

July 23, 2012

Cherokee Nation Main Warehouse 22361 Bald Hill Road Tahlequah, OK 74464

Attn: Ms. Vivian Kernes, Manager – Roads Operations

Re: Soil Boring Services – Four (4) Sites P.O. #125092 Tahlequah, Oklahoma ATSI Project No.: CN53012

Dear Ms. Kernes;

This letter report was prepared to summarize the results of a field investigation performed for the above referenced project. This investigation was authorized by a Cherokee Nation Purchase Order (PO #125092). The four (4) Cherokee Nation projects drilled during this investigation included;

- Hulbert/Lost City Rd. Cherokee County Project No. CN-0914-02
- CNI Industrial Access Rd. Adair County Project No. CN-0807-09
- Lyon Switch Rd. Adair County Project No. CN-0827-06
- Dreadfulwater Rd. Cherokee County CN-0957-07

It is understood this project will include design and construction of new roadways for the above existing alignments. The primary focus of this investigation was to characterize the existing subsurface conditions by field drilling and laboratory testing. Details regarding the field investigation and laboratory testing performed, as well as a summary of classification tests, are presented in the following sections of this report.

Field Investigation

Subsurface conditions at the sites were investigated by drilling a total of thirty (30) borings. Boring locations were selected and staked in the field by the Client's surveyor and are shown in Appendix I: Boring Locations Plans. It should be noted that all borings are designated with a "TH" followed by the boring number. For organization purposes all boring logs and references to boring locations in this report use a different lettering scheme followed by the same number given by Cherokee Nation. As previously mentioned, this numbering scheme was used to so that samples and logs could be organized more efficiently. All borings were drilled within or directly adjacent to the existing roadway to the depth specified by the Client, which ranged from 5 to 15 ft. below the existing ground surface. Borings were drilled using 4.5-inch O.D. continuous flight augers powered by a truck mounted BK-51 drill rig. Three (3) boring locations required the use

of rock coring procedures to extend the borings to the specified depth. Rock coring was performed using an NQ2 core barrel and diamond impregnated core bit. Following drilling, borings were backfilled and pavement patched, if present. Boring logs presenting the soil and rock types encountered, as well as results of laboratory testing (described below) are presented in Appendix II.

Laboratory Testing

Collected sampled were transported back to the laboratory for further evaluation and testing. Laboratory testing performed upon selected soil samples included grain size analyses and Atterberg Limit values. These tests were performed for verification of soil classification. Rock core obtained was logged and evaluated for recovery and rock quality designation (RQD). Laboratory testing results are presented on the Boring Logs in Appendix I. A summary of all laboratory soil testing is presented in the table below.

Project Location	Boring Number/Depth (ft)	Liquid Limit, LL	Plastic Limit, PL	Plasticity Index, PI	USCS Classification	USCS Symbol
Hulbert/Lost City	HL-1 / 0.5 -2	25	13	12	Clayey Sand w/ Gravel	SC
Hulbert/Lost City	HL-3 / 3-4.5	50	18	32	Sandy Fat Clay	СН
Hulbert/Lost City	HL-4 / 0.5-1.3	NP	NP	NP	Silty Sand w/ Gravel	SM
Hulbert/Lost City	HL-4 / 3-4.5	49	17	32	Clayey Sand	SC
Hulbert/Lost City	HL-5 / 3.5-5	NP	NP	NP	Clayey Sand w/ Gravel	SC
Hulbert/Lost City	HL-6 / 3.5-4.4	NP	NP	NP	Silty Sand w/ Gravel	SM
Hulbert/Lost City	HL-7 / 0.5-1.5	23	19	4	Silty, Clayey Sand w/ Gravel	SC-SM
Hulbert/Lost City	HL-8 / 0.5-2	34	26	8	Silty Sand w/ Gravel	SM
Hulbert/Lost City	HL-10 / 0.5-2	NP	NP	NP	Silty Gravel w/ Sand	GM
Hulbert/Lost City	HL-10 / 3.5-4.6	64	20	44	Clayey Sand w/ Gravel	SC
Lyon Switch	LS-1 / 3.5-5	30	14	16	Lean Clay	CL
Lyon Switch	LS-3 / 1-2.5	40	17	23	Lean Clay	CL
Lyon Switch	LS-4 / 0.5-2	NP	NP	NP	Gravelly Lean Clay w/ Sand	CL
Lyon Switch	LS-5 / 0.5-1.4	NP	NP	NP	Silty Sand w/ Gravel	SM
Lyon Switch	LS-6 / 1-2.5	23	15	8	Lean Clay w/ Gravel	CL
Lyon Switch	LS-7 / 0.5-2	61	24	37	Fat Clay	CH
Lyon Switch	LS-8 / 0.5-2	19	17	2	Silt	ML
Lyon Switch	LS-8 / 3.5-5	44	21	23	Lean Clay	CL
Lyon Switch	LS-9 / 3.5-5	28	12	16	Lean Clay	CL
Lyon Switch	LS-10 / 0.5-2	24	18	6	Gravelly Silty Clay	CL-ML

Summary of Soil Laboratory Testing

Note: NP = Non-Plastic

	Buillin	ury or bom	Laboratory	Testing C	ont u	
Project Location	Boring Number/Depth (ft)	Liquid Limit, LL	Plastic Limit, PL	Plasticity Index, PI	USCS Classification	USCS Symbol
Dreadfulwater	DW-1/0.5-1.4	NP	NP	NP	Silty Gravel w/ Sand	GM
Dreadfulwater	DW-1/3.5-3.8	NP	NP	NP	Sandy Silt w/ Gravel	SM
Dreadfulwater	DW-2 / 0.5-1.4	NP	NP	NP	Silty Gravel w/ Sand	GM
Dreadfulwater	DW-3 / 0.5-1.4	NP	NP	NP	Poorly Graded Gravel w/ Silt & Sand	GP-GM
Dreadfulwater	DW-4 / 1-2.5	NP	NP	NP	Silty Gravel w/ Sand	GM
Dreadfulwater	DW-4 / 3-4.5	NP	NP	NP	Silty Gravel w/ Sand	GM
Dreadfulwater	DW-5 / 0.5-2	27	16	11	Lean Clay	CL
Dreadfulwater	DW-5 / 3.5-5	27	17	20	Lean Clay	CL
CNI Industrial Access Rd.	CN-1/0-1.3	NP	NP	NP	Gravelly Silt w/ Sand	ML
CNI Industrial Access Rd.	CN-2 / 0-1.5	NP	NP	NP	Silt w/ Gravel	ML
CNI Industrial Access Rd.	CN-3 / 0-1.5	19	16	3	Silt	ML
CNI Industrial Access Rd.	CN-3 / 3.5-5	43	19	24	Lean Clay w/ Gravel	CL
CNI Industrial Access Rd.	CN-4 / 0-0.9	NP	NP	NP	Clayey Sand w/ Gravel & Asphalt Debris	SM
CNI Industrial Access Rd.	CN-5 / 1-2.5	26	14	12	Lean Clay w/ Gravel	CL
CNI Industrial Access Rd.	CN-5 / 3.5-5	28	13	15	Lean Clay	CL

Summary of Soil Laboratory Testing - Cont'd

Note: NP = Non-Plastic

Letter Report Limitations

This letter report has been prepared in accordance with generally accepted practices of other consultants undertaking similar studies at the same time and in the same geographical area. Adobe Testing Services, Inc., observed that degree of care and skill generally exercised by other consultants under similar circumstances and conditions. Adobe Testing's findings and conclusions must be considered not as scientific certainties, but as opinions based on our professional judgment concerning the significance of the data gathered during the course of this investigation. Other than this, no warranty is implied or intended.

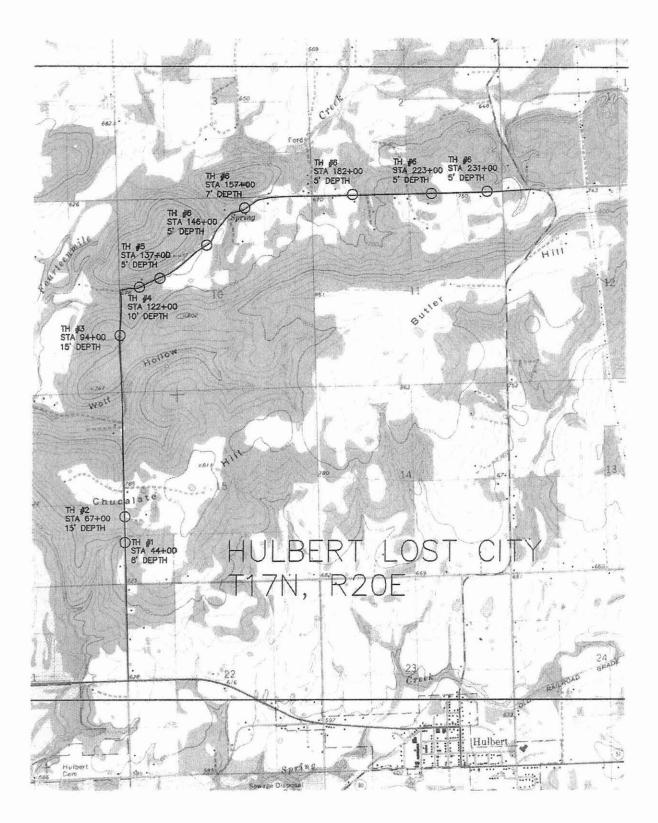
ADOBE TESTING SERVICES, INC. By:



Attachments:

Appendix I – Boring Location Plans Appendix II – Borings Logs Appendix III – General Notes Appendix IV – Grain Size Analysis Results

APPENDIX I BORING LOCATION PLANS



APPENDIX II BORING LOGS

TEST		e P	616 S. 122nd Ave. Tulsa, OK 74146 Ph.: (918) 872-9898 ax: (918) 994-6390					-	B	ORING NUME		HI	L -1
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DEPTH (ft)	DRILLING METHOD	STRATA SYMBOL	MATERIAL DES Unified Soil Classif			SAMPLE TYPE NUMBER	RECOVERY % (RQD %)	CORRECTED BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	20 40 20 4 PL 20 4 SHEAR S	HO 60	100 80 LL H 80	ELEVATION
0		1.1.1.1.	¬ ASPHALT (3")		0.3 ft						<u> 2 3</u>	4	
-			CLAYEY SAND w/ GRAVEL Medium Dense, Slightly Moi	_, Brown Red & Ta ist (SC)	an, 2.0 ft	SPT 1		14-8-8 (16)		I ▲ I			
-			LEAN CLAY w/ SAND, Brow Shaley (CL)	vn Tan & Red, Stif									
_	- 4.5" O.D					SPT 2		7-10-6 (16)		•			
	CFA				5.0 ft								
5			CHERT, Hard		604								
_	-		SANDY LEAN CLAY, Browr	n Tan. Verv Stiff to	6.0 ft Hard.								
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			Bottom of boreh	ala at 9.0 faat	8.0 ft								

	obe		T F	5616 S. 122nd Ave. Fulsa, OK 74146 Ph.: (918) 872-9898	GEO BO	TECI RINC			-	B	ORING NUMBER	HL	2
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LOG NOTI HL(1) 0 	2" I.D. CFA - 4.			Slightly Moist (CL) LIMESTONE, Tan Gray, Me Slightly Weathered, Fine Cr Occasional Shale Partings -Weathered & Soft from 4' to LIMESTONE, Light & Dark Moderately Hard, Fine Crys Interbedded Limestone & St	ystalline, Thick Bed o 4.5' Gray, Medium to talline, Thick Bedde	ded, w/	NQ 1	92 (63)					
	.		+ 1										
	ARREL		+			0 5 4							
	CORE BAF			SILTSTONE, Gray, Medium Thick Bedded, Slightly Calc Shale Partings SILTSTONE, Dark & Light (Crystalline, Thick Bedded, v Layers	areous, w/ Numerou Gray, Medium Hard,	us 10.2 ft Fine	NQ 2	100 (63)					
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15				Bottom of boreh	ole at 15.0 feet.								

			T P	616 S. 122nd Julsa, OK 741 h.: (918) 872-9 ax: (918) 994-	46 9898	GEO BO	TECI RING		-	-	B	ORING NUM	IBER	PAG	HL	
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5			××××××××××××××××××××××××××××××××××××××		E, Highly Weathe	ered, Soft Gray, Medium Hard,	7.0 ft									
		×	×	Weathered	to Slightly Weath ightly Calcareous	nered, Fine Grained	d, Thick 9.0 ft	NQ 1	55 (55)							
 10 	RE BARREL - 2" I.D.			SANDSTOI Slightly We Bedded	NE, Tan Brown C athered, Fine to	Gray, Medium Hard, Medium Grained, T	hick	NQ 2	100 (53)							
	CORE			SANDY SH Thick Bedd	ALE, Tan Brown ed	Gray, Soft, Weath	13.0 ft ered,									
							15.5 ft	NQ 3	100 (0)							
15					Bottom of boreh	JIE AL ID.D IEEL										

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ž 0			ASPHALT (2")		0.2 #					:	<u> </u>	3	4	
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	CFA - 4.5"				7 5 4									
			SHALE, Soft, Highly Weath	orod	7.5 ft									
<u> </u>			SHALE, Soit, highly weath	ered								:		
					10.0 ft	SPT 3		27-16-14 (30)	-					
			Bottom of boreho	ble at 10.0 feet.										

adobe TESTING SERVICES, IN	5616 S. 122nd Ave. Tulsa, OK 74146 Ph.: (918) 872-9898 IC. Fax: (918) 994-6390	GEO BO	TECH RING			-	В	ORING NUMBER	HL	L-5
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CFA - 4.5" O.D.	ASPHALT (3") CLAYEY SAND w/ GRAVEL	., Brown Tan, Dens	0.3 ft se (SC) 5.0 ft	SPT 1 SPT 2		23-23-12 (35) 12-20-30 (50)	-			
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			ASPHALT (SILTY, CLA Dense, Slig		GRAVEL, Tan Brow M)	0.3 ft_ /n, Very	SPT 1	•	15-33- 50/0"	-	H			•
CDOBE PROJECTS/CN63012	CFA - 4.5" O.D		SANDY LE/ Moist (CL)	AN CLAY w/ GR	AVEL, Brown Tan,	<u>3.0 ft</u> Stiff,	SPT 2	_	33-25-	-				▲
						7.0 ft	3		50/1"	-				
OR O		<u> </u>	1	Bottom of boreh	ole at 7.0 feet.									

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∃ ∃	M	MER T	YPE_C	Cathe	ad	_		1	AT TIME	E OF I	ORILLING	None				
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		е	5616 S. 122nd Ave. Tulsa, OK 74146 Ph.: (918) 872-9898 Fax: (918) 994-6390)TEC)RIN(<u>.</u>	B	ORING NUMBER	HI PAGE 1 (L-9
	NT Cher	okee Na	ation			PROJ		ME H	lulbert/Lo:	st City	- Project No. CN-09	914-02	
	JECT NO.					-			DN Hulbe		-		
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DRIL	LER MA		DR		K-51 TM	GROU	ND WA	TER L	EVELS				
HAN	IMER TYP	E Cath	ead					OF	RILLING	None	9		
LOG	GED BY	RA	СН	ECKED BY	BP	/	AT END	OF D	RILLING				
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DEPTH (ft)	DRILLING METHOD	TA :	Unified	Soil Classifi	cation System			RECOVERY (RQD %)	ORRECTE OW COUN (N VALUE)	CKE (ts			ELEVATION (ff)
		STRATA					SAMPLE TYPE NUMBER	RE(CORRECTED BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)			┤┙
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			SANDY LEAN C Dry (CL)	LAY w/ GR/	AVEL, Brown Red	, Stiff,	SPT 1		50/1"				Ţ
0			y (=)										
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	4 - 4.												
	CFA					4.0 ft	SPT		50/1"				▲
			GRAVELLY FAT	CLAY Rec	d Brown, Stiff, Slig		2		50/1				
			Moist (CH)	02,11,100		5.0 ft							
5			Botto	om of boreh	ole at 5.0 feet.		•				<u>.</u>		
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	adob	-	5616 S. 122nd Tulsa, OK 741 Ph.: (918) 872-	146		DTEC DRING			-	B	ORING NUMBE	R	HL-	10
TEST	TING SERVICE		Fax: (918) 994	-6390				•				PA	GE 1 O)F 1
CLIE	ENT Chero	okee Na	ation			_ PROJI	ECT NA	ME_	Hulbert/Lo	st City	- Project No. Cl	N-0914-02		
PRC	JECT NO.	CN53	012			_ PROJI	ECT LO	CATI	ON Hulbe	rt, OK				
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DEPTH (ft)	DRILLING METHOD			MATERIAL DE			SAMPLE TYPE NUMBER	RECOVERY % (RQD %)	CORRECTED BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)			.L	₹¥
	ME ⁻	STRATA	U	nified Soil Class	ification System		MPI	NOR NOR		NO NO		-0	.∟ 1 80	ELEVATION
		STR					S	R	BLO	۲ ۲				<u>ш</u>
0				(0))		0.3 <u>ft</u>					1 2	3	4	
		000	ASPHALT SILTY GR		Red Brown, Dense	/		-		-				
		Pap	Moist (GM)			, e.g,	SPT		14-17-12 (29)				•	
	O.O					2.0 ft			(20)	_				
	4.5"		CLAYEY S Moist (SC)		L, Red Brown, Ver	y Dense,								
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	ō						SPT	-	30-45-	1		i-I		
							<u>2</u>		50/0"	1				
5				Dottom of horo	hole at 5.0 feet.	5.0 ft								
CLIE PRC DAT DRII HAM LOG NOT HLd3Q 0 														
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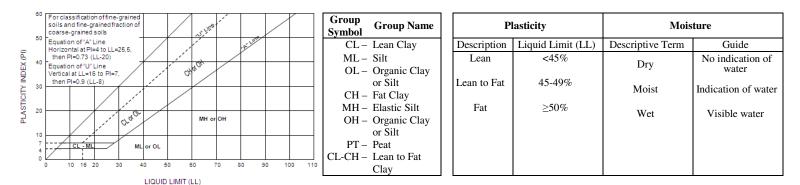
APPENDIX III GENERAL NOTES

GENERAL NOTES



SOIL PROPERTIES & DESCRIPTIONS

COHESIVE SOILS			
Consistency	Unconfined Compressive Strength (Qu)	Pocket Penetrometer Strength	N-Value
	(psf)	(tsf)	(blows/ft)
Very Soft	<500	<0.25	0-1
Soft	500-1000	0.25-0.50	2-4
Medium Stiff	1001-2000	0.50-1.00	5-8
Stiff	2001-4000	1.00-2.00	9-15
Very Stiff	4001-8000	2.00-4.00	16-30
Hard	>8000	>4.00	31-60
Very Hard			>60



Fine Grained Soil Subclassification	Percent (by weight) of Total Sample
Terms: SILT, LEAN CLAY, FAT CLAY, ELASTIC SILT	PRIMARY CONSTITUENT
Sandy,gravelly, abundant cobbles, abundant boulders with sand, with gravel, with cobbles, with boulders scattered sand, scattered gravel, scattered cobbles, scattered boulders a trace sand, a trace gravel, a few cobbles, a few boulders	>30-50] >15-30] – secondary coarse grained constituents 5-15] <5]
The relationship of clay and silt constituents is based on plasticity and no based on Atterberg Limits tests and the Plasticity Chart.	ormally determined by performing index tests. Refined classifications are

NON-COHESIVE (GRANULAR) SOILS

				**GRAIN SIZE IDENTIFICATION		
				Name	Size Limits	Familiar Example
RELATIVE DENSITY	N-VALUE	MOISTU	JRE CONDITION	Boulder Cobbles Coarse Gravel	12 in. or more 3 in. to 12 in. ³ 4-in. to 3 in.	Larger than basketball Grapefruit Orange or lemon
		Descriptive Term	Guide	Fine Gravel	No. 4 sieve to $\frac{3}{4}$ -in.	Grape or pea
Very Loose Loose Medium Dense Dense Very Dense	0-4 5-10 11-24 25-50 ≥51	Dry Moist Wet	No indication of water Damp but no visible water Visible free water, usually soil is below water table.	Coarse Sand Medium Sand Fine Sand* Fines	No. 10 sieve to No. 4 sieve No. 40 sieve to No. 10 sieve No. 200 sieve to No. 40 sieve Less than No. 200 sieve	Rock salt Sugar, table salt Powdered sugar
· · · ·				*Particles finer t	han fine sand cannot be discerned	with the naked eye at

*Particles finer than fine sand cannot be discerned with the naked eye as a distance of 8 in.

Coarse Grained Soil Subclassification	Percent (by weight) of Total Sample	
Terms: GRAVEL, SAND, COBBLES, BOULDERS	PRIMARY CONSTITUENT	
Sandy, gravelly, abundant cobbles, abundant boulders	>30-50]	
with gravel, with sand, with cobbles, with boulders	>15-30] – secondary coarse grained constituents	
scattered gravel, scattered sand, scattered cobbles, scattered boulders	5-15]	
a trace gravel, a trace sand, a few cobbles, a few boulders	<5]	
Silty (MH & ML)*, clayey (CL & CH)*	<15]	
(with silt, with clay)*	5-15] – secondary fine grained constituents	
(trace silt, trace clay)*	<5]	
*Index tests and/or plasticity tests are performed to determine whether the term "silt" or "clay" is used.		

GENERAL NOTES



Approx. Unconfined

Compressive Strength (tsf)

2.6-10

10-50

50-260

260-520

520-1040

1040-2610

>2610

BEDROCK PROPERTIES & DESCRIPTIONS

ROCK QUALITY DESIGNATION (RQD)		
Description of Rock Quality	<u>*RQD (%)</u>	
Very Poor	< 25	
Poor	25-50	
Fair	50-75	
Good	75-90	
Excellent	90-100	
*RQD is defined as the total length of sound core		

pieces 4 in. or greater in length, expressed as a percentage of the total length cored. RQD provides an indication of the integrity of the rock mass and relative extent of seams and bedding planes.

shaved with a knife.

Slightly

Weathered

Weathered

Highly Weathered

Pit

Vug

Cavity

Cave

DEGREE OF WEATHERING

core rings under hammer impact.

be broken by hand or scraped by knife.

VOIDS

Voids 6 to 50mm (1/4 to 2 in) in diameter 50 to 6000mm (2 to 24 in) in diameter

Voids barely seen with naked eye to 6mm (1/4-in)

Rock generally fresh, joints stained and discoloration extends

Rock mass is decomposed 50% or less, significant portions of rock show discoloration and weathering effects, cores cannot

basically restricted to soil containing little to no chert fragments and to softer shale deposits.

into rock up to 25mm (1 in), open joints may contain clay,

Rock mass is more than 50% decomposed, complete discoloration of rock fabric, core may be extremely broken and gives clunk sound when struck by hammer, may be

Term	Field Identification
Extremely Soft	Can be indented by thumbnail
Very Soft	Can be peeled by pocket knife
Soft	Can be peeled with difficulty by pocket knife
Medium Hard	Can be grooved 2 mm deep by firm pressure of knife
Moderately Hard	Requires one hammer blow to fracture
Hard	Can be scratched with knife or pick only with difficulty
Very Hard	Cannot be scratched by knife or sharp pick

GRAIN SIZE (TYPICALLY FOR SEDIMENTARY ROCKS)			
Description	Diameter (mm)	Field Identification	
Very Coarse Grained	>4.76		
Coarse Grained	2.0-4.76	Individual grains can easily be distinguished by eye.	
Medium Grained	0.42-2.0	Individual grains can be distinguished by eye.	
Fine Grained	0.074-0.42	Individual grains can be distinguished by eye with difficulty.	
Very Fine Grained	<0.074	Individual grains cannot be distinguished by unaided eye.	

BEDDING THICKNESS

Very Thick Bedded	> 3' thick
Thick Bedded	1' to 3' thick
Medium Bedded	4" to 1' thick
Thin Bedded	1¼" to 4" thick
Very Thin Bedded	1/2" to 11/4" thick
Thickly Laminated	¹ / ₈ " to ¹ / ₂ " thick
Thinly Laminated	$\frac{1}{8}$ " or less (paper thin)

SCALE OF RELATIVE ROCK HARDNESS

DRILLING NOTES

Drilling and Sampling Symbols

NO - Rock Core (2-in. diameter)

- HQ Rock Core (3 in. diameter)
- HSA Hollow Stem Auger

>600mm

- CFA Continuous Flight (Solid Stem) Auger SS - Split Spoon Sampler
- WB Wash Bore or Mud Rotary TP - Test-Pit
 - HA Hand Auger

Soil Sample Types Shelby Tube Samples: Relatively undisturbed soil samples were obtained from the borings using thin wall (Shelby) tube samplers pushed hydraulically into the soil in advance of drilling. This sampling, which is considered to be undisturbed, was performed in accordance with the requirements of ASTM D 1587. This type of sample is considered best for the testing of "in-situ" soil properties such as natural density and strength characteristics. The use of this sampling method is

ST - Shelby Tube

Split Spoon Samples: The Standard Penetration Test is conducted in conjunction with the split-barrel sampling procedure. The "N" value corresponds to the number of blows required to drive the last 1 foot of an 18-in. long, 2-in. O.D. split-barrel sampler with a 140 lb. hammer falling a distance of 30 in. The Standard Penetration Test is carried out according to ASTM D-1586.

Water Level Measurements

Water levels indicated on the boring logs are levels measured in the borings at the times indicated. In permeable materials, the indicated levels may reflect the location of groundwater. In low permeability soils, shallow groundwater may indicate a perched condition. Caution is merited when interpreting short-term water level readings from open bore holes. Accurate water levels are best determined from piezometers.

Automatic Hammer

Palmerton and Parrish's CME's are equipped with automatic hammers. The conventional method used to obtain disturbed soil samples used a safety hammer operated by company personnel with a cat head and rope. However, use of an automatic hammer allows a greater mechanical efficiency to be achieved in the field while performing a Standard Penetration resistance test based upon automatic hammer efficiencies calibrated using dynamic testing techniques

APPENDIX IV GRAIN SIZE ANALYSIS RESULTS

