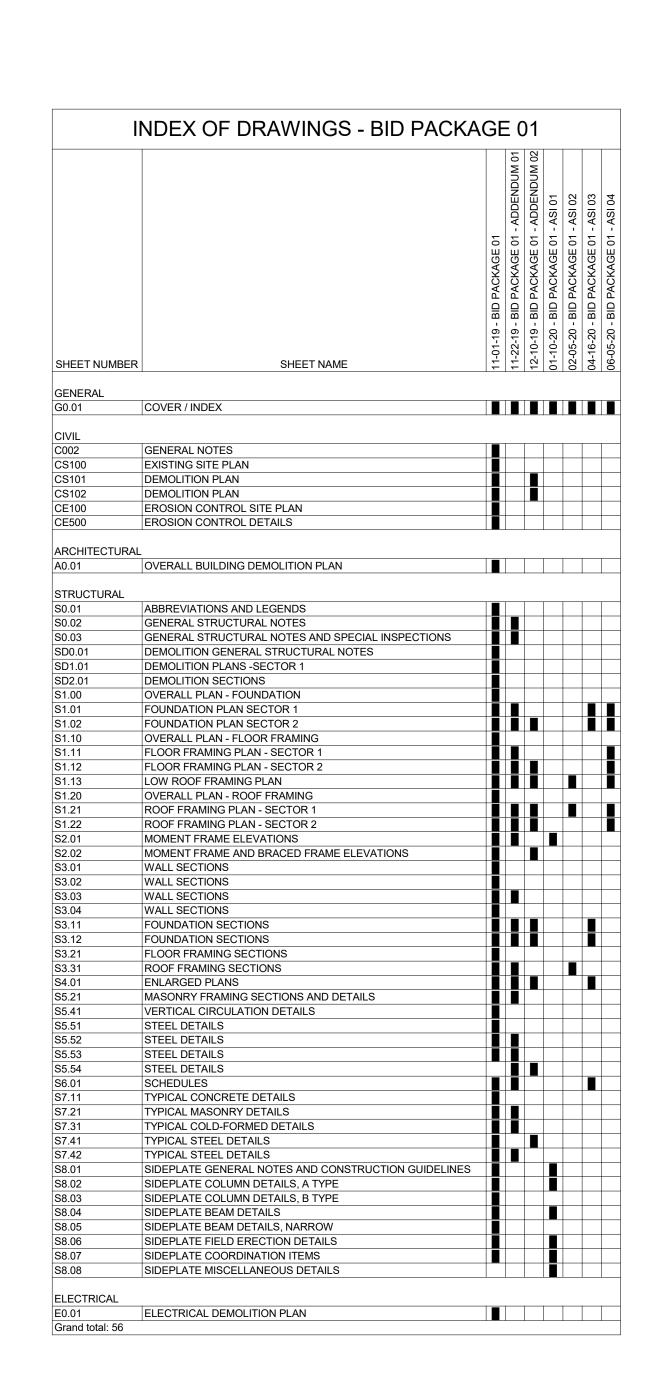
WILMA P. MANKILLER HEALTH CENTER EXPANSION

BID PACKAGE 01 (DEMOLITION / STEEL / FOUNDATIONS)



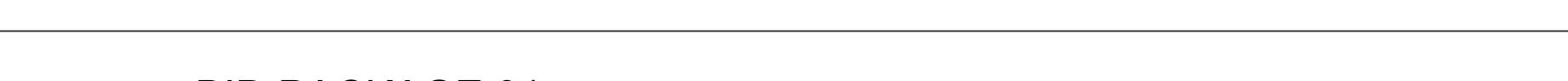


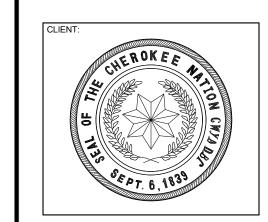












PROJECT PHASE: **BID PACKAGE 01**

| REVISIONS | # DATE | DESCRIPTION | 1 11/22/19 | BID PACKAGE 01 - ADD 01 | 2 12/10/19 | BID PACKAGE 01 - ADD 02 | 3 1/10/20 BID PACKAGE 01 - ASI 01 2/5/2020 BID PACKAGE 01 - ASI 02 5 4/16/2020 BID PACKAGE 01 - ASI 03 6 6/5/2020 BID PACKAGE 01 - ASI 04

11-01-19 18-01.01 SHEET NUMBER:

COVER / INDEX

MECHANICAL / ELECTRICAL / PLUMBING ENGINEER

(539) 664-4618

CIVIL ENGINEER

STRUCTURAL ENGINEER

FIRE PROTECTION / LIFE SAFETY

GENERAL NOTES

- 1. THE CONTRACTOR SHALL HAVE EXISTING UTILITIES LOCATED PRIOR TO CONSTRUCTION. CONTRACTOR SHALL CALL "OKIE" 1-800-522-6543 IN ADDITION TO DIRECT NOTIFICATION. CONTRACTOR SHALL BRACE UTILITY POLES AS NECESSARY. UTILITIES DAMAGED BY THE CONTRACTOR SHALL BE REPAIRED TO THE UTILITY OWNER'S SPECIFICATIONS BY THE CONTRACTOR AT NO COST TO THE OWNER.
- THE CONTRACTOR SHALL ESTABLISH, INSTALL, OPERATE, AND MAINTAIN COMPLETE AND ADEQUATE AND SAFE TRAFFIC CONTROLS DURING THE ENTIRE CONSTRUCTION PERIOD. ALL TRAFFIC CONTROL DEVICES SHALL BE APPROVED BY THE
- 3. ALL DIMENSIONS OR ELEVATIONS WITH ± SHALL BE CONFIRMED BY THE CONTRACTOR.
- 4. ALL DIMENSIONS OF EXISTING STRUCTURES AND EQUIPMENT SHALL BE VERIFIED BY THE CONTRACTOR. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER & OWNER.
- TOPSOIL IN THE DISTURBED AREAS SHALL BE REMOVED, STOCKPILED, AND RESTORED AFTER CONSTRUCTION OPERATIONS, IN ACCORDANCE WITH PROJECT SPECIFICATIONS. ALL EXCESS TOPSOIL SHALL BE CONSIDERED WASTE AND STOCKPILED ON-SITE BY THE CONTRACTOR, UNLESS OTHERWISE NOTED.
- 6. ALL DISTURBED ROADWAY AND DRIVEWAY SURFACES SHALL BE RESTORED TO THEIR PRE-CONSTRUCTION CONDITION.
- 7. FALL PROTECTION AROUND ALL OPENINGS AND EXCAVATION SHALL BE MAINTAINED AT ALL TIMES.
- 8. NORTH ARROWS SHOWN ON DRAWINGS INDICATE LOCAL COORDINATE SYSTEM ESTABLISHED BY THE SURVEYOR, UNLESS OTHERWISE NOTED.
- 9. TRENCH SAFETY AND SHORING IN ACCORDANCE WITH CURRENT OSHA REGULATIONS SHALL BE EMPLOYED BY CONTRACTOR AT ALL TIMES.
- IF AT ANY POINT CONSTRUCTION ACTIVITIES EXPOSE ARCHEOLOGICAL MATERIALS SUCH AS CHIPPED STONE, TOOLS, POTTERY, BONE, HISTORIC CROCKERY, GLASS, METAL ITEMS OR BUILDING MATERIALS, THE OKLAHOMA ARCHEOLOGICAL SURVEY STATE ARCHEOLOGIST, KARY L. STACKELBECK, SHALL BE CONTACTED IMMEDIATELY AT 405-325-7211.
- 11. ALL STATIONS SHOWN ON THE PLANS ARE CENTERLINE STATIONS UNLESS NOTED OTHERWISE.
- THE TOPOGRAPHIC SURVEY WAS COMPLETED BY NATIVE PLAINS SURVEYING & MAPPING, LLC. ALL EXISTING INFORMATION IS VERIFYING ALL EXISTING INFORMATION. IF CONTRACTOR BELIEVES EXISTING INFORMATION IS INACCURATE, THE CONTRACTOR MAY HAVE A NEW SURVEY COMPLETED AT NO ADDITIONAL COST TO THE OWNER, ARCHITECT, OR ENGINEER.
- 13. DIMENSIONS SHOWN ARE TO BACK OF CURB OR CENTERLINE OF PIPE UNLESS NOTED OTHERWISE.
- 14. CONTRACTOR SHALL REVIEW & COORDINATE W/ ARCHITECTURAL, MECH., ELEC., & PLUMBING DISCIPLINES DRAWINGS, SPEC'S & DETAILS. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER. IN THE EVENT THAT THE ARCH. &/OR CONTRACTOR DEVIATES CONSTRUCTION FROM THESE PLANS W/O THE EXPRESS WRITTEN APPROVAL OF THE ENGINEER, THE ARCH. &/OR CONTRACTOR ASSUMES FULL RESPONSIBILITY FOR THOSE MODIFICATIONS.

EROSION CONTROL NOTES:

- 1. SILT FENCE SHALL BE MAINTAINED AND SEDIMENT BUILDUP REGULARLY REMOVED UNTIL PAVING OPERATIONS ARE COMPLETE AND/OR SEEDING IS IN PLACE AND 75% VEGETATION STABILIZATION IS OBTAINED.
- 2. ALL TREES, BRUSH, AND OTHER DEBRIS THAT MIGHT INTERFERE WITH THE FLOW OF WATER IS TO BE CLEANED OUT TO THE RIGHT-OF-WAY LINE AT EACH STRUCTURE, IN A MANNER APPROVED BY THE ENGINEER.
- 3. ALL FLOW LINES THAT ARE TO BE FILLED SHALL BE THOROUGHLY COMPACTED TO 95% STANDARD PROCTOR DENSITY BEFORE CONSTRUCTION OR EXTENSION OF DRAINAGE STRUCTURES.
- 4. IN ORDER TO ALLEVIATE DUST CONDITIONS DURING GRADING OPERATIONS, AND AFTER GRADING OPERATIONS ARE COMPLETED. BUT BEFORE PAVEMENT AND/OR PERMANENT EROSION CONTROL WORK IS COMPLETED, THE CONTRACTOR SHALL SPRINKLE GRADING AT INTERVALS APPROVED BY THE OWNERS REPRESENTATIVE.
- 5. ALL UNPAVED DISTURBED AREAS SHALL RECEIVE SLAB SOD FOR PERMANENT EROSION CONTROL, UNLESS NOTED OTHERWISE. THIS SHALL INCLUDE FERTILIZER, WATERING & MOWING AS REQUIRED TO ESTABLISH A VIABLE TURF.
- 6. AT THE BEGINNING OF THE TURF OPERATIONS, ANY AREAS INCLUDED IN PLANNED QUANTITIES THAT HAVE GROWN A SATISFACTORY VOLUNTEER TURF OF PERENNIAL GRASS AS DETERMINED BY THE OWNER'S REPRESENTATIVE, SHALL BE FERTILIZED AND WATERED BUT SHALL NOT BE SEEDED, SODDED OR SPRIGGED.
- 7. VEGETATIVE MULCH AND SEEDING SHALL BE UTILIZED FOR TEMPORARY EROSION CONTROL.
- 8. SEED: THE FOLLOWING KINDS OF SEEDS, AT ACRES-RATES INDICATED BELOW, SHALL BE PLANTED ON THE AREAS DESIGNATED FOR SEEDING.

TEMPORARY SEEDING KINDS OF SEED TO BE FURNISHED QUANTITY PER ACRE **COOL SEASON MIX-**20 LBS. OF SEED PERENNIAL RYEGRASS (LOLIUM PERENNE) CRIMSOM CLOVER (TRIFOLIUM INCARNATUM) 12 LBS. OF SEED WARM SEASON MIX-12 LBS. OF SEED KOREAN LESPEDEZA (LESPEDEZA STRIATA) CRIMSOM CLOVER (TRIFOLIUM INCARNATUM) 20 LBS. OF SEED

ACRE, JUST PRIOR TO THE REPLACEMENT OF SALVAGED TOPSOIL.

LITTLE BLUESTEM (ANDROPOGON SCOPARIUS)

COMMON BERMUDA (CYNODON DACYLON) 4 LBS. OF SEED VEGETATIVE MULCHING: THE VEGETATIVE MULCH SHALL BE ANCHORED IN ACCORDANCE WITH THE "ADHESIVE SPRAY METHOD", AS SPECIFIED IN 233.04(b) OF THE ODOT STANDARD SPECIFICATIONS.

12 LBS. OF SEED

SEASONAL PLANTING RESTRICTIONS THE PLANTING OF SPRIGGING SHALL BE RESTRICTED TO THE PERIOD FROM APRIL 1ST TO JUNE 30TH.

THE PLANTING OF TEMPORARY SEEDS (COOL SEASON MIX) SHALL BE RESTRICTED TO THE PERIOD FROM SEPTEMBER 1ST TO

THE PLANTING OF TEMPORARY SEEDS (WARM SEASON MIX) SHALL BE RESTRICTED TO THE PERIOD FROM MARCH 15TH TO JUNE

AREAS ON WHICH SALVAGED TOPSOIL IS TO BE REPLACED SHALL HAVE 0-46-0 FERTILIZER APPLIED, AT THE RATE OF 150 LBS. PER

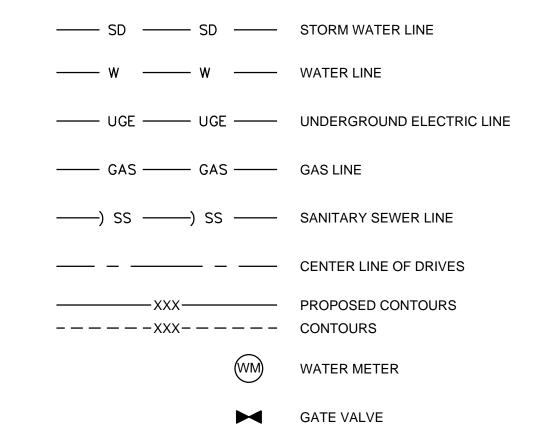
SITE WORK NOTES

- 1. ALL EARTHWORK & PAVING MATERIALS & METHODS SHALL CONFORM WITH OKLAHOMA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION, LATEST REVISION, UNLESS OTHERWISE NOTED.
- 2. CONTRACTOR SHALL REVIEW GEOTECHNICAL REPORT PREPARED BY BUILDING & EARTH. DATED AUGUST 30, 2018. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER.THIS REPORT SHOULD BE CONSIDERED A PART OF THESE CONSTRUCTION DOCUMENTS.
- 3. ONLY REMOVE TREES THAT DIRECTLY INTERFERE WITH CONSTRUCTION. CONTRACTOR SHALL LIMIT CLEARING & GRUBBING TO BUILDING & PARKING AREA FOOTPRINT, AS MUCH AS POSSIBLE.
- 4. CONTRACTOR SHALL DISPOSE OF TREES, STUMPS, DEBRIS, ETC. OFF SITE IN A MANNER APPROVED BY THE OWNER.
- ALL AREAS TO RECEIVE PAVING SHALL BE STRIPPED OF VEGETATION, TOPSOIL, SOFT OR OTHERWISE SUITABLE MATERIAL. THIS WOULD INCLUDE AREAS IDENTIFIED FOR UNDERCUT. AREA SHALL BE SCARIFIED TO A DEPTH OF 8 INCHES, MOISTURE CONDITIONED TO A RANGE OF 1% BELOW TO 3% ABOVE THE MATERIAL'S OPTIMUM MOISTURE CONTENT, & COMPACTED TO A DENSITY OF AT LEAST 95% OF THE STANDARD PROCTOR (ASTM D 698) MAXIMUM DRY DENSITY. SUBGRADE SHALL BE PROOF ROLLED WITH A ROLLER OR TRUCK (GROSS WEIGHT OF 25 TONS OR MORE). SOFT AREAS SHALL BE EXCAVATED & REPLACED WITH SUITABLE MATERIAL. PROOF ROLLING SHALL BE WITNESSED BY OWNER'S REPRESENTATIVE. OWNER SHALL DETERMINE SUITABILITY OF SUBGRADE. REFER TO GEOTECHNICAL RECOMMENDATIONS.
- WHERE LIMESTONE IS EXPOSED AT FINISHED SUBGRADE, IT IS RECOMMENDED TO UNDERCUT THE LIMESTONE ROCK UNTIL A LEVEL THAT WILL ALLOW FOR PLACEMENT OF AT LEAST 8" OF STRUCTURAL FILL TO PROVIDE FOR UNIFORM SUBGRADE CONDITIONS ACROSS PAVEMENT AREAS,
- 7. REMOVE ANY STUMPS, ROOTS LARGER THAN 2 INCHES IN DIAMETER, ROCKS LARGER THAN 3 INCHES AND ANY MATTED ROOTS, TO A DEPTH OF 18 INCHES BELOW ORIGINAL GROUND SURFACE.
- 8. SELECT FILL SHALL BE COMPOSED OF MATERIAL WITH MAXIMUM DRY DENSITY IN EXCESS OF 100 POUNDS PER CUBIC FOOT, PLASTICITY INDEX (PI) LESS THAN 18, AND A LIQUID LIMIT (LL) LESS THAN 40. STRUCTURAL FILL SHOULD BE FREE OF ANY ORGANICS, SHOULD NOT CONTAIN ROCK FRAGMENTS GREATER THAN 3 INCHES IN ANY DIMENSION, AND SHOULD BE PROPERLY MOISTURE CONDITIONED PRIOR TO USE AS SELECT FILL. SELECT FILL SHOULD BE COMPACTED TO A MINIMUM OF 95% OF THE STANDARD PROCTOR MAXIMUM DRY DENSITY AND WITHIN ±2% OF THE OPTIMUM MOISTURE AS DETERMINED BY ASTM D 698. THE FILL MATERIAL SHOULD BE SPREAD IN HORIZONTAL LIFTS STARTING AT THE LOWEST ELEVATION. THE LIFTS SHOULD NOT EXCEED 8 TO 12 INCHES IN LOOSE LIFT THICKNESS.
- 9. REUSE OF ON SITE SOILS AS FILL IS NOT RECOMMENDED BELOW PLANNED BUILDING OR PAVEMENT AREAS.
- 10. ALL DISTURBED AREAS THAT ARE NOT PAVED ARE TO RECEIVE SLAB SODDING.
- SHOWN AS ACCURATELY AS POSSIBLE BASE UPON FIELD RECONNAISSANCE AND RESEARCH. CONTRACTOR IS RESPONSIBLE FOR 11. EARTHWORKS SHALL BE PERFORMED IN SUCH A MANNER TO MINIMIZE PONDING WATER ON THE SUBGRADE. SITE SHALL MAINTAIN DRAINAGE AT ALL TIMES. MOISTURE CONTENT OF SOIL SHOULD BE MAINTAINED NEAR OPTIMUM DURING CONSTRUCTION.
 - 12. ALL ROCKS AND DEBRIS SHALL BE REMOVED FROM ALL DRAINS PRIOR TO FINAL INSPECTION.
 - 13. ROADSIDE HAZARDS SHALL BE COMPLETELY BARRICADED AROUND THEIR PERIMETER FOR THE SAFETY OF PEDESTRIANS AND VEHICLES.
 - 14. ONLY THE AMOUNT OF TRENCH THAT CAN BE BACK FILLED OR SURFACED IN (2) DAYS SHALL BE ALLOWED OPEN UNLESS APPROVED BY OWNER'S REPRESENTATIVE.
 - 15. ANY EXISTING FOUNDATIONS OR FOOTINGS SHALL BE REMOVED FULL DEPTH, & BACKFILLED WITH APPROVED COMPACTED MATERIAL.
 - 16. THIS BID PACKAGE IS INTENDED TO INCLUDE ONLY DEMOLITION RELATED TASKS INCLUDING BUT NOT LIMITED TO EROSION CONTROL ITEMS, STRIPPING/SALVAGING TOP SOIL, DEMOLITION, SUBGRADE MAINTENANCE.

UTILITY NOTES

- THE CONTRACTOR SHALL PROTECT ALL EXISTING UTILITIES AND SERVICES FROM DAMAGE. UTILITIES SHALL REMAIN IN SERVICE AT ALL TIMES, AND ANY DISRUPTION OF SERVICE SHALL BE AT THE CONTRACTOR'S SOLE
- 2. CONTRACTOR SHALL VERIFY EXISTING PIPE SIZE, TYPE AND LOCATION TO INSURE PROPER CONNECTION.
- THE PLANS HAVE BEEN PREPARED TO SHOW THE APPROXIMATE LOCATION OF EXISTING KNOWN UTILITIES. THE CONTRACTOR SHALL CONTACT OKIE, EACH RESPECTIVE UTILITY COMPANY AND THE PROJECT OWNER TO DETERMINE THE EXACT LOCATION OF UNDERGROUND UTILITIES PRIOR TO EXCAVATION. ANY CHANGE IN ALIGNMENT OR GRADE CAUSED BY INTERFERING UTILITIES SHALL BE MADE BY THE CONTRACTOR AT NO COST TO THE OWNER AND THE ENGINEER NOTIFIED.
- DEPTHS OF ANY EXISTING UTILITIES SHOWN ON THE PLANS ARE APPROXIMATE. CONTRACTOR SHALL AT HIS OWN EXPENSE UNCOVER AND VERIFY THE LOCATION AND ELEVATION OF EXISTING UTILITIES IN ADVANCE OF THE CONSTRUCTION.

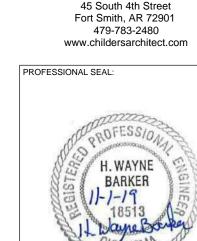
PROPOSED SITE LEGEND





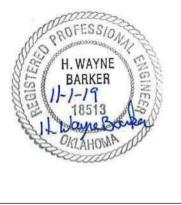
SIDEWALK AREA

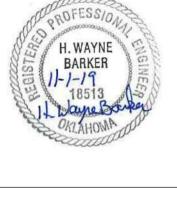
GRAVEL PARKING

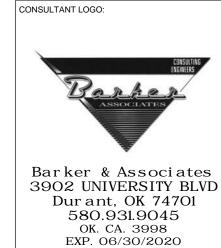


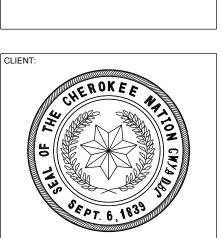
Architect, Inc.







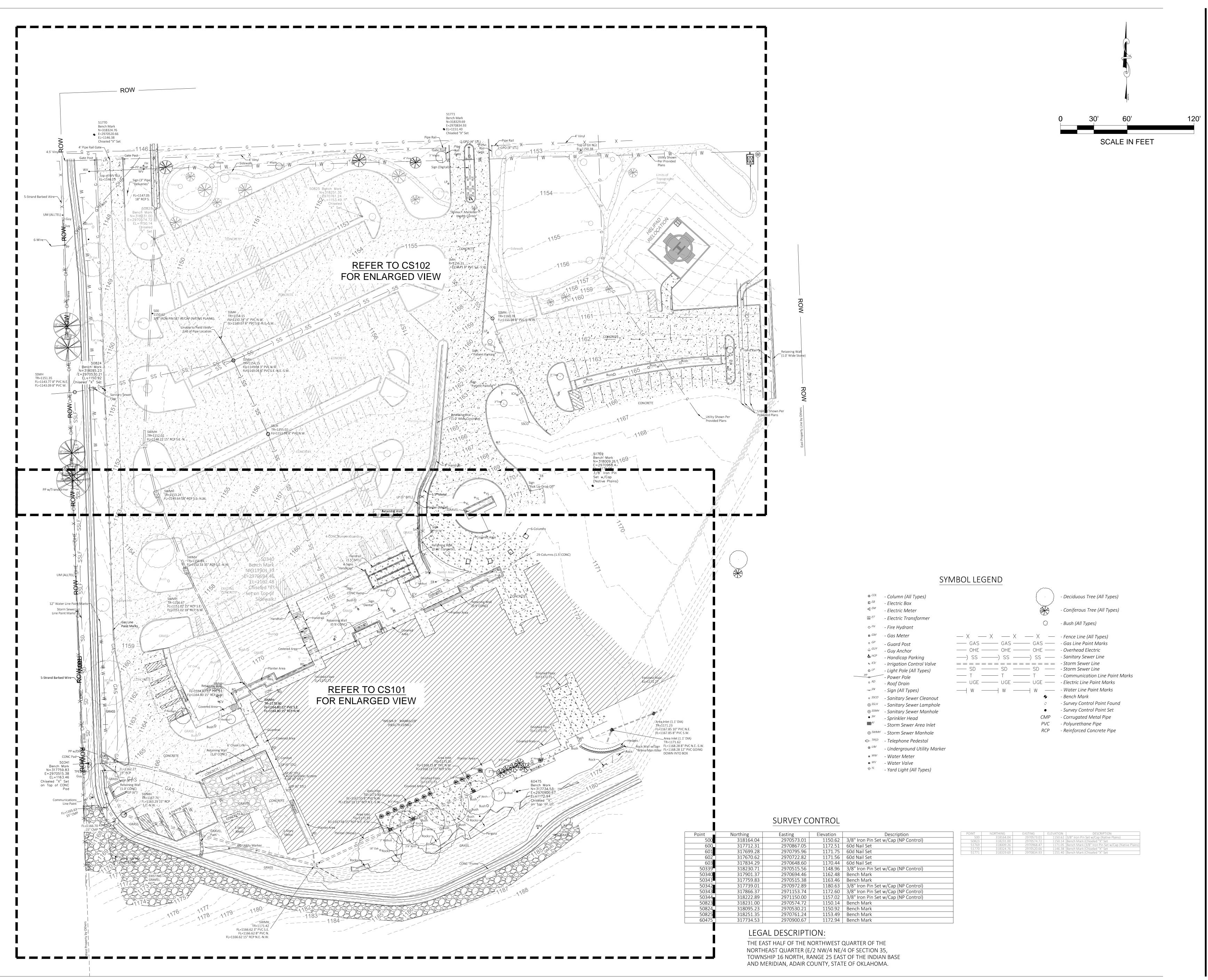


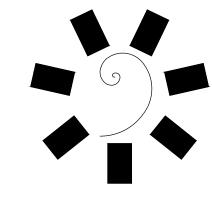


KEY PLAN:

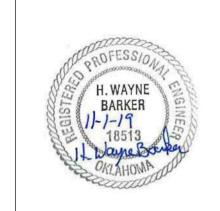
PROJECT PHASE: **BID PACKAGE 01**

GENERAL NOTES

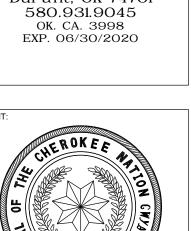


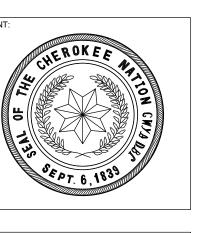


Architect, Inc. 45 South 4th Street Fort Smith, AR 72901 479-783-2480 www.childersarchitect.com









PROJECT PHASE:

BID PACKAGE 01

		REV	ISIONS
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ATE		1.1	OB NUMBER:

11.01.19 18-01.01 SHEET NUMBER:

CS100

EXISTING SITE PLAN



SCALE IN FEET

<u>LEGEND</u>

APROX. LIMITS OF DEMOLITION.

95,152± SQFT. OF EXISTING CONCRETE

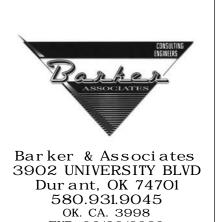
PAVING TO BE REMOVED.

IS PLACED.

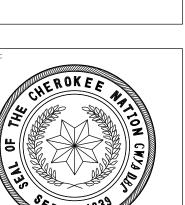
(5) UTILITY CUT OFF POINT SYMBOL,

LINE TO BE REMOVED.

THE BEGINNING POINT OF ANY UTILITY (GAS, WATER, SEWER, & ELECTRIC)



EXP. 06/30/2020



PROJECT PHASE:

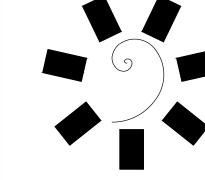
BID PACKAGE 01

REVISIONS # DATE DESCRIPTION

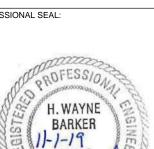
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CS101

DEMOLITION PLAN



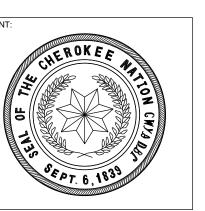
Architect, Inc. 45 South 4th Street Fort Smith, AR 72901 479-783-2480 www.childersarchitect.com







Barker & Associates 3902 UNIVERSITY BLVD Durant, OK 74701 580.931.9045 OK. CA. 3998 EXP. 06/30/2020



Retaining \ (1.0' Wide

KEY PLAN:

PROJECT PHASE:

BID PACKAGE 01

11.01.19 | 18-01.01

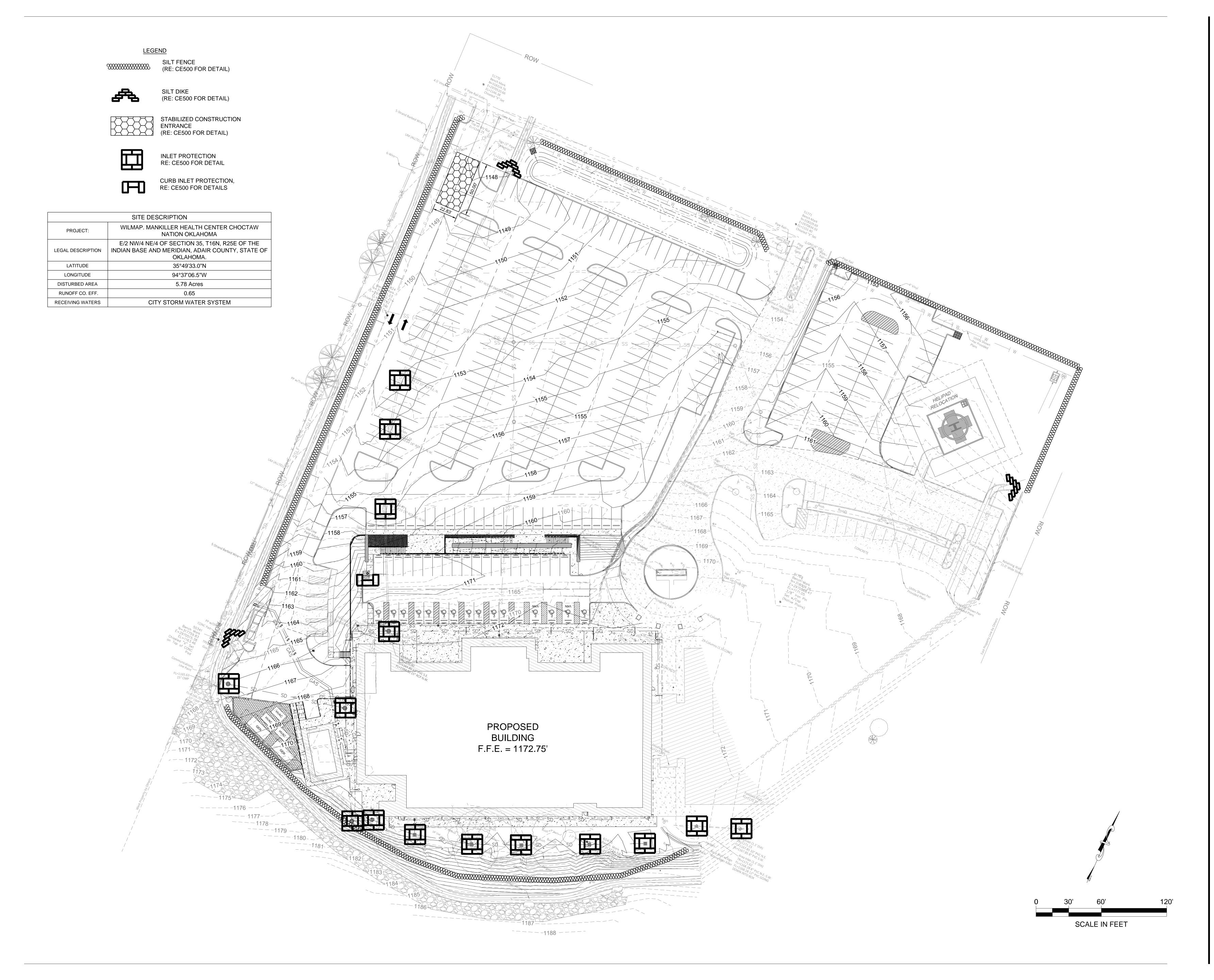
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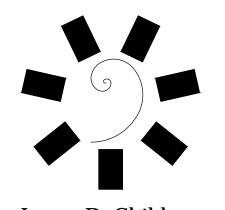
(5) UTILITY CUT OFF POINT SYMBOL,

LINE TO BE REMOVED.

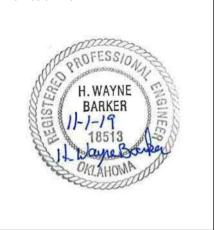
THE BEGINNING POINT OF ANY UTILITY (GAS, WATER, SEWER, & ELECTRIC)

DEMOLITION PLAN



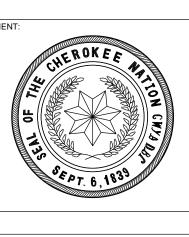


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BID PACKAGE 01

11.01.19 18-01.01

EROSION CONTROL SITE PLAN

GENERAL EROSION NOTES

- 1. THE STORM WATER POLLUTION PREVENTION PLAN IS COMPRISED OF THIS DRAWING (SITE MAP), THE STANDARD DETAILS, PLUS THE PERMIT AND ALL SUBSEQUENT REPORTS AND RELATED DOCUMENTS.
- ALL CONTRACTORS AND SUBCONTRACTORS INVOLVED WITH STORM POLLUTION PREVENTION SHALL OBTAIN A COPY OF THE STORM WATER POLLUTION PREVENTION PLAN AND THE STATE NATIONAL POLLUTION DISCHARGE ELIMINATION SYSTEM GENERAL PERMIT (NPDES PERMIT) AND BECOME FAMILIAR WITH THEIR CONTENTS.
- CONTRACTOR SHALL IMPLEMENT BEST MANAGEMENT PRACTICES AS REQUIRED BY THE SWPPP. ADDITIONAL BEST MANAGEMENT PRACTICES SHALL BE IMPLEMENTED AS DICTATED BY CONDITIONS AT NO ADDITIONAL COST OF OWNER THROUGHOUT ALL PHASES OF CONSTRUCTION
- BEST MANAGE PRACTICES (BMP'S) AND CONTROLS SHALL CONFORM TO FEDERAL, STATE, OR LOCAL REQUIREMENTS OR MANUAL OF PRACTICE, AS APPLICABLE. CONTRACTOR SHALL IMPLEMENT ADDITIONAL CONTROLS AS DIRECTED BY PERMITTING AGENCY OR OWNER.
- SITE MAP MUST CLEARLY DELINEATE ALL STATE WATERS, PERMITS FOR ANY CONSTRUCTION ACTIVITY IMPACTING STATE WATER OR REGULATED WETLANDS MUST BE MAINTAINED ON SITE AT ALL TIMES.
- CONTRACTOR SHALL MINIMIZE CLEARING TO THE MAXIMUM EXTENT PRACTICAL OR AS REQUIRED BY THE GENERAL PERMIT.
- GENERAL CONTRACTOR SHALL DENOTE ON PLAN THE TEMPORARY PARKING AND STORAGE AREA WHICH SHALL ALSO BE USED AS THE EQUIPMENT MAINTENANCE AND CLEANING AREA, EMPLOYEE PARKING AREA, AND AREA FOR LOCATING PORTABLE FACILITIES, OFFICE TRAILERS, AND TOILET FACILITIES. CONTRACTOR SHALL CONSTRUCT TEMPORARY BERM ON DOWN STREAM SIDES.
- ALL WASH WATER (CONCRETE TRUCKS, VEHICLE CLEANING, EQUIPMENT CLEANING, ETC.) SHALL BE DETAINED AND PROPERLY TREATED OR
- SUFFICIENT OIL AND GREASE ABSORBING MATERIALS AND FLOTATION BOOMS SHALL BE MAINTAINED ON SITE OR READILY AVAILABLE TO
- CONTAIN AND CLEAN-UP FUEL OR CHEMICAL SPILLS AND LEAKS.
- 10. DUST ON THE SITE SHALL BE CONTROLLED. THE USE OF MOTOR OILS AND OTHER PETROLEUM BASED OR TOXIC LIQUIDS FOR DUST SUPPRESSION OPERATIONS IS PROHIBITED.
- 11. RUBBISH, TRASH, GARBAGE, LITTER, OR OTHER SUCH MATERIALS SHALL BE DEPOSITED INTO SEALED CONTAINERS, MATERIALS SHALL BE PREVENTED FROM LEAVING THE PREMISES THROUGH THE ACTION OF WIND OR STORM WATER DISCHARGE INTO DRAINAGE DITCHES OR WATERS OF THE STATE.
- ALL STORM WATER POLLUTION PREVENTION MEASURES PRESENTED ON THIS SITE MAP, AND IN THE STORM WATER POLLUTION PREVENTION PLAN, SHALL BE INITIATED AS SOON AS PRACTICABLE.
- 13. ALL DENUDED AREAS THAT WILL BE INACTIVE FOR 14 DAYS OR MORE, MUST BE STABILIZED TEMPORARY WITH THE USE OF FAST-GERMINATING ANNUAL GRASS/GRAIN VARIETIES, STRAW/HAY MULCH, WOOD CELLULOSE FIBERS, TACKIFIERS NETTING OR BLANKETS AS SHOWN ON SITE MAP.
- 14. DISTURBED PORTIONS OF THE SITE WHERE CONSTRUCTION ACTIVITY HAS PERMANENTLY STOPPED SHALL BE PERMANENTLY STABILIZED AS SHOWN ON THE PLANS. THESE AREAS SHALL BE SEEDED, SODDED, AND/OR VEGETATED NO LATER THAN 14 DAYS AFTER THE LAST CONSTRUCTION ACTIVITY OCCURRING IN THESE AREAS. REFER TO THE GRADING PLAN AND/OR LANDSCAPE PLAN.
- 15. IF THE ACTION OF VEHICLES TRAVELING OVER THE GRAVEL CONSTRUCTION ENTRANCES IS NOT SUFFICIENT TO REMOVE THE MAJORITY OF DIRT OR MUD, THEN THE TIRES MUST BE WASHED BEFORE THE VEHICLES ENTER A PUBLIC ROAD. IF WASHING IS USED, PROVISION MUST BE MADE TO INTERCEPT THE WASH WATER AND TRAP THE SEDIMENT BEFORE IT IS CARRIED OFF THE SITE ONLY USE INGRESS/ENGRESS LOCATIONS AS PROVIDED.
- 16. ALL MATERIALS SPILLED, DROPPED, WASHED, OR TRACKED FROM VEHICLES ONTO ROADWAY OR INTO STORM DRAINS MUST BE
- 17. CONTRACTORS OR SUBCONTRACTORS WILL BE RESPONSIBLE FOR REMOVING SEDIMENT IN DETENTION POND AND ANY SEDIMENT THAT MAY HAVE COLLECTED IN THE STORM SEWER DRAINAGE SYSTEMS IN CONJUNCTION WITH THE STABILIZATION OF THE SITE.
- 18. ON-SITE AND OFF-SITE SOIL STOCKPILE AND BORROW AREAS SHALL BE PROTECTED FROM EROSION AND SEDIMENTATION THROUGH IMPLEMENTATION OF BEST MANAGEMENT PRACTICES. STOCKPILE AND BORROW AREA LOCATIONS SHALL BE NOTED ON THE SITE MAP AND PERMITTED IN ACCORDANCE WITH GENERAL PERMIT REQUIREMENTS.
- 19. SLOPES SHALL BE LEFT IN A ROUGHENED CONDITION DURING THE GRADING PHASE TO REDUCE RUNOFF VELOCITIES AND EROSION.
- 20. DUE TO THE GRADE CHANGES DURING THE DEVELOPMENT OF THE PROJECT, THE CONTRACTOR SHALL BE RESPONSIBLE FOR ADJUSTING THE EROSION AND SEDIMENT CONTROL MEASURES (SILT FENCES, ETC.) TO PREVENT EROSION AND POLLUTANT DISCHARGE.
- 21. GENERAL CONTRACTOR IS TO DESIGNATE/IDENTIFY AREAS ON THE SITE MAPS, INSIDE OF THE LIMITS OF DISTURBANCE, FOR WASTE
- DISPOSAL AND DELIVERY AND MATERIAL STORAGE.

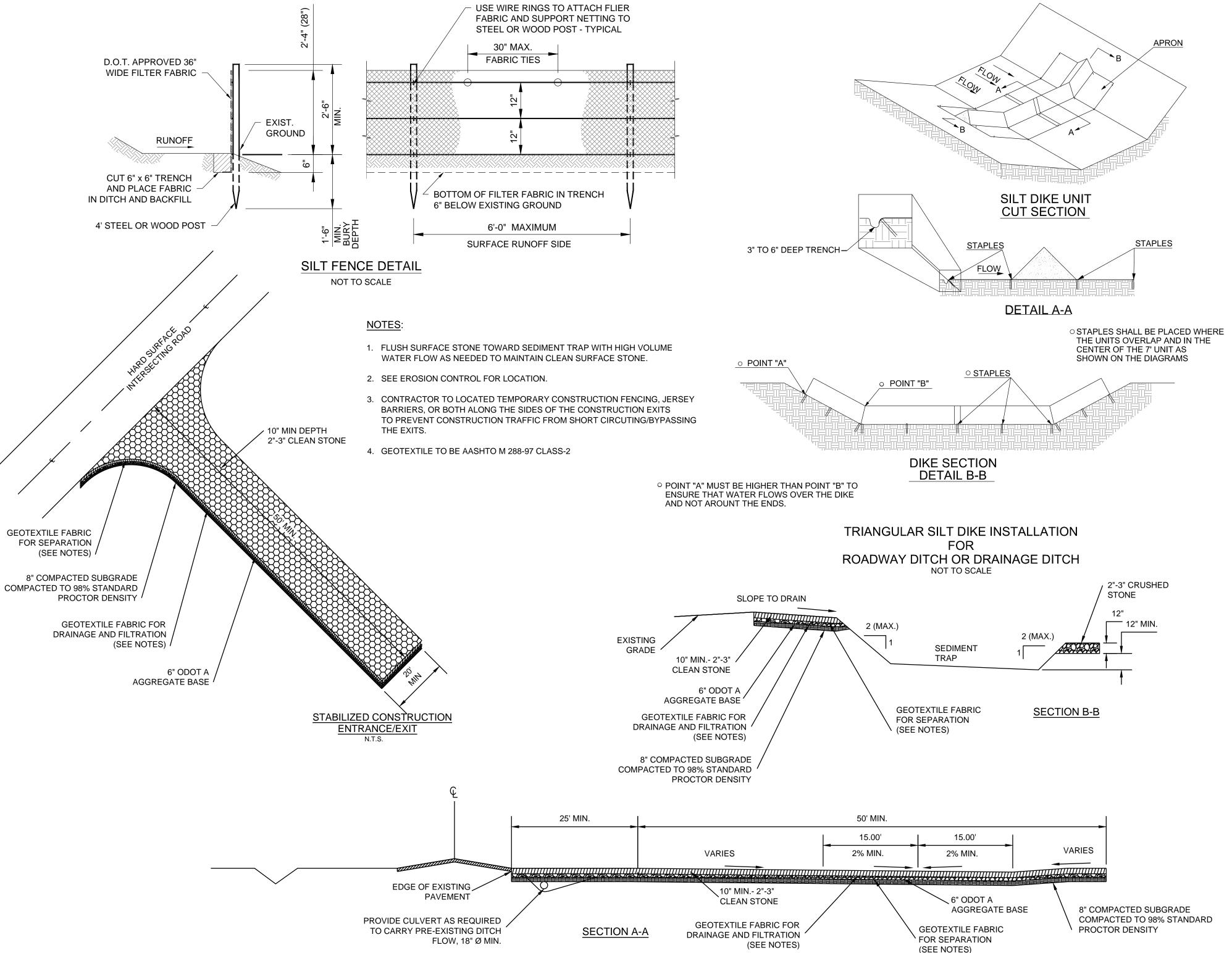
22. CONTRACTOR SHALL BE RESPONSIBLE FOR PREPARING & SUBMITTING NOTICE OF INTENT(N.O.I.) & NOTICE OF TERMINATION (N.O.T.).

23. CONTRACTOR TO LIMIT DISTURBANCE OF SITE IN STRICT ACCORDANCE WITH EROSION CONTROL SEQUENCING SHOWN ON THIS PLAN. NO UNNECESSARY OR IMPROPERLY SEQUENCED CLEARING AND/OR GRADING SHALL BE PERMITTED.

BMP MAINTENANCE EROSION NOTES

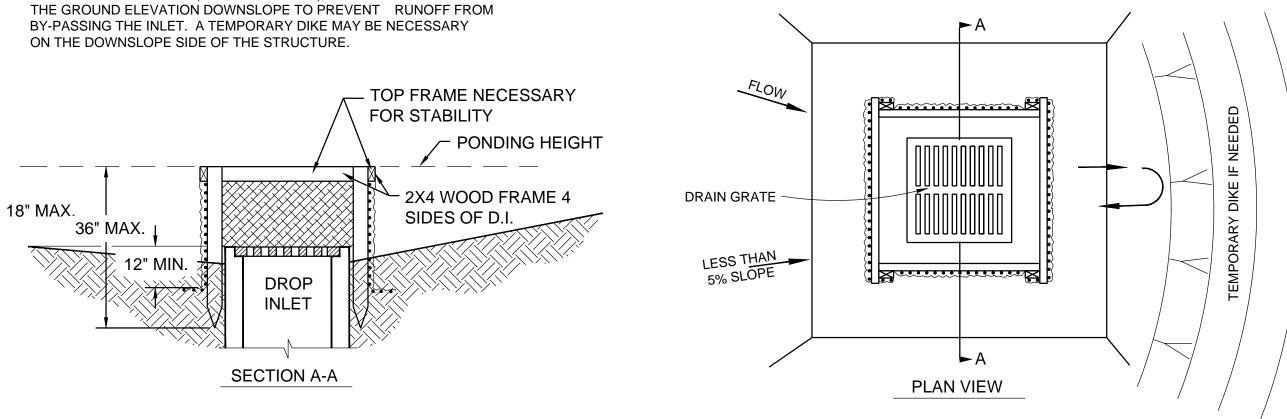
ALL MEASURES STATED ON THIS SITE MAP, AND IN THE STORM WATER POLLUTION PREVENTION PLAN, SHALL BE MAINTAINED IN FULLY FUNCTIONAL CONDITION UNTIL NO LONGER REQUIRED FOR A COMPLETED PHASE OF WORK OR FINAL STABILIZATION OF THE SITE. ALL EROSION AND SEDIMENTATION CONTROL MEASURES SHALL BE CHECKED BY A QUALIFIED PERSON IN ACCORDANCE WITH THE CONTRACT DOCUMENTS OR THE APPLICABLE PERMIT, WHICHEVER IS MORE STRINGENT, AND REPAIRED IN ACCORDANCE WITH THE FOLLOWING:

- 1. INLET PROTECTION DEVICES AND BARRIERS SHALL BE REPAIRED OR REPLACED IF THEY SHOW SIGNS OF UNDERMINING OR DETERIORATION.
- ALL SEEDED AREAS SHALL BE CHECKED REGULARLY TO SEE THAT A GOOD STAND IS MAINTAINED. AREAS SHOULD BE FERTILIZED, WATERED, AND RESEEDED AS NEEDED.
- SILT FENCES SHALL BE REPAIRED TO THEIR ORIGINAL CONDITIONS IF DAMAGED. SEDIMENT SHALL DE REMOVED FROM THE SILT FENCES WHEN IT REACHES ONE-HALF THE HEIGHT OF THE SILT FENCE.
- THE CONSTRUCTION EXITS SHALL BE MAINTAINED IN A CONDITION WHICH WILL PREVENT TRACKING OR FLOW OF MUD ONTO PUBLIC RIGHT-A-WAY. THIS MAY REQUIED PERIODIC TOP DRESSING OF THE CONSTRUCTION EXITS AS CONDITIONS DEMAND.
- THE TEMPORARY PARKING AND STORAGE AREA SHALL BE KEPT IN GOOD CONDITION (SUITABLE FOR PARKING AND STORAGE). THIS MAY REQUIRE PERIODIC TOP DRESSING OF THE TEMPORARY PARKING AREA AS CONDITIONS DEMAND.
- OUTLET STRUCTURES IN THE SEDIMENTATION BASINS SHALL BE MAINTAINED IN OPERATIONAL CONDITIONS AT ALL TIMES. SEDIMENT SHALL BE REMOVED FROM SEDIMENT BASINS OR TRAPS WHEN THE DESIGN CAPACITY HAS BEEN REDUCED BY 50%.
- 7. PRIOR TO LEAVING THE SITE, ALL VEHICLES SHALL BE CLEANED OF DEBRIS. ANY DEBRIS AND/OR SEDIMENT REACHING THE PUBLIC STREET SHALL BE CLEANED IMMEDIATELY BY A METHOD OTHER THAN FLUSHING.



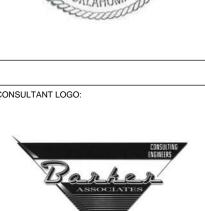
NOTES:

- 1. DROP INLET SEDIMENT BARRIERS ARE TO BE USED FOR SMALL, NEARLY LEVEL DRAINAGE AREAS. (LESS THAN 5%.)
- 2. USE 2X4 WOOD OR EQUIVALENT METAL STAKES, 3' MINIMUM
- 3. INSTALL 2X4 WOOD TOP FRAME TO INSURE STABILITY.
- 4. THE TOP OF THE FRAME (PONDING HEIGHT), MUST BE WELL BELOW THE GROUND ELEVATION DOWNSLOPE TO PREVENT RUNOFF FROM BY-PASSING THE INLET. A TEMPORARY DIKE MAY BE NECESSARY



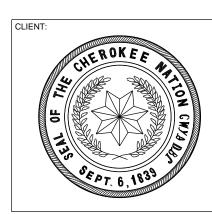
STABILIZED CONSTRUCTION EXIT

SCALE: N.T.S.



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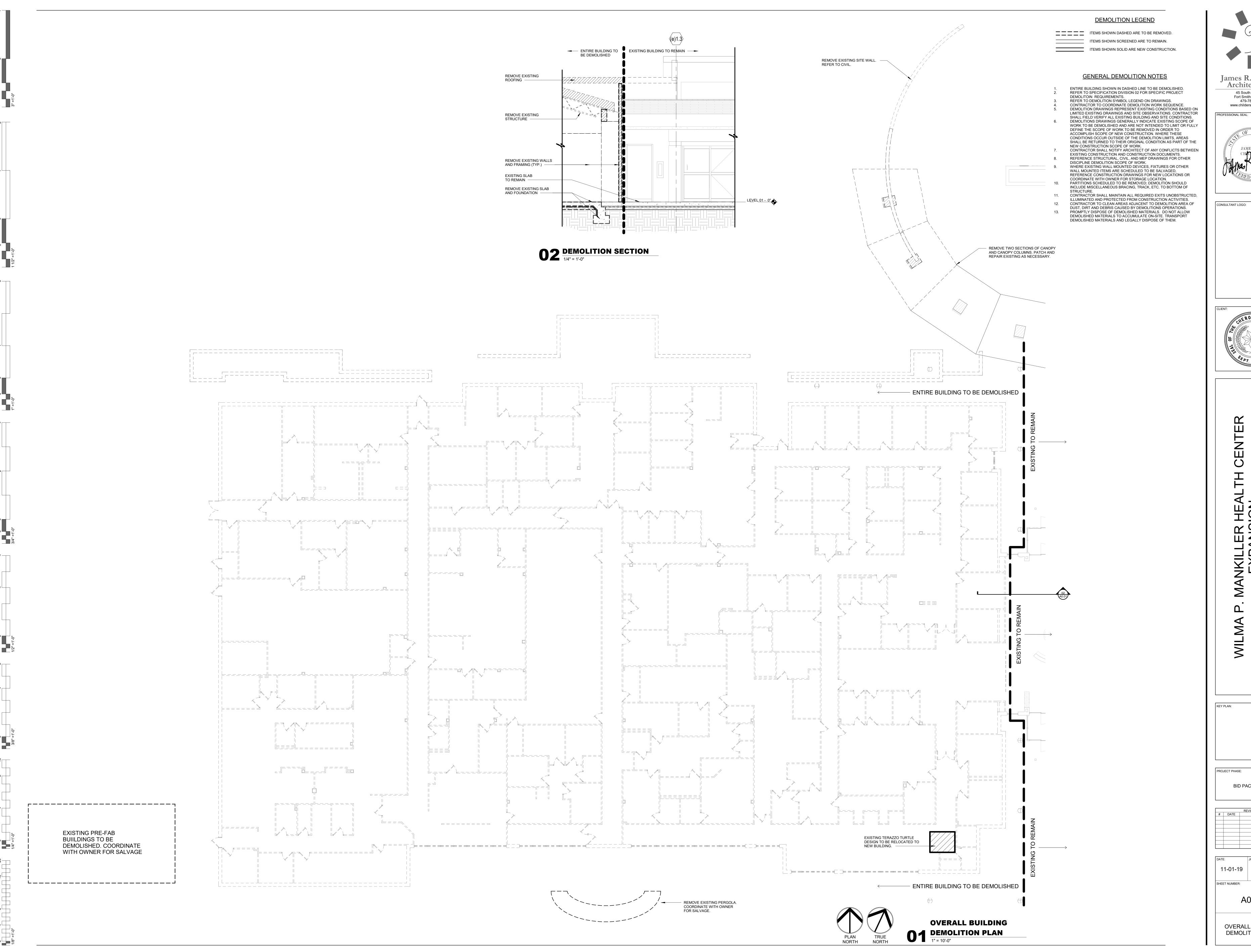
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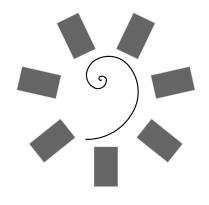
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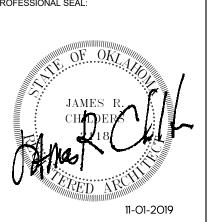
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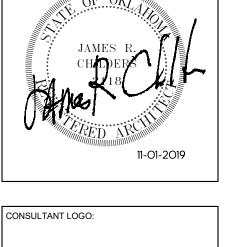
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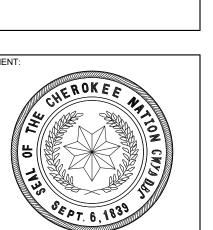


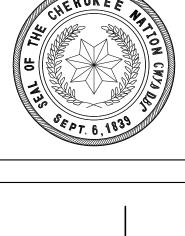


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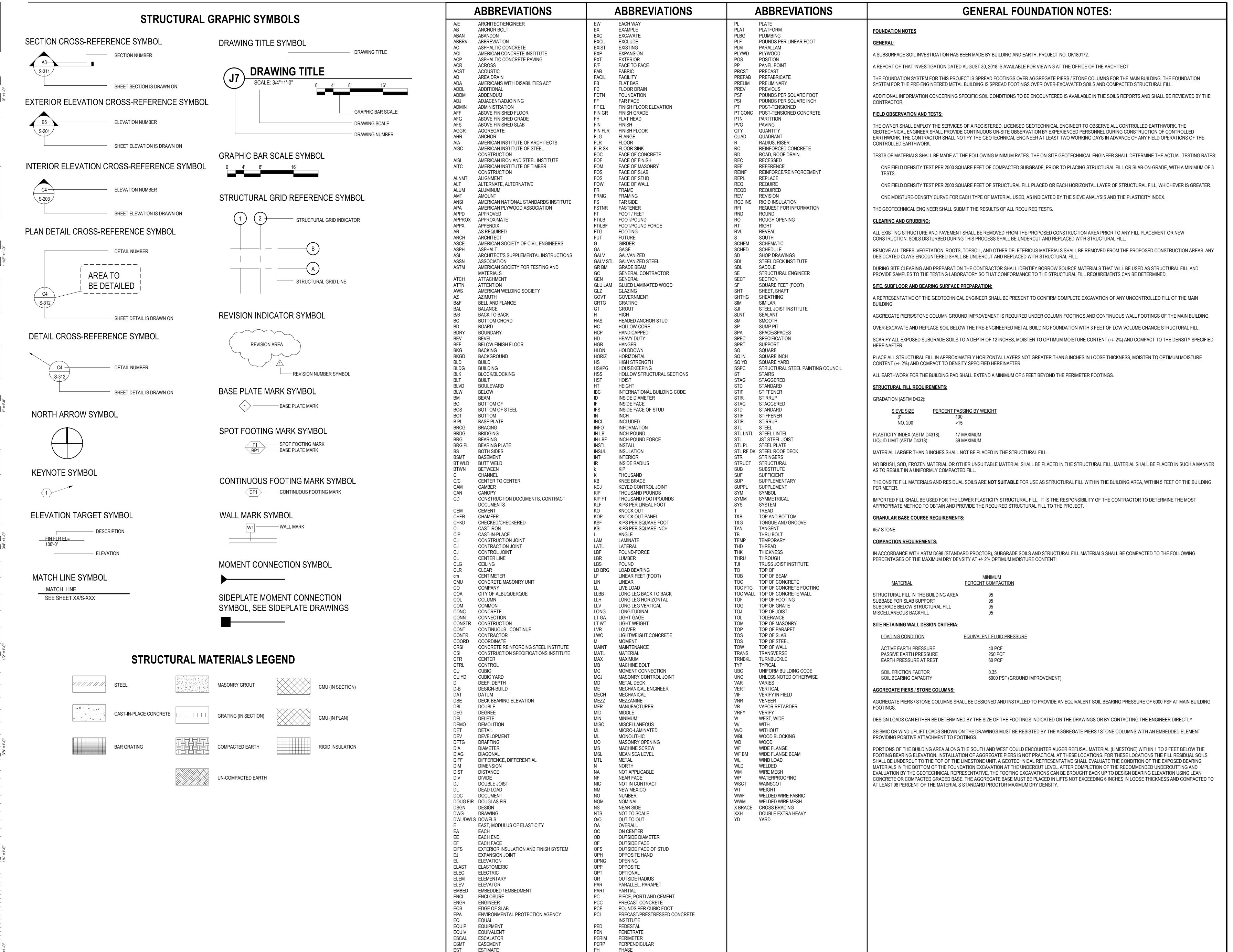


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OVERALL BUILDING DEMOLITION PLAN

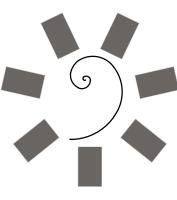


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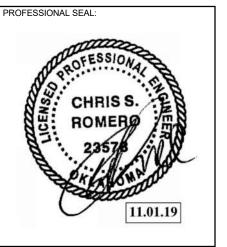
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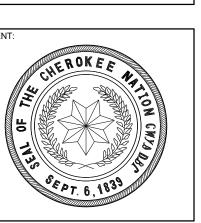


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ABBREVIATIONS AND LEGENDS

GENERAL STRUCTURAL NOTES

POST INSTALLED ANCHORS: THE STRUCTURAL DESIGN IS BASED ON THE POST INSTALLED ANCHORING SYSTEMS NOTED BELOW. SINCE ANCHOR CAPACITIES VARY BY MANUFACTURER. THE CONTRACTOR SHALL USE ONLY THE SYSTEMS NOTED BELOW UNLESS AN ALTERNATE IS APPROVED BY THE ENGINEER OF RECORD. ALTERNATE ANCHORING SYSTEMS MAY REQUIRE RE-DESIGN TO

ADDITIONAL CONSTRUCTION AND RE-DESIGN COSTS ASSOCIATED WITH THE ALTERNATE ANCHORING SYSTEM. ALL ADHESIVE (EPOXY) FOR POST INSTALLED ANCHORS AND/OR REBAR INTO CONCRETE SHALL BE HILTI HIT-RE 500 V3 OR HIT-HY 200 EPOXY ADHESIVE ANCHORING SYSTEM, HILTI HIT-RE 100 OR HIT-HY 200 EPOXY ADHESIVE SYSTEM OR

ALL ADHESIVE (EPOXY) FOR POST INSTALLED ANCHORS AND/OR REBAR INTO GROUT FILLED MASONRY SHALL BE HILTI HIT HY 70 ADHESIVE ANCHORING SYSTEM OR APPROVED EQUAL. INSTALLATION SHALL BE PER MANUFACTURER'S

RECOMMENDATIONS. ALL ADHESIVE (EPOXY) FOR POST INSTALLED ANCHORS AND/OR REBAR INTO HOLLOW MASONRY AND/OR BRICK SHALL BE

HILTI HIT HY 70 ADHESIVE ANCHORING SYSTEM OR APPROVED EQUAL. INSTALLATION SHALL BE PER MANUFACTURER'S RECOMMENDATIONS.

ALL POST INSTALLED MECHANICAL ANCHORS INTO CONCRETE SHALL BE HILTI KWIK BOLT TZ EXPANSION ANCHOR OR APPROVED EQUAL. INSTALLATION SHALL BE PER MANUFACTURER'S RECOMMENDATIONS.

ALL POST INSTALLED MECHANICAL ANCHORS INTO GROUT FILLED MASONRY SHALL BE HILTI KWIK BOLT 3 EXPANSION ANCHOR OR APPROVED EQUAL. INSTALLATION SHALL BE PER MANUFACTURER'S RECOMMENDATIONS.

ANCHOR LENGTHS SHOWN FOR ATTACHMENT TO CONCRETE AND/OR MASONRY ARE REQUIRED EMBEDMENT LENGTHS THE CONTRACTOR SHALL PROVIDE ANCHORS WITH ADDITIONAL LENGTH TO FACILITATE THE REQUIRED CONNECTION.

SUBMIT ALL PROPOSED ANCHORING SYSTEMS INCLUDING ICC-ES REPORTS TO STRUCTURAL ENGINEER FOR REVIEW PRIOR TO INSTALLATION. THE ICC-ES FORMS SHALL MEET THE REQUIREMENTS OF THE IBC REFERENCED IN THESE NOTES

STRUCTURAL AND MISCELLANEOUS STEEL

ALL STRUCTURAL STEEL SHALL BE DETAILED AND FABRICATED IN ACCORDANCE WITH THE AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS".

ALL WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A992, GRADE 50, UNLESS NOTED OTHERWISE.

ALL MISCELLANEOUS STEEL MEMBERS. SUCH AS CHANNELS. ANGLES. FLAT BARS. AND PLATES SHALL CONFORM TO ASTM A36 UNLESS NOTED OTHERWISE.

ALL RECTANGULAR AND SQUARE STRUCTURAL TUBING SHALL CONFORM TO ASTM A500, GRADE B, FY = 46 KSI OR ASTM 1085, GRADE B, FY = 50 KSI.

ALL ROUND STRUCTURAL TUBING SHALL CONFORM TO ASTM A500, GRADE B, FY = 42 KSI OR ASTM 1085, GRADE B, FY = 50

ALL STRUCTURAL PIPE SHALL CONFORM TO ASTM A53, TYPE E OR S, GRADE B, FY = 35 KSI.

BOLTS SHALL CONFORM TO ASTM A325N TENSION CONTROL BOLTS UNLESS NOTED OTHERWISE, WITH SIZES AS SHOWN ON THE DRAWINGS. WHERE CLEARANCE WITHIN A CONNECTION DOES NOT PERMIT THE USE OF TENSION CONTROL BOLTS, STANDARD A325N BOLTS SHALL BE USED AND INSPECTED IN ACCORDANCE WITH THE AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS".

ALL BOLTS SHALL BE INSTALLED IN A SNUG TIGHT CONDITION EXCEPT AT MOMENT CONNECTIONS. BRACED FRAME CONNECTIONS, AND AT CONNECTIONS DETAILED WITH A325SC BOLTS. AT THESE LOCATIONS, THE BOLTS SHALL BE TIGHTENED SO AS TO SHEAR THE SPLINE OFF THE BOLT.

ANCHOR BOLTS EMBEDDED IN CONCRETE SHALL BE ASTM F1554 GRADE 36 THREADED RODS WITH DOUBLE NUTS. PROVIDE FLAT WASHERS BETWEEN NUTS AND BASEPLATE SURFACES. ANCHOR BOLT LENGTHS SHOWN FOR ATTACHMENT TO CONCRETE AND/OR MASONRY ARE REQUIRED EMBEDMENT LENGTHS. THE CONTRACTOR SHALL PROVIDE ANCHOR BOLTS WITH ADDITIONAL BOLT LENGTH TO FACILITATE THE REQUIRED CONNECTION.

ANCHOR BOLT FLAT WASHERS SHALL BE PROVIDED IN ACCORDANCE WITH TABLE 14-2 OF AISC 360, AISC MANUAL OF STEEL CONSTRUCTION LATEST EDITION.

ALL WELDING SHALL BE DONE IN ACCORDANCE WITH THE LATEST STANDARDS OF THE AWS STRUCTURAL WELDING CODE

ALL BOLT HOLES THAT ARE REQUIRED TO BE FIELD DRILLED SHALL BE DRILLED WITH A MAG DRILL. FLAME CUTTING OF HOLES OR ENLARGING OF MISALIGNED HOLES WILL NOT BE ALLOWED. HEADED CONCRETE ANCHORS AND SHEAR CONNECTORS SHALL BE MADE FROM STEEL CONFORMING TO ASTM A108 AND MEET THE MECHANICAL PROPERTIES OF TYPE B, AS REQUIRED BY CHAPTER 7 OF AWS D1.1 "STRUCTURAL WELDING CODE-

STEEL", LATEST EDITION. STRUCTURAL STEEL TO RECEIVE SHEAR CONNECTORS SHALL BE FREE OF PAINT. WELDING PREQUALIFICATION REQUIRED. PROVIDE A SLIDE BEARING CONNECTION FOR STEEL BEAMS BEARING ON MASONRY WALLS UNLESS NOTED OTHERWISE.

SEE SHEET S7.41 FOR TYPICAL CONNECTION DETAIL SEE S7.00 SERIES SHEETS FOR TYPICAL STEEL DETAILS.

COMPOSITE FLOORS:

THE METAL DECK FOR COMPOSITE FLOORS SHALL BE UNSHORED UNLESS NOTED OTHERWISE

THE SHEAR CONNECTORS SHALL BE 3/4" DIAMETER X 4 1/2" AT 3" DEEP DECK UNLESS NOTED OTHERWISE. THE SHEAR CONNECTORS SHALL BE MADE FROM STEEL CONFORMING TO ASTM A108 AND MEET THE MECHANICAL PROPERTIES OF TYPE B, AS REQUIRED BY CHAPTER 7 OF AWS D1.1 "STRUCTURAL WELDING CODE STEEL", LATEST EDITION. STRUCTURAL STEEL TO RECEIVE SHEAR CONNECTIONS SHALL BE FREE OF PAINT. WELDING PREQUALIFICATION REQUIRED.

THE SHEAR CONNECTIONS SHALL NOT BE ADDED UNTIL THE METAL FLOOR DECK IS INSTALLED.

WHERE SHEAR CONNECTIONS AND PUDDLE WELDS COINCIDE, THE SHEAR CONNECTOR MAY REPLACE THE PUDDLE WELD.

CAMBERED BEAMS SHALL HAVE THE CAMBER PUT IN AT 1/3 POINTS OR ALONG A PARABOLIC CURVE.

THE CONTRACTOR SHALL SURVEY THE CAMBER OF THE BEAMS AFTER THE BEAMS HAVE BEEN ERECTED. THE CONTRACTOR SHALL SUBMIT THE SURVEY TO THE ENGINEER FOR REVIEW. THE CONTRACTOR SHALL NOT POUR THE SLAB UNTIL THE ENGINEER HAS REVIEWED AND APPROVED THE BEAM CAMBERS.

CONTRACTOR SHALL SHORE BEAMS WITH A CAMBER MORE THAN 1/2" LOWER THAN SPECIFIED. THE BEAM SHALL BE ALLOWED TO DEFLECT TO LEVEL.

THE CONCRETE FOR THE SLAB SHALL BE POURED AND PLACED TO THE ELEVATION INDICATED ON THE DRAWINGS WHILE MAINTAINING THE MINIMUM THICKNESS. SPREAD CONCRETE OVER AREA OF INFLUENCE TO ROUGH DEPTH IN ORDER TO LOAD BEAMS AND GIRDERS PRIOR TO SETTING SCREED ELEVATIONS.

THE WEIGHT OF THE WET CONCRETE WILL CAUSE DEFLECTIONS OF THE STEEL FRAMING. THEREFORE, CONCRETE OVERRRUNS ARE TO BE ANTICIPATED BY THE CONTRACTOR.

CONTRACTOR SHALL CONTINUOUSLY MONITOR THE THICKNESS AND ELEVATIONS DURING CONCRETE PLACING

PROVIDE #4 X 6'-0" AT 12" ON CENTER OVER ALL GIRDERS OF COMPOSITE FLOORS.

PROVIDE #4 X 6'-0" AT 12" ON CENTER OVER SHORED BEAMS THAT ARE NOT ALLOWED TO DEFLECT TO LEVEL.

PROVIDE WELDED WIRE FABRIC AS INDICATED ON DRAWINGS IN FLAT SHEETS ONLY. PROVIDE DECK CHAIRS FOR ALL WELDED WIRE FABRIC IN SLABS OVER METAL DECK.

STEEL JOISTS SHALL BE MANUFACTURED BY A MEMBER OF SJI.

FOR FUTURE CEILINGS ARE NOT REQUIRED.

STEEL JOISTS SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE STEEL JOIST INSTITUTE STANDARD SPECIFICATIONS.

NO CONSTRUCTION LOADS OF ANY KIND SHALL BE PLACED ON UNBRIDGED JOISTS.

WHERE COLUMNS ARE NOT FRAMED IN AT LEAST TWO DIRECTIONS WITH STRUCTURAL STEEL MEMBERS, JOISTS AT OR CLOSEST TO COLUMN LINES SHALL BE FIELD BOLTED TO ADD LATERAL STABILITY DURING CONSTRUCTION.

PROVIDE BRIDGING IN ACCORDANCE WITH THE LATEST EDITION OF THE SJI STANDARD SPECIFICATIONS AND OSHA REQUIREMENTS.

THE STRUCTURAL DRAWINGS ARE NOT STAND-ALONE DOCUMENTS. THE CONTRACTOR SHALL COORDINATE THE LOCATIONS AND WEIGHTS OF ALL MECHANICAL, PLUMBING, ELECTRICAL AND OTHER EQUIPMENT WITH THE APPLICABLE DRAWINGS. THE JOIST SUPPLIER SHALL ACCOUNT FOR THE LOADS IN THEIR DESIGN.

THE STEEL JOIST MANUFACTURER SHALL DESIGN ROOF JOISTS SUPPORTING MECHANICAL UNITS, INDICATED AS SP JOISTS ON THE DRAWINGS, FOR 1.2x MECHANICAL UNIT WEIGHTS SHOWN. USE 25 PSF DEAD LOAD AND 20 PSF LIVE LOAD (NON-REDUCIBLE) UNLESS NOTED OTHERWISE. CONTRACTOR SHALL VERIFY ACTUAL MECHANICAL LOADS. NOTIFY STEEL JOIST MANUFACTURER OF ANY DISCREPANCIES.

JOIST DEFLECTIONS SHALL BE LIMITED PER SJI REQUIREMENTS AND SHALL NOT BE LESS THAN THE FOLLOWING: L/360 FOR ROOF LIVE LOAD WITH STANDARD SJI CAMBER WHERE PLASTER OR STUCCO CEILINGS ARE SUPPORTED. L/240 FOR ROOF LIVE LOAD WITH STANDARD SJI CAMBER WHERE NONPLASTER CEILINGS ARE SUPPORTED. L/180 FOR ROOF LIVE LOAD WITH STANDARD SJI CAMBER WHERE NO CEILINGS ARE SUPPORTED AND PROVISIONS

STEEL ROOF JOISTS SHALL BE DESIGNED FOR A NET WIND UPLIFT LOAD OF 15 PSF UNLESS NOTED OTHERWISE. THE DEAD LOAD OF MISCELLANEOUS ROOFTOP ITEMS, INCLUDING SCREEN WALLS, SKYLIGHTS, FIRE SUPPRESSION SYSTEM, SOLAR PHOTOVOLTAIC SYSTEM, ETC... SHALL BE ACCOUNTED FOR IN THE DESIGN OF THE STEEL ROOF JOISTS. THE CONTRACTOR SHALL COORDINATE THE MISCELLANEOUS LOADS WITH THE STEEL JOIST MANUFACTURER.

GENERAL STRUCTURAL NOTES

JOIST BEARING SEATS SHALL BEAR ON STEEL SUPPORTS AND SHALL BE CONNECTED AS FOLLOWS UNLESS NOTED OTHERWISE: K/KCS/E: TWO 1/8" x 2 1/2" LONG FILLET WELDS

LH02-06 (OR 2 1/2" AND SMALLER TOP CHORD ANGLE LEG): TWO 3/16" x 2 1/2" LONG FILLET WELDS LH/DLH07-17 (OR 3 1/2" AND SMALLER TOP CHORD ANGLE LEG): TWO 1/4" x 2 1/2" LONG FILLET WELDS LH/DLH 18-25 (OR 4" AND LARGER TOP CHORD ANGLE LEG): TWO 1/4" x 4" LONG FILLET WELDS ALL WELDS SHALL MEET CURRENT MINIMUM SJI REQUIREMENTS

STEEL DECK:

ALL STEEL DECK SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE STEEL DECK INSTITUTE SPECIFICATIONS

SEE PLANS FOR STEEL DECK TYPE, GAGE, FINISH AND CONNECTIONS.

PROVIDE A MINIMUM OF 1 1/2" BEARING FOR ALL STEEL DECK.

ALL SPLICES AND LAPS SHALL BE A MINIMUM OF 2" IN LENGTH AND SHALL BE LOCATED DIRECTLY ABOVE SUPPORTS.

ALL DECKING SHALL BE CONTINUOUS OVER TWO OR MORE SPANS.

POWDER DRIVEN FASTENERS SHALL BE EQUIVALENT TO HILTI X-HSN 24 FOR STEEL BASE MATERIAL tf UP TO 3/8". HILTI ENP-19 FOR STEEL BASE MATERIAL tf 1/4" OR THICKER

MASONRY:

ALL MASONRY UNITS SHALL COMPLY WITH ASTM C 90 WITH A COMPRESSIVE STRENGTH OF 2000 PSI (NET AREA).

F'M = 1900 PSI

MORTAR SHALL BE TYPE S.

GROUT - F'C = 2000 PSI, MINIMUM.

CELLS CONTAINING REBAR SHALL BE GROUTED SOLID FROM THE BOTTOM TO THE TOP OF THE WALL IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE.

ALL CELLS BELOW GRADE SHALL BE GROUTED SOLID UP TO GRADE.

CELLS CONTAINING EXPANSION ANCHORS SHALL BE GROUTED SOLID.

ALL VERTICAL REBAR SHALL BE IN PLACE AND SECURED WITH REBAR POSITIONERS PRIOR TO GROUTING.

COVER FOR REINFORCING SHALL BE AS FOLLOWS UNLESS OTHERWISE NOTED:

MASONRY FACE NOT EXPOSED TO EARTH OR WEATHER: 1 1/2" B. MASONRY FACE EXPOSED TO EARTH OR WEATHER

> 1. BARS LARGER THAN NO. 5: 2" 2. BARS NO. 5 OR SMALLER: 1 1/2"

UNLESS OTHERWISE NOTED MASONRY CELLS SHALL BE GROUTED IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE (MAXIMUM 5 FOOT GROUT LIFTS).

LAP REBAR PER THE SCHEDULE ON \$6.01

WHERE REBAR LAP SPLICES EXCEED 5 FT GROUT LIFTS, 8 FT GROUT LIFTS MAY BE USED WITH CLEANOUTS PROVIDED AT THE BOTTOM OF EACH VERTICALLY REINFORCED CELL. SOLID GROUTED WALLS SHALL HAVE CLEANOUTS AT 32" ON CENTER MAXIMUM.

ALL HORIZONTAL REINFORCING IN BOND BEAMS SHALL BE CONTINUOUS AROUND CORNERS OR HAVE BENT (CORNER) BARS OF THE SAME SIZE AND A LAP AS NOTED ABOVE. VERTICAL STEEL SHALL CONTINUE THROUGH BOND BEAMS.

PROVIDE STANDARD LADDER TYPE JOINT REINFORCING AT 16" ON CENTER (ALTERNATE COURSES) UNLESS NOTED OTHERWISE IN THE PROJECT DOCUMENTS. USE PREFABRICATED CORNERS AND TEES AT ALL WALL CORNERS AND INTERSECTIONS RESPECTIVELY.

PROVIDE A SLIDE BEARING CONNECTION FOR STEEL BEAMS BEARING ON MASONRY WALLS UNLESS NOTED OTHERWISE. SEE SHEET S-741 FOR TYPICAL CONNECTION DETAIL.

SEE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR INFORMATION REGARDING MASONRY COLORS, FINISHES, BOND, ETC. AT ALL EXPOSED

ALL MASONRY WALL CONFIGURATIONS INCLUDING WALL OPENINGS SHALL BE COORDINATED WITH CIVIL, MECHANICAL, PLUMBING, ELECTRICAL AND DRAWINGS FROM ALL OTHER DISCIPLINES.

EXPOSED MASONRY SITE WALLS AND RETAINING WALLS GREATER THAN 16 FEET IN LENGTH SHALL HAVE MASONRY CONTROL JOINTS INSTALLED AT

THE FOLLOWING MINIMUM SPACING:

12'-0" ON CENTER FOR WALLS 6'-0" MAXIMUM HEIGHT 18'-0" ON CENTER FOR WALLS 10'-0" MAXIMUM HEIGHT

20'-0" ON CENTER FOR WALLS GREATER THAN 10'-0" IN HEIGHT

SEE S7.00 SERIES SHEETS FOR TYPICAL MASONRY DETAILS

FOR CMU OR BRICK VENEER (5" MAXIMUM, 3" MINIMUM THICKNESS) ATTACHMENT TO STRUCTURAL MASONRY, PROVIDE ADJUSTABLE INTEGRAL ANCHOR TIES. ADJUSTABLE INTEGRAL ANCHOR TIES SHALL BE CORROSION RESISTANT AND HAVE TWO PINTLE LEGS MINIMUM WITH W2.8 (3/16") WIRE OR APPROVED EQUAL. PROVIDE DUR-O-WALL DA370 ADJUSTABLE INTEGRAL ANCHOR TIES OR APPOVED EQUAL.

FOR CMU OR BRICK VENEER (5" MAXIMUM, 3" MINIMUM THICKNESS) ATTACHMENT TO STRUCTURAL CONCRETE, PROVIDE ADJUSTABLE ANCHOR TIES. ADJUSTABLE ANCHOR TIES SHALL BE CORROSION RESISTANT AND HAVE A TWO PINTLE LEGS MINIMUM WITH A MINIMUM W2.8 (3/16") WIRE. ATTACH TO CONCRETE WITH 2-1/4" DIAMETER CONCRETE SCREWS, HILTI KWIKCON 11 x 1 1/2" OR APPROVED EQUAL.

FOR CMU OR BRICK VENEER (5" MAXIMUM, 3" MINIMUM THICKNESS) ATTACHMENT TO STRUCTURAL COLD FORMED METAL STUDS, PROVIDE ADJUSTABLE ANCHOR TIES. ADJUSTABLE ANCHOR TIES SHALL BE CORROSION RESISTANT AND HAVE TWO PINTLE LEGS MINIMUM W2.8 (3/16") WIRE. PROVIDE DUR-O-WALL DA213 ADJUSTABLE ANCHOR TIE OR APPROVED EQUAL. ATTACH THROUGH SHEATHING TO STUDS WITH 2-1/4" x 1 1/2" CORROSION RESISTANT TEK SCREWS.

SEE TYPICAL DETAILS ON SHEET S7.31 FOR VENEER TIE SPACING.

PROVIDE ADDITIONAL ANCHORS AROUND ALL OPENINGS LARGER THAN 16" IN EITHER DIMENSION. SPACE ANCHORS WITHIN 12" OF OPENING PERIMETER AND MATCH HORIZONTAL OR VERTICAL ANCHOR TIE SPACING.

COORDINATE VENEER LOCATION, TYPE, BOND PATTERN, ETC. WITH ARCHITECTURAL DRAWINGS

PRE-ENGINEERED METAL BUILDING:

FOUNDATION CONFIGURATION AND SIZES SHOWN ON THESE DRAWINGS ARE BASED ON PRELIMINARY DESIGN CALCULATIONS. THESE SIZES MAY REQUIRE MODIFICATIONS PER THE METAL BUILDING MANUFACTURER'S FINAL GRAVITY AND LATERAL DESIGN CALCULATIONS.

THE METAL BUILDING MANUFACTURER SHALL PROVIDE FINAL GRAVITY AND LATERAL DESIGN CALCULATIONS FOR APPROVAL PRIOR TO THE COMMENCEMENT OF FOUNDATION EARTHWORK.

THE BUILDING SHALL BE A MANUFACTURER'S STANDARD PREFABRICATED METAL STRUCTURE OF THE APPROXIMATE INSIDE AREA SHOWN, EXCEPT AS NOTED. RIGID FRAMES SHALL BE SPACED AS SPECIFIED ON THE DRAWINGS, BUT OVERALL DIMENSIONS AND CONSTRUCTION DETAILS MAY VARY TO SUIT MANUFACTURER'S STANDARD DESIGN. MINIMUM WEB THICKNESS OF RIGID FRAMES SHALL BE 3/16".

THE BUILDING SHALL BE DESIGNED AND FABRICATED ACCORDING TO AISC, MBMA AND AISI SPECIFICATIONS. THE DIMENSIONAL TOLERANCES APPLICABLE TO ROLLED FORM STEEL UNDER THE LATEST EDITION OF THE AISC "STANDARD MILL PRACTICE" SECTION SHALL BE REQUIRED IN THE FABRICATION OF THE STEEL BUILDING FRAMES.

THE BUILDING FRAME SHALL BE DESIGNED TO LIMIT THE LATERAL DEFLECTION TO H/240 INCH AT THE BUILDING EAVE FOR THE SPECIFIED BASIC WIND

THE BUILDING SHALL BE DESIGNED TO SUPPORT ALL MECHANICAL EQUIPMENT INCLUDING HEATERS, SPRINKLERS, EXHAUST SYSTEMS AND ALL OTHER DEVICES. ADDITIONAL GIRTS OR PURLINS SHALL BE PLACED IN CONVENIENT LOCATIONS FOR ATTACHMENT OF ALL MECHANICAL EQUIPMENT. THE CONTRACTOR SHALL COORDINATE THE MECHANICAL LOADS WITH THE METAL BUILDING MANUFACTURER AND THE MECHANICAL DRAWINGS.

DESIGN LOADS SHALL CONFORM WITH THESE GENERAL NOTES. LOAD COMBINATIONS SHALL COMPLY WITH MBMA SPECIFICATIONS.

ANCHOR BOLTS SHOWN ON THESE DRAWINGS ARE BASED ON PRELIMINARY DESIGN CALCULATIONS. THESE SIZES MAY REQUIRE MODIFICATIONS PER THE METAL BUILDING MANUFACTURER'S FINAL GRAVITY AND LATERAL DESIGN CALCULATIONS.

THE METAL BUILDING MANUFACTURER SHALL DESIGN THE SUPPORTS FOR ALL CONNECTIONS OF MASONRY AND/OR METAL STUD WALLS TO THE METAL BUILDING COMPONENTS AND PROVIDE CALCULATIONS FOR THE DESIGN OF THE SUPPORTS

PREPARE THE SHOP DRAWINGS AND CALCULATIONS UNDER THE SEAL OF A PROFESSIONAL STRUCTURAL ENGINEER REGISTERED IN THE STATE THE PROJECT IS LOCATED.

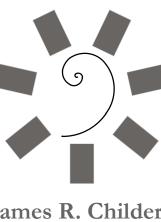
COORDINATE WITH ARCHITECTURAL DRAWINGS.

GLASS CURTAIN WALL SYSTEM:

ALL LATERAL AND GRAVITY SUPPORT FOR THE GLASS CURTAIN WALL SYSTEM SHALL BE PER THE MANUFACTURER'S RECOMMENDATIONS. SHOP DRAWINGS AND STAMPED CALCULATIONS SHALL BE SUBMITTED FOR APPROVAL BY THE ENGINEER OF RECORD AND THE ARCHITECT PRIOR TO INSTALLATION.

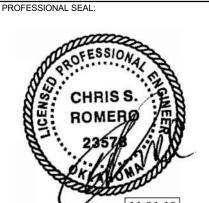
THE ENGINEER STAMPING THE SHOP DRAWINGS SHALL BE REGISTERED IN THE STATE THAT THE PROJECT IS LOCATED.

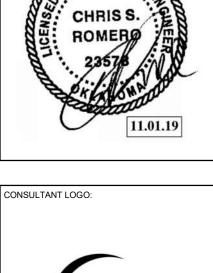
THE BEAMS AT ALL FLOORS HAVE BEEN DESIGNED TO SUPPORT THE GRAVITY LOAD OF THE GLASS CURTAIN WALL SYSTEM. THE GLASS CURTAIN WALL SYSTEM SHALL BE LATERALLY SUPPORTED AT ALL FLOORS AND ROOF LEVEL.



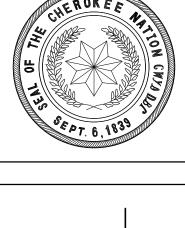
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S0.02

GENERAL STRUCTURAL NOTES

GENERAL STRUCTURAL NOTES ELEVATORS: THE STRUCTURE HAS BEEN DESIGNED FOR A KONE ELEVATOR. LOCATIONS AND DIMENSIONS. ADDITIONAL CONSTRUCTION COST AND DESIGN COST. THE ELEVATOR MANUFACTURER. OF ELEVATOR SHOP DRAWINGS. **SPECIAL INSPECTION:** SPECIAL INSPECTION ITEMS NOTED ON SHEET S0.03. **DEFERRED SUBMITTALS:** SPECIAL STEEL JOISTS METAL STAIRS EXTERIOR COLD-FORMED METAL FRAMING INTERIOR COLD-FORMED METAL FRAMING TEMPORARY SHORING HANDRAILS CURTAIN WALL AND STOREFRONT AGGREGATE PIERS / STONE COLUMN GROUND IMPROVEMENT PRECAST CONCRETE COLUMNS AND ASSOCIATED ATTACHMENTS AND ANCHORAGE

COMPONENTS AND CLADDING WIND PRESSURES (PSF CALCULATED AT MEAN ROOF HEIGHT = [] FEET

5 PARAPETS 121.5 100.9 85.1

ZONE 1 ZONE 4

ZONE 2 ZONE 5

ZONE 3

COMPONENT AND CLADDING WIND LOADING DIAGRAM

96.3 79.9 67.4 62.3 51.7 43.6

SCHEDULE OF STRUCTURAL SPECIAL INSPECTIONS

ALL STRUCTURAL SUPPORTS, FLOOR PENETRATION SIZES AND PIT DIMENSIONS HAVE BEEN DESIGNED BASED ON THE ABOVE INFORMATION. SHOULD THE ACTUAL ELEVATOR(S) SELECTED DIFFER FROM THE INFORMATION PROVIDED ABOVE, THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY ADDITIONAL CONSTRUCTION AND REDESIGN COSTS ASSOCIATED WITH THE ALTERNATE ELEVATOR(S).

THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING ALL ELEVATOR PIT AND FLOOR PENETRATION

THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING ALL ELEVATOR OVERRUN REQUIREMENTS.

ELEVATOR CAR RAIL AND COUNTERWEIGHT RAIL SUPPORTS SHALL BE PROVIDED BY THE CONTRACTOR. CAR RAIL AND COUNTERWEIGHT RAIL SUPPORTS SHALL BE PROVIDED AT AND BETWEEN ALL FLOOR LEVELS SERVICED BY THE ELEVATOR, ABOVE THE LAST STOP OF THE ELEVATOR, AND BETWEEN THE BASEMENT AND THE GROUND FLOOR AS REQUIRED BY THE ELEVATOR MANUFACTURER. IF THE ELEVATOR MANUFACTURER REQUIRES RAIL SUPPORTS THAT DIFFER FROM THOSE PROVIDED, THE ELEVATOR MANUFACTURER SHALL BE RESPONSIBLE FOR

THE CONTRACTOR SHALL VERIFY THE DESIGN OF THE HOIST/SAFETY BEAM AND CONNECTIONS AS REQUIRED PER

STRUCTURAL ELEMENTS AFFECTED BY THE ELEVATOR LAYOUT SHALL NOT BE FABRICATED PRIOR TO APPROVAL

THE OWNER SHALL PROVIDE FOR SERVICES OF A CERTIFIED INSPECTOR (APPROVED BY THE BUILDING OFFICIAL OR THE ENGINEER OF RECORD) IN ACCORDANCE WITH CHAPTER 17 OF THE INTERNATIONAL BUILDING CODE FOR THE

THE DEFERRED SUBMITTALS LISTED BELOW ARE THOSE PORTIONS OF THE DESIGN THAT ARE NOT COMPLETED AT THE TIME OF APPLICATION AND ARE TO BE SUBMITTED TO THE BUILDING OFFICIAL AND APPROVED PRIOR TO THE INSTALLATION OF THOSE ITEMS. THE MANUFACTURER, CONSULTANT, OR CONTRACTOR, AS APPROPRIATE, SHALL PROVIDE SUBMITTALS TO THE ENGINEER OF RECORD FOR REVIEW FOR THE FOLLOWING ITEMS:

1. SPECIAL INSPECTIONS / TESTING - "SPECIAL STRUCTURAL INSPECTION" SHALL NOT RELIEVE THE OWNER OR THEIR AGENT FROM HAVING THE INSPECTIONS OF THE JURISDICTION BUILDING DEPARTMENT PER SECTION 110 OF THE IBC PERFORMED. BOTH THE JURISDICTION BUILDING DEPARTMENT INSPECTIONS AND "SPECIAL STRUCTURAL INSPECTION" SHALL BE PERFORMED.

2. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR NOTIFYING THE JURISDICTION BUILDING OFFICIAL AND SPECIAL INSPECTOR WHEN WORK IS READY FOR INSPECTION.

3. REPORTING FOR SPECIAL INSPECTION - SPECIAL INSPECTION AND TESTING REPORTS SHALL BE COMPLETED AND DISTRIBUTED AT THE COMPLETION OF EACH TASK. IF A TASK IS TO TAKE LONGER THAN THREE (3) DAYS, PROVIDE REPORTS FOR EACH DAY. PROVIDE COPIES OF REPORTS TO CONTRACTOR, OWNER, ARCHITECT AND STRUCTURAL ENGINEER OF RECORD. SPECIAL INSPECTOR TO KEEP A NON-COMPLIANCE LIST DOCUMENTING ITEMS INSPECTED NOT MEETING APPROVED CONSTRUCTION DOCUMENTS AND WHEN / HOW RESOLVED.

4. SEE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING CONSTRUCTION DOCUMENTS FOR ADDITIONAL NON-STRUCTURAL SPECIAL INSPECTION ITEMS.

5. SPECIAL INSPECTION OF SHOP FABRICATED MEMBERS AND ASSEMBLIES SHALL BE IN ACCORDANCE WITH SECTION 1704.2, UNLESS FABRICATOR IS APPROVED TO PERFORM WORK WITHOUT SPECIAL INSPECTION.

6. IN ACCORDANCE WITH IBC CHAPTER 17, THE OWNER OR THE OWNER'S AGENT, OTHER THAN THE CONTRACTOR, SHALL EMPLOY ONE OR MORE APPROVED AGENCIES TO PROVIDE SPECIAL INSPECTIONS AND TESTS, DURING CONSTRUCTION FOR THE TYPES OF WORK LISTED BELOW THESE SPECIAL INSPECTIONS AND TESTS ARE IN ADDITION TO THE INSPECTIONS BY THE BUILDING OFFICIAL IDENTIFIED IN IBC SECTION 110

7. DEFINITIONS:

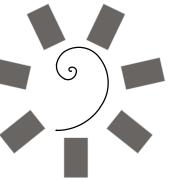
* SPECIAL INSPECTION: INSPECTION AS HEREIN REQUIRED BY A QUALIFIED SPECIAL INSPECTOR COMPETENT WITH THE MATERIALS, INSTALLATION, FABRICATION, ERECTION OR PLACEMENT OF COMPONENTS AND CONNECTIONS REQUIRING SPECIAL EXPERTISE TO ENSURE COMPLIANCE WITH APPROVED CONSTRUCTION DOCUMENTS AND REFERENCED STANDARDS (SEE SECTION 1704).

* CONTINUOUS SPECIAL INSPECTION: FULL-TIME OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED. * PERIODIC SPECIAL INSPECTION: THE PART-TIME OR INTERMITTENT OBSERVATION OF WORK REQUIRING SPECIAL

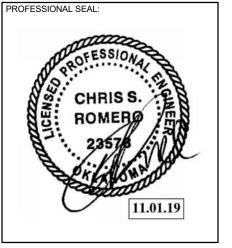
INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK HAS BEEN OR IS BEING PERFORMED AND AT THE COMPLETION OF THE WORK.

ITEM	DESCRIPTION OF REQUIREMENTS	REQUIRED (YES/NO)
SPECIAL INSPECTION OF STRUCTURAL STEEL	TO BE PERFORMED IN ACCORDANCE WITH CHAPTER N OF AISC 360-10	YES
SPECIAL INSPECTION AND VERIFICATION OF STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.2	YES
SPECIAL INSPECTIONS AND VERIFICATIONS FOR CONCRETE CONSTRUCTION	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.3	YES
SPECIAL INSPECTIONS AND VERIFICATIONS FOR MASONRY CONSTRUCTION	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.4 AND REFERENCED STANDARDS	YES
SPECIAL INSPECTIONS AND VERIFICATIONS FOR WOOD CONSTRUCTION	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.5	NO
SPECIAL INSPECTIONS AND VERIFICATIONS OF SOILS	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.6, THE GEOTECHNICAL REPORT LISTED IN THE GENERAL FOUNDATION NOTES, AND ANY OTHER REQUIREMENTS LISTED IN THE GENERAL FOUNDATION NOTES	YES
SPECIAL INSPECTIONS AND VERIFICATIONS FOR DEEP FOUNDATIONS (DRIVEN PILES, CAST-IN-PLACE, OR HELICAL PILES AS APPLICABLE)	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTIONS 1705.7-1705.9 AS APPLICABLE, THE GEOTECHNICAL REPORT LISTED IN THE GENERAL FOUNDATION NOTES, AND ANY OTHER REQUIREMENTS LISTED IN THE CONSTRUCTION DOCUMENTS	NO
SPECIAL INSPECTIONS FOR WIND RESISTANCE (REQUIRED ONLY FOR Vult= 155MPH OR GREATER IN EXPOSURE CATEGORY B, OR Vult=142MPH OR GREATER IN EXPOSURE CATEGORY C OR D)	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.11	NO
SPECIAL INSPECTIONS AND VERIFICATIONS FOR SEISMIC RESISTANCE (REQUIRED FOR STRUCTURES ASSIGNED TO CATEGORIES C, D, E, OR F)	TO BE PERFORMED IN ACCORDANCE WITH ALL APPLICABLE PORTIONS OF IBC SECTIONS 1705.12 AND 1705.13	NO

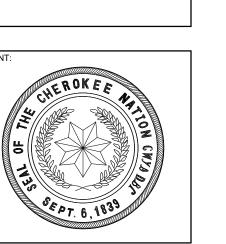
ADDITIONAL INSPECTIONS REQUIRED PER SIDEPLATE SYSTEMS ON SHEET S8.01



James R. Childers Architect, Inc. 45 South 4th Street Fort Smith, AR 72901 479-783-2480 www.childersarchitect.com







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		REVISIONS
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GENERAL STRUCTURAL NOTES AND SPECIAL INSPECTIONS

GENERAL STRUCTURAL NOTES

STRUCTURAL DRAWINGS ARE NOT STAND-ALONE DOCUMENTS AND ARE INTENDED TO BE USED IN CONJUNCTION WITH CIVIL. ARCHITECTURAL. MECHANICAL. ELECTRICAL. AND DRAWINGS FROM OTHER DISCIPLINES. THE CONTRACTOR SHALL COORDINATE ALL REQUIREMENTS OF THE CONTRACT DOCUMENTS INTO THE SHOP DRAWINGS AND FIELD WORK.

COORDINATE DIMENSIONS OF ALL OPENINGS, DEPRESSIONS, BLOCKOUTS, ETC. WITH ARCHITECTURAL DRAWINGS, DRAWINGS FROM OTHER DISCIPLINES, PROJECT SHOP DRAWINGS, AND FIELD CONDITIONS PRIOR TO SHOP DRAWING SUBMITTAL. THE STRUCTURAL DRAWINGS ONLY REPRESENT A PORTION OF THE REQUIREMENTS FOR THE PROJECT.

SEE ARCHITECTURAL PLANS FOR INTERIOR NON-BEARING PARTITION WALLS, PARTITION FRAMING SHALL BE CONNECTED TO THE PRIMARY STRUCTURE TO ALLOW FOR VERTICAL LIVE LOAD DEFLECTIONS OF SPAN/360 FOR FLOOR FRAMING AND SPAN/240 FOR ROOF FRAMING.

CONTRACTOR'S CONSTRUCTION AND/OR ERECTION SEQUENCES SHALL RECOGNIZE AND CONSIDER THE EFFECTS OF THERMAL MOVEMENTS OF STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PERIOD.

THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS IN THE FIELD.

SHOP DRAWINGS SHALL BE FURNISHED AND REVIEWED BEFORE ANY FABRICATION OR ERECTION IS STARTED. THE CONTRACTOR SHALL REVIEW AND APPROVE SHOP DRAWINGS PRIOR TO SUBMITTAL TO THE ARCHITECT FOR REVIEW. POORLY EXECUTED SHOP DRAWINGS WILL BE REJECTED AND SHALL BE RESUBMITTED.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING SAFE AND ADEQUATE SHORING FOR ALL PARTS OF THE STRUCTURE DURING CONSTRUCTION.

TEMPORARY PROVISIONS SHALL BE MADE FOR STRUCTURAL STABILITY DURING CONSTRUCTION. THE STRUCTURE SHOWN ON THE DRAWINGS HAS BEEN DESIGNED FOR STABILITY UNDER FINAL CONFIGURATION.

NOTCHING OR CUTTING ANY STRUCTURAL MEMBER IN THE FIELD IS PROHIBITED.

THE CONTRACTOR SHALL VERIFY THE SIZE AND LOCATION OF FOUNDATIONS UNDER MECHANICAL AND ELECTRICAL EQUIPMENT AS REQUIRED. NO CONCRETE PADS SHALL BE LOCATED ON ROOF UNLESS SHOWN ON THE STRUCTURAL

BACKFILL SHALL NOT BE PLACED BEHIND RETAINING WALLS UNTIL CONCRETE HAS ATTAINED 100 PERCENT OF DESIGN

BACKFILL SHALL NOT BE PLACED BEHIND BASEMENT WALLS UNTIL THE CONCRETE HAS ATTAINED 100 PERCENT OF DESIGN STRENGTH AND THE ELEVATED FLOOR PROVIDING LATERAL SUPPORT AT THE TOP OF THE WALL IS COMPLETELY CONSTRUCTED, OR TEMPORARY BRACING/SHORING OF THE WALL IS PROVIDED. DESIGN OF ANY TEMPORARY WALL BRACING/SHORING IS THE RESPONSIBILITY OF THE CONTRACTOR.

REMOVAL OF FORMS AND SHORING SHALL BE IN ACCORDANCE WITH ACI 347. WHERE CONCRETE MUST SUPPORT SUPERIMPOSED LOADS PRIOR TO ATTAINING THE SPECIFIED DESIGN STRENGTH, RESHORE CONCRETE IN ACCORDANCE WITH ACI 347. RESHORING SHALL NOT BE REMOVED SOONER THAN 28 DAYS FROM THE DATE OF POUR OR UNTIL CONCRETE HAS ATTAINED THE SPECIFIED DESIGN STRENGTH.

THE CONTRACTOR SHALL SUBMIT FOR PRIOR APPROVAL THE END OF POUR LOCATIONS FOR CONCRETE GRADE BEAMS, CONCRETE COLUMNS, AND CONCRETE BEAMS.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR ADHERING TO ALL APPLICABLE STANDARDS SET FORTH BY OSHA, INCLUDING THE FOLLOWING REQUIREMENTS FROM STANDARDS - 29 CFR, SECTION 1926,

- THE STEEL ERECTION CONTRACTOR SHALL NOT ERECT STEEL UNLESS THEY HAVE RECEIVED WRITTEN NOTIFICATION FROM THE CONTRACTOR THAT THE CONCRETE IN THE FOOTINGS, PIERS AND WALLS OR THE MORTAR IN THE MASONRY PIERS AND WALLS HAS ATTAINED, ON THE BASIS OF AN APPROPRIATE ASTM STANDARD TEST METHOD OF FIELD-CURED SAMPLES, EITHER 75 PERCENT OF THE INTENDED MINIMUM COMPRESSIVE DESIGN STRENGTH OR SUFFICIENT STRENGTH TO SUPPORT THE LOADS IMPOSED DURING STEEL ERECTION. PROVIDE STRUCTURAL ENGINEER A COPY OF WRITTEN NOTIFICATION WHEN IT IS PROVIDED TO THE STEEL ERECTOR.
- B. ANCHOR RODS (ANCHOR BOLTS) SHALL NOT BE REPAIRED, REPLACED OR FIELD-MODIFIED WITHOUT THE APPROVAL OF THE PROJECT STRUCTURAL ENGINEER OF RECORD.
- PRIOR TO ERECTION OF COLUMNS, THE CONTRACTOR SHALL PROVIDE WRITTEN NOTIFICATION TO THE STEEL ERECTOR IF THERE HAS BEEN ANY REPAIR, REPLACEMENT OR MODIFICATION OF THE ANCHOR RODS (ANCHOR

PROVIDE STRUCTURAL ENGINEER A COPY OF WRITTEN NOTIFICATION WHEN IT IS PROVIDED TO THE STEEL

- C. NO MODIFICATION THAT AFFECTS THE STRENGTH OF A STEEL JOIST OR STEEL JOIST GIRDER SHALL BE MADE WITHOUT THE APPROVAL OF THE PROJECT STRUCTURAL ENGINEER OF RECORD.
- D. METAL DECKING HOLES AND OPENINGS SHALL NOT BE CUT UNTIL IMMEDIATELY PRIOR TO BEING PERMANENTLY FILLED WITH THE EQUIPMENT OR STRUCTURE, OR SHALL BE IMMEDIATELY COVERED.

PROTECTION: PROPER PRECAUTIONS SHALL BE TAKEN AT ALL TIMES TO PROTECT VEHICULAR AND PEDESTRIAN TRAFFIC FROM ANY DAMAGE OR INJURY WHICH MAY BE CAUSED, EITHER DIRECTLY OR INDIRECTLY, BY THE WORK INCLUDED ON THESE DRAWINGS. SUCH PRECAUTIONS SHALL INCLUDE THE ERECTION AND MAINTENANCE OF FENCES, BARRICADES, RAILINGS, GUARDS, SIGNS, COVERINGS, LIGHTS, AND OTHER PRECAUTIONS AS MAY BE REQUIRED. IF AT ANY TIME, IN THE OPINION OF THE OWNER OR THE OWNER'S REPRESENTATIVE, PROPER PRECAUTIONS ARE NOT BEING TAKEN TO SECURE THIS PROTECTION, THE CONTRACTOR SHALL AT NO ADDITIONAL COST TO THE OWNER, INSTALL AND

POLLUTION CONTROLS: USE WATER SPRINKLING, TEMPORARY ENCLOSURES, AND OTHER SUITABLE METHODS TO LIMIT DUST AND DIRT RISING AND SCATTERING IN THE AIR TO LOWEST PRACTICAL LEVEL. COMPLY WITH GOVERNING REGULATIONS PERTAINING TO ENVIRONMENTAL PROTECTION.

DO NOT SCALE DRAWINGS.

WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL NOTES, AND SPECIFICATIONS, THE MORE STRINGENT REQUIREMENTS SHALL GOVERN. DETAILS ON DRAWINGS TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS. DETAILS NOTED "TYPICAL" APPLY TO ALL SIMILAR CONDITIONS. WHERE NO SPECIFIC DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ELSEWHERE ON THE PROJECT.

NOTCHING OR CUTTING ANY STRUCTURAL MEMBER IN THE FIELD IS PROHIBITED, UNLESS DETAILED OTHERWISE ON THE STRUCTURAL PLANS.

CONTRACTOR SHALL BE RESPONSIBLE FOR ADHERING TO ALL APPLICABLE STANDARDS SET FORTH BY OSHA.

PRIOR TO STARTING DEMOLITION WORK, THE CONTRACTOR SHALL MAKE AN INSPECTION OF ALL SURROUNDING IMPROVEMENTS TO REMAIN, TO DETERMINE AND RECORD THEIR EXISTING PHYSICAL CONDITION.

SHORING AND BRACING: THE CONTRACTOR SHALL FURNISH ALL SHORING, BRACING, AND INCIDENTALS NECESSARY AND REQUIRED FOR THE PROPER SUPPORT AND SAFETY OF ALL MEMBERS AFFECTED BY DEMOLITION WORK.

WHERE DEMOLITION WOULD AFFECT THE STRUCTURAL INTEGRITY OF THE REMAINING STRUCTURE. THE CONTRACTOR SHALL PROVIDE TEMPORARY SUPPORTS. THE CONTRACTOR SHALL NOTIFY THE ENGINEER OF ANY FIELD CONDITION WHICH WOULD PRESENT A HAZARDOUS CONDITION TO THE STRUCTURE BEFORE PROCEEDING.

PROTECTION: PROPER PRECAUTIONS SHALL BE TAKEN AT ALL TIMES TO PROTECT VEHICULAR AND PEDESTRIAN TRAFFIC FROM ANY DAMAGE OR INJURY WHICH MAY BE CAUSED, EITHER DIRECTLY OR INDIRECTLY, BY THE WORK INCLUDED ON THESE DRAWINGS. SUCH PRECAUTIONS SHALL INCLUDE THE ERECTION AND MAINTENANCE OF FENCES, BARRICADES, RAILINGS, GUARDS, SIGNS, COVERINGS, LIGHTS, AND OTHER PRECAUTIONS AS MAY BE REQUIRED. IF AT ANY TIME, IN THE OPINION OF THE OWNER OR THE OWNER'S REPRESENTATIVE, PROPER PRECAUTIONS ARE NOT BEING TAKEN TO SECURE THIS PROTECTION, THE CONTRACTOR SHALL AT NO ADDITIONAL COST TO THE OWNER, INSTALL AND MAINTAIN SUCH ADDITIONAL PROTECTION AS MAY BE DIRECTED BY THE OWNER.

POLLUTION CONTROLS: USE WATER SPRINKLING, TEMPORARY ENCLOSURES, AND OTHER SUITABLE METHODS TO LIMIT DUST AND DIRT RISING AND SCATTERING IN THE AIR TO LOWEST PRACTICAL LEVEL. COMPLY WITH GOVERNING REGULATIONS PERTAINING TO ENVIRONMENTAL PROTECTION.

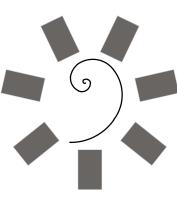
REMOVE DEBRIS FROM THE SITE AS IT ACCUMULATES. UNLESS OTHERWISE NOTED, DO NOT STORE, SELL, BURN, OR OTHERWISE DISPOSE OF DEBRIS ON THE SITE. REMOVAL OF DEBRIS INCLUDES CLEARING OF ALL LOWER LEVELS AND SIMILAR BELOW GRADE STRUCTURES. REMOVE ALL DEBRIS IN SUCH A MANNER AS TO PREVENT SPILLAGE. KEEP ALL PAVEMENTS AND AREAS ADJACENT TO THE SITE CLEAN AND FREE FROM MUD, DIRT, AND DEBRIS AT ALL TIMES.

USE OF EXPLOSIVES: THE CONTRACTOR IS ABSOLUTELY PROHIBITED FROM USING DYNAMITE OR ANY OTHER EXPLOSIVES IN ANY OF THE WORK OR OPERATIONS SHOWN ON THESE PLANS AT THE PROJECT SITE.

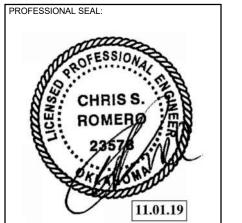
DEMOLITION SHALL BE PERFORMED IN A MANNER THAT WILL NOT DAMAGE ADJOINING SURFACES INDICATED TO REMAIN. SURFACES SHALL BE PATCHED, IF REQUIRED, TO PROVIDE A SUITABLE SUBSTRATE FOR NEW CONSTRUCTION.

SPECIFIC DEMOLITION NOTES ARE NOT TO BE CONSIDERED ALL INCLUSIVE OR COMPLETE IN THEMSELVES. CONTRACTOR SHALL PROVIDE ALL DEMOLITION INCIDENTAL TO OR REQUIRED FOR CONSTRUCTION WHETHER SPECIFICALLY NOTED OR NOT.

STRUCTURAL DEMOLITION DRAWINGS SHOW STRUCTURAL DEMOLITION ONLY. SEE ARCHITECTURAL DRAWINGS FOR DEMOLITION OF EXISTING ARCHITECTURAL ELEMENTS.

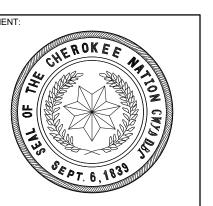


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CONSULTANT LOGO:





PROJECT PHASE: **BID PACKAGE 01**

		REVISIONS
#	DATE	DESCRIPTION

11-01-19 18-01.01 SHEET NUMBER:

DEMOLITION GENERAL STRUCTURAL NOTES

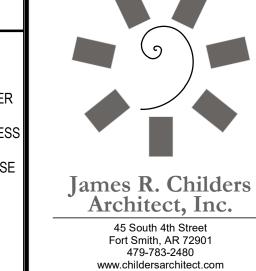
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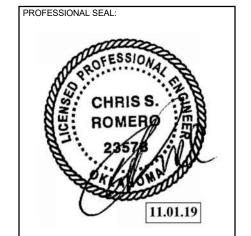
GENERAL SHEET NOTES

EXISTING CONSTRUCTION IS PER AVAILIBLE EXISTING
 DRAWINGS. ALL EXITING CONSTRUCTION AND DIMENSIONS
 SHALL BE VERIFIED PRIOR TO CONSTRUCTION. SHOULD
 CONDITIONS VARY FROM THOSE SHOWN, CONTRACT ENGINEER
 BEFORE PROCEEDING.
 NO STRUCTURAL MEMBERS SHALL BE CUT OR DEMOVED LINESS.

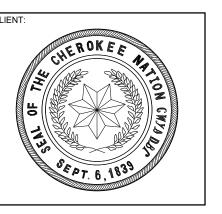
BEFORE PROCEEDING.

NO STRUCTURAL MEMBERS SHALL BE CUT OR REMOVED UNLESS SPECIFICALLY INDICATED ON THESE DRAWINGS. NOTIFY ARCHITECT AND ENGINEER IF CONDITIONS DIFFER FROM THOSE SHOWN HERE.









CENTER

MANKILLER HEALTH
EXPANSION
STILWELL, OKLAHOMA

KEY PLAN:

PROJECT PHASE:
BID PACKAGE 01

REVISIONS
DATE DESCRIPTION

DATE: JOB NUMBER:

11-01-19 18-01.01

SHEET NUMBER:

SD1.01

DEMOLITION PLANS -SECTOR 1

NOTE: CONTRACTOR TO CONFIRM WITH ARCH, PLUMBING AND CIVIL BEFORE

STRATA FOR THE NEW STRUCTURAL FOUNDATIONS.

EXISTING CONTINUOUS

EXISTING CONTINUOUS

WALL FOOTING AND

STEM WALL TO BE

REMOVED IN THIER

ENTIRETY

EXISTING CONTINUOUS

EXISTING CONTINUOUS

WALL FOOTING AND

STEM WALL TO BE REMOVED IN THIER

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WALL FOOTING AND

STEM WALL TO BE

ENTIRETY

REMOVED IN THIER

EXISTING STRUCTURE, COLUMNS, FOOTINGS,

SLAB, ETC. TO BE REMOVED IN THIER ENTIRETY. SEE ARCH FOR EXTENTS OF

STRUCTURE TO BE DEMOLISHED

EXISTING FOOTING

EXISTING FOOTING

- AND COLUMN TO

EXISTING FOOTING AND COLUMN TO REMAIN

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SEE SD0.01 FOR GENERAL DEMOLITION NOTES.

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AND COLUMN TO

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EXISTING FOOTING AND COLUMN TO

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THIER ENTIRETY

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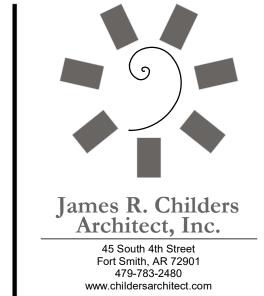
EXISTING FOOTING

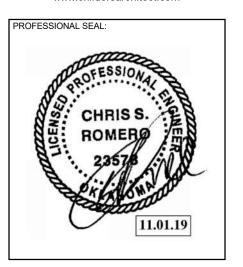
AND COLUMN TO BE REMOVED IN THIER ENTIRETY

AND COLUMN TO BE REMOVED IN bf1

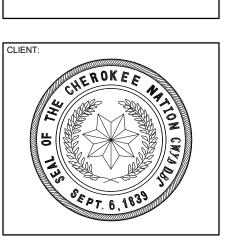
REMOVAL OF UNDERGROUND PLUMBING AND UTILITIES. ANY PIPING OR OTHER UNDERGROUND CONDUIT TO BE ABANDONED SHALL BE CONFIRMED WITH THE

GEOTECHNICAL ENGINEER FOR REQUIREMENTS TO MEET THE SPECIFIED BEARING









HEALTH CENTER ION

WILMA P. MANKILLER HEAL
EXPANSION

KEY PLAN:

PROJECT PHASE:

BID PACKAGE 01

REVISIONS

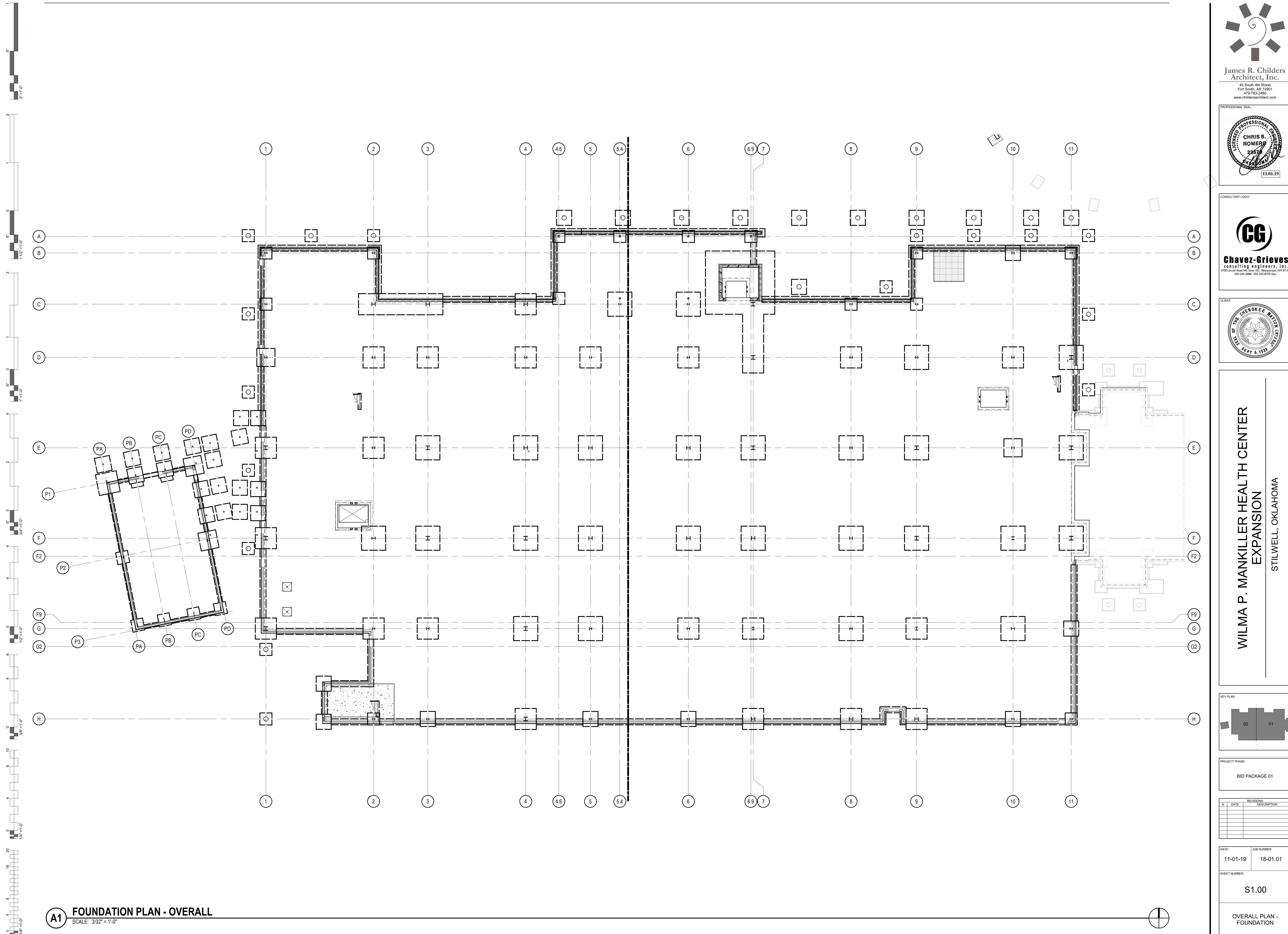
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DATE: JOB NUMBER: 11-01-19 18-01 01

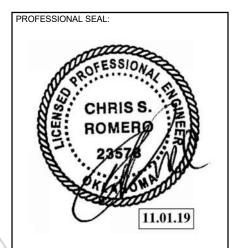
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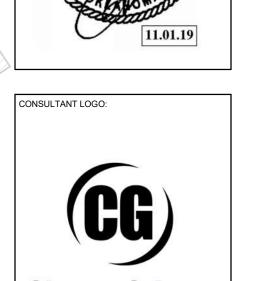
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DEMOLITION SECTIONS

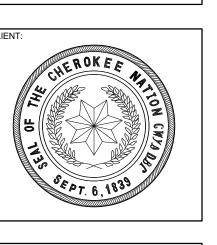


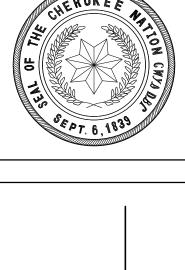


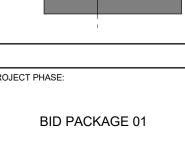




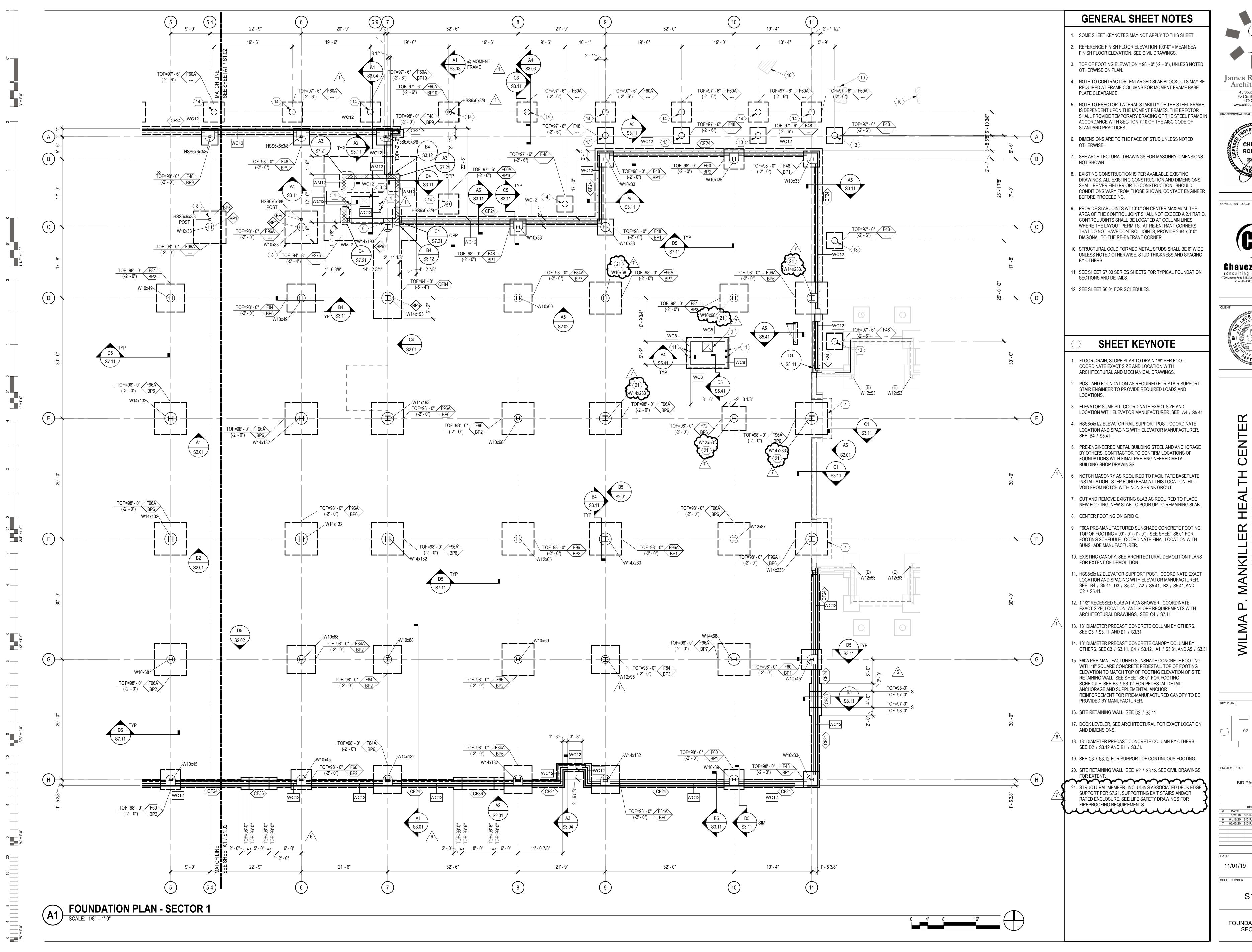


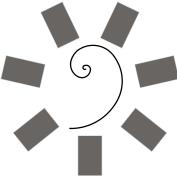




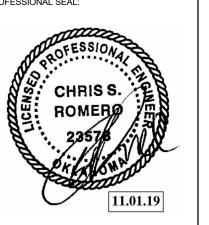


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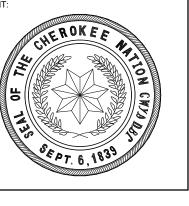




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ANKILLER HEAL EXPANSION

BID PACKAGE 01

REVISIONS

DATE DESCRIPTION

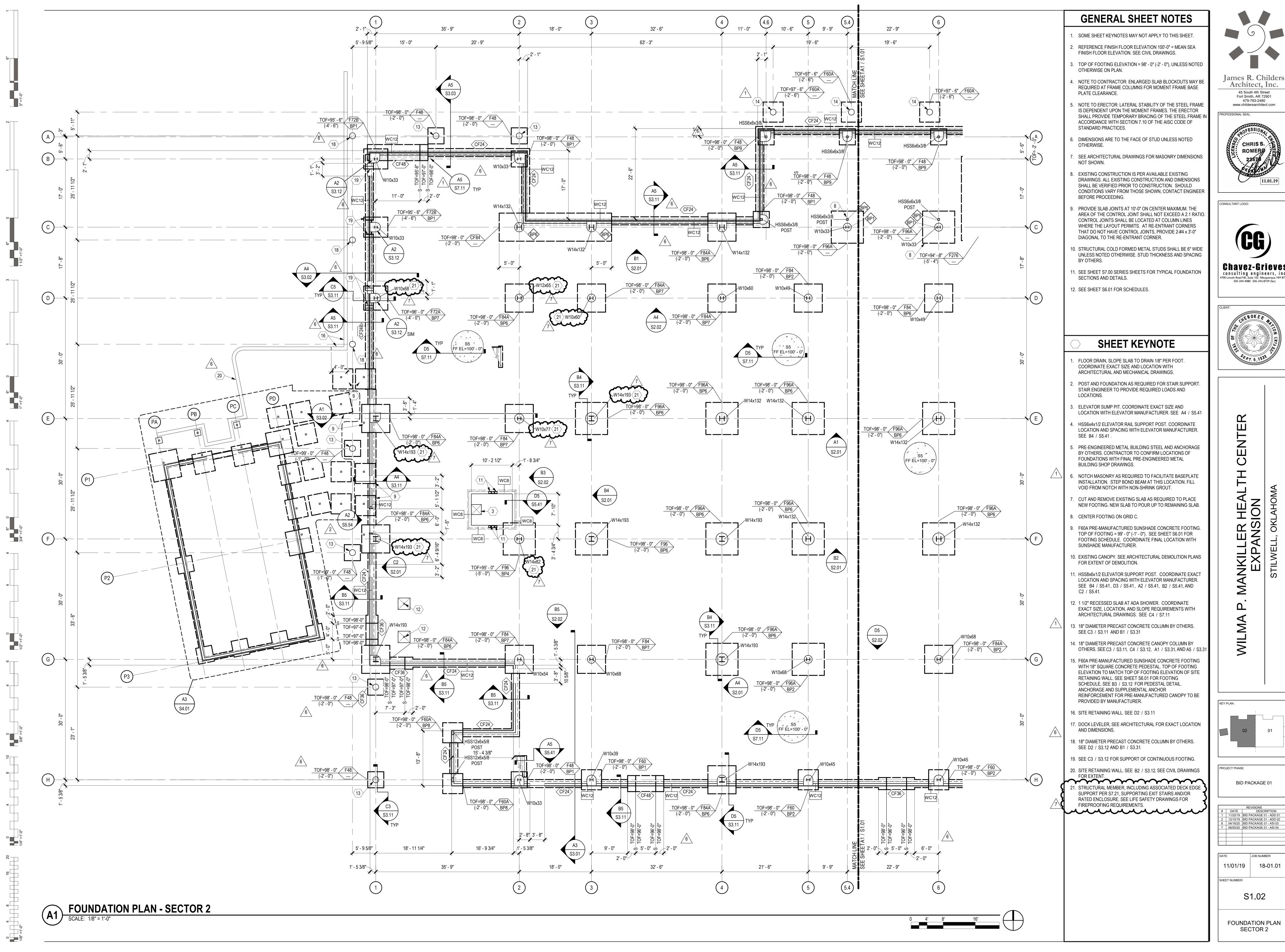
1 11/22/19 BID PACKAGE 01 - ADD 01

0 04/16/20 BID PACKAGE 01 - ASI 03 06/05/20 BID PACKAGE 01 - ASI 04

11/01/19 18-01.01

S1.01

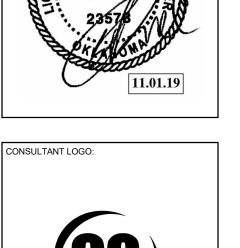
FOUNDATION PLAN SECTOR 1



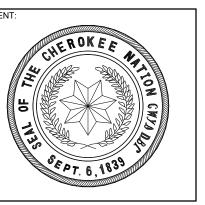


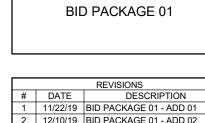
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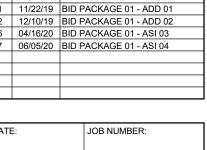








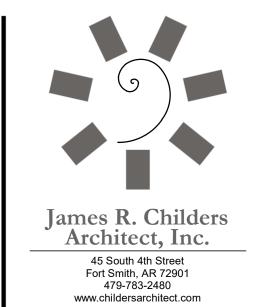


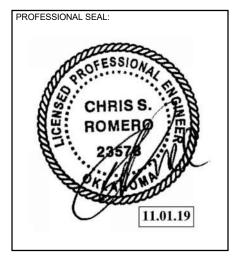


18-01.01 11/01/19

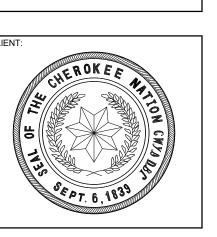
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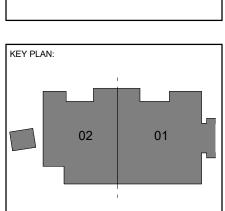






CENTER

A P. MANKILLER HEALTH C EXPANSION



PROJECT PHASE:

BID PACKAGE 01

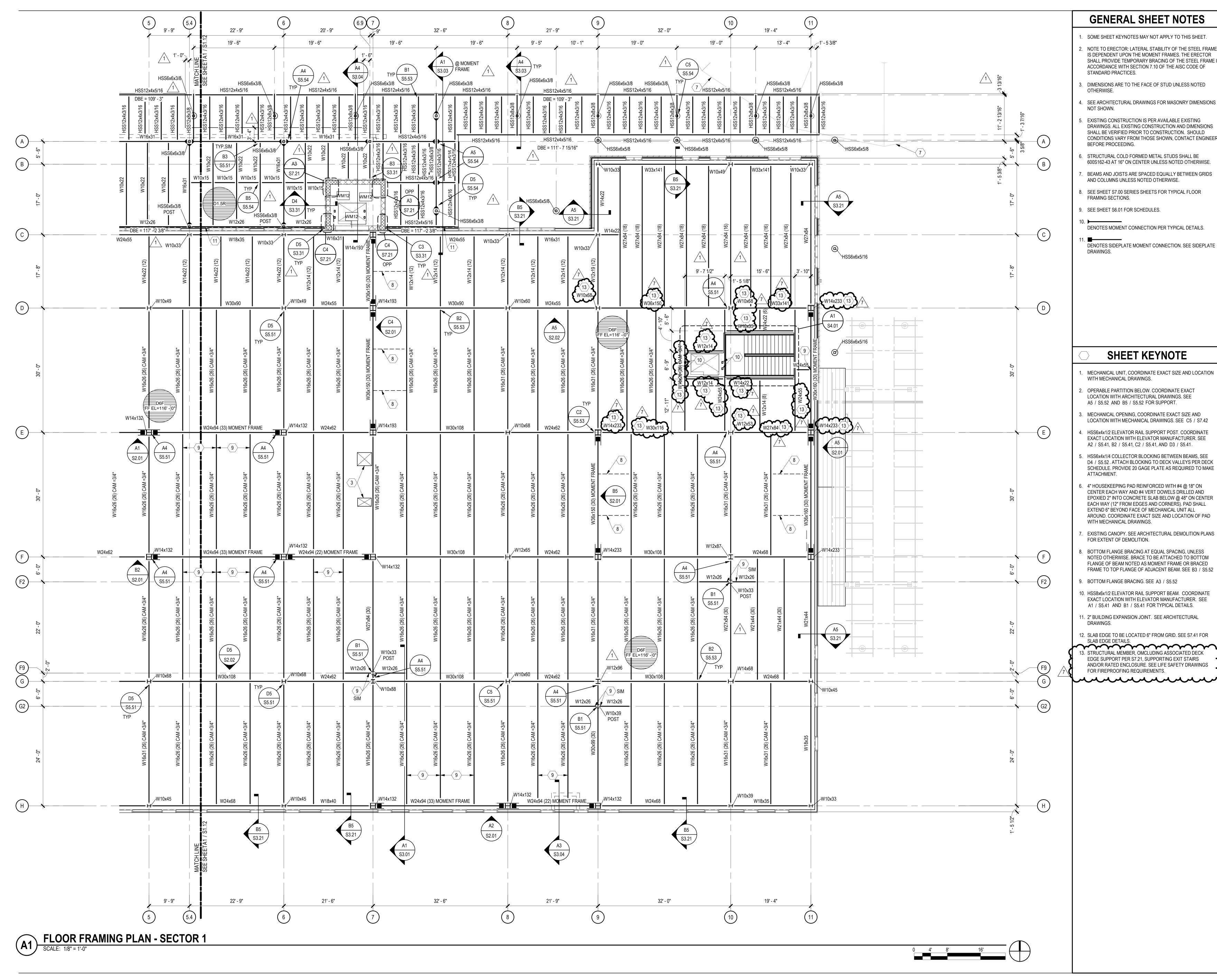
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DATE DESCRIPTION

DATE: JOB NUMBER: 11-01-19 18-01.01

SHEET NUMBER:

S1.10

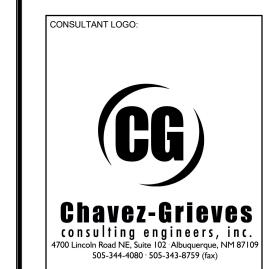
OVERALL PLAN - FLOOR FRAMING



GENERAL SHEET NOTES

- NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE MOMENT FRAMES. THE ERECTOR
- - SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS
 - EXISTING CONSTRUCTION IS PER AVAILABLE EXISTING DRAWINGS. ALL EXISTING CONSTRUCTION AND DIMENSIONS SHALL BE VERIFIED PRIOR TO CONSTRUCTION. SHOULD CONDITIONS VARY FROM THOSE SHOWN, CONTACT ENGINEER
 - 600S162-43 AT 16" ON CENTER UNLESS NOTED OTHERWISE.
 - BEAMS AND JOISTS ARE SPACED EQUALLY BETWEEN GRIDS
 - SEE SHEET S7.00 SERIES SHEETS FOR TYPICAL FLOOR

 - DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
- DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE



James R. Childers

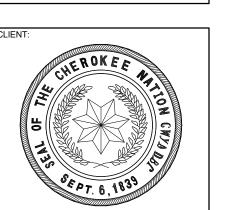
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- MECHANICAL UNIT, COORDINATE EXACT SIZE AND LOCATION
- OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE
- MECHANICAL OPENING, COORDINATE EXACT SIZE AND
- HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST. COORDINATE
- HSS6x4x1/4 COLLECTOR BLOCKING BETWEEN BEAMS, SEE
- D4 / S5.52. ATTACH BLOCKING TO DECK VALLEYS PER DECK SCHEDULE. PROVIDE 20 GAGE PLATE AS REQUIRED TO MAKE
- CENTER EACH WAY AND #4 VERT DOWELS DRILLED AND EPOXIED 2" INTO CONCRETE SLAB BELOW @ 48" ON CENTER EACH WAY (12" FROM EDGES AND CORNERS). PAD SHALL EXTEND 6" BEYOND FACE OF MECHANICAL UNIT ALL AROUND. COORDINATE EXACT SIZE AND LOCATION OF PAD
- EXISTING CANOPY. SEE ARCHITECTURAL DEMOLITION PLANS
- NOTED OTHERWISE. BRACE TO BE ATTACHED TO BOTTOM FLANGE OF BEAM NOTED AS MOMENT FRAME OR BRACED FRAME TO TOP FLANGE OF ADJACENT BEAM. SEE B3 / S5.52
- 10. HSS8x6x1/2 ELEVATOR RAIL SUPPORT BEAM. COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE
- 12. SLAB EDGE TO BE LOCATED 6" FROM GRID. SEE S7.41 FOR
- EDGE SUPPORT PER S7.21, SUPPORTING EXIT STAIRS AND/OR RATED ENCLOSURE. SEE LIFE SAFETY DRAWINGS

02

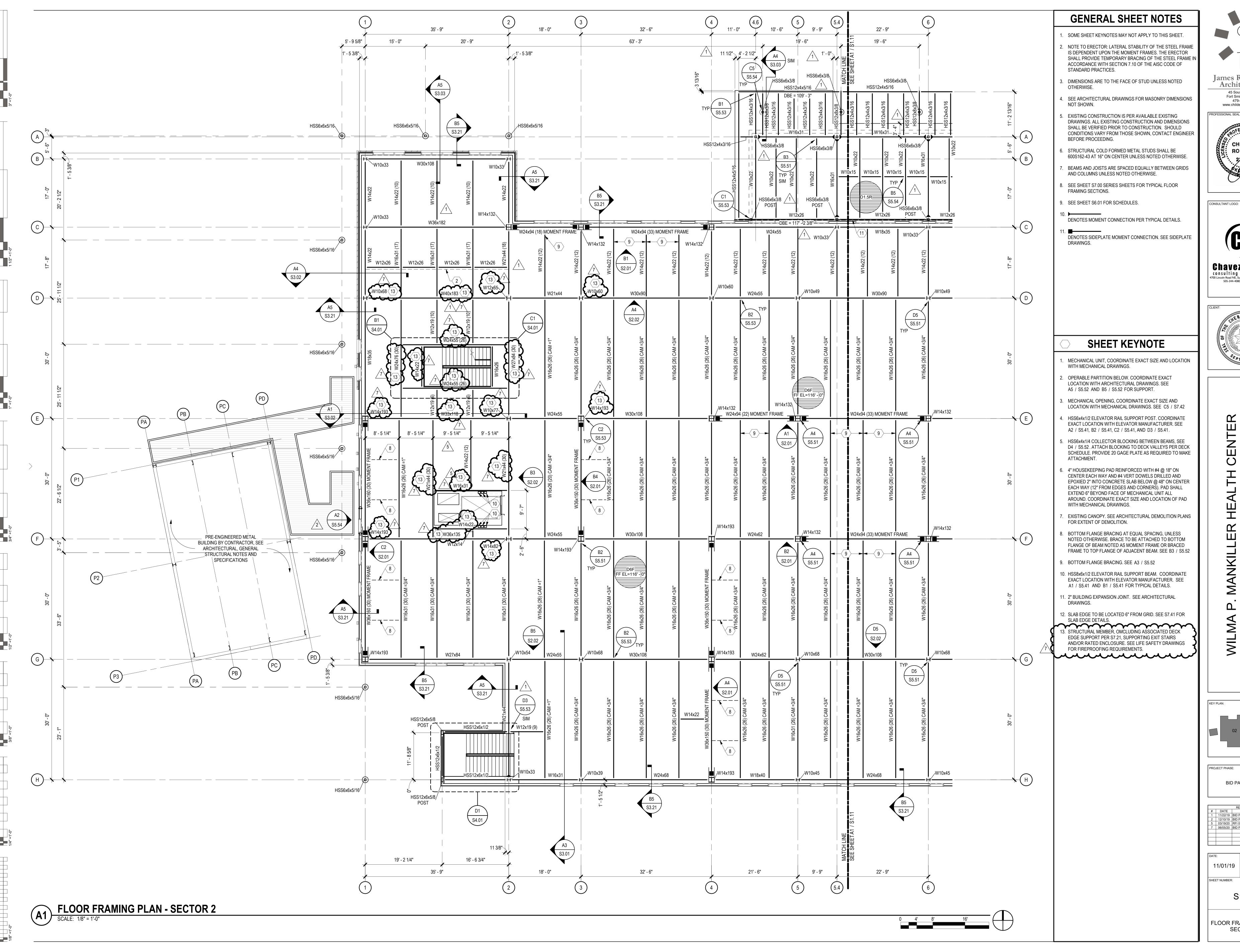
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| REVISIONS | | DATE | DESCRIPTION | 1 | 11/22/19 | BID PACKAGE 01 - ADD 01 | 7 | 06/05/20 | BID PACKAGE 01 - ASI 04 |

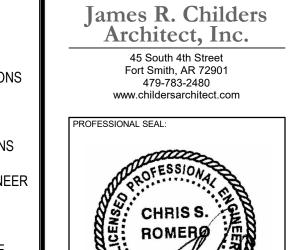
11/01/19

S1.11

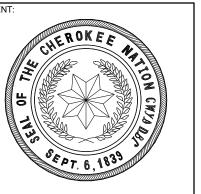
FLOOR FRAMING PLAN SECTOR 1

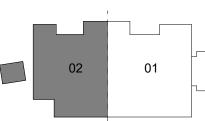












BID PACKAGE 01

DATE DESCRIPTION
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2 12/10/19 BID PACKAGE 01 - ADD 02 7 06/05/20 BID PACKAGE 01 - ASI 04

11/01/19 18-01.01

S1.12

FLOOR FRAMING PLAN SECTOR 2

LOW ROOF FRAMING PLAN

GENERAL SHEET NOTES

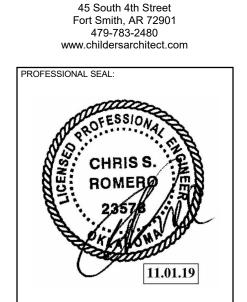
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- NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME
 IS DEPENDENT UPON THE MOMENT FRAMES. THE ERECTOR
 SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN
- 3. DIMENSIONS ARE TO THE FACE OF STUD UNLESS NOTED OTHERWISE.

STANDARD PRACTICES.

4. SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.

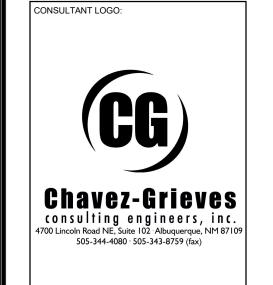
ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF

- 5. EXISTING CONSTRUCTION IS PER AVAILABLE EXISTING DRAWINGS. ALL EXISTING CONSTRUCTION AND DIMENSIONS SHALL BE VERIFIED PRIOR TO CONSTRUCTION. SHOULD CONDITIONS VARY FROM THOSE SHOWN, CONTACT ENGINEER BEFORE PROCEEDING.
- 6. STRUCTURAL COLD FORMED METAL STUDS SHALL BE 600S162-43 AT 16" ON CENTER UNLESS NOTED OTHERWISE.
- 7. BEAMS AND JOISTS ARE SPACED EQUALLY BETWEEN GRIDS AND COLUMNS UNLESS NOTED OTHERWISE.
- 8. SEE SHEET S7.00 SERIES SHEETS FOR TYPICAL FLOOR FRAMING SECTIONS.
- 9. SEE SHEET S6.01 FOR SCHEDULES.
- DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
- 11. DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE DRAWINGS.



James R. Childers

Architect, Inc.





SHEET KEYNOTE

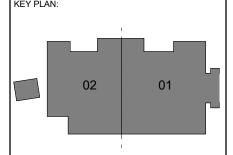
- . MECHANICAL UNIT, COORDINATE EXACT SIZE AND LOCATION WITH MECHANICAL DRAWINGS.
- 2. OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A5 / S5.52 AND B5 / S5.52 FOR SUPPORT.
- MECHANICAL OPENING, COORDINATE EXACT SIZE AND LOCATION WITH MECHANICAL DRAWINGS. SEE C5 / S7.42
- 4. HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST. COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE

A2 / S5.41, B2 / S5.41, C2 / S5.41, AND D3 / S5.41.

- HSS6x4x1/4 COLLECTOR BLOCKING BETWEEN BEAMS, SEE D4 / S5.52. ATTACH BLOCKING TO DECK VALLEYS PER DECK SCHEDULE. PROVIDE 20 GAGE PLATE AS REQUIRED TO MAKE ATTACHMENT.
- 6. 4" HOUSEKEEPING PAD REINFORCED WITH #4 @ 18" ON CENTER EACH WAY AND #4 VERT DOWELS DRILLED AND EPOXIED 2" INTO CONCRETE SLAB BELOW @ 48" ON CENTER EACH WAY (12" FROM EDGES AND CORNERS). PAD SHALL EXTEND 6" BEYOND FACE OF MECHANICAL UNIT ALL AROUND. COORDINATE EXACT SIZE AND LOCATION OF PAD WITH MECHANICAL DRAWINGS.
- EXISTING CANOPY. SEE ARCHITECTURAL DEMOLITION PLANS FOR EXTENT OF DEMOLITION.
- BOTTOM FLANGE BRACING AT EQUAL SPACING, UNLESS NOTED OTHERWISE. BRACE TO BE ATTACHED TO BOTTOM FLANGE OF BEAM NOTED AS MOMENT FRAME OR BRACED FRAME TO TOP FLANGE OF ADJACENT BEAM. SEE B3 / S5.52
- 9. BOTTOM FLANGE BRACING. SEE A3 / S5.52
- 10. HSS8x6x1/2 ELEVATOR RAIL SUPPORT BEAM. COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE A1 / S5.41 AND B1 / S5.41 FOR TYPICAL DETAILS.
- 11. 2" BUILDING EXPANSION JOINT. SEE ARCHITECTURAL DRAWINGS.
- 12. SLAB EDGE TO BE LOCATED 6" FROM GRID. SEE S7.41 FOR
- SLAB EDGE DETAILS.

 13. STRUCTURAL MEMBER, OMCLUDING ASSOCIATED DECK EDGE SUPPORT PER S7.21, SUPPORTING EXIT STAIRS AND/OR RATED ENCLOSURE. SEE LIFE SAFETY DRAWINGS

mmmm



PROJECT PHASE:

REVISIONS
DATE DESCRIPTION
1 11/22/19 BID PACKAGE 01 - ADD 01
2 12/10/19 BID PACKAGE 01 - ADD 02
4 02/05/20 BID PACKAGE 01 - ASI 02
5 03/19/20 RFI 005
7 06/05/20 BID PACKAGE 01 - ASI 04

BID PACKAGE 01

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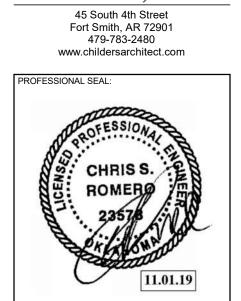
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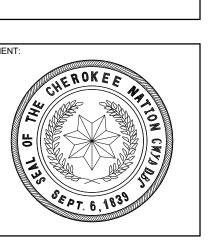
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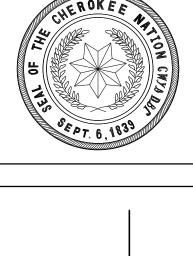
LOW ROOF FRAMING PLAN



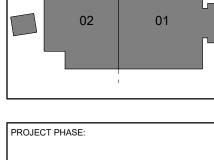








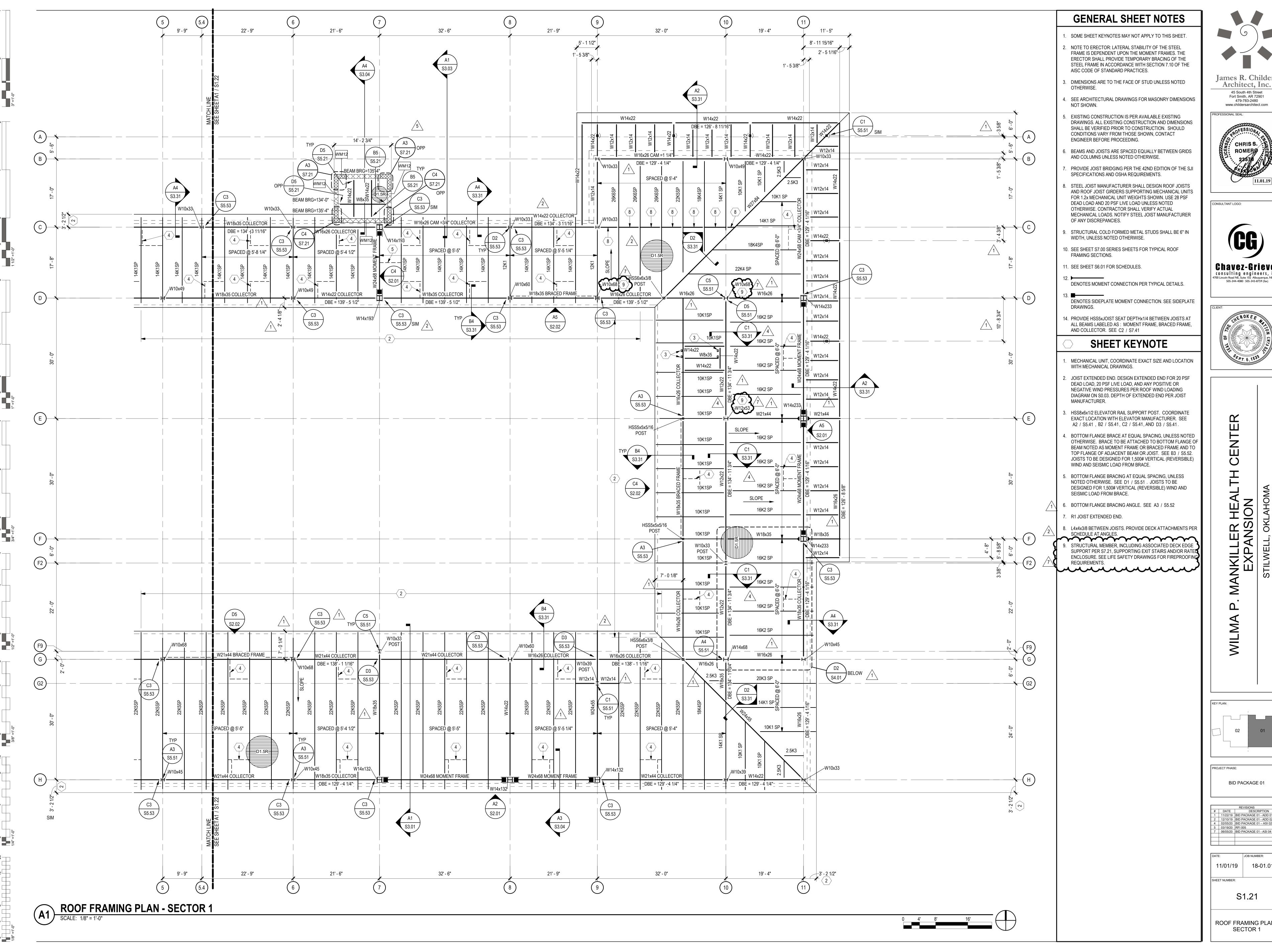
AANKILLER HEALT EXPANSION



BID PACKAGE 01

11-01-19 18-01.01

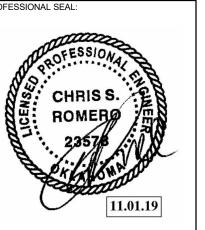
S1.20 OVERALL PLAN - ROOF FRAMING



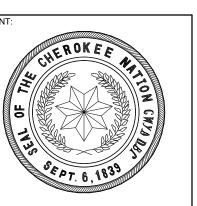


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MANKILLER HEAL EXPANSION

02

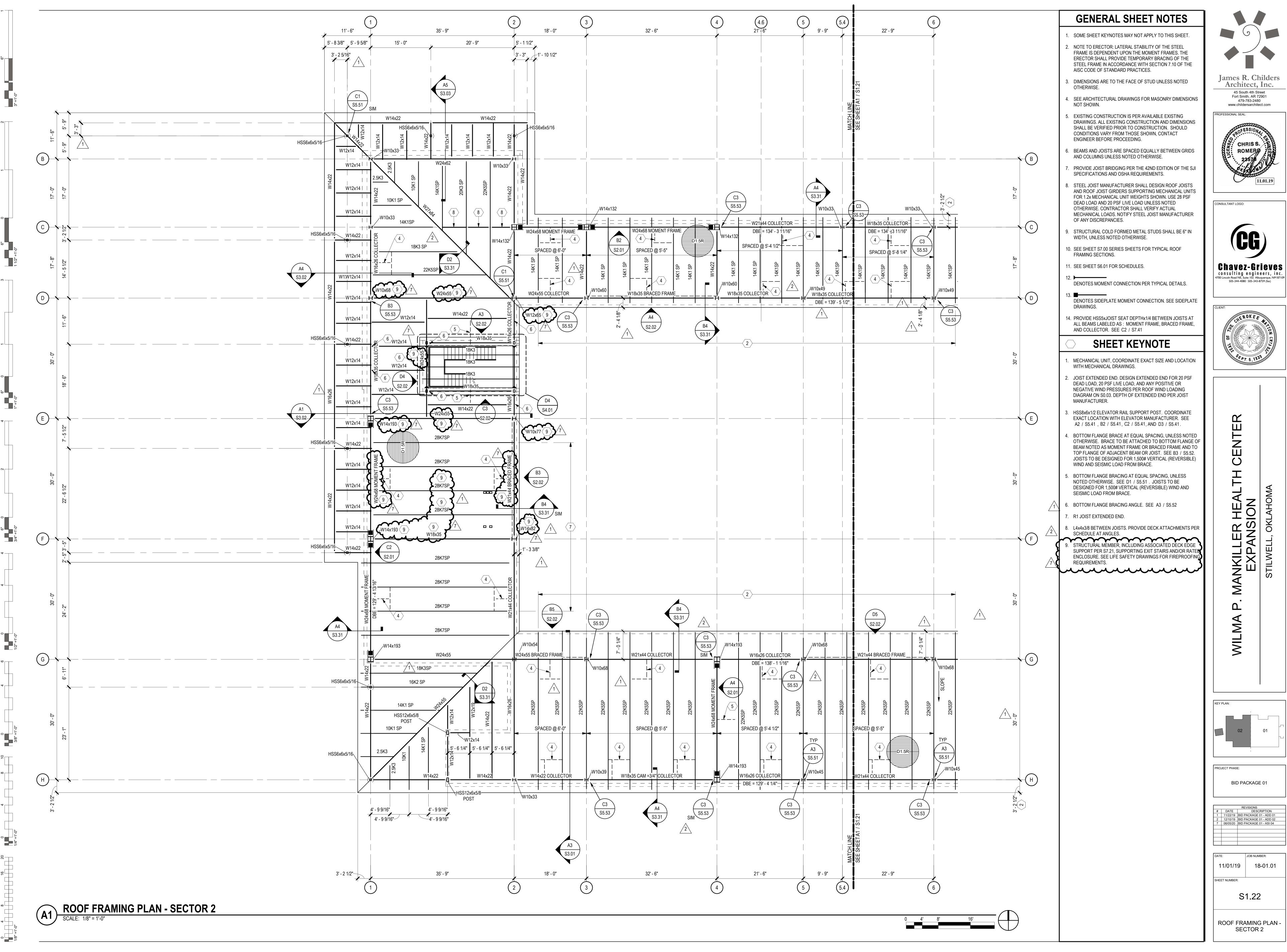
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| REVISIONS | DATE | DESCRIPTION | 11/22/19 | BID PACKAGE 01 - ADD 01 | 12/10/19 | BID PACKAGE 01 - ADD 02 | DESCRIPTION | DESCR 9 02/05/20 BID PACKAGE 01 – ASI 02 5 03/19/20 RFI 005 7 06/05/20 BID PACKAGE 01 - ASI 04

11/01/19 18-01.01

S1.21

ROOF FRAMING PLAN -SECTOR 1

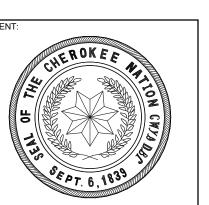


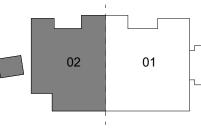


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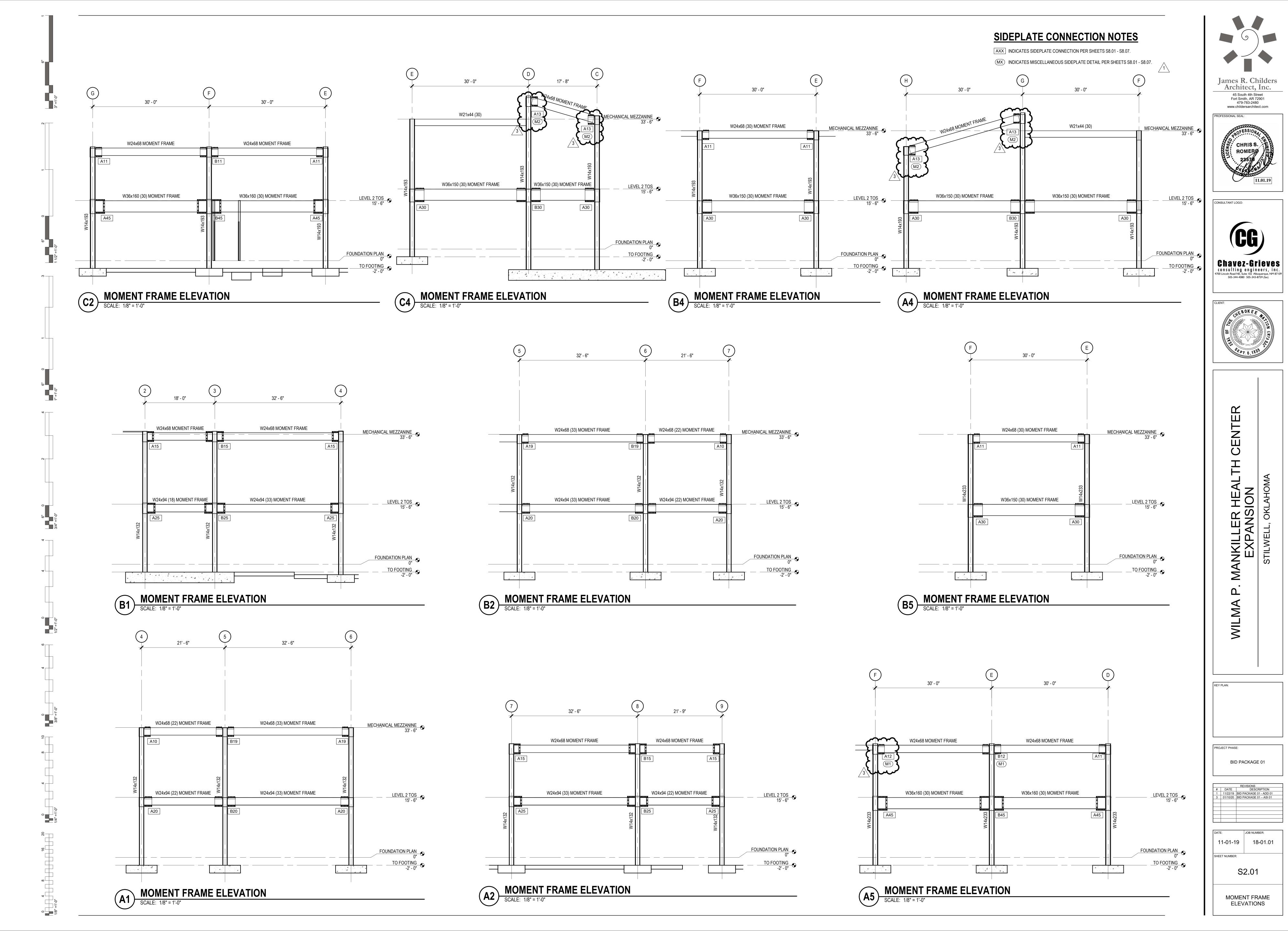


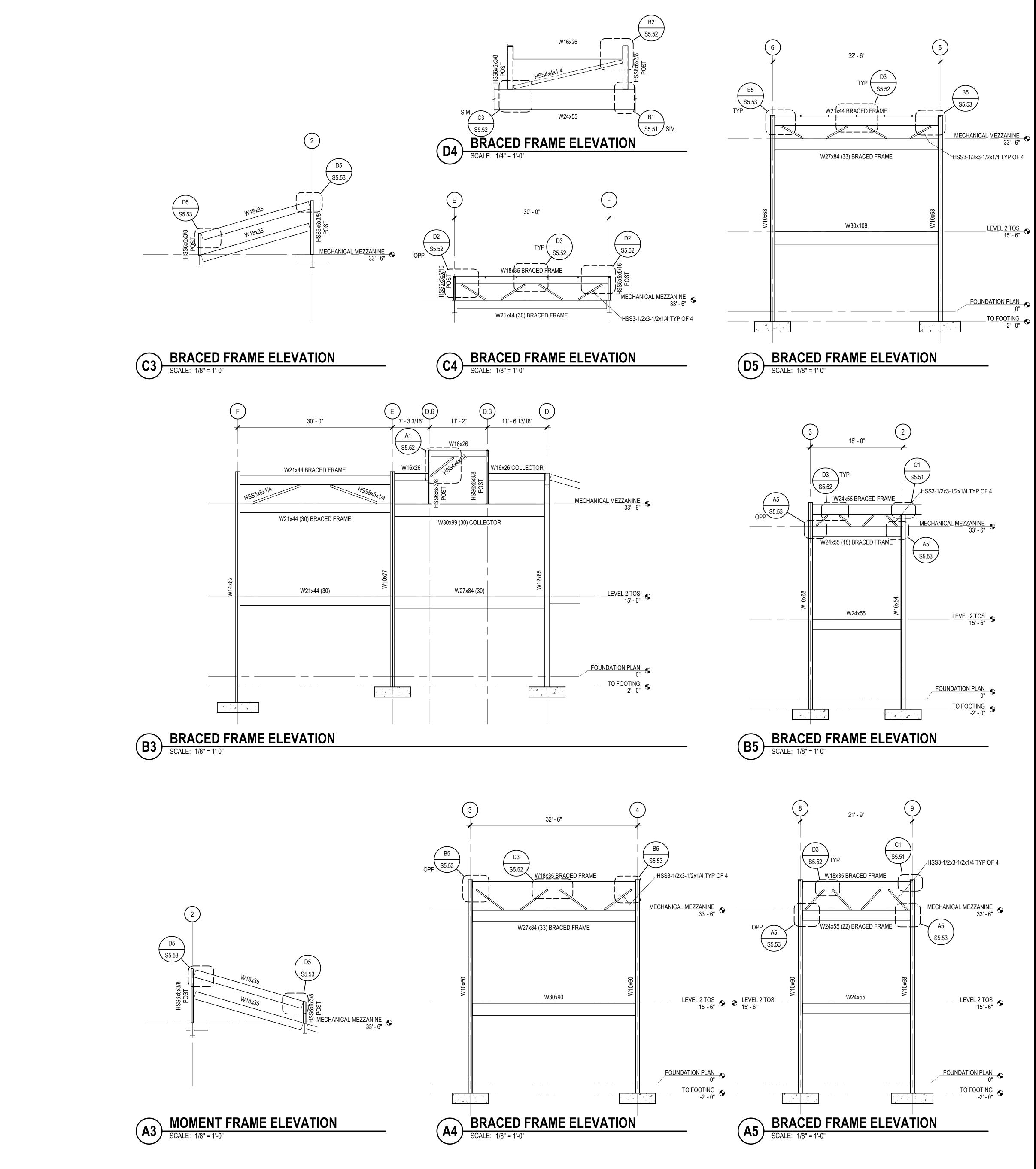
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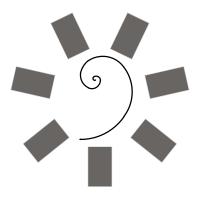
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2 12/10/19 BID PACKAGE 01 - ADD 02 06/05/20 BID PACKAGE 01 - ASI 04

11/01/19

ROOF FRAMING PLAN -SECTOR 2





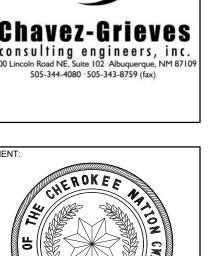


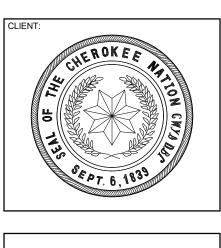
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Fort Smith, AR 72901
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WILMA P. MANKILLER HEALTH CENTE
EXPANSION
STILWELL, OKLAHOMA

KEY PLAN:

PROJECT PHASE:

BID PACKAGE 01

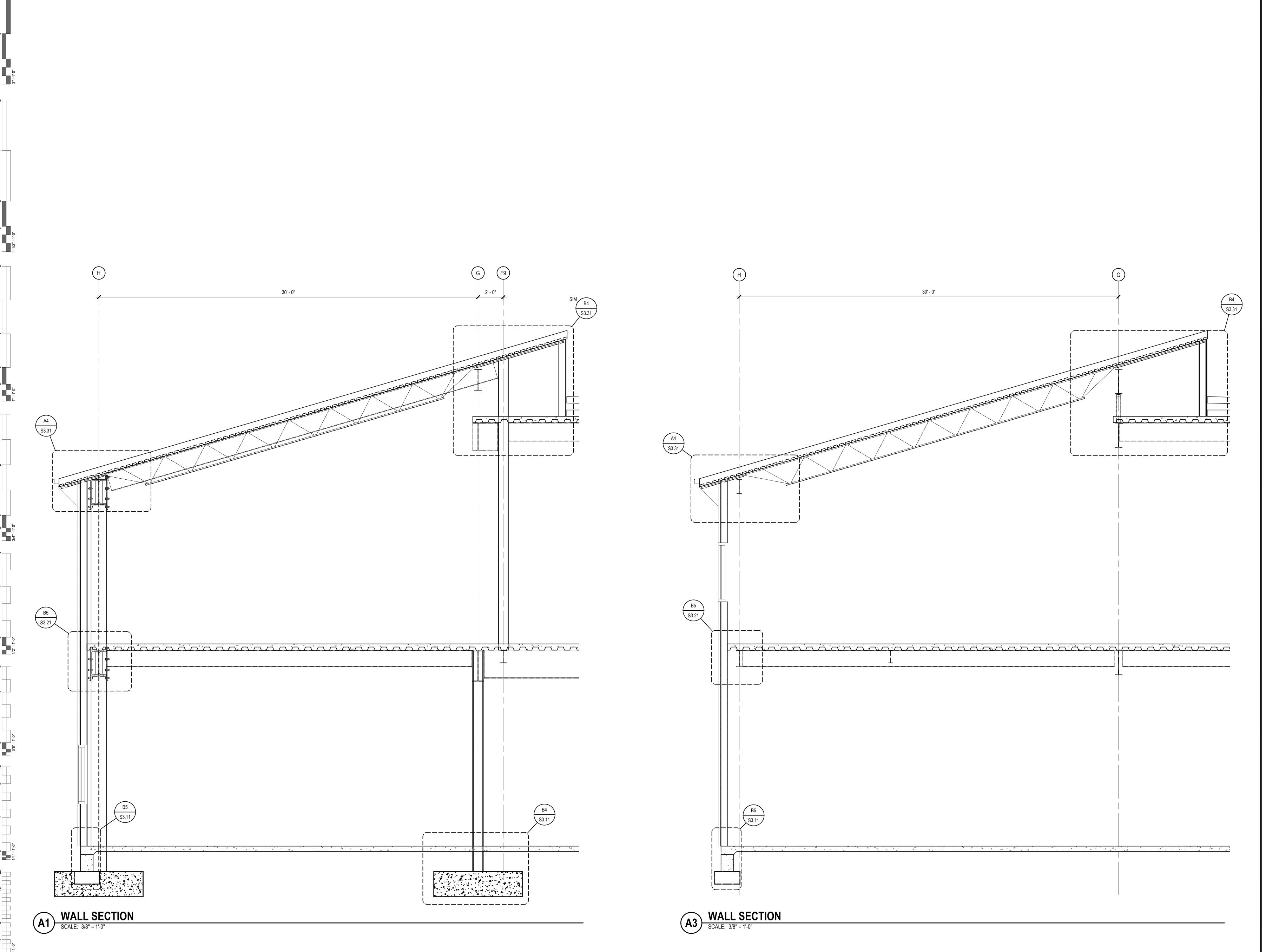
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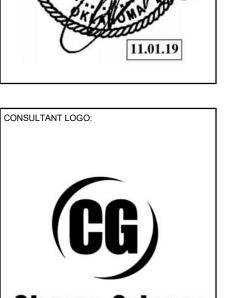
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MOMENT FRAME AND BRACED FRAME ELEVATIONS

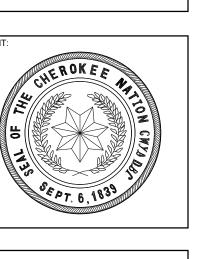














WILMA P. MANKILLER HEALTH CE EXPANSION

EY PLAN:

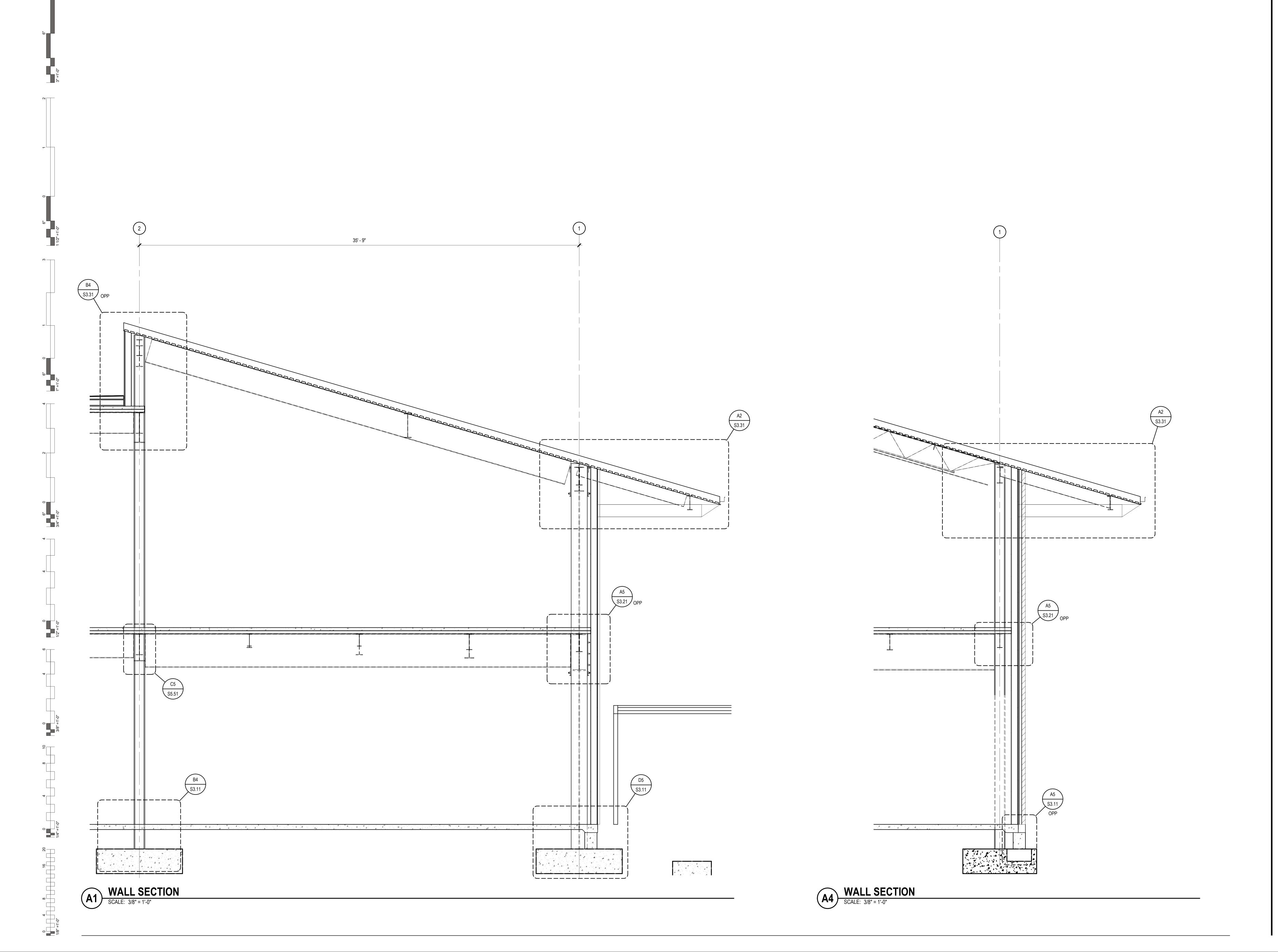
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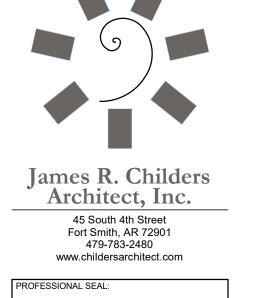
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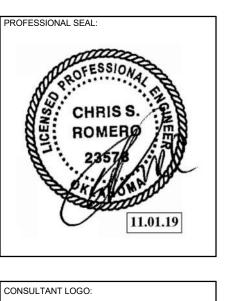
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SHEET NUMBER:

S3.01

WALL SECTIONS







Chavez-Grieves
consulting engineers, inc.
4700 Lincoln Road NE, Suite 102 · Albuquerque, NM 87109
505-344-4080 · 505-343-8759 (fax)

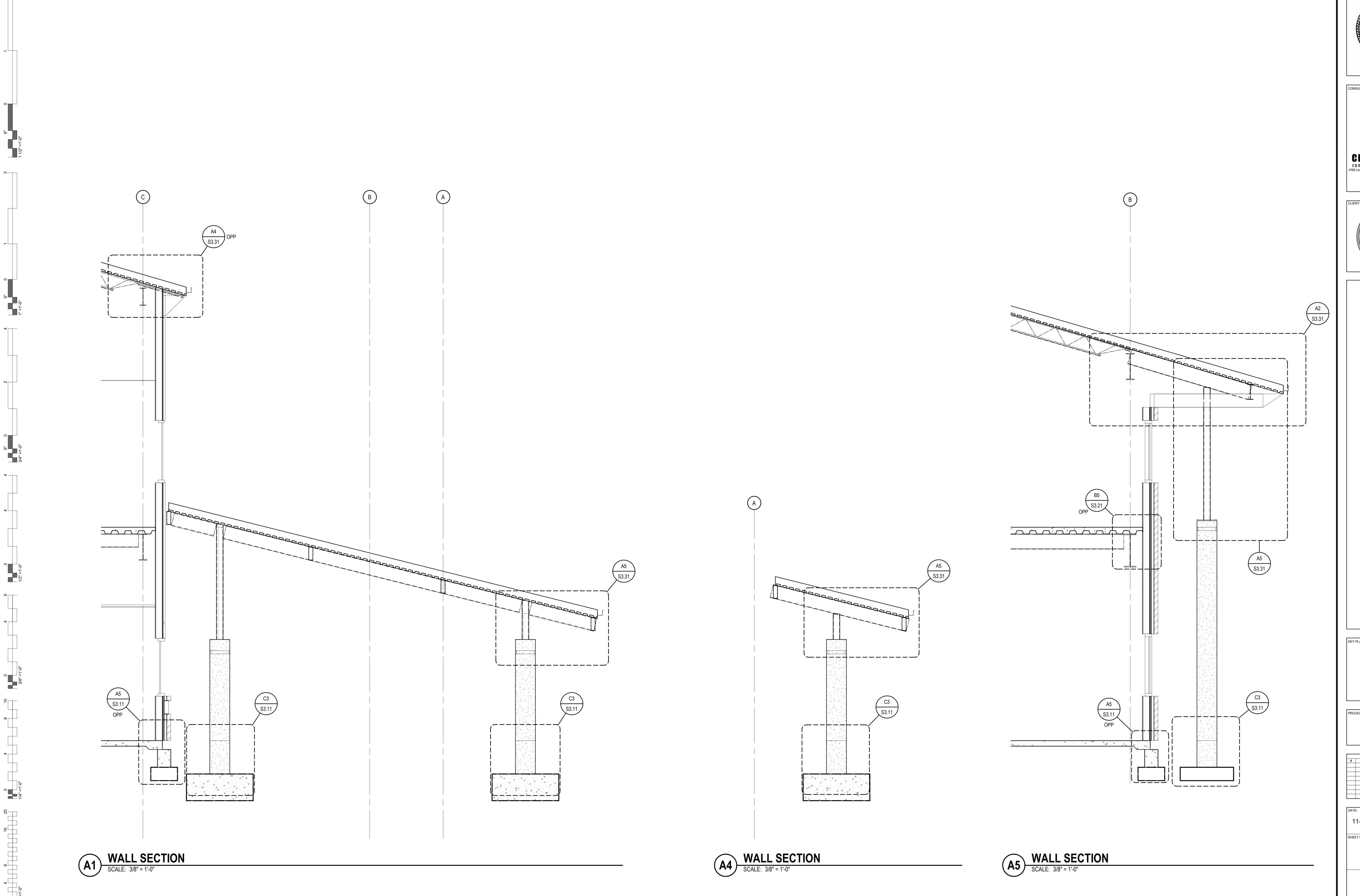
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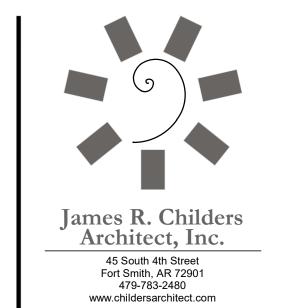
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S3.02

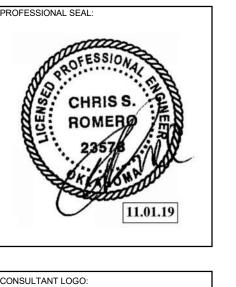
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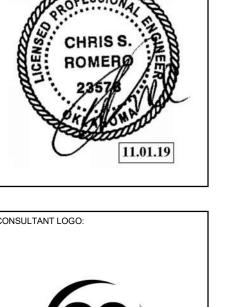
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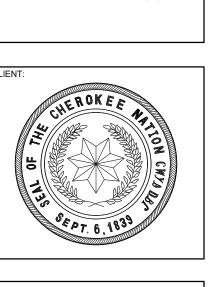


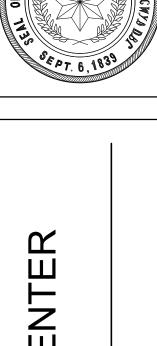






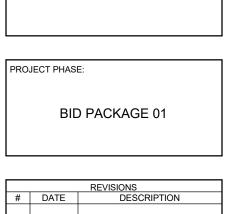








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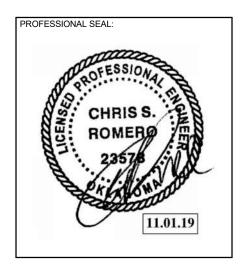


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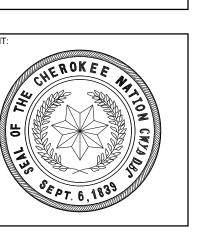
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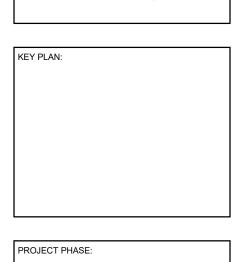








WILMA P. MANKILLER HEAL
EXPANSION



PROJECT PHASE:

BID PACKAGE 01

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#	DATE	DESCRIPTION

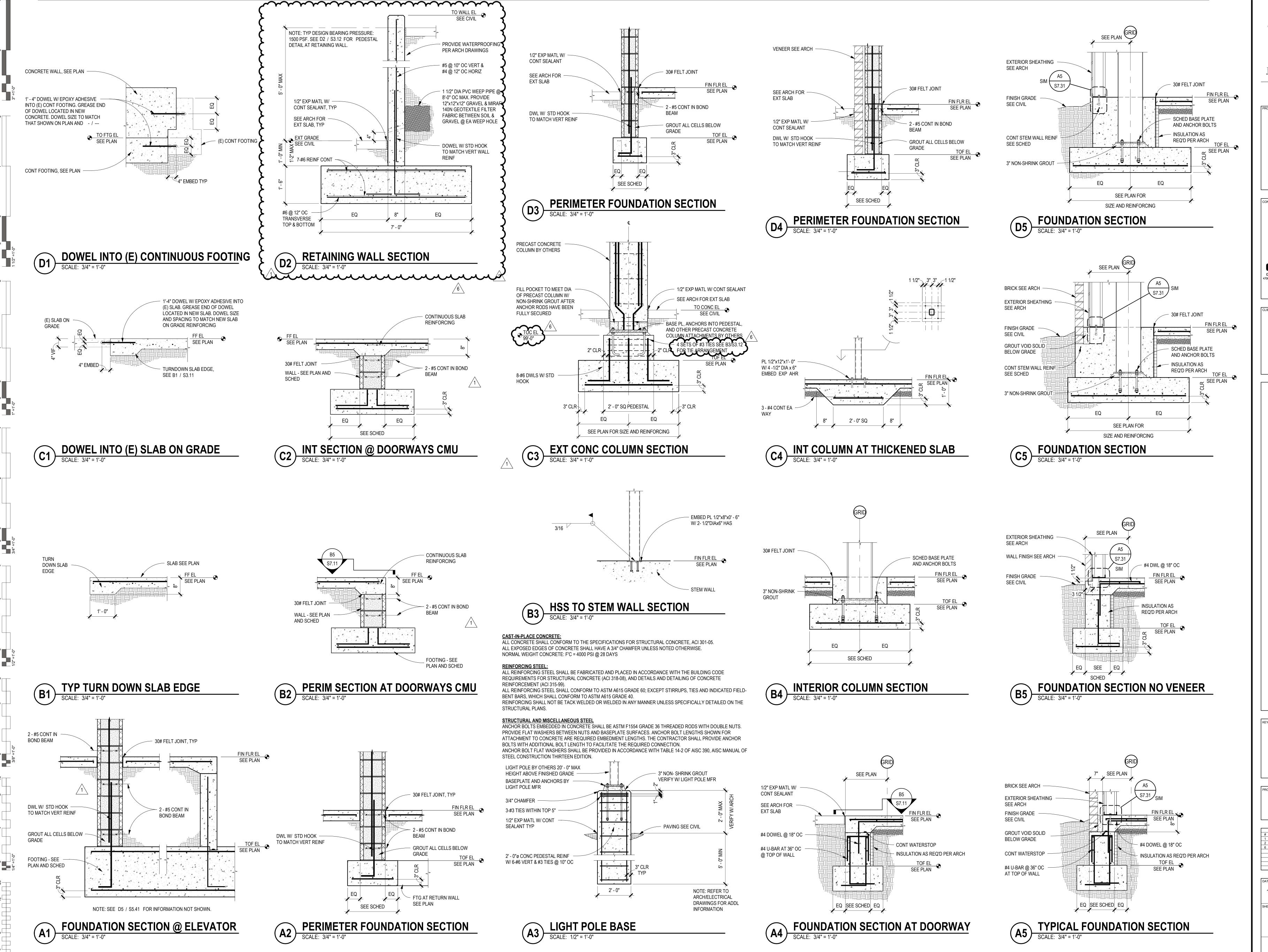
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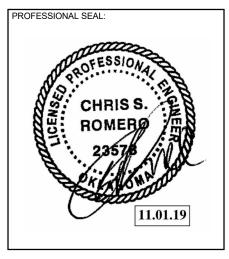
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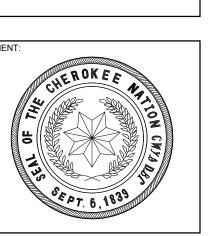
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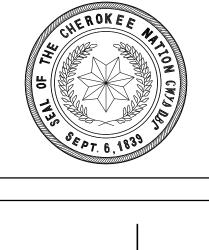


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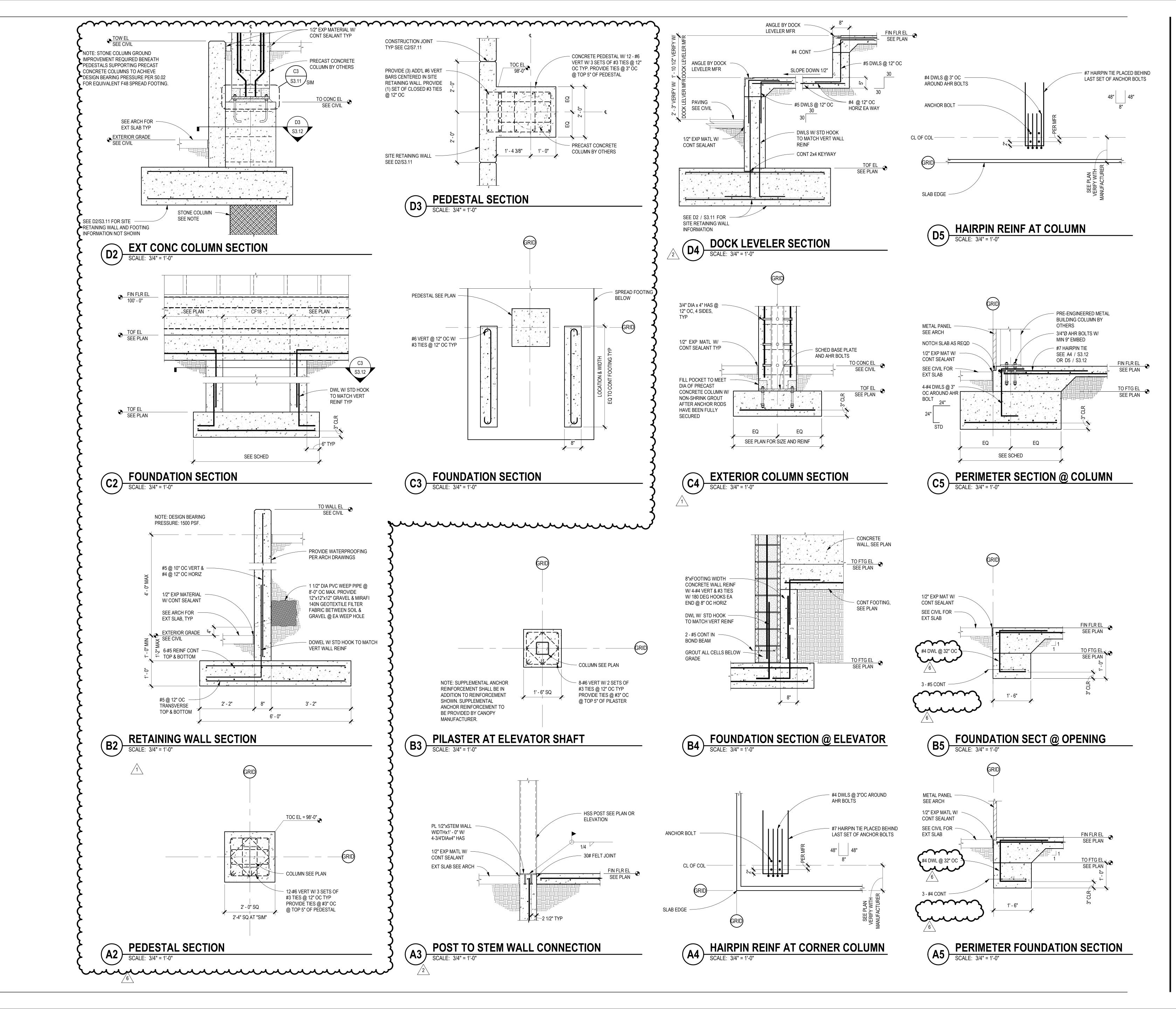
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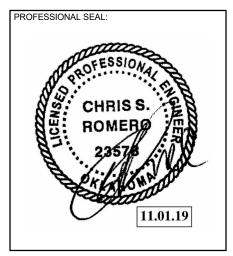
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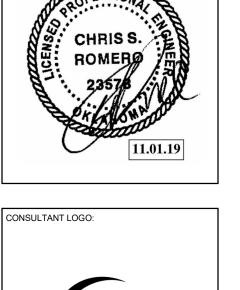
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FOUNDATION SECTIONS

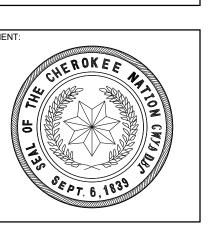


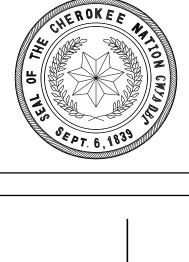
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PROJECT PHASE: **BID PACKAGE 01**

REVISIONS

DATE DESCRIPTION

11/22/19 BID PACKAGE 01 - ADD 01

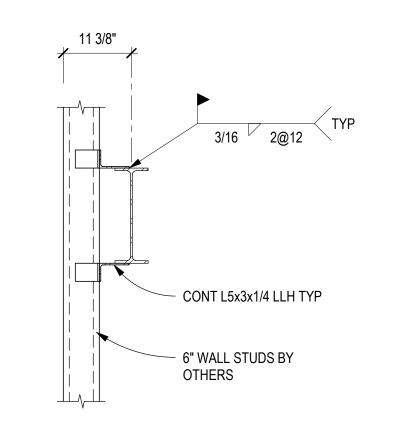
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11/01/19 18-01.01

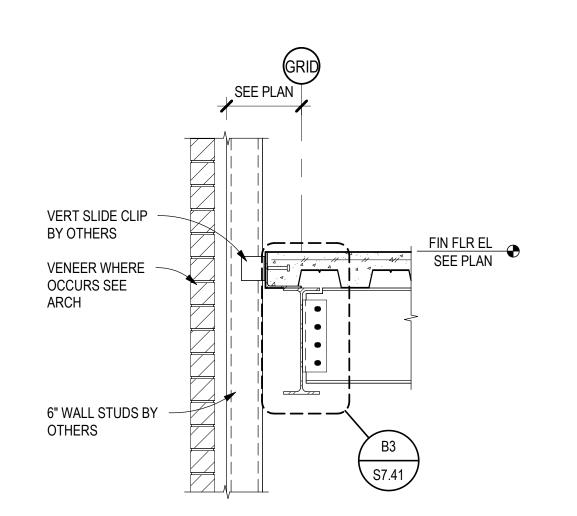
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FOUNDATION SECTIONS

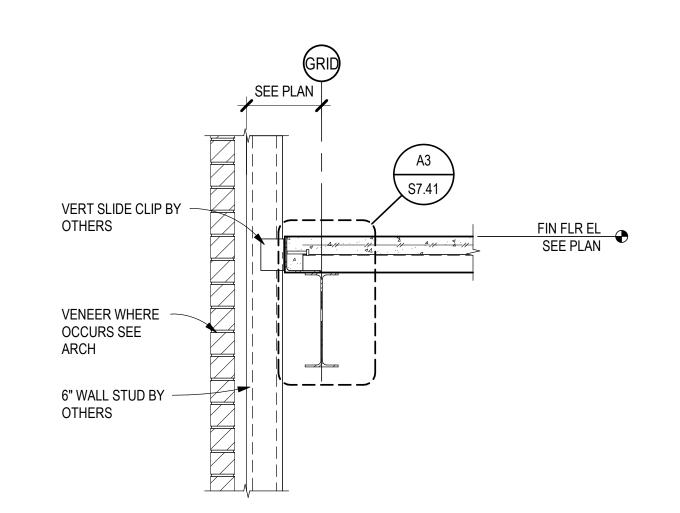




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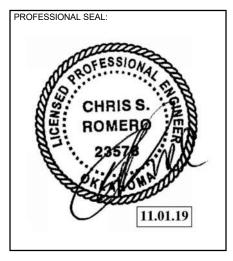


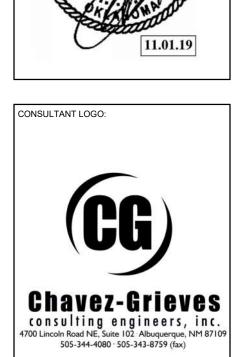
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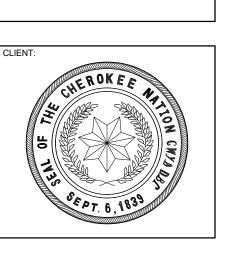


FLOOR FRAMING SECTION SCALE: 3/4" = 1'-0"

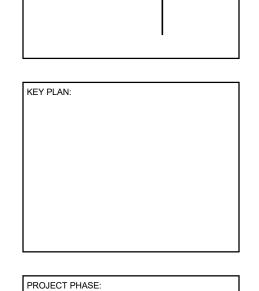
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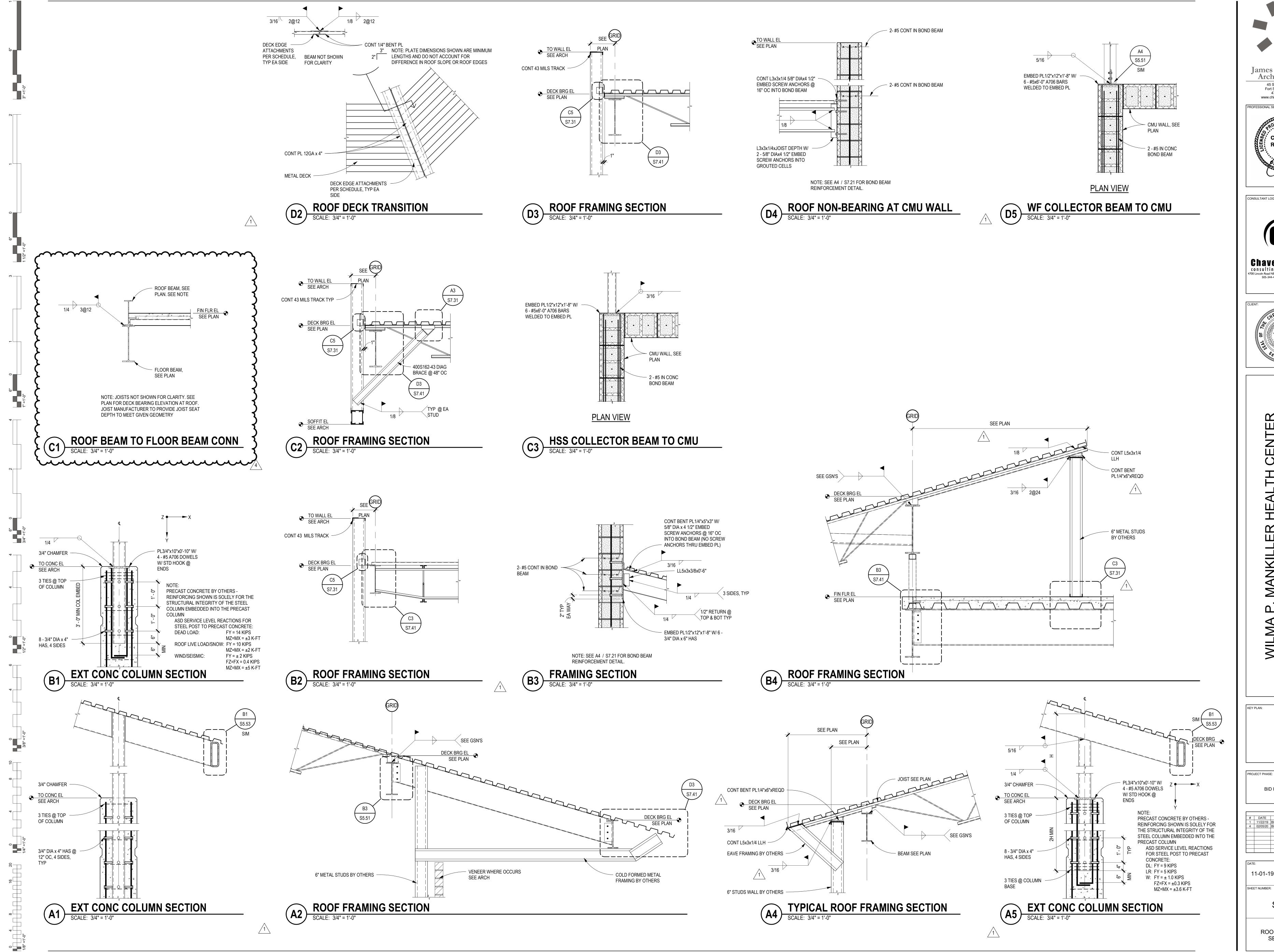


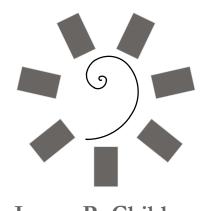


PROJECT PHASE:			
BID PACKAGE 01			
REVISIONS			
#	DATE	DESCRIPTION	

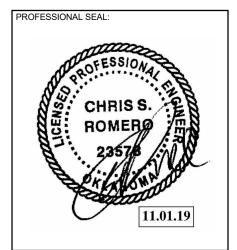
DATE:	JOB NUMBER:	
11-01-19	18-01.01	
SHEET NUMBER:		
S3.21		

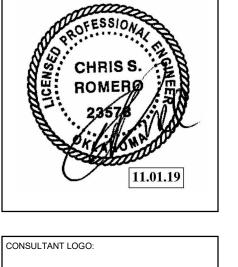
FLOOR FRAMING SECTIONS



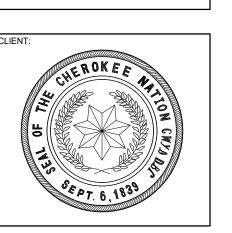


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NKILL EXPA

PROJECT PHASE:

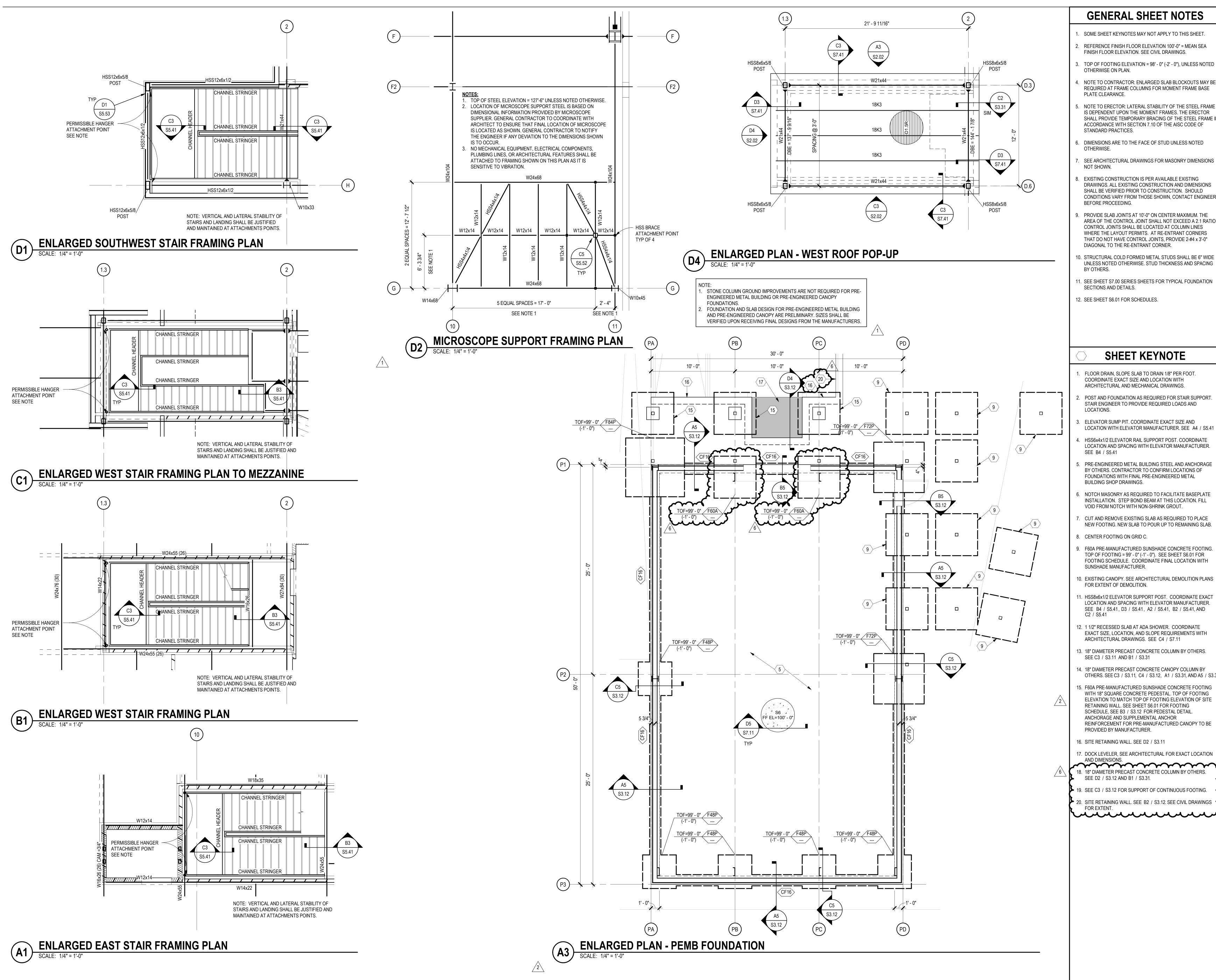
BID PACKAGE 01

| REVISIONS | # DATE | DESCRIPTION | 1 11/22/19 | BID PACKAGE 01 - ADD 01 | 4 02/05/20 | BID PACKAGE 01 - ASI 02 |

11-01-19 18-01.01

S3.31

ROOF FRAMING SECTIONS



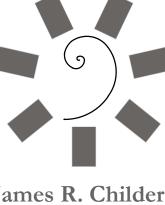
GENERAL SHEET NOTES

- SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET
- REFERENCE FINISH FLOOR ELEVATION 100'-0" = MEAN SEA
- FINISH FLOOR ELEVATION. SEE CIVIL DRAWINGS.
- TOP OF FOOTING ELEVATION = 98' 0" (-2' 0"), UNLESS NOTED OTHERWISE ON PLAN.
- NOTE TO CONTRACTOR: ENLARGED SLAB BLOCKOUTS MAY BE REQUIRED AT FRAME COLUMNS FOR MOMENT FRAME BASE PLATE CLEARANCE.
- NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE MOMENT FRAMES. THE ERECTOR
- 6. DIMENSIONS ARE TO THE FACE OF STUD UNLESS NOTED
- SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS
- 3. EXISTING CONSTRUCTION IS PER AVAILABLE EXISTING DRAWINGS. ALL EXISTING CONSTRUCTION AND DIMENSIONS SHALL BE VERIFIED PRIOR TO CONSTRUCTION. SHOULD CONDITIONS VARY FROM THOSE SHOWN, CONTACT ENGINEER
- PROVIDE SLAB JOINTS AT 10'-0" ON CENTER MAXIMUM. THE AREA OF THE CONTROL JOINT SHALL NOT EXCEED A 2.1 RATIO. CONTROL JOINTS SHALL BE LOCATED AT COLUMN LINES WHERE THE LAYOUT PERMITS. AT RE-ENTRANT CORNERS THAT DO NOT HAVE CONTROL JOINTS, PROVIDE 2-#4 x 3'-0" DIAGONAL TO THE RE-ENTRANT CORNER.
- 0. STRUCTURAL COLD FORMED METAL STUDS SHALL BE 6" WIDE UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING
- 1. SEE SHEET S7.00 SERIES SHEETS FOR TYPICAL FOUNDATION SECTIONS AND DETAILS.
- 12. SEE SHEET S6.01 FOR SCHEDULES.

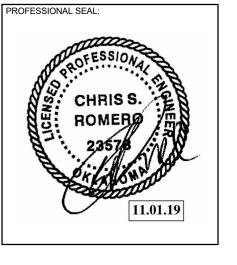
SHEET KEYNOTE

- FLOOR DRAIN, SLOPE SLAB TO DRAIN 1/8" PER FOOT. COORDINATE EXACT SIZE AND LOCATION WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
- POST AND FOUNDATION AS REQUIRED FOR STAIR SUPPORT STAIR ENGINEER TO PROVIDE REQUIRED LOADS AND
- ELEVATOR SUMP PIT. COORDINATE EXACT SIZE AND LOCATION WITH ELEVATOR MANUFACTURER. SEE A4 / S5.41
- HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION AND SPACING WITH ELEVATOR MANUFACTURER.
- PRE-ENGINEERED METAL BUILDING STEEL AND ANCHORAGE BY OTHERS. CONTRACTOR TO CONFIRM LOCATIONS OF
- NOTCH MASONRY AS REQUIRED TO FACILITATE BASEPLATE INSTALLATION. STEP BOND BEAM AT THIS LOCATION. FILL
- CUT AND REMOVE EXISTING SLAB AS REQUIRED TO PLACE NEW FOOTING. NEW SLAB TO POUR UP TO REMAINING SLAB.
- 8. CENTER FOOTING ON GRID C.
- F60A PRE-MANUFACTURED SUNSHADE CONCRETE FOOTING. TOP OF FOOTING = 99' - 0" (-1' - 0"). SEE SHEET S6.01 FOR FOOTING SCHEDULE. COORDINATE FINAL LOCATION WITH SUNSHADE MANUFACTURER.
- 10. EXISTING CANOPY. SEE ARCHITECTURAL DEMOLITION PLANS FOR EXTENT OF DEMOLITION.
- LOCATION AND SPACING WITH ELEVATOR MANUFACTURER. SEE B4 / S5.41, D3 / S5.41, A2 / S5.41, B2 / S5.41, AND
- 12. 1 1/2" RECESSED SLAB AT ADA SHOWER. COORDINATE EXACT SIZE, LOCATION, AND SLOPE REQUIREMENTS WITH ARCHITECTURAL DRAWINGS. SEE C4 / S7.11
- 13. 18" DIAMETER PRECAST CONCRETE COLUMN BY OTHERS. SEE C3 / S3.11 AND B1 / S3.31
- 14. 18" DIAMETER PRECAST CONCRETE CANOPY COLUMN BY
- OTHERS. SEE C3 / S3.11, C4 / S3.12, A1 / S3.31, AND A5 / S3.3 15. F60A PRE-MANUFACTURED SUNSHADE CONCRETE FOOTING WITH 18" SQUARE CONCRETE PEDESTAL. TOP OF FOOTING
- RETAINING WALL. SEE SHEET S6.01 FOR FOOTING SCHEDULE, SEE B3 / S3.12 FOR PEDESTAL DETAIL. ANCHORAGE AND SUPPLEMENTAL ANCHOR REINFORCEMENT FOR PRE-MANUFACTURED CANOPY TO BE PROVIDED BY MANUFACTURER.
- 16. SITE RETAINING WALL. SEE D2 / S3.11
- 17. DOCK LEVELER, SEE ARCHITECTURAL FOR EXACT LOCATION AND DIMENSIONS. 18. 18" DIAMETER PRECAST CONCRETE COLUMN BY OTHERS.
- 19. SEE C3 / S3.12 FOR SUPPORT OF CONTINUOUS FOOTING.
- ▶ 20. SITE RETAINING WALL. SEE B2 / S3.12. SEE CIVIL DRAWINGS

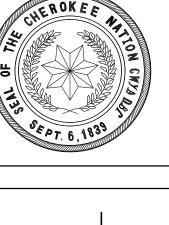


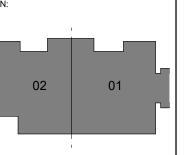












PROJECT PHASE:

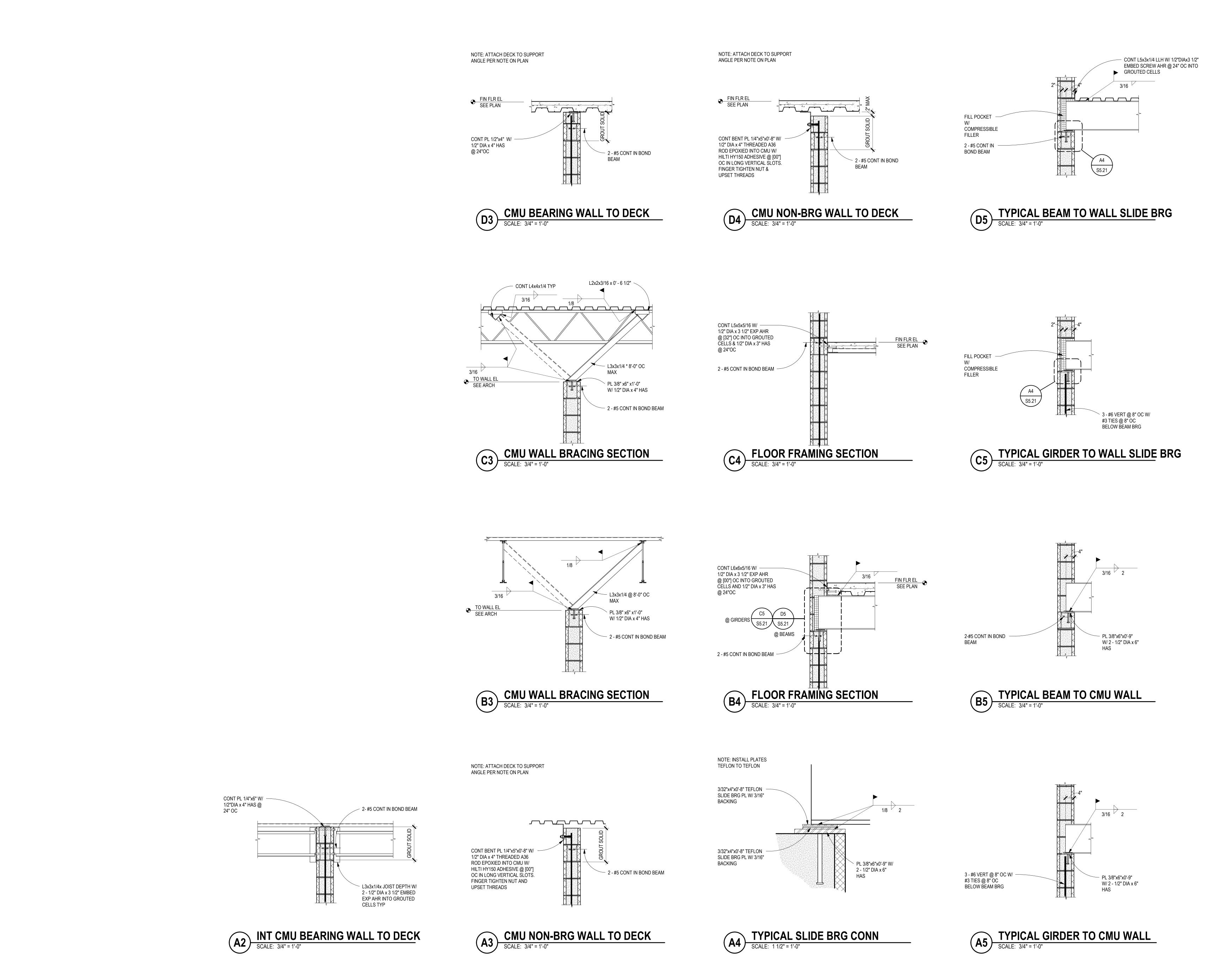
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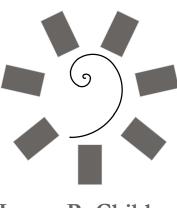
11/22/19 BID PACKAGE 01 - ADD 01 12/10/19 BID PACKAGE 01 - ADD 02 6 04/16/20 BID PACKAGE 01 - ASI 03

11/01/19 18-01.01 SHEET NUMBER:

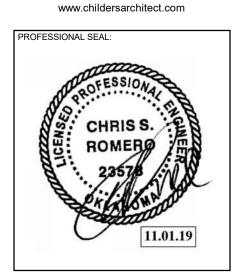
S4.01

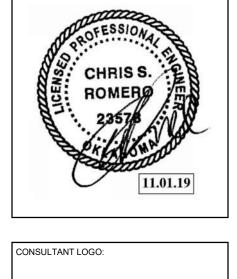
ENLARGED PLANS



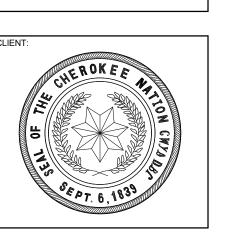


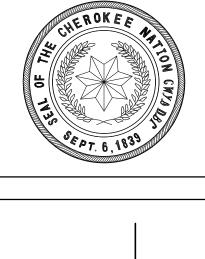
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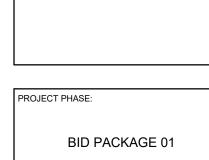






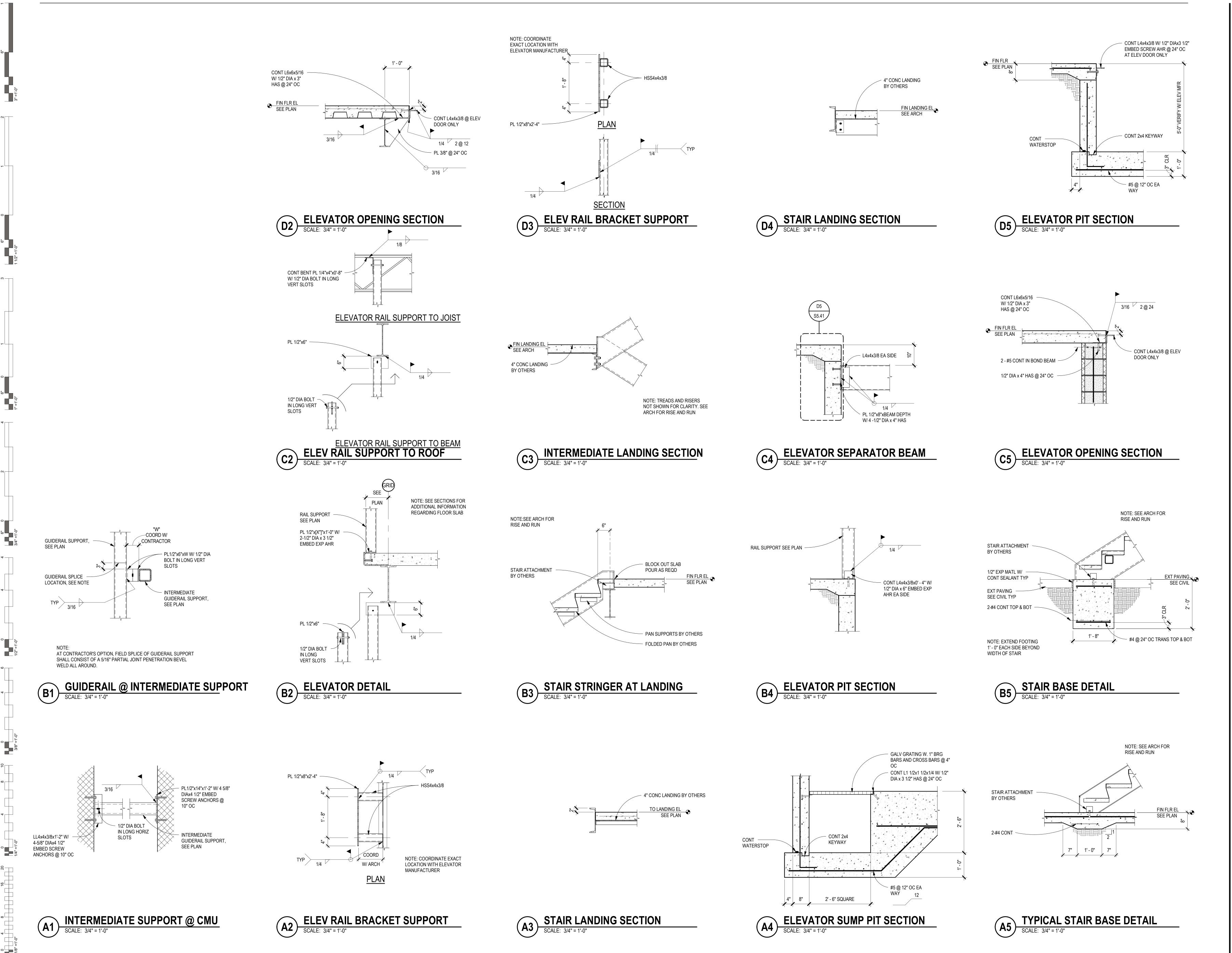


MANKILLER HEAL EXPANSION

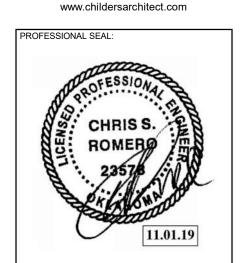


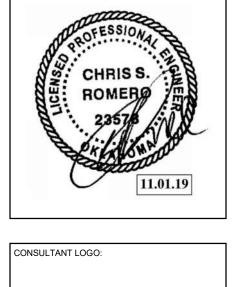
11-01-19 18-01.01 SHEET NUMBER: S5.21

MASONRY FRAMING SECTIONS AND DETAILS

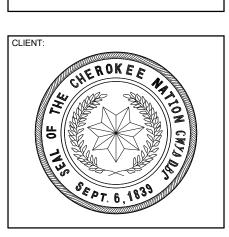


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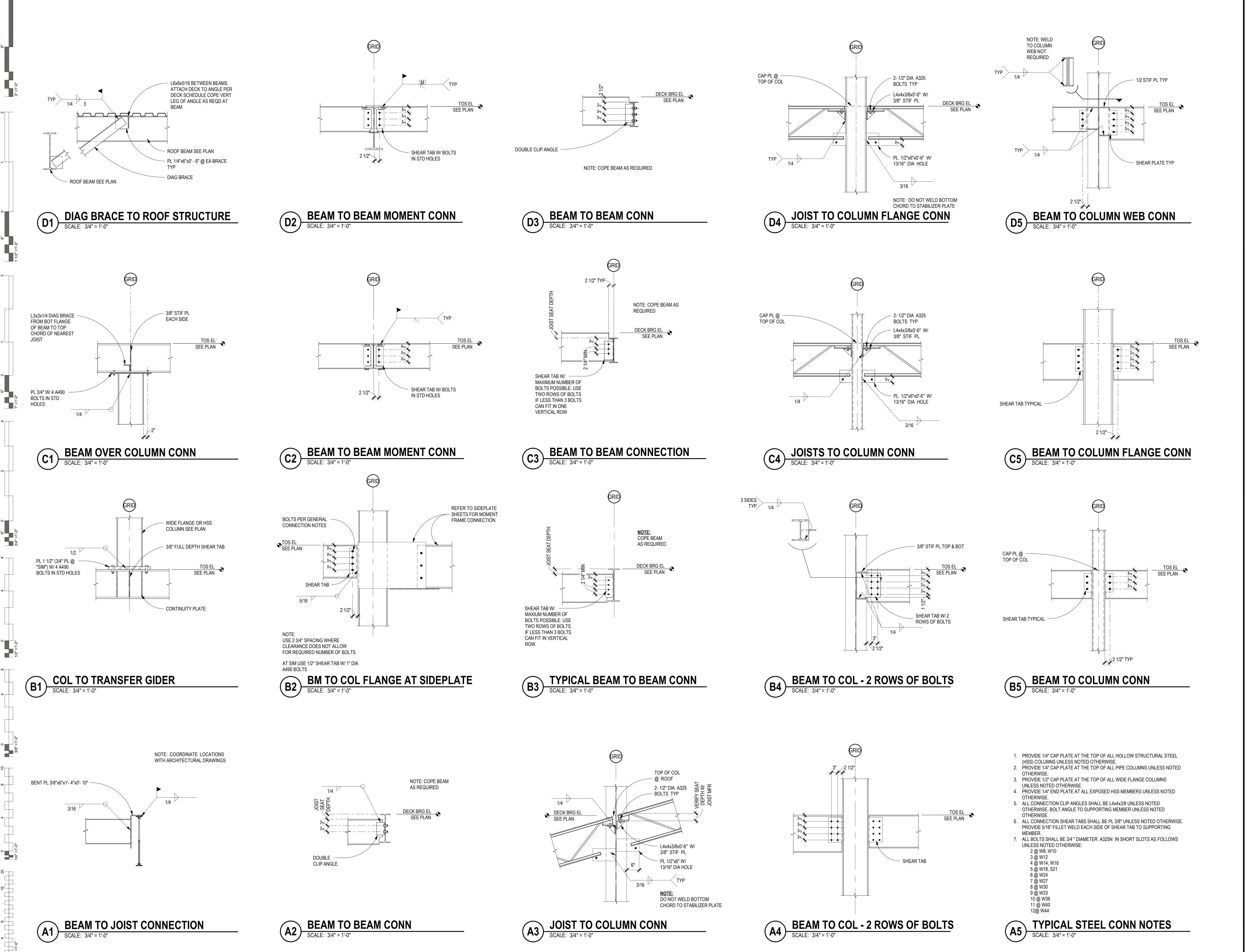
MANKILLER HEAL EXPANSION

PROJECT PHASE: **BID PACKAGE 01**

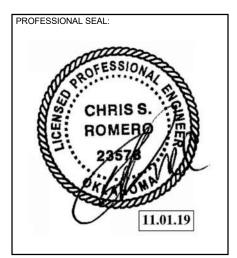
REVISIONS
DESCRIPTION

18-01.01 11-01-19 SHEET NUMBER: S5.41

VERTICAL CIRCULATION DETAILS

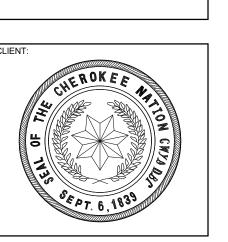


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MANKILLER HEAL EXPANSION

PROJECT PHASE: **BID PACKAGE 01**

REVISIONS DESCRIPTION

11-01-19 18-01.01 SHEET NUMBER:

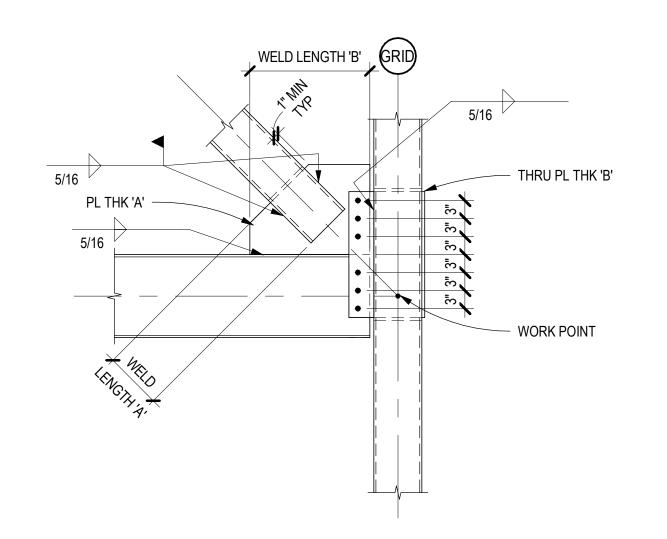
S5.51

STEEL DETAILS

	CONNECTION SCHEDULE								
BRACE SIZE	PL THK 'A'	THRU PL THK 'B'	WELD LENGTH 'A'	WELD LENGTH 'B'	GUSSET PL BOI				
HSS4x4	5/8"	5/8"	4"	20"	3				
HSS5x5	5/8"	5/8"	5"	20"	3				
HSS6x6	5/8"	5/8"	6"	22"	3				
HSS8x8	3/4"	3/4"	8"	24"	4				
HSS12x8	1"	1"	12"	28"	5				

1. LENGTHS GIVEN ARE SINGLE-SIDE LENGTHS AND MINIMUM LENGTHS 2. LONGER GUSSET/WELD LENGTHS MAY BE REQ'D WHERE BRACE SLOPE VARIES FROM 1:1 3. ALL BOLTS IN STANDARD HOLES

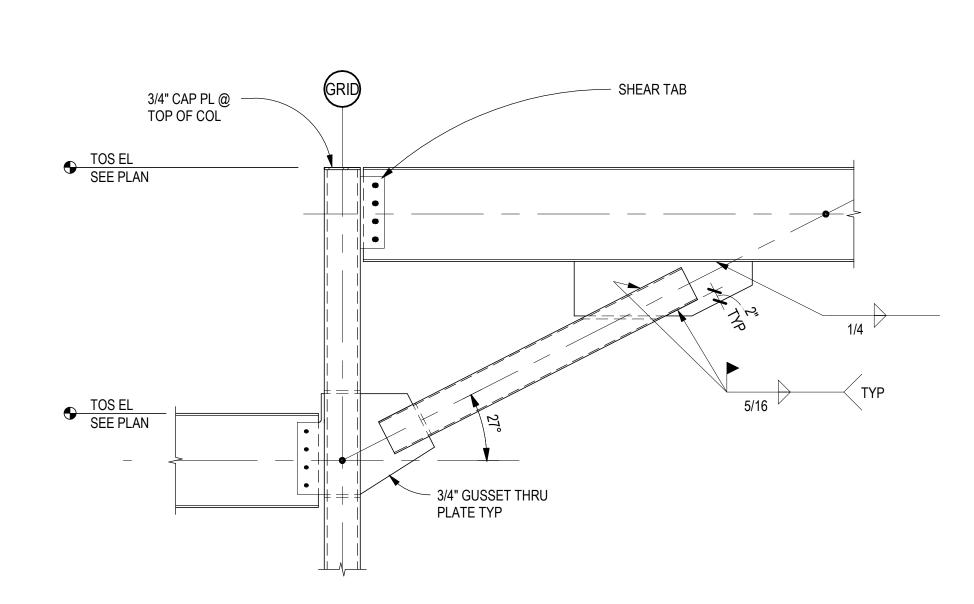
BRACED FRAME CONNECTION SCALE: 3/4" = 1'-0"



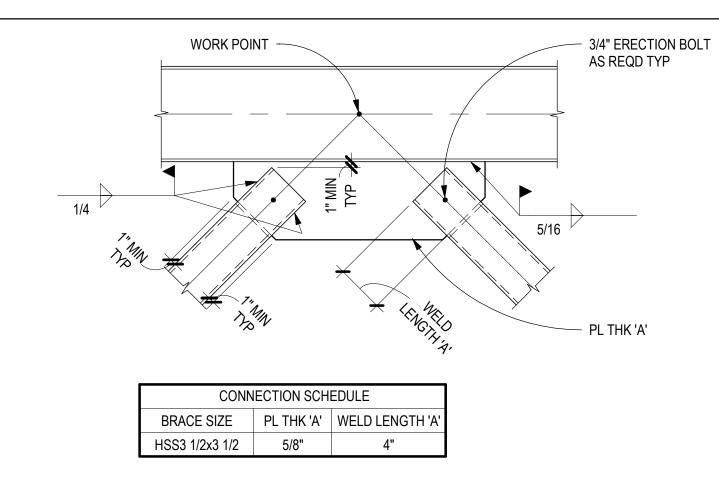
CONNECTION SCHEDULE								
BRACE SIZE	PL THK 'A'	THRU PL THK 'B'	WELD LENGTH 'A'	WELD LENGTH 'B'	GUSSET PL BOLT			
HSS4x4	5/8"	5/8"	4"	20"	3			
HSS5x5	5/8"	5/8"	5"	20"	3			
HSS6x6	5/8"	5/8"	6"	22"	3			
HSS8x8	3/4"	3/4"	8"	24"	4			
HSS12x8	1"	1"	12"	28"	5			

1. LENGTHS GIVEN ARE SINGLE-SIDE LENGTHS AND MINIMUM LENGTHS 2. LONGER GUSSET/WELD LENGTHS MAY BE REQ'D WHERE BRACE SLOPE VARIES FROM 1:1 3. ALL BOLTS IN STANDARD HOLES

BRACED FRAME CONNECTION SCALE: 3/4" = 1'-0"

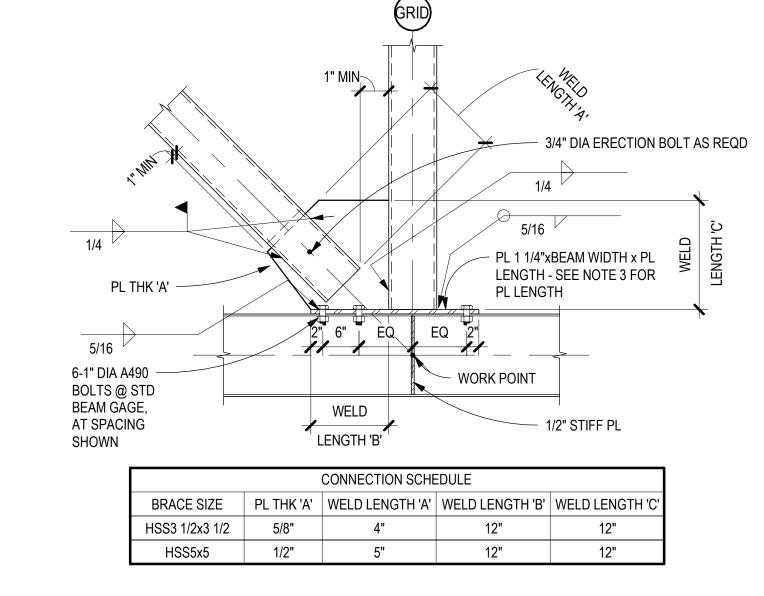


COLLECTOR FRAMING DETAIL



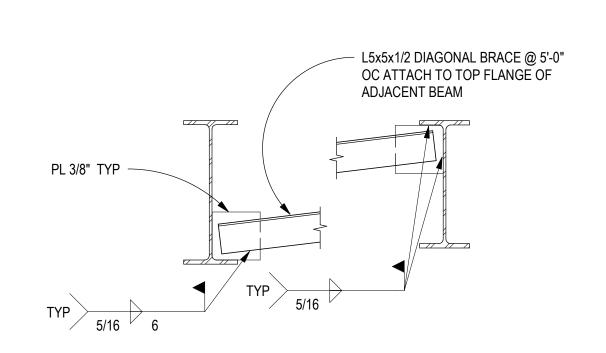
1. LENGTHS GIVEN ARE SINGLE-SIDE LENGTHS AND MINIMUM REQUIRED LENGTHS 2. LONGER GUSSET/WELD LENGTHS MAY BE REQ'D WHERE BRACE SLOPE VARIES FROM 1:1 3. BRACE AND GUSSET CONNECTION CAN OCCUR ON TOP AND BOTTOM FLANGES

BRACED FRAME CONNECTION

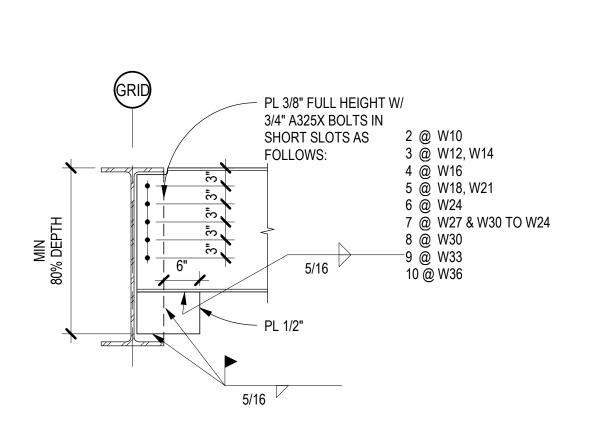


1. LENGTHS GIVEN ARE SINGLE-SIDE LENGTHS AND MINIMUM LENGTHS 2. LONGER GUSSET/WELD LENGTHS MAY BE REQ'D WHERE BRACE SLOPE VARIES FROM 1:1 3. PLATE LENGTHS: BASED ON POST SIZE A. HSS5x5 & HSS6x6: 2' - 0"

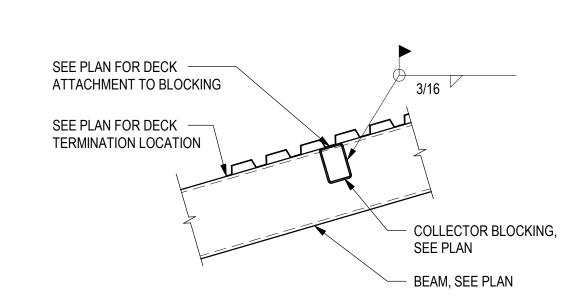
BRACED FRAME CONNECTION



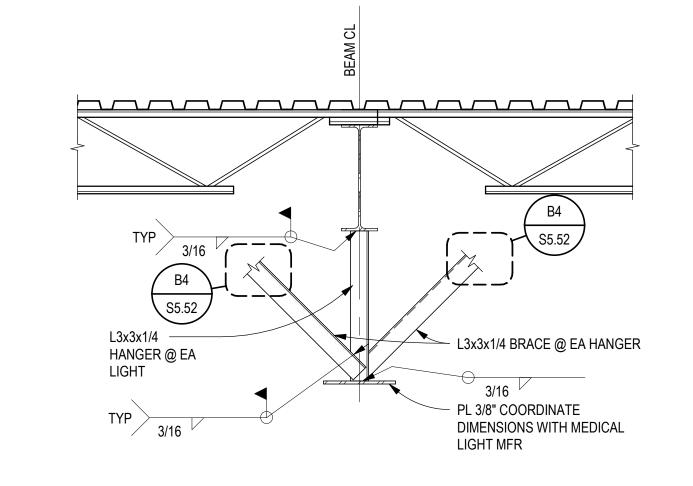
DIAG ANGLE AT MOMENT CONN



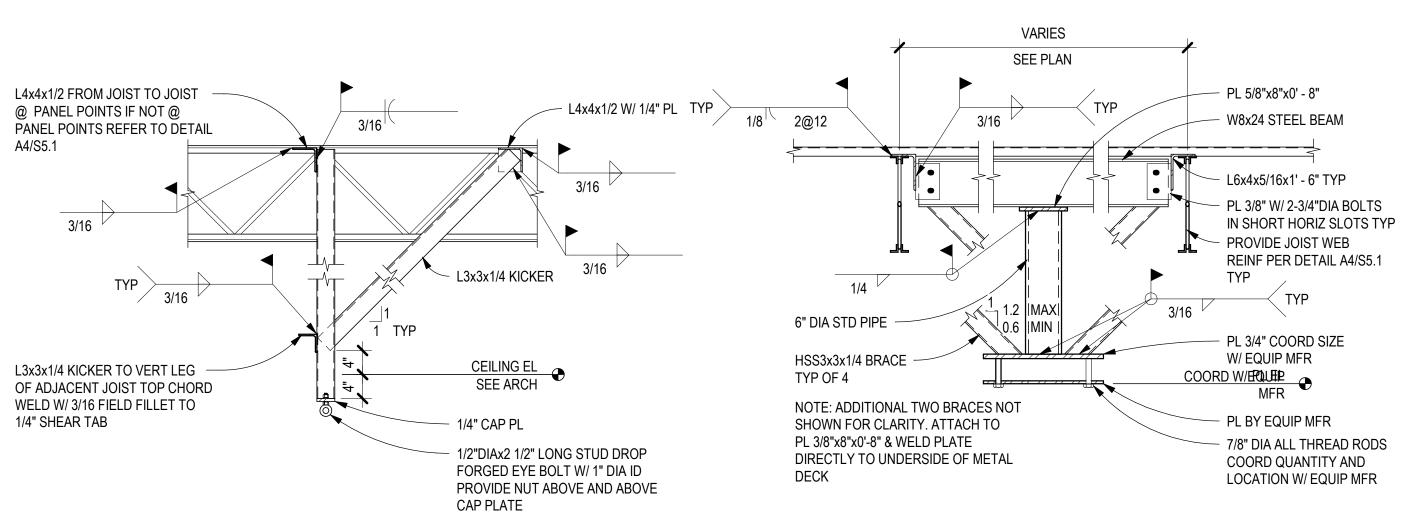
BEAM CONN AT MOMENT (WUF) CONN



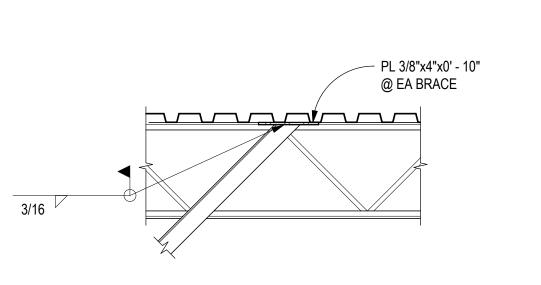


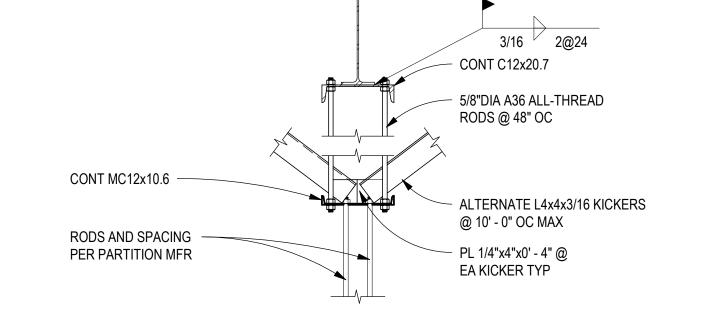










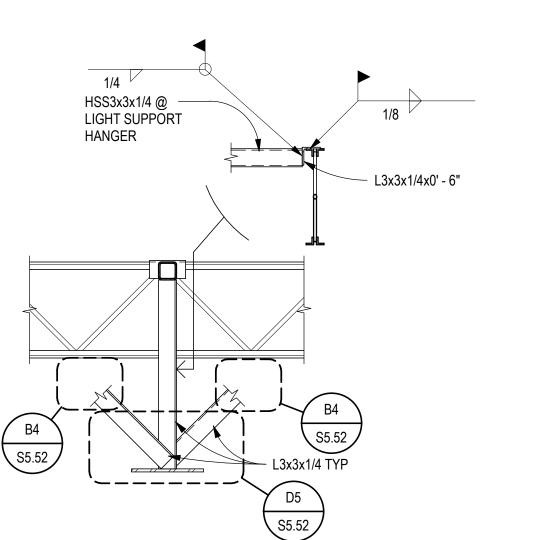


PARTITION SUPPORT SECTION

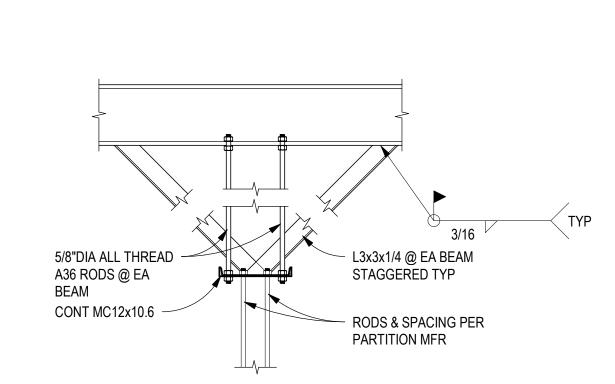
SCALE: 3/4" = 1'-0"

SURGICAL LIGHT SUPPORT

B4 LIGHT SUPPORT BETWEEN JOISTS SCALE: 3/4" = 1'-0"



LIGHT SUPPORT BETWEEN JOISTS

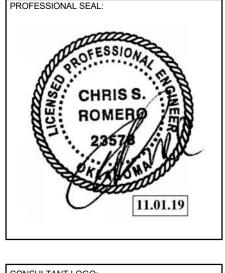


PARTITION FRAMING SECTION

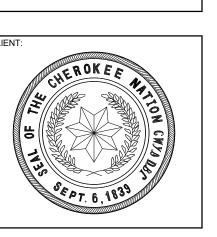


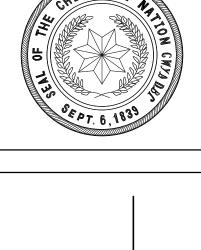
479-783-2480









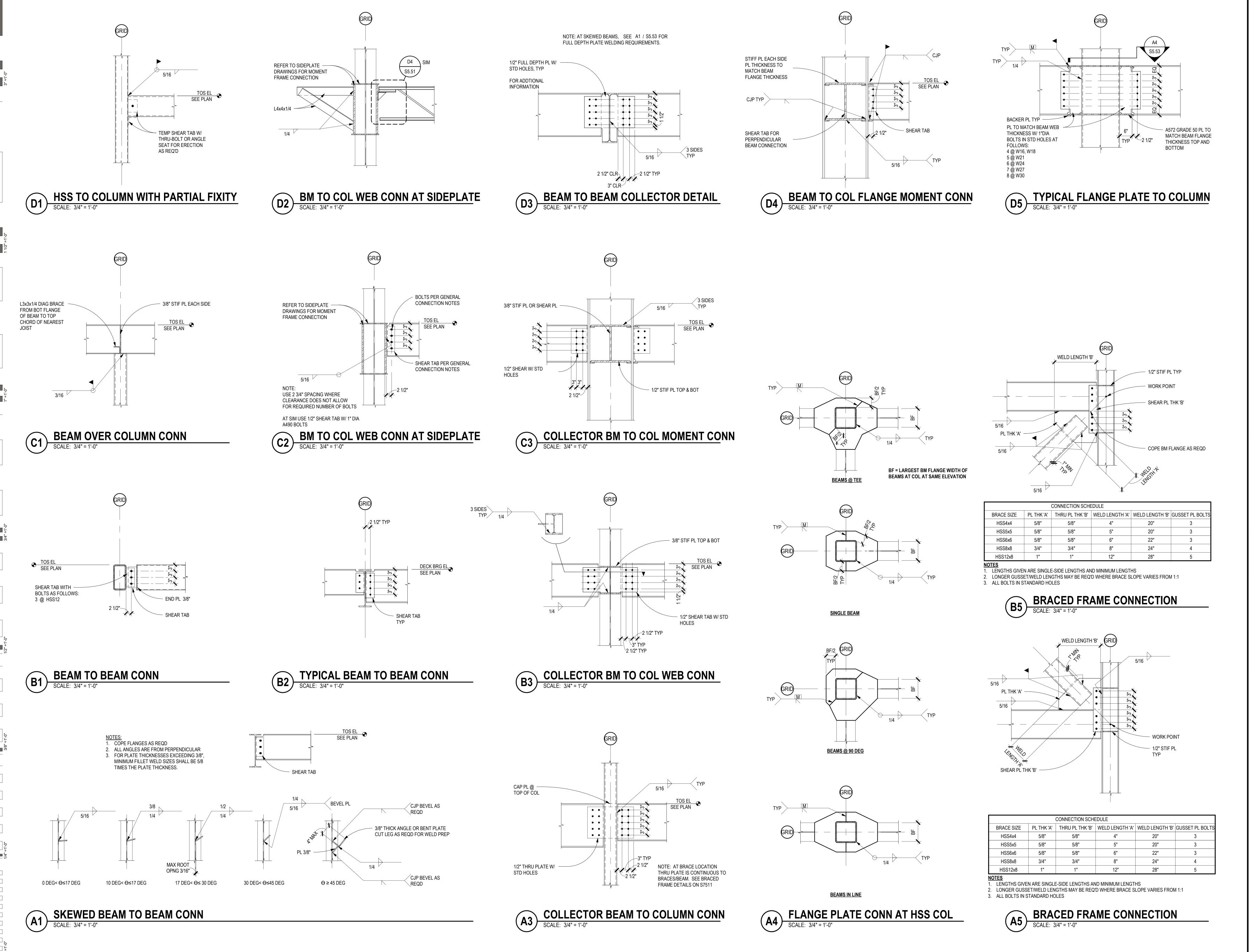


MANKILLER HEAL EXPANSION

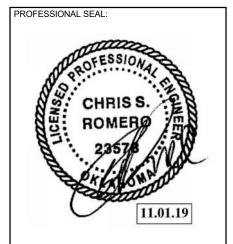
PROJECT PHASE: **BID PACKAGE 01**

18-01.01 11-01-19 SHEET NUMBER: S5.52

STEEL DETAILS

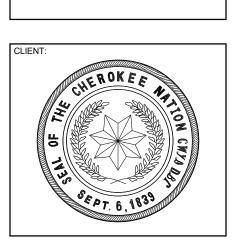


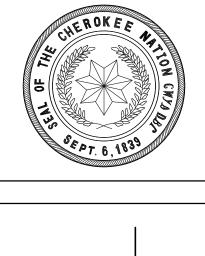
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ANKILLER HEAL EXPANSION

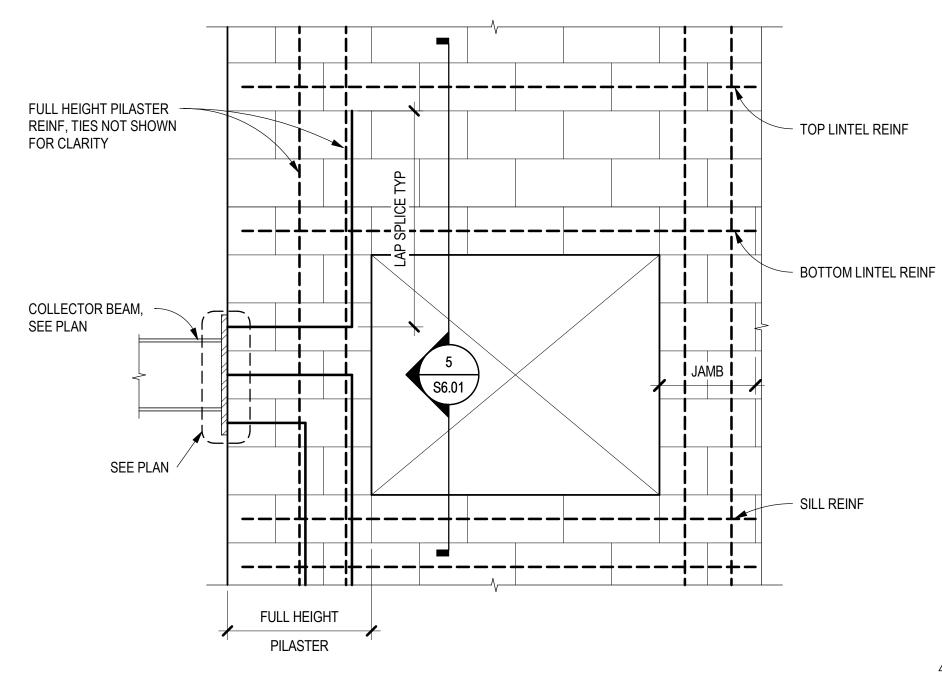
PROJECT PHASE: **BID PACKAGE 01**

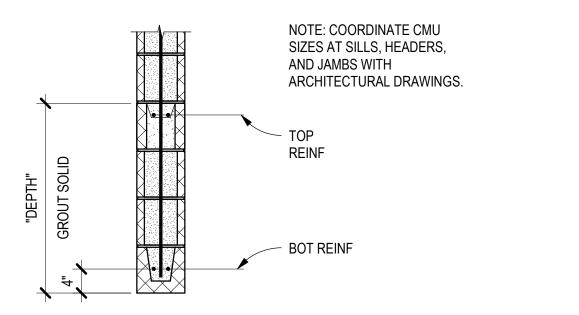
11-01-19 18-01.01 SHEET NUMBER:

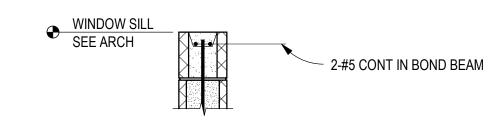
S5.53

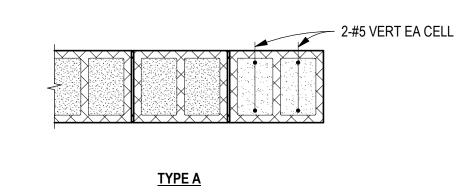
STEEL DETAILS

CMU LINTEL SCHEDULE								
ODENING WIDTH	MIDTH	DEPTH	LINTEL RE	INFORCING	S	ILL	LINTEL JAMB TYPE	
OPENING WIDTH	WIDTH		TOP	BOTTOM	DEPTH	REINFORCING		
0' - 0" - 8' - 0"	12"	32"	2 - #5	2 - #5	8"	2 - #5	TYPE A	
_								









NOTE: SEE TYPICAL CMU PLAN DETAILS SHEET S7.21 FOR TYPICAL HOIRZONTAL REINFORCING REQUIREMENTS.

									DECK SCHEDULE				
	COMPOSITE			SLAB		METAL	DECK			DECK ATTACHMENTS		TOTAL SLAB / DECK	
MARK	SLAB	THICK	MATL	REINF	THICK	TYPE	GAGE	FINISH	ATTACH PERP TO RIBS	ATTACH PARALLEL TO RIBS	ATTACH SIDELAPS	THICKNESS	COMMENTS
D1.5R					1 1/2"	HSB	20	GALVANIZED	7-5/8 "DIA PUDDLE WELDS PER 36 "WIDE SHEET	5/8" DIA PUDDLE WELDS @ 6" OC	#10 SCREWS @ 9" OC	1 1/2"	
D6F	X	3"	NW CONC	6x6 - W2.1xW2.1 WELD WIRE FABRIC IN FLAT SHEETS	3"	VLI	18	GALVANIZED	4-5/8 "DIA PUDDLE WELDS PER 36 "WIDE SHEET	5/8" DIA PUDDLE WELDS @ 12" OC	#10 SCREWS @ 9" OC	6"	

	SLAB-ON-GRADE SCHEDULE										
	SLAB										
MARK	THICKNESS	MATL	REINFORCING	BEARING STRATA	COMMENTS						
S5	5"	CONC	#4 @ 18" OC EA WAY	15 MIL VAPOR RETARDER OVER 1/2" SAND BLOTTER LAYER OVER 4" COMPACTED GRANULAR FILL OVER 14" OF COMPACTED STRUCTURAL FILL OVER COMPACTED SUBGRADE. SUBGRADE WILL BE PLACED WITH LASER LEVEL	PREPARE SUBGRADE AND STRUCTURAL FILL PER GEOTECHNICA REPORT						
S6	6"	CONC	#4 @ 12" OC EA WAY	15 MIL VAPOR RETARDER OVER 1/2" SAND BLOTTER LAYER OVER 4" COMPACTED GRANULAR FILL OVER 14" OF COMPACTED STRUCTURAL FILL OVER COMPACTED SUBGRADE. SUBGRADE WILL BE PLACED WITH LASER LEVEL	PREPARE SUBGRADE AND STRUCTURAL FILL PER GEOTECHNICA REPORT						

			BASE PLATE SCHE	DULE		
		BASE P	LATE	ANCHOR BO	OLTS	
	MARK	TYPE	SIZE	F1554	TYPF	
	BP1		"T"x"A"x"B"	ANCHOR BOLTS		
		A	PL 1 1/4"x18"x1'-6"	4 - 3/4" DIA x 9"	GRAVITY	
	BP2 BP3	A A	PL 1 3/4"x18"x1'-6" PL 1 3/4"x20"x1'-8"	4 - 3/4" DIA x 9" 4 - 3/4" DIA x 9"	GRAVITY GRAVITY	
	BP4	A	PL 1 3/4"x22"x1'-10"	6 - 1" DIA x 18"	GRAVITY	
	BP5	A	PL 3/4"x14"x1'-2"	4 - 3/4" DIA x 9"	GRAVITY	
	BP6	В	PL 1 3/4"x26"x1'-8"	6 - 1" DIA x 18"	LATERAL	
	BP7	В	PL 1 3/4"x22"x1'-8"	6 - 1" DIA x 18"	LATERAL	
	BP8	C	PL 1 1/4"x20"x1'-4"	6 - 1" DIA x 18"	LATERAL	
	BP9	D	PL 3/4"x14"x1'-2"	4 - 3/4" DIA x 9"	LATERAL	
٨	BP10	D	PL 1"x18"x1'-6"	8 - 3/4" DIA x 9"	LATERAL	
1	D1 10		121710710	1 0 0/1 Di/(X0	L/ (TETO LE	
				SC	CHED 'B'	
						7
	SCHED 'B'	<u> </u>		2" EQ	EQ	2"
7	0"	0"		11		1
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		- TOP OF			- TOP OF
STD WASHER W/ DBL HEX		BASE PLATE - TOP OF	STD WASHER W/ DBL HEX		BASE PLATE TOP OF
NUTS PER AISC 360-10 TABLE 14-2	EMBED	CONCRETE	NUTS PER AISC 360-10 TABLE 14-2	EMBED	CONCRETE
	SCHED	- STD WASHER W/ DBL HEX NUTS		SCHED	- PLATE WASHER 1/2"x3" SQ W/ DBL HEX NUTS

GRAVITY ANCHOR BOLT

LATERAL ANCHOR BOLT

WALL SCHEDULE									
				REINFORCING					
1ARK	VENEER	WALL	VERTICAL	HORIZONTAL	GRADE	COMMENTS			
WC8		8" CONC	#4 @ 12" OC	#4 @ 12" OC	A615				
VC10		10" CONC	#4 @ 12" OC	#4 @ 12" OC	A615				
VC12	SEE ARCH	12" CONC	#5 @ 12" OC EA FACE	#5 @ 12" OC EA FACE	A615				
VM12	SEE ARCH	12" CMU	#7 @ 16" OC EA FACE	#5 @ 24" OC EA FACE & STD LADDER TYPE JOINT REINF @ 16" OC	A615	GROUT ALL CELLS SOLID. SEE 7.21 FOR MASONRY DETAILS. D4/S7.21 FOR HORIZONTAL REINFORCEMENT LOCATION			

R	EQUIRED L	AP SPLICES	S ACI318-14	/ IBC 2015				
REINFORCEMENT TYPE	#6 AND SMALLER (#db)			#7 TH	ROUGH #11	(#db)	MINIMUM LENGTH (IN)	COMMENTS
	3000PSI	4000PSI	5000PSI	3000PSI	4000PSI	5000PSI		
CONTINUOUS WALL FOOTINGS AND HORIZONTAL REINFORCEMENT IN SITE WALLS	30	30	30	30	30	30	18	
CONCRETE WALLS: ALL VERTICAL REINFORCEMENT	44	38	34	55	48	43	12	
CONCRETE WALLS: ALL HORIZONTAL REINFORCEMENT, EXCLUDING SITE WALLS AND STEMWALLS	57	50	45	72	62	56	12	
CONCRETE COLUMNS	44	38	34	55	48	43	12	
TOP FLEXURAL REINFORCEMENT, INCLUDING BEAMS, GRADE BEAMS, AND COMBINED COLUMN FOOTINGS	57	50	45	72	62	56	12	
BOTTOM FLEXURAL REINFORCEMENT, INCLUDING BEAMS, GRADE BEAMS, AND COMBINED COLUMN FOOTINGS	44	38	34	55	48	43	12	
MINIMUM EMBEDMENT OF STANDARD HOOKS INTO CONCRETE BASE	22	19	17	22	19	17	6	ALLOWED FOR BARS LARGER THAN #1
SLABS ON GRADE	30	30	30	30	30	30	12	
SLABS ON METAL DECK	30	30	30	30	30	30	12	WWF MINIMUM LAP LENGTH = 6 IN

LAP SPLICES SHALL NOT BE PERMITTED FOR BARS LARGER THAN #11.

LAP SPLICES FOR BUNDLED BARS SHALL BE IN ACCORDANCE WITH ACI 318-14 SECTION 25.5.1.4

LAP LENGTHS FOR LIGHTWEIGHT CONCRETE SHALL BE INCREASED BY 33%

LAP LENGTHS FOR EPOXY COATED BARS SHALL BE INCREASED BY 50% FOR INTERMEDIATE OR LARGER VALUES OF F'c, USE THE CLOSEST LOWER VALUE IN THE TABLE. DO NOT INTERPOLATE

MASONRY LAP SPLICES (#db) ACI 530-13/ IBC 2015									
	#3	#4	#5	#6	#7	#8	#9		
6" BLOCK WITH 1-LAYER OF REINFORCEMENT	32	40	51	72	N/A	N/A	N/A		
8" BLOCK WITH 1-LAYER OF REINFORCEMENT	32	29	36	58	68	72	N/A		
8" BLOCK WITH 2-LAYERS OF REINFORCEMENT	51	68	72	72	N/A	N/A	N/A		
12" BLOCK WITH 1-LAYER OF REINFORCEMENT	32	24	23	37	43	57	65		
12" BLOCK WITH 2-LAYERS OF REINFORCEMENT	51	68	72	72	72	N/A	N/A		
16" BLOCK WITH 1-LAYER OF REINFORCEMENT	32	24	23	30	32	42	48		
16" BLOCK WITH 2-LAYERS OF REINFORCEMENT	51	68	72	72	72	72	72		
BARS LARGER THAN #9 SHALL BE SPLICED US	ING MI	ECHAN	ICAL C	ONNE	CTIONS	3			

			CONTINU	JOUS FOOTING SCHEDUL	E
	SI	ZE	REIN	FORCING	
MARK	WIDTH	DEPTH	CONTINUOUS	TRANSVERSE	COMMENTS
CE16	1'-4"	1'-0"	3-#4	#4 @ 48" OC	
CF18	1' - 6"	1' - 0"	3 - #4	#4 @ 48" OC	1
CF24	2' - 0"	1' - 0"	3 - #4	#4 @ 48" OC	7
CF24B	2' - 0"	1' - 0"	3 - #4 TOP & BOT	#4 @ 48" OC TOP & BOT	3
CF36	3' - 0"	1' - 0"	4 - #4	#4 @ 24" OC	1
CF48	4'0"	1' - 3"	4 - #5 TOP & BOT	#5 @12" OC TOP & BOT)
CF84	7'-0"	2-9	8-#8	#8 @ 9" OC	TOP BARS TO HAVE STD HOOKS AT ENDS

SPOT FOOTING SCHEDULE

6' - 0"

REINFORCING

7' - 0" 2' - 0" 9 - #6 EA WAY TOP & BOT A615 TOP BARS TO HAVE STD

HOOKS AT ENDS

HOOKS AT ENDS

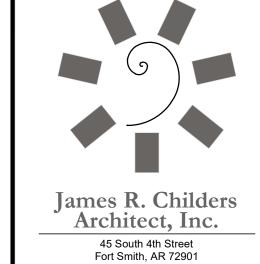
HOOKS AT ENDS

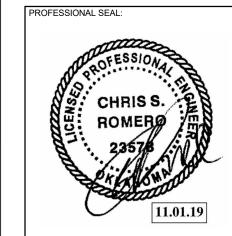
1' - 6" 6 - #6 EA WAY BOT A615 6' - 0" 6' - 0" 2' - 0" 8 - #6 EA WAY TOP & BOT A615 TOP BARS TO HAVE STD

7' - 0" 7' - 0" 2' - 0" 9 - #6 EA WAY TOP & BOT A615 TOP BARS TO HAVE STD HOOKS AT ENDS

8' - 0" 8' - 0" 2' - 0" 7 - #7 EA WAY BOT A615 8' - 0" 8' - 0" 2' - 0" 7 - #7 EA WAY TOP & BOT A615 TOP BARS TO HAVE STD

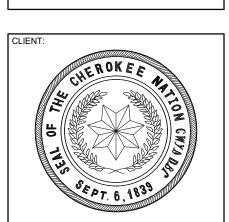
F276 23' - 0" 21' - 0" 2' - 9" #8 @ 9" OC EA WAY TOP & A615 TOP BARS TO HAVE STD

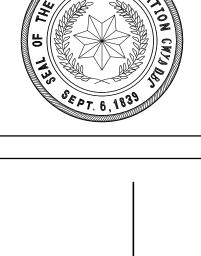


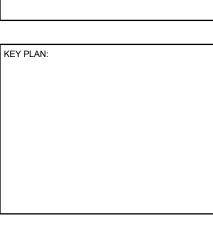


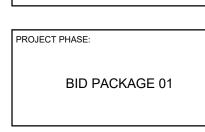
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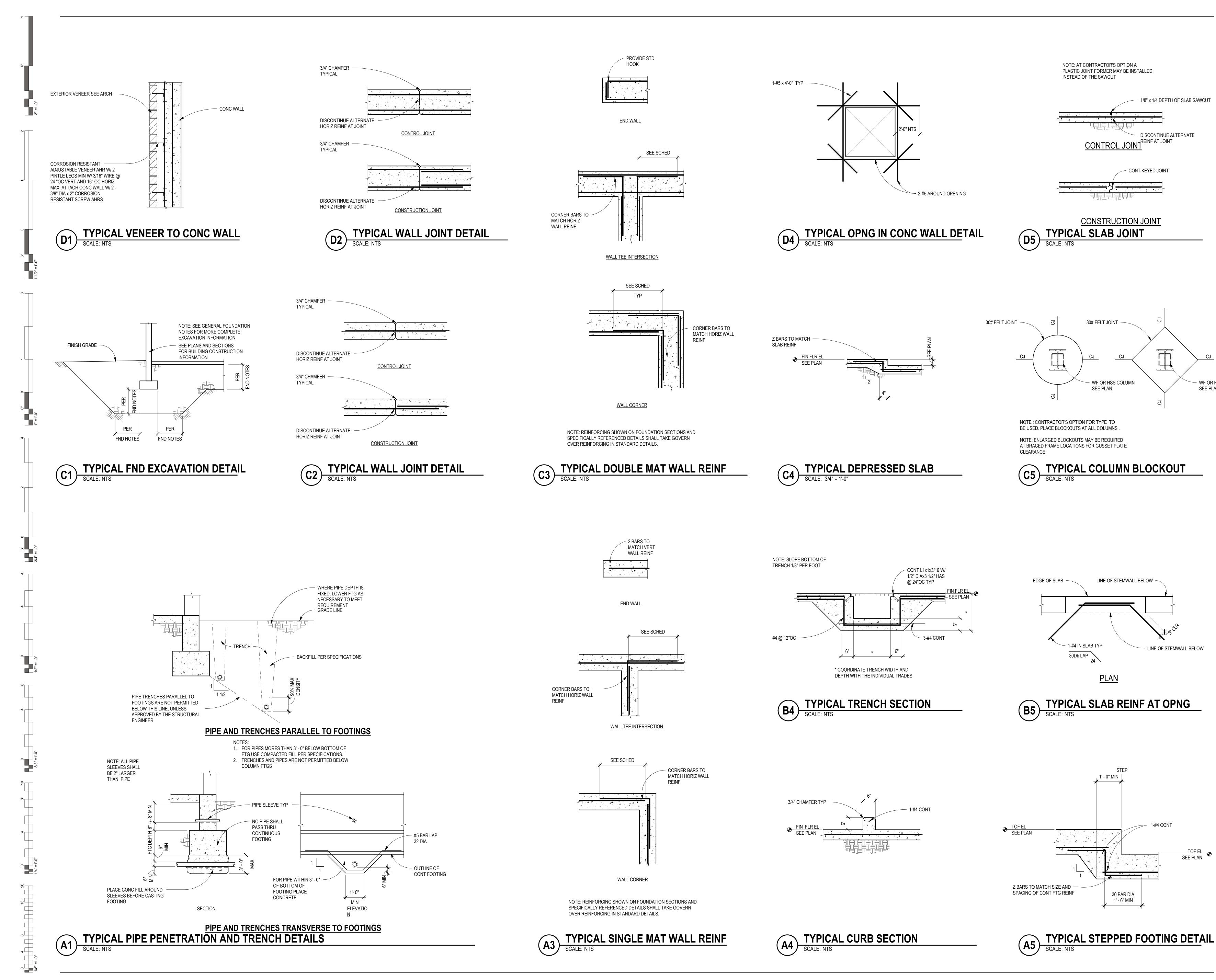


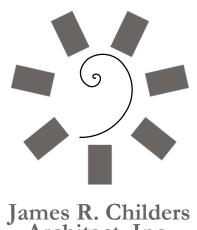


	REVISIONS							
#	DATE	DESCRIPTION						
1	11/22/19	BID PACKAGE 01 - ADD 01						
6	04/16/20	BID PACKAGE 01 - ASI 03						

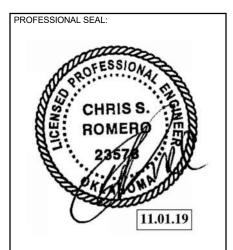
E:	JOB NUMBER:								
1/01/19	18-01.01								
ET NUMBER:									
S6.01									

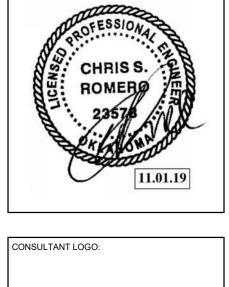
SCHEDULES



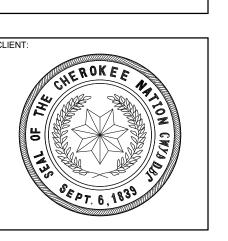


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WF OR HSS COLUMN

SEE PLAN

ANKILLER HEAL EXPANSION

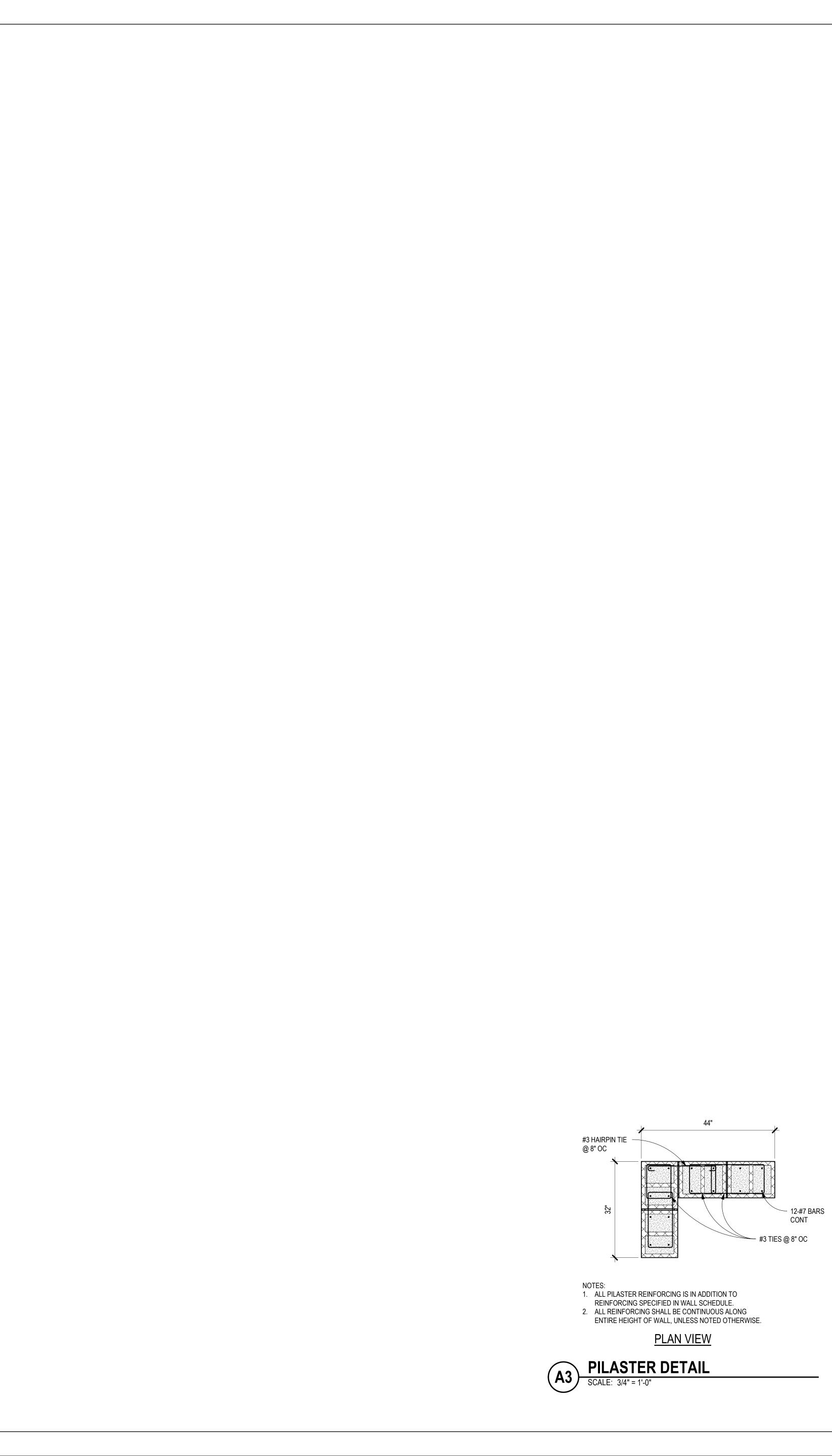
PROJECT PHASE: **BID PACKAGE 01**

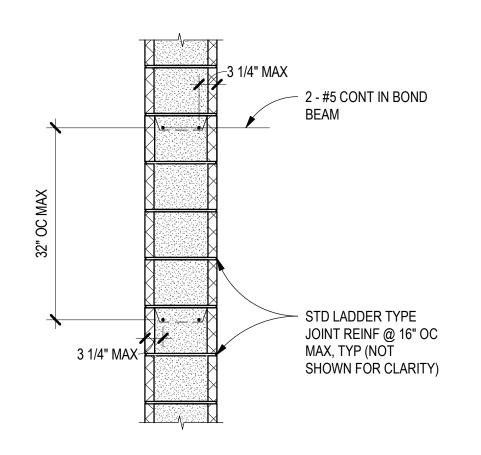
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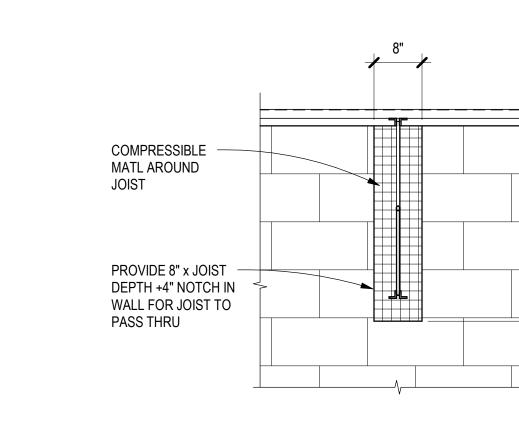
11-01-19 18-01.01 SHEET NUMBER:

S7.11

TYPICAL CONCRETE **DETAILS**



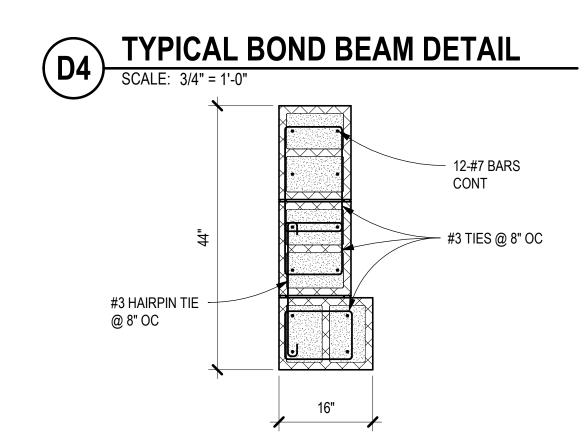




EXTERIOR

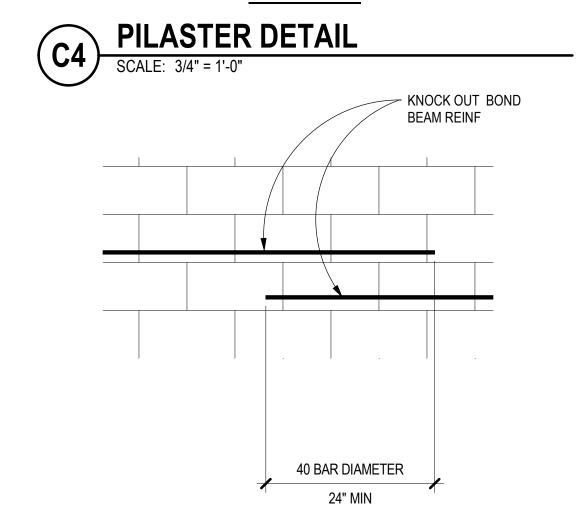
VENEER SEE ARCH

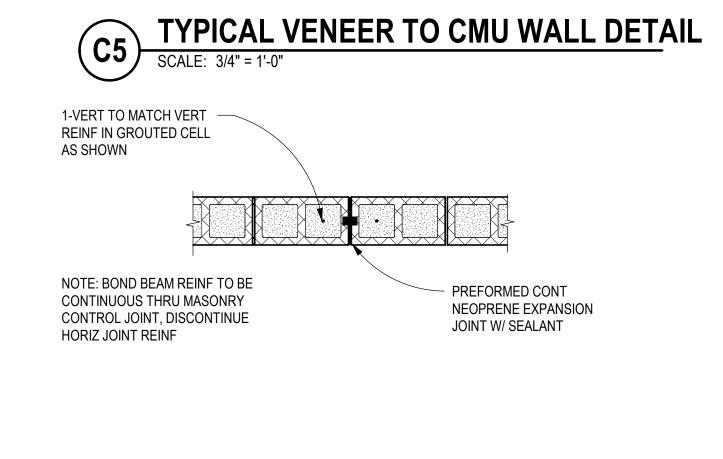
CORROSION RESISTANT ADJUSTABLE INTEGRAL VENEER AHR W/ 2 PINTLE LEGS MIN W/ 3/16" WIRE @ 16" OC VERT AND 24" OC HORIZ OR APPROVED



1. ALL PILASTER REINFORCING IS IN ADDITION TO REINFORCING SPECIFIED IN WALL SCHEDULE. 2. ALL REINFORCING SHALL BE CONTINUOUS ALONG ENTIRE HEIGHT OF WALL, UNLESS NOTED OTHERWISE.

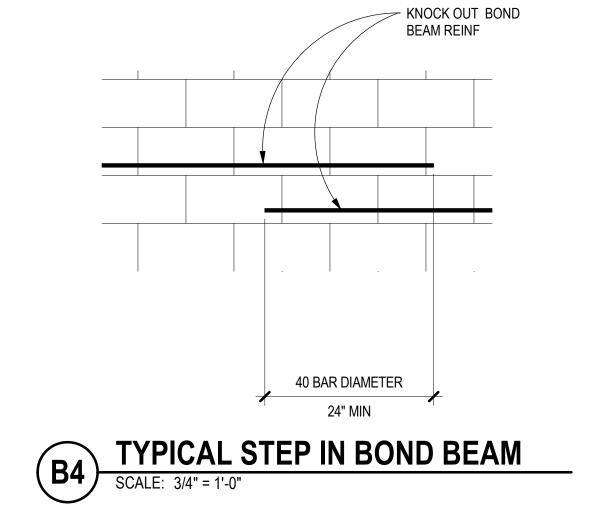
PLAN VIEW

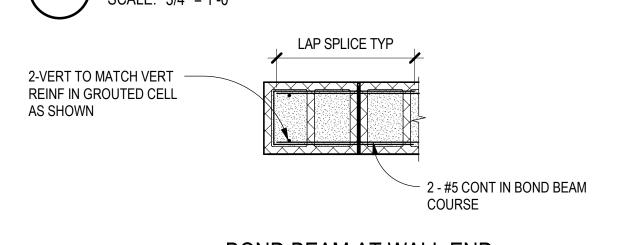


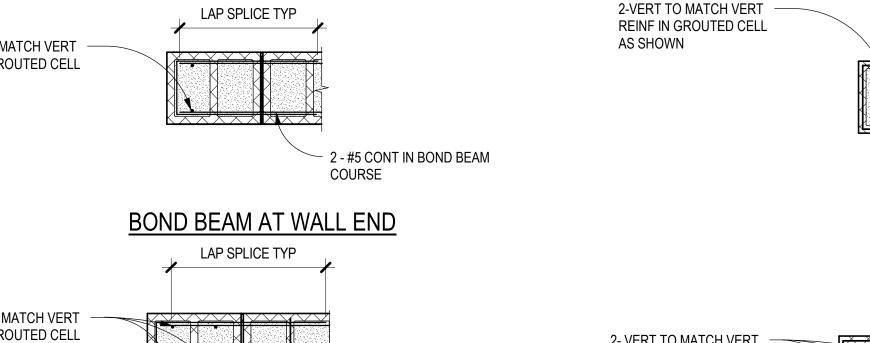


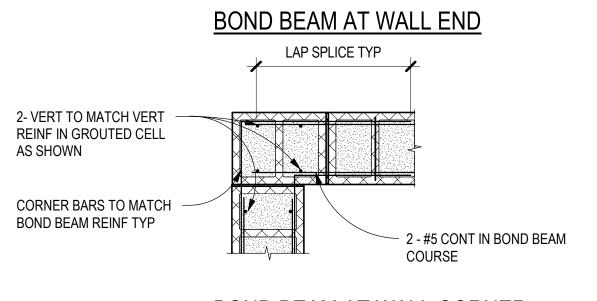
TYPICAL JOIST THRU CMU WALL

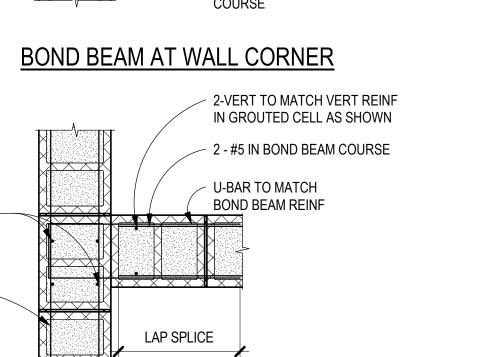
SCALE: 3/4" = 1'-0"

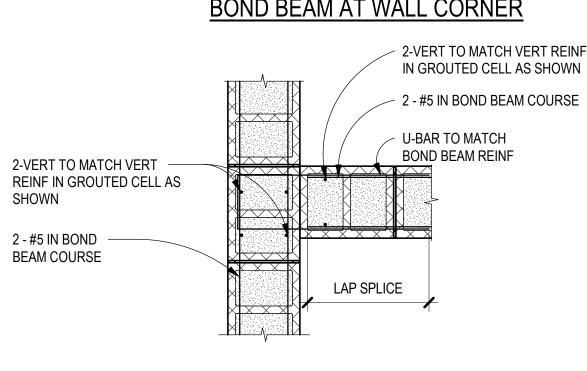


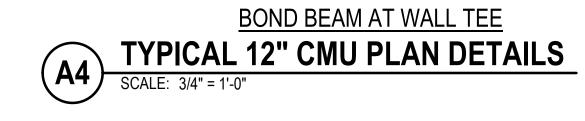


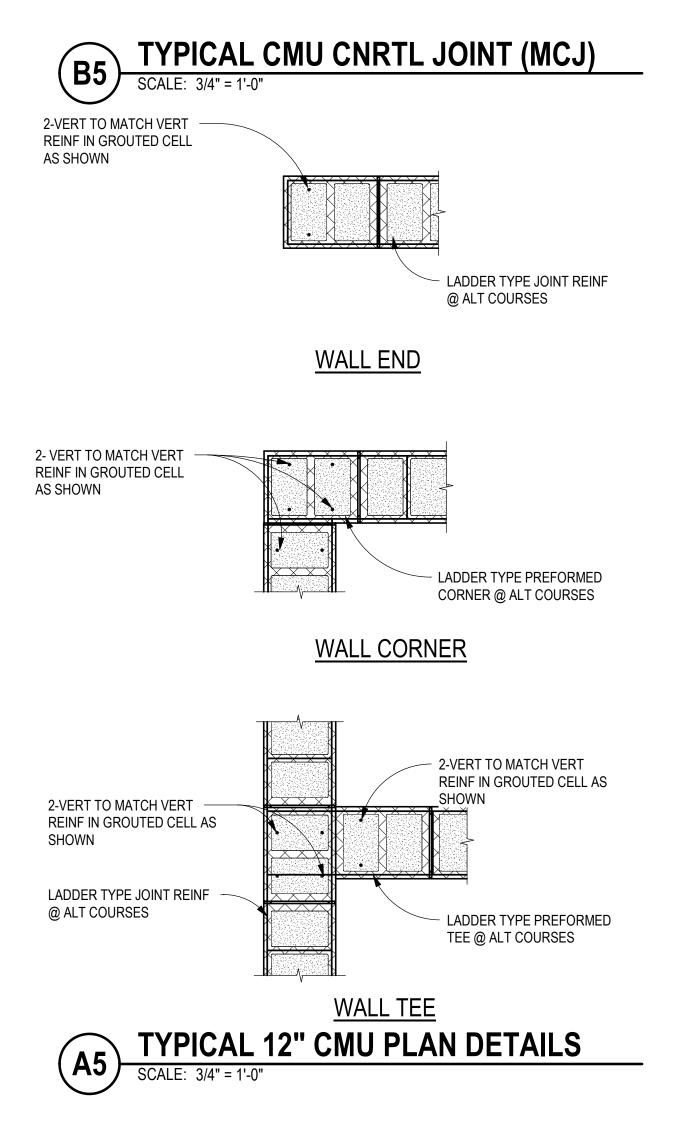


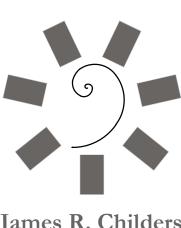




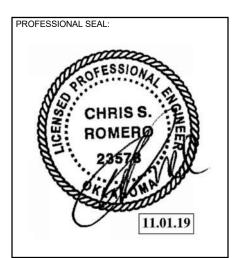


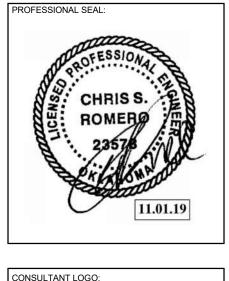




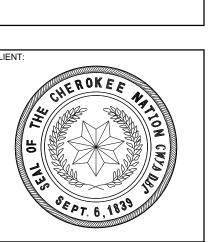


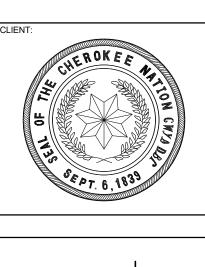
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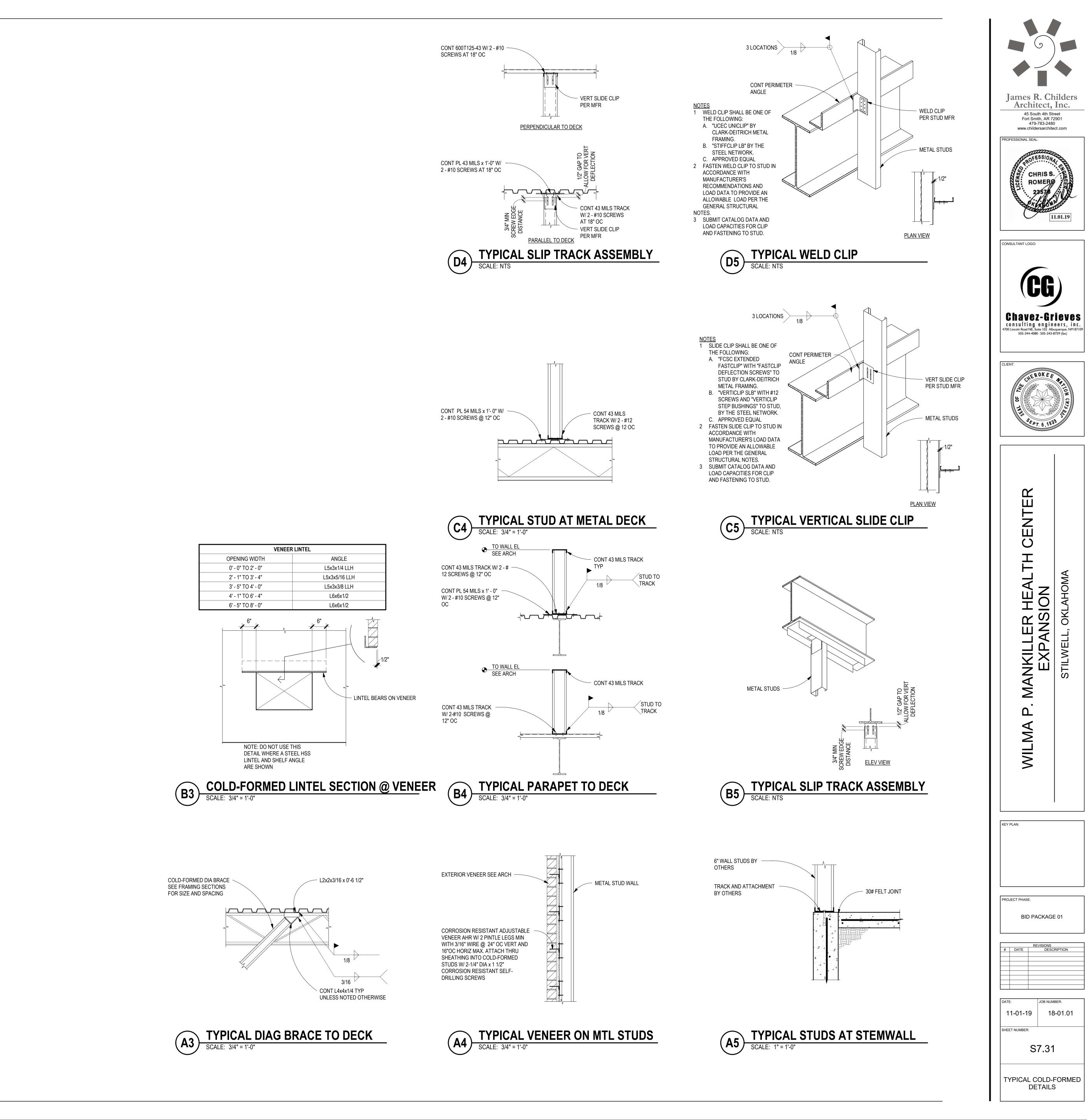


MANKILLER HEAL EXPANSION

PROJECT PHASE: BID PACKAGE 01

11-01-19 18-01.01 SHEET NUMBER: S7.21

TYPICAL MASONRY **DETAILS**



0 4 4 1/8" = 1'-0"

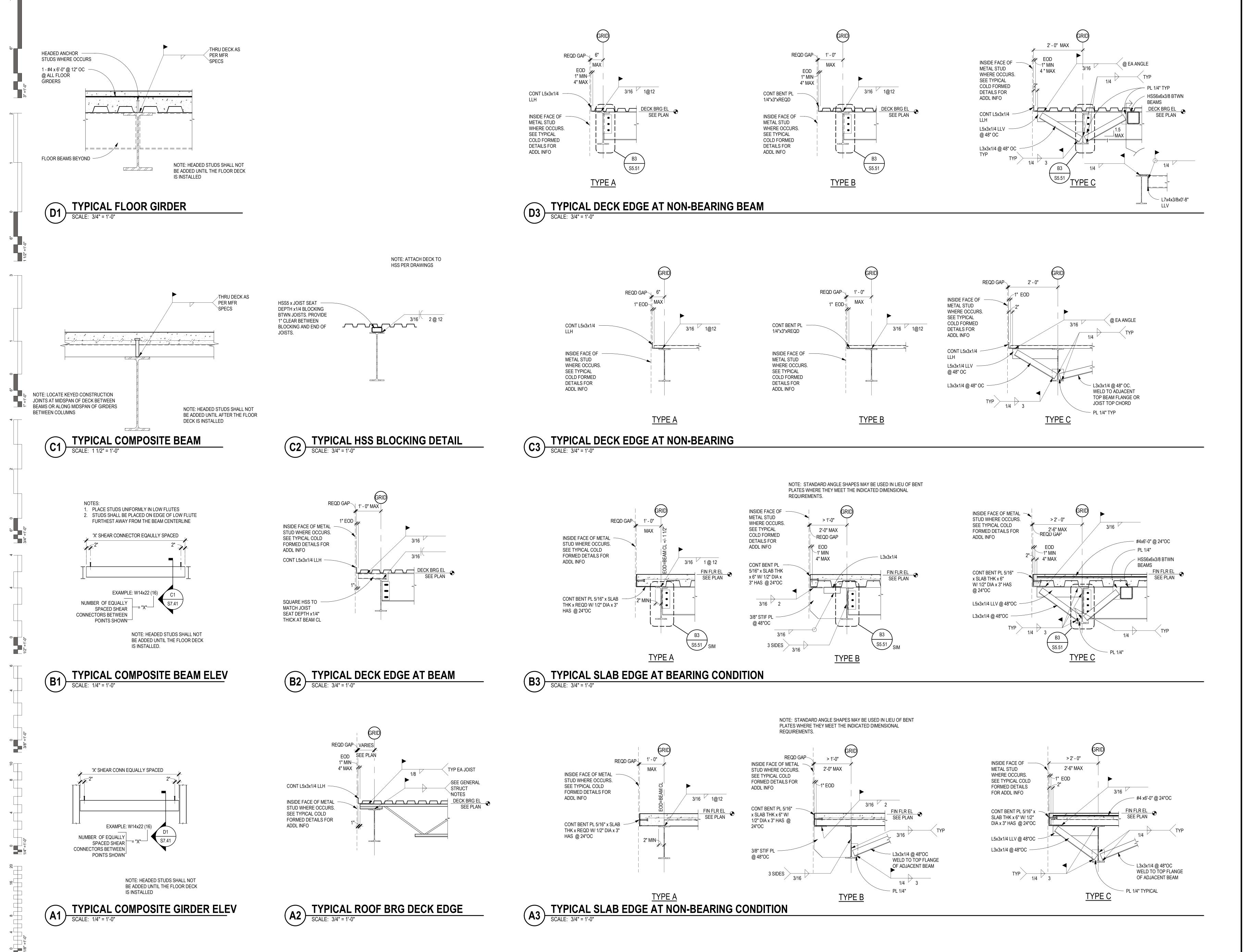
45 South 4th Street

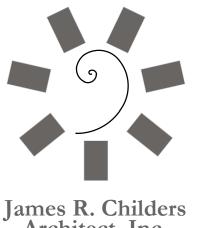
Fort Smith, AR 72901

18-01.01

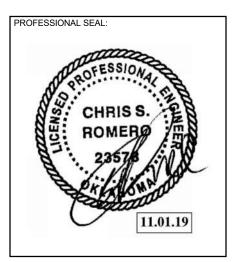
S7.31

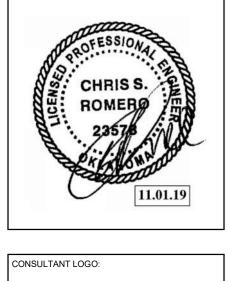
DETAILS



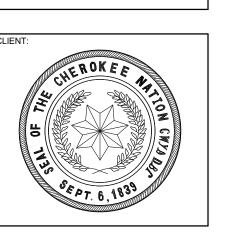


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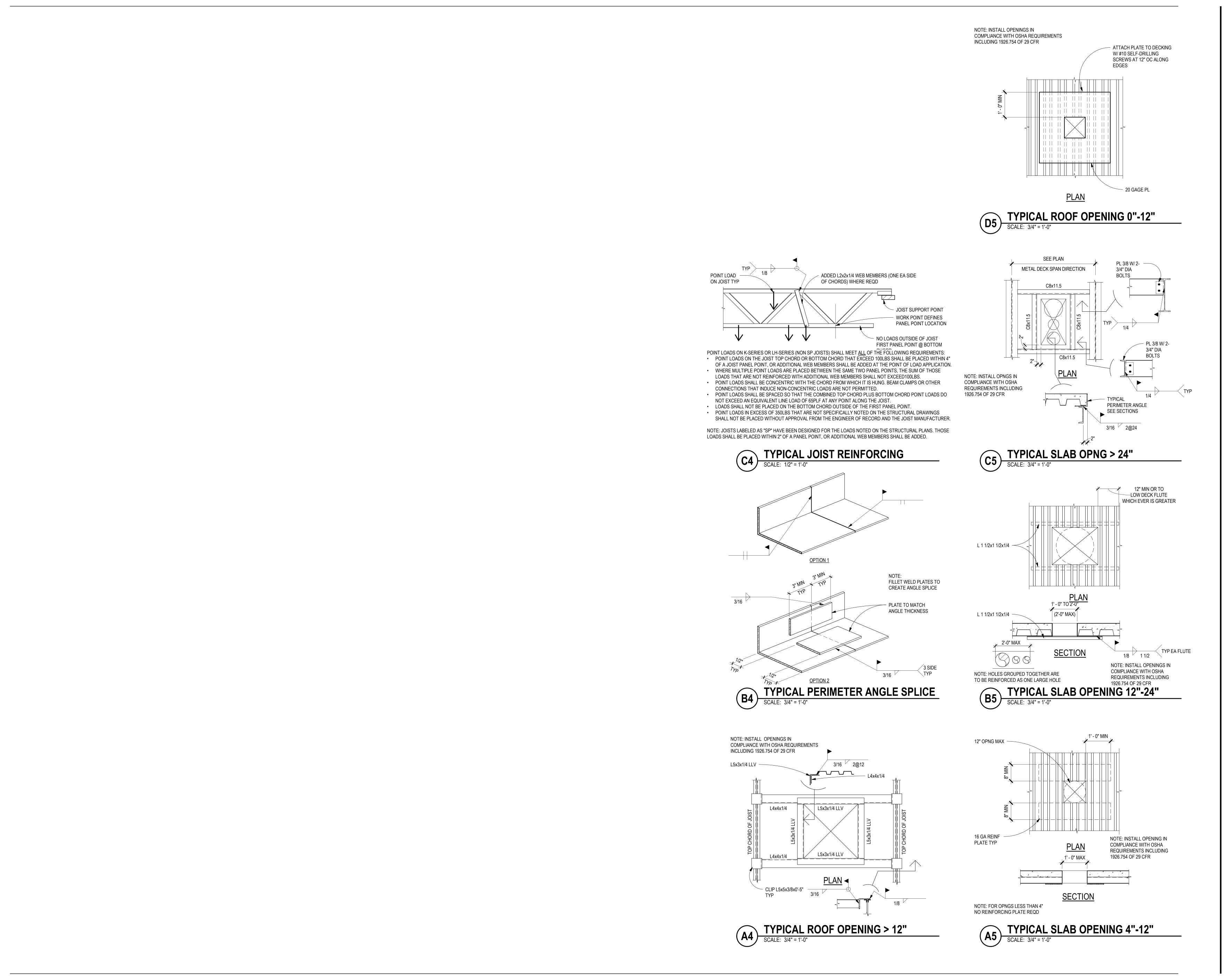
ANKILLER HEAL EXPANSION

PROJECT PHASE: **BID PACKAGE 01**

11-01-19 18-01.01 SHEET NUMBER:

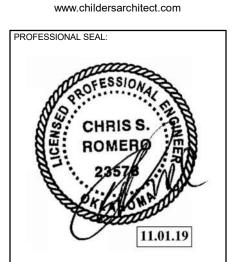
S7.41

TYPICAL STEEL **DETAILS**



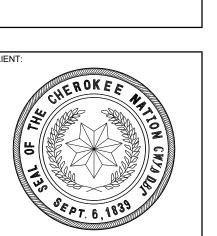


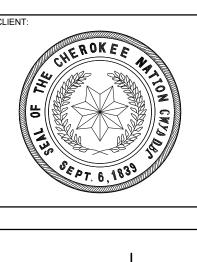
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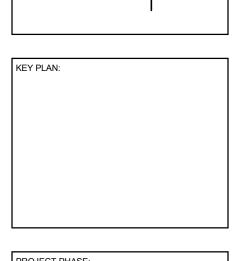








ANKILLER HEAL EXPANSION



PROJECT PHASE: BID PACKAGE 01 REVISIONS
DATE DESCRIPTION

#	DATE	DESCRIPTION	
			1
			7
			7
DATE	Ē:	JOB NUMBER:	7
			1
			-

11-01-19 18-01.01 SHEET NUMBER: S7.42

TYPICAL STEEL DETAILS

LOCATION: **CONNECTION TYPE:** SIDEPLATE NUMBER OF BUILDINGS:

APPROX. TOTAL GROSS SQUARE FOOTAGE: 104714 NUMBER OF STORIES:

a. THERE MAY BE eDATA AVAILABLE FOR YOUR PROJECT WHICH IS AVAILABLE FOR DOWNLOAD AT WWW.SIDEPLATE.COM. eDATA MAY

 eSTIMATE FILE IN EXCEL FORMAT FOR USE IN AFFIRMING SIDEPLATE CONNECTION MATERIAL QUANTITIES. • ComponentXML FILE FOR USE IN ASSISTING DETAILING EFFORTS. b. ESTIMATED NUMBER OF SIDEPLATE JOINTS FOR THIS PROJECT = 54

c. ESTIMATED NUMBER OF SIDEPLATE JOINTS FOR THIS PROJECT THAT ARE **NOT** SUPPORTED BY eDATA = (10)/1 d. MISCELLANEOUS DETAILS, TYPICALLY DESIGNATED BY M#, ARE NOT SUPPORTED.

INSTRUCTIONS TO STEEL FABRICATOR

a. THE STEEL FABRICATOR'S BID PRICE FOR PROCUREMENT, FABRICATION AND ERECTION OF STRUCTURAL AND MISCELLANEOUS STEEL SHALL INCLUDE THE SIDEPLATE LICENSE FEE FOR THE PROJECT. EACH PROSPECTIVE STEEL FABRICATOR WHO BIDS THE PROJECT SHALL FORMALLY REQUEST THE SIDEPLATE LICENSE FEE BY ACCESSING THE SIDEPLATE WEBSITE (http://www.sideplate.com) b. UPON THE SUCCESSFUL STEEL FABRICATOR SIGNING A CONTRACT TO FABRICATE STRUCTURAL STEEL FOR THIS PROJECT, THE STEEL FABRICATOR SHALL SUBMIT A PURCHASE ORDER (PO) TO SIDEPLATE SYSTEMS, INC. FOR THE TOTAL AMOUNT OF THE SIDEPLATE LICENSE

FEE AND SHALL INCLUDE SAID FEE IN ITS FIRST CONSTRUCTION DRAW. c. THE STEEL FABRICATOR SHALL MAKE PAYMENT OF THE SIDEPLATE LICENSE FEE DIRECTLY TO:

SIDEPLATE SYSTEMS, INC. 25909 PALA, SUITE 200 MISSION VIEJO, CA 92691 TEL: 949-238-8900

SUBMITTALS

1. IN ADDITION TO THE REQUIRED SUBMITTALS SPECIFIED BY THE BALANCE OF THE CONTRACT DOCUMENTS, THE FOLLOWING SUBMITTALS SHALL BE SENT TO SIDEPLATE SYSTEMS, INC. ELECTRONICALLY VIA THE STRUCTURAL ENGINEER OF RECORD FOR THEIR REVIEW AND DISPOSITION: a. QUALITY CONTROL PROGRAM (REQUIRED IF NOT AISC CERTIFIED) b. ONE ELECTRONIC COPY OF ALL STRUCTURAL STEEL DRAWINGS THAT EITHER DIRECTLY PERTAINS TO AND/OR AFFECTS THE SHOP FABRICATION OR FIELD ERECTION OF THE SIDEPLATE STEEL FRAME CONNECTION SYSTEM, INCLUDING THE INITIAL SUBMITTAL AND ALL CORRECTED RE-SUBMITTALS OF AFFECTED DRAWINGS. SIDEPLATE SYSTEMS, INC. SHALL BE GIVEN, AS A MINIMUM, THE SAME SPECIFIED REVIEW TIME (NOT LESS THAN SEVEN BUSINESS DAYS) AS THE ENGINEER OF RECORD.

1. PRE-DETAILING MEETING

a. PRIOR TO THE START OF DETAILING OF THE SHOP DRAWINGS, THE FABRICATION CONTRACTOR SHALL FORMALLY REQUEST A PRE-DETAILING MEETING FROM SIDEPLATE SYSTEMS, INC. THIS MEETING IS TYPICALLY A WEBINAR TO DISCUSS BEST PRACTICES FOR THE DETAILING OF THE SIDEPLATE CONNECTIONS, AND TO CREATE A PROACTIVE FORUM TO ANSWER ANY QUESTIONS.

a. PRIOR TO THE START OF FABRICATION, THE FABRICATION CONTRACTOR SHALL FORMALLY REQUEST A PRE-FABRICATION MEETING FROM SIDEPLATE SYSTEMS, INC. THIS MEETING IS TYPICALLY A WEBINAR TO DISCUSS BEST PRACTICES FOR THE FABRICATION OF THE SIDEPLATE CONNECTIONS, AND TO CREATE A PROACTIVE FORUM TO ANSWER ANY QUESTIONS.

a. PRIOR TO THE START OF STEEL ERECTION, THE ERECTION CONTRACTOR SHALL FORMALLY REQUEST A PRE-ERECTION MEETING FROM SIDEPLATE SYSTEMS, INC. THIS MEETING IS TYPICALLY A WEBINAR TO DISCUSS BEST PRACTICES FOR FIELD ERECTION OF THE SIDEPLATE BEAMS AND COLUMNS, AND TO CREATE A PROACTIVE FORUM TO ANSWER ANY QUESTIONS.

BELOW:

1. THE GOVERNING CODES SHALL CONSIST OF ANSI/AWS D1.1-2010 (AWS D1.1), AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES (APRIL 14, 2010), 2009 RCSC SPECIFICATIONS FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS, AND ALL APPLICABLE BUILDING AND JURISDICTIONAL CODES AND PROJECT STANDARDS SPECIFIED IN THE PROJECT SPECIFICATION STRUCTURAL STEEL SECTION. WHERE THE REQUIREMENTS DIFFER BETWEEN SIDEPLATE CONNECTION NOTES, THE GENERAL STRUCTURAL NOTES, AND THE GOVERNING CODES, THE MORE STRINGENT SECTION CRITERIA SHALL CONTROL. . ALPHA AND NUMERIC DESIGNATORS {X} & {#} USED HEREIN TO SIMPLIFY THE IDENTIFICATION OF PLATES, ANGLES, AND WELDS ARE DEFINED

SIDE PLATE FOR UNIAXIAL CONNECTIONS

- BEAM FLANGE COVER PLATE, AS REQUIRED
- VERTICAL SHEAR PLATE OR FLAT BAR, AS REQUIRED
- {D} HORIZONTAL SHEAR PLATE OR FLAT BAR, AS REQUIRED
- VERTICAL ANGLE WELDED TO THE VERTICAL SHEAR PLATE {C}, AS REQUIRED
- VERTICAL SHEAR ELEMENT (VSE) WHICH CONSISTS OF PLATE {C} AND ANGLE {E} MATERIAL, AS REQUIRED
- {G} LONGITUDINAL ANGLE WELDED TO THE OUTSIDE FACE OF SIDE PLATE {A}, AS REQUIRED
- LONGITUDINAL ANGLE WELDED TO THE BOTTOM BEAM FLANGE (OR TOP BEAM FLANGE AS REQUIRED)
- HORIZONTAL PLATE WELDED TO THE OUTSIDE FACE OF SIDE PLATE {A}, AS REQUIRED
- FILLET WELD CONNECTING SIDE PLATE {A} TO HORIZONTAL SHEAR PLATE {D} OR COLUMN
- FILLET (AND/OR FLARE BEVEL) WELD CONNECTING INSIDE FACE OF SIDE PLATE {A} TO COLUMN
- FILLET WELD CONNECTING HORIZONTAL SHEAR PLATE {D} TO COLUMN, AS REQUIRED FILLET WELD TO CONSTRUCT VSE {F} AND TO CONNECT IT TO THE WEB OF THE BEAM, AS REQUIRED
- FILLET (AND/OR PJP) WELD CONNECTING BEAM FLANGE TIPS TO COVER PLATE {B} AND/OR LONGITUDINAL ANGLE {H}, AS REQUIRED
- {5a} FILLET WELD CONNECTING OUTSIDE FACE OF BEAM FLANGE TO COVER PLATE {B} AND/OR LONGITUDINAL ANGLE {H}, AS REQUIRED
- {5b} FILLET WELD CONNECTING COVER PLATE {B} EDGE TO TOP FACE OF BEAM FLANGE, ACROSS ITS WIDTH
- ({5p}) PJP WELD CONNECTING ANGLE {H} TO BEVELED BEAM FLANGE
- FILLET (AND/OR PJP) WELD CONNECTING LONGITUDINAL ANGLE {G} (AND/OR PLATE {T}) TO SIDE PLATE {A}, AS REQUIRED
- ({8p}) PJP WELD CONNECTING PLATE {T} TO SIDE PLATE {A} AND/OR CONNECTING BUILT UP ANGLE {H} PLATES TOGETHER, AS REQUIRED
- FILLET WELD CONNECTING SIDE PLATE {A} TO COLUMN FACE, WRAPPED AROUND THREE SIDES OF SIDE PLATE {A}
- {10} FILLET WELD TO CONSTRUCT SIDE PLATE SLOTTED INTERLOCK ASSEMBLY
- {10p} PJP WELD TO CONSTRUCT SIDE PLATE SLOTTED INTERLOCK ASSEMBLY
- {10r} REINFORCING FILLET WELD TO CONSTRUCT SIDE PLATE SLOTTED INTERLOCK ASSEMBLY

3. ALPHA DESIGNATORS, USED HEREIN TO SIMPLIFY THE IDENTIFICATION OF DIMENSIONS OF THE SIDEPLATE CONNECTIONS, ARE DEFINED BELOW:

- GAP PHYSICAL SEPARATION BETWEEN THE END OF THE MOMENT FRAME BEAM AND THE ADJOINING FACE OF THE COLUMN FLANGE DEPTH OF SIDE PLATE {A}
- C LENGTH OF COVER PLATE {B} AND/OR LONGITUDINAL ANGLE {H}
- LENGTH OF SLOT FROM THE TOE OF THE RADIUS IN THE COVER PLATE {B}, AS REQUIRED
- EDGE DISTANCE OF BOLT HOLES IN COVER PLATE {B}, AS REQUIRED
- GAGE DISTANCE TO CENTERLINE OF BOLT HOLES IN ANGLES (G) AND (H), AND PLATE (T), AS REQUIRED
- ADDED DIMENSION TO COLUMN FLANGE WIDTH TO DEFINE TOTAL COVER PLATE {B} WIDTH
- DISTANCE FROM END OF THE BEAM TO CENTERLINE OF VERTICAL BOLT HOLES IN VSE {F}, AS REQUIRED
- RADIUS OF SLOT DIMENSION IN COVER PLATE {B}
- HORIZONTAL SPACING BETWEEN BOLT HOLES
- ADDED DIMENSION TO COLUMN FLANGE WIDTH FOR ALLOWABLE SPREAD OF SIDE PLATES {A}

1. PLATE, FLAT BAR, AND ANGLE MATERIAL

- a. ALL PLATE MATERIAL SHALL HAVE A MINIMUM YIELD STRENGTH (Fy) OF 50 KSI. b. ANGLE AND BAR MATERIAL SHALL HAVE A HIGH STRENGTH STEEL SPECIFICATION AND SHALL HAVE A MINIMUM YIELD STRENGTH (Fy) OF 50
- a. BOLTS SHALL BE TYPE 1 OR TYPE 3 AND SHALL BE A490-X HEAVY HEX, F2280 TWIST-OFF-TYPE TENSION-CONTROL BOLT ASSEMBLIES, OR F3148 FIXED SPLINE BOLT ASSEMBLIES. THE BOLT HEAD SHALL BE DISTINCTIVELY MARKED WITH A MINIMUM MARKING OF A490, A490TC, OR 144 RESPECTIVELY. AN ALTERNATIVE DESIGN THAT MEETS THE REQUIREMENTS OF RCSC SECTION 2.8 MAY BE USED, WITH THE WRITTEN APPROVAL FROM SIDEPLATE SYSTEMS, INC. b. WASHERS SHALL BE ORDINARY THICKNESS AND ASTM F436 TYPE 1 OR TYPE 3. c. NUTS SHALL BE ASTM A563 GRADE DH OR DH3.
- THE BOLT ASSEMBLY SHALL BE COVERED IN A LIGHT PROTECTIVE OIL. F2280 AND F3148 ASSEMBLIES SHALL ONLY BE LUBRICATED BY THE e. THE MILL TEST REPORT (MTR) MUST HAVE DOCUMENTED LOT TRACEABILITY, STATEMENT OF DIMENSIONAL RESULTS, FULL CHEMICAL AND MECHANICAL TEST RESULTS TO THE SPECIFICATIONS ABOVE.
- THE USE OF FINGER SHIMS ARE ACCEPTABLE PER BOLTING SECTION 8. a. ALL ROLLED SHAPES USED FOR COLUMNS AND BEAMS IN CONSTRUCTING SIDEPLATE MOMENT FRAMES SHALL BE ASTM A992 GRADE 50

. HSS TUBE SHAPES a. ALL HSS SHAPES USED FOR COLUMNS AND BEAMS IN CONSTRUCTING SIDEPLATE MOMENT FRAMES SHALL, AS A MINIMUM, BE ASTM A500 GRADE B OR GRADE C OR ASTM1085.

PREPARATION . THE STEEL FABRICATION AND ERECTION SUBCONTRACTORS SHALL EMPLOY A DISTORTION CONTROL PROGRAM PRIOR TO THE START OF SIDEPLATE MOMENT FRAME FABRICATION. THE DISTORTION CONTROL PROGRAM SHALL BE IN ACCORDANCE WITH THE PROVISIONS OF AWS D1.1 SECTION 5.21 AND 5.22 TO ENSURE THAT THE FOLLOWING ARE MAINTAINED:

- DIMENSIONAL ACCURACY FRAMING AND ALIGNMENT TOLERANCES COMPLIANCE WITH AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES, SECTION 7.0, ERECTION PROVISIONS
- CONTROL OF DISTORTION AND WELD SHRINKAGE

. WELDER QUALIFICATION: THE PERFORMANCE OF ALL WELDERS, WELDING OPERATORS AND TACK WELDERS SHALL BE QUALIFIED IN

CONFORMANCE WITH AWS D1.1, SECTION 4, PART C TO DEMONSTRATE ABILITY TO PRODUCE SOUND WELDS.

CALIBRATED WRENCH (A490), TWIST-OFF-TYPE TENSION-CONTROL BOLT (F2280), OR TORQUE AND ANGLE METHOD(F3148). FOR ALL PRETENSIONING MÉTHODOLOGIES. ALL FASTENER ASSEMBLIES WITHIN THE JOINT SHALL FIRST BE BROUGHT TO A SNUG TIGHT

1. BOLTS/FASTENERS SHALL BE INSTALLED TO PRETENSIONED CONDITION USING ONE OF THE METHODS PRESCRIBED HERE: TURN-OF-NUT (A490), CONDITION, FOLLOWED BY A SYSTEMATIC PRETENSIONING PROCESS. PRETENSIONING SHALL BEGIN AT THE MOST RIGID PART OF THE JOINT AND CONTINUE IN A MANNER THAT WILL MINIMIZE THE RELAXATION OF PREVIOUSLY PRETENSIONED FASTENERS. UNTIL THE CONNECTED PLIES ARE IN AS FIRM CONTACT AS POSSIBLE.

- 3. REUSE OF A490, F2280, AND F3148 BOLT ASSEMBLIES SHALL NOT BE ALLOWED. TOUCHING UP OR RE-TIGHTENING BOLTS THAT MAY HAVE BEEN LOOSENED BY THE INSTALLATION OF ADJACENT BOLTS SHALL NOT BE CONSIDERED TO BE A REUSE.
- 4. ALL BOLT HOLES SHALL BE ALIGNED TO PERMIT INSERTION OF THE BOLTS WITHOUT UNDUE DAMAGE TO THE THREADS THE BOLT LENGTH USED SHALL BE SUCH THAT THE BOLT THREAD EXTENDS BEYOND OR IS AT LEAST FLUSH WITH THE OUTER FACE OF THE NUT
- FASTENER COMPONENTS SHALL BE PROTECTED FROM DIRT AND MOISTURE IN CLOSED CONTAINERS AT THE SITE OF INSTALLATION. F2280 OR F3148 ASSEMBLIES AND ALTERNATIVE DESIGN FASTENERS THAT MEET THE SPECIFIED REQUIREMENTS PREVIOUSLY MENTIONED
- FINGER SHIMS MAY BE USED UP TO 1/4 INCH WITHOUT RESTRICTION, SHIM REQUIREMENTS GREATER THAN 1/4 INCH SHALL BE SUBMITTED TO SIDEPLATE SYSTEMS INC FOR APPROVAL PRIOR TO USE. WASHERS SHALL BE ASTM F436 ORDINARY THICKNESS AND SHALL BE USED UNDER THE NUT OF THE FASTENER ASSEMBLY SO AS TO PROVIDE A HARDENED NON-GALLING SURFACE OF THE TURNED ELEMENT. WHEN USING THE TURN-OF-NUT OR CALIBRATED WRENCH METHOD, THE TURNED ELEMENT MUST BE THE SAME AS WAS USED WHEN PERFORMING PREINSTALLATION VERIFICATION TESTING.
- 1. THE FABRICATOR AND ERECTOR SHALL BE RESPONSIBLE FOR QUALITY CONTROL BY PROVIDING, AS A MINIMUM, IN-PROCESS VISUAL INSPECTION OF ALL FABRICATION AND ERECTION ACTIVITIES TO ENSURE THAT MATERIALS AND WORKMANSHIP MEET THE REQUIREMENTS OF THE CONTRACT DOCUMENTS, AND SHALL INCLUDE WORK PERFORMED PRIOR TO ASSEMBLY. SUCH WORK SHALL INCLUDE, BUT NOT BE LIMITED TO, VERIFYING THAT EFFECTIVE PROCEDURES AND METHODS HAVE BEEN EMPLOYED IN THE FORM OF A DISTORTION CONTROL PROGRAM TO ACCOUNT FOR AND COUNTERACT THE EFFECTS OF WELD SHRINKAGE, EXISTING BEAM SWEEP AND CAMBER, AND CHANGES IN MOMENT FRAME GEOMETRY DUE TO SKEWED AND CURVED DESIGN CONFIGURATIONS (AS OCCURS), TO ENSURE COMPLIANCE WITH SPECIFIED ERECTION AND

TOLERANCES (-3/16 INCH MAX), CONTACT SIDEPLATE SYSTEMS, INC FOR APPROPRIATE RECOMMENDATIONS

ALIGNMENT TOLERANCES. QC INSPECTION SHALL INCLUDE **HOLD POINTS** FOR THE FOLLOWING: VERIFICATION THAT ACTUAL COLUMN FLANGE WIDTH IS AT LEAST NOMINAL COLUMN FLANGE WIDTH WHERE THE SIDE PLATES (A) ARE

TO BE INSTALLED. IN THE UNLIKELY EVENT ACTUAL COLUMN FLANGE WIDTH IS LESS THAN NOMINAL, BUT WITHIN AISC STANDARD MILL

- 2. MINIMUM CLEAR DIMENSION SHALL BE VERIFIED AFTER PLACEMENT OF WELD {2}, COOLING OF WELD {2}, AND REMOVAL OF TEMPORARY SHOP CONSTRUCTION AID(S). VERIFY THAT A MINIMUM ACTUAL COLUMN FLANGE WIDTH DIMENSION OCCURS ANYWHERE IN BETWEEN THE SIDE PLATES {A} FROM TOP TO BOTTOM. THE SIDE PLATES SHALL BE PARALLEL TO ONE ANOTHER. IN NO CASE SHALL THEY BE LESS THAN THE ACTUAL COLUMN FLANGE WIDTH. 3. MAXIMUM SPREAD DIMENSION OF SIDE PLATE {A} SHALL NOT EXCEED ACTUAL COLUMN FLANGE WIDTH PLUS THE SCHEDULED SPREAD DIMENSION Y. THE FIELD CONSTRUCTION AID SHALL BE PLACED AND HOLD THE SIDE PLATES IN THIS FLARED CONDITION UNTIL THE BEAM HAS BEEN SAFELY ERECTED. IN NO CASE SHALL THE SPREAD CAUSE PERMANENT DEFORMATION IN THE SIDE PLATES. 4. VERIFICATION OF BOLT HOLE ELEVATION AND SPACING FOR POSITION OF SIDE PLATE (A) AND PROPER POSITION AND ELEVATION OF
- ANGLES {G}. BEAM ASSEMBLY VERIFICATION OF PERPENDICULAR ALIGNMENT BETWEEN THE TOP COVER PLATE (B) AND BOTTOM ANGLES (H) TO THE WEB OF THE BEAM, TO MINIMIZE, IF NOT ELIMINATE, ANY MISALIGNMENT OF BOLT HOLES DUE TO BEAM FLANGE TILT WHEN THE BEAM HAS BEEN 2. VERIFICATION OF BOLT HOLE SPACING AND POSITION ON COVER PLATE (B) AND ANGLES (H). CONSIDERATION SHALL BE GIVEN TO THE CUPPING EFFECT OF THE TOP COVER PLATE (B), DUE TO WELD SHRINKAGE.

3. VERIFICATION OF THE DISTANCE BETWEEN EXTERIOR ANGLE {H} FACES AND THEIR RESPECTIVE BOLT HOLE PLACEMENT TO EACH

4. VERIFICATION THAT IN NO CASE SHALL THE OUTSIDE FACE OF VSE {F} EXTEND BEYOND THE OUTSIDE FACES OF THE LONGITUDINAL 5. VERIFICATION THAT VERTICAL PLACEMENT OF VSE {F} IS IN THE CORRECT LOCATION.

SHALL NOT BE RE-LUBRICATED, EXCEPT BY THE MANUFACTURER.

OTHER (VERTICALLY AND HORIZONTALLY).

- 2. <u>FILLET WELD FIT-UP TOLERANCES</u>:
 a. THE PARTS TO BE JOINED BY FILLET WELDS SHALL BE BROUGHT INTO AS CLOSE CONTACT AS PRACTICABLE, USING AS NECESSARY SUITABLE CLAMPING MEANS. THE ROOT OPENING (I.E., THE FIT-UP GAP) SHALL NOT EXCEED 1/4 INCH. FOR FILLET WELD ROOT GAPS GREATER THAN 1/16 INCH, THE LEG SIZE (I.E., THE SPECIFIED SIZE) OF FILLET WELD SHALL BE INCREASED BY THE AMOUNT OF THE ROOT OPENING.
- a. THE ROUGHNESS OF ALL THERMAL-CUT SURFACES SHALL BE NO GREATER THAN AN ANSI SURFACE ROUGHNESS VALUE OF 1000 MICRO-INCHES. ROUGHNESS EXCEEDING THIS VALUE AND NOTCHES OR GOUGES NOT MORE THAN 3/16 INCH DEEP SHALL BE REMOVED BY MACHINING OR GRINDING. NOTCHES OR GOUGES IN THE THERMALLY CUT EDGES DEEPER THAN 3/16 INCH SHALL BE REPAIRED PER AWS.
- 4. TENSION CALIBRATION FOR PRE-INSTALLATION a. TENSION CALIBRATION SHALL BE USED TO CONFIRM THE SUITABILITY OF THE COMPLETE FASTENER ASSEMBLY, AND THE PROCEDURE TO BE USED BY THE BOLTING CREW.

IN ADDITION TO ALL OTHER QUALITY ASSURANCE INSPECTION ACTIVITIES, THE OWNER'S VERIFICATION INSPECTOR SHALL BE RESPONSIBLE FOR:

a. TO ASSURE THE PROPER AMPERAGE AND VOLTAGE OF THE WELDING PROCESS, THE USE OF HAND HELD CALIBRATED AMP AND VOLT

- METERS SHALL BE USED. THIS EQUIPMENT SHALL BE USED BY THE FABRICATOR AND THE INSPECTOR. AMPERAGE AND VOLTAGE SHALL BE MEASURED NEAR THE ARC. TRAVEL SPEED AND ELECTRODE STICK OUT SHALL BE VERIFIED TO BE IN COMPLIANCE WITH THE APPROVED b. VISUAL INSPECTION SHALL BE PERFORMED ON ALL SHOP WELDS.
- c. EACH WELDER EMPLOYED ON THE PROJECT SHALL UNDERSTAND ALL THE REQUIREMENTS OF THE WELDING PROCEDURE SPECIFICATION(S) BEFORE WELDING ON THE PROJECT. d. AS-BUILT BEAM TO COLUMN GAP PER CONNECTION SCHEDULE IS ALLOWED TO BE INSTALLED WITH A TOLERANCE OF PLUS OR MINUS 1/2
- a. THE SURFACES ADJACENT TO THE BOLT HEAD AND NUT SHALL BE FREE OF DIRT AND OTHER FOREIGN MATERIAL OTHER THAN THE SPECIFIED COATINGS.
- b. FAYING SURFACES ARE PERMITTED TO BE UNCOATED AND COATED WITH ANY COATINGS OF ANY FORMULATION OR GALVANIZATION c. AFTER THE CONNECTIONS HAVE BEEN ASSEMBLED, VISUALLY ENSURE THAT THE PLIES OF THE CONNECTED ELEMENTS HAVE BEEN BROUGHT INTO AS CLOSE OF CONTACT AS PRACTICABLE WITH ONE ANOTHER. GAPS UP TO 1/8 INCH BETWEEN THE SURFACES SHALL BE ALLOWED. GAPS GREATER THAN 1/8 INCH UP TO 1/4 INCH SHALL HAVE FINGER SHIMS INSTALLED BEFORE PRETENSIONING. FOR GAPS GREATER THAN 1/4 INCH, CONTACT SIDEPLATE SYSTEMS, INC.

- 1. SIDEPLATE CONNECTIONS REQUIRING THIS TYPE OF FINISH SHALL FOLLOW THE SAME CONSTRUCTION SEQUENCING AS PREVIOUSLY OUTLINED WITH THE FOLLOWING MODIFICATIONS: a. HORIZONTAL SHEAR PLATES (D) SHALL HAVE AN INCREASED CLIP SIZE WHICH SHALL BE 1 5/8 INCH BY 1 5/8 INCH TO PROVIDE ADEQUATE VENTILATION AND DRAINAGE. CONTACT SIDEPLATE SYSTEMS, INC. IN THE EVENT THAT THE GALVANIZING CONTRACTOR SPECIFICATIONS REQUIRE A LARGER OPENING THAN THAT SPECIFIED HEREIN.
- b. SEAL WELDING SHALL BE ALLOWED ON THE PLATES {B} AND ANGLES. c. ANY DEVIATIONS TO THESE MODIFICATIONS SHALL BE COORDINATED WITH SIDEPLATE SYSTEMS, INC. AND THE SEOR.

1. WHEN REQUIRED BY THE GOVERNING CODE FOR CERTAIN TYPES OF CONSTRUCTION, SIDEPLATE CONNECTIONS SHALL HAVE A FIRE-

- RESISTANCE RATING LIKE THAT OF A STEEL "STRUCTURAL FRAME". THE MINIMUM THICKNESS OF SPRAY-APPLIED FIRE-RESISTIVE MATERIAL (SFRM) FOR STEEL SIDEPLATE CONNECTIONS PLATES THAT ARE NOT ENCASED IN CONCRETE, SHALL BE DETERMINED JUST LIKE THAT OF A PIPE/TUBE COLUMN SECTION WITH A CONSTANT STEEL WALL THICKNESS USING THE THICKNESS OF SIDE PLATE (A) FOR EACH SIDEPLATE CONNECTION ID PER THE SIDEPLATE CONNECTION SCHEDULE, WHICH ARE UNIFORMLY HEATED AND PROTECTED (THE FIRE EXPOSURE OF A PIPE/TUBE COLUMN IS DIRECTLY ANALOGOUS TO A PLATE WITH A 1-SIDED
- FIRE EXPOSURE AND PROTECTION). THE SFRM SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ASTM E119 AND LISTED FOR FIRE RESISTIVE PIPE/TUBE COLUMN APPLICATIONS FOR NO LESS THAN THE REQUIRED RATED TIME. AS REQUIRED, WHEN NO VERTICAL SHEAR ELEMENT (F) EXISTS IN THE BEAM, SPRAY THE MINIMUM THICKNESS OF SFRM BETWEEN INSIDE OF
- 4. WHEN VERTICAL SHEAR ELEMENT {F} IS USED, THE CONTRACTOR SHALL PROVIDE THE MEANS, TYPICALLY DONE WITH A LAYERING TECHNIQUE, FOR FIREPROOFING ACROSS THE BOTTOM OF THE GAP. 5. SEE GRAPHIC NUMBER 10 IN FIELD ERECTION OF THE SIDEPLATE BOLTED SYSTEM FOR FIREPROOFING ACROSS THE BOTTOM OF THE GAP.

SIDE PLATE {A} AND BEAM WEB COVERING ALL SURFACES INCLUDING COLUMN FLANGE. NOTE: THIS DOES NOT NECESSITATE FILLING THE

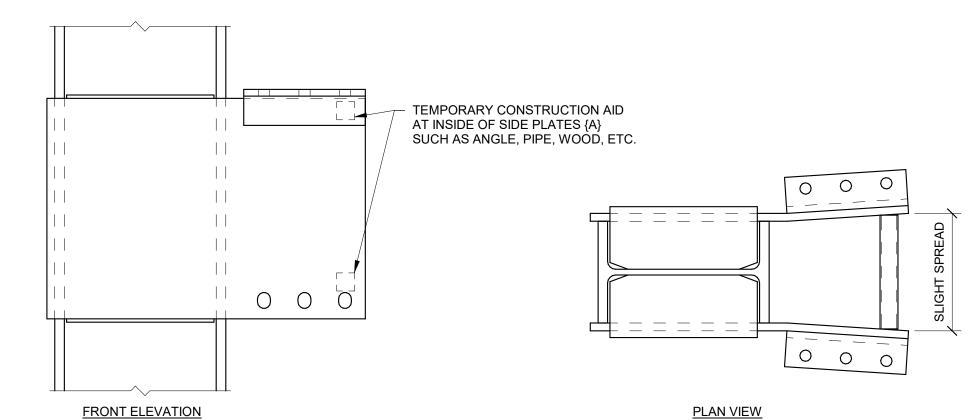
1. IN ORDER TO SAFEGUARD THE AUTHORIZED USE AND INTELLECTUAL PROPERTY OF THE PATENTED SIDEPLATE CONNECTION TECHNOLOGY,

- THE STEEL FABRICATION SUBCONTRACTOR SHALL SATISFY THE FOLLOWING REQUIREMENTS: a. A NOTICE OF INTELLECTUAL PROPERTY, IDENTICAL TO THAT PROVIDED ON THIS SHEET, SHALL BE AFFIXED ON EACH SHEET OF SHOP DETAIL AND FIELD ERECTION DRAWINGS CONTAINING SIDEPLATE SYSTEM INFORMATION WHICH DISCLOSES IN ANY WAY THE SIDEPLATE CONNECTION CONCEPT PRIOR TO RELEASING SUCH INFORMATION FOR ITS INTENDED USE. SUCH NOTICE SHALL BE PROVIDED TO THE STEEL FABRICATION SUBCONTRACTOR BY SIDEPLATE SYSTEMS, INC. IN A FORMAT (E.G. WORD OR AUTOCAD) SUITABLE TO THE NEEDS OF THE STEEL FABRICATION SUBCONTRACTOR'S DETAILER.
- b. PATENT LABELS SHALL BE APPLIED ON THE OUTSIDE FACE OF ONE OF THE TWO BOTTOM HORIZONTAL SHEAR PLATES {D} OF EACH MOMENT CONNECTION AND ON ONE END OF THE BEAM WEB IN COMPLIANCE WITH THE PATENT AND INTELLECTUAL PROPERTY LAWS.

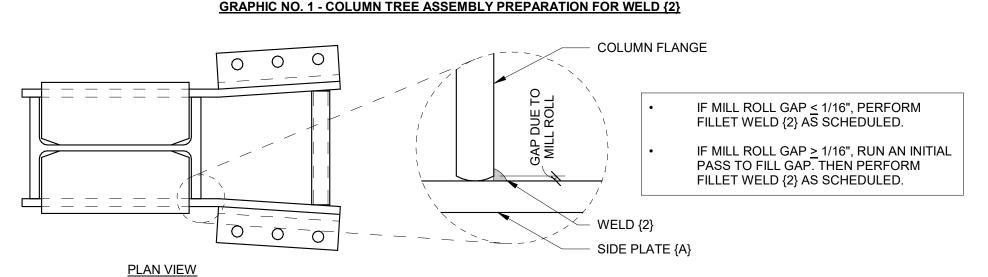
- 1. THE CONTRACTOR SHALL ASSUME FULL AND COMPLETE RESPONSIBILITY FOR THE MEANS AND METHODS OF CONSTRUCTING THE STEEL FRAME USING THE SIDEPLATE BOLTED SYSTEM. CONSTRUCTION MEANS AND METHODS SHALL BE COMPLIANT WITH THE CURRENT PROVISIONS OF AWS D1.1, THE AISC 360 CODE OF STANDARD PRACTICE, THE RCSC HIGH-STRENGTH BOLTING SPECIFICATIONS, AND THE CONSTRUCTION GUIDELINES PROVIDED HEREIN AND SHALL INCLUDE, BUT ARE NOT LIMITED TO:
- a. DIMENSIONAL VERIFICATION AND CONTROL b. FABRICATION AND ERECTION PROCEDURES (INCLUDING METHODS FOR CONTROLLING DISTORTION DUE TO WELD SHRINKAGE, AND FOR CONTROLLING COMBINED MILL, FABRICATION AND ERECTION TOLERANCES) c. CONSTRUCTION AIDS SUCH AS ERECTION RIGGING AND SHORING
- d. PROPER BOLT HOLE ALIGNMENT e. PROPER PRETENSIONING OF BOLTS THE SEQUENCE OF CONSTRUCTION OPTIONS PROVIDED BELOW IN THESE CONSTRUCTION GUIDELINES HAVE PROVEN TO BE SUCCESSFUL BY STEEL FABRICATORS AND ERECTORS TO COST EFFICIENTLY CONSTRUCT THE BOLTED SIDEPLATE CONNECTION SYSTEM. VARIATIONS TO THESE
- CONSTRUCTION SEQUENCE OPTIONS PROVIDED BELOW SHALL BE SUBMITTED FOR REVIEW AND DISPOSITION TO SIDEPLATE SYSTEMS, INC. 3. A PRE-FABRICATION COORDINATION MEETING WITH A SIDEPLATE SYSTEMS, INC. REPRESENTATIVE IS REQUIRED FOR ALL PROJECTS. THE PRE-FABRICATION COORDINATION MEETING IS INTENDED TO SHARE BEST PRACTICES AND COMMON MISTAKES TO AVOID.

SHOP FABRICATION OF THE SIDEPLATE BOLTED SYSTEM

https://portal.sideplate.com/account/login



GRAPHIC NO. 1 - COLUMN TREE ASSEMBLY PREPARATION FOR WELD {2}

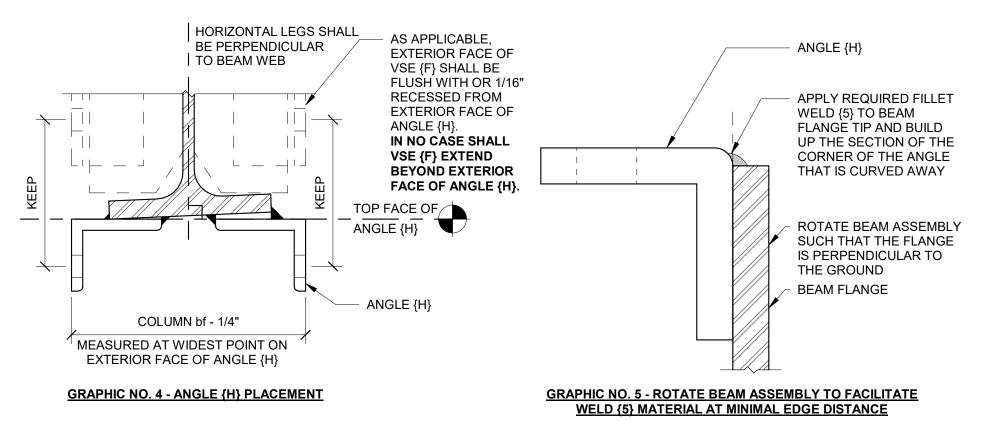


GRAPHIC NO. 2 - MILL ROLL INDUCED GAP

ANGLE (G) -SIDE PLATE {A} PLATE {D} -PLACE ONE PATENT STICKER ON OUTSIDE FACE OF ONE BOTTOM PLATE {D} **ERECTION AID** AKA BOTTOM DOG **GRAPHIC NO. 3 - COMPLETED SIDEPLATE BOLTED COLUMN TREE ASSEMBLY**

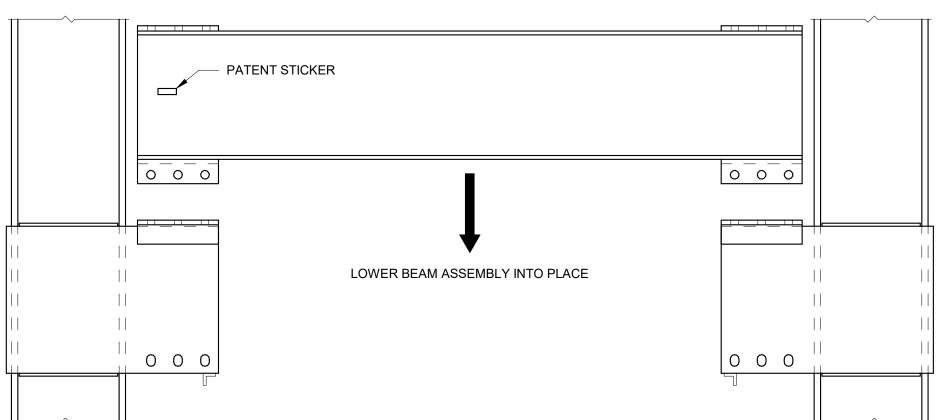
2. WATCH OUR SIDEPLATE BEAM ASSEMBLY VIDEO AT

https://portal.sideplate.com/account/login

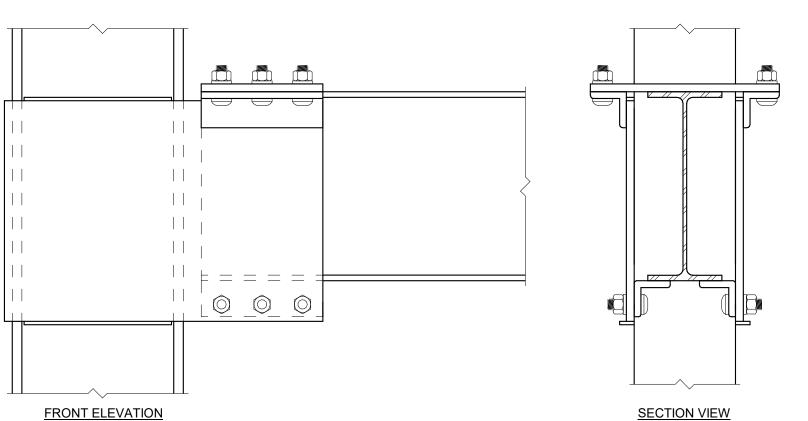


FIELD ERECTION OF SIDEPLATE BOLTED SYSTEM

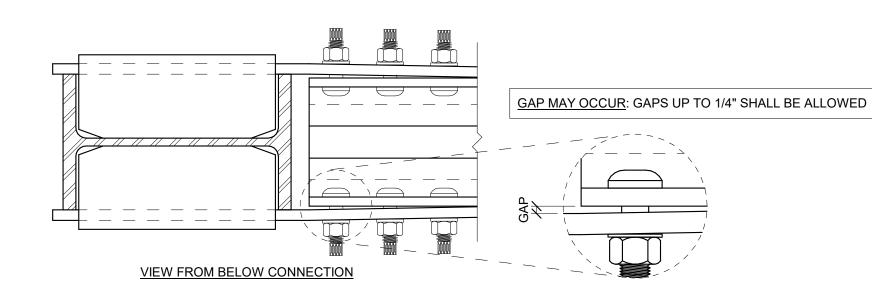
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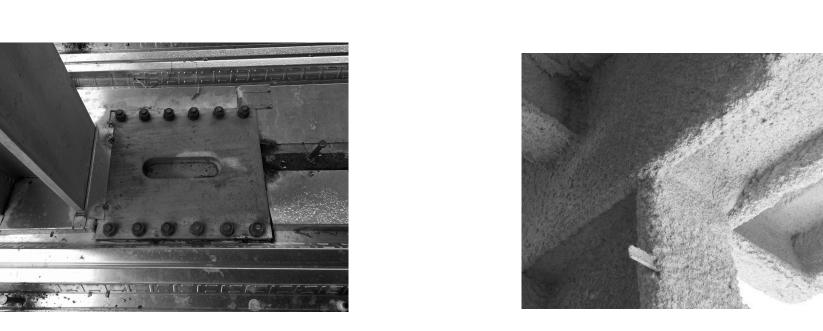
GRAPHIC NO. 6 - FIELD ERECTION OF SIDEPLATE BEAM ASSEMBLY



GRAPHIC NO. 7 - COMPLETED SIDEPLATE BOLTED CONNECTION



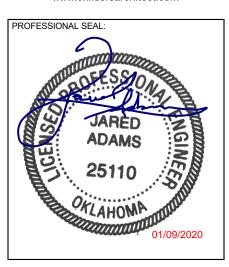
GRAPHIC NO. 8 - SNUG TIGHT CONDITION PRIOR TO PRETENSIONING BOLTS



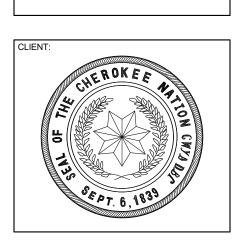
GRAPHIC NO. 9 - TYPICAL GAP CLOSURE AT THE TOP OF THE GAP

GRAPHIC NO. 10 - FIREPROOFING ACROSS THE BOTTOM OF THE GAP









PROJECT PHASE: STEEL DETAILS

REVISIONS
DESCRIPTION

01/10/20 BID PACKAGE 01 - ASI 01

STUAL is cove 322; 8, S. and

JOB NUMBER: 11-01-19 18-01.01 SHEET NUMBER:

SIDEPLATE GENERAL NOTES AND CONSTRUCTION

GUIDELINES

be used only following payment of a license fee to SidePlate Systems, Inc. and for the design, construction, operation, repair, maintenance, restoration or demolition of the building(s) specifically identified.

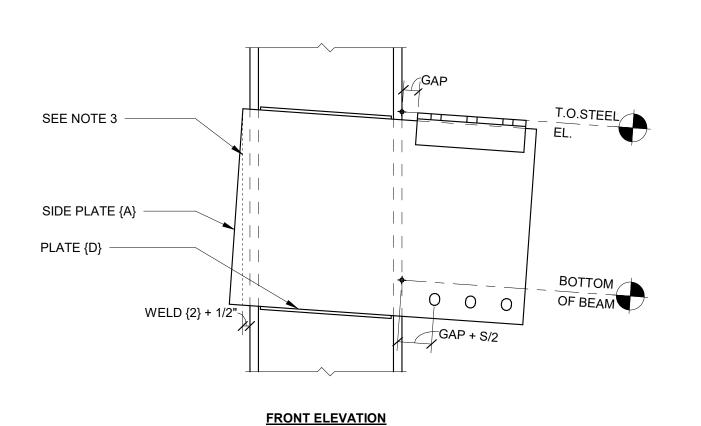
The SIDEPLATE® steel frame connection system is covered by one or more of U.S. Pat. Nos. 6,138,427; 6,516,583; 6,591,573;

3/8" SHEAR PLATE {D}, (4) LOCATIONS ╤╪══╛╘═┇ /PLATE {D} TO INSIDE FACE OF SIDE PLATE {A}, (4) LOCATIONS COLUMN WEB SIDE PLATE {A} /PLATE {D} TO COLUMN WEB, (4) LOCATIONS

NOTE(S):
1. LONGITUDINAL ANGLES {G} NOT SHOWN FOR CLARITY.

SIDE ELEVATION

8 PLATE {D} DETAIL FOR SLOPED CONDITIONS N.T.S.

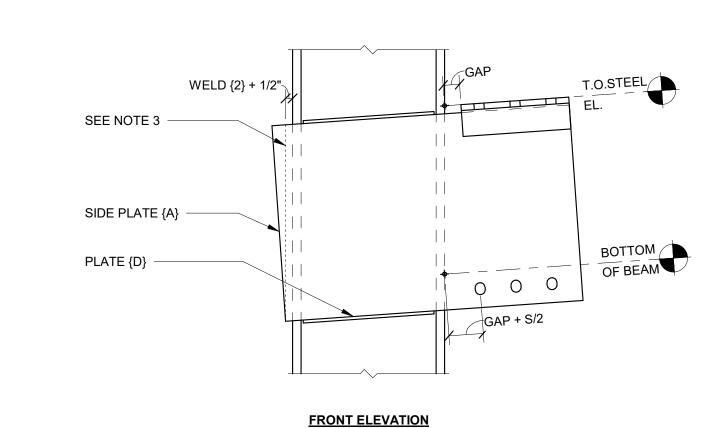


NOTE(S):

1. FOR BEAM SLOPES > 1" PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC. 2. COORDINATE PLATES, ANGLES, AND DIMENSIONS WITH RESPECT TO THE SLOPE OF THE CONNECTION. 3. AT CONTRACTOR'S DISCRETION, SIDE PLATE (A) MAY BE CUT AS SHOWN. 4. HORIZONTAL SHEAR PLATES (D) AND ASSOCIATED WELDS ARE REQUIRED FOR SLOPED SIDE PLATE CONDITIONS. SEE 8 / S8.02

4 SLOPED DOWN CONNECTION (AS APPLICABLE)
N.T.S.

SIDE PLATE {A}



NOTE(S):

1. FOR BEAM SLOPES > 1" PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC.

AND DIMENSIONS WITH RESPECT TO THE 2. COORDINATE PLATES, ANGLES, AND DIMENSIONS WITH RESPECT TO THE SLOPE OF THE CONNECTION. 3. AT CONTRACTOR'S DISCRETION, SIDE PLATE {A} MAY BE CUT AS SHOWN. 4. HORIZONTAL SHEAR PLATES (D) AND ASSOCIATED WELDS ARE REQUIRED FOR SLOPED SIDE PLATE CONDITIONS. SEE 8 / S8.02

3 SLOPED UP CONNECTION (AS APPLICABLE)
N.T.S.

0 0 0 0 T.O.STEEL 0 0 0 1 1/4" X 1 1/2" ↔ SLOTTED HOLES ___ _ _ _ _ _ _ _ AT SIDE PLATE {A}, TYPICAL 1 1/4" X 1 1/2" ↔ SLOTTED HOLE AT SIDE PLATE {A}, TYPICAL

0 0 0 0	OF BEAIN	0	0	0	
MULTIPLE VERTICAL BOLT HOLES	SIDE PLATE {A}	SINGLE VE	ERTICAL	BOLT HO	DLE
NOTE(S): 1. SEE COLUMN SCHEDULE FOR B	OLT QUANTITY.				

	COLUMN PANEL Z	ONE DESI	GN (INCHE	S)				SIDE PLATE {A}	EXTENSION D	ESIGN (INCH	IES)				
	COLUMN	WELD	BEAN	Л	PLATE				ANGLE		WELD		BOLT		
ID	SERIES	{2}	SHAPE	GAP	{/	4}			{G}		{8}	DIAMETER	, HORIZONTAL	G	s
	OLIVILO	SIZE	SHAPE	GAP	THICKNESS	В	Y	SUGGESTED SIZE	HORIZONTAL LEG	VERTICAL LEG	SIZE	DIAIVIETER	#	J	
A10, A11, A19	W14x	3/8	W24X68	2	5/8	27 1/4	2 1/2	L5X3-1/2X5/8	3-1/2 to 6	4 to 6	5/16	1 1/8	4	2 1/8	4 1
A12	W14x	7/16	W24X68	2 1/4	1	27 1/4	1 7/8	L5X3-1/2X5/8	3-1/2 to 6	4 to 6	5/16	1 1/8	4	2 1/8	4 1
1(A13)	W14x	3/8	W24X68	2	3/4	27 1/4	2 1/8	L5X3-1/2X5/8	3-1/2 to 6	4 to 6	5/16	1 1/8	4	2 1/8	4 1
A20	W14x	3/8	W24X94	2	7/8	27 3/4	2 7/8	L5X3-1/2X5/8	3-1/2 to 6	4 to 6	5/16	1 1/8	5	2 1/8	4 1
A30	W14x	3/8	W36X150	2	5/8	39 3/8	5	L5X3-1/2X5/8	3-1/2 to 6	4 to 6	5/16	1 1/8	6	2 1/8	4 1

ANGLE {G} TO SIDE PLATE {A}, (2) LOCATIONS

SIDE PLATE {A}, (2) LOCATIONS

PLACE ONE PATENT STICKER ON THE

OUTSIDE FACE OF ONE SIDE PLATE {A}

6 A TYPE NARROW COLUMN CONNECTION SCHEDULE N.T.S.

PLAN VIEW

5 A TYPE NARROW BOLTED CONNECTION N.T.S.

SIDE PLATE {A} EXTENSION DESIGN (INCHES)

31 3/4 | 1 3/8 | 2 1/2 | 1 1/8

32 1/4 | 1 3/8 | 3 5/8 | 1 1/8

44 | 1 3/8 | 5 | 1 1/8

BOLT

DIAMETER HORIZONTAL VERTICAL G S

2 2 1/8 4 1/2

2 2 1/8 4 1/2 3 2 1/8 4 1/2

COLUMN PANEL ZONE DESIGN (INCHES)

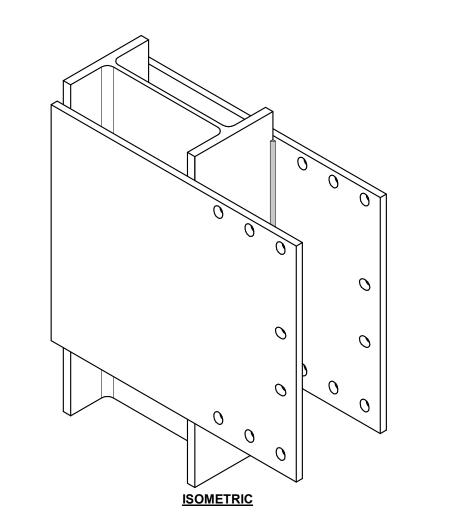
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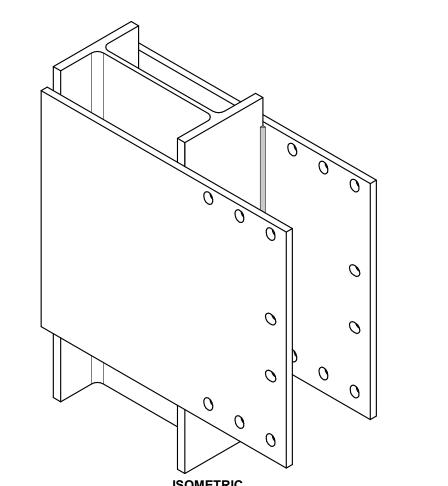
A15

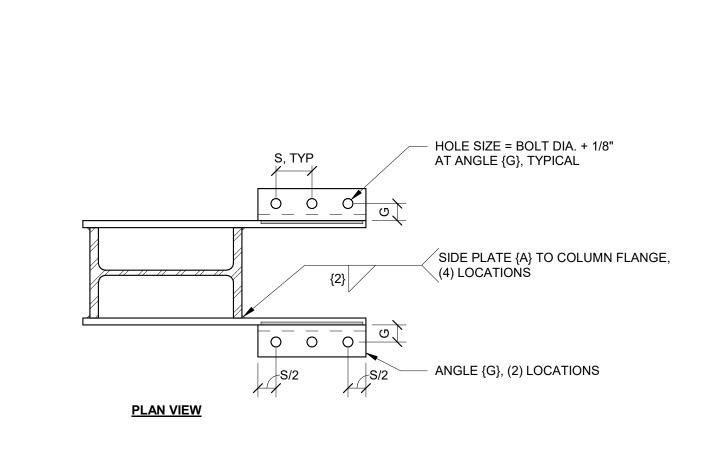
A25

7 SIDE PLATE {A} VSE BOLT HOLE DETAIL N.T.S.

2 A TYPE COLUMN CONNECTION SCHEDULE N.T.S.



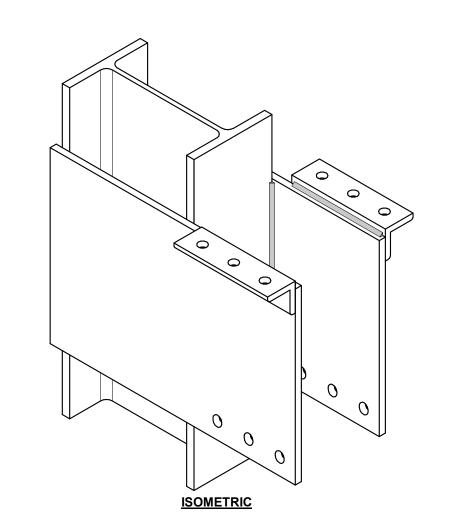




HORIZ. BOLTS S/2

GAP + S/2

FRONT ELEVATION



SIDE ELEVATION

COORDINATE ANGLE (G) ORIENTATION WITH DECK SUPPORT AS NEEDED



PLACE ONE PATENT STICKER ON THE BOTTOM / OUTSIDE FACE OF ONE SIDE PLATE {A} — 1 1/4" X 1 1/2"

\$\times\$ SLOTTED HOLE AT SIDE PLATE {A}, TYPICAL SIDE ELEVATION FRONT ELEVATION

WELD {2} + 1/2" (+/-) SEE NOTE 1 VSE HOLES, SEE DETAIL 7 / S8.02 SIDE PLATE {A}, (2) LOCATIONS -

SIDE PLATE {A} TO COLUMN FLANGE, (4) LOCATIONS,

NOTE(S):

1. THE 1/2 INCH OVERHANG ON THE SIDE PLATE {A} IS TO ENSURE SUFFICIENT ROOM FOR WELD {2}, THE +/- TOLERANCE IS APPLIED SO THAT IF DESIRED, THE DETAILER CAN MAKE THE SIDE PLATES {A} THE SAME LENGTH WITH SLIGHTLY VARYING COLUMN DEPTHS WITHIN A GROUP OF THE SAME CONNECTION ID'S.

NOTE(S):

1. THE +/- 1/4 INCH TOLERANCE FOR PLACEMENT OF ANGLES {G} IS TO ENSURE CORRECT TOP OF STEEL PLACEMENT RELATIVE TO THE CENTERLINE OF THE BOTTOM HORIZONTAL ROW OF BOLT HOLES. THE PLACEMENT OF ANGLES (G) SHALL NEVER BE MEASURED FROM THE BOTTOM EDGE OF SIDE PLATE (A) TO ESTABLISH THE CORRECT TOP OF STEEL.

2. THE 1/2 INCH OVERHANG ON THE SIDE PLATE (A) IS TO ENSURE SUFFICIENT ROOM FOR WELD (2), THE +/- TOLERANCE IS APPLIED SO THAT IF DESIRED, THE DETAILER CAN MAKE THE SIDE PLATES (A) THE SAME LENGTH WITH SLIGHTLY VARYING COLUMN DEPTHS WITHIN A GROUP OF THE SAME CONNECTION ID'S.

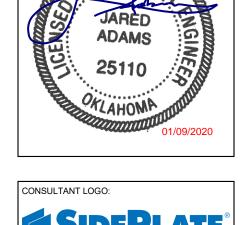
1 1/4" X 1 1/2" ↑ SLOTTED HOLE AT SIDE PLATE {A}, TYPICAL

WELD {2} + 1/2" (+/-) SEE NOTE 2

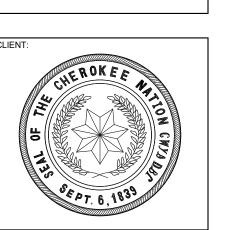
James R. Childers Architect, Inc.

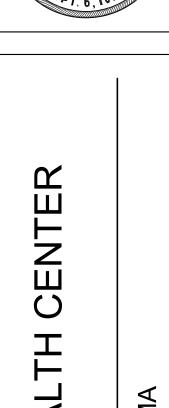
> 45 South 4th Street Fort Smith, AR 72901 479-783-2480 www.childersarchitect.com

PROFESSIONAL SEAL:









MANKILLER HEAL EXPANSION

PROJECT PHASE: STEEL DETAILS

REVISIONS DESCRIPTION 01/10/20 BID PACKAGE 01 - ASI 01

11-01-19 18-01.01 SHEET NUMBER:

SIDEPLATE COLUMN DETAILS, A TYPE

INTELLECTUAL PROPERTY RIGHTS NOTICE

The SIDEPLATE® steel frame connection system is covered by one or more of U.S. Pat. Nos. 6,138,427; 6,516,583; 6,591,573; 7,178,296; 8,122,671; 8,122,672; 8,146,322; 8,176,706; 8,205,408; and 9,091,065 and foreign counterparts.

Other U.S. and foreign applications pending.

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T.O.STEEL

SIDE PLATE (A), TYPICAL

TYPICAL

TYPICAL

SIDE PLATE (A), TYPICAL

TYPICAL

SIDE PLATE (A), TYPICAL

SINGLE VERTICAL BOLT HOLE

NOTE(S):

1. SEE COLUMN SCHEDULE FOR BOLT QUANTITY.

	COLUMN PANEL Z	ONE DESI	GN (INCHE	S)				SIDE PLATE {A} EXTENSION DESIGN (INCHES)								
	COLUMN	WELD	BEAN	М	PL	ATE			ANGLE		WELD	BOLT				
ID	SERIES	{2}	SHAPE	GAP	{/	A}			{G}		{8}	DIAMETER	HORIZONTAL	G	S	
	SENIES	SIZE	OTIVE E	OAI	THICKNESS	В	Y	SUGGESTED SIZE	HORIZONTAL LEG	VERTICAL LEG	SIZE	DIAMETER	#			
B11	W14x	3/8	W24X68	2	5/8	27 1/4	2 1/2	L5X3-1/2X5/8	3-1/2 to 6	4 to 6	5/16	1 1/8	4	2 1/8	4 1/2	
B12	W14x	7/16	W24X68	2 1/4	1	27 1/4	1 7/8	L5X3-1/2X5/8	3-1/2 to 6	4 to 6	5/16	1 1/8	4	2 1/8	4 1/2	
B19	W14x	1/2	W24X68	2	5/8	27 1/4	2 1/2	L5X3-1/2X5/8	3-1/2 to 6	4 to 6	5/16	1 1/8	4	2 1/8	4 1/2	
B20	W14x	3/4	W24X94	2	7/8	27 3/4	2 7/8	L5X3-1/2X5/8	3-1/2 to 6	4 to 6	5/16	1 1/8	5	2 1/8	4 1/2	
B30	W14x	1/2	W36X150	2	5/8	39 3/8	5	L5X3-1/2X5/8	3-1/2 to 6	4 to 6	5/16	1 1/8	6	2 1/8	4 1/2	

B15 W14x 3/8 W24X68 2 5/8 31 3/4 1 3/8 2 1/2 1 1/8 4 2 2 1/8 4 1/2
B25 W14x 1/2 W24X94 2 5/8 32 1/4 1 3/8 3 5/8 1 1/8 5 2 2 1/8 4 1/2
B45 W14x 1/2 W36X160 2 5/8 44 1 3/8 5 1 1/8 6 3 2 1/8 4 1/2

COLUMN PANEL ZONE DESIGN (INCHES)

6 B TYPE NARROW COLUMN CONNECTION SCHEDULE N.T.S.

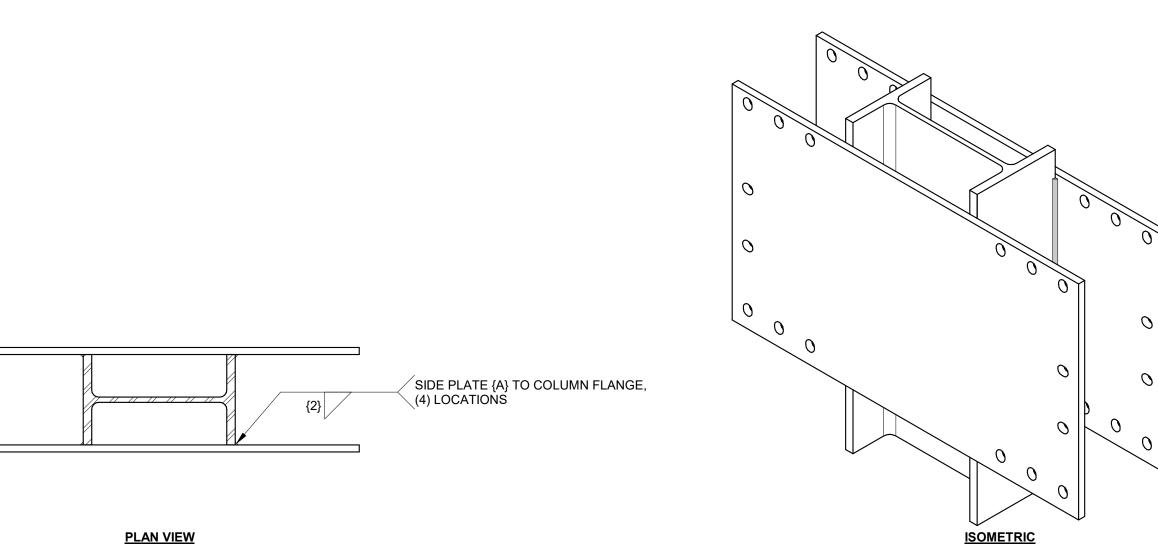
SIDE PLATE {A} EXTENSION DESIGN (INCHES)

BOLT

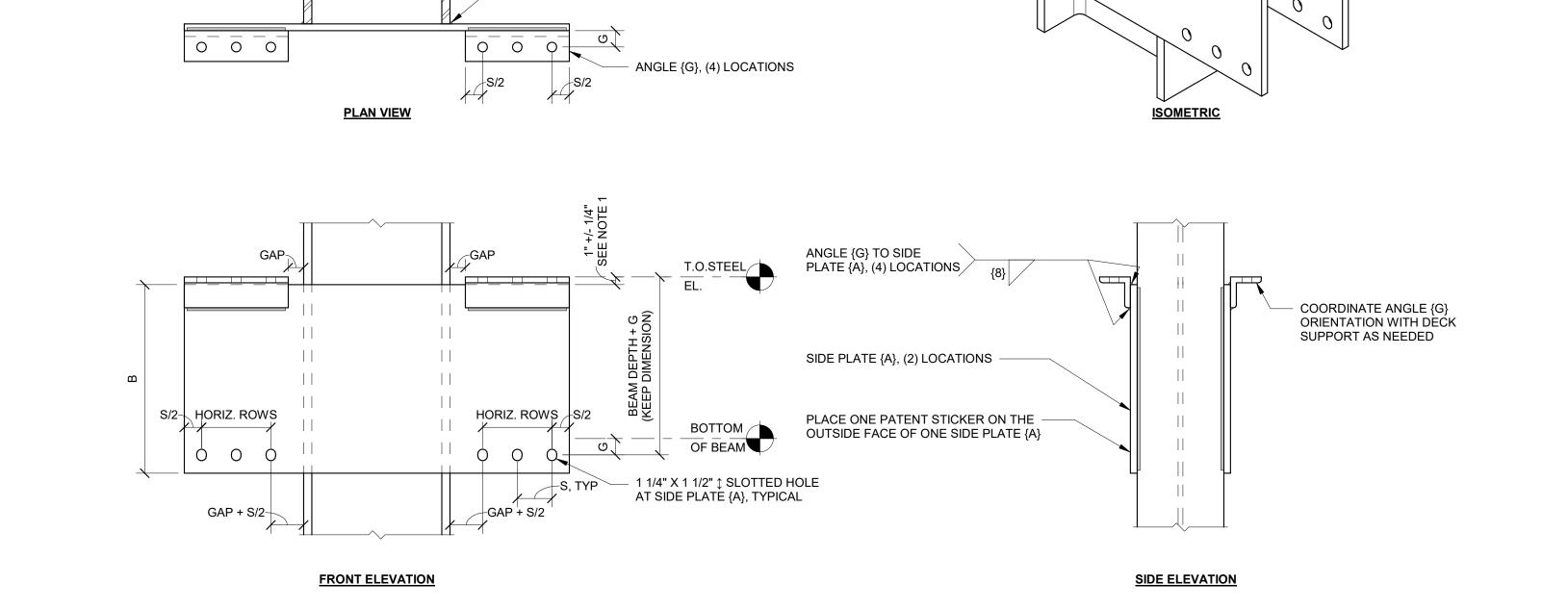
7 SIDE PLATE {A} VSE BOLT HOLE DETAIL N.T.S.

SIDE ELEVATION

2 B TYPE COLUMN CONNECTION SCHEDULE N.T.S.



		SIDE PLATE {A} TO COLUMN FLANGE, (4) LOCATIONS	
	<u>PLAN VIEW</u>		ISOMETRIC
	GAP + S/2 O O	GAP + S/2 O O O U T.O.STEEL EL.	
В	O VSE HOLES, SEE DETAIL 7 / S8.03	SIDE PLATE {A}, (2) LOCATIONS	
_	S/2 HORIZ. BOLTS	HORIZ. BOLTS OF BEAM PLACE ONE PATENT STICKER ON THE OUTSIDE FACE OF ONE SIDE PLATE {A} S, TYP 1 1/4" X 1 1/2" SLOTTED HOLE AT SIDE PLATE {A}, TYPICAL	



- HOLE SIZE = BOLT DIA. + 1/8" AT ANGLE {G}, TYPICAL

SIDE PLATE {A} TO COLUMN FLANGE, (4) LOCATIONS

NOTE(S):

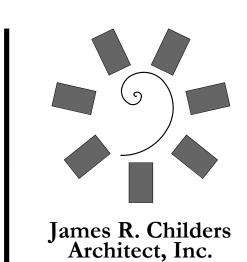
1. THE +/- 1/4 INCH TOLERANCE FOR PLACEMENT OF ANGLES {G} IS TO ENSURE CORRECT TOP OF STEEL PLACEMENT RELATIVE TO THE CENTERLINE OF THE BOTTOM HORIZONTAL ROW OF BOLT HOLES. THE PLACEMENT OF ANGLES {G} SHALL NEVER BE MEASURED FROM THE BOTTOM EDGE OF SIDE PLATE {A} TO ESTABLISH THE CORRECT TOP OF STEEL.

5 B TYPE NARROW BOLTED CONNECTION N.T.S.

FRONT ELEVATION

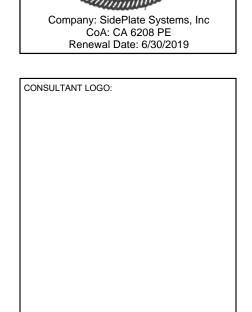
- 1 B TYPE BOLTED CONNECTION N.T.S.

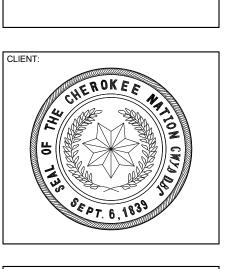
0 0 0





45 South 4th Street Fort Smith, AR 72901 479-783-2480





ILLER HEALTH CENTER (PANSION

WILMA P. MANKILLER EXPANS

KEY PLAN:

PROJECT PHASE:

BID PACKAGE 01

DATE DESCRIPTION

DATE: JOB NUMBER: 11-01-19 18-01.01

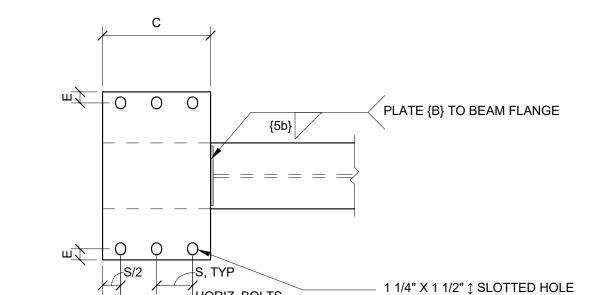
SHEET NUMBER:

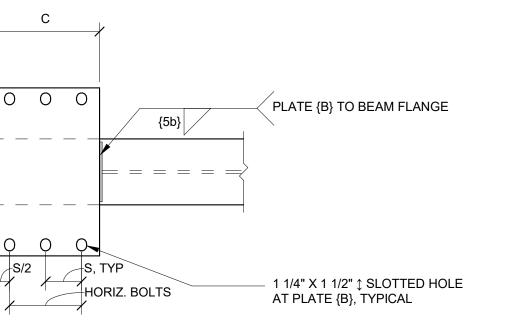
S8.03

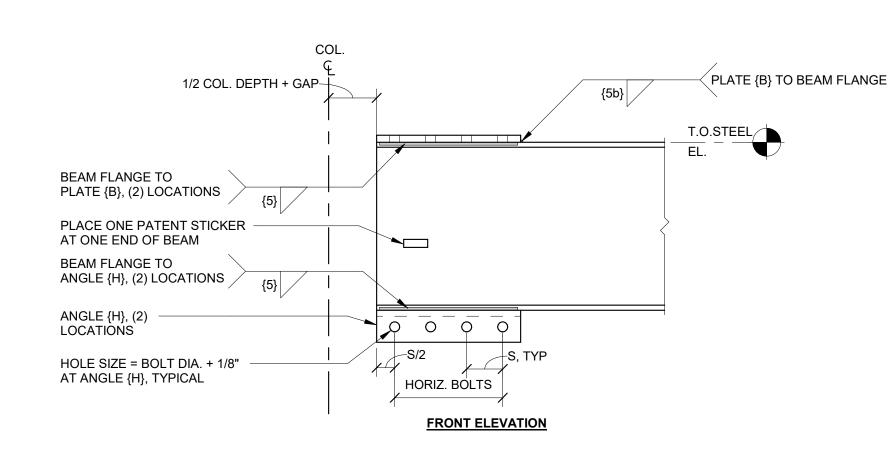
SIDEPLATE COLUMN DETAILS, B TYPE

NOTE(S):
1. FOR ITEMS NOT NOTED, SEE DETAIL 1 / S8.04

PLATE {B} TO BEAM FLANGE S/2 S, TYP
HORIZ. BOLTS **PLAN VIEW**

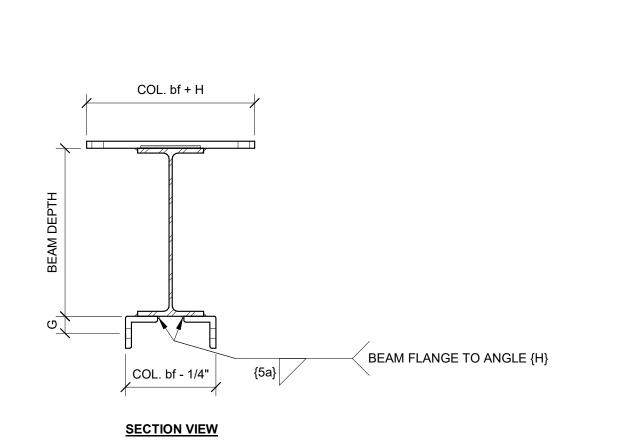






NOTE(S):

1. USE SLOTTED OR RECTANGULAR COVER PLATE {B} PER SCHEDULE. FOR RECTANGULAR COVER PLATE, SEE DETAIL 5 / S8.04



6 STIFFENER PLATES N.T.S.

INTELLECTUAL PROPERTY RIGHTS NOTICE The SIDEPLATE® steel frame connection system is covered by one or more of U.S. Pat. Nos. 6,138,427; 6,516,583; 6,591,573; 7,178,296; 8,122,671; 8,122,672; 8,146,322; 8,176,706; 8,205,408; and 9,091,065 and foreign counterparts. Other U.S. and foreign applications pending.

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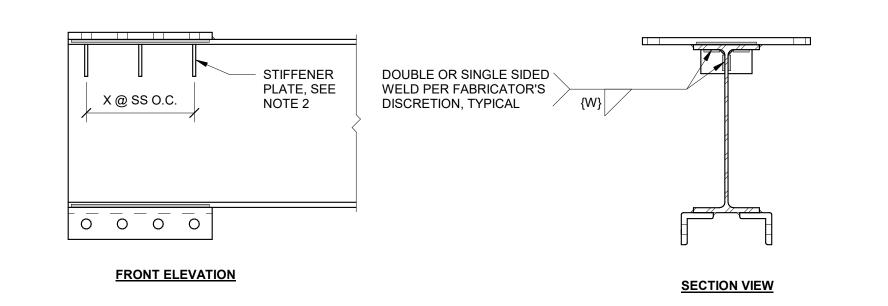
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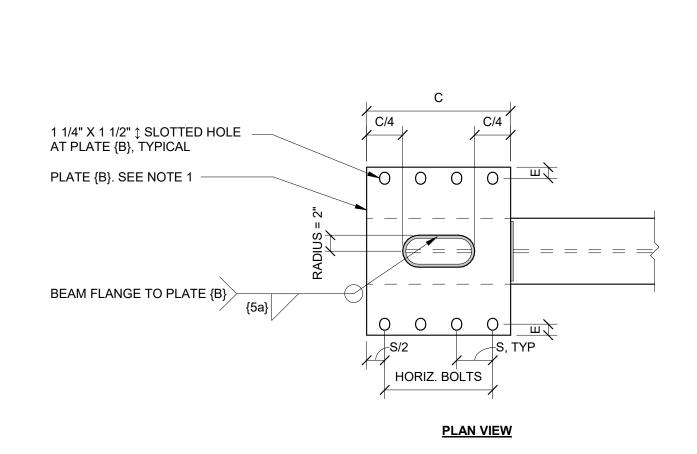
NOTE(S):

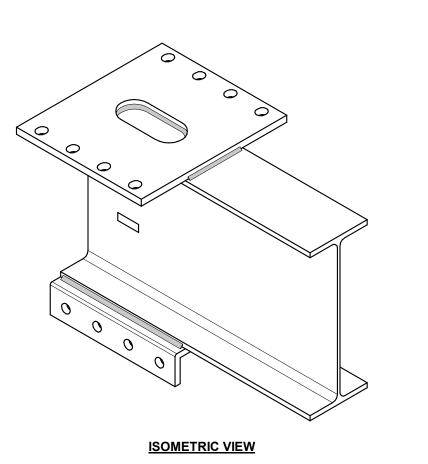
1. SEE BEAM END SCHEDULE FOR QUANTITY, SPACING, AND WELDING OF STIFFENER PLATES.

2. STIFFENER PLATES SHALL BE MADE OF GRADE 50 MATERIAL.

3. STIFFENER PLATES AND WELDS ARE NOT CREATED BY SIDEPLATE CUSTOM COMPONENT TOOL







2 BEAM END SCHEDULE N.T.S.

										BEA	AM DESI	GN (INCHES)										
	BEA	M				PLATE					ANGLE						WELD			BOLT		
ID	OLIADE	045		{B}			STIFFENER (SEE DETAIL	. 6 THIS	SHEET)		{H}		{5}	{5a}	{5b}	{W} (SEE DETA	IL 6 THIS SHE		HORIZONTAL		
	SHAPE	GAP	COVER PLATE TYPE	THICKNESS	E	Н	THICKNESS	LENGTH X WIDTH	x ss	SUGGESTED SIZE	С	HORIZONTAL LEG	VERTICAL LEG	SIZE	SIZE	SIZE	SIZE (SINGLE)	SIZE (DOUBLE)	DIAMETER	#	G	S
A10	W24X68	2	Slotted	1 1/8	1 3/8	8 1/4	-	-		L6X4X5/8	18	6	4	5/16	5/16	5/16	-	-	1 1/8	4	2 1/8	4 1/2
A11, B11	W24X68	2	Slotted	3/4	1 3/8	8 1/4	-	-		L7X4X5/8	18	7	4	5/16	5/16	5/16	-	-	1 1/8	4	2 1/8	4 1/2
A12, B12	W24X68	2 1/4	Slotted	3/4	1 3/8	9	-	-		L7X4X5/8	18	7	4	5/16	5/16	5/16	-	-	1 1/8	4	2 1/8	4 1/2
1 (A13)	W24X68	2	Slotted	7/8	1 3/8	8 1/2	-	-		L7X4X5/8	18	7	4	5/16	5/16	5/16	-	-	1 1/8	4	2 1/8	4 1/2
A19, B19	W24X68	2	Slotted	1	1 3/8	8 1/4	1/4	4 X 4	3 6 3/	4 L6X4X5/8	18	6	4	5/16	5/16	5/16	1/4	1/8	1 1/8	4	2 1/8	4 1/2
A20, B20	W24X94	2	Slotted	1 1/4	1 3/8	8 3/4	-	-		L6X4X5/8	22 1/2	6	4	5/16	5/16	5/16	-	-	1 1/8	5	2 1/8	4 1/
A30, B30	W36X150	2	Slotted	1 1/4	1 3/8	8 1/4	-	-		L5X3-1/2X5/8	27	5	3 1/2	5/16	5/16	5/16	-	-	1 1/8	6	2 1/8	4 1/

COVER PLATE {B}

- ANGLE {H}

4 SLOPED DOWN BEAM END (AS APPLICABLE)
N.T.S.

3 SLOPED UP BEAM END (AS APPLICABLE) N.T.S.

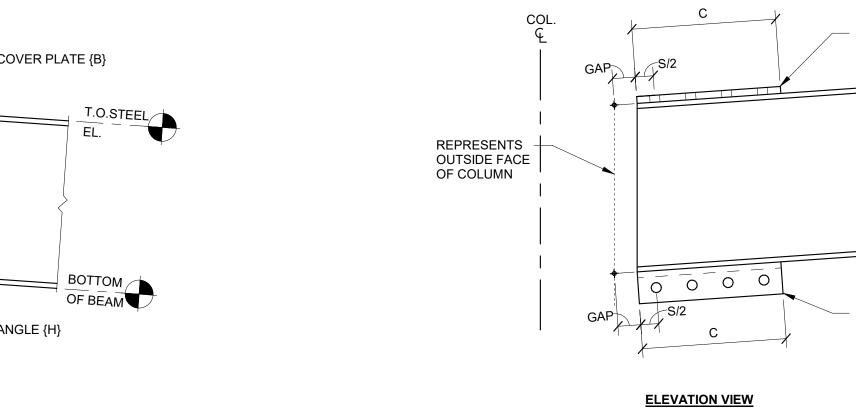
NOTE(S):
1. FOR BEAM SLOPES > 1" PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC.

190001

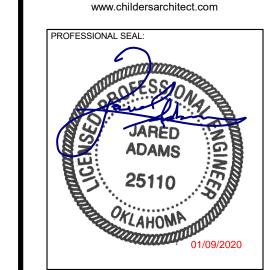
ELEVATION VIEW

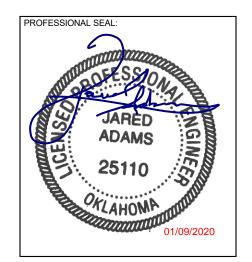
NOTE(S):
1. FOR BEAM SLOPES > 1" PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC.

REPRESENTS OUTSIDE FACE OF COLUMN

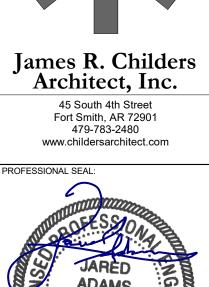


— COVER PLATE (B) OF BEAM - ANGLE {H}





25909 Pala, Suite 200, Mission Viejo, CA 92691 www.sideplate.com





S8.04 DETAILS

SIDEPLATE BEAM

PROJECT PHASE:

STEEL DETAILS

| REVISIONS | # DATE | DESCRIPTION | 1 01/10/20 | BID PACKAGE 01 - ASI 01 |

11-01-19 18-01.01

SHEET NUMBER:

7,178,296; 8,122,671; 8,122,672; 8,146,322; 8,176,706; 8,205,408; and 9,091,065 and foreign counterparts.

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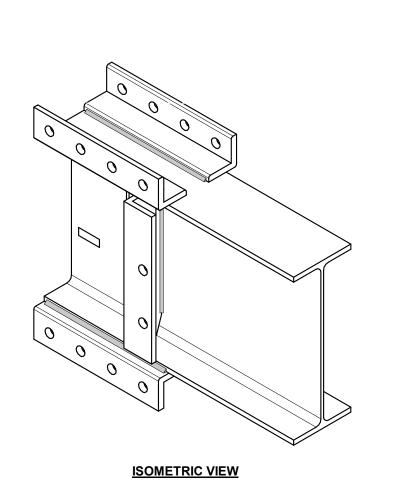
BEAM FLANGE TO ANGLE {H}, (4) LOCATIONS {5a}

PLAN VIEW

T.O.STEEL EL.

NOTE(S):

1. DIMENSION BETWEEN PLATE {C} AND INSIDE FACE OF BEAM FLANGE SHALL NOT EXCEED 1/4 INCH, AND MAY VARY DEPENDING ON BEAM MILL TOLERANCES. PLATE {C} SHALL BE CENTERED ON THE DEPTH OF THE BEAM.



SECTION VIEW

- VSE {F}, SEE DETAIL 4 / S8.05

PLATE {C} TO BEAM WEB, TYPICAL

2 NARROW BEAM END SCHEDULE N.T.S.

1/2 COL. DEPTH + GAP

HOLE SIZE = BOLT DIA. + 1/8" -AT ANGLE {H}, TYPICAL

PLACE ONE PATENT STICKER — AT ONE END OF BEAM

VSE {F} HOLE, TYPICAL SEE DETAIL 3 / S8.05

BEAM FLANGE TO ANGLE {H}, (4) LOCATIONS

ANGLE {H}, (4) LOCATIONS

1 NARROW BEAM END DETAIL N.T.S.

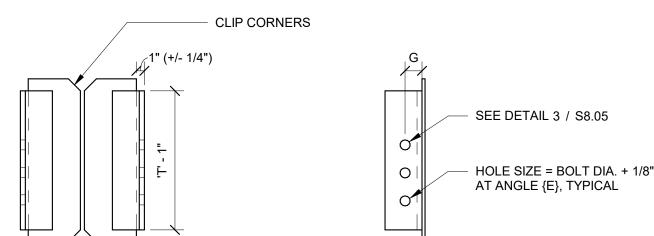
NOTE(S):

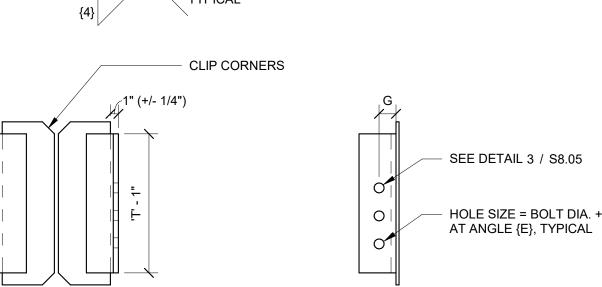
1. SEE SIDEPLATE SCHEDULE FOR BOLT QUANTITY.

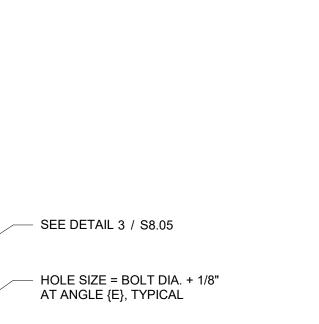
4 VSE {F} DETAIL N.T.S.

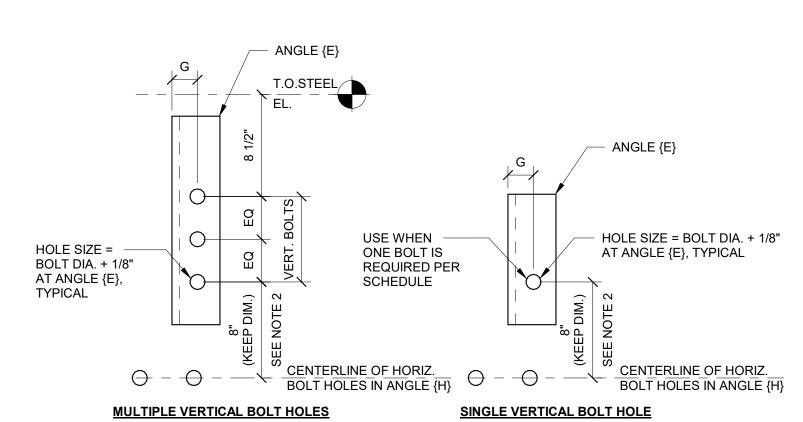
		BEAM DESIGN (INCHES)															
ID	BEAN	/	PLATE	ANGLE						WELD		BOLT					
	CHADE	GAP	{C}			{H}		{E}	{4}	{5}	{5a}	DIAMETER HORIZONTAL		VERTICAL	G		S
	SHAPE	GAF	THICKNESS	SUGGESTED SIZE	С	HORIZONTAL LEG	VERTICAL LEG	SIZE	SIZE	SIZE	SIZE	DIAWETER	# #	#	G	J	3
A15, B15	W24X68	2	3/8	L6X4X5/8	18	6	4	L4X4X1/2	1/4	5/16	5/16	1 1/8	4	2	2 1/8	15 3/4	4 1/2
A25, B25	W24X94	2	3/8	L6X4X5/8	22 1/2	6	4	L4X4X1/2	1/4	5/16	5/16	1 1/8	5	2	2 1/8	20 1/4	4 1/2
A45, B45	W36X160	2	3/8	L5X3-1/2X5/8	27	5	3 1/2	L4X4X1/2	1/4	5/16	5/16	1 1/8	6	3	2 1/8	24 3/4	4 1/2

3 VSE {F} HOLE DETAIL N.T.S.







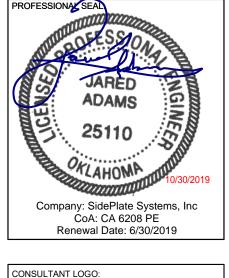


NOTE(S):
1. SEE BEAM END SCHEDULE FOR BOLT QUANTITY.
2. EFFECTS OF MILL AND FABRICATION TOLERANCES ARE ACCOUNTED FOR BY MEASURING FROM THE CENTERLINE OF THE HORIZONTAL ROW OF BOLTS IN THE BOTTOM ANGLES {H}.

Company: SidePlate Systems, Inc CoA: CA 6208 PE Renewal Date: 6/30/2019

James R. Childers Architect, Inc.

45 South 4th Street Fort Smith, AR 72901 479-783-2480 www.childersarchitect.com



CONSULTANT LOGO:

PROJECT PHASE: **BID PACKAGE 01**

11-01-19 18-01.01

S8.05

SIDEPLATE BEAM DETAILS, NARROW

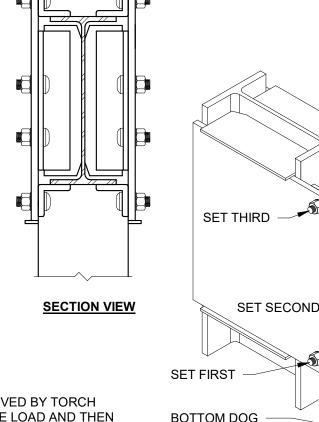
PLAN VIEW

FRONT ELEVATION

TYPICAL SEQUENCE OF ERECTION:

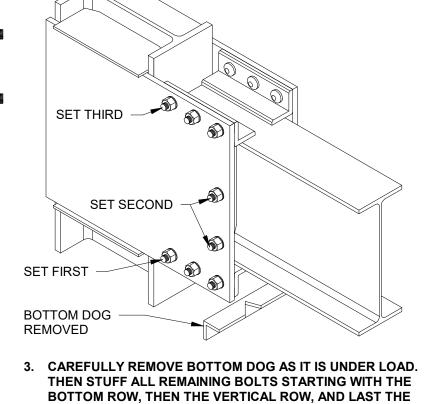
1. LOWER THE BEAM INTO PLACE FROM ABOVE.

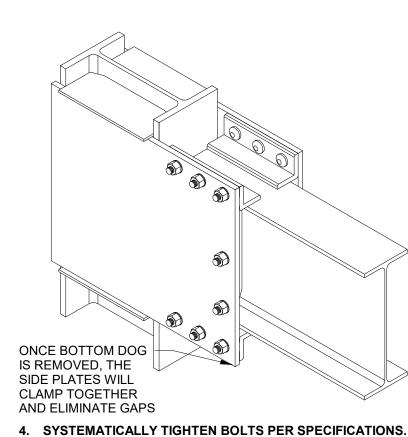
- 6. THE WELD REMNANTS OF THE BOTTOM DOG MAY REMAIN IN PLACE AND DO NOT NEED TO BE GROUND SMOOTH.
- 3. BOTTOM DOG SHALL BE REMOVED. IT IS RECOMMENDED THAT IT BE REMOVED BY TORCH THE WELDS WHILE THE DOG IS UNDER LOAD! 4. BOLTS SHALL BE STUFFED INTO HOLES IN THE BEAM COVER PLATE (B) AND THE SIDE PLATES (A). 5. SYSTEMATICALLY TIGHTEN BOLTS PER RCSC SPECIFICATIONS.
- 2. STUFF A FEW BOLTS TO SECURE ASSEMBLY. CUTTING A 'V' SECTION OUT OF ONE OF THE ANGLE LEGS TO ALLEVIATE THE LOAD AND THEN PROCEED TO REMOVE IT. IT IS NOT RECOMMENDED TO USE A GRINDING WHEEL TO REMOVE



BOTTOM DOG – UNDER LOAD

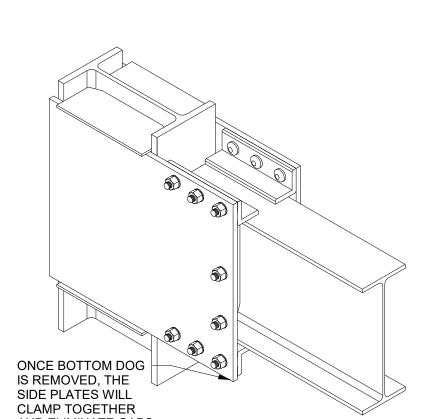
LOWER BEAM INTO PLACE

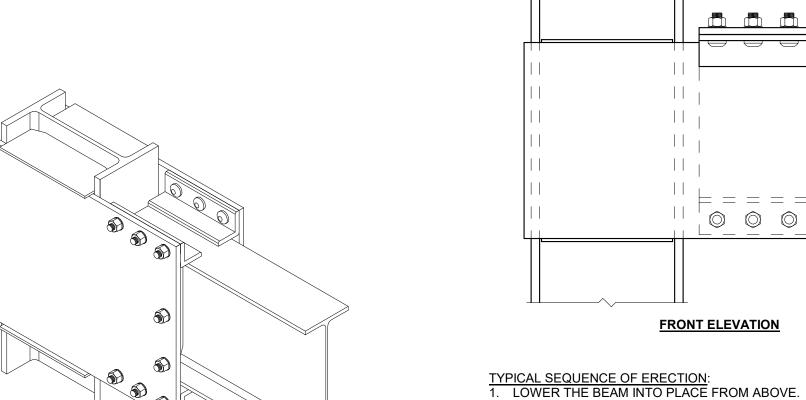


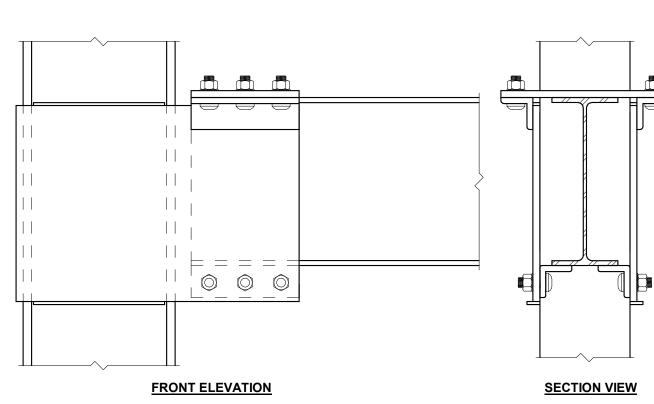


2. STUFF A FEW BOLTS TO SECURE ASSEMBLY

BOTTOM DOG -UNDER LOAD







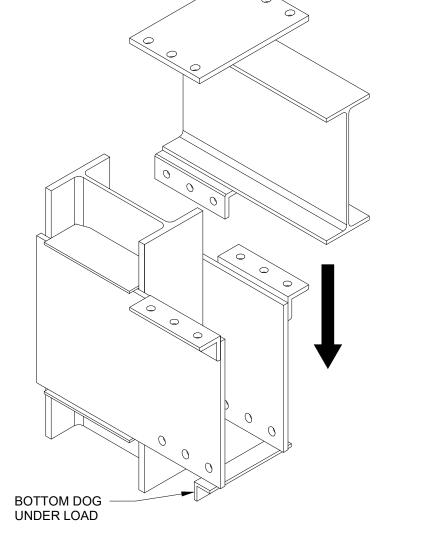
3. BOTTOM DOG SHALL BE REMOVED. IT IS RECOMMENDED THAT IT BE REMOVED BY TORCH

CUTTING A 'V' SECTION OUT OF ONE OF THE ANGLE LEGS TO ALLEVIATE THE LOAD AND THEN

PROCEED TO REMOVE IT. IT IS NOT RECOMMENDED TO USE A GRINDING WHEEL TO REMOVE

4. BOLTS SHALL BE STUFFED INTO HOLES IN THE BEAM COVER PLATE {B} AND THE SIDE PLATES {A}.

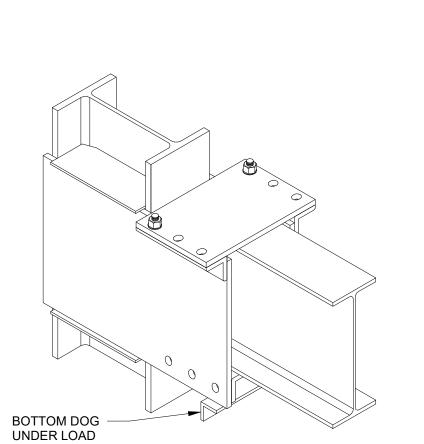
6. THE WELD REMNANTS OF THE BOTTOM DOG MAY REMAIN IN PLACE AND DO NOT NEED TO BE



1. LOWER BEAM INTO PLACE

BOTTOM DOG -

REMOVED



2. STUFF A FEW BOLTS TO SECURE ASSEMBLY

ONCE BOTTOM DOG IS REMOVED, THE

SIDE PLATES WILL

CLAMP TOGETHER

3. CAREFULLY REMOVE BOTTOM DOG AS IT IS UNDER LOAD. 4. SYSTEMATICALLY TIGHTEN BOLTS PER

THEN STUFF ALL REMAINING BOLTS STARTING WITH THE

BOTTOM ROW AND THEN THE TOP ROW.

AND ELIMINATE GAPS

SPECIFICATIONS.

6 NARROW BEAM ERECTION SCHEDULE N.T.S.

		EREC ⁻	TION DESIGN (II	NCHES)					
ID	BEAM	BOLT							
ID	SHAPE	DIAMETER	HORIZONTAL #	VERTICAL #	TOTAL # PER BEAM END				
A15, B15	W24X68	1 1/8	4	2	20				
A25, B25	W24X94	1 1/8	5	2	24				
A45, B45	W36X160	1 1/8	6	3	30				

3 FIELD BOLTING DETAIL N.T.S.

2. STUFF A FEW BOLTS TO SECURE ASSEMBLY.

GROUND SMOOTH.

BEAM ERECTION DETAIL

THE WELDS WHILE THE DOG IS UNDER LOAD!

5. SYSTEMATICALLY TIGHTEN BOLTS PER RCSC SPECIFICATIONS.

PLAN VIEW

- 5. THE BOLT/FASTENER ASSEMBLY SHALL BE COVERED IN A LIGHT PROTECTIVE OIL. 6. FOLLOW QUALITY CONTROL SECTION FOR EXPOSURE LIMITATION ON BOLTS/FASTENERS.
- NOTE(S):

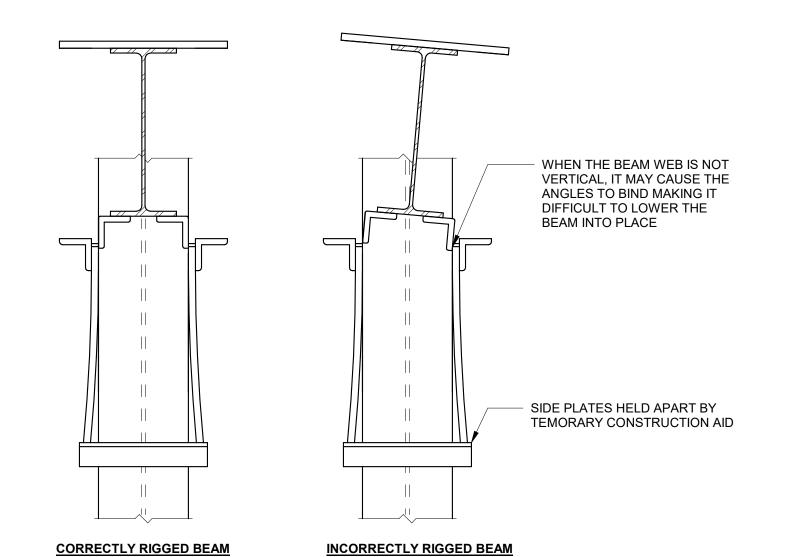
 1. BOLTS SHALL BE INSTALLED AS SHOWN TO KEEP THREADS OUTSIDE OF SHEAR PLANE. 2. BOLTS SHALL BE SYSTEMATICALLY INSTALLED AS OUTLINED IN THE BOLTING SPECIFICATIONS. FIRST TO A SNUG BOLTS SHALL BE STSTEMATICALLY INSTALLED AS COTLINED IN THE BOLTING SPECIFICATIONS. FIRST TO A TIGHT CONDITION, AND THEN PRETENSIONED.
 THE USE OF FINGER SHIMS ARE ALLOWED FOR GAPS GREATER THAN 1/8 INCH UP TO 1/4 INCH. CONTACT SIDEPLATE SYSTEMS, INC. IF GAPS ARE GREATER THAN 1/4 INCH.
 NUT SHALL BE ASTM A563.

		ERECTION DESIGN (INCHES)									
	15	BEAM	BEAM BOLT								
	ID	SHAPE	DIAMETER	HORIZONTAL #	TOTAL # PER BEAM END						
<u>/1</u>	A10, A11, A12, (A13, A19, B11, B12, B19	W24X68	1 1/8	4	16						
	A20, B20	W24X94	1 1/8	5	20						
_	A30, B30	W36X150	1 1/8	6	24						

2 BEAM ERECTION SCHEDULE N.T.S.

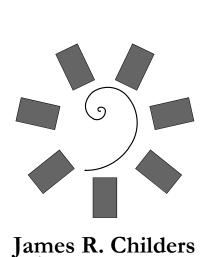
THE BOLT SHALL EXTEND BEYOND -ALL BOLT HOLES SHALL BE ALIGNED OR BE AT LEAST FLUSH WITH THE TO PERMIT INSERTION OF THE OUTER FACE OF THE NUT WHEN BOLTS WITHOUT UNDUE DAMAGE PRETENSIONED TO THE THREADS SLOTTED HOLE MINIMUM (1) ORDINARY THICKNESS ASTM F436 OR F959 WASHER SHEAR PLANE -SIDE PLATE {A} OR PLATE {B} FINGER SHIMS MAY BE PLACED BETWEEN FAYING SURFACES OF ANGLES AND PLATE HOLE SIZE = BOLT DIA. + 1/8" -ANGLE {E}, ANGLE {G}, ANGLE {H}, OR PLATÉ (T)

4 BEAM INSTALLATION DETAIL N.T.S.



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Architect, Inc.

45 South 4th Street

Fort Smith, AR 72901

479-783-2480 www.childersarchitect.com

PROJECT PHASE:

SHEET NUMBER:

STEEL DETAILS

REVISIONS DESCRIPTION 01/10/20 BID PACKAGE 01 - ASI 01

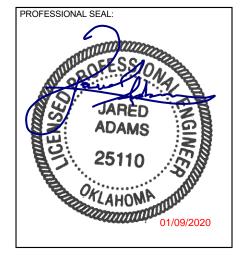
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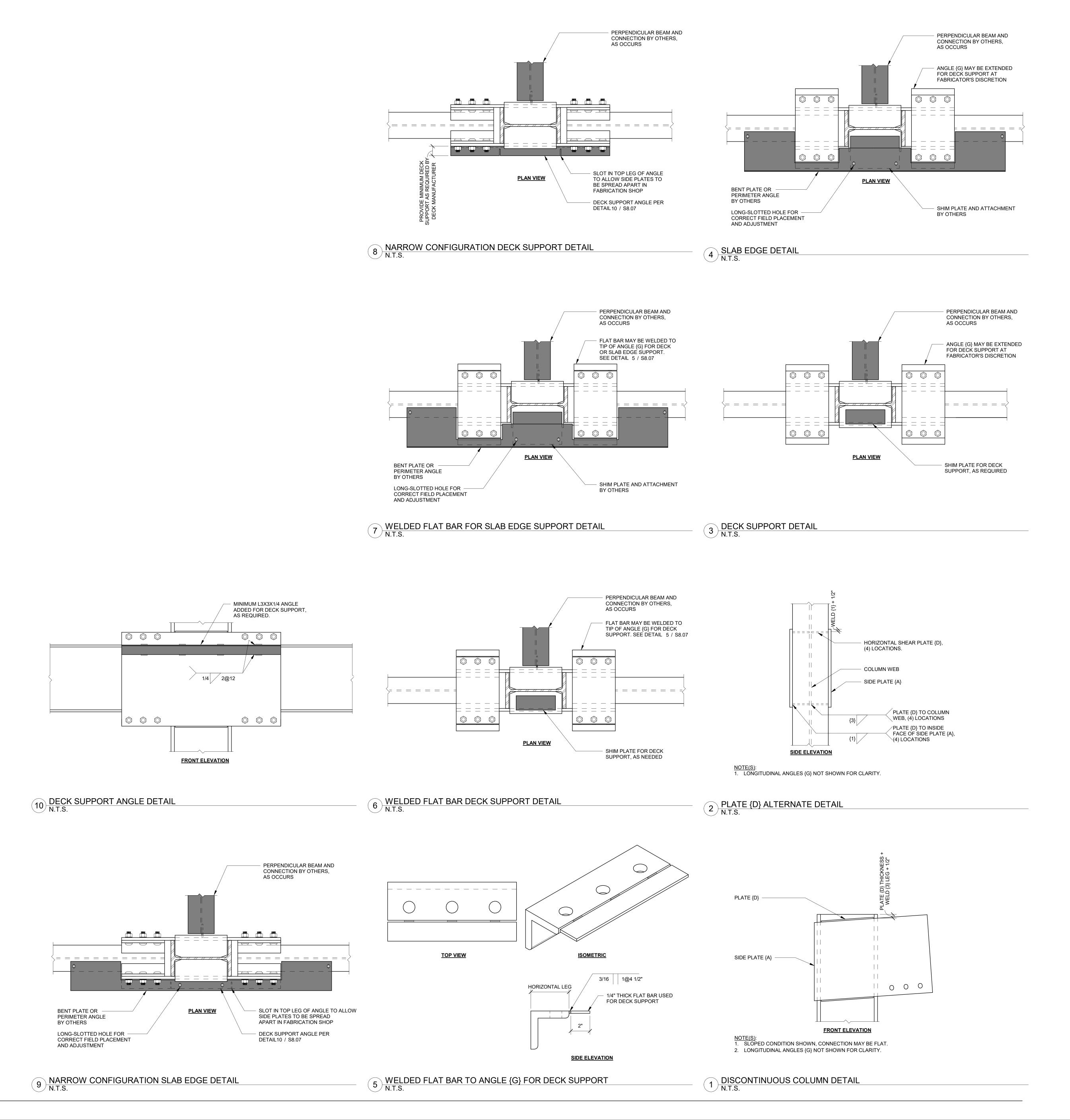
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SIDEPLATE FIELD

ERECTION DETAILS



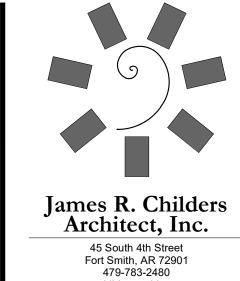




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CONSULTANT LOGO:

25909 Pala, Suite 200, Mission Viejo, CA 92691 www.sideplate.com

MANKILLI EXPA

PROJECT PHASE:

STEEL DETAILS

| REVISIONS | DATE | DESCRIPTION | 1 01/10/20 | BID PACKAGE 01 - ASI 01 |

11-01-19 18-01.01

S8.07

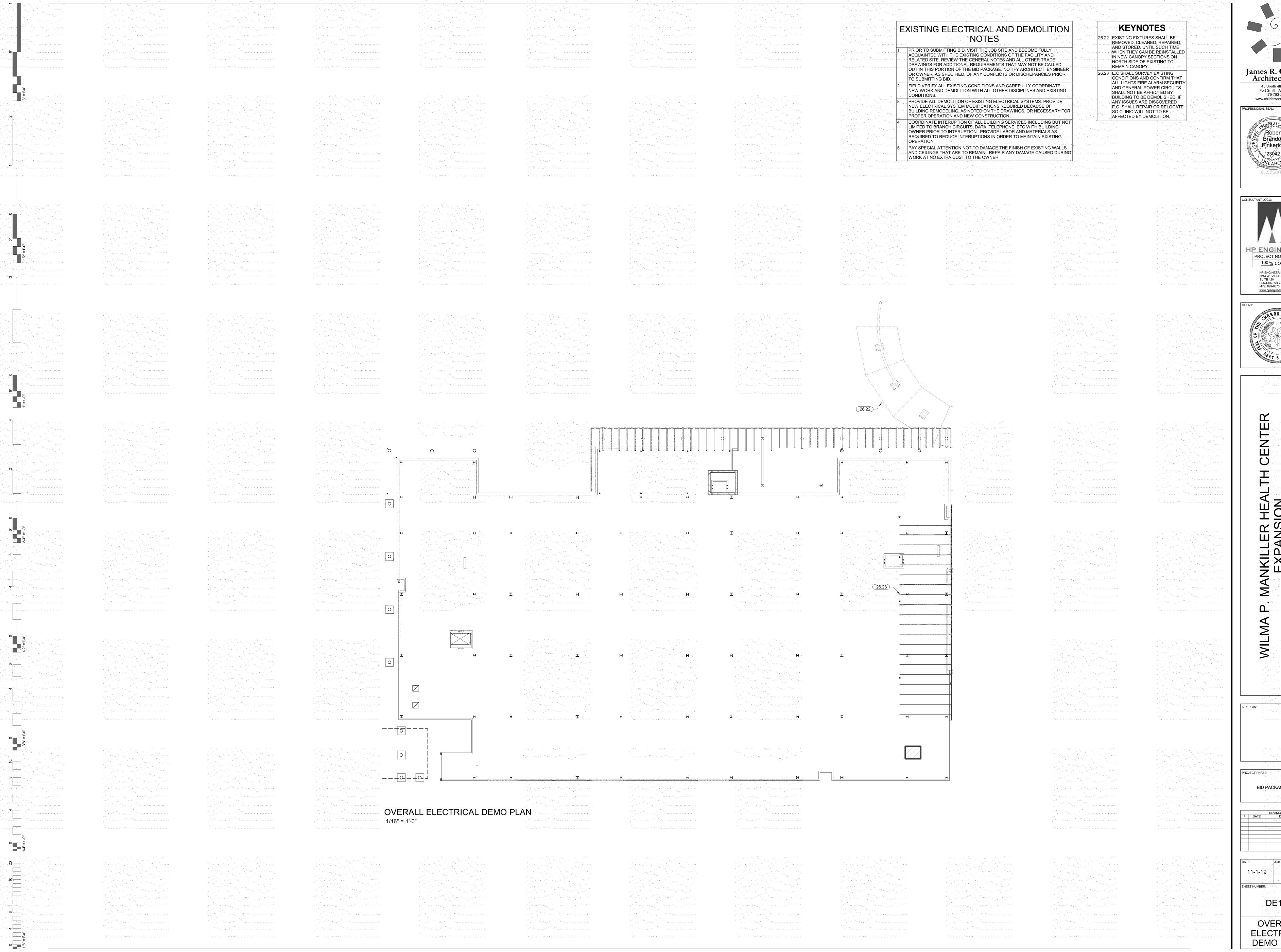
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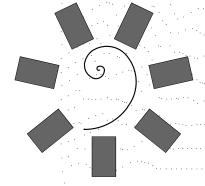
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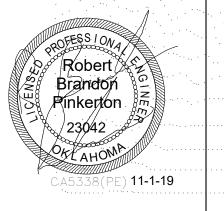


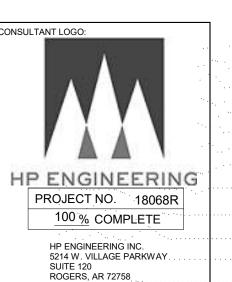


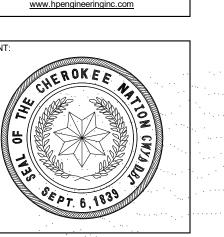


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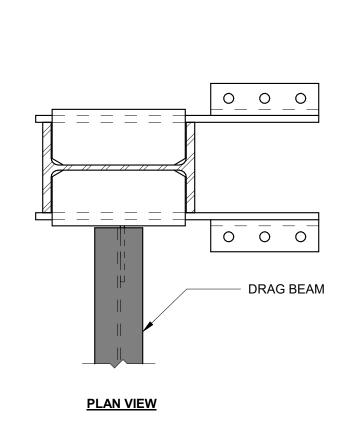
MANKILLER HEALT EXPANSION

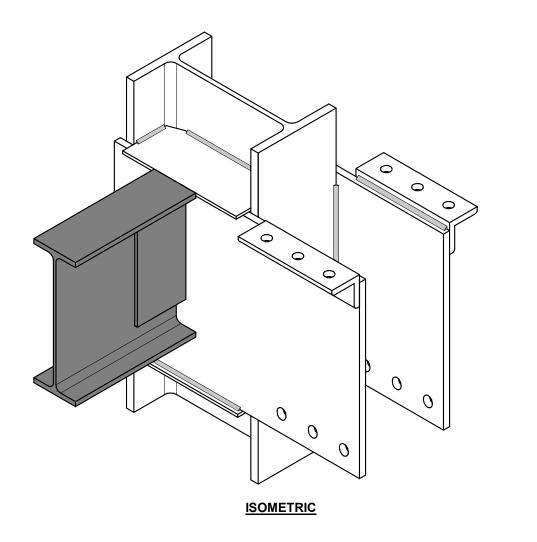
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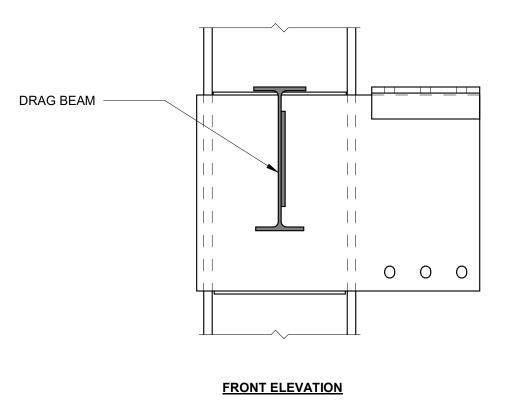
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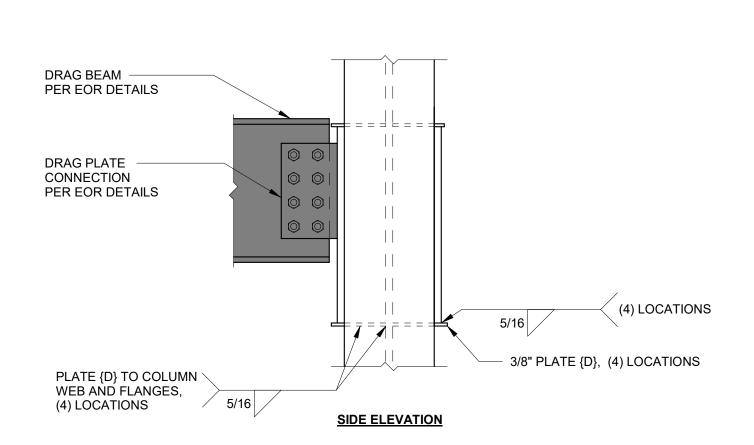
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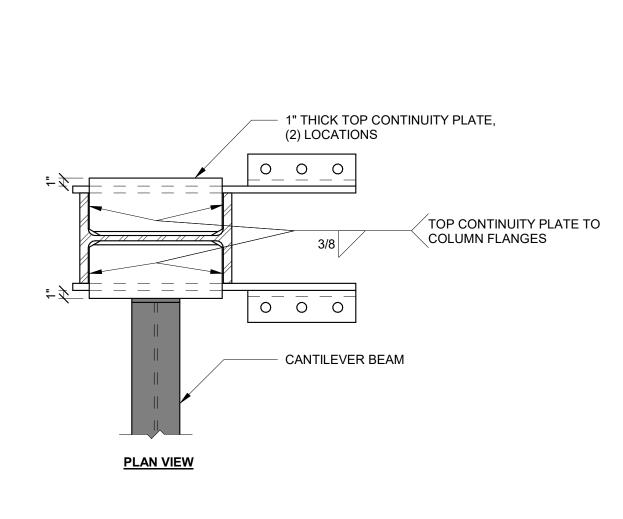


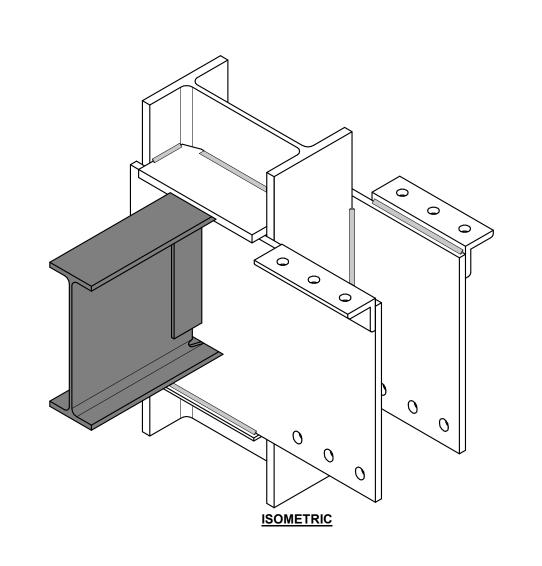


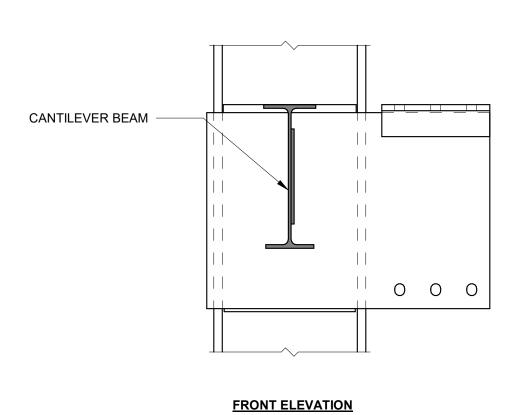
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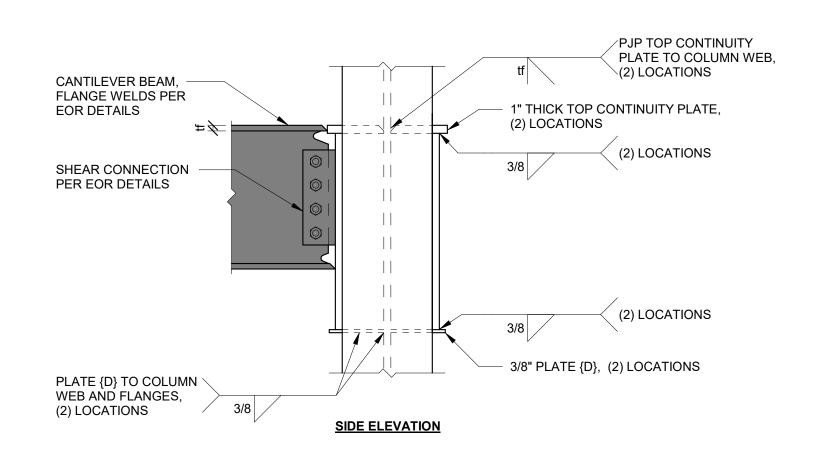
1. ATTACHMENT SHOWN ON ONE SIDE OF SIDEPLATE CONNECTION FOR ILLUSTRATION. ATTACHMENT CAN OCCUR ON LEFT SIDE, RIGHT SIDE, OR BOTH SIDES OF CONNECTION AS APPLICABLE.

M2 SHEAR PLATE DRAG BEAM TO SIDEPLATE CONNECTION N.T.S.









NOTE(S):

1. ATTACHMENT SHOWN ON ONE SIDE OF SIDEPLATE CONNECTION FOR ILLUSTRATION. ATTACHMENT CAN OCCUR ON LEFT SIDE, RIGHT SIDE, OR BOTH SIDES OF CONNECTION AS APPLICABLE.

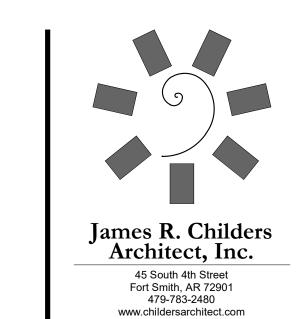
M1 CANTILEVER TO SIDEPLATE CONNECTION N.T.S.

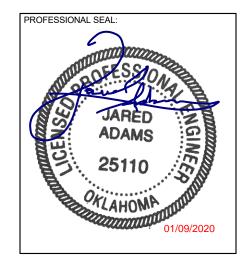
1 MISCELLANEOUS DETAILS SCHEDULE N.T.S.

Coordinate with Detail

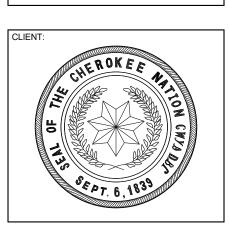
M1/S8.08

M2/S8.08









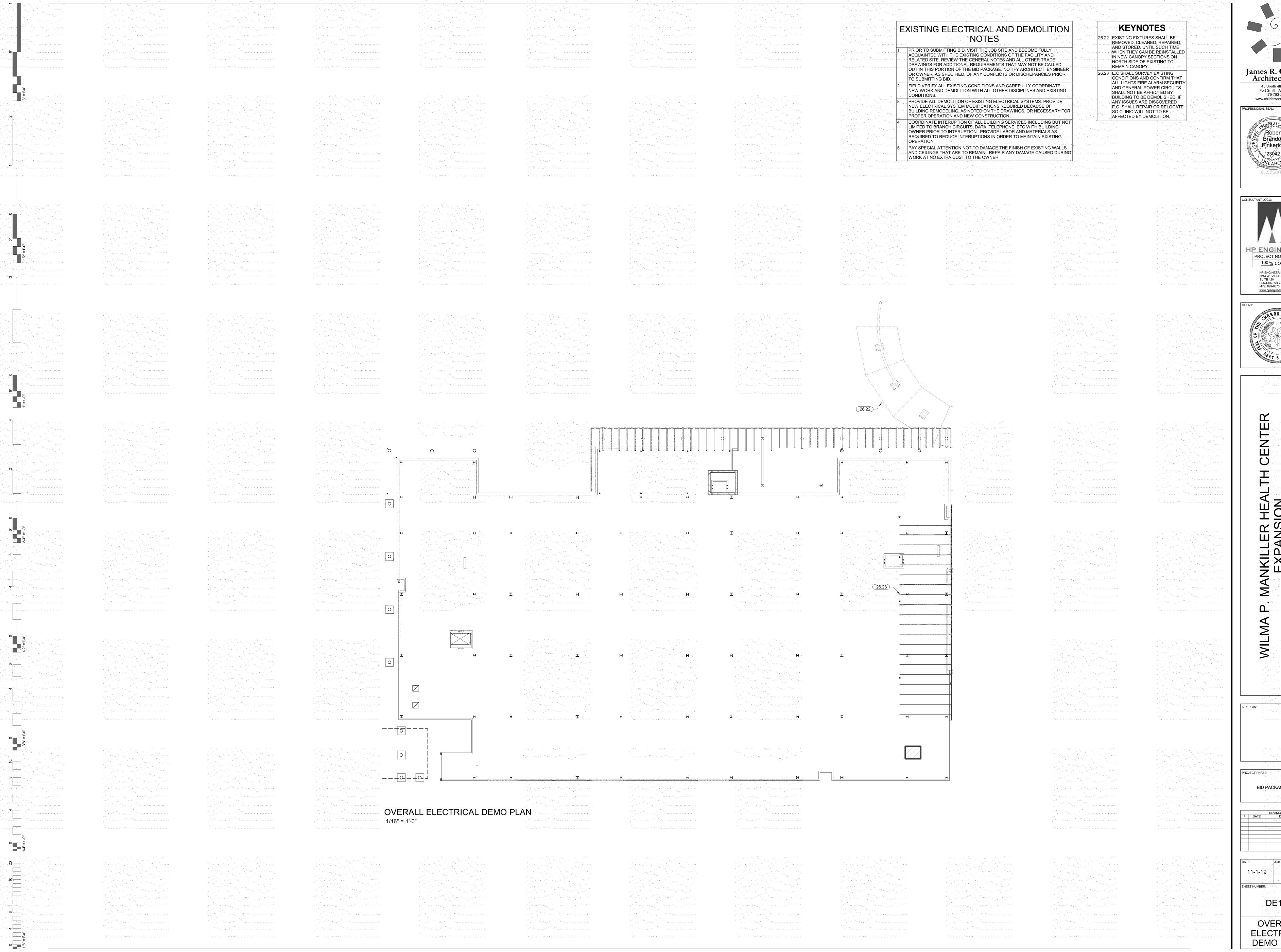
PROJECT PHASE: STEEL DETAILS

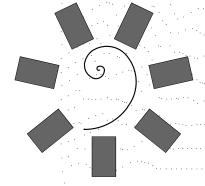
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SIDEPLATE MISCELLANEOUS DETAILS

Misc ID

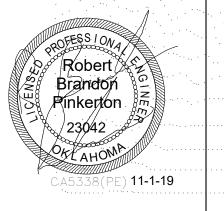
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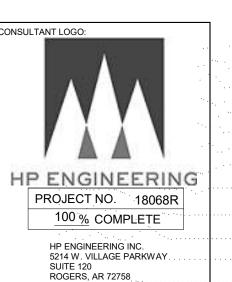


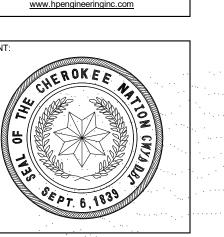


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