
- POINT LOADS SHALL BE CONCENTRIC WITH THE CHORD FROM WHICH IT IS HUNG. BEAM CLAMPS OR OTHER CONNECTIONS THAT INDUCE NON-CONCENTRIC LOADS ARE NOT PERMITTED.
- POINT LOADS SHALL BE SPACED SO THAT THE COMBINED TOP CHORD PLUS BOTTOM CHORD POINT LOADS DO NOT EXCEED AN EQUIVALENT LINE LOAD OF 65PLF AT ANY POINT ALONG THE JOIST.
- LOADS SHALL NOT BE PLACED ON THE BOTTOM CHORD OUTSIDE OF THE FIRST PANEL POINT.
- POINT LOADS IN EXCESS OF 350LBS THAT ARE NOT SPECIFICALLY NOTED ON THE STRUCTURAL DRAWINGS SHALL NOT BE PLACED WITHOUT APPROVAL FROM THE ENGINEER OF RECORD AND THE JOIST MANUFACTURER.

NOTE: JOISTS LABELED AS "SP" HAVE BEEN DESIGNED FOR THE LOADS NOTED ON THE STRUCTURAL PLANS. THOSE LOADS SHALL BE PLACED WITHIN 2" OF A PANEL POINT, OR ADDITIONAL WEB MEMBERS SHALL BE ADDED.

C4 TYPICAL JOIST REINFORCING
SCALE: 1/2" = 1'-0"

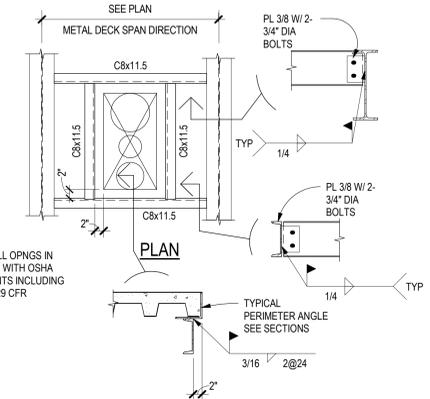


B4 TYPICAL PERIMETER ANGLE SPLICE
SCALE: 3/4" = 1'-0"

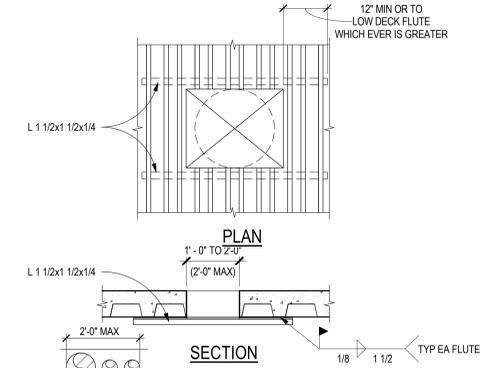


A4 TYPICAL ROOF OPENING > 12''
SCALE: 3/4" = 1'-0"

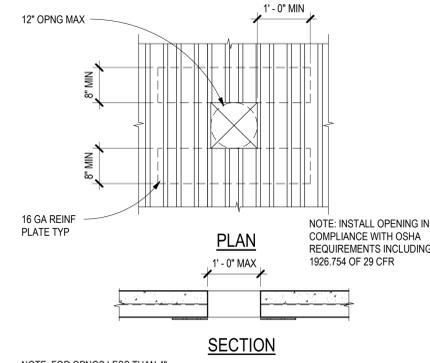
NOTE: INSTALL OPNGS IN COMPLIANCE WITH OSHA REQUIREMENTS INCLUDING 1926.754 OF 29 CFR



C5 TYPICAL SLAB OPNG > 24''
SCALE: 3/4" = 1'-0"



B5 TYPICAL SLAB OPENING 12''-24''
SCALE: 3/4" = 1'-0"



A5 TYPICAL SLAB OPENING 4''-12''
SCALE: 3/4" = 1'-0"



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STILWELL, OKLAHOMA

KEY PLAN:

PROJECT PHASE:
BID PACKAGE 01

| # | DATE | REVISIONS / DESCRIPTION |
|---|----------|-------------------------|
| 1 | 11/02/19 | BID PACKAGE 01 - ADD 01 |

DATE: 11-01-19
JOB NUMBER: 18-01.01

SHEET NUMBER:
S7.42
TYPICAL STEEL DETAILS