

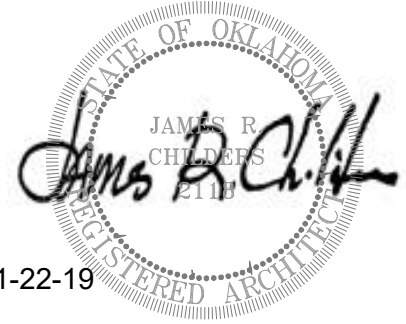


BID PACKAGE 01 – ADDENDUM 01

Date: November 22, 2019

Re: Wilma P Mankiller Health Center Expansion

From: James R Childers Architect, Inc.
45 South 4th Street
Fort Smith, Arkansas 72901



11-22-19

This addendum forms part of the Contract Documents, and modifies the documents as noted below. Acknowledge receipt of this addendum in the space provided on the bid form. Failure to do so may subject the bidder to disqualification.

Item 01 See attached revised structural drawings and narrative list from Chavez Grieves.

Bid Package 01- Addendum 01 – Wilma P. Mankiller Health Center Expansion

Chavez-Grieves would like to incorporate the following revisions into the drawings for the above referenced project.

<u>Sheet</u>	<u>Description</u>
S0.02	Revised floor loading information.
S0.03	Revised wind loading diagram.
S1.01	Sheet keynotes 6, 13, and 14 revised.
S1.01	Column size at Grid G/9 revised.
S1.01	Sections added at canopy near Grid A/7.
S1.01	Column and spread footing sizes updated at canopy near Grid A/7.
S1.02	Canopy footings and column sizes revised near Grid A/4.6.
S1.11	Column size at Grid G/9 revised.
S1.11	Beam along Grid 10, between Grids G and F updated.
S1.11	Beams between Grids 9 and 11, near Grid D updated.
S1.11	Pilaster tags added to masonry shaft wall.
S1.11	Canopy/lobby area top of steel elevations added near Grids A and C.
S1.11	Canopy/lobby area details and sections added near Grids A and C.
S1.11	Canopy/lobby area clarifying dimensions added near Grids A and C.
S1.12	Canopy/lobby area top of steel elevations added near Grids A and C.
S1.12	Canopy/lobby area details and sections added near Grids A and C.
S1.12	Canopy/lobby area clarifying dimensions added near Grids A and C.
S1.12	Operable partition support beam sizes updated near Grid D, between Grids 1 and 2.
S1.12	Detail D3/S5.53 added near Grid H/2.
S1.13	Sheet keynote 12 revised.
S1.13	Snapping point between Grid E and D corrected.
S1.13	Beams/girders supporting rooftop mechanical units revised.
S1.13	Framing between Grid G/8 and F/10 revised.
S1.13	Connection details at Grid G/4 and D/7 revised.
S1.13	Beam removed and bottom flange bracing (keynote 8) added to moment frame beam at Grid F/3.
S1.13	Connection details updated at Grids E/4, E/6, D/7, and F/5.
S1.13	Bottom flange bracing (keynote 9) added to girders along Grid F and E, between Grids 4/5 and 6/7, respectively.
S1.13	Beam/bottom flange bracing added at post near Grid G/7.
S1.21	Clarifying dimensions and sections added around interior and exterior roof perimeter.
S1.21	Location of enlarged framing plan tag D2/S4.01 adjusted.
S1.21	Beam sizes near Grid G/11 revised.
S1.21	Beams added near post at Grid G2/9.
S1.21	Beam sizes near Grid E/11 revised.

S1.21 Beam sizes near Grid D/11 revised.
S1.21 Beam details revised at Grid B/9 and D/6.
S1.22 Sheet keynote 7 added.
S1.22 Clarifying dimensions and sections added around interior and exterior roof perimeter.
S1.22 Bottom flange bracing (keynote 4) added along Grids D and G.
S1.22 Joist size updates made along Grid 1, between Grids G and E.
S1.22 Beam size updates made near Grid E/1.
S2.01 SidePlate sheet numbers added to elevation key.
S3.03 Section A1 revised.
S3.11 Sections C3 and D2 revised.
S3.12 Section C4 added.
S3.31 Sections A5, B3, B4, D5, and D2 revised.
S3.31 Sections A1 and B1 added.
S4.01 Plans A3, D2, and D4 revised.
S5.21 Detail D5 revised.
S5.52 Details A5 and B5 revised.
S5.53 Detail C3 revised.
S5.54 Sheet added.
S6.01 Footing F60A information revised.
S6.01 Baseplate schedule information revised.
S6.01 Deck schedule information revised.
S6.01 Masonry opening information revised.
S7.21 Details A3 and D4 revised.
S7.31 Detail C3 added.
S7.42 Detail C4 revised.

GENERAL STRUCTURAL NOTES

CODES AND MANUALS:
IBC-15 INTERNATIONAL BUILDING CODE 2015
ASCE/SEI 3-91 STRUCTURAL DESIGN OF COMPOSITE SLABS
ASCE/SEI 7-10 MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES
AISC 360-10 SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS
AISC 341-10 SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS
AISC MANUAL OF STEEL CONSTRUCTION 14TH EDITION
SJI-K-1.1-10 STANDARD SPECIFICATION FOR OPEN WEB STEEL JOISTS, K-SERIES
SJI-L-HDLH-1-10 STANDARD SPECIFICATION FOR LONGSPAN STEEL JOISTS, LH-SERIES AND DEEP LONGSPAN STEEL JOISTS, LH-SERIES
SJI-G-1.1-10 STANDARD SPECIFICATION FOR JOIST GIRDERS
SJI-CJ-1.0-10 STANDARD SPECIFICATION FOR COMPOSITE STEEL JOIST, CJ-SERIES
SDI DIAPHRAGM DESIGN MANUAL, 3RD EDITION
ANSI/SKI NC1 0-06 STANDARD FOR NONCOMPOSITE STEEL FLOOR DECK
ANSI/SKI R01 0-06 STANDARD FOR STEEL ROOF DECK
ANSI/SKI C1 0-06 STANDARD FOR COMPOSITE STEEL FLOOR DECK
AISI S100-12 NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS
AISI S200-12 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - GENERAL PROVISIONS
AISI S210-12 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - FLOOR AND ROOF SYSTEM DESIGN
AISI S211-07 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - WALL STUD DESIGN WITH 2012 SUPPLEMENT
AISI S212-12 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - HEADER DESIGN
AISI S213-12 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - LATERAL DESIGN WITH 2010 SUPPLEMENT
AISI S214-12 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - TRUSS DESIGN, WITH SUPPLEMENT 2, DATED 2008
MBMA METAL BUILDING SYSTEMS MANUAL, 2007 EDITION
ACI 318-14 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE
PCI DESIGN HANDBOOK - MANUAL OF DESIGN FOR PRECAST/PREFABRICATED CONCRETE
ACI 530-13 BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES
ACI 530.1-13 SPECIFICATIONS FOR MASONRY STRUCTURES
AWS D1.1-04 STRUCTURAL WELDING CODE - STEEL
AWS D1.3-98 STRUCTURAL WELDING CODE - SHEET STEEL
AWS D1.4-11 STRUCTURAL WELDING CODE - REINFORCING STEEL

DESIGN CRITERIA:
VERTICAL:
LIVE LOAD
FLOOR
STAIRS AND EXIT-WAYS*
MINIMUM CONCENTRATED LOAD
ADDITIONAL SUPERIMPOSED LOADS
PARTITIONS
SUSPENDED EQUIPMENT
CONCENTRATED LOAD
ROOF LIVE LOAD: LR = 20'R1'R2
REDUCTION FACTOR BASED ON TRIB AREA
REDUCTION FACTOR BASED ON ROOF SLOPE
SNOW LOAD
GROUND SNOW LOAD
FLAT ROOF SNOW LOAD**
SNOW EXPOSURE FACTOR
SNOW LOAD IMPORTANCE FACTOR
THERMAL FACTOR
**INCLUDES 5 PSF RAIN-ON SNOW SURCHARGE LOAD
HORIZONTAL:
WIND
ULTIMATE DESIGN WIND SPEED
RISK CATEGORY
INTERNAL PRESSURE COEFFICIENT
NATURAL FREQUENCY
STRUCTURE IS FLEXIBLE
SEISMIC
SEISMIC IMPORTANCE FACTOR
MAPPED SPECTRAL RESPONSE ACCELERATIONS
SHORT PERIOD
1 SECOND PERIOD
SITE CLASS
SPECTRAL RESPONSE COEFFICIENTS
SHORT PERIOD
1 SECOND PERIOD
SEISMIC DESIGN CATEGORY
BASIC SEISMIC FORCE RESISTING SYSTEM: STRUCTURAL STEEL SYSTEMS
STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE
SEISMIC RESPONSE COEFFICIENT
RESPONSE MODIFICATION FACTOR
DESIGN BASE SHEAR
ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE
BASIC SEISMIC FORCE RESISTING SYSTEM: REINFORCED MASONRY SYSTEMS
INTERMEDIATE REINFORCED MASONRY SHEAR WALLS
SEISMIC RESPONSE COEFFICIENT
RESPONSE MODIFICATION FACTOR
DESIGN BASE SHEAR
ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE
ALLOWABLE SOIL BEARING PRESSURE =
(STONE COLUMNS REQUIRED TO ACHIEVE BEARING PRESSURE)
FROST DEPTH =
FUTURE BUILDING EXPANSION:
GENERAL:
STRUCTURAL DRAWINGS ARE NOT STAND-ALONE DOCUMENTS AND ARE INTENDED TO BE USED IN CONJUNCTION WITH CIVIL, ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND DRAWINGS FROM OTHER DISCIPLINES: THE CONTRACTOR SHALL COORDINATE ALL REQUIREMENTS OF THE CONTRACT DOCUMENTS INTO THE SHOP DRAWINGS AND FIELD WORK.
COORDINATE DIMENSIONS OF ALL OPENINGS, DEPRESSIONS, BLOCKOUTS, ETC. WITH ARCHITECTURAL DRAWINGS, DRAWINGS FROM OTHER DISCIPLINES, PROJECT SHOP DRAWINGS, AND FIELD CONDITIONS PRIOR TO SHOP DRAWING SUBMITTAL. THE STRUCTURAL DRAWINGS ONLY REPRESENT A PORTION OF THE REQUIREMENTS FOR THE PROJECT.
SEE ARCHITECTURAL PLANS FOR INTERIOR NON-BEARING PARTITION WALLS. PARTITION FRAMING SHALL BE CONNECTED TO THE PRIMARY STRUCTURE TO ALLOW FOR VERTICAL LIVE LOAD DEFLECTIONS OF SPAN/360 FOR FLOOR FRAMING AND SPAN/240 FOR ROOF FRAMING.
CONTRACTOR'S CONSTRUCTION AND/OR ERECTION SEQUENCES SHALL RECOGNIZE AND CONSIDER THE EFFECTS OF THERMAL MOVEMENTS OF STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PERIOD.
THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS IN THE FIELD.
SHOP DRAWINGS SHALL BE FURNISHED AND REVIEWED BEFORE ANY FABRICATION OR ERECTION IS STARTED. THE CONTRACTOR SHALL REVIEW AND APPROVE SHOP DRAWINGS PRIOR TO SUBMITTAL TO THE ARCHITECT FOR REVIEW. POORLY EXECUTED SHOP DRAWINGS WILL BE REJECTED AND SHALL BE RESUBMITTED.
THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING SAFE AND ADEQUATE SHORING FOR ALL PARTS OF THE STRUCTURE DURING CONSTRUCTION.
TEMPORARY PROVISIONS SHALL BE MADE FOR STRUCTURAL STABILITY DURING CONSTRUCTION. THE STRUCTURE SHOWN ON THE DRAWINGS HAS BEEN DESIGNED FOR STABILITY UNDER FINAL CONFIGURATION.
NOTCHING OR CUTTING ANY STRUCTURAL MEMBER IN THE FIELD IS PROHIBITED.
THE CONTRACTOR SHALL VERIFY THE SIZE AND LOCATION OF FOUNDATIONS UNDER MECHANICAL AND ELECTRICAL EQUIPMENT AS REQUIRED. NO CONCRETE PADS SHALL BE LOCATED ON ROOF UNLESS SHOWN ON THE STRUCTURAL DRAWINGS.
BACKFILL SHALL NOT BE PLACED BEHIND RETAINING WALLS UNTIL CONCRETE HAS ATTAINED 100 PERCENT OF DESIGN STRENGTH.
BACKFILL SHALL NOT BE PLACED BEHIND BASEMENT WALLS UNTIL THE CONCRETE HAS ATTAINED 100 PERCENT OF DESIGN STRENGTH AND THE ELEVATED FLOOR PROVIDING LATERAL SUPPORT AT THE TOP OF THE WALL IS COMPLETELY CONSTRUCTED, OR TEMPORARY BRACING/SHORING OF THE WALL IS PROVIDED. DESIGN OF ANY TEMPORARY WALL BRACING/SHORING IS THE RESPONSIBILITY OF THE CONTRACTOR.
REMOVAL OF FORMS AND SHORING SHALL BE IN ACCORDANCE WITH ACI 347. WHERE CONCRETE MUST SUPPORT SUPERIMPOSED LOADS PRIOR TO ATTAINING THE SPECIFIED DESIGN STRENGTH, RESHORE CONCRETE IN ACCORDANCE WITH ACI 347. RESHORING SHALL NOT BE REMOVED SOONER THAN 28 DAYS FROM THE DATE OF POUR OR UNTIL CONCRETE HAS ATTAINED THE SPECIFIED DESIGN STRENGTH.
THE CONTRACTOR SHALL SUBMIT FOR PRIOR APPROVAL THE END OF POUR LOCATIONS FOR CONCRETE GRADE BEAMS, CONCRETE COLUMNS, AND CONCRETE BEAMS.

GENERAL STRUCTURAL NOTES

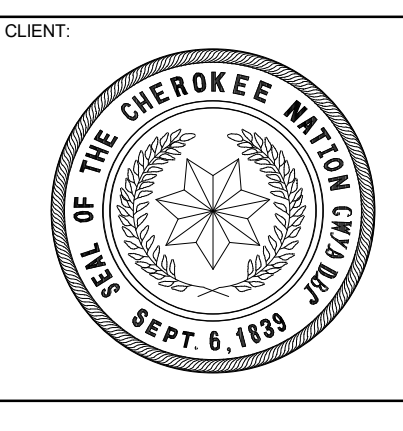
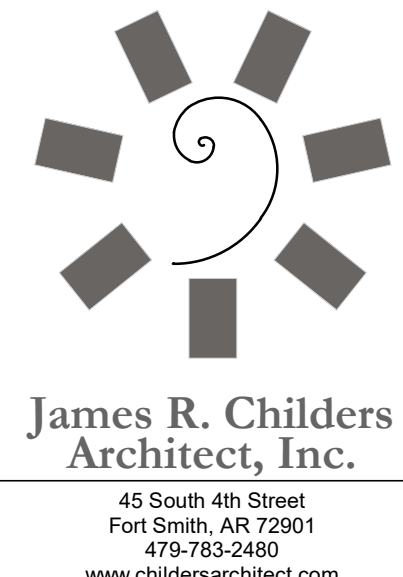
THE CONTRACTOR SHALL BE RESPONSIBLE FOR ADHERING TO ALL APPLICABLE STANDARDS SET FORTH BY OSHA, INCLUDING THE FOLLOWING REQUIREMENTS FROM STANDARDS - 29 CFR, SECTION 1926, SUBPART R:
A. THE STEEL ERECTION CONTRACTOR SHALL NOT ERECT STEEL UNLESS THEY HAVE RECEIVED WRITTEN NOTIFICATION FROM THE CONTRACTOR THAT THE CONCRETE IN THE FOOTINGS, PIERS AND WALLS OR THE MORTAR IN THE MASONRY PIERS AND WALLS HAS ATTAINED, ON THE BASIS OF AN APPROPRIATE ASTM STANDARD TEST METHOD OF FIELD-CURED SAMPLES, EITHER 75 PERCENT OF THE INTENDED MINIMUM COMPRESSIVE DESIGN STRENGTH OR SUFFICIENT STRENGTH TO SUPPORT THE LOADS IMPOSED DURING STEEL ERECTION.
PROVIDE STRUCTURAL ENGINEER A COPY OF WRITTEN NOTIFICATION WHEN IT IS PROVIDED TO THE STEEL ERECTOR.
B. ANCHOR RODS (ANCHOR BOLTS) SHALL NOT BE REPAIRED, REPLACED OR FIELD-MODIFIED WITHOUT THE APPROVAL OF THE PROJECT STRUCTURAL ENGINEER OF RECORD.
PRIOR TO ERECTION OF COLUMNS, THE CONTRACTOR SHALL PROVIDE WRITTEN NOTIFICATION TO THE STEEL ERECTOR IF THERE HAS BEEN ANY REPAIR, REPLACEMENT OR MODIFICATION OF THE ANCHOR RODS (ANCHOR BOLTS).
PROVIDE STRUCTURAL ENGINEER A COPY OF WRITTEN NOTIFICATION WHEN IT IS PROVIDED TO THE STEEL ERECTOR.
C. NO MODIFICATION THAT AFFECTS THE STRENGTH OF A STEEL JOIST OR STEEL JOIST GIRDER SHALL BE MADE WITHOUT THE APPROVAL OF THE PROJECT STRUCTURAL ENGINEER OF RECORD.
D. METAL DECKING HOLES AND OPENINGS SHALL NOT BE CUT UNTIL IMMEDIATELY PRIOR TO BEING PERMANENTLY FILLED WITH THE EQUIPMENT OR STRUCTURE, OR SHALL BE IMMEDIATELY COVERED.
PROTECTION: PROPER PRECAUTIONS SHALL BE TAKEN AT ALL TIMES TO PROTECT VEHICULAR AND PEDESTRIAN TRAFFIC FROM ANY DAMAGE OR INJURY WHICH MAY BE CAUSED, EITHER DIRECTLY OR INDIRECTLY, BY THE WORK INCLUDED ON THESE DRAWINGS. SUCH PRECAUTIONS SHALL INCLUDE THE ERECTION AND MAINTENANCE OF FENCES, BARRICADES, RAILINGS, GUARDS, SIGNS, COVERINGS, LIGHTS, AND OTHER PRECAUTIONS AS MAY BE REQUIRED. IF AT ANY TIME THE OWNER OR THE OWNER'S REPRESENTATIVE, PROPER PRECAUTIONS ARE NOT BEING TAKEN TO SECURE THIS PROTECTION, THE CONTRACTOR SHALL AT NO ADDITIONAL COST TO THE OWNER, INSTALL AND MAINTAIN SUCH ADDITIONAL PROTECTION AS MAY BE DIRECTED BY THE OWNER.
POLLUTION CONTROLS: USE WATER SPRINKLING, TEMPORARY ENCLOSURES, AND OTHER SUITABLE METHODS TO LIMIT DUST AND DIRT RISING AND SCATTERING IN THE AIR TO LOWEST PRACTICAL LEVEL. COMPLY WITH GOVERNING REGULATIONS PERTAINING TO ENVIRONMENTAL PROTECTION.
TYPICAL DETAIL SHEETS:
THE S7.00 SERIES SHEETS IN THESE DRAWINGS CONTAIN TYPICAL STRUCTURAL DETAILS FOR VARIOUS BUILDING MATERIALS. SOME OF THESE DETAILS MAY NOT BE PART OF THIS PROJECT.
DRAWINGS:
DO NOT SCALE DRAWINGS.
WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL NOTES, AND SPECIFICATIONS, THE MORE STRINGENT REQUIREMENTS SHALL GOVERN. DETAILS ON DRAWINGS TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS. DETAILS NOTED "TYPICAL" APPLY TO ALL SIMILAR CONDITIONS. WHERE NO SPECIFIC DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ELSEWHERE ON THE PROJECT.
CAST-IN-PLACE CONCRETE:
ALL CONCRETE SHALL CONFORM TO THE SPECIFICATIONS FOR STRUCTURAL CONCRETE, ACI 301-10.
ALL EXPOSED EDGES OF CONCRETE SHALL HAVE A 3/4" CHAMFER UNLESS NOTED OTHERWISE.
NORMALWEIGHT CONCRETE:
A. FC = 4500 PSI @ 28 DAYS - ALL CONCRETE EXPOSED TO FREEZE/THAW CYCLES AND OCCASIONAL MOISTURE, INCLUDING CONCRETE FLAT WORK, EXPOSED BUILDING STEM WALLS, SITE WALLS, ETC...
EXTERIOR CONCRETE SHALL MEET EXPOSURE CATEGORY AND CLASS F1 ACCORDING TO ACI 318 TABLE 19.3.1.1.
B. FC = 3000 PSI @ 28 DAYS - ALL INTERIOR CONCRETE (I.E. FOOTINGS, PEDESTALS, THE BEAMS, GRADE BEAMS, RETAINING WALLS, ETC.).
C. FC = 3000 PSI @ 28 DAYS - ALL INTERIOR SLABS ON GRADE, UNLESS NOTED OTHERWISE.
D. FC = 3500 PSI @ 28 DAYS - ALL CONCRETE FILL OVER METAL DECK, UNLESS NOTED OTHERWISE.
CONCRETE MIX DESIGNS (INCLUDING AIR CONTENT, WATER TO CEMENT RATIOS, AND OTHER CRITERIA) SHALL CONFORM TO THE REQUIREMENTS SET FORTH IN ACI 318 TABLE 4.3.1, BASED ON THE EXPOSURE CATEGORIES AND CLASSES DEFINED IN ACI 318 TABLE 4.2.1. USE AIR ENTRAINING ADMIXTURE IN ALL EXTERIOR CONCRETE. AIR CONTENT IN FIRE RATED SLABS SHALL ALSO COMPLY WITH THE REQUIREMENTS IN THE SPECIFIED UL LISTING.
COLD WEATHER CONCRETING: PROTECT CONCRETE WORK FROM PHYSICAL DAMAGE OR REDUCED STRENGTH CAUSED BY FROST, FREEZING OR LOW TEMPERATURES. COMPLY WITH ACI 306.1.
HOT WEATHER CONCRETING: WHEN HOT WEATHER CONDITIONS EXIST THAT WOULD IMPAIR THE QUALITY AND STRENGTH OF THE CONCRETE, REDUCE DELIVERY TIME OF READY MIX CONCRETE, LOWER THE TEMPERATURE OF MATERIALS, OR ADD RETARDER TO ENSURE THAT THE CONCRETE IS PLASTIC. RETEMPERING WITH WATER IS NOT ALLOWED. COMPLY WITH ACI 305R.
SLAB CURING: ALL INTERIOR CONCRETE SLABS, EXCEPT EXPOSED INTEGRALLY COLORED SLABS, ARE TO BE CURED WITH A MOISTURE RETAINING COVER FOR THE FIRST 7 DAYS (MINIMUM) AFTER PLACEMENT.
THE CONTRACTOR IS ALLOWED TO CAST FOUNDATIONS AGAINST EXCAVATED SOIL SURFACES, PROVIDED THE FOLLOWING IS ADHERED TO:
A. THE SIDE SLOPES OF THE EXCAVATION SHALL BE ABLE TO MAINTAIN VERTICAL SOIL SURFACE WITHOUT SOIL SLOUGHAGE.
B. THE BOTTOM WIDTH OF THE EXCAVATION SHALL BE ONE INCH WIDER MINIMUM ON EACH SIDE THAN THE SPECIFIED FOOTING WIDTH.
C. THE SIDE WALLS OF THE EXCAVATION SHALL BE BATTERED A MINIMUM OF ONE INCH HORIZONTAL TO TWELVE INCHES VERTICAL.
D. IF SANDY OR LOOSE MATERIALS ARE ENCOUNTERED, THE FOOTING MUST BE FORMED.
E. THE CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVAL OF ANY SOIL SLOUGHAGE FROM THE WET CONCRETE DURING THE CASTING OPERATION.
F. THE CONTRACTOR AGREES TO REMOVE AND RECAST ANY FOOTING WHERE THE ABOVE CONDITIONS ARE NOT MET.
EXPOSED SITE WALLS, RETAINING WALLS, AND STEM WALLS GREATER THAN 30 FEET IN LENGTH SHALL HAVE CONTROL JOINTS INSTALLED AT THE FOLLOWING MAXIMUM SPACING:
12'-0" ON CENTER FOR WALLS 6'-0" MAXIMUM HEIGHT
18'-0" ON CENTER FOR WALLS 10'-0" MAXIMUM HEIGHT
20'-0" ON CENTER FOR WALLS GREATER THAN 10'-0" IN HEIGHT
ALL CONCRETE EXPOSED TO GROUND SHALL BE MANUFACTURED WITH PORTLAND CEMENT TYPE II OR TYPE V.
SEE THE S7.00 SERIES SHEETS FOR TYPICAL CONCRETE DETAILS.
REINFORCING STEEL:
ALL REINFORCING STEEL SHALL BE FABRICATED AND PLACED IN ACCORDANCE WITH THE BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI 318-14), AND DETAILS AND DETAILING OF CONCRETE REINFORCEMENT (ACI 315-99).
ALL REINFORCING STEEL SHALL CONFORM TO ASTM A615 GRADE 60, EXCEPT STIRRUPS, TIES AND INDICATED FIELD-BENT BARS, WHICH SHALL CONFORM TO ASTM A615 GRADE 40.
ALL WELDED WIRE FABRIC SHALL BE DEFORMED AND SHALL CONFORM TO ASTM A479. PROVIDE IN FLAT SHEETS ONLY.
TENSION AND COMPRESSION LAPS IN REINFORCING SHALL CONFORM TO THE LAP SPlice SCHEDULE ON SHEET S-601 AND BE IN ACCORDANCE WITH ACI 318, CHAPTER 12, UNLESS NOTED OTHERWISE.
ALL HORIZONTAL REINFORCING IN FOOTINGS, WALLS AND BEAMS SHALL BE CONTINUOUS AROUND CORNERS OR HAVE BENT (CORNER) BARS OF THE SAME SIZE AND SPACING AS THE HORIZONTAL BARS AND LAP 30 BAR DIAMETERS (24" MINIMUM).
CONCRETE COVER FOR REINFORCING SHALL BE AS FOLLOWS UNLESS OTHERWISE NOTED:
A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3"
B. CONCRETE CAST AGAINST FORMS BUT EXPOSED TO EARTH OR WEATHER:
1. BARS LARGER THAN NO. 5: 2"
2. BARS NO. 5 OR SMALLER: 1 1/2"
C. CONCRETE NOT EXPOSED TO WEATHER OR NOT IN CONTACT WITH GROUND:
1. COLUMNS, GIRDERS AND BEAMS: 1 1/2"
2. STRUCTURAL SLABS, WALLS AND JOISTS (NO. 11 AND SMALLER): 3/4"
D. SLAB ON GRADE: 1 1/2" FROM TOP OF SLAB
E. STRUCTURAL SLAB ON METAL DECK: 1" FROM TOP OF SLAB
FORM TIES SHALL BE EITHER OF THE THREADED OR SNAP-OFF TYPE SO THAT NO METAL WILL BE LEFT WITHIN 1 INCH OF THE SURFACE OF THE WALL. FOLLOWING REMOVAL OF FORM TIES, RECESSES ARE TO BE CAREFULLY FILLED AND POINTED WITH MORTAR.
REINFORCING SHALL NOT BE TACK WELDED OR WELDED IN ANY MANNER UNLESS SPECIFICALLY DETAILED ON THE STRUCTURAL PLANS.
BAR SUPPORTS AND SPACERS FOR REINFORCING SHALL BE PROVIDED IN ACCORDANCE WITH ACI 315-99. REINFORCING SHALL BE SECURELY TIED TO SUPPORTS.
CHAIRS WITH 22 GAGE SAND PLATES OR PRECAST BLOCKS SHALL BE PROVIDED FOR ALL REINFORCING OF CONCRETE IN CONTACT WITH GRADE.
DECK CHAIRS SHALL BE PROVIDED FOR ALL WELDED WIRE FABRIC IN SLABS OVER METAL DECK.

GENERAL STRUCTURAL NOTES

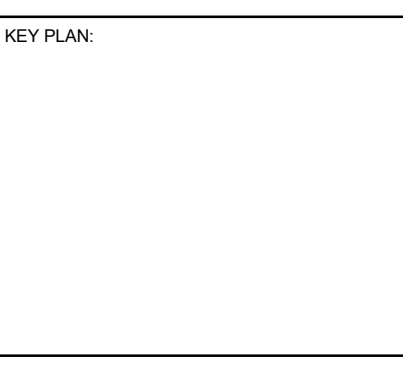
POST INSTALLED ANCHORS:
THE STRUCTURAL DESIGN IS BASED ON THE POST INSTALLED ANCHORING SYSTEMS NOTED BELOW. SINCE ANCHOR CAPACITIES VARY BY MANUFACTURER, THE CONTRACTOR SHALL USE ONLY THE SYSTEMS NOTED BELOW UNLESS AN ALTERNATE IS APPROVED BY THE ENGINEER OF RECORD. ALTERNATE ANCHORING SYSTEMS MAY REQUIRE RE-DESIGN TO VERIFY ANCHOR QUANTITIES, SPACING, AND EMBED DEPTHS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY ADDITIONAL CONSTRUCTION AND RE-DESIGN COSTS ASSOCIATED WITH THE ALTERNATE ANCHORING SYSTEM.
ALL ADHESIVE (EPOXY) FOR POST INSTALLED ANCHORS AND/OR REBAR INTO CONCRETE SHALL BE HILTI HIT-RE 500 V3 OR HIT-HY 200 EPOXY ADHESIVE ANCHORING SYSTEM, HILTI HIT-RE 100 OR HIT-HY 200 EPOXY ADHESIVE SYSTEM OR APPROVED EQUAL. INSTALLATION SHALL BE PER MANUFACTURER'S RECOMMENDATIONS.
ALL ADHESIVE (EPOXY) FOR POST INSTALLED ANCHORS AND/OR REBAR INTO GROUT FILLED MASONRY SHALL BE HILTI HIT HY 70 ADHESIVE ANCHORING SYSTEM OR APPROVED EQUAL. INSTALLATION SHALL BE PER MANUFACTURER'S RECOMMENDATIONS.
ANCHOR LENGTHS SHOWN FOR POST INSTALLED ANCHORS AND/OR REBAR INTO HOLLOW MASONRY AND/OR BRICK SHALL BE HILTI HIT HY 70 ADHESIVE ANCHORING SYSTEM OR APPROVED EQUAL. INSTALLATION SHALL BE PER MANUFACTURER'S RECOMMENDATIONS.
ALL POST INSTALLED MECHANICAL ANCHORS INTO CONCRETE SHALL BE HILTI KWIK BOLT TZ EXPANSION ANCHOR OR APPROVED EQUAL. INSTALLATION SHALL BE PER MANUFACTURER'S RECOMMENDATIONS.
ALL POST INSTALLED MECHANICAL ANCHORS INTO GROUT FILLED MASONRY SHALL BE HILTI KWIK BOLT 3 EXPANSION ANCHOR OR APPROVED EQUAL. INSTALLATION SHALL BE PER MANUFACTURER'S RECOMMENDATIONS.
ANCHOR LENGTHS SHOWN FOR ATTACHMENT TO CONCRETE AND/OR MASONRY ARE REQUIRED EMBEDMENT LENGTHS. THE CONTRACTOR SHALL PROVIDE ANCHORS WITH ADDITIONAL LENGTH TO FACILITATE THE REQUIRED CONNECTION.
SUBMIT ALL PROPOSED ANCHORING SYSTEMS INCLUDING ICC-ES REPORTS TO STRUCTURAL ENGINEER FOR REVIEW PRIOR TO INSTALLATION. THE ICC-ES FORMS SHALL MEET THE REQUIREMENTS OF THE IBC REFERENCED IN THESE NOTES.
STRUCTURAL AND MISCELLANEOUS STEEL:
ALL STRUCTURAL STEEL SHALL BE DETAILED AND FABRICATED IN ACCORDANCE WITH THE AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS".
ALL WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A992, GRADE 50, UNLESS NOTED OTHERWISE.
ALL MISCELLANEOUS STEEL MEMBERS, SUCH AS CHANNELS, ANGLES, FLAT BARS, AND PLATES SHALL CONFORM TO ASTM A36 UNLESS NOTED OTHERWISE.
ALL RECTANGULAR AND SQUARE STRUCTURAL TUBING SHALL CONFORM TO ASTM A500, GRADE B, FY = 46 KSI OR ASTM 1085, GRADE B, FY = 50 KSI.
ALL ROUND STRUCTURAL TUBING SHALL CONFORM TO ASTM A500, GRADE B, FY = 42 KSI OR ASTM 1085, GRADE B, FY = 50 KSI.
ALL STRUCTURAL PIPE SHALL CONFORM TO ASTM A53, TYPE E OR S, GRADE B, FY = 35 KSI.
BOLTS SHALL CONFORM TO ASTM A325N TENSION CONTROL BOLTS UNLESS NOTED OTHERWISE, WITH SIZES AS SHOWN ON THE DRAWINGS. WHERE CLEARANCE WITHIN A CONNECTION DOES NOT PERMIT THE USE OF TENSION CONTROL BOLTS, STANDARD A325N BOLTS SHALL BE USED AND INSPECTED IN ACCORDANCE WITH THE AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS".
ALL BOLTS SHALL BE INSTALLED IN A SNUG TIGHT CONDITION EXCEPT AT MOMENT CONNECTIONS, BRACED FRAME CONNECTIONS, AND AT CONNECTIONS DETAILED WITH A325SC BOLTS. AT THESE LOCATIONS, THE BOLTS SHALL BE TIGHTENED SO AS TO SHEAR THE SPLINE OFF THE BOLT.
ANCHOR BOLTS EMBEDDED IN CONCRETE SHALL BE ASTM F1554 GRADE 36 THREADED RODS WITH DOUBLE NUTS. PROVIDE FLAT WASHERS BETWEEN NUTS AND BASEPLATE SURFACES. ANCHOR BOLT LENGTHS SHOWN FOR ATTACHMENT TO CONCRETE AND/OR MASONRY ARE REQUIRED EMBEDMENT LENGTHS. THE CONTRACTOR SHALL PROVIDE ANCHOR BOLTS WITH ADDITIONAL BOLT LENGTH TO FACILITATE THE REQUIRED CONNECTION.
ANCHOR BOLT FLAT WASHERS SHALL BE PROVIDED IN ACCORDANCE WITH TABLE 14-2 OF AISC 360, AISC MANUAL OF STEEL CONSTRUCTION LATEST EDITION.
ALL WELDING SHALL BE DONE IN ACCORDANCE WITH THE LATEST STANDARDS OF THE AWS STRUCTURAL WELDING CODE.
ALL BOLT HOLES THAT ARE REQUIRED TO BE FIELD DRILLED SHALL BE DRILLED WITH A MAG DRILL. FLAME CUTTING OF HOLES OR ENLARGING OF MISALIGNED HOLES WILL NOT BE ALLOWED.
HEADED CONCRETE ANCHORS AND SHEAR CONNECTORS SHALL BE MADE FROM STEEL CONFORMING TO ASTM A108 AND MEET THE MECHANICAL PROPERTIES OF TYPE B, AS REQUIRED BY CHAPTER 7 OF AWS D1.1 "STRUCTURAL WELDING CODE-STEEL", LATEST EDITION. STRUCTURAL STEEL TO RECEIVE SHEAR CONNECTORS SHALL BE FREE OF PAINT. WELDING PREQUALIFICATION REQUIRED.
PROVIDE A SLIDE BEARING CONNECTION FOR STEEL BEAMS BEARING ON MASONRY WALLS UNLESS NOTED OTHERWISE. SEE SHEET S7.41 FOR TYPICAL CONNECTION DETAIL.
SEE S7.00 SERIES SHEETS FOR TYPICAL STEEL DETAILS.
COMPOSITE FLOORS:
THE METAL DECK FOR COMPOSITE FLOORS SHALL BE UNSHORED UNLESS NOTED OTHERWISE.
THE SHEAR CONNECTORS SHALL BE 3/4" DIAMETER X 4 1/2" AT 3' DEEP DECK UNLESS NOTED OTHERWISE. THE SHEAR CONNECTORS SHALL BE MADE FROM STEEL CONFORMING TO ASTM A108 AND MEET THE MECHANICAL PROPERTIES OF TYPE B, AS REQUIRED BY CHAPTER 7 OF AWS D1.1 "STRUCTURAL WELDING CODE-STEEL", LATEST EDITION. STRUCTURAL STEEL TO RECEIVE SHEAR CONNECTIONS SHALL BE FREE OF PAINT. WELDING PREQUALIFICATION REQUIRED.
THE SHEAR CONNECTIONS SHALL NOT BE ADDED UNTIL THE METAL FLOOR DECK IS INSTALLED.
WHERE SHEAR CONNECTIONS AND PUDDLE WELDS COINCIDE, THE SHEAR CONNECTOR MAY REPLACE THE PUDDLE WELD.
CAMBERED BEAMS SHALL HAVE THE CAMBER PUT IN AT 1/3 POINTS OR ALONG A PARABOLIC CURVE.
THE CONTRACTOR SHALL SURVEY THE CAMBER OF THE BEAMS AFTER THE BEAMS HAVE BEEN ERECTED. THE CONTRACTOR SHALL SUBMIT THE SURVEY TO THE ENGINEER FOR REVIEW. THE CONTRACTOR SHALL NOT POUR THE SLAB UNTIL THE ENGINEER HAS REVIEWED AND APPROVED THE BEAM CAMBERS.
CONTRACTOR SHALL SHORE BEAMS WITH A CAMBER MORE THAN 1/2" LOWER THAN SPECIFIED. THE BEAM SHALL BE ALLOWED TO DEFLECT TO LEVEL.
THE CONCRETE FOR THE SLAB SHALL BE POURED AND PLACED TO THE ELEVATION INDICATED ON THE DRAWINGS WHILE MAINTAINING THE MINIMUM THICKNESS. SPREAD CONCRETE OVER AREA OF INFLUENCE TO ROUGH DEPTH IN ORDER TO LOAD BEAMS AND GIRDERS PRIOR TO SETTING SCREED ELEVATIONS.
THE WEIGHT OF THE WET CONCRETE WILL CAUSE DEFLECTIONS OF THE STEEL FRAMING. THEREFORE, CONCRETE OVERRUNS ARE TO BE ANTICIPATED BY THE CONTRACTOR.
CONTRACTOR SHALL CONTINUOUSLY MONITOR THE THICKNESS AND ELEVATIONS DURING CONCRETE PLACING OPERATIONS.
PROVIDE #4 X 6'-0" AT 12" ON CENTER OVER ALL GIRDERS OF COMPOSITE FLOORS.
PROVIDE #4 X 6'-0" AT 12" ON CENTER OVER SHORED BEAMS THAT ARE NOT ALLOWED TO DEFLECT TO LEVEL.
PROVIDE WELDED WIRE FABRIC AS INDICATED ON DRAWINGS IN FLAT SHEETS ONLY.
PROVIDE DECK CHAIRS FOR ALL WELDED WIRE FABRIC IN SLABS OVER METAL DECK.
STEEL JOISTS:
STEEL JOISTS SHALL BE MANUFACTURED BY A MEMBER OF SJI.
STEEL JOISTS SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE STEEL JOIST INSTITUTE STANDARD SPECIFICATIONS.
NO CONSTRUCTION LOADS OF ANY KIND SHALL BE PLACED ON UNBRIDGED JOISTS.
WHERE COLUMNS ARE NOT FRAMED IN AT LEAST TWO DIRECTIONS WITH STRUCTURAL STEEL MEMBERS, JOISTS AT OR CLOSEST TO COLUMN LINES SHALL BE FIELD BOLTED TO ADD LATERAL STABILITY DURING CONSTRUCTION.
PROVIDE BRIDGING IN ACCORDANCE WITH THE LATEST EDITION OF THE SJI STANDARD SPECIFICATIONS AND OSHA REQUIREMENTS.
THE STRUCTURAL DRAWINGS ARE NOT STAND-ALONE DOCUMENTS. THE CONTRACTOR SHALL COORDINATE THE LOCATIONS AND WEIGHTS OF ALL MECHANICAL, PLUMBING, ELECTRICAL AND OTHER EQUIPMENT WITH THE APPLICABLE DRAWINGS. THE JOIST SUPPLIER SHALL ACCOUNT FOR THE LOADS IN THEIR DESIGN.
THE STEEL JOIST MANUFACTURER SHALL DESIGN ROOF JOISTS SUPPORTING MECHANICAL UNITS, INDICATED AS SP JOISTS ON THE DRAWINGS, FOR 1.2x MECHANICAL UNIT WEIGHTS SHOWN. USE 25 PSF DEAD LOAD AND 20 PSF LIVE LOAD (NON-REDUCIBLE) UNLESS NOTED OTHERWISE. CONTRACTOR SHALL VERIFY ACTUAL MECHANICAL LOADS. NOTIFY STEEL JOIST MANUFACTURER OF ANY DISCREPANCIES.
JOIST DEFLECTIONS SHALL BE LIMITED PER SJI REQUIREMENTS AND SHALL NOT BE LESS THAN THE FOLLOWING:
L/360 FOR ROOF LIVE LOAD WITH STANDARD SJI CAMBER WHERE PLASTER OR STUCCO CEILINGS ARE SUPPORTED.
L/240 FOR ROOF LIVE LOAD WITH STANDARD SJI CAMBER WHERE NONPLASTER CEILINGS ARE SUPPORTED.
L/180 FOR ROOF LIVE LOAD WITH STANDARD SJI CAMBER WHERE NO CEILINGS ARE SUPPORTED AND PROVISIONS FOR FUTURE CEILINGS ARE NOT REQUIRED.
STEEL ROOF JOISTS SHALL BE DESIGNED FOR A NET WIND UPLIFT LOAD OF 15 PSF UNLESS NOTED OTHERWISE. THE DEAD LOAD OF MISCELLANEOUS ROOFTOP ITEMS, INCLUDING SCREEN WALLS, SKYLIGHTS, FIRE SUPPRESSION SYSTEM, SOLAR PHOTOVOLTAIC SYSTEM, ETC., SHALL BE ACCOUNTED FOR IN THE DESIGN OF THE STEEL ROOF JOISTS. THE CONTRACTOR SHALL COORDINATE THE MISCELLANEOUS LOADS WITH THE STEEL JOIST MANUFACTURER.

GENERAL STRUCTURAL NOTES

JOIST BEARING SEATS SHALL BEAR ON STEEL SUPPORTS AND SHALL BE CONNECTED AS FOLLOWS UNLESS NOTED OTHERWISE:
KKG5C: TWO 1/8" x 2 1/2" LONG FILLET WELDS
LH02-06 (OR 2 1/2" AND SMALLER TOP CHORD ANGLE LEG): TWO 3/16" x 2 1/2" LONG FILLET WELDS
LH0LH07-17 (OR 3 1/2" AND SMALLER TOP CHORD ANGLE LEG): TWO 1/4" x 2 1/2" LONG FILLET WELDS
LH0LH 18-25 (OR 4" AND LARGER TOP CHORD ANGLE LEG): TWO 1/4" x 4" LONG FILLET WELDS
ALL WELDS SHALL MEET CURRENT MINIMUM SJI REQUIREMENTS.
STEEL DECK:
ALL STEEL DECK SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE STEEL DECK INSTITUTE SPECIFICATIONS.
SEE PLANS FOR STEEL DECK TYPE, GAGE, FINISH AND CONNECTIONS.
PROVIDE A MINIMUM OF 1 1/2" BEARING FOR ALL STEEL DECK.
ALL SPLICES AND LAPS SHALL BE A MINIMUM OF 2" IN LENGTH AND SHALL BE LOCATED DIRECTLY ABOVE SUPPORTS.
ALL DECKING SHALL BE CONTINUOUS OVER TWO OR MORE SPANS.
POWDER DRIVEN FASTENERS SHALL BE EQUIVALENT TO:
HILTI X-HSN 24 FOR STEEL BASE MATERIAL 1/4" UP TO 3/8".
HILTI ENP-19 FOR STEEL BASE MATERIAL 1/4" OR THICKER.
MASONRY:
ALL MASONRY UNITS SHALL COMPLY WITH ASTM C 90 WITH A COMPRESSIVE STRENGTH OF 2000 PSI (NET AREA).
FM = 1900 PSI
MORTAR SHALL BE TYPE S.
GROUT - FC = 2000 PSI, MINIMUM.
CELLS CONTAINING REBAR SHALL BE GROUTED SOLID FROM THE BOTTOM TO THE TOP OF THE WALL IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE.
ALL CELLS BELOW GRADE SHALL BE GROUTED SOLID UP TO GRADE.
CELLS CONTAINING EXPANSION ANCHORS SHALL BE GROUTED SOLID.
ALL VERTICAL REBAR SHALL BE IN PLACE AND SECURED WITH REBAR POSITIONERS PRIOR TO GROUTING.
COVER FOR REINFORCING SHALL BE AS FOLLOWS UNLESS OTHERWISE NOTED:
A. MASONRY FACE NOT EXPOSED TO EARTH OR WEATHER: 1 1/2"
B. MASONRY FACE EXPOSED TO EARTH OR WEATHER:
1. BARS LARGER THAN NO. 5: 2"
2. BARS NO. 5 OR SMALLER: 1 1/2"
UNLESS OTHERWISE NOTED MASONRY CELLS SHALL BE GROUTED IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE (MAXIMUM 5 FOOT GROUT LIFTS).
LAP REBAR PER THE SCHEDULE ON S6.01.
WHERE REBAR LAP SPLICES EXCEED 5 FT GROUT LIFTS, 8 FT GROUT LIFTS MAY BE USED WITH CLEANOUTS PROVIDED AT THE BOTTOM OF EACH VERTICALLY REINFORCED CELL. SOLID GROUTED WALLS SHALL HAVE CLEANOUTS AT 32" ON CENTER MAXIMUM.
ALL HORIZONTAL REINFORCING IN BOND BEAMS SHALL BE CONTINUOUS AROUND CORNERS OR HAVE BENT (CORNER) BARS OF THE SAME SIZE AND A LAP AS NOTED ABOVE. VERTICAL STEEL SHALL CONTINUE THROUGH BOND BEAMS.
PROVIDE STANDARD LADDER TYPE JOINT REINFORCING AT 16" ON CENTER (ALTERNATE COURSES) UNLESS NOTED OTHERWISE IN THE PROJECT DOCUMENTS. USE PREFABRICATED CORNERS AND TEES AT ALL WALL CORNERS AND INTERSECTIONS RESPECTIVELY.
PROVIDE A SLIDE BEARING CONNECTION FOR STEEL BEAMS BEARING ON MASONRY WALLS UNLESS NOTED OTHERWISE. SEE SHEET S7.41 FOR TYPICAL CONNECTION DETAIL.
SEE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR INFORMATION REGARDING MASONRY COLORS, FINISHES, BOND, ETC. AT ALL EXPOSED MASONRY WALLS.
ALL MASONRY WALL CONFIGURATIONS INCLUDING WALL OPENINGS SHALL BE COORDINATED WITH CIVIL, MECHANICAL, PLUMBING, ELECTRICAL AND DRAWINGS FROM ALL OTHER DISCIPLINES.
EXPOSED MASONRY SITE WALLS AND RETAINING WALLS GREATER THAN 16 FEET IN LENGTH SHALL HAVE MASONRY CONTROL JOINTS INSTALLED AT THE FOLLOWING MINIMUM SPACING:
12'-0" ON CENTER FOR WALLS 6'-0" MAXIMUM HEIGHT
18'-0" ON CENTER FOR WALLS 10'-0" MAXIMUM HEIGHT
20'-0" ON CENTER FOR WALLS GREATER THAN 10'-0" IN HEIGHT
SEE S7.00 SERIES SHEETS FOR TYPICAL MASONRY DETAILS.
VENEER:
FOR CMU OR BRICK VENEER (5" MAXIMUM, 3" MINIMUM THICKNESS) ATTACHMENT TO STRUCTURAL MASONRY, PROVIDE ADJUSTABLE INTEGRAL ANCHOR TIES. ADJUSTABLE INTEGRAL ANCHOR TIES SHALL BE CORROSION RESISTANT AND HAVE TWO PINTLE LEGS MINIMUM WITH W2.8 (3/16)" WIRE OR APPROVED EQUAL. PROVIDE DUR-O-WALL DA370 ADJUSTABLE INTEGRAL ANCHOR TIES OR APPROVED EQUAL.
FOR CMU OR BRICK VENEER (5" MAXIMUM, 3" MINIMUM THICKNESS) ATTACHMENT TO STRUCTURAL CONCRETE, PROVIDE ADJUSTABLE ANCHOR TIES. ADJUSTABLE ANCHOR TIES SHALL BE CORROSION RESISTANT AND HAVE A TWO PINTLE LEGS MINIMUM WITH A MINIMUM W2.8 (3/16)" WIRE. ATTACH TO CONCRETE WITH 2-1/4" DIAMETER CONCRETE SCREWS, HILTI KWIKCON 11 x 1 1/2" OR APPROVED EQUAL.
FOR CMU OR BRICK VENEER (5" MAXIMUM, 3" MINIMUM THICKNESS) ATTACHMENT TO STRUCTURAL COLD FORMED METAL STUDS, PROVIDE ADJUSTABLE ANCHOR TIES. ADJUSTABLE ANCHOR TIES SHALL BE CORROSION RESISTANT AND HAVE TWO PINTLE LEGS MINIMUM W2.8 (3/16)" WIRE. PROVIDE DUR-O-WALL DA213 ADJUSTABLE ANCHOR TIE OR APPROVED EQUAL. ATTACH THROUGH SHEATHING TO STUDS WITH 2-1/4" x 1 1/2" CORROSION RESISTANT TEK SCREWS.
SEE TYPICAL DETAILS ON SHEET S7.31 FOR VENEER TIE SPACING.
PROVIDE ADDITIONAL ANCHORS AROUND ALL OPENINGS LARGER THAN 16" IN EITHER DIMENSION. SPACE ANCHORS WITHIN 12" OF OPENING PERIMETER AND MATCH HORIZONTAL OR VERTICAL ANCHOR TIE SPACING.
COORDINATE VENEER LOCATION, TYPE, BOND PATTERN, ETC. WITH ARCHITECTURAL DRAWINGS.
PRE-ENGINEERED METAL BUILDING:
FOUNDATION CONFIGURATION AND SIZES SHOWN ON THESE DRAWINGS ARE BASED ON PRELIMINARY DESIGN CALCULATIONS. THESE SIZES MAY REQUIRE MODIFICATIONS PER THE METAL BUILDING MANUFACTURER'S FINAL GRAVITY AND LATERAL DESIGN CALCULATIONS.
THE METAL BUILDING MANUFACTURER SHALL PROVIDE FINAL GRAVITY AND LATERAL DESIGN CALCULATIONS FOR APPROVAL PRIOR TO THE COMMENCEMENT OF FOUNDATION EARTHWORK.
THE BUILDING SHALL BE A MANUFACTURER'S STANDARD PREFABRICATED METAL STRUCTURE OF THE APPROXIMATE INSIDE AREA SHOWN, EXCEPT AS NOTED. RIGID FRAMES SHALL BE SPACED AS SPECIFIED ON THE DRAWINGS, BUT OVERALL DIMENSIONS AND CONSTRUCTION DETAILS MAY VARY TO SUIT MANUFACTURER'S STANDARD DESIGN. MINIMUM WEB THICKNESS OF RIGID FRAMES SHALL BE 3/16".
THE BUILDING SHALL BE DESIGNED AND FABRICATED ACCORDING TO AISC, MBMA AND AISI SPECIFICATIONS. THE DIMENSIONAL TOLERANCES APPLICABLE TO ROLLED FORM STEEL UNDER THE LATEST EDITION OF THE AISC "STANDARD MILL PRACTICE" SECTION SHALL BE REQUIRED IN THE FABRICATION OF THE STEEL BUILDING FRAMES.
THE BUILDING FRAME SHALL BE DESIGNED TO LIMIT THE LATERAL DEFLECTION TO H/240 INCH AT THE BUILDING EAVE FOR THE SPECIFIED BASIC WIND SPEED.
THE BUILDING SHALL BE DESIGNED TO SUPPORT ALL MECHANICAL EQUIPMENT INCLUDING HEATERS, SPRINKLERS, EXHAUST SYSTEMS AND ALL OTHER DEVICES. ADDITIONAL GIRTS OR PURLINS SHALL BE PLACED IN CONVENIENT LOCATIONS FOR ATTACHMENT OF ALL MECHANICAL EQUIPMENT. THE CONTRACTOR SHALL COORDINATE THE MECHANICAL LOADS WITH THE METAL BUILDING MANUFACTURER AND THE MECHANICAL DRAWINGS.
DESIGN LOADS SHALL CONFORM WITH THESE GENERAL NOTES. LOAD COMBINATIONS SHALL COMPLY WITH MBMA SPECIFICATIONS.
ANCHOR BOLTS SHOWN ON THESE DRAWINGS ARE BASED ON PRELIMINARY DESIGN CALCULATIONS. THESE SIZES MAY REQUIRE MODIFICATIONS PER THE METAL BUILDING MANUFACTURER'S FINAL GRAVITY AND LATERAL DESIGN CALCULATIONS.
THE METAL BUILDING MANUFACTURER SHALL DESIGN THE SUPPORTS FOR ALL CONNECTIONS OF MASONRY AND/OR METAL STUD WALLS TO THE METAL BUILDING COMPONENTS AND PROVIDE CALCULATIONS FOR THE DESIGN OF THE SUPPORTS.
PREPARE THE SHOP DRAWINGS AND CALCULATIONS UNDER THE SEAL OF A PROFESSIONAL STRUCTURAL ENGINEER REGISTERED IN THE STATE THE PROJECT IS LOCATED.
COORDINATE WITH ARCHITECTURAL DRAWINGS.
GLASS CURTAIN WALL SYSTEM:
ALL LATERAL AND GRAVITY SUPPORT FOR THE GLASS CURTAIN WALL SYSTEM SHALL BE PER THE MANUFACTURER'S RECOMMENDATIONS. SHOP DRAWINGS AND STAMPED CALCULATIONS SHALL BE SUBMITTED FOR APPROVAL BY THE ENGINEER OF RECORD AND THE ARCHITECT PRIOR TO INSTALLATION.
THE ENGINEER STAMPING THE SHOP DRAWINGS SHALL BE REGISTERED IN THE STATE THAT THE PROJECT IS LOCATED.
THE BEAMS AT ALL FLOORS HAVE BEEN DESIGNED TO SUPPORT THE GRAVITY LOAD OF THE GLASS CURTAIN WALL SYSTEM. THE GLASS CURTAIN WALL SYSTEM SHALL BE LATERALLY SUPPORTED AT ALL FLOORS AND ROOF LEVEL.



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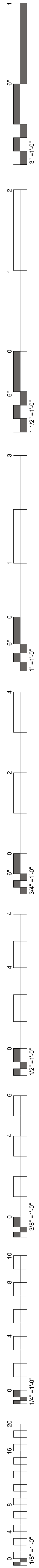


PROJECT PHASE:
BID PACKAGE 01

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DATE: 11-01-19
JOB NUMBER: 18-01.01

SHEET NUMBER:
S0.02
GENERAL STRUCTURAL NOTES



GENERAL STRUCTURAL NOTES

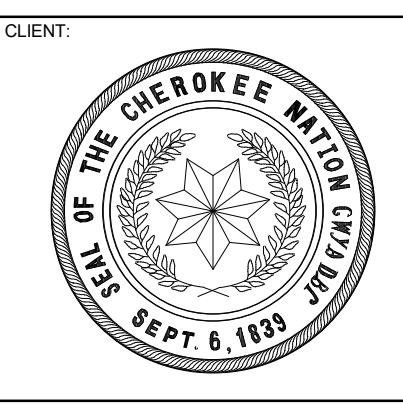
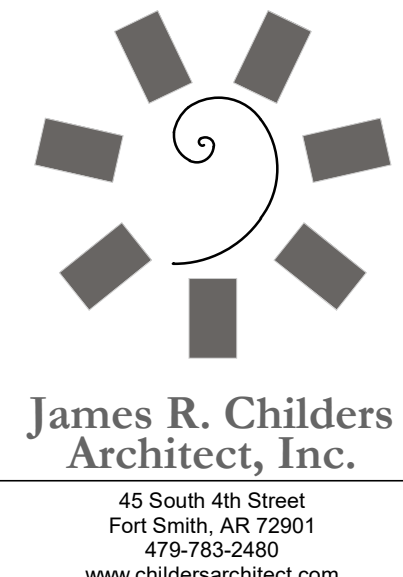
ELEVATORS:
 THE STRUCTURE HAS BEEN DESIGNED FOR A KONE ELEVATOR.
 ALL STRUCTURAL SUPPORTS, FLOOR PENETRATION SIZES AND PIT DIMENSIONS HAVE BEEN DESIGNED BASED ON THE ABOVE INFORMATION. SHOULD THE ACTUAL ELEVATOR(S) SELECTED DIFFER FROM THE INFORMATION PROVIDED ABOVE, THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY ADDITIONAL CONSTRUCTION AND REDESIGN COSTS ASSOCIATED WITH THE ALTERNATE ELEVATOR(S).
 THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING ALL ELEVATOR PIT AND FLOOR PENETRATION LOCATIONS AND DIMENSIONS.
 THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING ALL ELEVATOR OVERRUN REQUIREMENTS.
 ELEVATOR CAR RAIL AND COUNTERWEIGHT RAIL SUPPORTS SHALL BE PROVIDED BY THE CONTRACTOR. CAR RAIL AND COUNTERWEIGHT RAIL SUPPORTS SHALL BE PROVIDED AT AND BETWEEN ALL FLOOR LEVELS SERVICED BY THE ELEVATOR, ABOVE THE LAST STOP OF THE ELEVATOR, AND BETWEEN THE BASEMENT AND THE GROUND FLOOR AS REQUIRED BY THE ELEVATOR MANUFACTURER. IF THE ELEVATOR MANUFACTURER REQUIRES RAIL SUPPORTS THAT DIFFER FROM THOSE PROVIDED, THE ELEVATOR MANUFACTURER SHALL BE RESPONSIBLE FOR ADDITIONAL CONSTRUCTION COST AND DESIGN COST.
 THE CONTRACTOR SHALL VERIFY THE DESIGN OF THE HOISTS/SAFETY BEAM AND CONNECTIONS AS REQUIRED PER THE ELEVATOR MANUFACTURER.
 STRUCTURAL ELEMENTS AFFECTED BY THE ELEVATOR LAYOUT SHALL NOT BE FABRICATED PRIOR TO APPROVAL OF ELEVATOR SHOP DRAWINGS.
SPECIAL INSPECTION:
 THE OWNER SHALL PROVIDE FOR SERVICES OF A CERTIFIED INSPECTOR (APPROVED BY THE BUILDING OFFICIAL OR THE ENGINEER OF RECORD) IN ACCORDANCE WITH CHAPTER 17 OF THE INTERNATIONAL BUILDING CODE FOR THE SPECIAL INSPECTION ITEMS NOTED ON SHEET S0.03.
DEFERRED SUBMITTALS:
 THE DEFERRED SUBMITTALS LISTED BELOW ARE THOSE PORTIONS OF THE DESIGN THAT ARE NOT COMPLETED AT THE TIME OF APPLICATION AND ARE TO BE SUBMITTED TO THE BUILDING OFFICIAL AND APPROVED PRIOR TO THE INSTALLATION OF THOSE ITEMS. THE MANUFACTURER, CONSULTANT, OR CONTRACTOR, AS APPROPRIATE, SHALL PROVIDE SUBMITTALS TO THE ENGINEER OF RECORD FOR REVIEW FOR THE FOLLOWING ITEMS:
 SPECIAL STEEL JOISTS
 METAL STAIRS
 EXTERIOR COLD-FORMED METAL FRAMING
 INTERIOR COLD-FORMED METAL FRAMING
 TEMPORARY SHORING
 HANDRAILS
 CURTAIN WALL AND STOREFRONT
 AGGREGATE PIERS / STONE COLUMN GROUND IMPROVEMENT
 PRECAST CONCRETE COLUMNS AND ASSOCIATED ATTACHMENTS AND ANCHORAGE

SCHEDULE OF STRUCTURAL SPECIAL INSPECTIONS

- SPECIAL INSPECTIONS / TESTING - "SPECIAL STRUCTURAL INSPECTION" SHALL NOT RELIEVE THE OWNER OR THEIR AGENT FROM HAVING THE INSPECTIONS OF THE JURISDICTION BUILDING DEPARTMENT PER SECTION 110 OF THE IBC PERFORMED, BOTH THE JURISDICTION BUILDING DEPARTMENT INSPECTIONS AND "SPECIAL STRUCTURAL INSPECTION" SHALL BE PERFORMED.
- THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR NOTIFYING THE JURISDICTION BUILDING OFFICIAL AND SPECIAL INSPECTOR WHEN WORK IS READY FOR INSPECTION.
- REPORTING FOR SPECIAL INSPECTION - SPECIAL INSPECTION AND TESTING REPORTS SHALL BE COMPLETED AND DISTRIBUTED AT THE COMPLETION OF EACH TASK. IF A TASK IS TO TAKE LONGER THAN THREE (3) DAYS, PROVIDE REPORTS FOR EACH DAY. PROVIDE COPIES OF REPORTS TO CONTRACTOR, OWNER, ARCHITECT AND STRUCTURAL ENGINEER OF RECORD. SPECIAL INSPECTOR TO KEEP A NON-COMPLIANCE LIST DOCUMENTING ITEMS INSPECTED NOT MEETING APPROVED CONSTRUCTION DOCUMENTS AND WHEN / HOW RESOLVED.
- SEE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING CONSTRUCTION DOCUMENTS FOR ADDITIONAL NON-STRUCTURAL SPECIAL INSPECTION ITEMS.
- SPECIAL INSPECTION OF SHOP FABRICATED MEMBERS AND ASSEMBLIES SHALL BE IN ACCORDANCE WITH SECTION 1704.2, UNLESS FABRICATOR IS APPROVED TO PERFORM WORK WITHOUT SPECIAL INSPECTION.
- IN ACCORDANCE WITH IBC CHAPTER 17, THE OWNER OR THE OWNER'S AGENT, OTHER THAN THE CONTRACTOR, SHALL EMPLOY ONE OR MORE APPROVED AGENCIES TO PROVIDE SPECIAL INSPECTIONS AND TESTS, DURING CONSTRUCTION FOR THE TYPES OF WORK LISTED BELOW THESE SPECIAL INSPECTIONS AND TESTS ARE IN ADDITION TO THE INSPECTIONS BY THE BUILDING OFFICIAL IDENTIFIED IN IBC SECTION 110.
- DEFINITIONS:
 * **SPECIAL INSPECTION:** INSPECTION AS HEREIN REQUIRED BY A QUALIFIED SPECIAL INSPECTOR COMPETENT WITH THE MATERIALS, INSTALLATION, FABRICATION, ERECTION OR PLACEMENT OF COMPONENTS AND CONNECTIONS REQUIRING SPECIAL EXPERTISE TO ENSURE COMPLIANCE WITH APPROVED CONSTRUCTION DOCUMENTS AND REFERENCED STANDARDS (SEE SECTION 1704).
 * **CONTINUOUS SPECIAL INSPECTION:** FULL-TIME OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED.
 * **PERIODIC SPECIAL INSPECTION:** THE PART-TIME OR INTERMITTENT OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK HAS BEEN OR IS BEING PERFORMED AND AT THE COMPLETION OF THE WORK.

ITEM	DESCRIPTION OF REQUIREMENTS	REQUIRED (YES/NO)
SPECIAL INSPECTION OF STRUCTURAL STEEL	TO BE PERFORMED IN ACCORDANCE WITH CHAPTER N OF AISC 360-10	YES
SPECIAL INSPECTION AND VERIFICATION OF STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.2	YES
SPECIAL INSPECTIONS AND VERIFICATIONS FOR CONCRETE CONSTRUCTION	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.3	YES
SPECIAL INSPECTIONS AND VERIFICATIONS FOR MASONRY CONSTRUCTION	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.4 AND REFERENCED STANDARDS	YES
SPECIAL INSPECTIONS AND VERIFICATIONS FOR WOOD CONSTRUCTION	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.5	NO
SPECIAL INSPECTIONS AND VERIFICATIONS OF SOILS	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.6, THE GEOTECHNICAL REPORT LISTED IN THE GENERAL FOUNDATION NOTES, AND ANY OTHER REQUIREMENTS LISTED IN THE GENERAL FOUNDATION NOTES	YES
SPECIAL INSPECTIONS AND VERIFICATIONS FOR DEEP FOUNDATIONS (DRIVEN PILES, CAST-IN-PLACE, OR HELICAL PILES AS APPLICABLE)	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTIONS 1705.7-1705.9 AS APPLICABLE, THE GEOTECHNICAL REPORT LISTED IN THE GENERAL FOUNDATION NOTES, AND ANY OTHER REQUIREMENTS LISTED IN THE CONSTRUCTION DOCUMENTS	NO
SPECIAL INSPECTIONS FOR WIND RESISTANCE (REQUIRED ONLY FOR V _W = 155MPH OR GREATER IN EXPOSURE CATEGORY B, OR V _W = 142MPH OR GREATER IN EXPOSURE CATEGORY C OR D)	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.11	NO
SPECIAL INSPECTIONS AND VERIFICATIONS FOR SEISMIC RESISTANCE (REQUIRED FOR STRUCTURES ASSIGNED TO CATEGORIES C, D, E, OR F)	TO BE PERFORMED IN ACCORDANCE WITH ALL APPLICABLE PORTIONS OF IBC SECTIONS 1705.12 AND 1705.13	NO

ADDITIONAL INSPECTIONS REQUIRED PER SIDEPLATE SYSTEMS ON SHEET S8.01



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KEY PLAN:

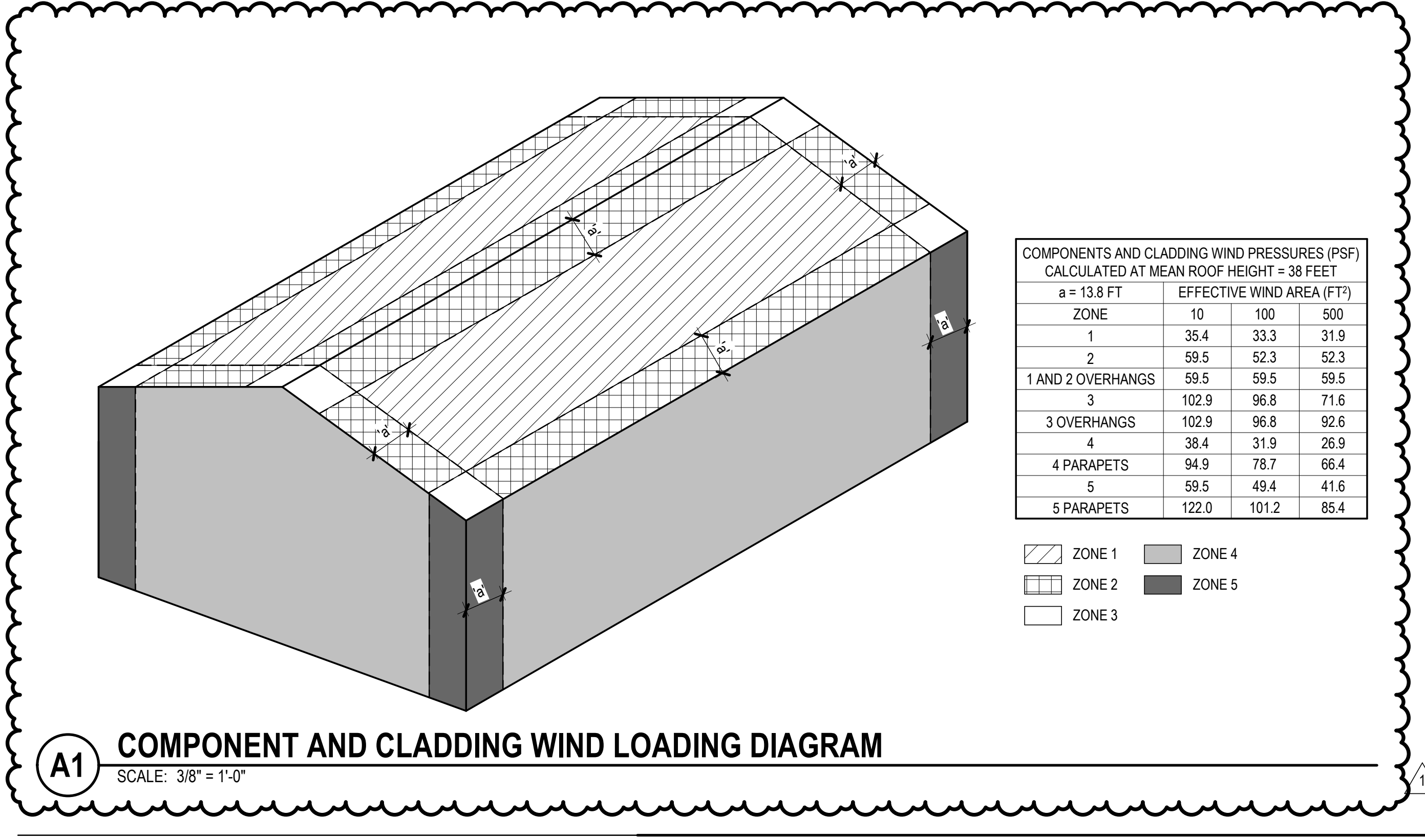
PROJECT PHASE:
 BID PACKAGE 01

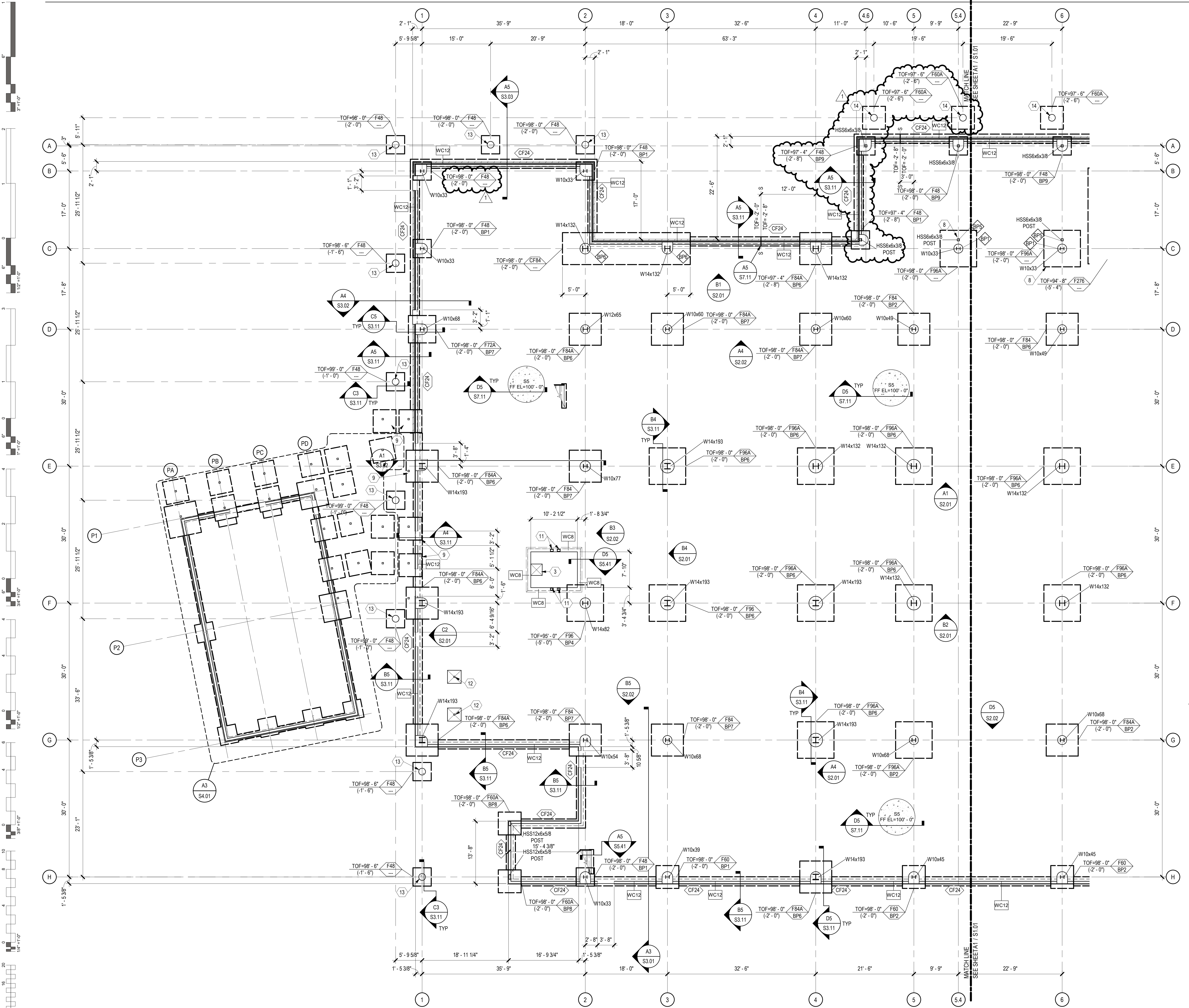
#	DATE	REVISIONS / DESCRIPTION
1	11/22/19	BID PACKAGE 01 - ADD 01

DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER:
 S0.03

GENERAL STRUCTURAL NOTES AND SPECIAL INSPECTIONS





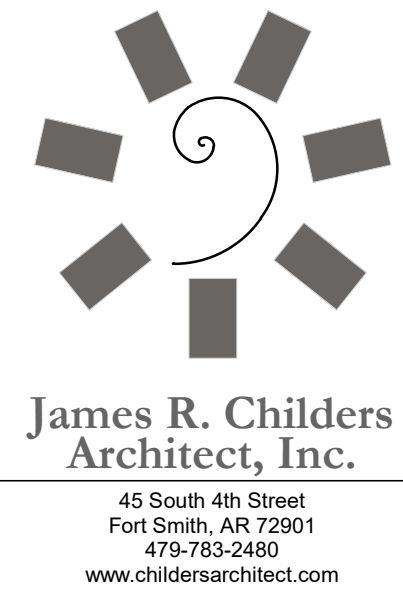
GENERAL SHEET NOTES

- SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- REFERENCE FINISH FLOOR ELEVATION 100'-0" = MEAN SEA FINISH FLOOR ELEVATION. SEE CIVIL DRAWINGS.
- TOP OF FOOTING ELEVATION = 98'-0" (-2'-0"), UNLESS NOTED OTHERWISE ON PLAN.
- NOTE TO CONTRACTOR: ENLARGED SLAB BLOCKOUTS MAY BE REQUIRED AT FRAME COLUMNS FOR MOMENT FRAME BASE PLATE CLEARANCE.
- NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
- DIMENSIONS ARE TO THE FACE OF STUD UNLESS NOTED OTHERWISE.
- SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
- EXISTING CONSTRUCTION IS PER AVAILABLE EXISTING DRAWINGS. ALL EXISTING CONSTRUCTION AND DIMENSIONS SHALL BE VERIFIED PRIOR TO CONSTRUCTION. SHOULD CONDITIONS VARY FROM THOSE SHOWN, CONTACT ENGINEER BEFORE PROCEEDING.
- PROVIDE SLAB JOINTS AT 10'-0" ON CENTER MAXIMUM. THE AREA OF THE CONTROL JOINT SHALL NOT EXCEED A 2:1 RATIO. CONTROL JOINTS SHALL BE LOCATED AT COLUMN LINES WHERE THE LAYOUT PERMITS. AT RE-ENTRANT CORNERS THAT DO NOT HAVE CONTROL JOINTS, PROVIDE 2#4 x 3'-0" DIAGONAL TO THE RE-ENTRANT CORNER.
- STRUCTURAL COLD FORMED METAL STUDS SHALL BE 6" WIDE UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING BY OTHERS.
- SEE SHEET S7.00 SERIES SHEETS FOR TYPICAL FOUNDATION SECTIONS AND DETAILS.
- SEE SHEET S6.01 FOR SCHEDULES.

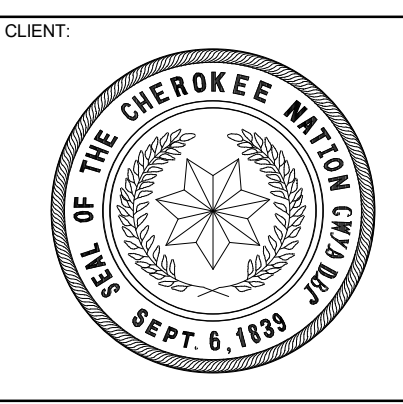
SHEET KEYNOTE

- FLOOR DRAIN, SLOPE SLAB TO DRAIN 1/8" PER FOOT. COORDINATE EXACT SIZE AND LOCATION WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
- POST AND FOUNDATION AS REQUIRED FOR STAIR SUPPORT. STAIR ENGINEER TO PROVIDE REQUIRED LOADS AND LOCATIONS.
- ELEVATOR SUMP PIT. COORDINATE EXACT SIZE AND LOCATION WITH ELEVATOR MANUFACTURER. SEE A4 / S5.41
- HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION AND SPACING WITH ELEVATOR MANUFACTURER. SEE B4 / S5.41
- PRE-ENGINEERED METAL BUILDING STEEL AND ANCHORAGE BY OTHERS. CONTRACTOR TO CONFIRM LOCATIONS OF FOUNDATIONS WITH FINAL PRE-ENGINEERED METAL BUILDING SHOP DRAWINGS.
- NOTCH MASONRY AS REQUIRED TO FACILITATE BASEPLATE INSTALLATION. STEP BOND BEAM AT THIS LOCATION. FILL VOID FROM NOTCH WITH NON-SHRINK GROUT.
- CUT AND REMOVE EXISTING SLAB AS REQUIRED TO PLACE NEW FOOTING. NEW SLAB TO POUR UP TO REMAINING SLAB.
- CENTER FOOTING ON GRID C.
- F60A PRE-MANUFACTURED SUNSHADE CONCRETE FOOTING. TOP OF FOOTING = 99'-0" (-1'-0"). SEE SHEET S6.01 FOR FOOTING SCHEDULE. COORDINATE FINAL LOCATION WITH SUNSHADE MANUFACTURER.
- EXISTING CANOPY. SEE ARCHITECTURAL DEMOLITION PLANS FOR EXTENT OF DEMOLITION.
- HSS6x6x1/2 ELEVATOR SUPPORT POST. COORDINATE EXACT LOCATION AND SPACING WITH ELEVATOR MANUFACTURER. SEE B4 / S5.41, D3 / S5.41, A2 / S5.41, B2 / S5.41, AND C2 / S5.41
- 1 1/2" RECESSED SLAB AT ADA SHOWER. COORDINATE EXACT SIZE, LOCATION, AND SLOPE REQUIREMENTS WITH ARCHITECTURAL DRAWINGS. SEE C4 / S7.11
- 18" DIAMETER PRECAST CONCRETE COLUMN BY OTHERS. SEE C3 / S3.11 AND B1 / S3.31
- 18" DIAMETER PRECAST CONCRETE CANOPY COLUMN BY OTHERS. SEE C3 / S3.11, C4 / S3.12, A1 / S3.31, AND A5 / S3.31

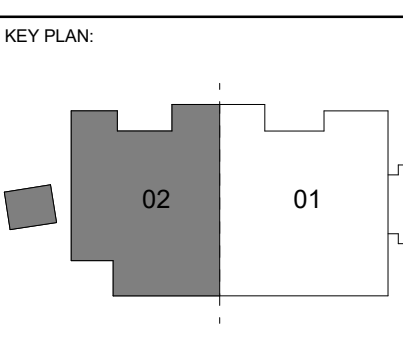
A1 FOUNDATION PLAN - SECTOR 2
SCALE: 1/8" = 1'-0"



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WILMA P. MANKILLER HEALTH CENTER
EXPANSION
STILWELL, OKLAHOMA



PROJECT PHASE:
BID PACKAGE 01

#	DATE	REVISIONS	DESCRIPTION
1	11/02/19	BID PACKAGE 01 - ADD 01	

DATE: 11-01-19
JOB NUMBER: 18-01.01

SHEET NUMBER:
S1.02

FOUNDATION PLAN
SECTOR 2

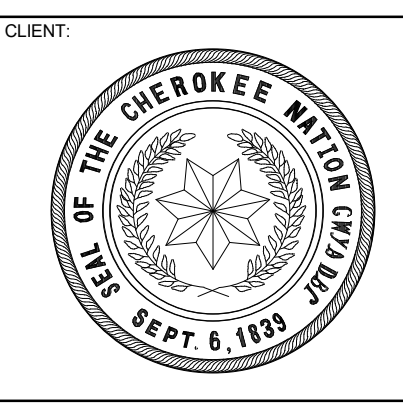
GENERAL SHEET NOTES

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4. SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
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6. STRUCTURAL COLD FORMED METAL STUDS SHALL BE 60S162-43 AT 16" ON CENTER UNLESS NOTED OTHERWISE.
7. BEAMS AND JOISTS ARE SPACED EQUALLY BETWEEN GRIDS AND COLUMNS UNLESS NOTED OTHERWISE.
8. SEE SHEET S7.00 SERIES SHEETS FOR TYPICAL FLOOR FRAMING SECTIONS.
9. SEE SHEET S6.01 FOR SCHEDULES.
10. DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
11. DENOTES SIDEPATE MOMENT CONNECTION. SEE SIDEPATE DRAWINGS.

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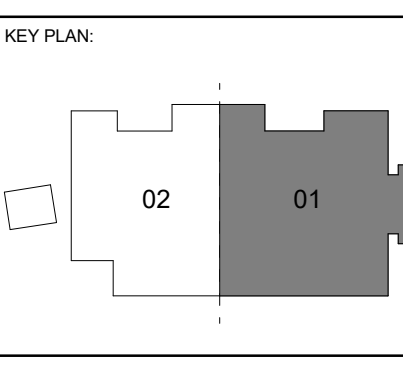
CG Chavez-Grievos consulting engineers, inc. 4300 Lincoln Road, Suite 102, Chickasha, OK 73019 505-344-4080 505-343-8759 (fax)



SHEET KEYNOTE

- 1. MECHANICAL UNIT, COORDINATE EXACT SIZE AND LOCATION WITH MECHANICAL DRAWINGS.
2. OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A5 / S5.52 AND B5 / S5.52 FOR SUPPORT.
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5. HSS6x4x1/4 COLLECTOR BLOCKING BETWEEN BEAMS. SEE D4 / S5.52. ATTACH BLOCKING TO DECK VALLEYS PER DECK SCHEDULE. PROVIDE 20 GAGE PLATE AS REQUIRED TO MAKE ATTACHMENT.
6. 4" HOUSEKEEPING PAD REINFORCED WITH #4 @ 18" ON CENTER EACH WAY AND #4 VERT DOWELS DRILLED AND EPOXIED 2" INTO CONCRETE SLAB BELOW @ 48" ON CENTER EACH WAY (12" FROM EDGES AND CORNERS). PAD SHALL EXTEND 6" BEYOND FACE OF MECHANICAL UNIT ALL AROUND. COORDINATE EXACT SIZE AND LOCATION OF PAD WITH MECHANICAL DRAWINGS.
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8. BOTTOM FLANGE BRACING AT EQUAL SPACING. UNLESS NOTED OTHERWISE, BRACE TO BE ATTACHED TO BOTTOM FLANGE OF BEAM NOTED AS MOMENT FRAME OR BRACED FRAME TO TOP FLANGE OF ADJACENT BEAM. SEE B3 / S5.52
9. BOTTOM FLANGE BRACING. SEE A3 / S5.52
10. HSS6x4x1/2 ELEVATOR RAIL SUPPORT BEAM. COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE A1 / S5.41 AND B1 / S5.41 FOR TYPICAL DETAILS.
11. 2" BUILDING EXPANSION JOINT. SEE ARCHITECTURAL DRAWINGS.
12. SLAB EDGE TO BE LOCATED 6" FROM GRID. SEE S7.41 FOR SLAB EDGE DETAILS.

WILMA P. MANKILLER HEALTH CENTER EXPANSION STILWELL, OKLAHOMA



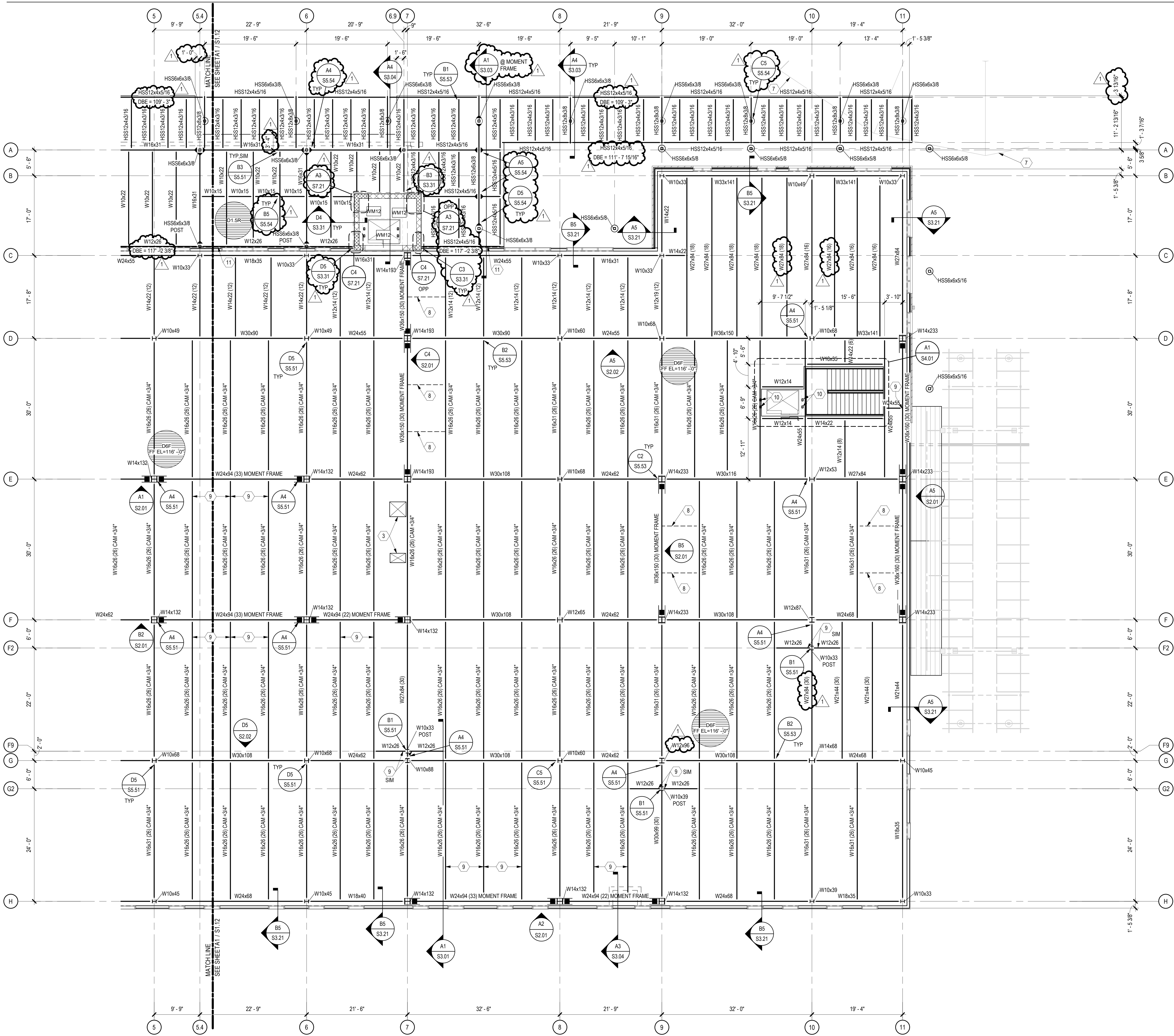
PROJECT PHASE: BID PACKAGE 01

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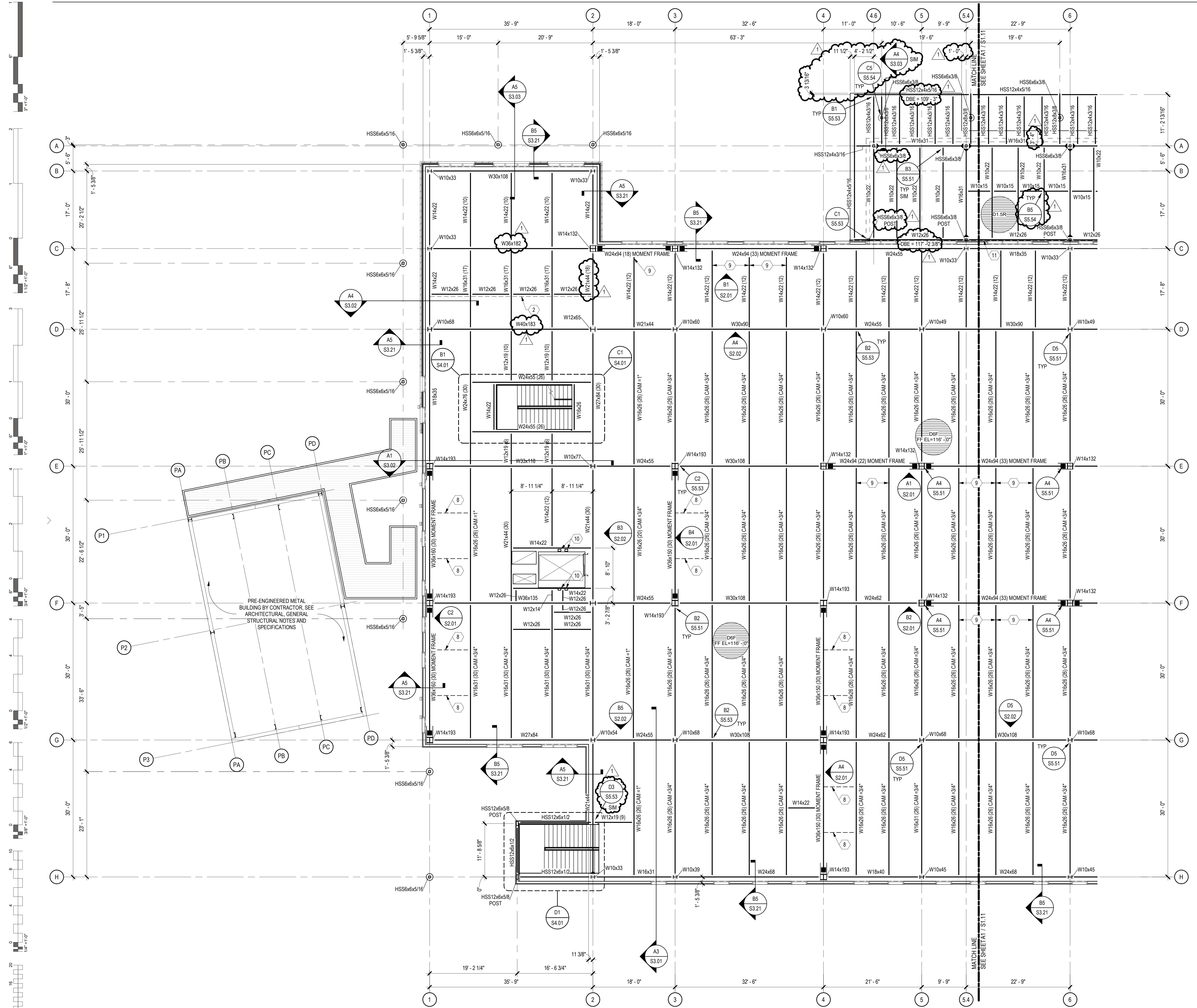
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FLOOR FRAMING PLAN - SECTOR 1



A1 FLOOR FRAMING PLAN - SECTOR 1 SCALE: 1/8" = 1'-0"



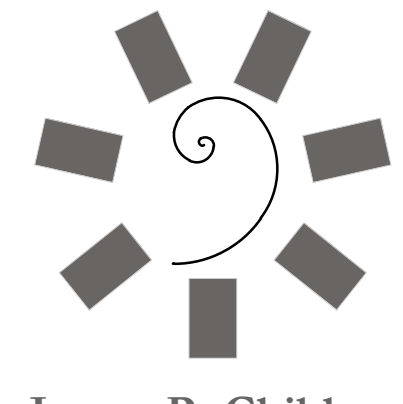


GENERAL SHEET NOTES

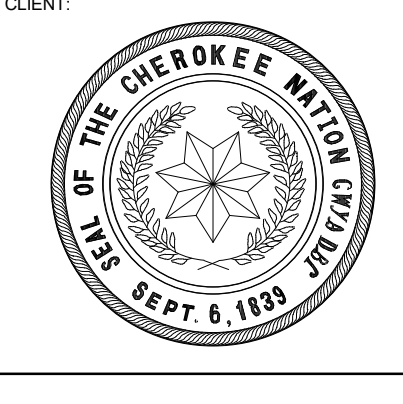
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- SEE SHEET S7.00 SERIES SHEETS FOR TYPICAL FLOOR FRAMING SECTIONS.
- SEE SHEET S6.01 FOR SCHEDULES.
- DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
- DENOTES SIDEPATE MOMENT CONNECTION. SEE SIDEPATE DRAWINGS.

SHEET KEYNOTE

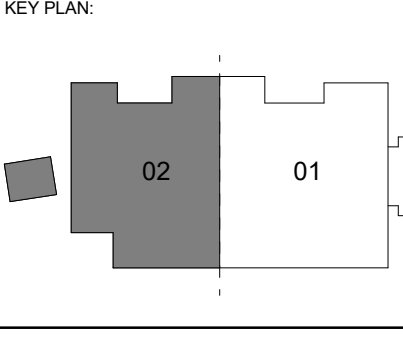
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- OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A5 / S5.52 AND B5 / S5.52 FOR SUPPORT.
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- BOTTOM FLANGE BRACING. SEE A3 / S5.52
- HSS6x1/2 ELEVATOR RAIL SUPPORT BEAM. COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE A1 / S5.41 AND B1 / S5.41 FOR TYPICAL DETAILS.
- 2" BUILDING EXPANSION JOINT. SEE ARCHITECTURAL DRAWINGS.
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PROJECT PHASE:
BID PACKAGE 01

#	DATE	REVISIONS	DESCRIPTION
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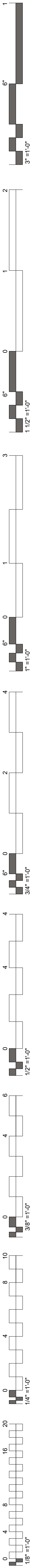
DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER:
S1.12

FLOOR FRAMING PLAN -
SECTOR 2

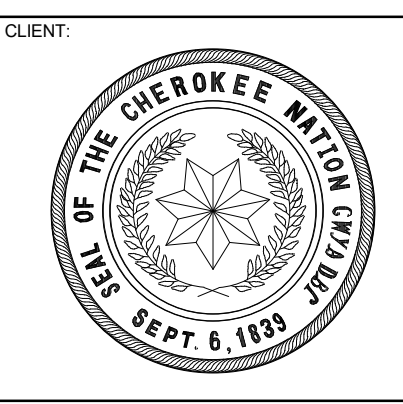
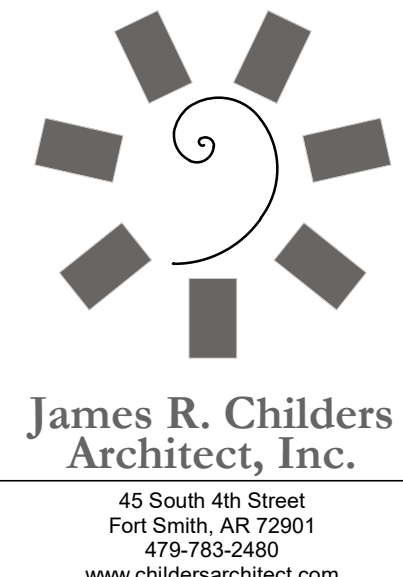
A1 FLOOR FRAMING PLAN - SECTOR 2
SCALE: 1/8" = 1'-0"





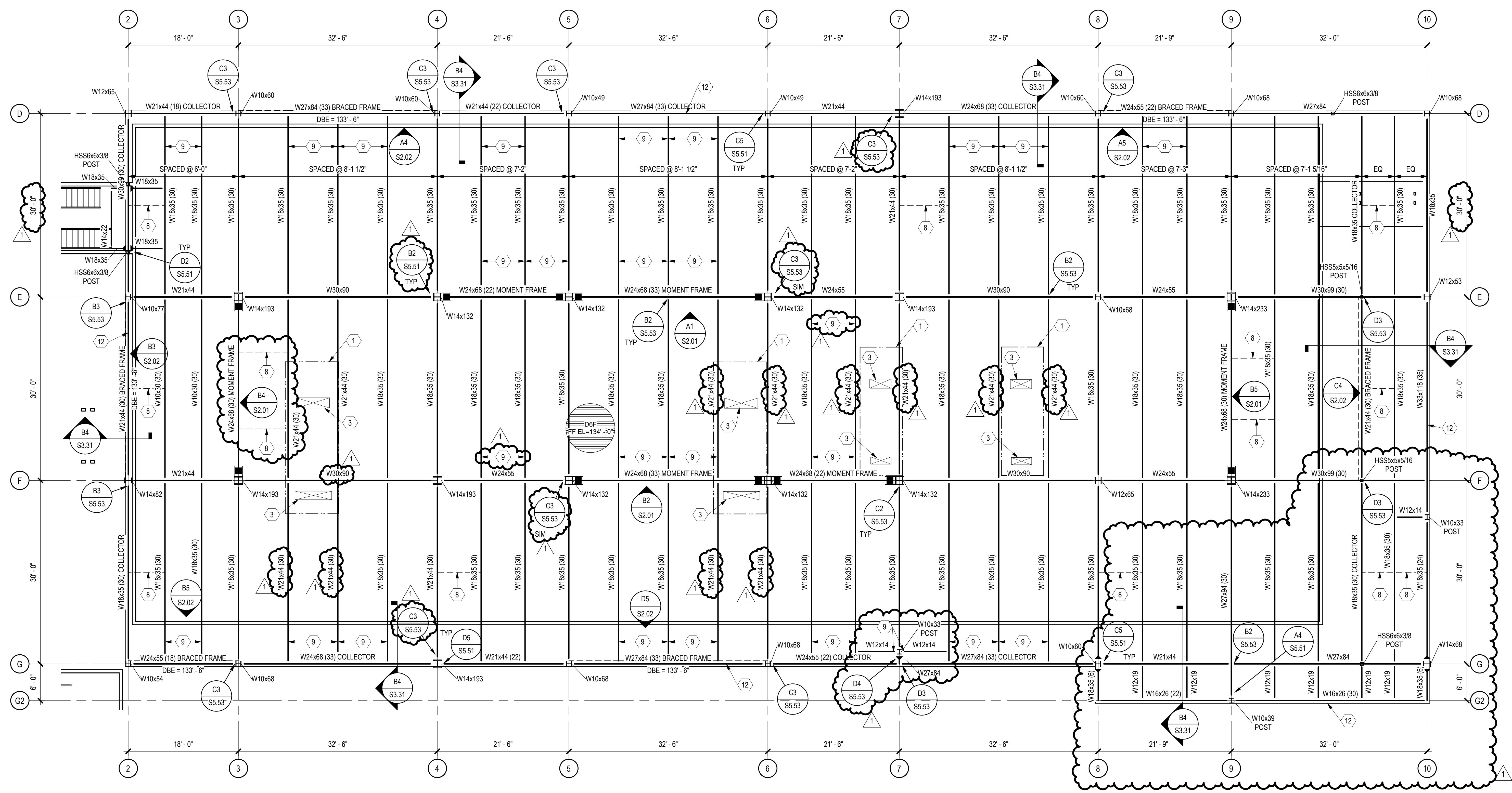
GENERAL SHEET NOTES

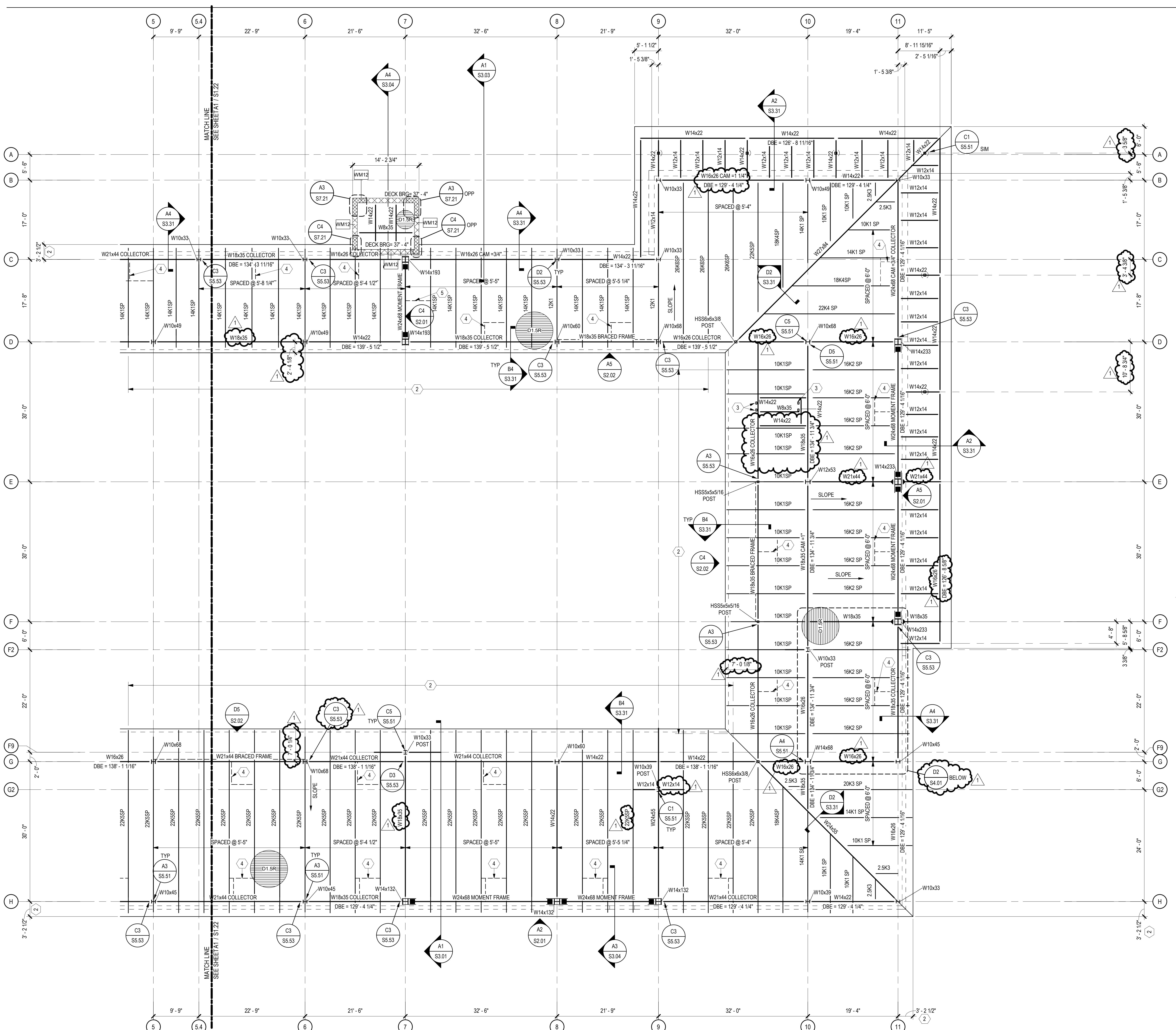
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- SEE SHEET S7.00 SERIES SHEETS FOR TYPICAL FLOOR FRAMING SECTIONS.
- SEE SHEET S6.01 FOR SCHEDULES.
- ▬ DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
- ▬ DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE DRAWINGS.



SHEET KEYNOTE

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- OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A5 / S5.52 AND B5 / S5.52 FOR SUPPORT.
- MECHANICAL OPENING. COORDINATE EXACT SIZE AND LOCATION WITH MECHANICAL DRAWINGS. SEE C5 / S7.42
- HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST. COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE A2 / S5.41, B2 / S5.41, C2 / S5.41, AND D3 / S5.41
- HSS6x4x1/4 COLLECTOR BRACING BETWEEN BEAMS. SEE D4 / S5.52. ATTACH BRACING TO DECK VALLEYS PER DECK SCHEDULE. PROVIDE 20 GAGE PLATE AS REQUIRED TO MAKE ATTACHMENT.
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- BOTTOM FLANGE BRACING AT EQUAL SPACING. UNLESS NOTED OTHERWISE, BRACE TO BE ATTACHED TO BOTTOM FLANGE OF BEAM NOTED AS MOMENT FRAME OR BRACED FRAME TO TOP FLANGE OF ADJACENT BEAM. SEE B3 / S5.52
- BOTTOM FLANGE BRACING. SEE A3 / S5.52
- HSS6x4x1/2 ELEVATOR RAIL SUPPORT BEAM. COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE A1 / S5.41 AND B1 / S5.41 FOR TYPICAL DETAILS.
- 2" BUILDING EXPANSION JOINT. SEE ARCHITECTURAL DRAWINGS.
- SLAB EDGE TO BE LOCATED 6" FROM GRID. SEE S7.41 FOR SLAB EDGE DETAILS.



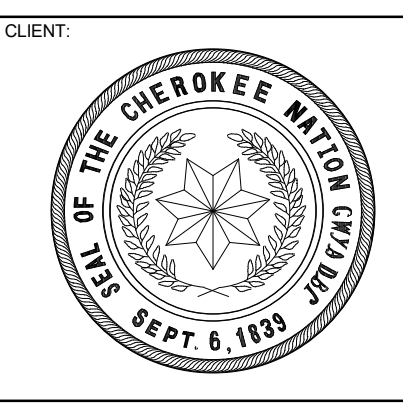


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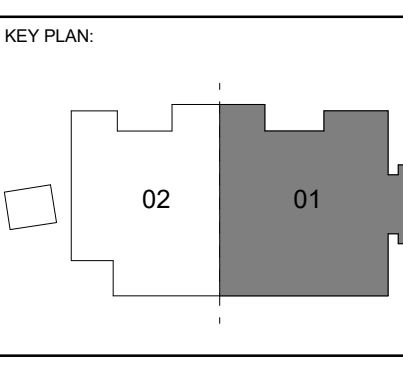
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- PROVIDE JOIST BRIDGING PER THE 42ND EDITION OF THE SJI SPECIFICATIONS AND OSHA REQUIREMENTS.
- STEEL JOIST MANUFACTURER SHALL DESIGN ROOF JOISTS AND ROOF JOIST GIRDERS SUPPORTING MECHANICAL UNITS FOR 1.2x MECHANICAL UNIT WEIGHTS SHOWN. USE 28 PSF DEAD LOAD AND 20 PSF LIVE LOAD UNLESS NOTED OTHERWISE. CONTRACTOR SHALL VERIFY ACTUAL MECHANICAL LOADS. NOTIFY STEEL JOIST MANUFACTURER OF ANY DISCREPANCIES.
- STRUCTURAL COLD FORMED METAL STUDS SHALL BE 6" IN WIDTH, UNLESS NOTED OTHERWISE.
- SEE SHEET S6.01 FOR SCHEDULES.
- NOTES DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
- NOTES DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE DRAWINGS.
- PROVIDE HSSxJOIST SEAT DEPTHx1/4 BETWEEN JOISTS AT ALL BEAMS LABELED AS: MOMENT FRAME, BRACED FRAME, AND COLLECTOR. SEE C2 / S7.41

SHEET KEYNOTE

- MECHANICAL UNIT. COORDINATE EXACT SIZE AND LOCATION WITH MECHANICAL DRAWINGS.
- JOIST EXTENDED END. DESIGN EXTENDED END FOR 20 PSF DEAD LOAD, 20 PSF LIVE LOAD, AND ANY POSITIVE OR NEGATIVE WIND PRESSURES PER ROOF WIND LOADING DIAGRAM ON S0.03. DEPTH OF EXTENDED END PER JOIST MANUFACTURER.
- HSS8x12 ELEVATOR RAIL SUPPORT POST. COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE A2 / S5.41, B2 / S5.41, C2 / S5.41, AND D3 / S5.41
- BOTTOM FLANGE BRACE AT EQUAL SPACING, UNLESS NOTED OTHERWISE. BRACE TO BE ATTACHED TO BOTTOM FLANGE OF BEAM NOTED AS MOMENT FRAME OR BRACED FRAME AND TO TOP FLANGE OF ADJACENT BEAM OR JOIST. SEE B3 / S5.52. JOISTS TO BE DESIGNED FOR 1,500# VERTICAL, (REVERSIBLE) WIND AND SEISMIC LOAD FROM BRACE.
- BOTTOM FLANGE BRACING AT EQUAL SPACING, UNLESS NOTED OTHERWISE. SEE D1 / S5.51. JOISTS TO BE DESIGNED FOR 1,500# VERTICAL, (REVERSIBLE) WIND AND SEISMIC LOAD FROM BRACE.
- BOTTOM FLANGE BRACING ANGLE. SEE A3 / S5.52
- R1 JOIST EXTENDED END.



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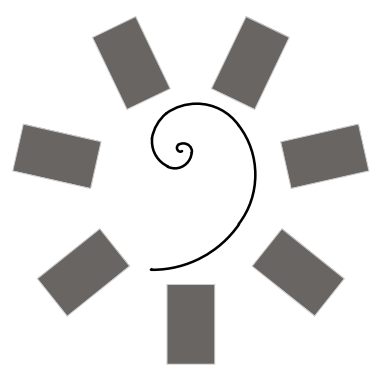
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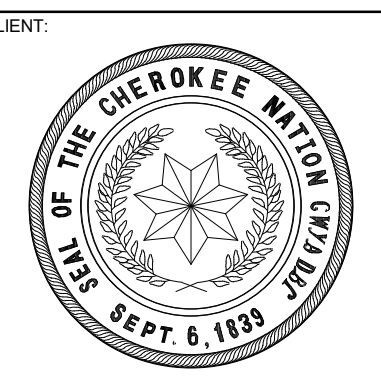
ROOF FRAMING PLAN - SECTOR 1

A1 ROOF FRAMING PLAN - SECTOR 1
SCALE: 1/8" = 1'-0"

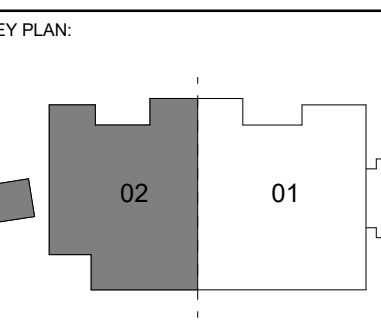




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SHEET NUMBER: S1.22

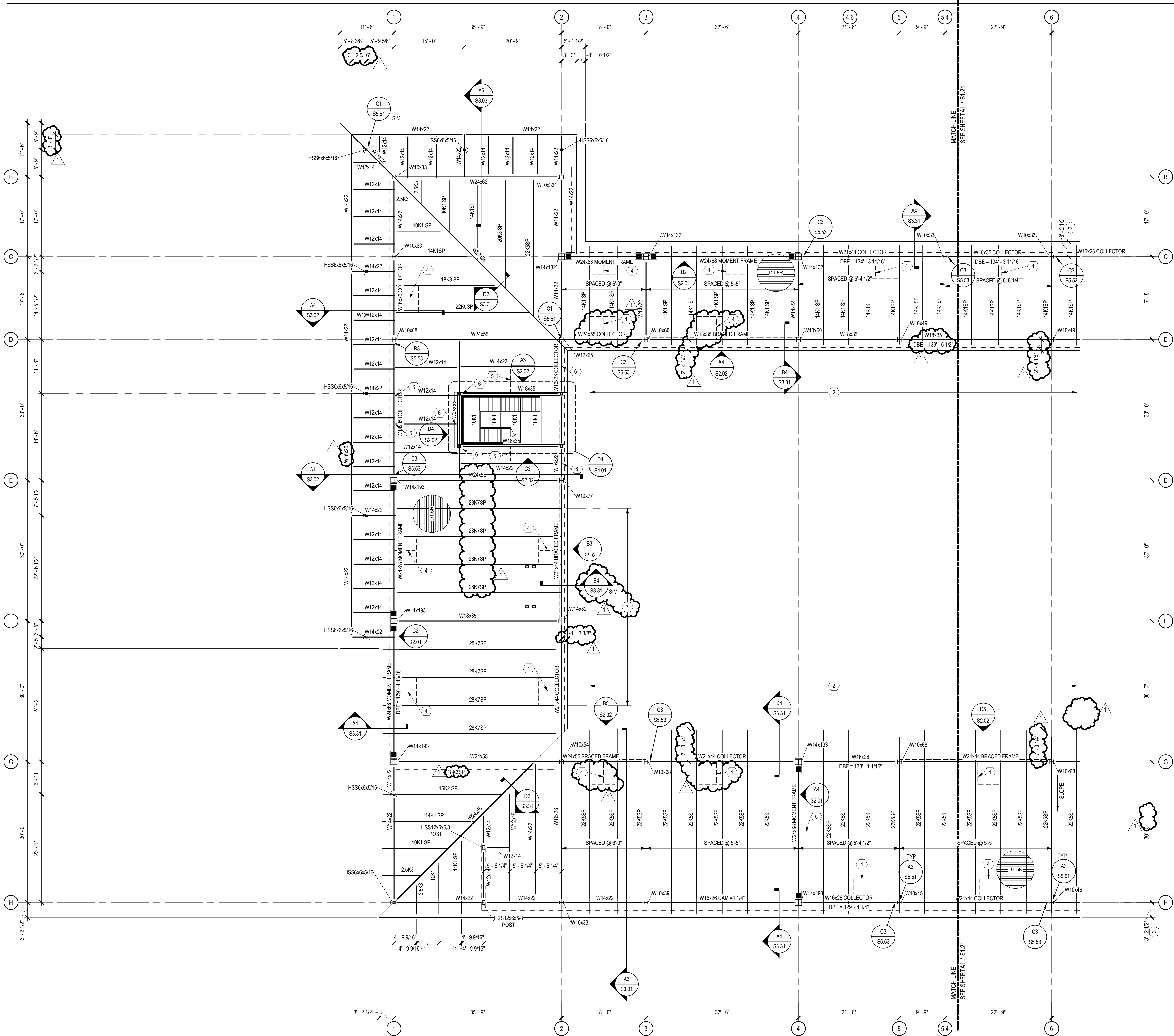
ROOF FRAMING PLAN - SECTOR 2

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- SEE SHEET S6.01 FOR SCHEDULES.
- 4 DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
- 4 DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE DRAWINGS.
- PROVIDE HSSxJOIST SEAT DEPTHx1/4 BETWEEN JOISTS AT ALL BEAMS LABELED AS: MOMENT FRAME, BRACED FRAME, AND COLLECTOR. SEE C2 / S7.41

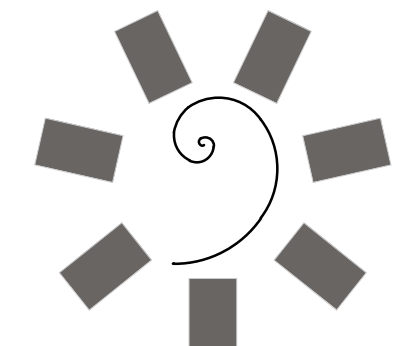
SHEET KEYNOTE

- MECHANICAL UNIT. COORDINATE EXACT SIZE AND LOCATION WITH MECHANICAL DRAWINGS.
- JOIST EXTENDED END. DESIGN EXTENDED END FOR 20 PSF DEAD LOAD, 20 PSF LIVE LOAD, AND ANY POSITIVE OR NEGATIVE WIND PRESSURES PER ROOF WIND LOADING DIAGRAM ON S0.03. DEPTH OF EXTENDED END PER JOIST MANUFACTURER.
- HSS6x1/2 ELEVATOR RAIL SUPPORT POST. COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE A2 / S5.41, B2 / S5.41, C2 / S5.41, AND D3 / S5.41
- BOTTOM FLANGE BRACE AT EQUAL SPACING, UNLESS NOTED OTHERWISE. BRACE TO BE ATTACHED TO BOTTOM FLANGE OF BEAM NOTED AS MOMENT FRAME OR BRACED FRAME AND TO TOP FLANGE OF ADJACENT BEAM OR JOIST. SEE B3 / S5.52. JOISTS TO BE DESIGNED FOR 1,500# VERTICAL (REVERSIBLE) WIND AND SEISMIC LOAD FROM BRACE.
- BOTTOM FLANGE BRACING AT EQUAL SPACING, UNLESS NOTED OTHERWISE. SEE D1 / S5.51. JOISTS TO BE DESIGNED FOR 1,500# VERTICAL (REVERSIBLE) WIND AND SEISMIC LOAD FROM BRACE.
- BOTTOM FLANGE BRACING ANGLE. SEE A3 / S5.52
- R1 JOIST EXTENDED END.

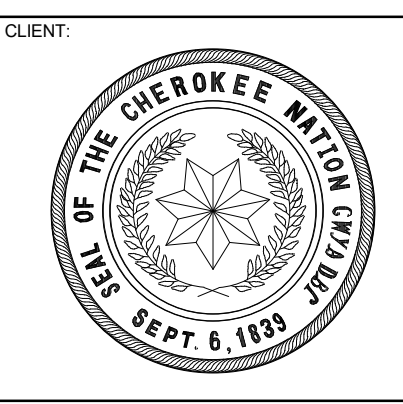


SIDEPLATE CONNECTION NOTES

(AXX) INDICATES SIDEPLATE CONNECTION PER SHEETS S8.01 - S8.07.
 (MX) INDICATES MISCELLANEOUS SIDEPLATE DETAIL PER SHEETS S8.01 - S8.07.



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 EXPANSION**
 STILLWELL, OKLAHOMA

KEY PLAN:

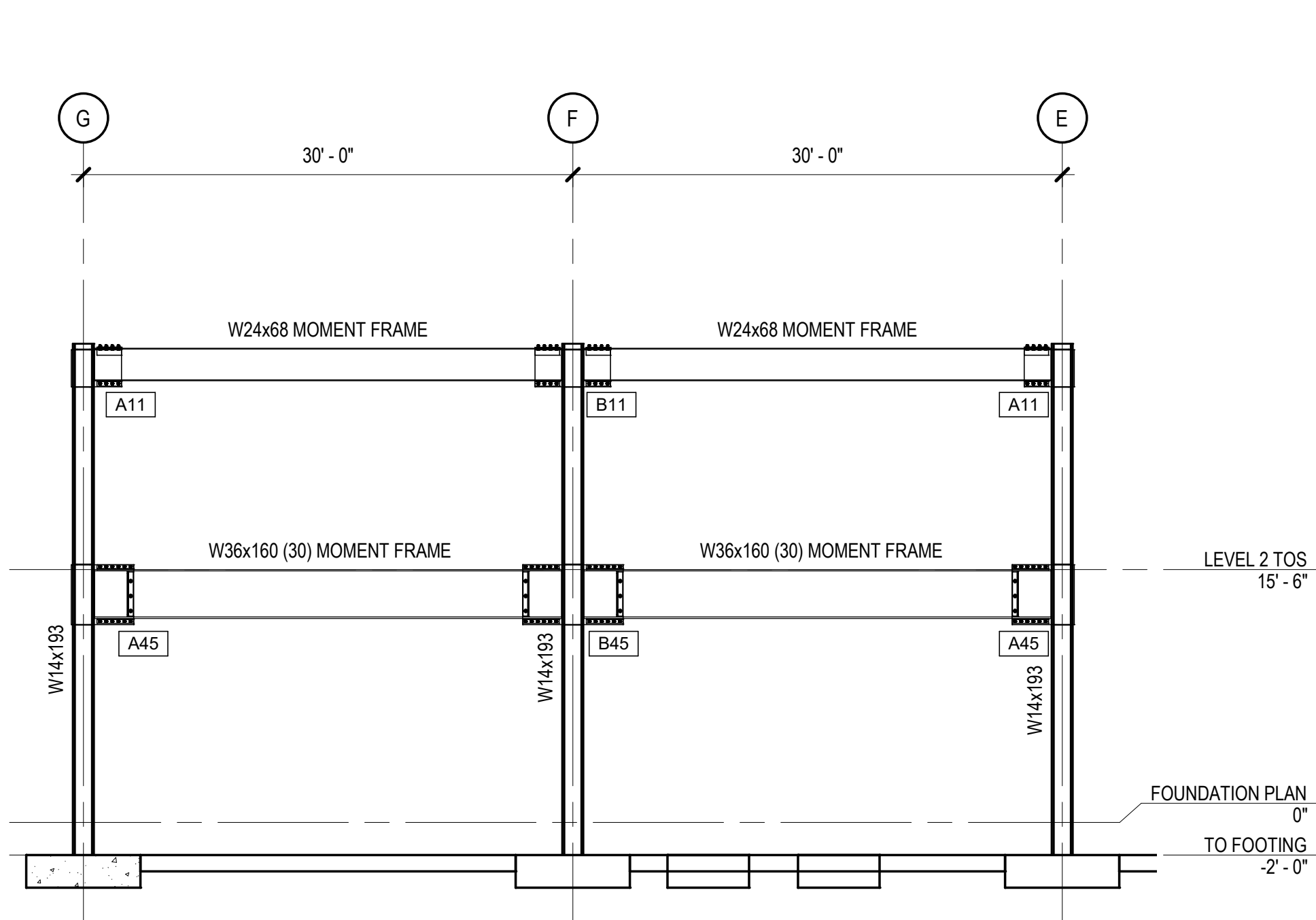
PROJECT PHASE:
 BID PACKAGE 01

#	DATE	REVISIONS	DESCRIPTION
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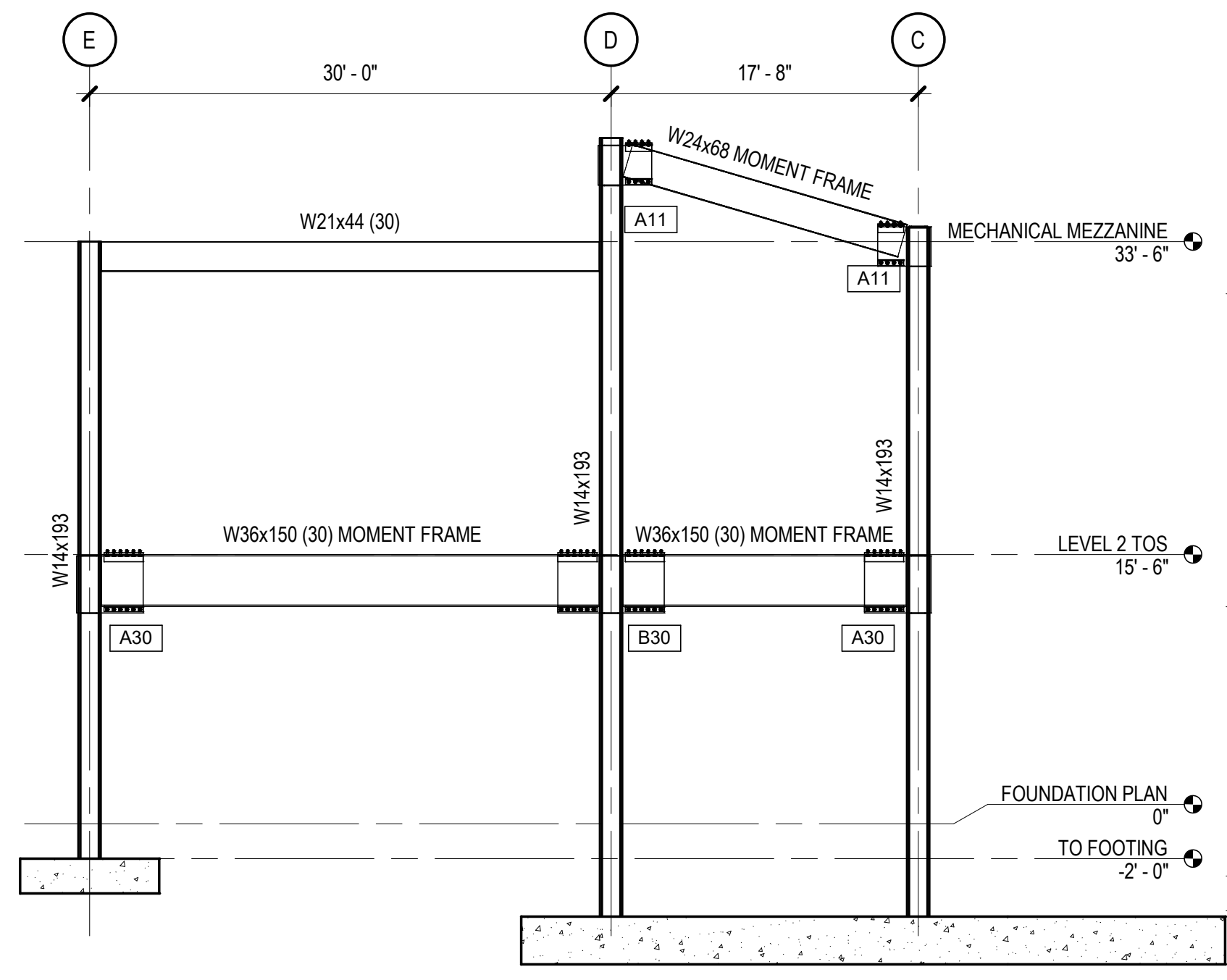
DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER:
S2.01

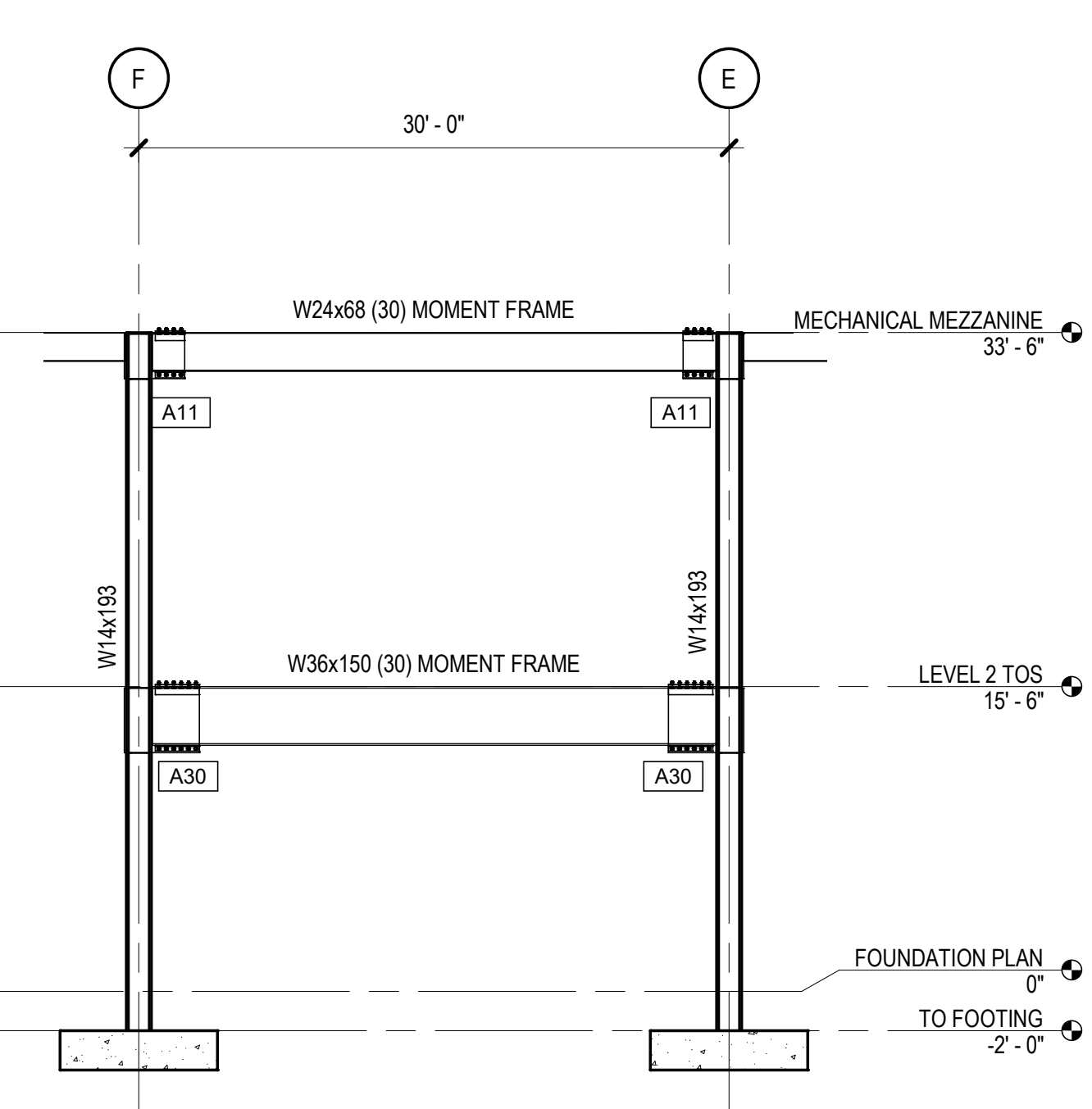
MOMENT FRAME ELEVATIONS



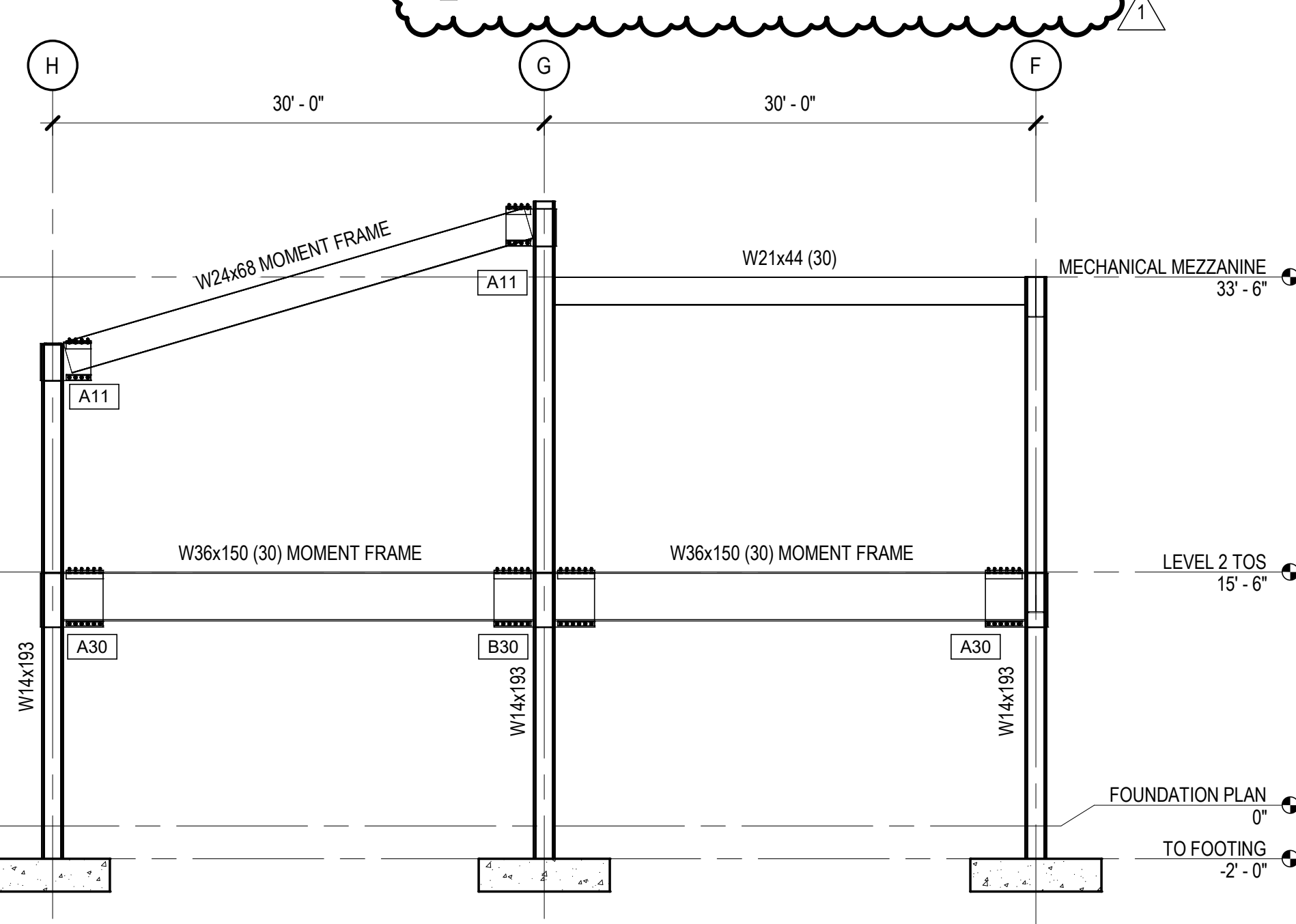
C2 MOMENT FRAME ELEVATION
 SCALE: 1/8" = 1'-0"



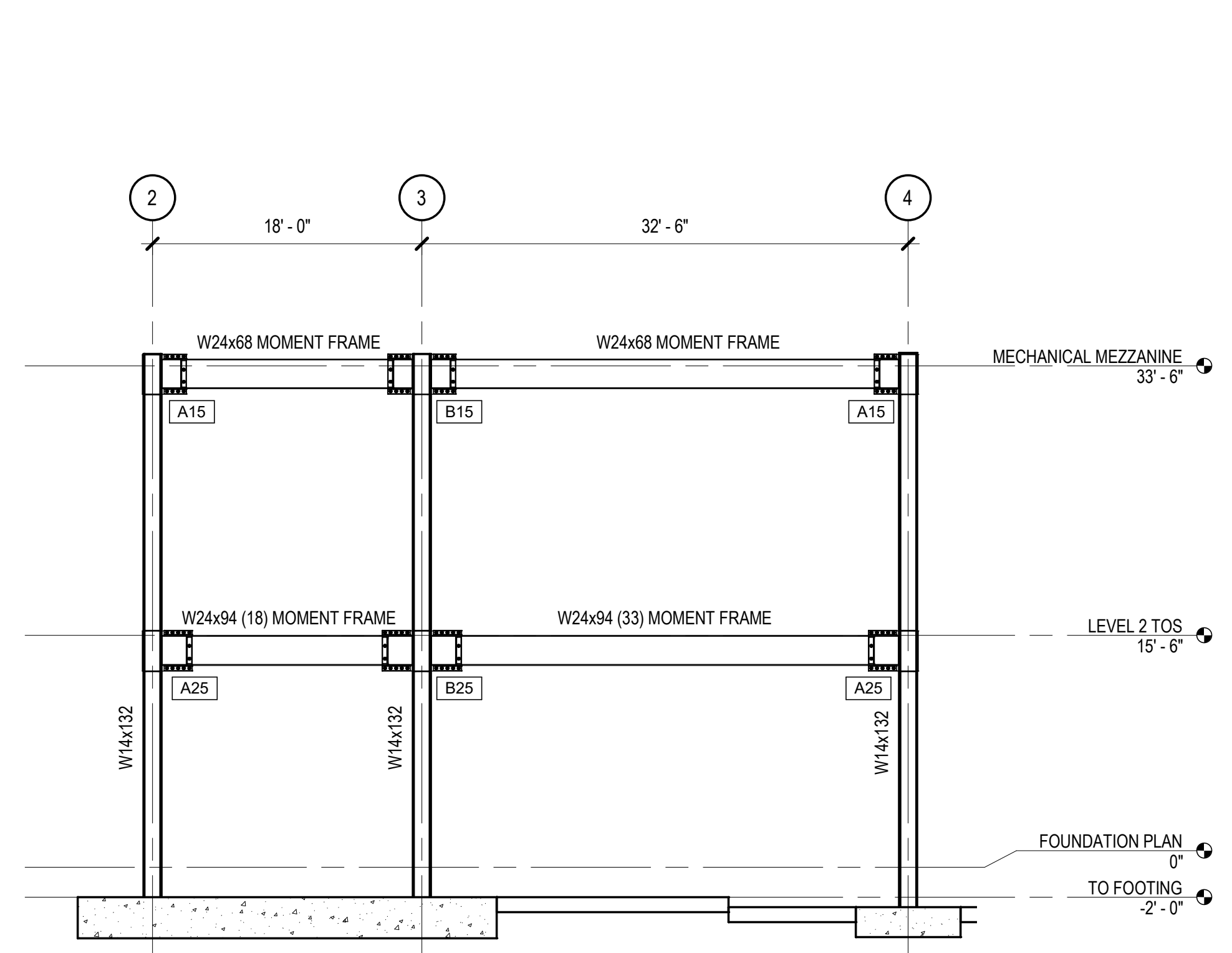
C4 MOMENT FRAME ELEVATION
 SCALE: 1/8" = 1'-0"



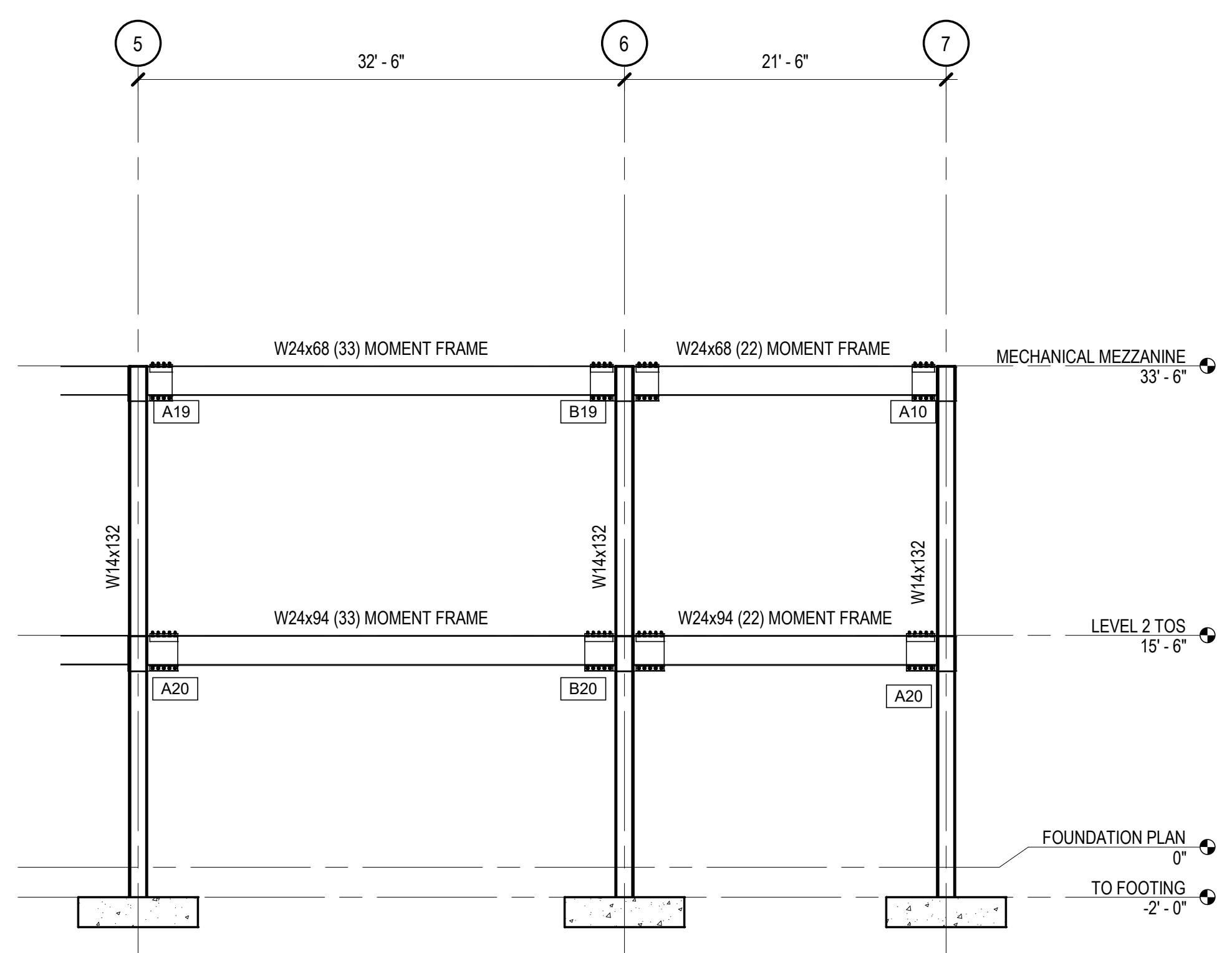
B4 MOMENT FRAME ELEVATION
 SCALE: 1/8" = 1'-0"



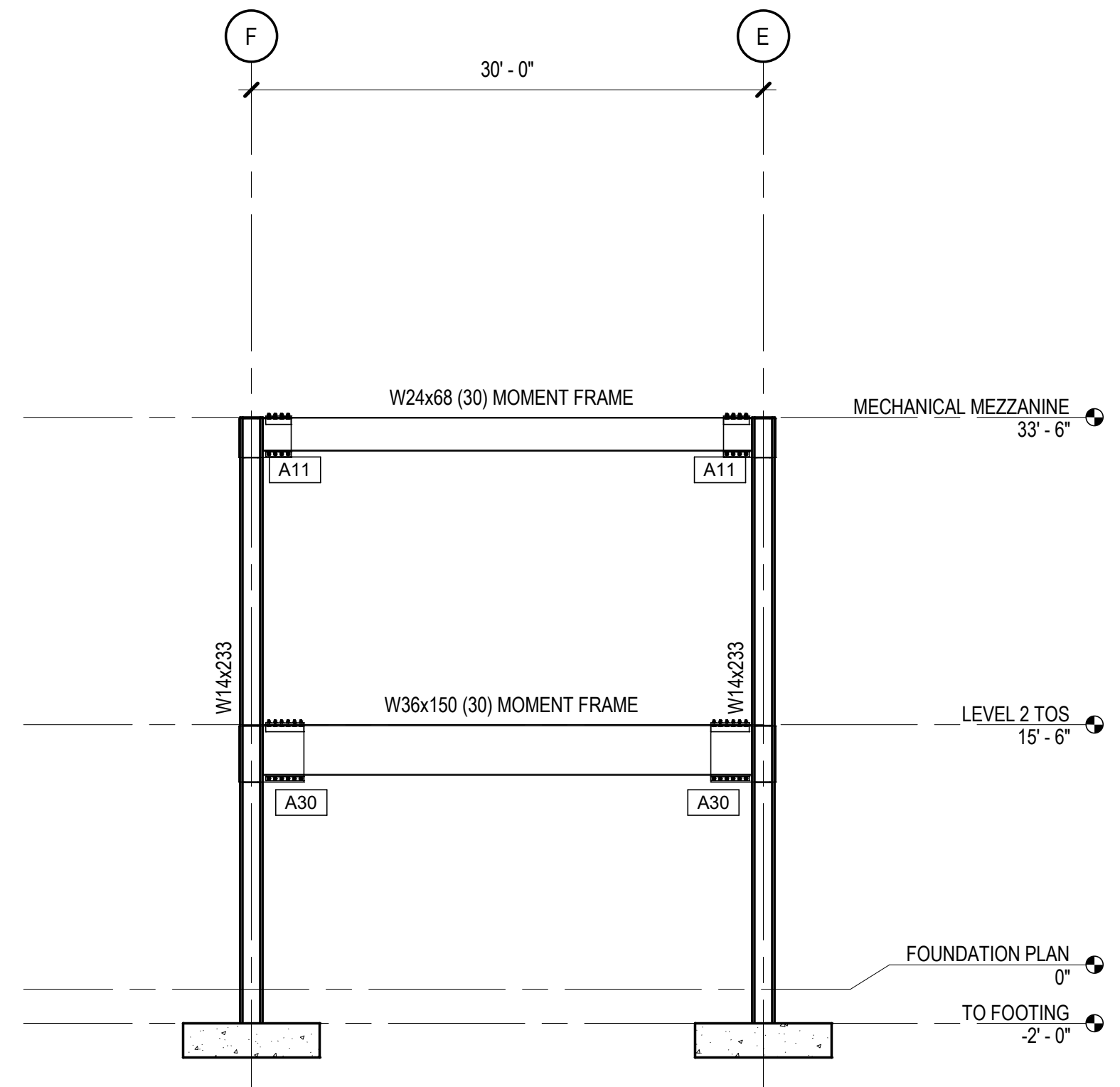
A4 MOMENT FRAME ELEVATION
 SCALE: 1/8" = 1'-0"



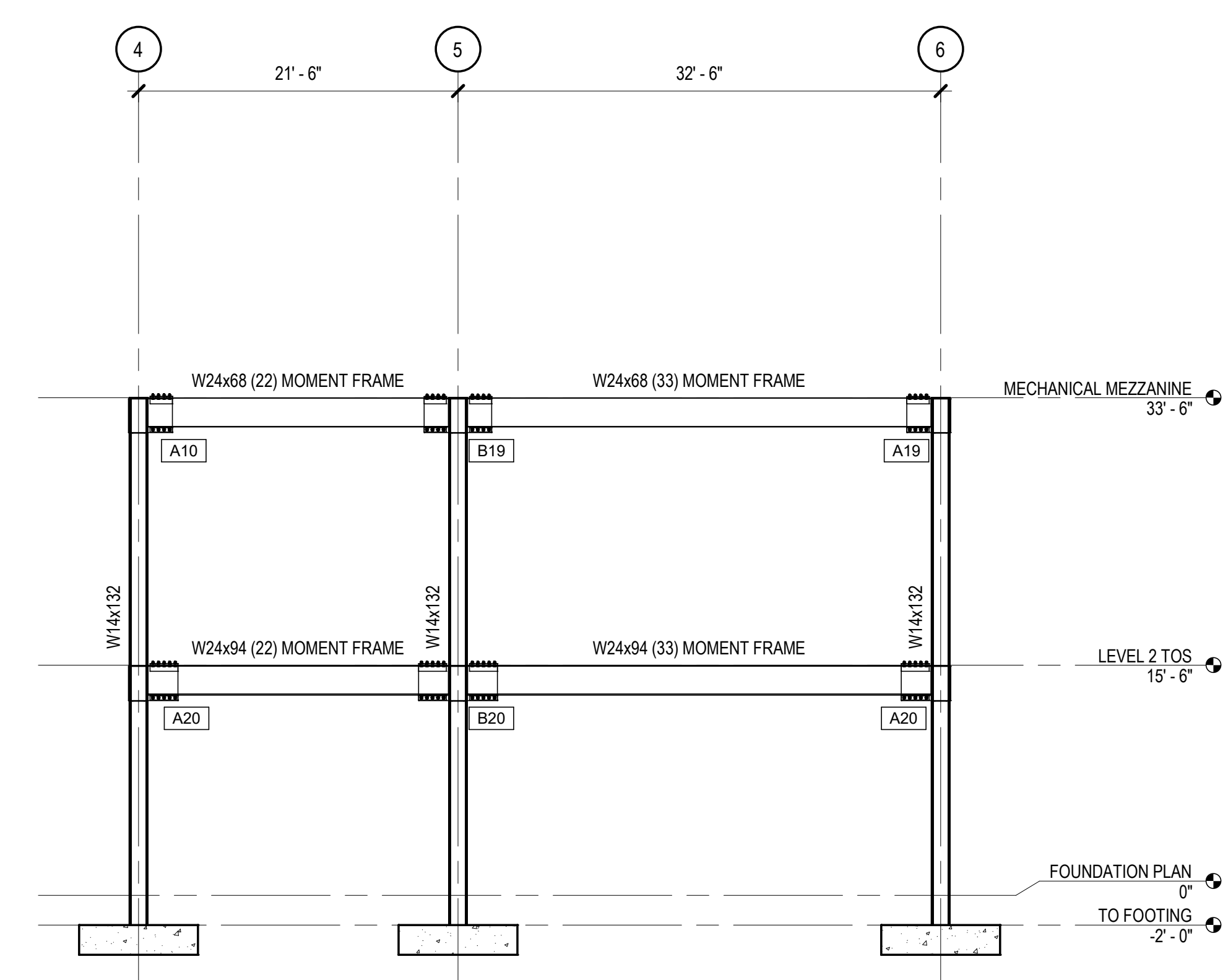
B1 MOMENT FRAME ELEVATION
 SCALE: 1/8" = 1'-0"



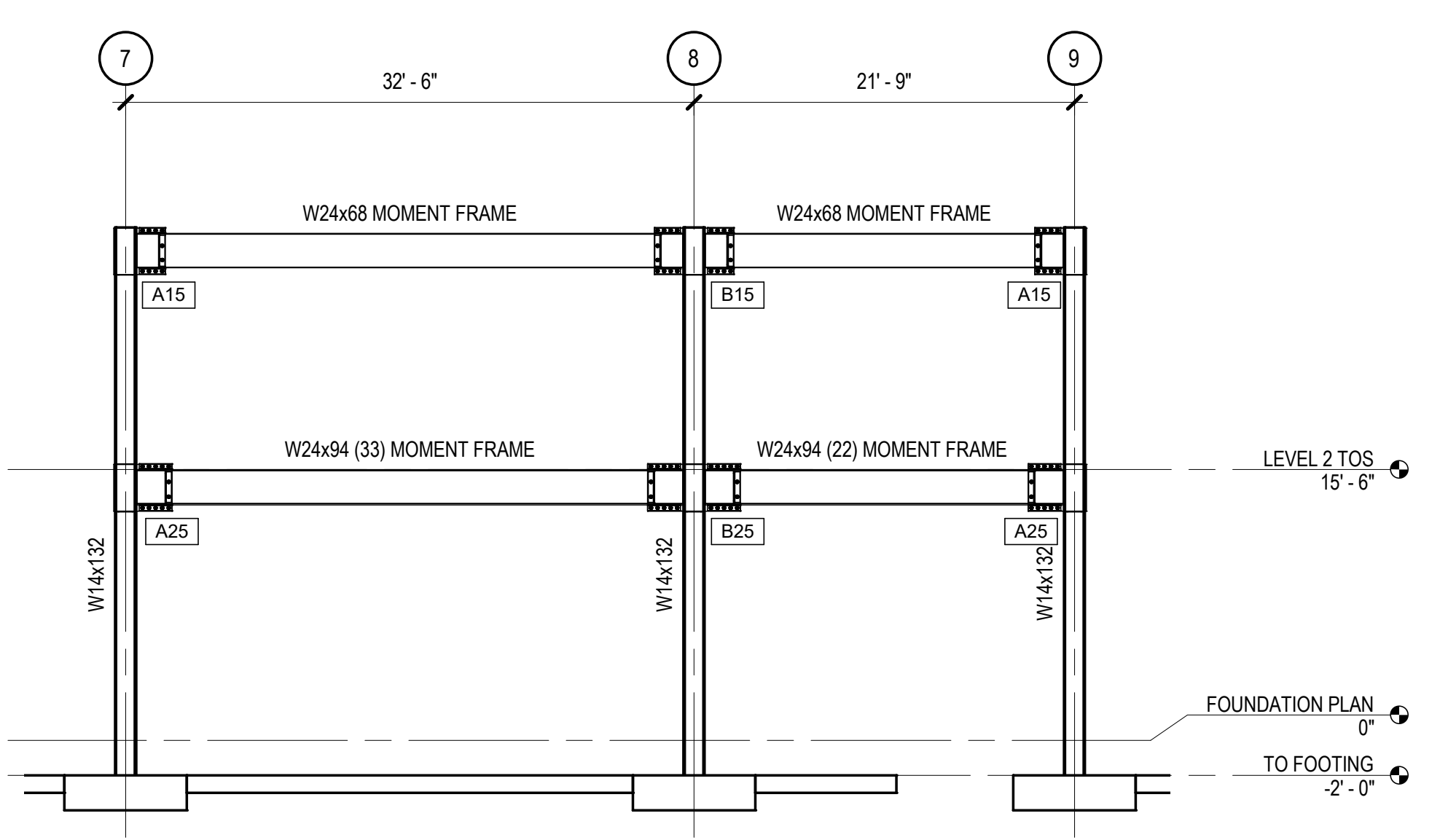
B2 MOMENT FRAME ELEVATION
 SCALE: 1/8" = 1'-0"



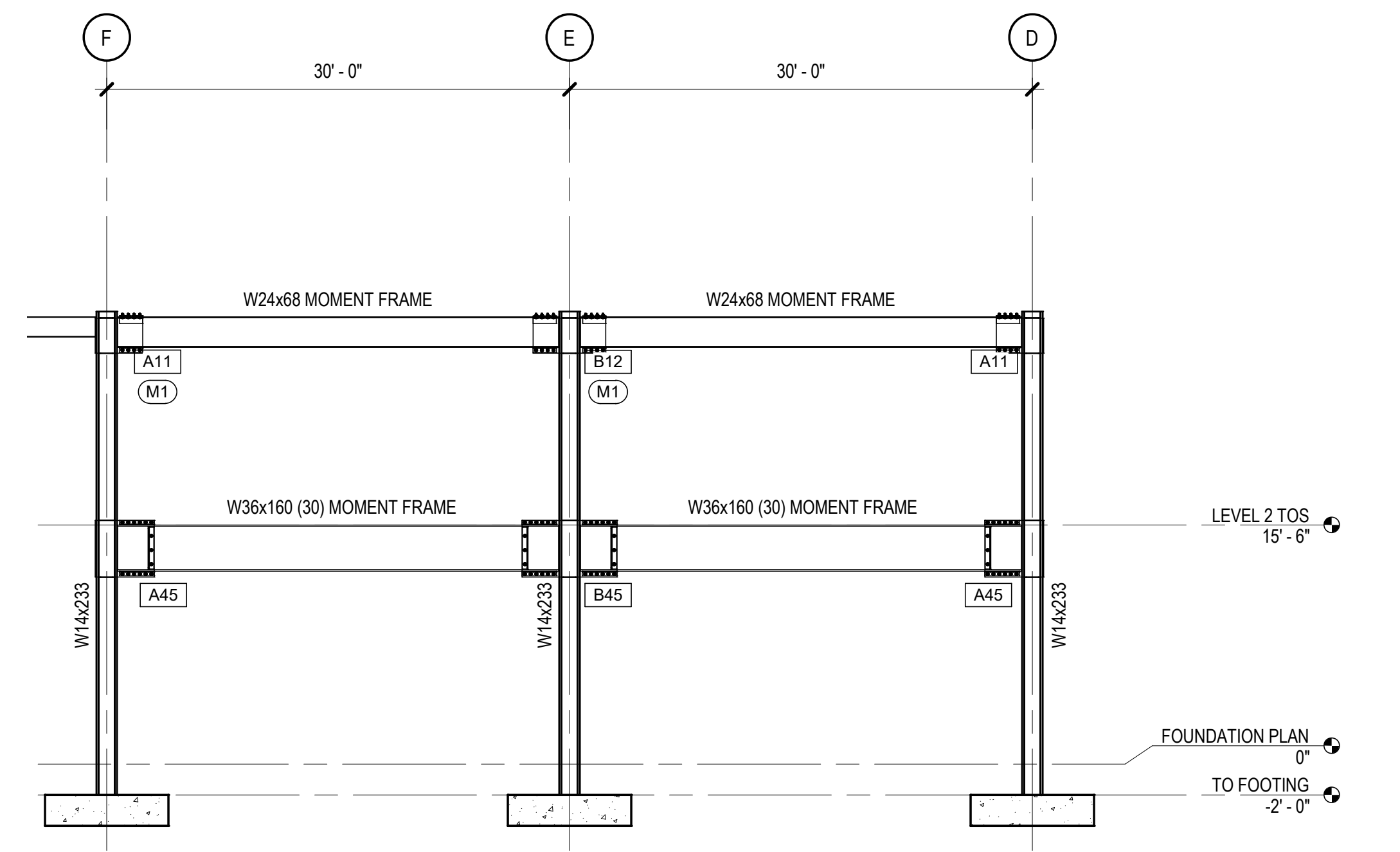
B5 MOMENT FRAME ELEVATION
 SCALE: 1/8" = 1'-0"



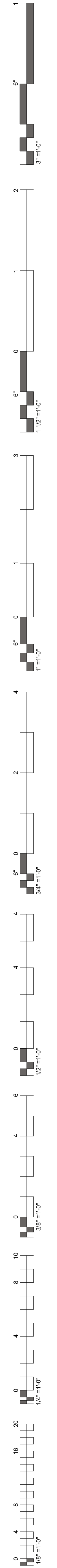
A1 MOMENT FRAME ELEVATION
 SCALE: 1/8" = 1'-0"

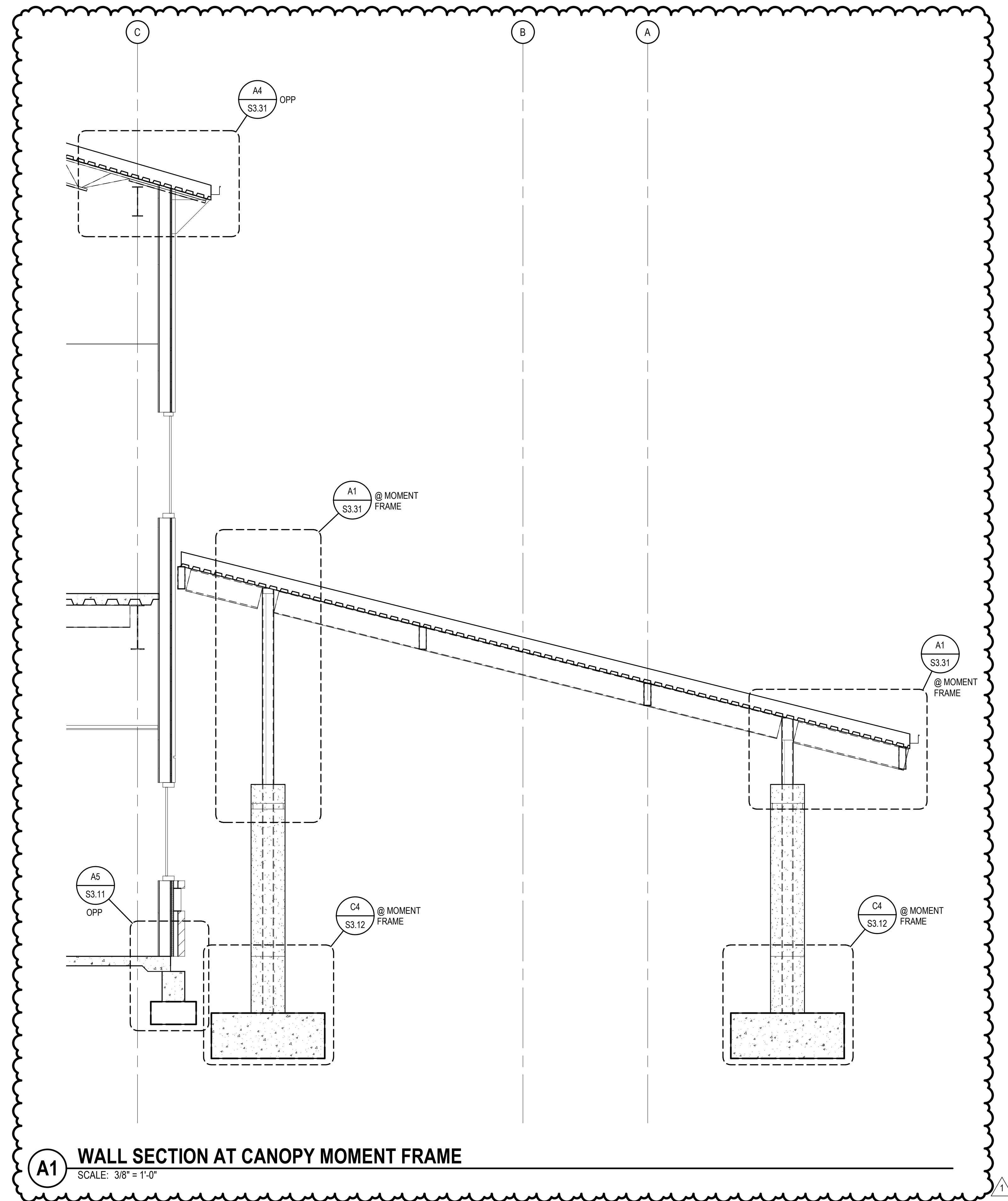
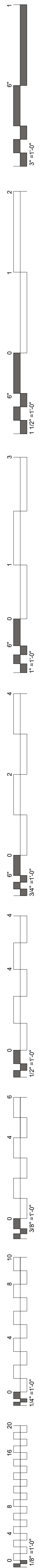


A2 MOMENT FRAME ELEVATION
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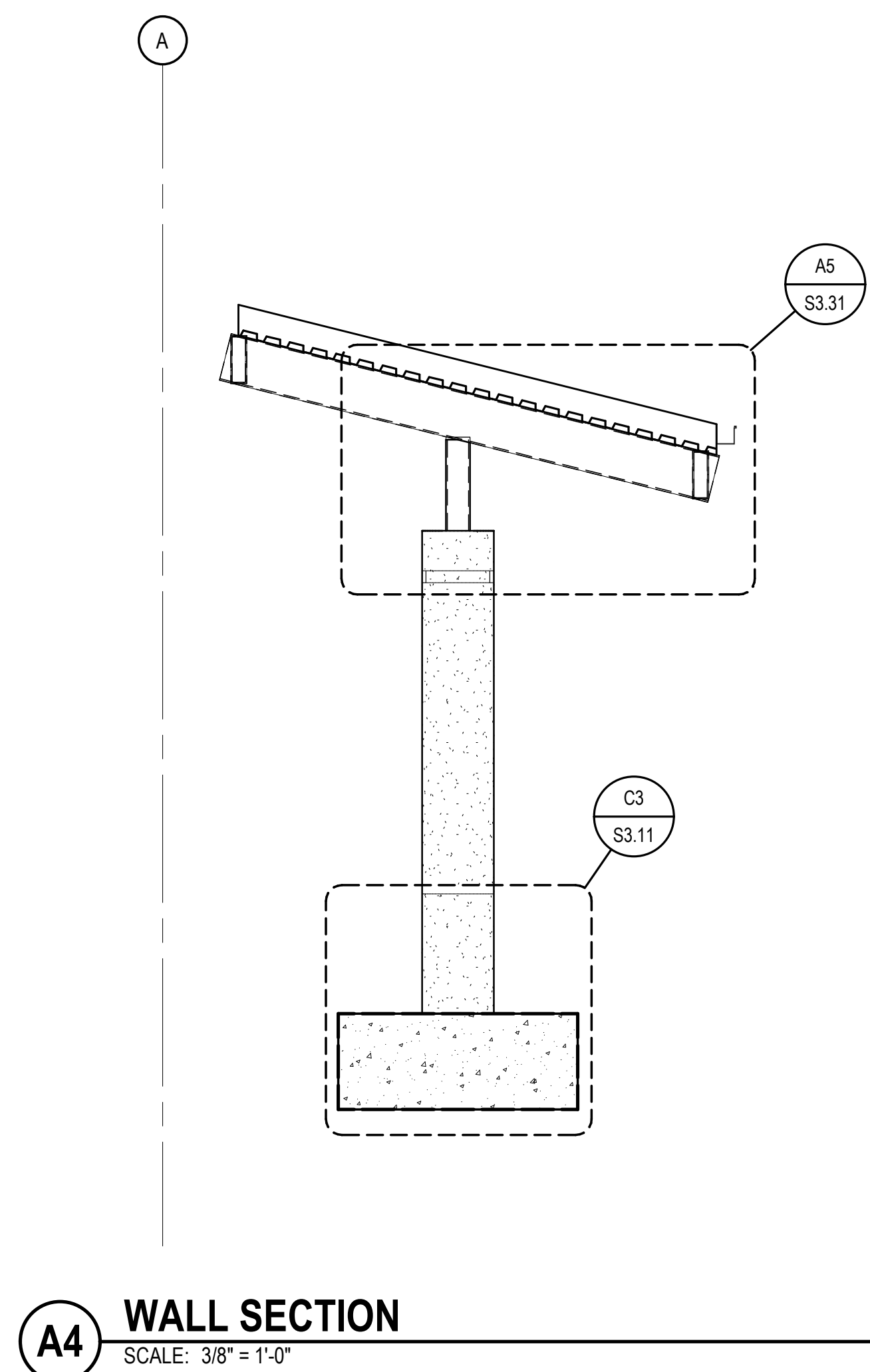


A5 MOMENT FRAME ELEVATION
 SCALE: 1/8" = 1'-0"

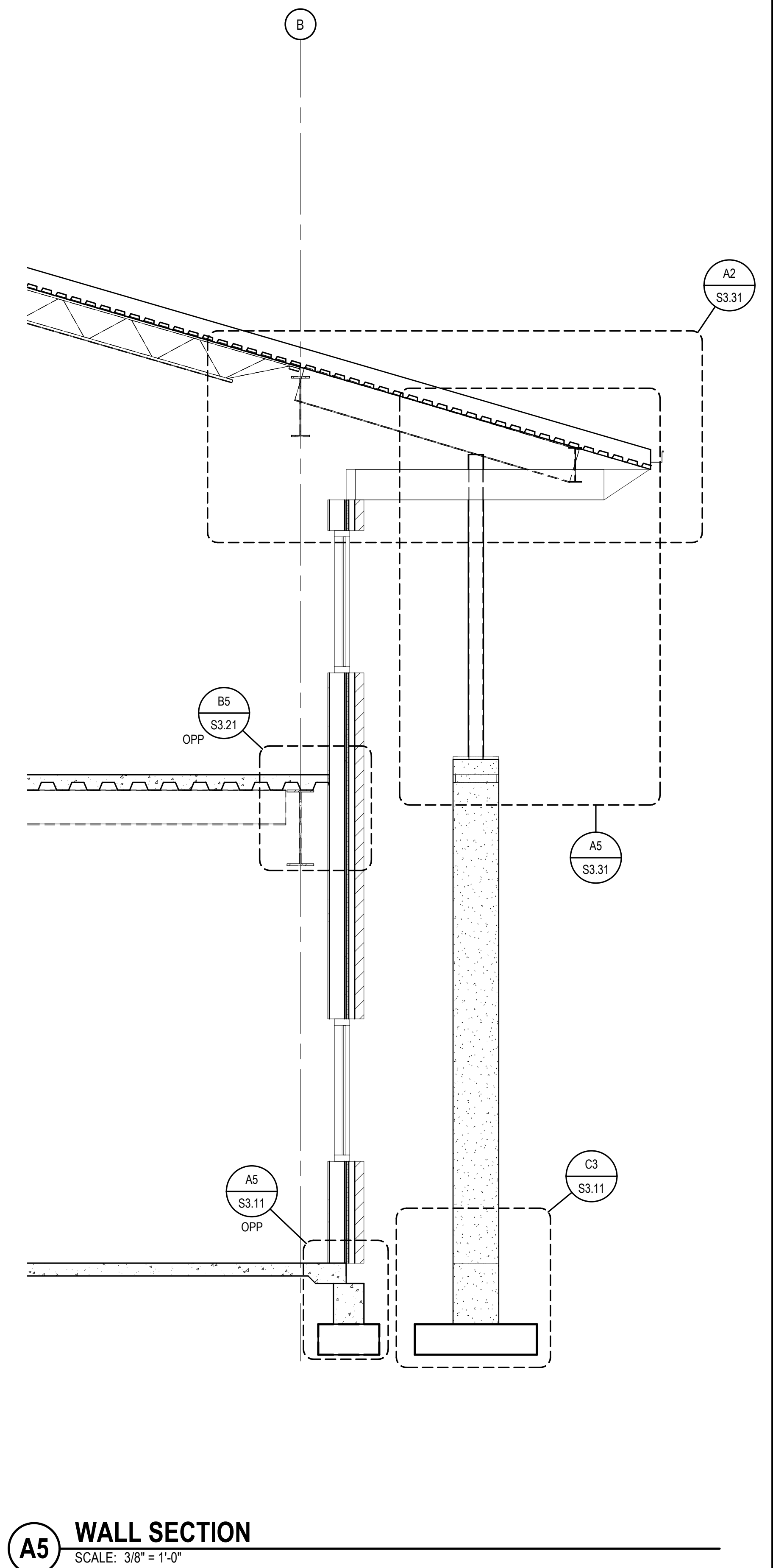




A1 WALL SECTION AT CANOPY MOMENT FRAME
SCALE: 3/8" = 1'-0"

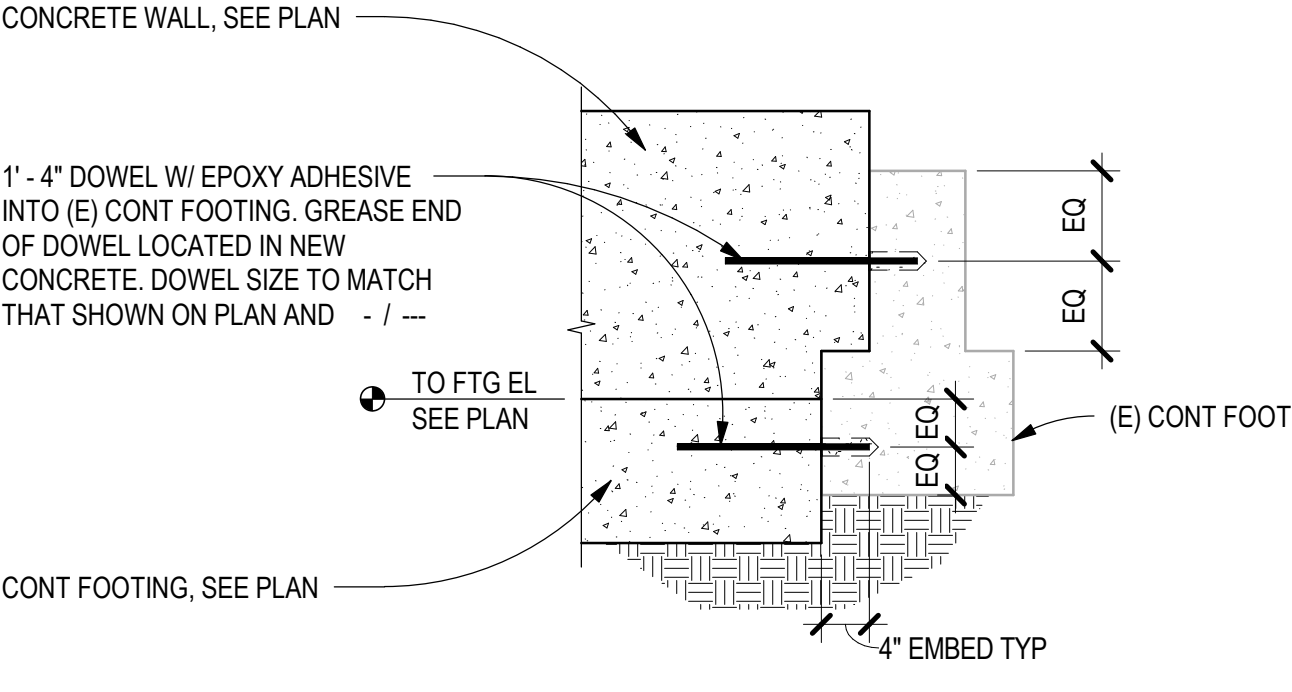
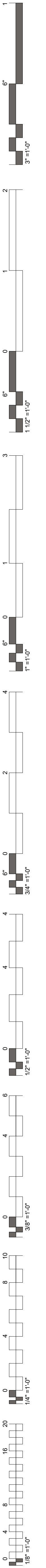


A4 WALL SECTION
SCALE: 3/8" = 1'-0"

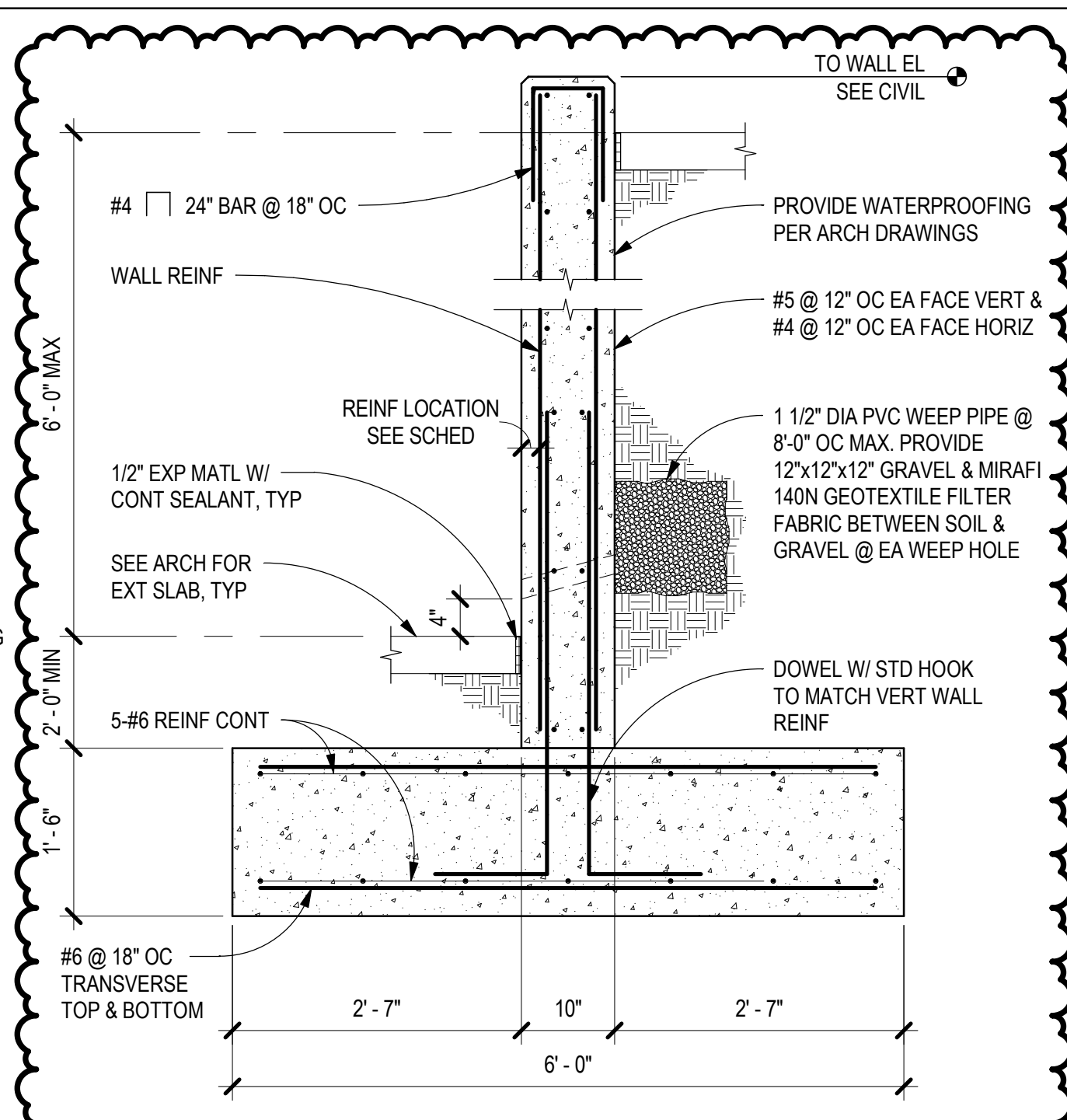


A5 WALL SECTION
SCALE: 3/8" = 1'-0"

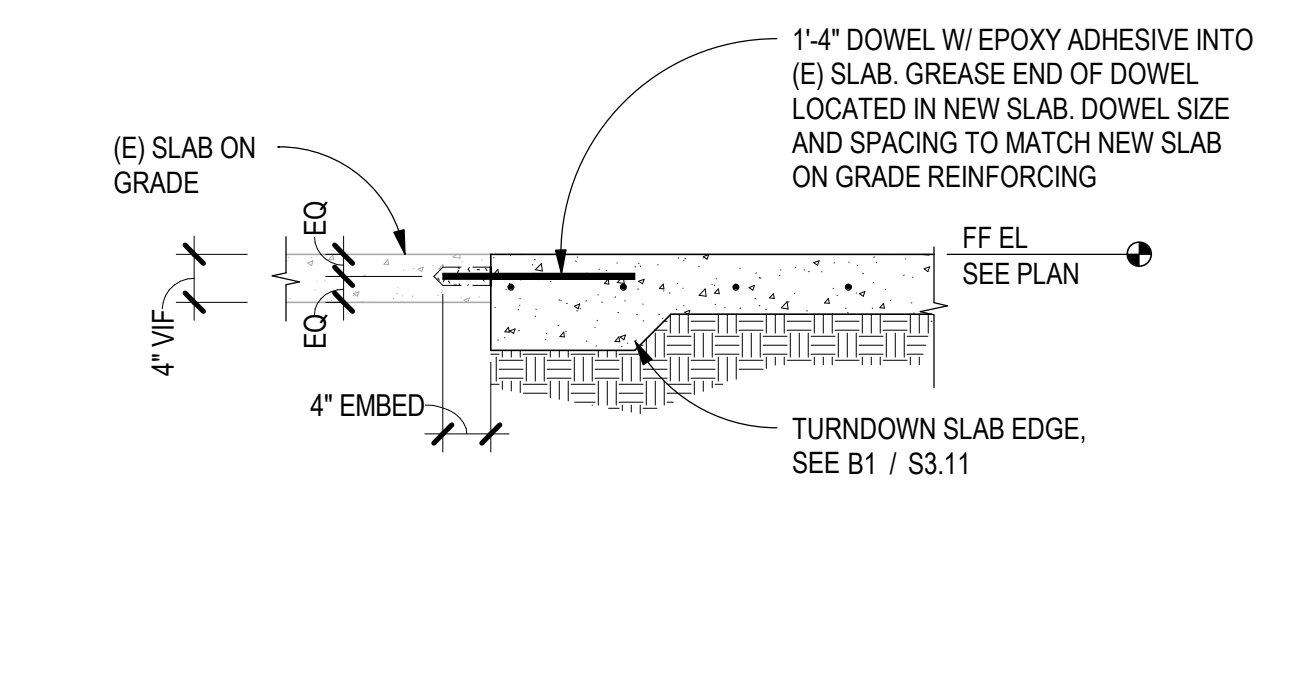
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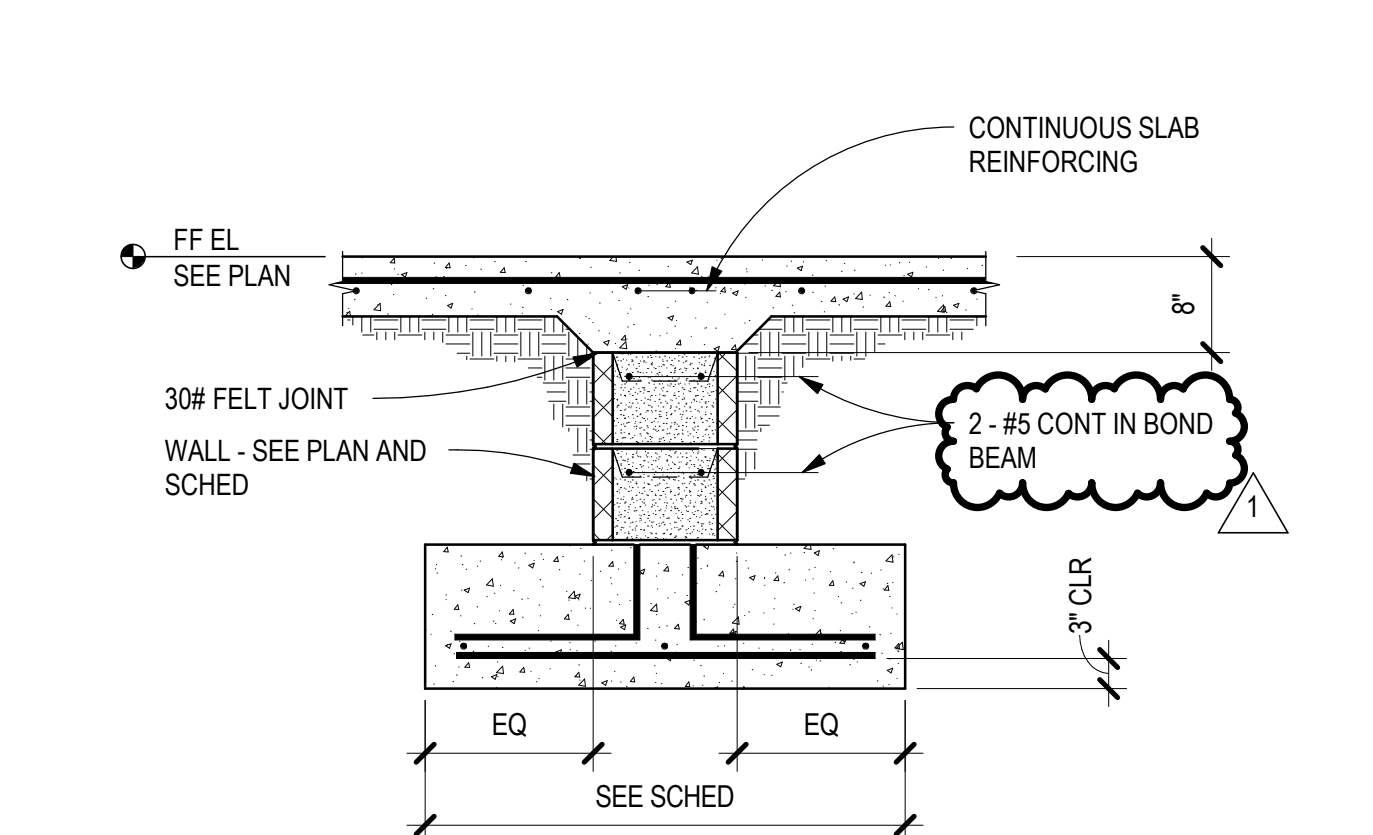
D1 DOWEL INTO (E) CONTINUOUS FOOTING
SCALE: 3/4" = 1'-0"



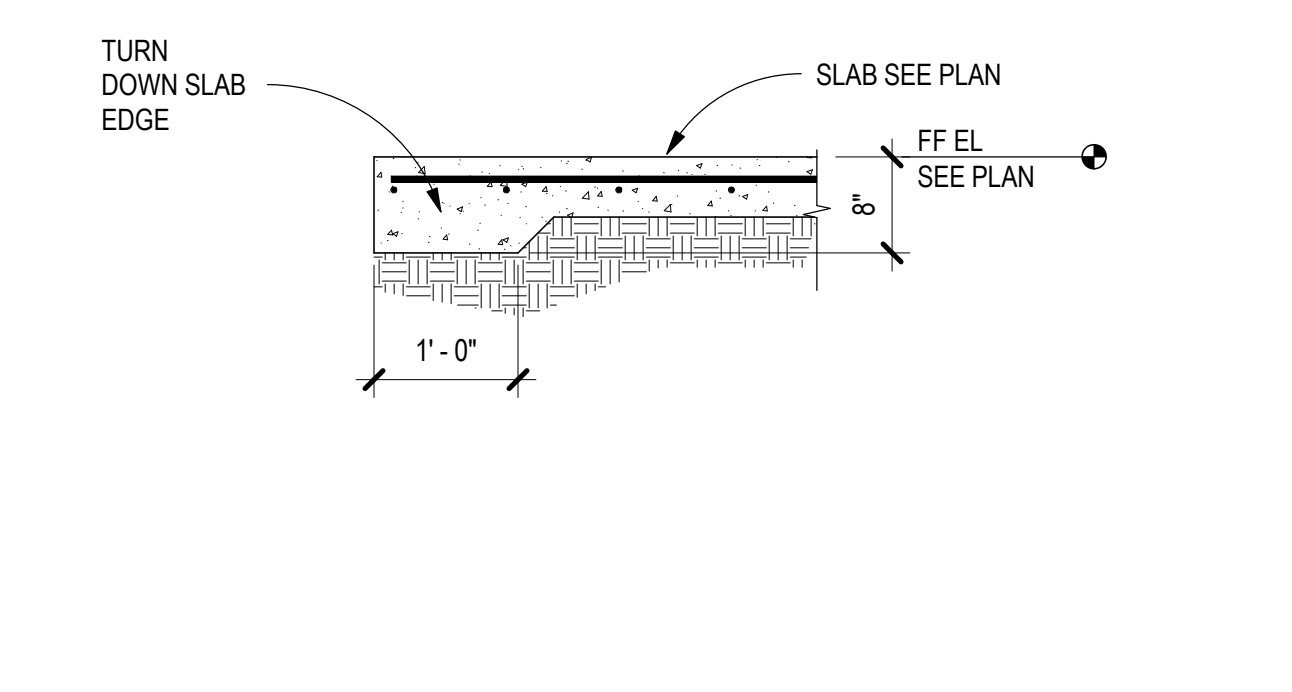
D2 RETAINING WALL SECTION
SCALE: 3/4" = 1'-0"



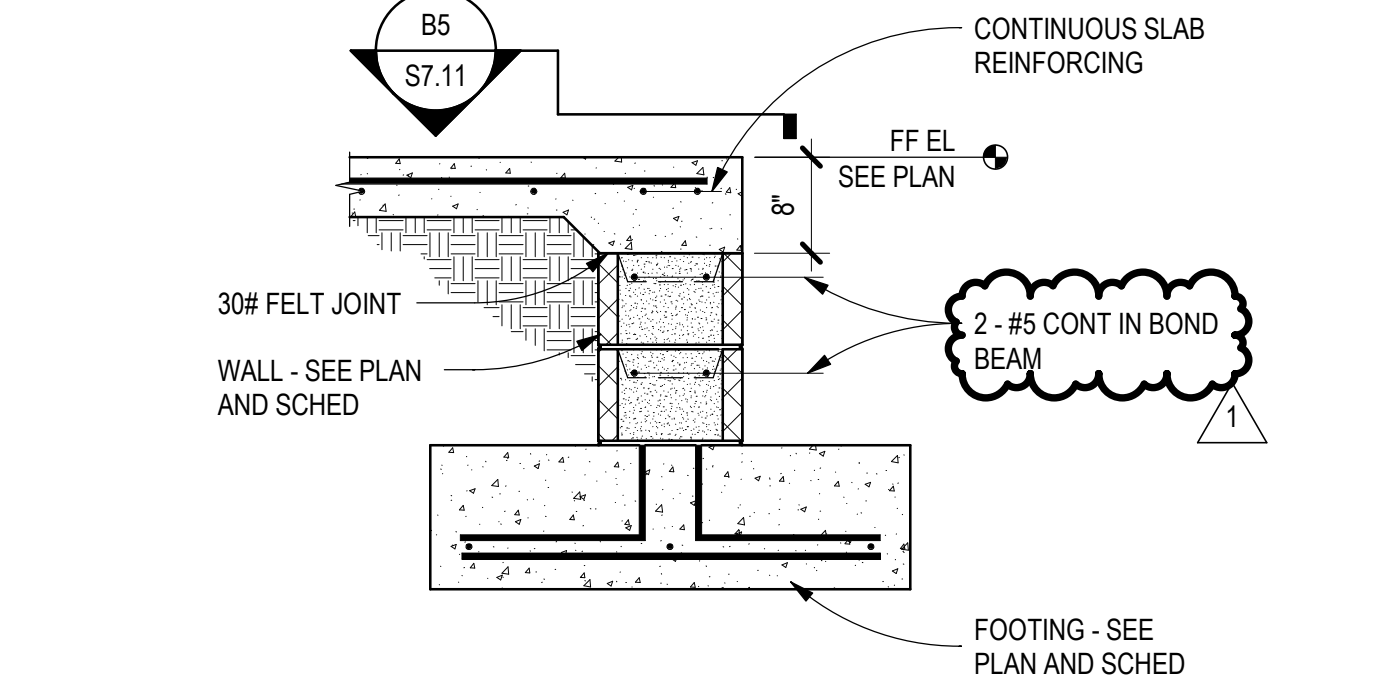
C1 DOWEL INTO (E) SLAB ON GRADE
SCALE: 3/4" = 1'-0"



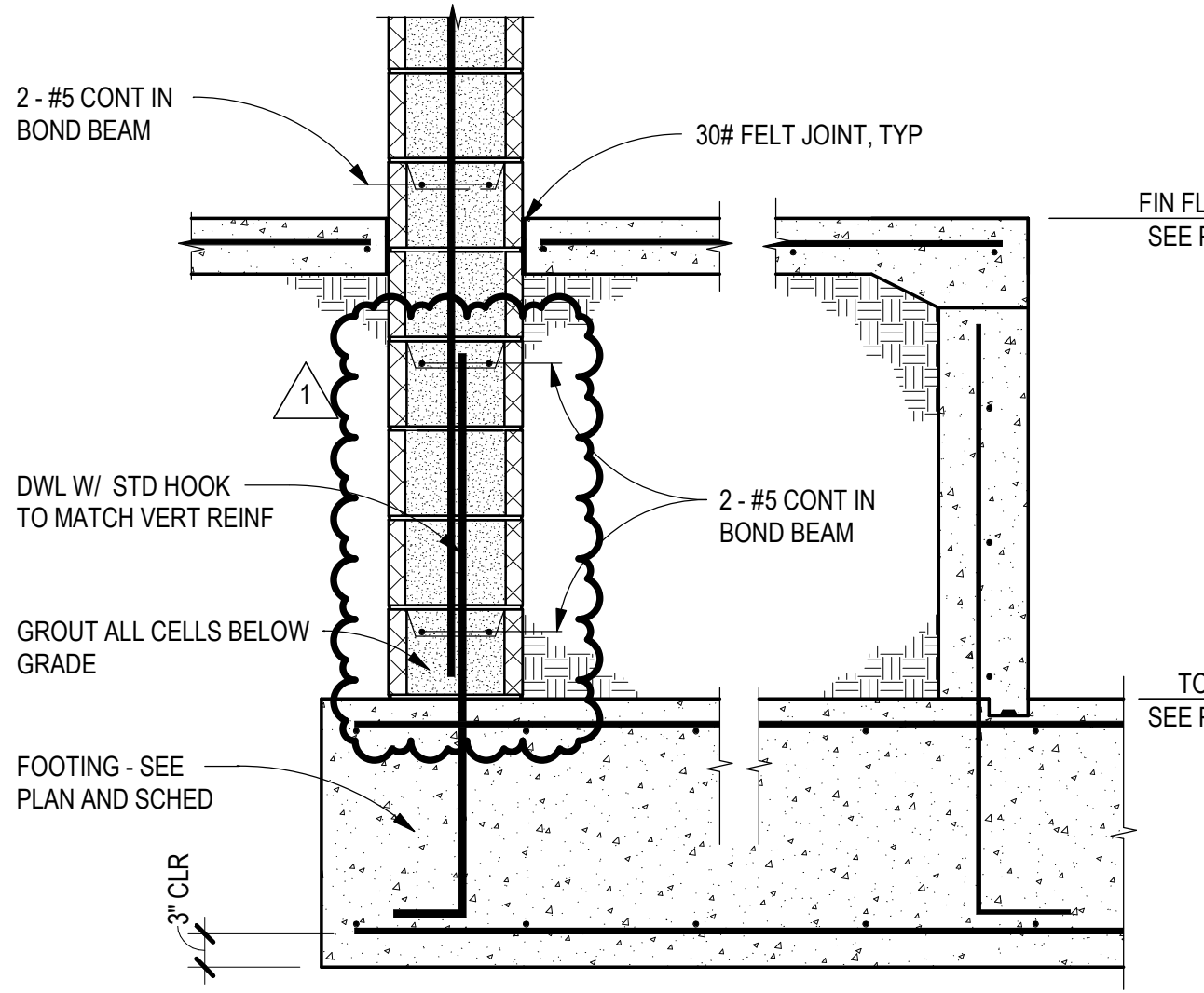
C2 INT SECTION @ DOORWAYS CMU
SCALE: 3/4" = 1'-0"



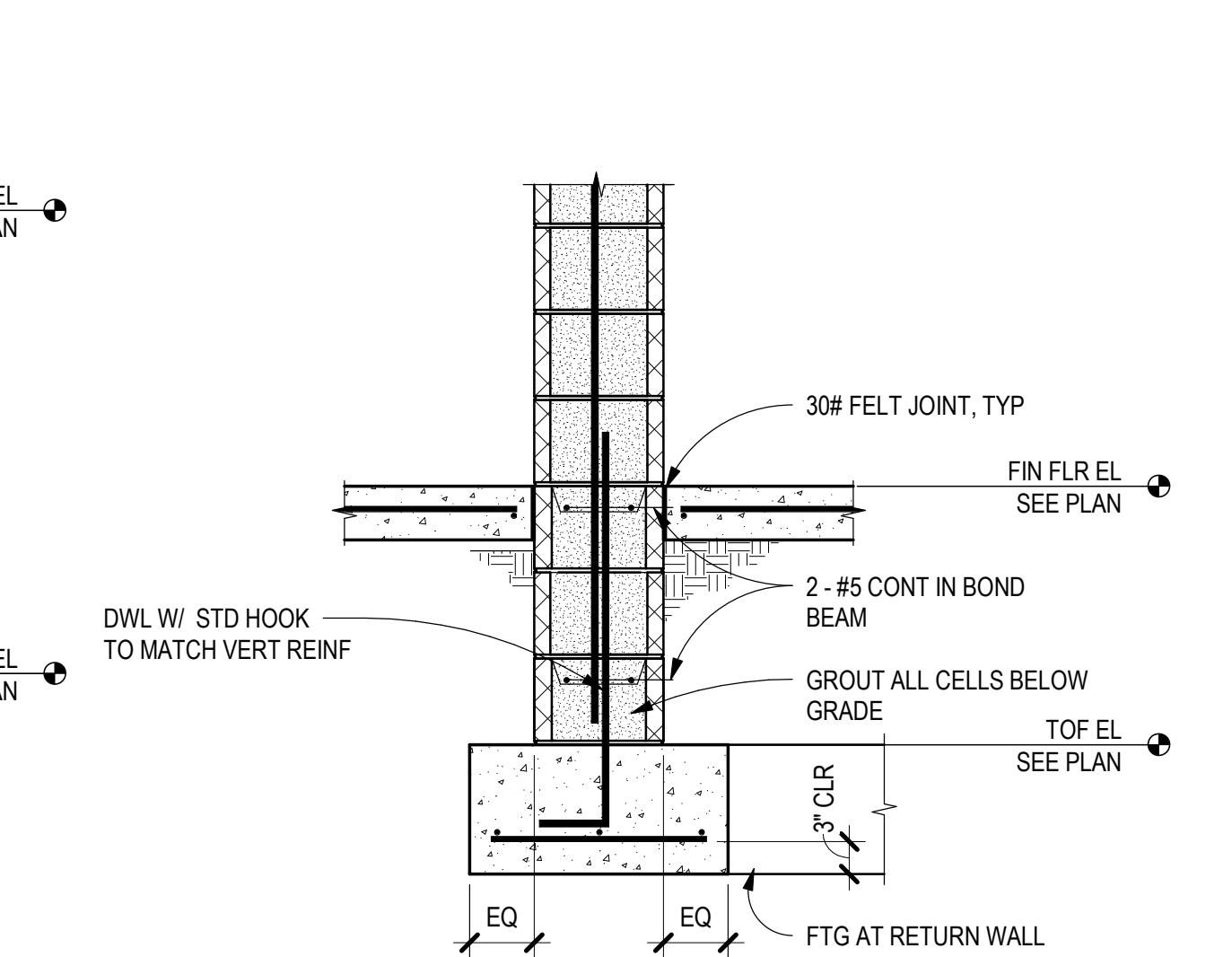
B1 TYP TURN DOWN SLAB EDGE
SCALE: 3/4" = 1'-0"



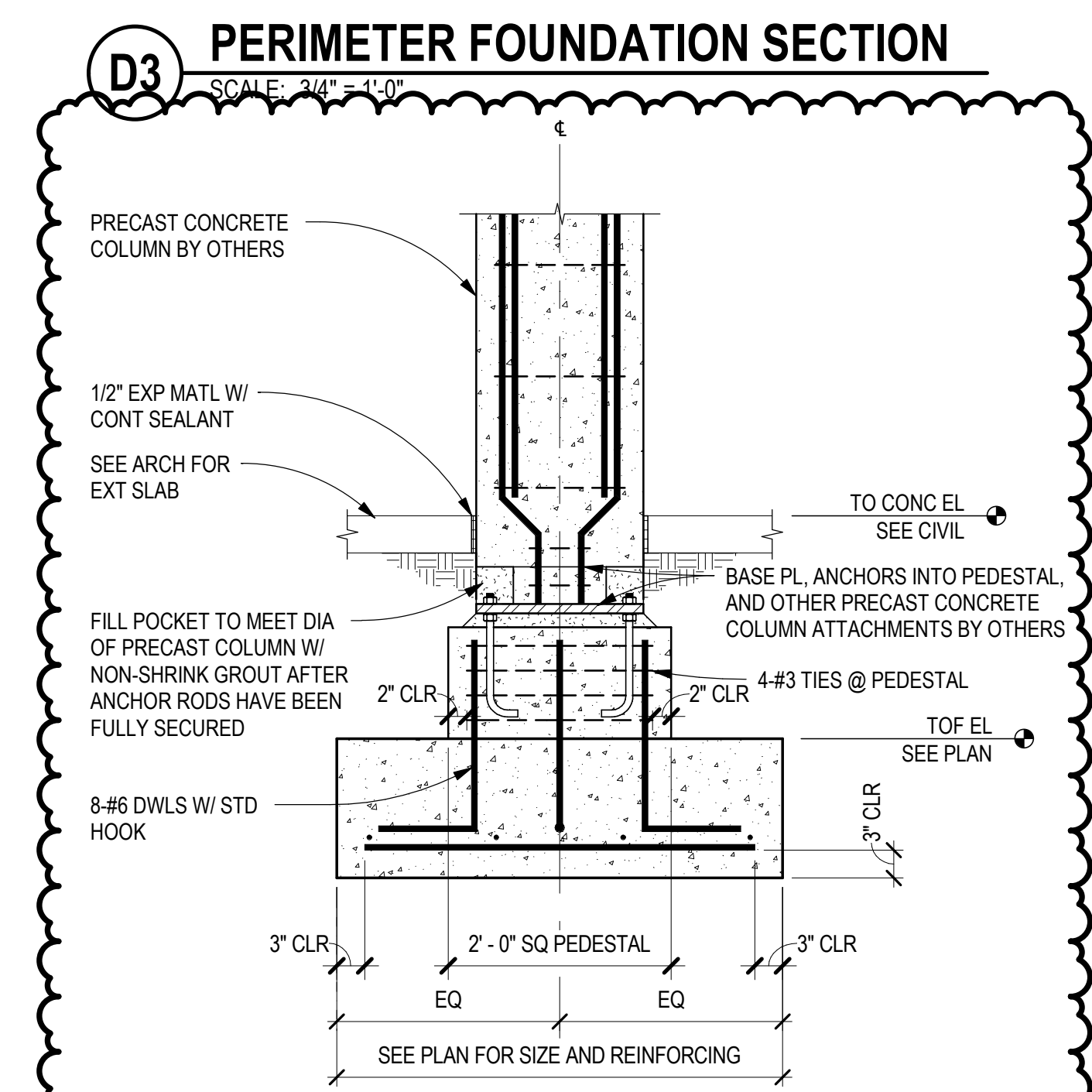
B2 PERIM SECTION AT DOORWAYS CMU
SCALE: 3/4" = 1'-0"



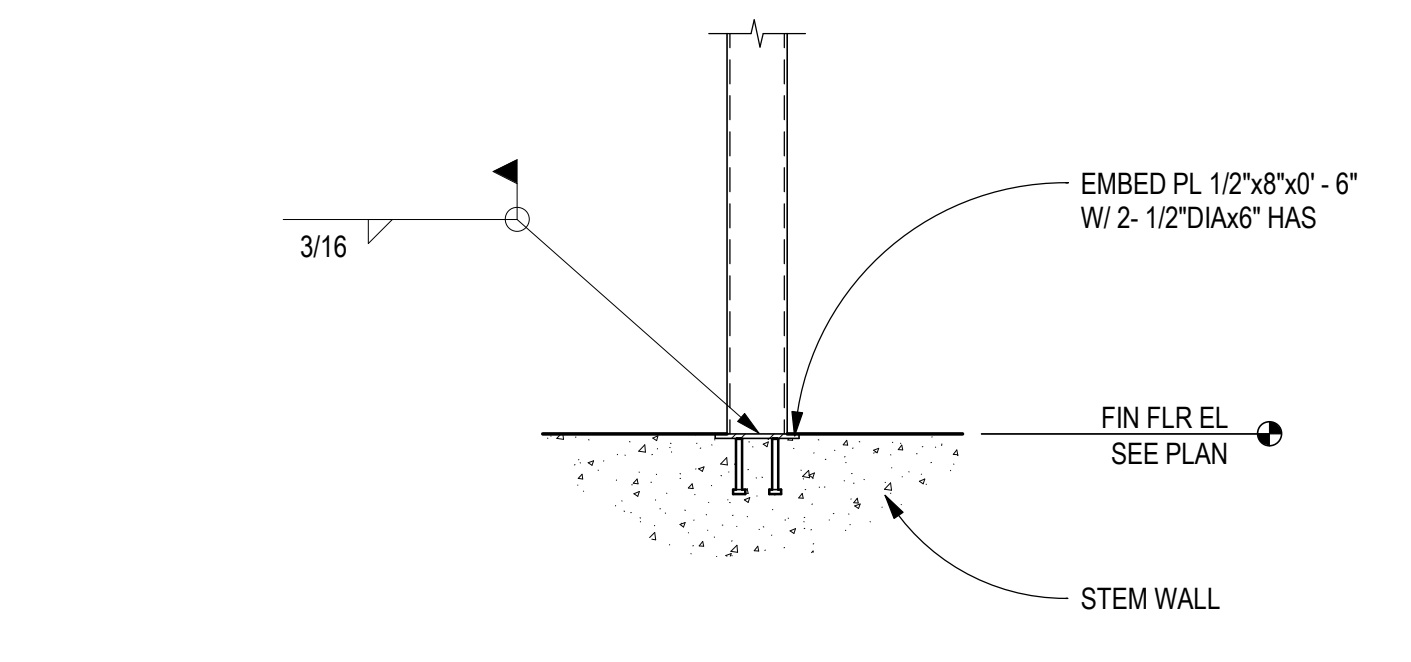
A1 FOUNDATION SECTION @ ELEVATOR
SCALE: 3/4" = 1'-0"



A2 PERIMETER FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



D3 PERIMETER FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



B3 HSS TO STEM WALL SECTION
SCALE: 3/4" = 1'-0"

CAST-IN-PLACE CONCRETE:
ALL CONCRETE SHALL CONFORM TO THE SPECIFICATIONS FOR STRUCTURAL CONCRETE, ACI 301-05. ALL EXPOSED EDGES OF CONCRETE SHALL HAVE A 3/4" CHAMFER UNLESS NOTED OTHERWISE. NORMAL WEIGHT CONCRETE: FC = 4000 PSI @ 28 DAYS

REINFORCING STEEL:
ALL REINFORCING STEEL SHALL BE FABRICATED AND PLACED IN ACCORDANCE WITH THE BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI 318-08), AND DETAILS AND DETAILING OF CONCRETE REINFORCEMENT (ACI 315-99). ALL REINFORCING STEEL SHALL CONFORM TO ASTM A615 GRADE 60; EXCEPT STIRRUPS, TIES AND INDICATED FIELD-BENT BARS, WHICH SHALL CONFORM TO ASTM A615 GRADE 40. REINFORCING SHALL NOT BE TACK WELDED OR WELDED IN ANY MANNER UNLESS SPECIFICALLY DETAILED ON THE STRUCTURAL PLANS.

STRUCTURAL AND MISCELLANEOUS STEEL
ANCHOR BOLTS EMBEDDED IN CONCRETE SHALL BE ASTM F1554 GRADE 36 THREADED RODS WITH DOUBLE NUTS. PROVIDE FLAT WASHERS BETWEEN NUTS AND BASEPLATE SURFACES. ANCHOR BOLT LENGTHS SHOWN FOR ATTACHMENT TO CONCRETE ARE REQUIRED EMBEDMENT LENGTHS. THE CONTRACTOR SHALL PROVIDE ANCHOR BOLTS WITH ADDITIONAL BOLT LENGTH TO FACILITATE THE REQUIRED CONNECTION. ANCHOR BOLT FLAT WASHERS SHALL BE PROVIDED IN ACCORDANCE WITH TABLE 14-2 OF AISC 390, AISC MANUAL OF STEEL CONSTRUCTION THIRTEEN EDITION.

LIGHT POLE BY OTHERS 20'-0" MAX HEIGHT ABOVE FINISHED GRADE
BASEPLATE AND ANCHORS BY LIGHT POLE MFR

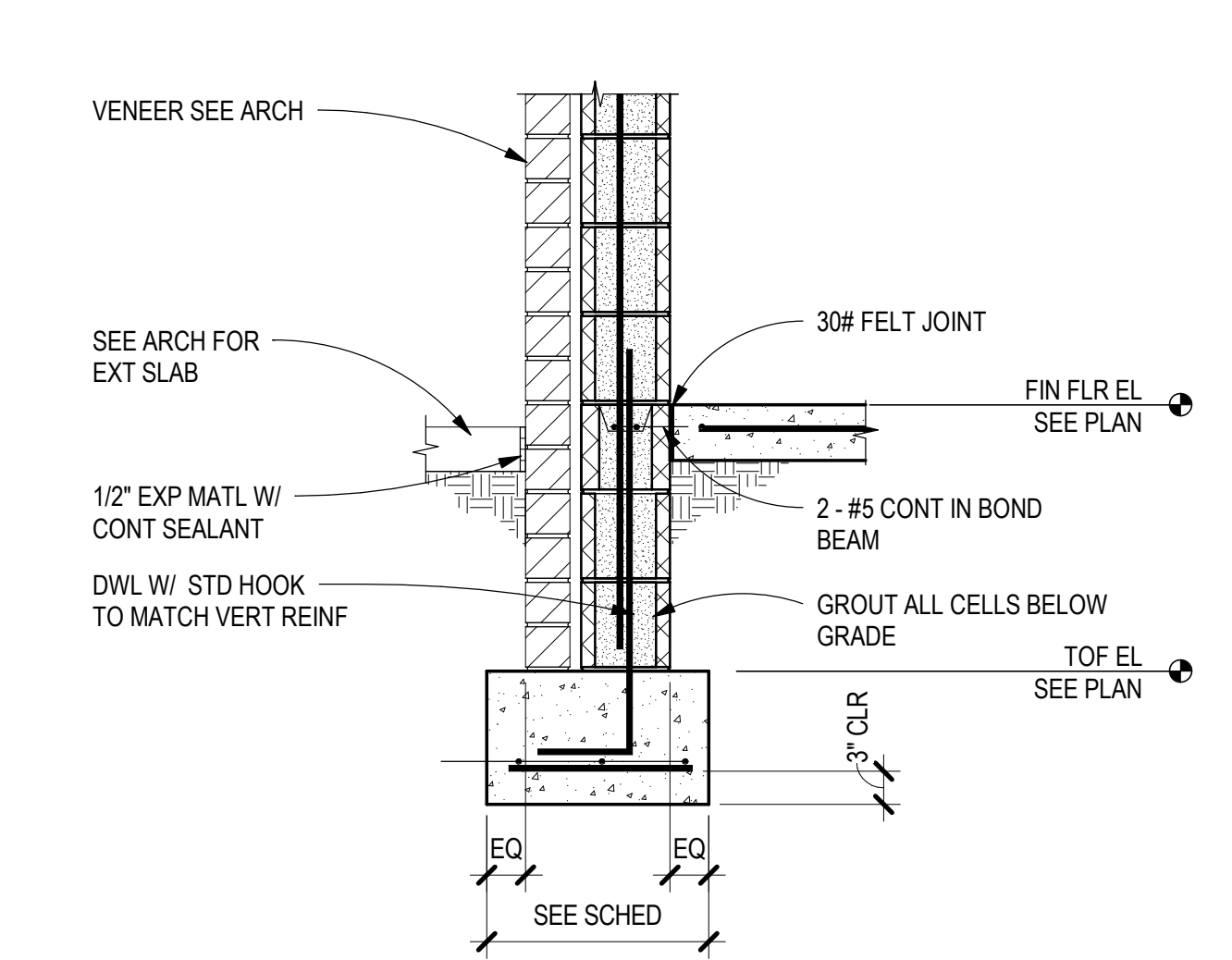
3/4" CHAMFER
3-#3 TIES WITHIN TOP 5"
1/2" EXP MATL W/ CONT SEALANT TYP

2'-0" CONC PEDESTAL REINF W/ 6-#6 VERT & #3 TIES @ 10" OC

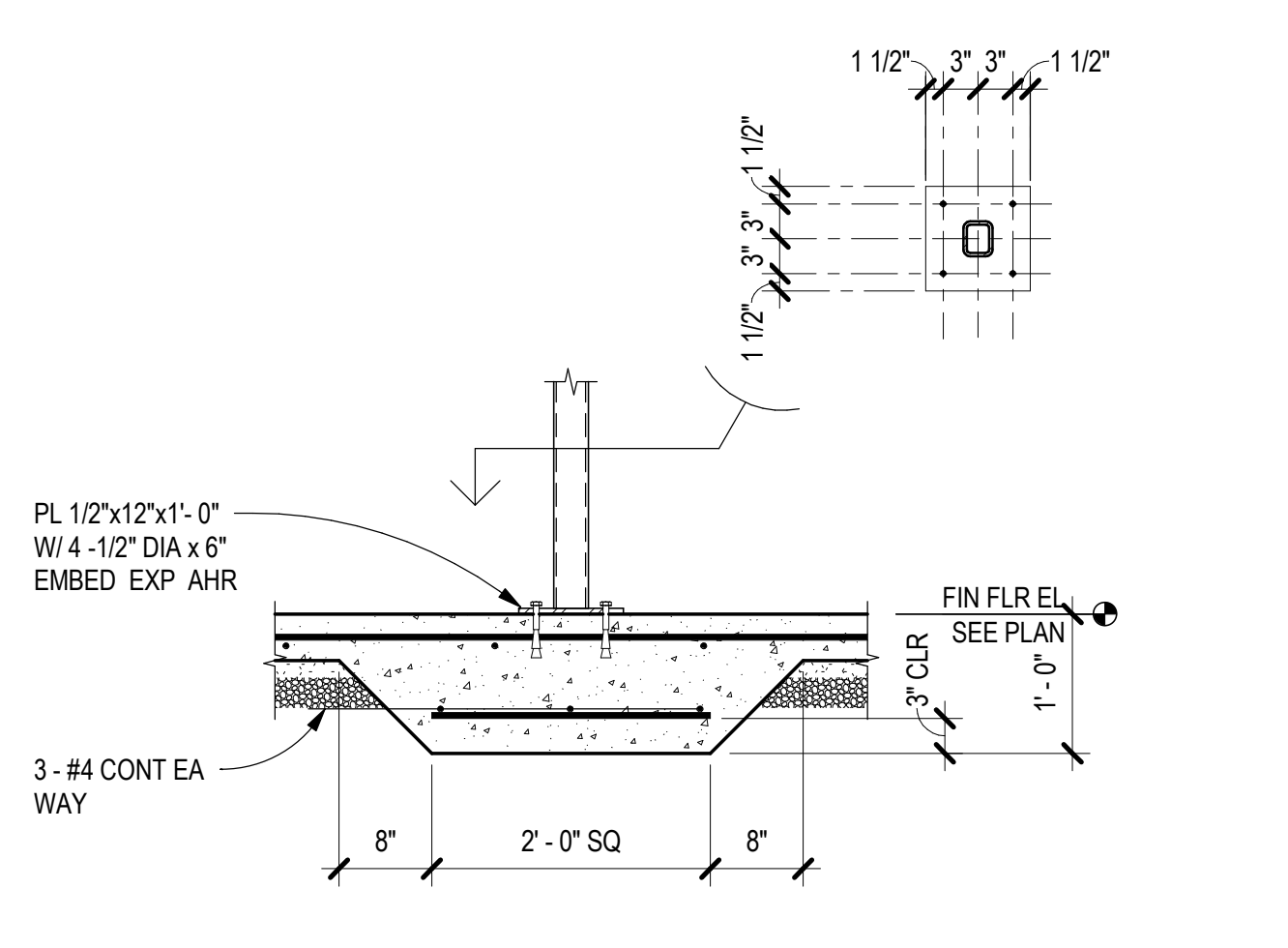
3" CLR TYP

NOTE: REFER TO ARCHIELECTRICAL DRAWINGS FOR ADDL INFORMATION

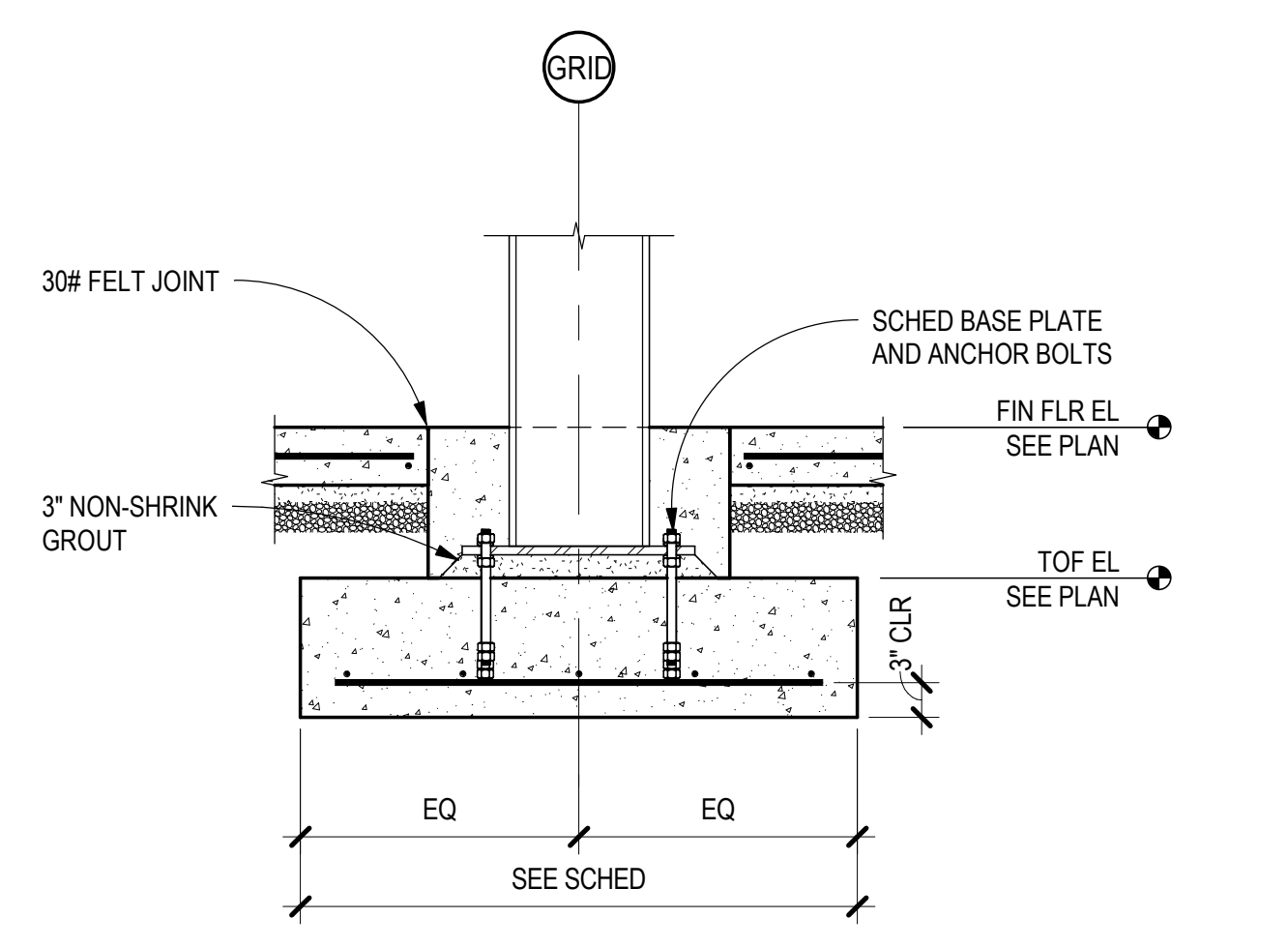
A3 LIGHT POLE BASE
SCALE: 1/2" = 1'-0"



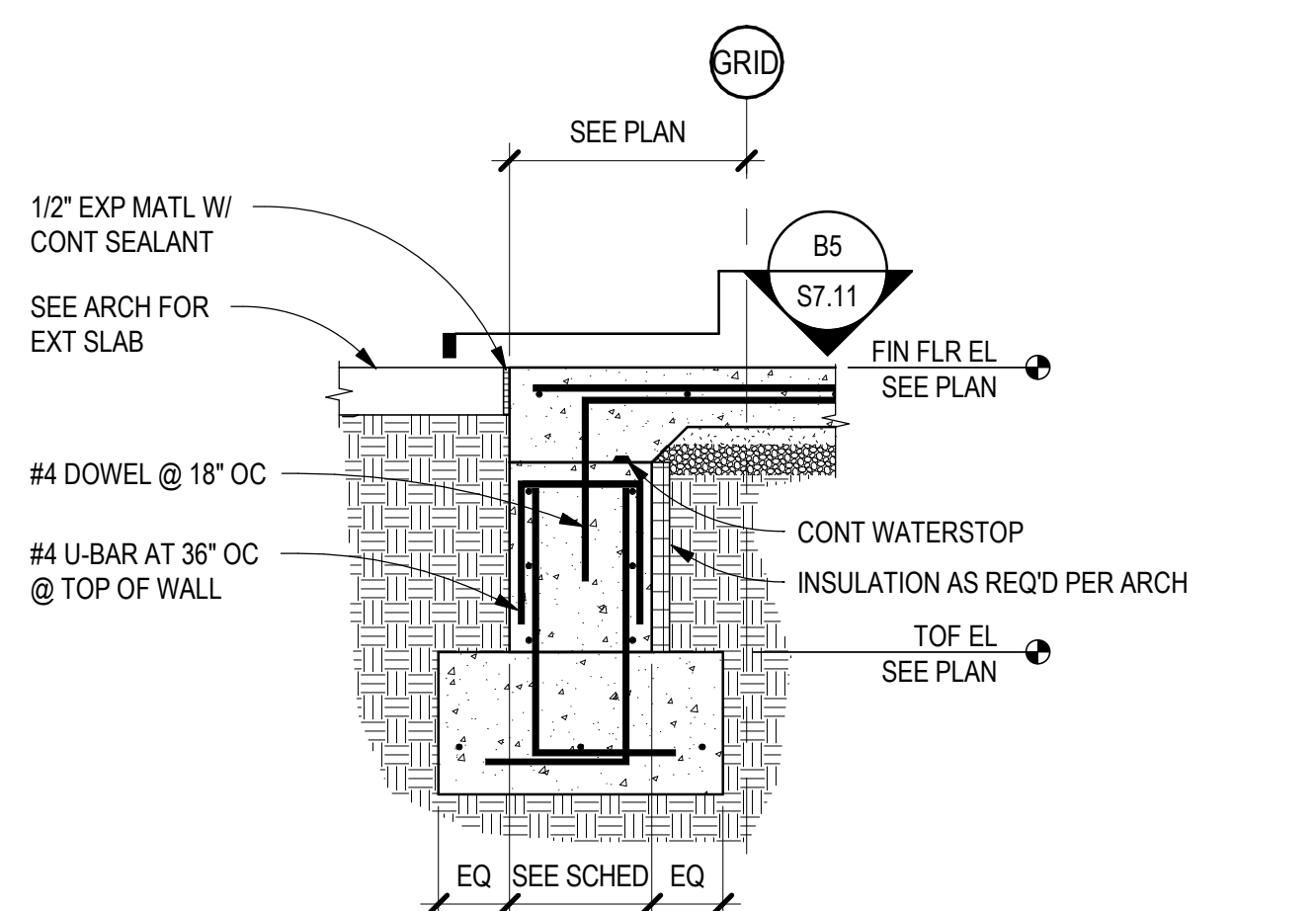
D4 PERIMETER FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



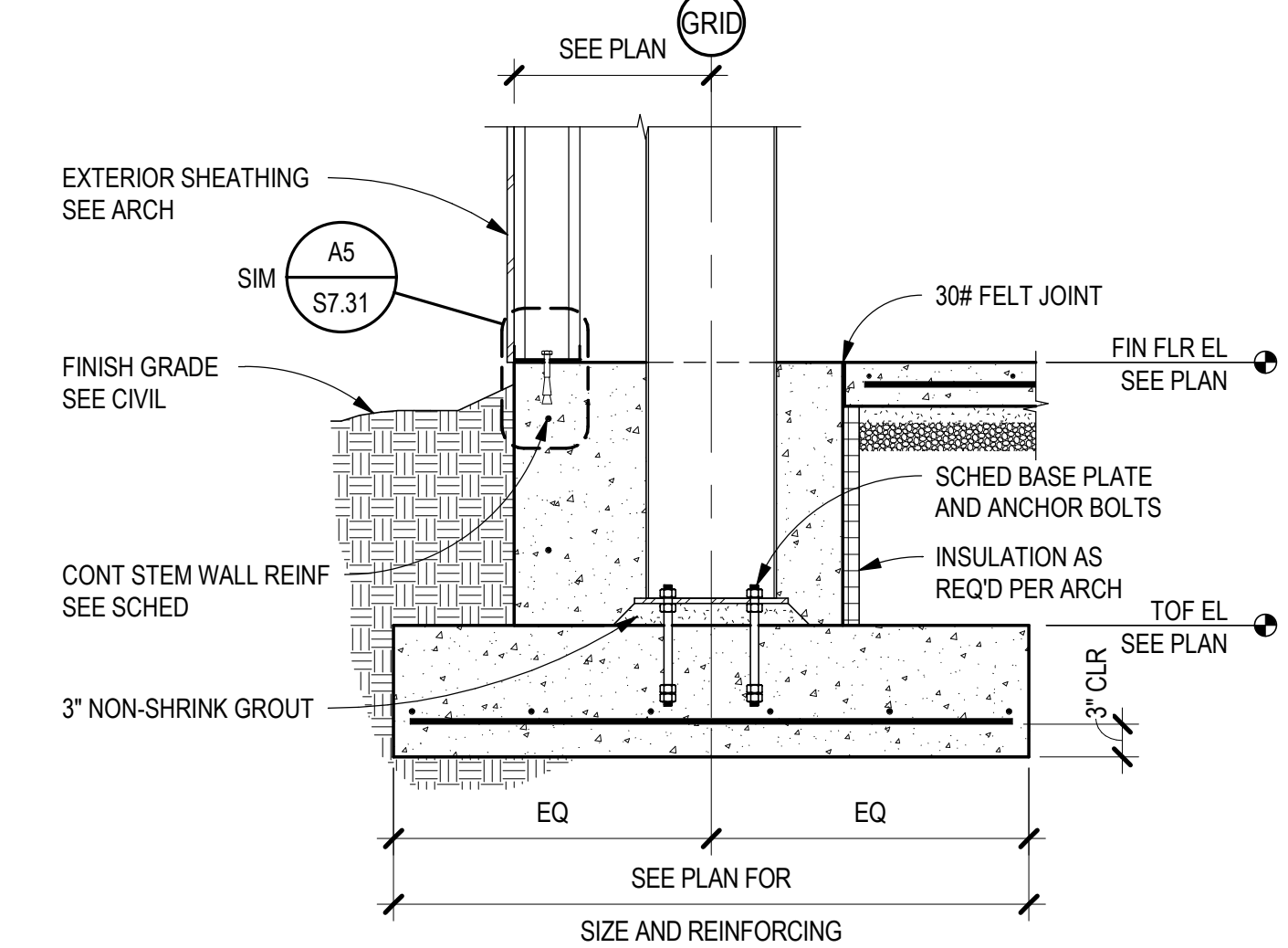
C4 INT COLUMN AT THICKENED SLAB
SCALE: 3/4" = 1'-0"



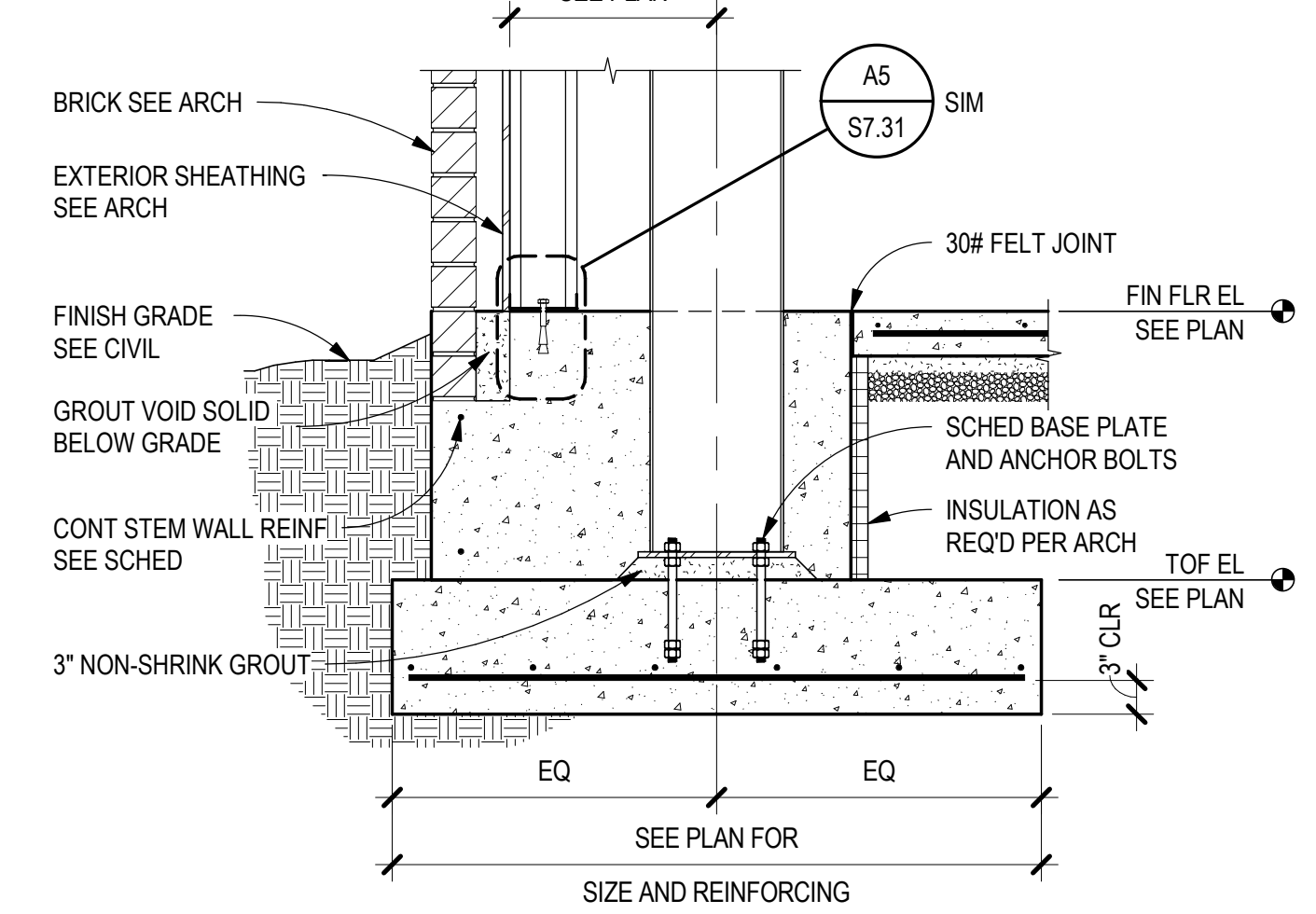
B4 INTERIOR COLUMN SECTION
SCALE: 3/4" = 1'-0"



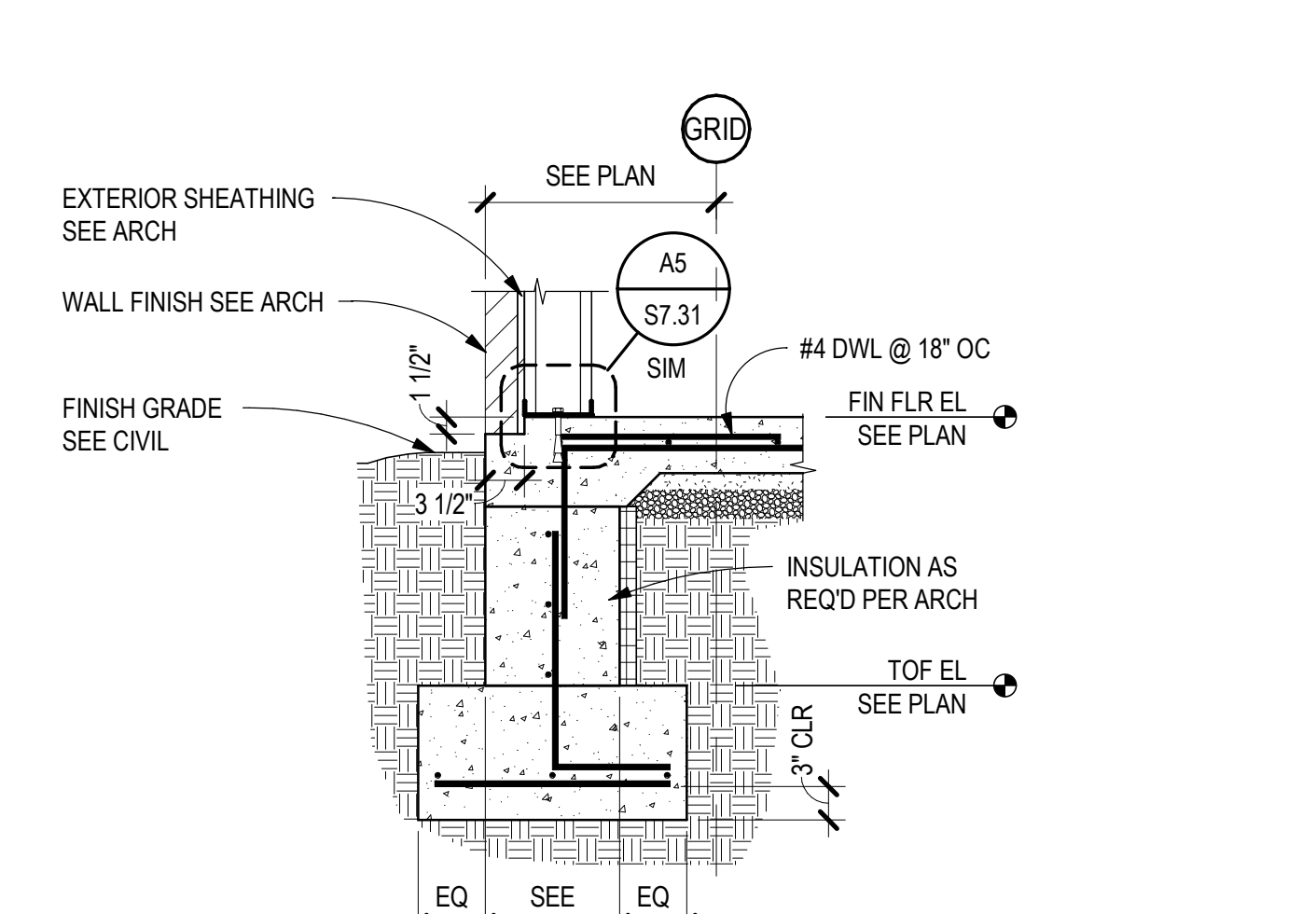
A4 FOUNDATION SECTION AT DOORWAY
SCALE: 3/4" = 1'-0"



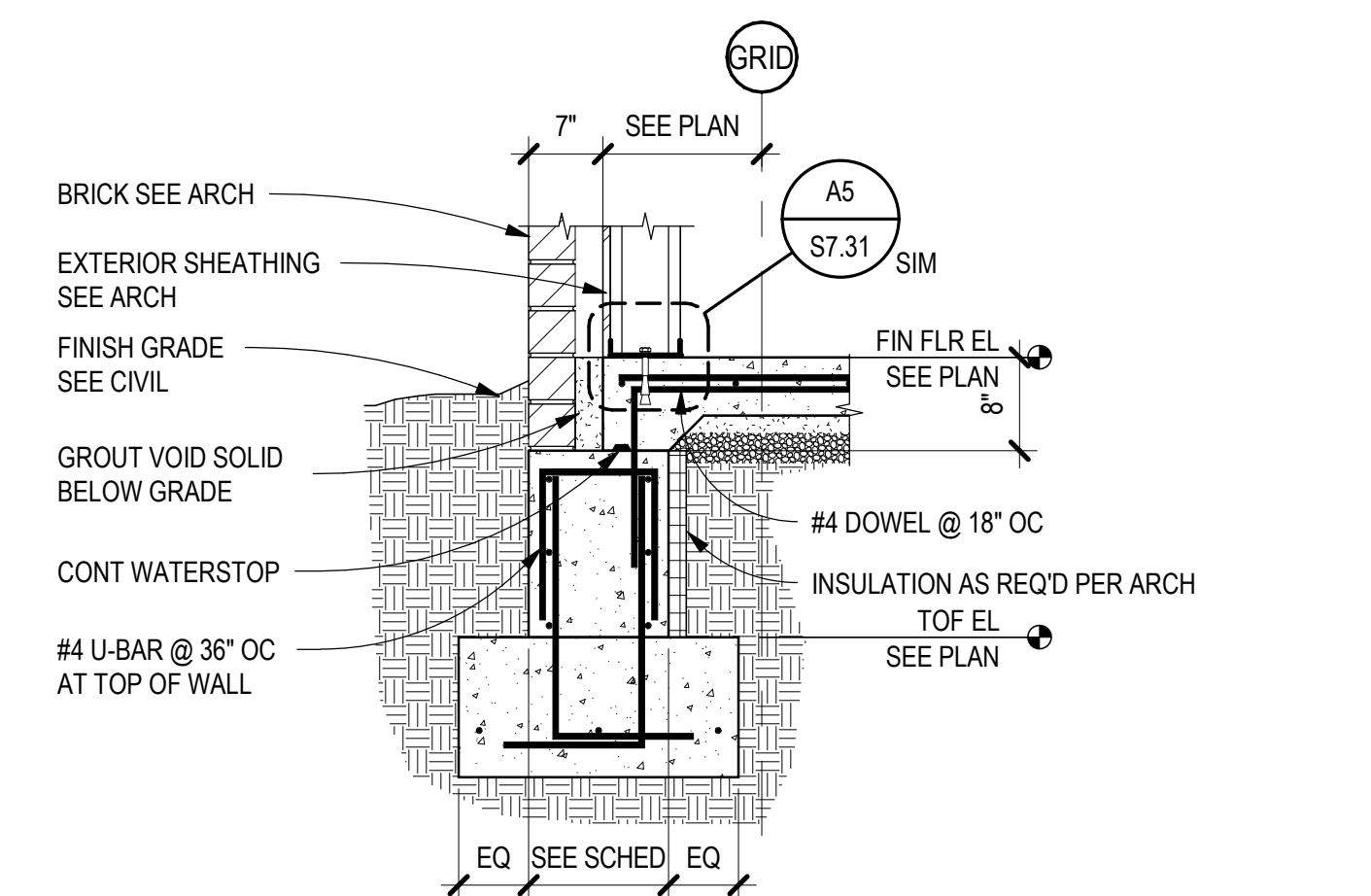
D5 FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



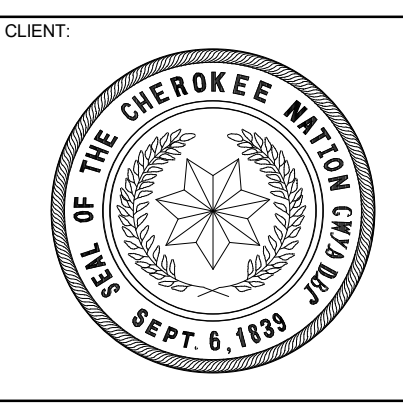
C5 FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



B5 FOUNDATION SECTION NO VENEER
SCALE: 3/4" = 1'-0"



A5 TYPICAL FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



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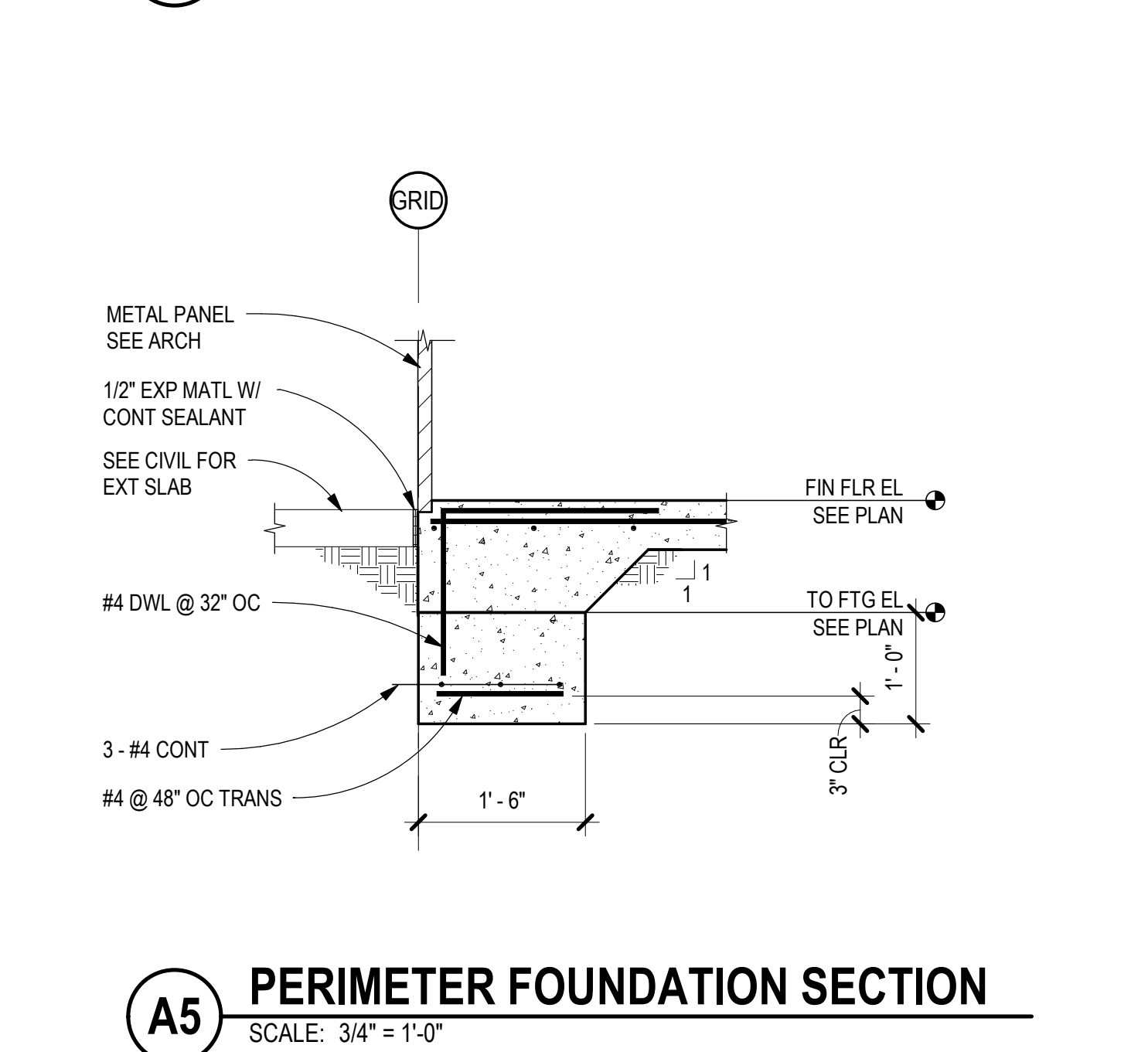
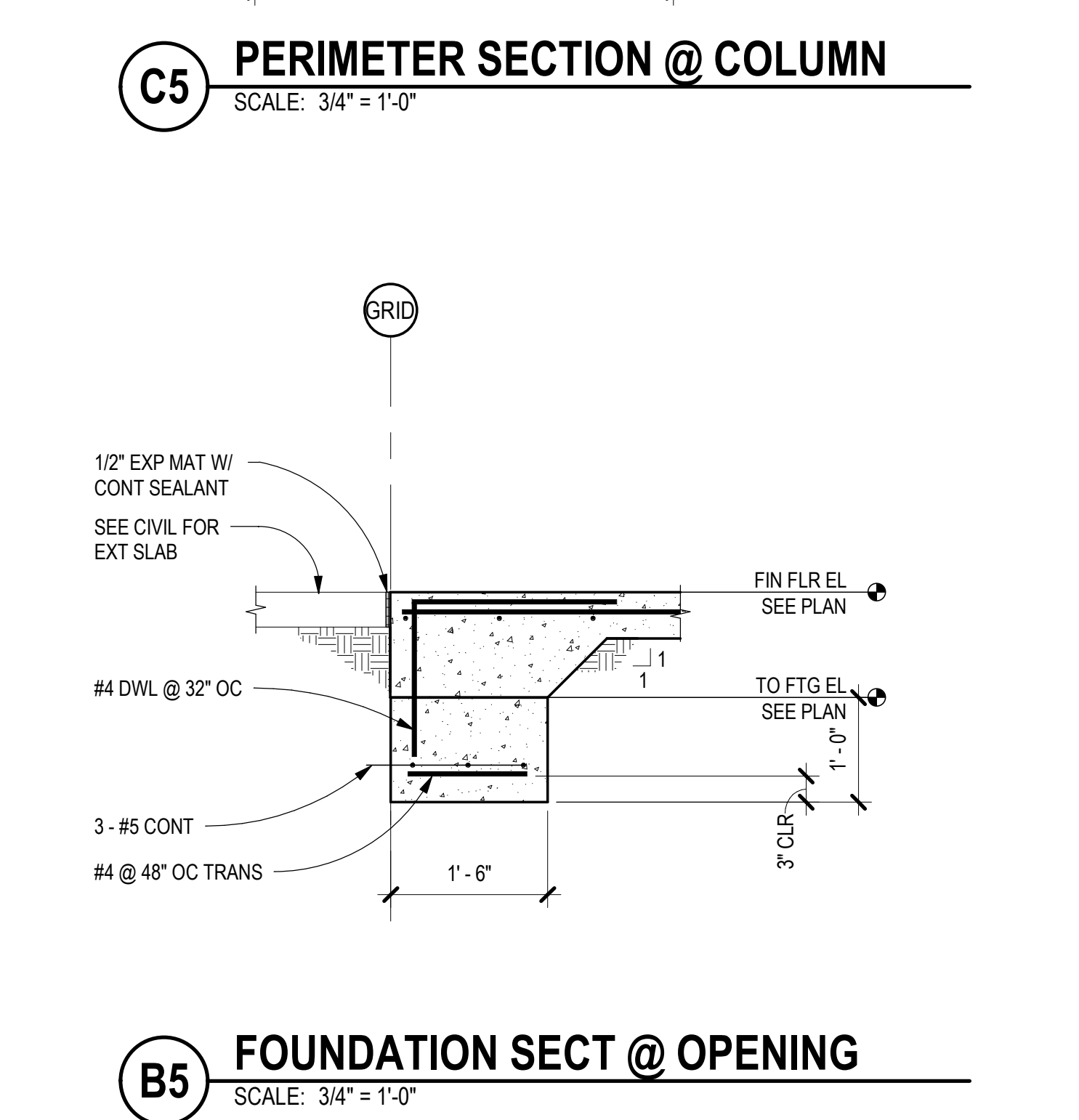
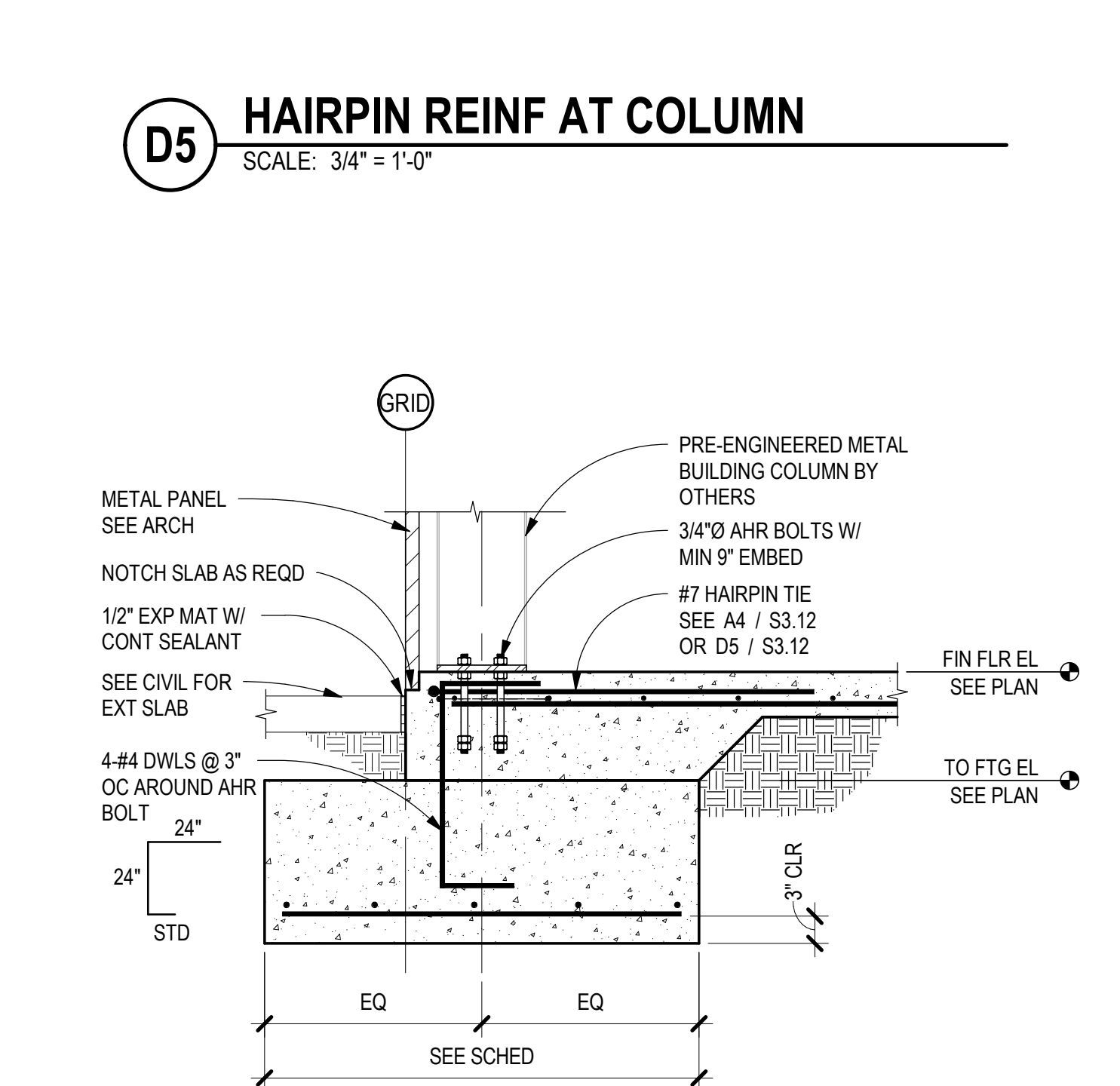
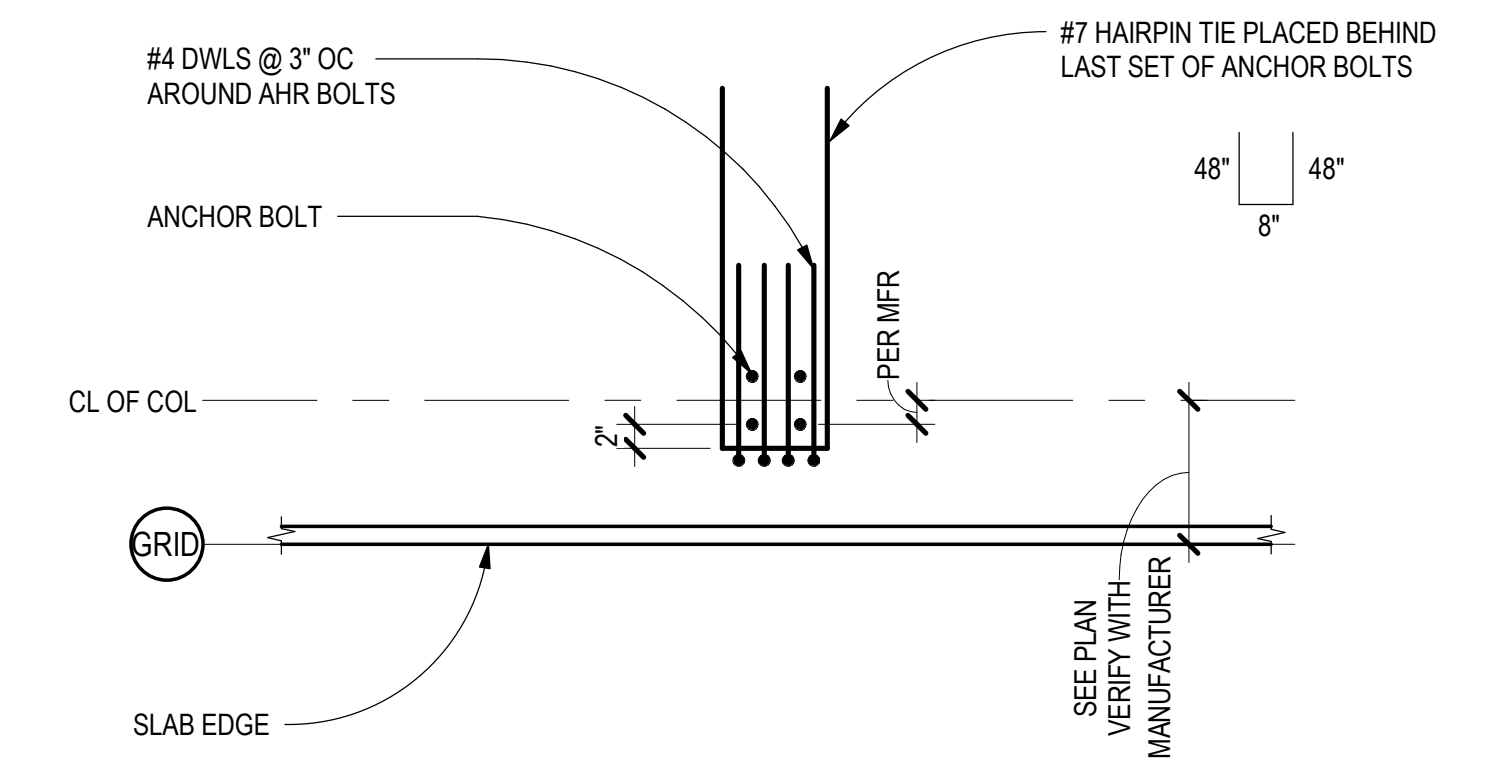
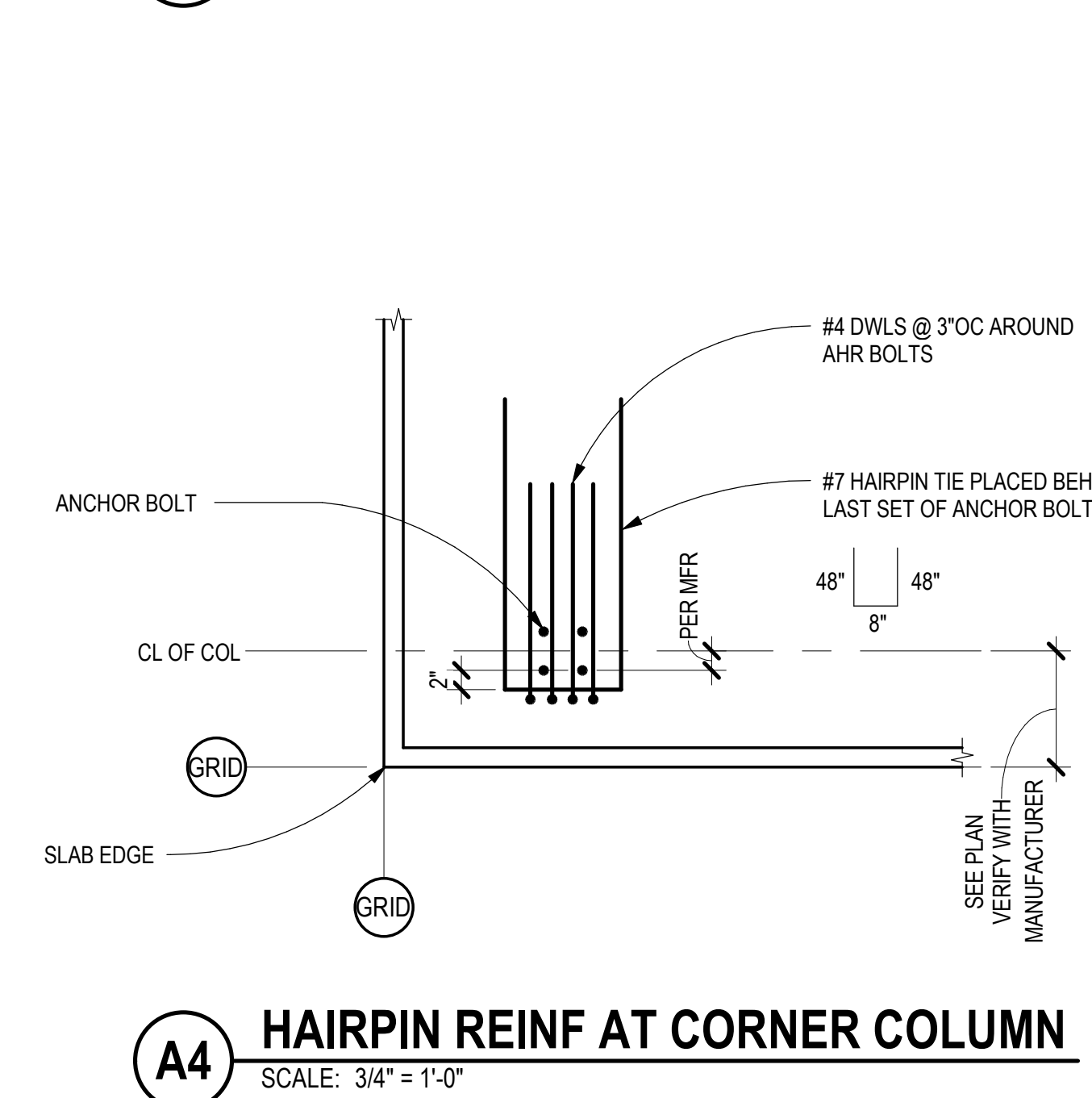
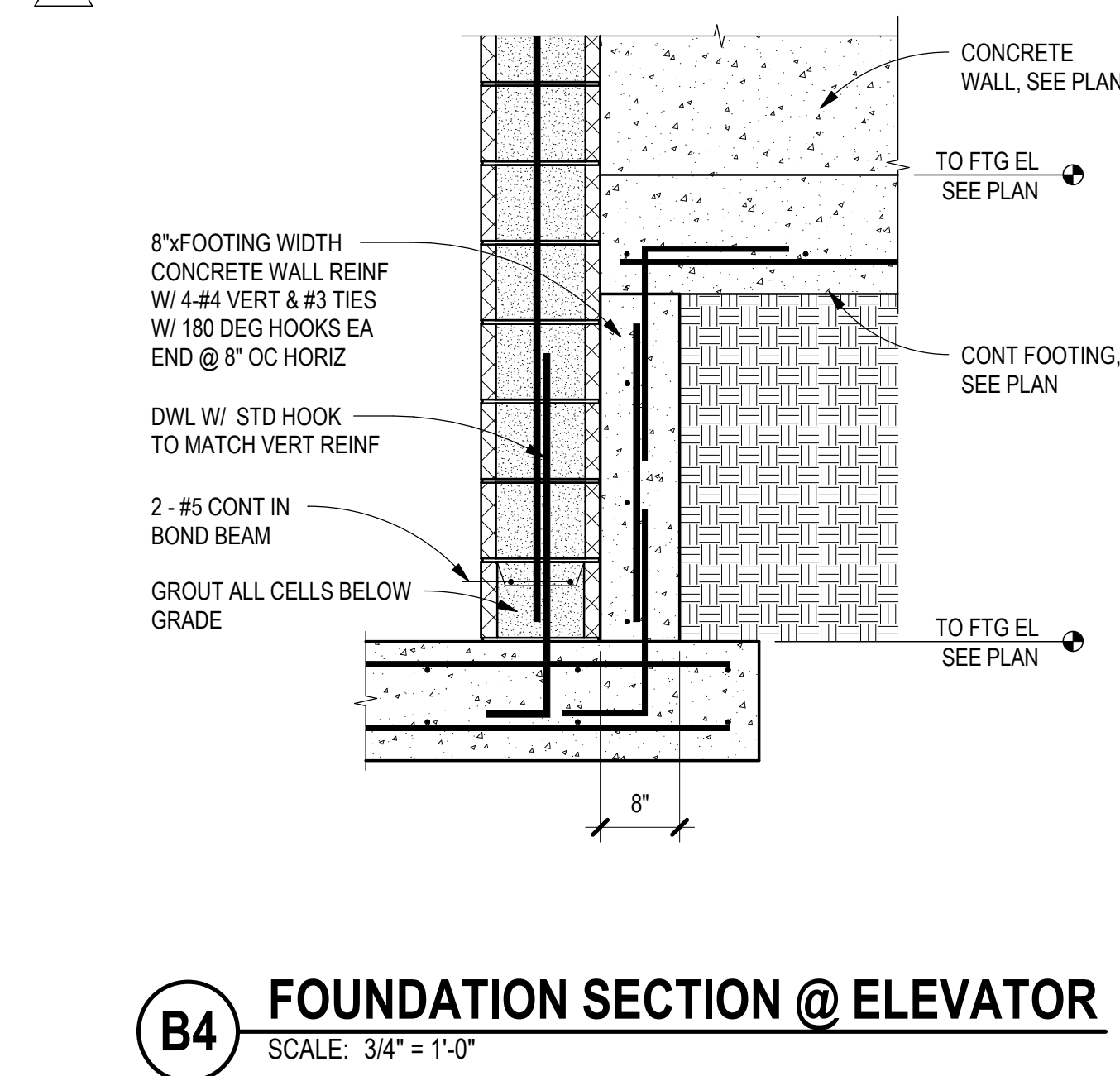
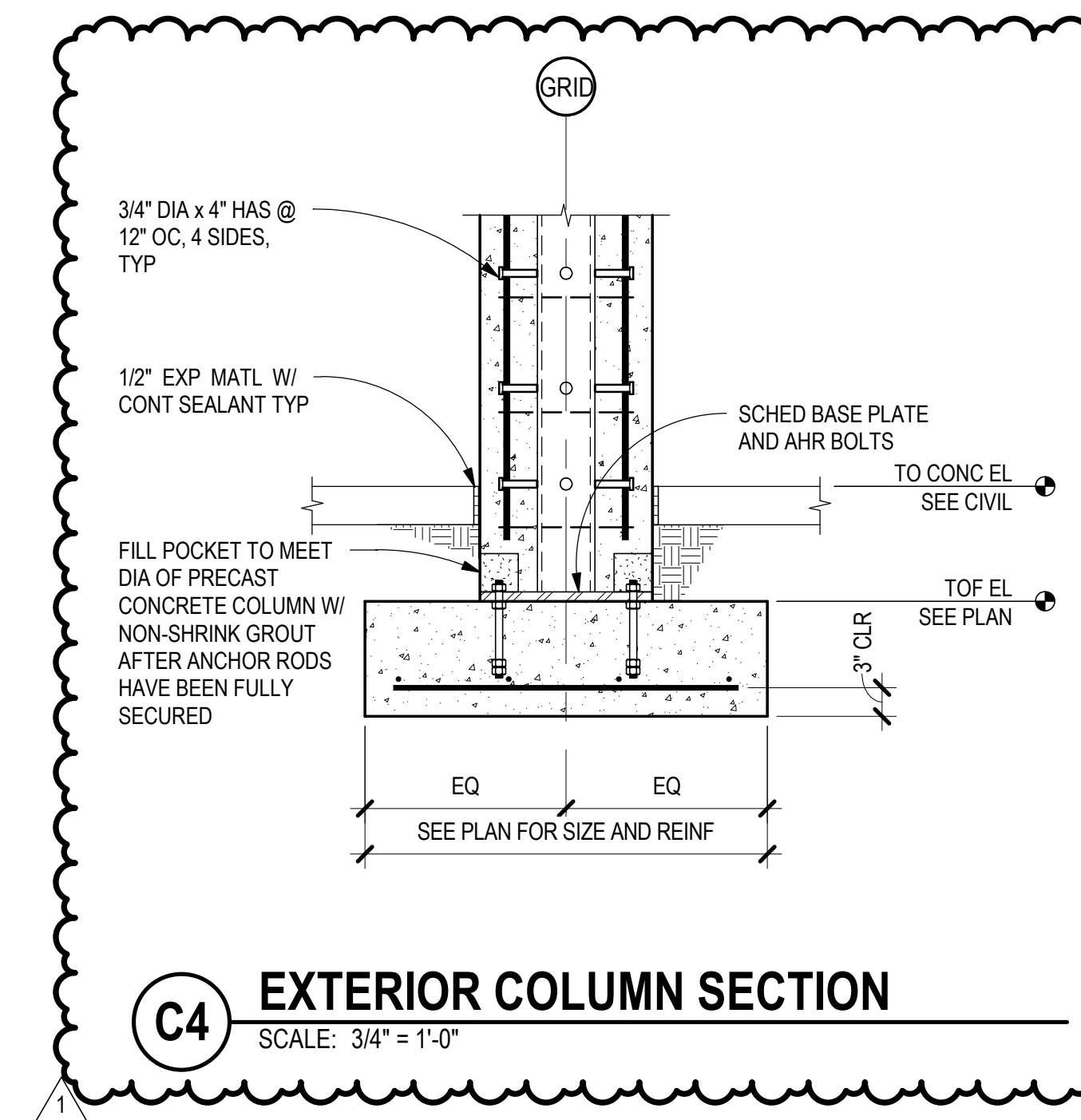
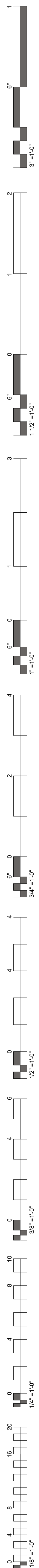
KEY PLAN:

PROJECT PHASE:
BID PACKAGE 01

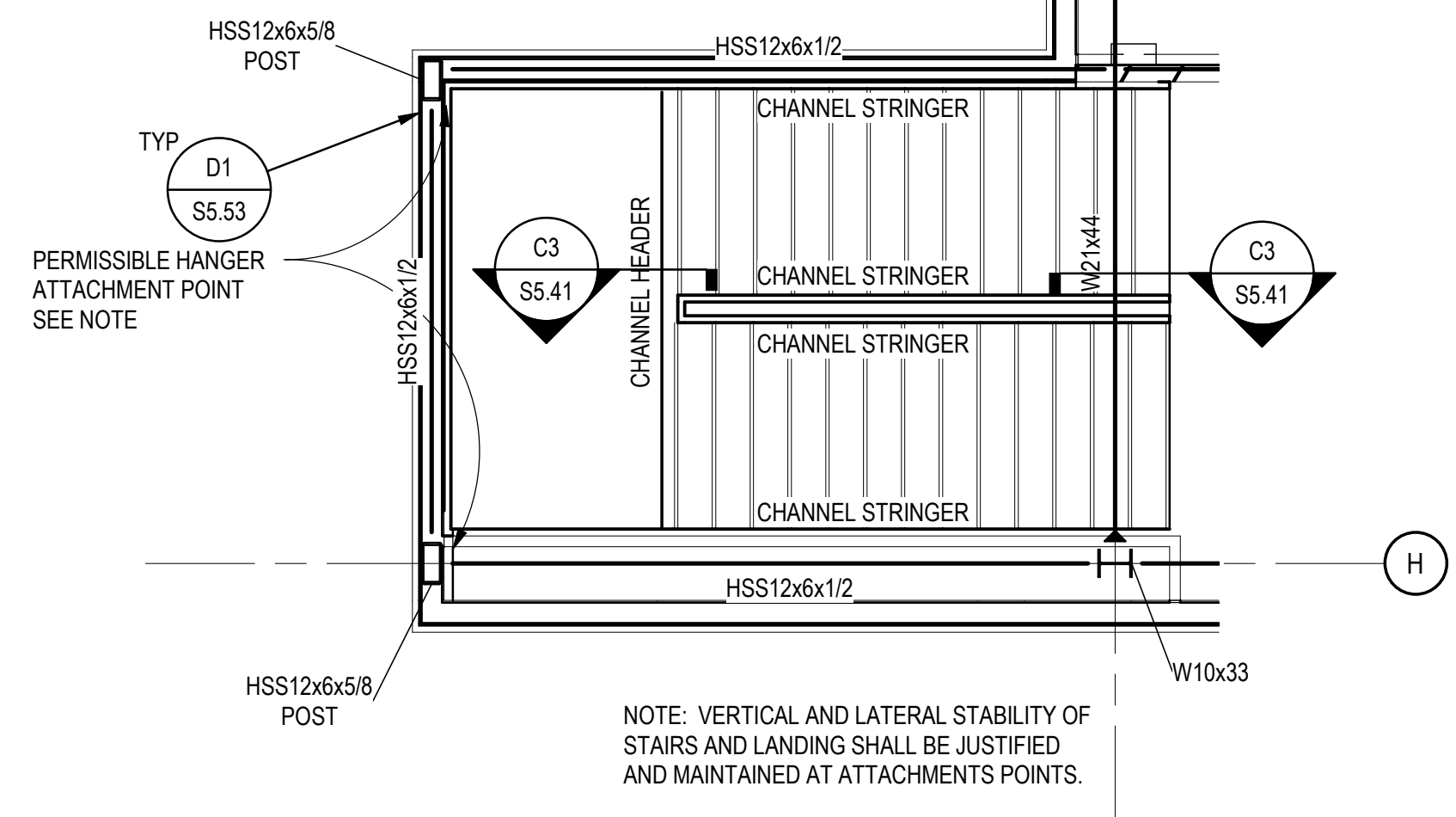
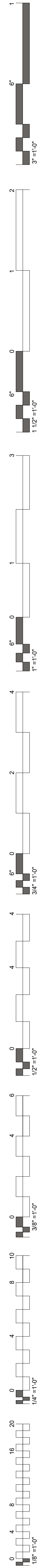
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DATE: 11-01-19
JOB NUMBER: 18-01.01

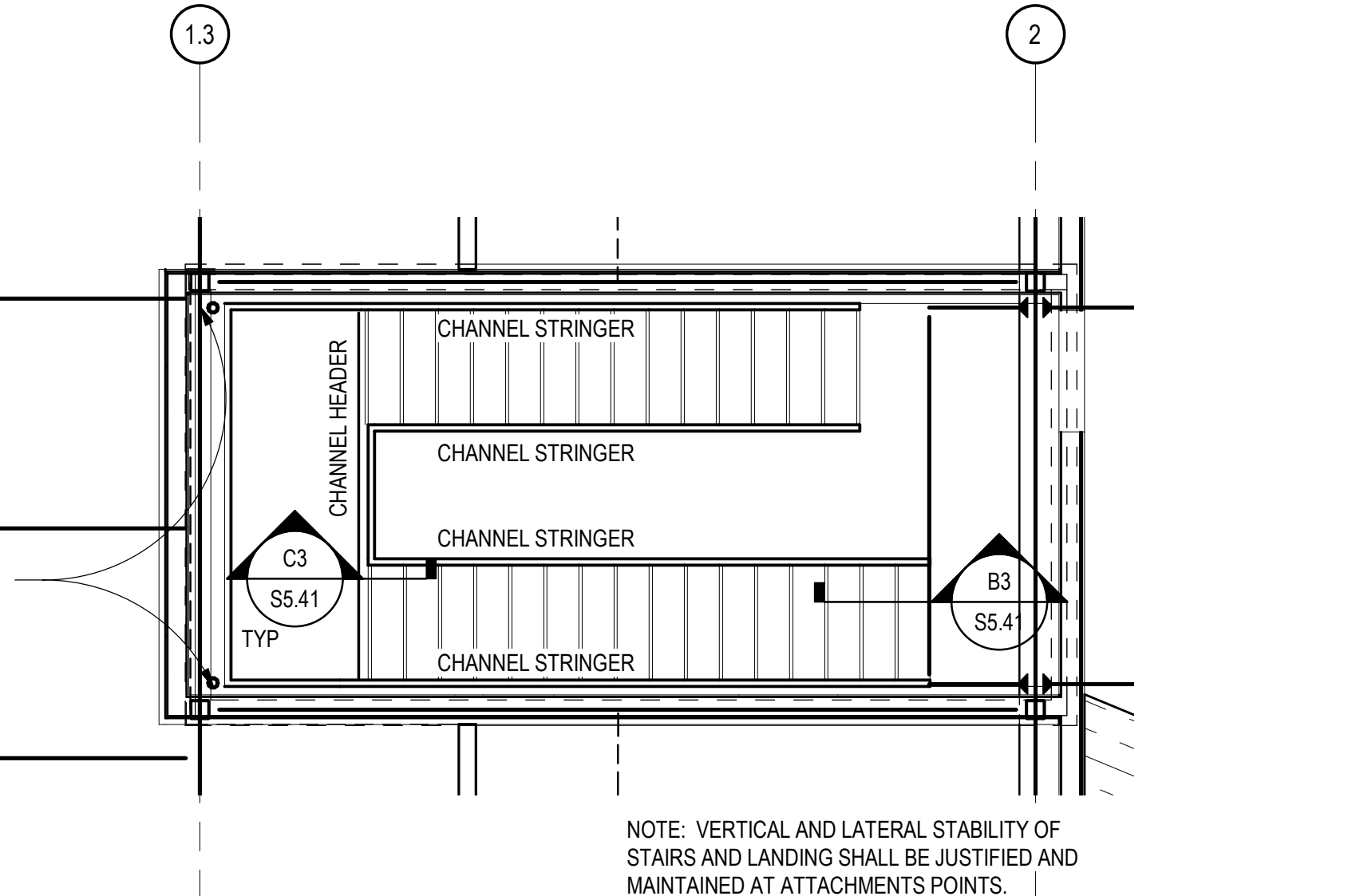
SHEET NUMBER:
S3.11
FOUNDATION SECTIONS



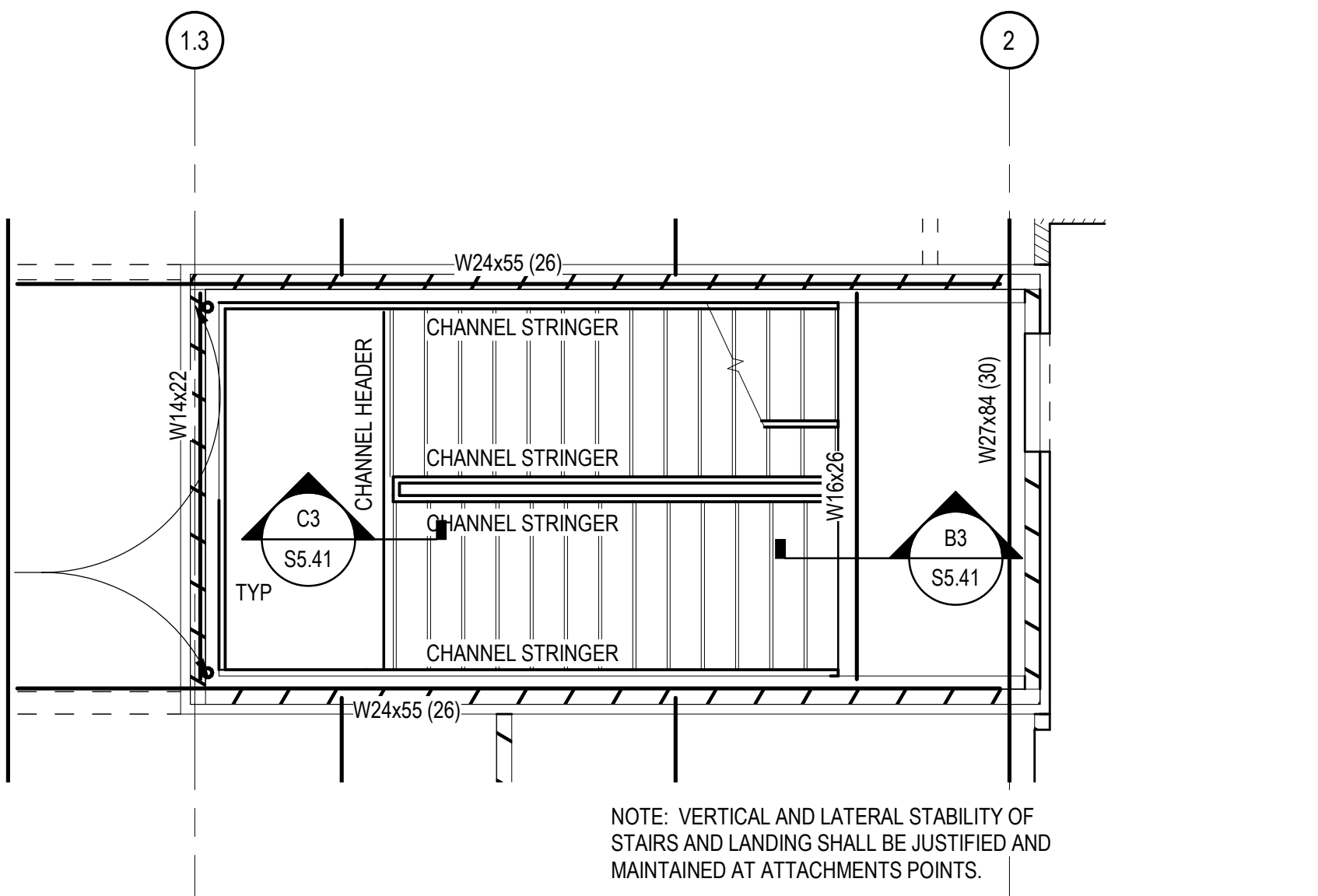
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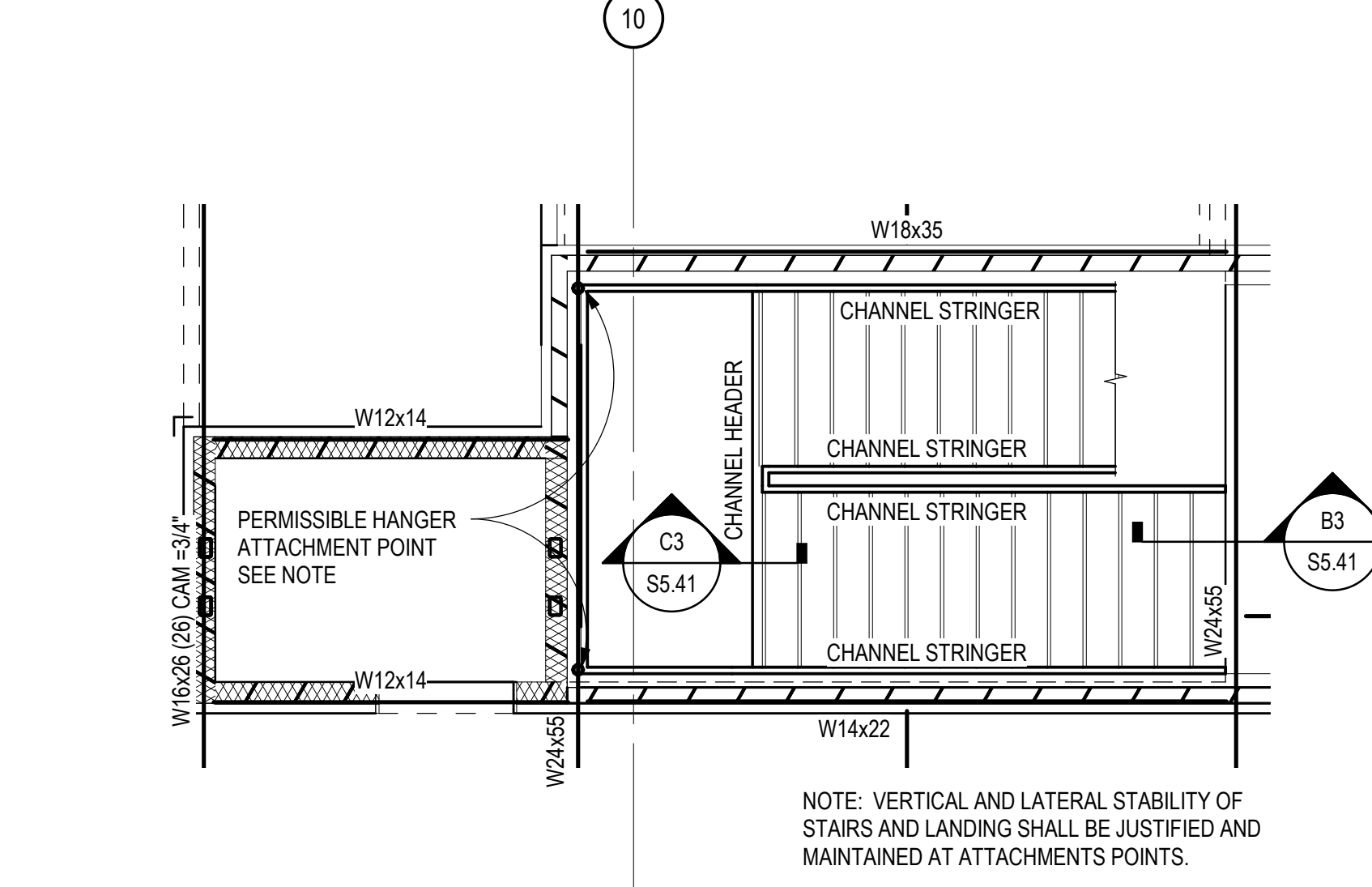
D1 ENLARGED SOUTHWEST STAIR FRAMING PLAN
SCALE: 1/4" = 1'-0"



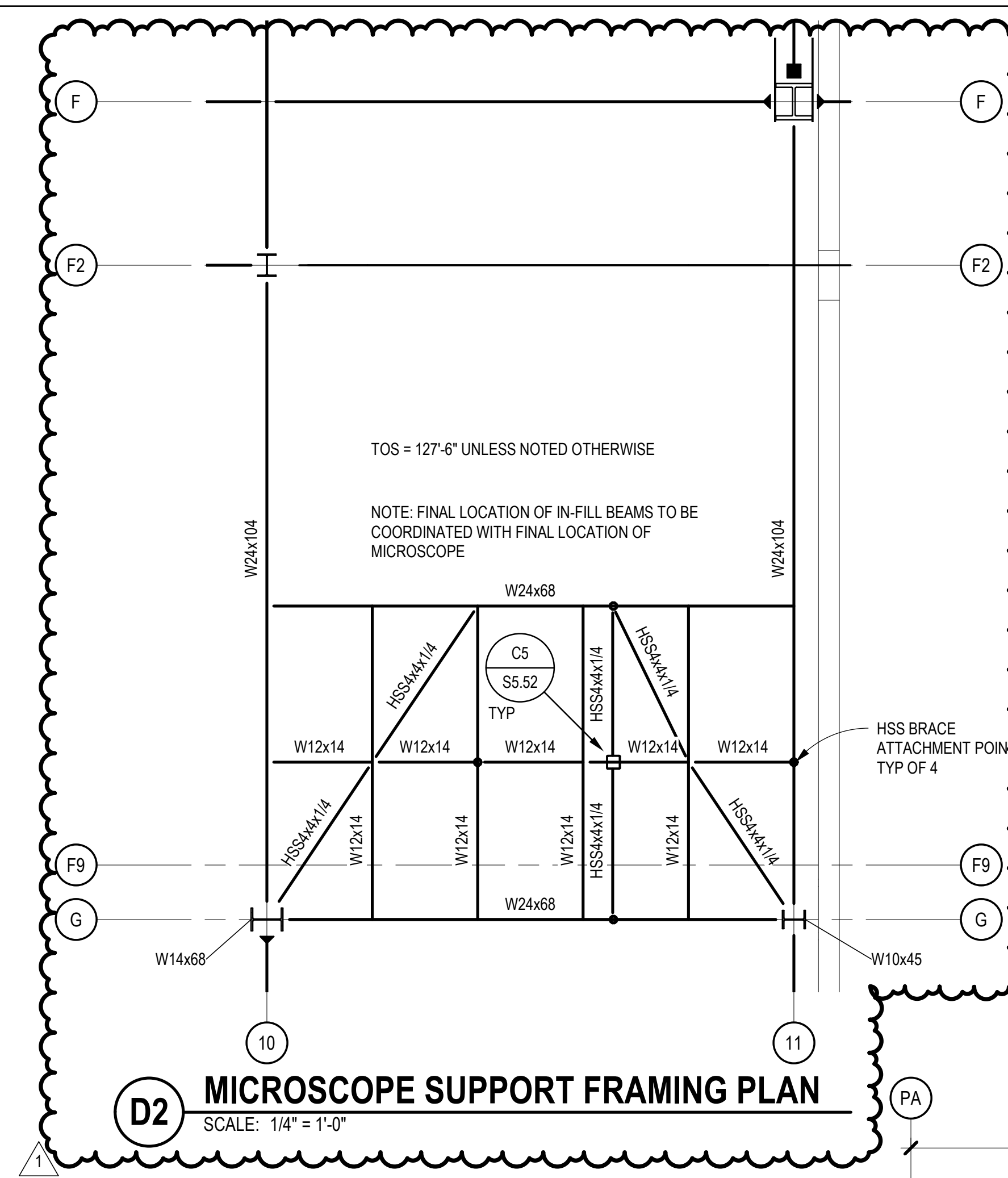
C1 ENLARGED WEST STAIR FRAMING PLAN TO MEZZANINE
SCALE: 1/4" = 1'-0"



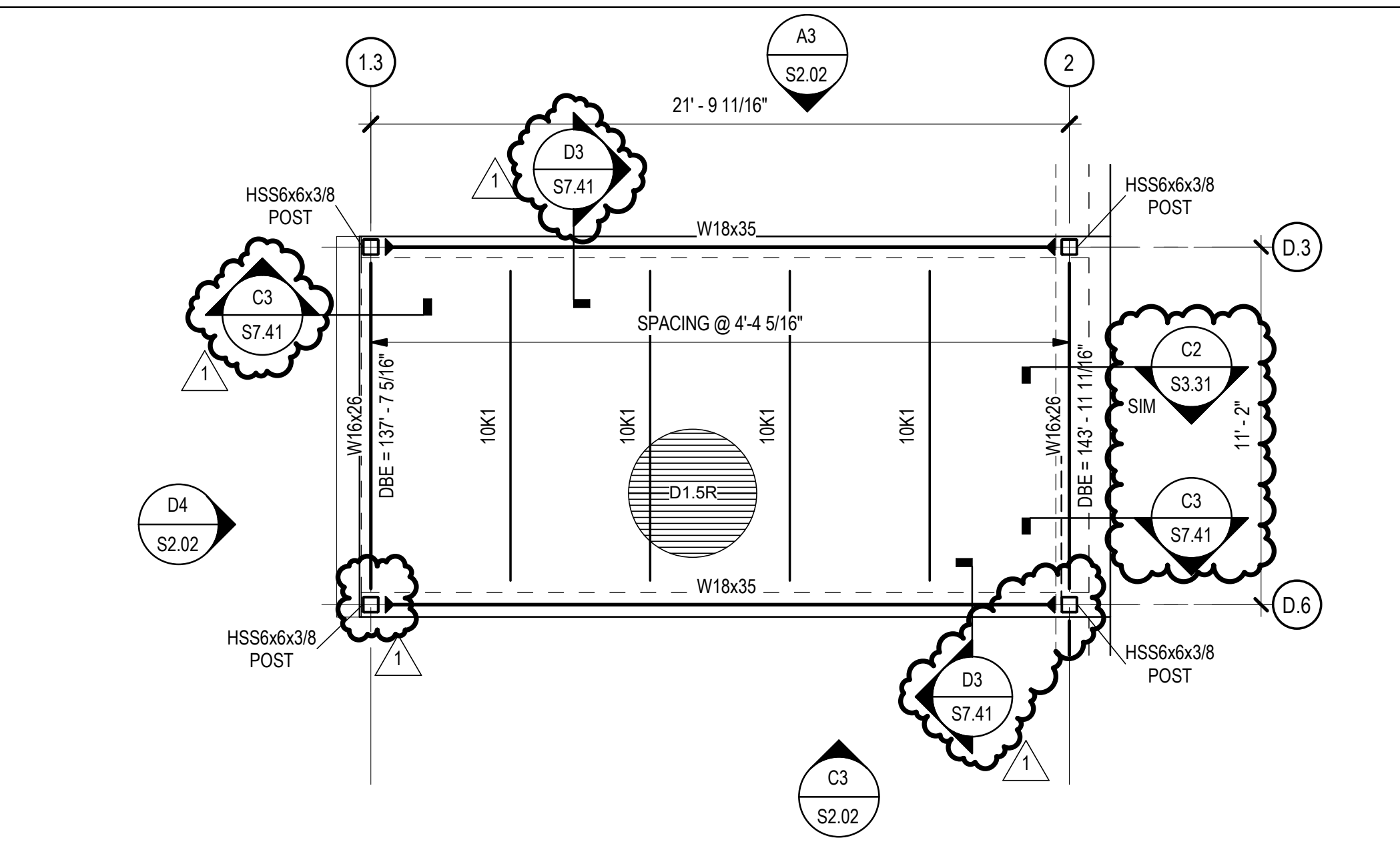
B1 ENLARGED WEST STAIR FRAMING PLAN
SCALE: 1/4" = 1'-0"



A1 ENLARGED EAST STAIR FRAMING PLAN
SCALE: 1/4" = 1'-0"

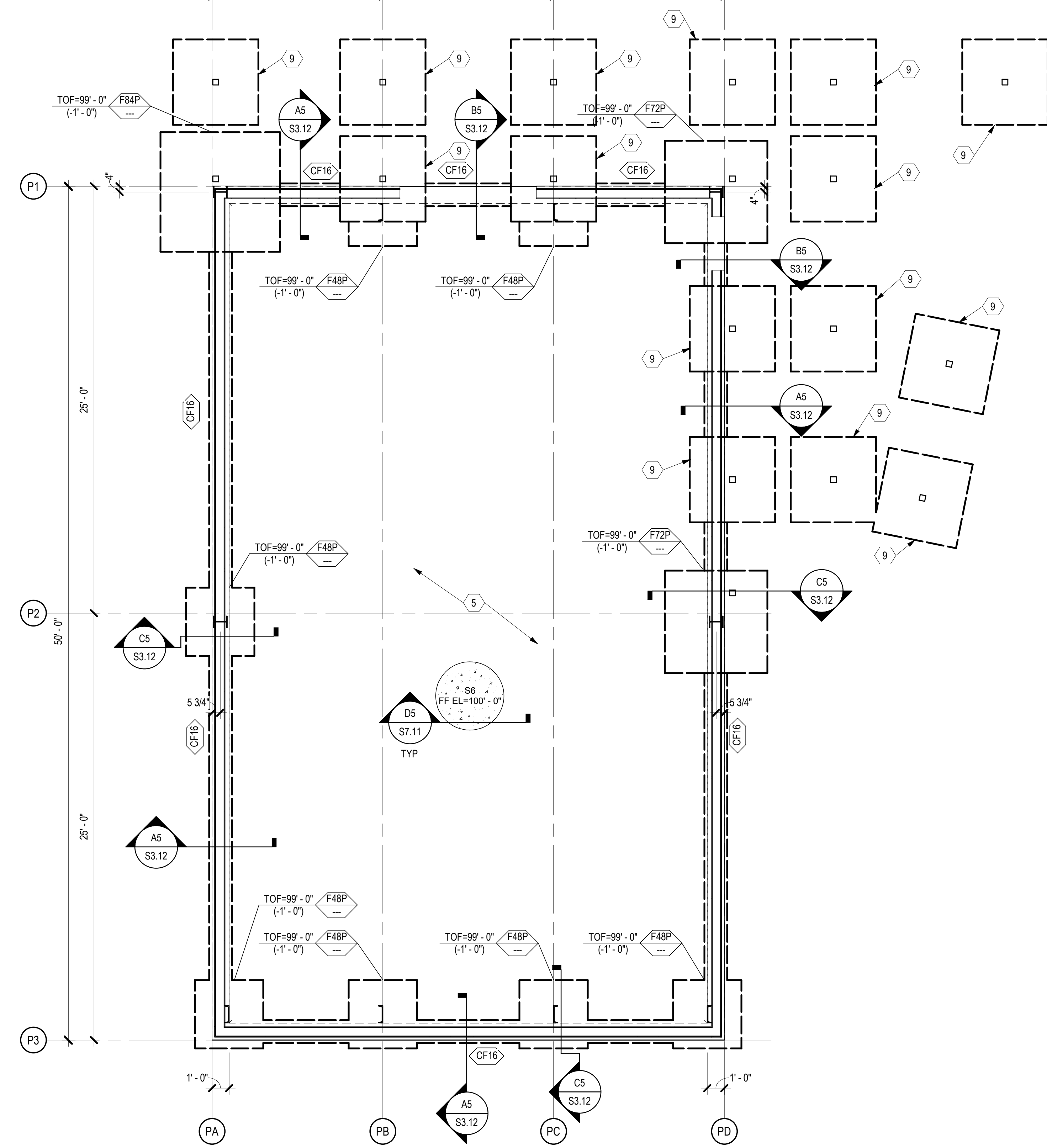


D2 MICROSCOPE SUPPORT FRAMING PLAN
SCALE: 1/4" = 1'-0"



D4 ENLARGED PLAN - WEST ROOF POP-UP
SCALE: 1/4" = 1'-0"

NOTE:
1. STONE COLUMN GROUND IMPROVEMENTS ARE NOT REQUIRED FOR PRE-ENGINEERED METAL BUILDING OR PRE-ENGINEERED CANOPY FOUNDATIONS.
2. FOUNDATION AND SLAB DESIGN FOR PRE-ENGINEERED METAL BUILDING AND PRE-ENGINEERED CANOPY ARE PRELIMINARY. SIZES SHALL BE VERIFIED UPON RECEIVING FINAL DESIGNS FROM THE MANUFACTURERS.



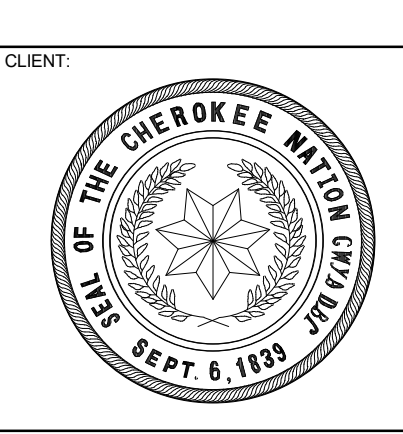
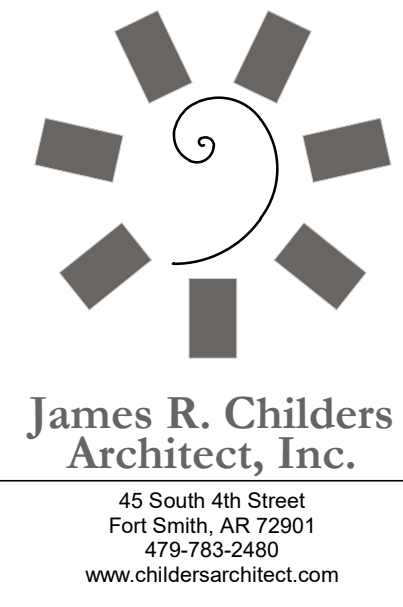
A3 ENLARGED PLAN - PEMB FOUNDATION
SCALE: 1/4" = 1'-0"

GENERAL SHEET NOTES

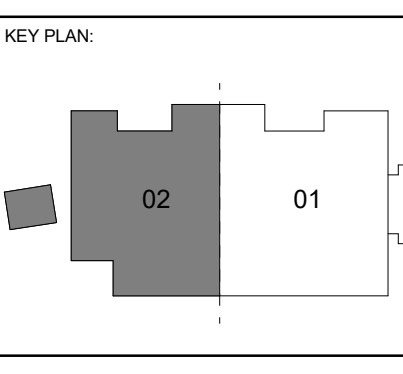
- SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- REFERENCE FINISH FLOOR ELEVATION 100'-0" = MEAN SEA FINISH FLOOR ELEVATION. SEE CIVIL DRAWINGS.
- TOP OF FOOTING ELEVATION = 98'-0" (-2'-0"), UNLESS NOTED OTHERWISE ON PLAN.
- NOTE TO CONTRACTOR: ENLARGED SLAB BLOCKOUTS MAY BE REQUIRED AT FRAME COLUMNS FOR MOMENT FRAME BASE PLATE CLEARANCE.
- NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACINGS OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
- DIMENSIONS ARE TO THE FACE OF STUD UNLESS NOTED OTHERWISE.
- SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
- EXISTING CONSTRUCTION IS PER AVAILABLE EXISTING DRAWINGS. ALL EXISTING CONSTRUCTION AND DIMENSIONS SHALL BE VERIFIED PRIOR TO CONSTRUCTION. SHOULD CONDITIONS VARY FROM THOSE SHOWN, CONTACT ENGINEER BEFORE PROCEEDING.
- PROVIDE SLAB JOINTS AT 10'-0" ON CENTER MAXIMUM. THE AREA OF THE CONTROL JOINT SHALL NOT EXCEED A 2:1 RATIO. CONTROL JOINTS SHALL BE LOCATED AT COLUMN LINES WHERE THE LAYOUT PERMITS. AT RE-ENTRANT CORNERS THAT DO NOT HAVE CONTROL JOINTS, PROVIDE 2#4 x 3'-0" DIAGONAL TO THE RE-ENTRANT CORNER.
- STRUCTURAL COLD FORMED METAL STUDS SHALL BE 6" WIDE UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING BY OTHERS.
- SEE SHEET S7.00 SERIES SHEETS FOR TYPICAL FOUNDATION SECTIONS AND DETAILS.
- SEE SHEET S6.01 FOR SCHEDULES.

SHEET KEYNOTE

- FLOOR DRAIN: SLOPE SLAB TO DRAIN 1/8" PER FOOT. COORDINATE EXACT SIZE AND LOCATION WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
- POST AND FOUNDATION AS REQUIRED FOR STAIR SUPPORT. STAIR ENGINEER TO PROVIDE REQUIRED LOADS AND LOCATIONS.
- ELEVATOR SUMP PIT. COORDINATE EXACT SIZE AND LOCATION WITH ELEVATOR MANUFACTURER. SEE A4 / S5.41
- HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION AND SPACING WITH ELEVATOR MANUFACTURER. SEE B4 / S5.41
- PRE-ENGINEERED METAL BUILDING STEEL AND ANCHORAGE BY OTHERS. CONTRACTOR TO CONFIRM LOCATIONS OF FOUNDATIONS WITH FINAL PRE-ENGINEERED METAL BUILDING SHOP DRAWINGS.
- NOTCH MASONRY AS REQUIRED TO FACILITATE BASEPLATE INSTALLATION. STEP BOND BEAM AT THIS LOCATION. FILL VOID FROM NOTCH WITH NON-SHRINK GROUT.
- CUT AND REMOVE EXISTING SLAB AS REQUIRED TO PLACE NEW FOOTING. NEW SLAB TO POUR UP TO REMAINING SLAB.
- CENTER FOOTING ON GRID C.
- F8A PRE-MANUFACTURED SUNSHADE CONCRETE FOOTING. TOP OF FOOTING = 99'-0" (-1'-0"). SEE SHEET S6.01 FOR FOOTING SCHEDULE. COORDINATE FINAL LOCATION WITH SUNSHADE MANUFACTURER.
- EXISTING CANOPY. SEE ARCHITECTURAL DEMOLITION PLANS FOR EXTENT OF DEMOLITION.
- HSS6x6x1/2 ELEVATOR SUPPORT POST. COORDINATE EXACT LOCATION AND SPACING WITH ELEVATOR MANUFACTURER. SEE B4 / S5.41, D3 / S5.41, A2 / S5.41, B2 / S5.41, AND C2 / S5.41
- 1 1/2" RECESSED SLAB AT ADA SHOWER. COORDINATE EXACT SIZE, LOCATION, AND SLOPE REQUIREMENTS WITH ARCHITECTURAL DRAWINGS. SEE C4 / S7.11
- 18" DIAMETER PRECAST CONCRETE COLUMN BY OTHERS. SEE C3 / S3.11 AND B1 / S3.31
- 18" DIAMETER PRECAST CONCRETE CANOPY COLUMN BY OTHERS. SEE C3 / S3.11, C4 / S3.12, A1 / S3.31, AND A5 / S3.31



**WILMA P. MANKILLER HEALTH CENTER
EXPANSION**
STILWELL, OKLAHOMA



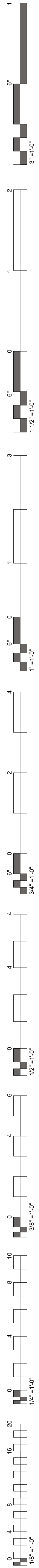
PROJECT PHASE:
BID PACKAGE 01

#	DATE	REVISIONS / DESCRIPTION
1	11/22/19	BID PACKAGE 01 - ADD 01

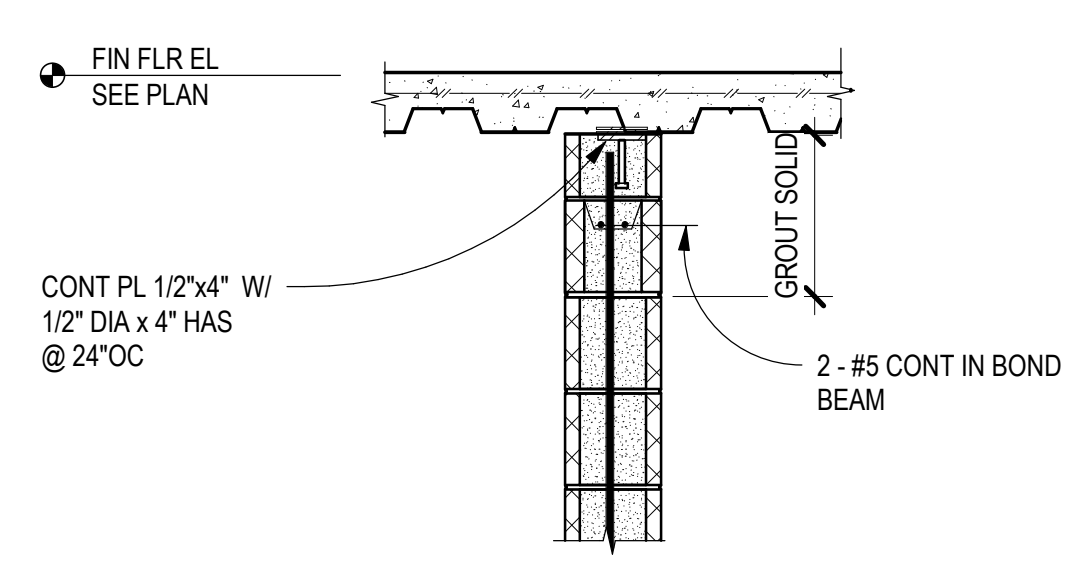
DATE: 11-01-19
JOB NUMBER: 18-01.01

SHEET NUMBER:
S4.01

ENLARGED PLANS

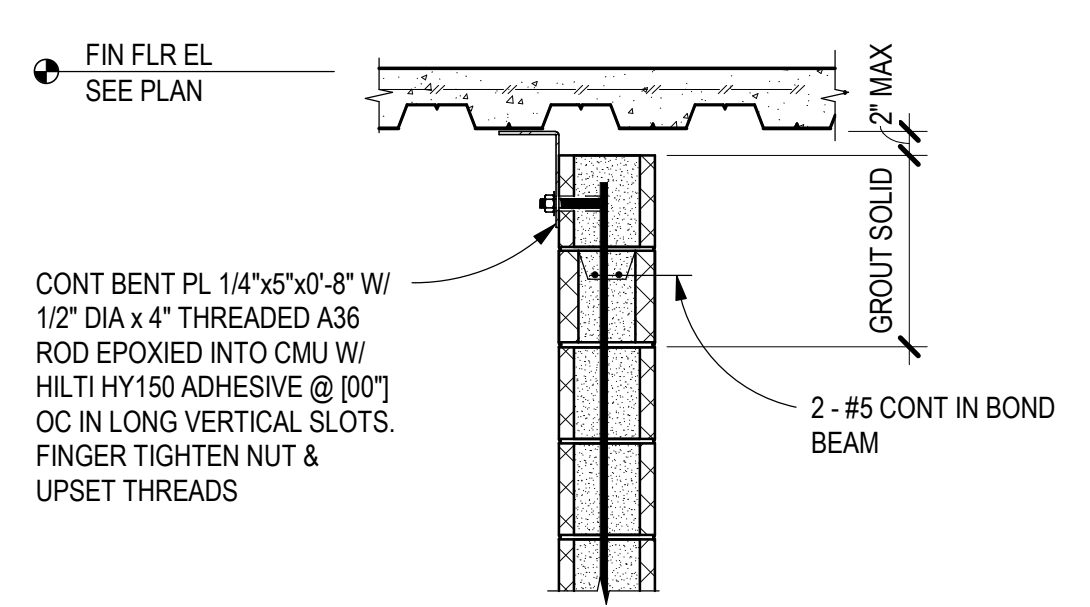


NOTE: ATTACH DECK TO SUPPORT ANGLE PER NOTE ON PLAN

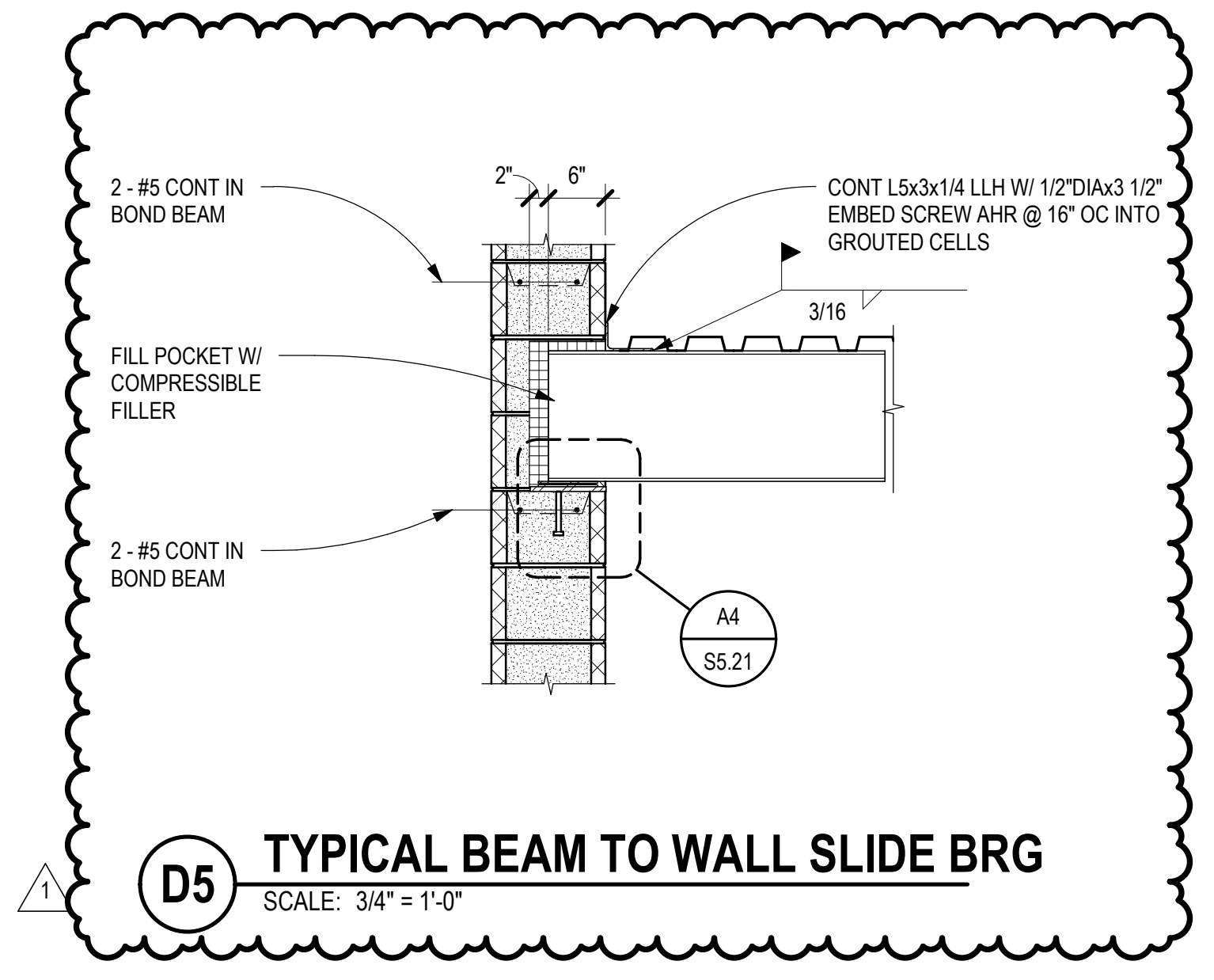


D3 CMU BEARING WALL TO DECK
SCALE: 3/4" = 1'-0"

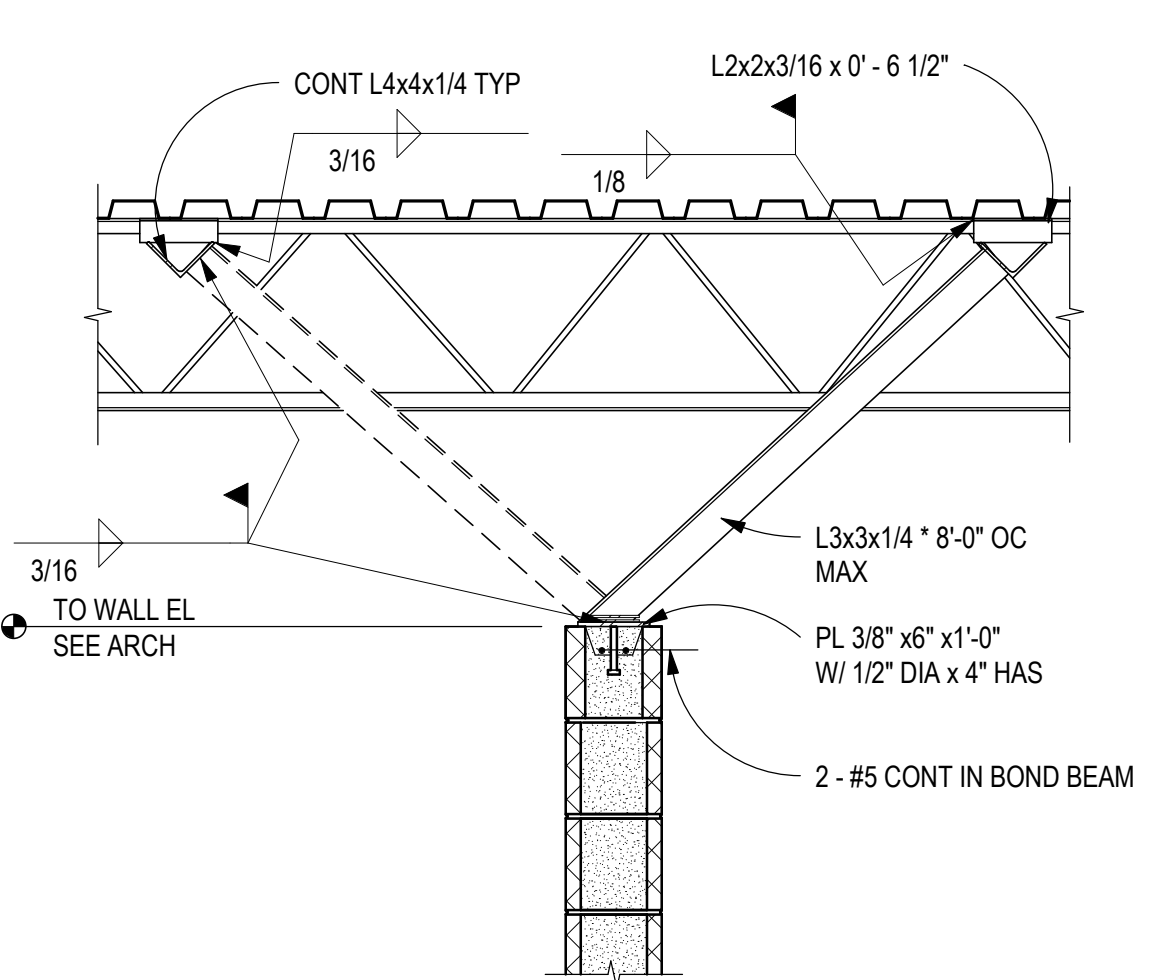
NOTE: ATTACH DECK TO SUPPORT ANGLE PER NOTE ON PLAN



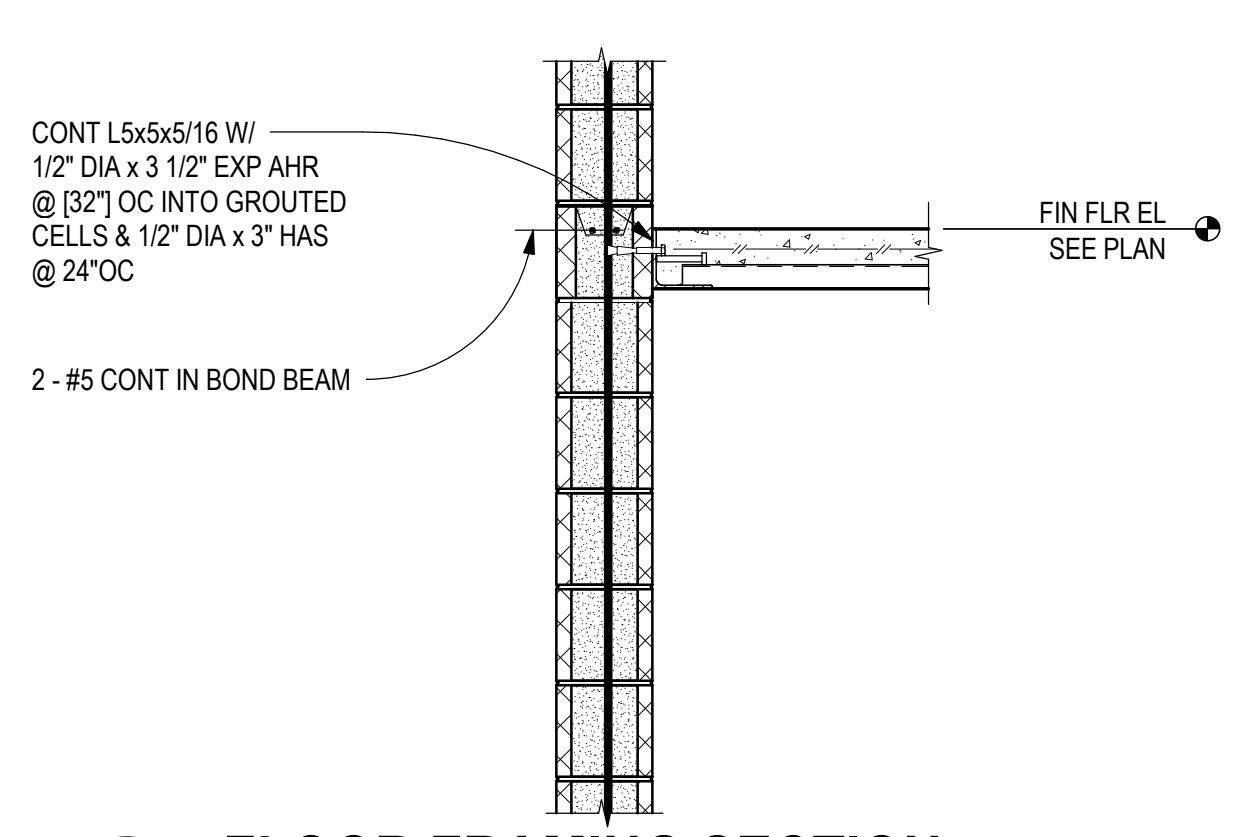
D4 CMU NON-BRG WALL TO DECK
SCALE: 3/4" = 1'-0"



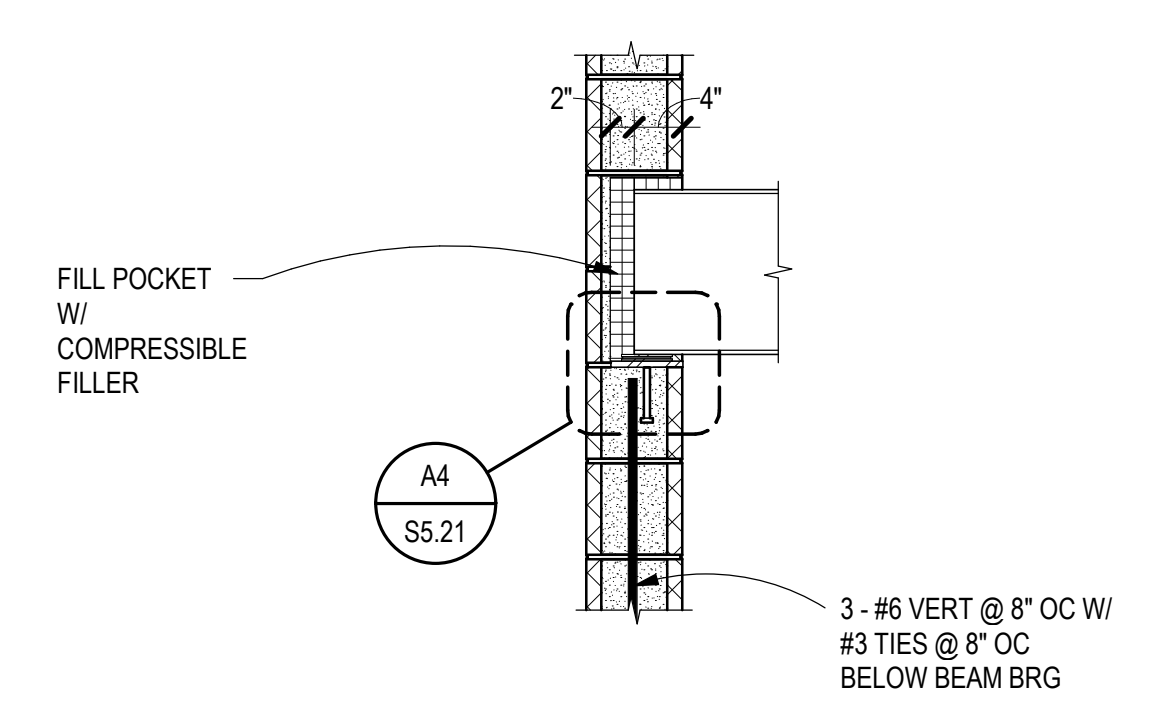
D5 TYPICAL BEAM TO WALL SLIDE BRG
SCALE: 3/4" = 1'-0"



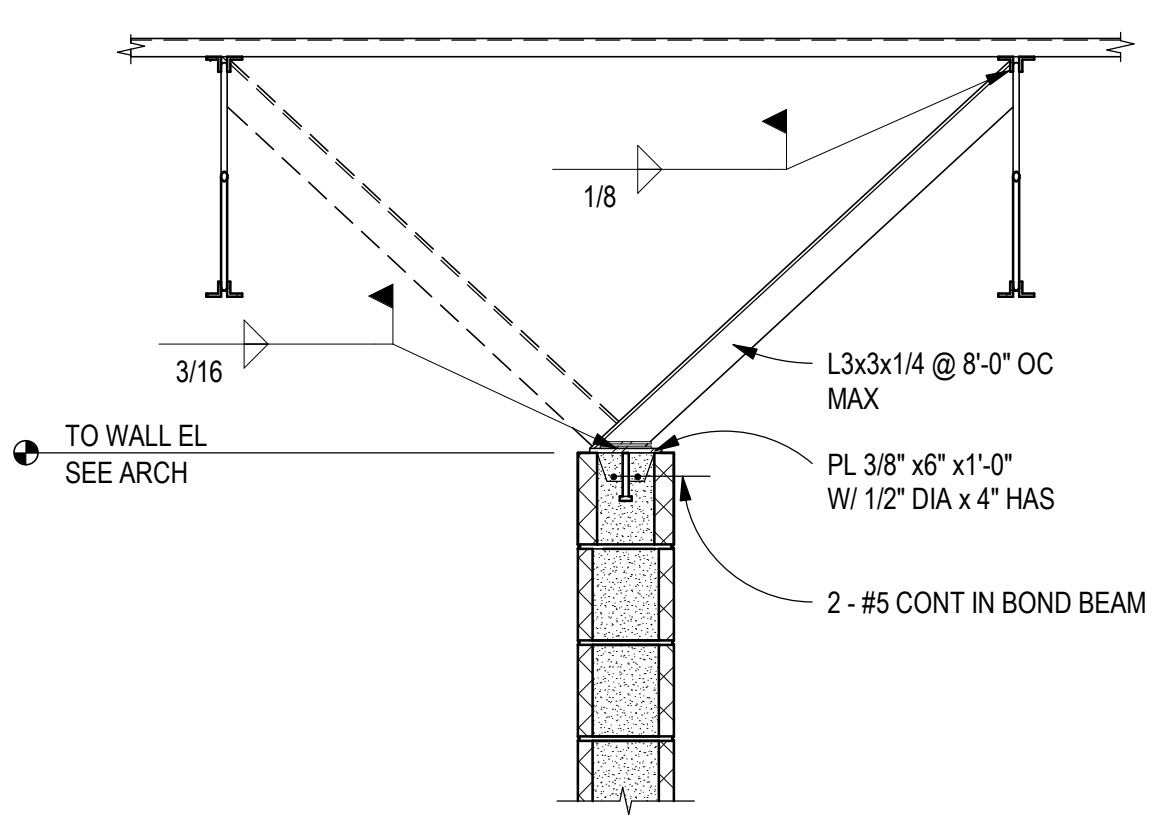
C3 CMU WALL BRACING SECTION
SCALE: 3/4" = 1'-0"



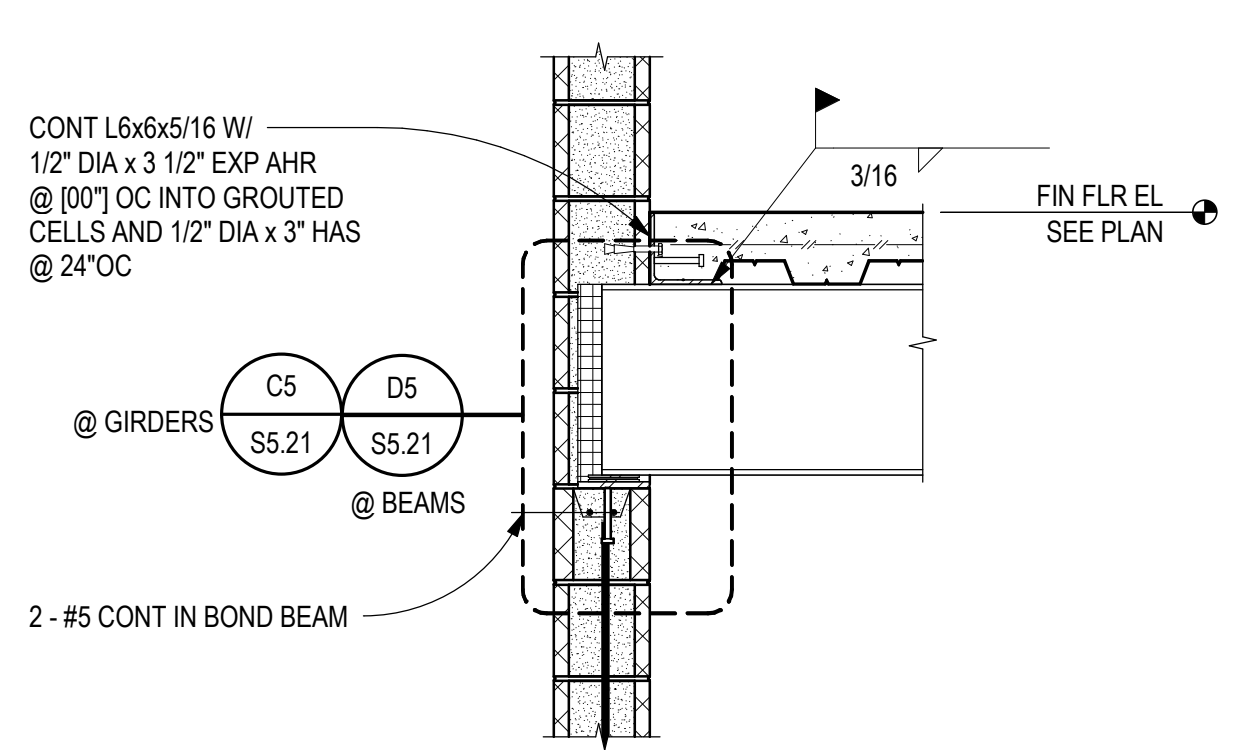
C4 FLOOR FRAMING SECTION
SCALE: 3/4" = 1'-0"



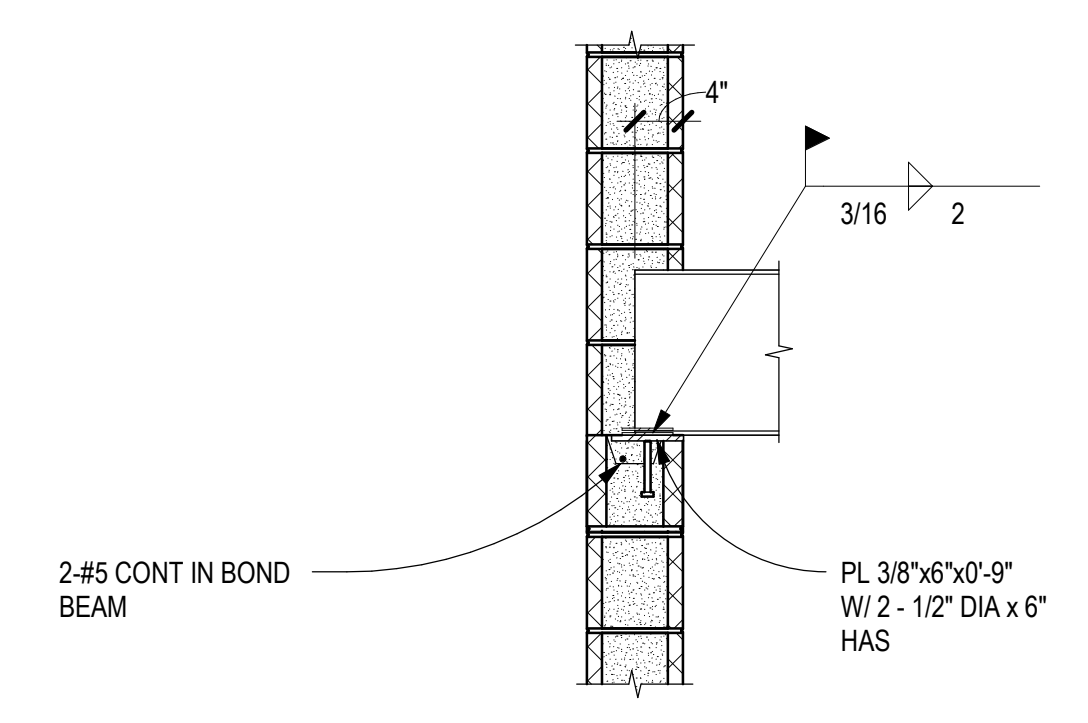
C5 TYPICAL GIRDER TO WALL SLIDE BRG
SCALE: 3/4" = 1'-0"



B3 CMU WALL BRACING SECTION
SCALE: 3/4" = 1'-0"

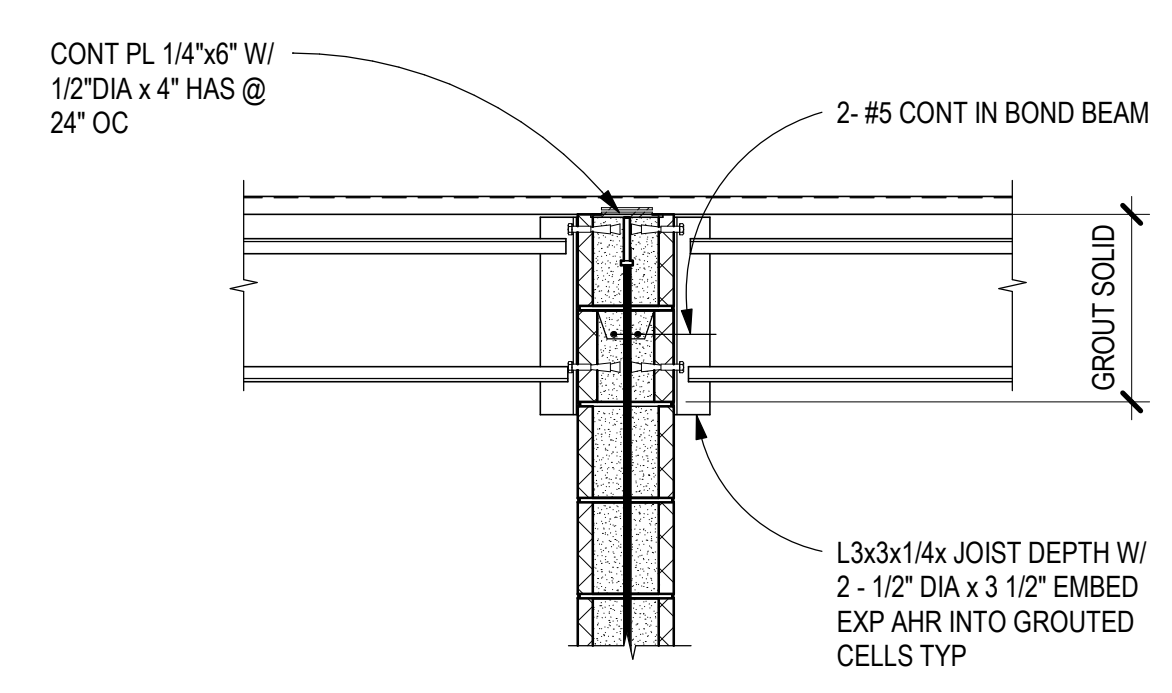


B4 FLOOR FRAMING SECTION
SCALE: 3/4" = 1'-0"

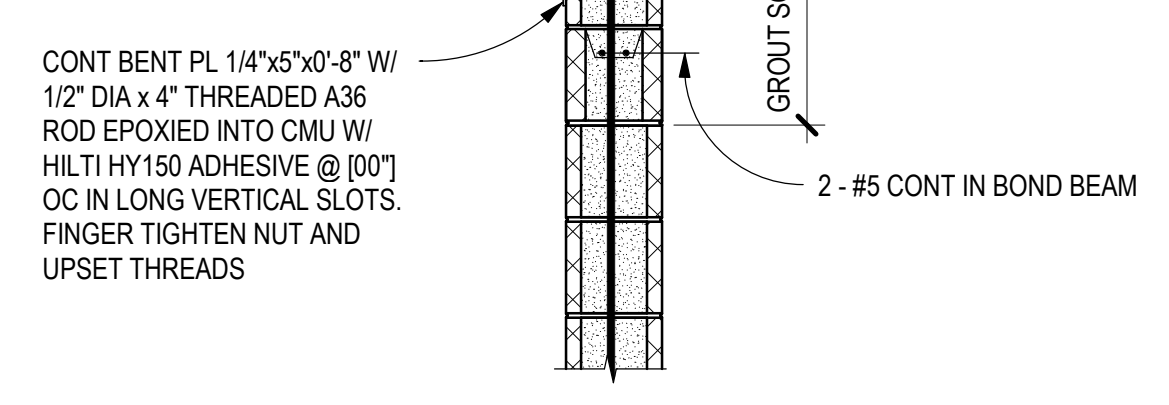


B5 TYPICAL BEAM TO CMU WALL
SCALE: 3/4" = 1'-0"

NOTE: ATTACH DECK TO SUPPORT ANGLE PER NOTE ON PLAN

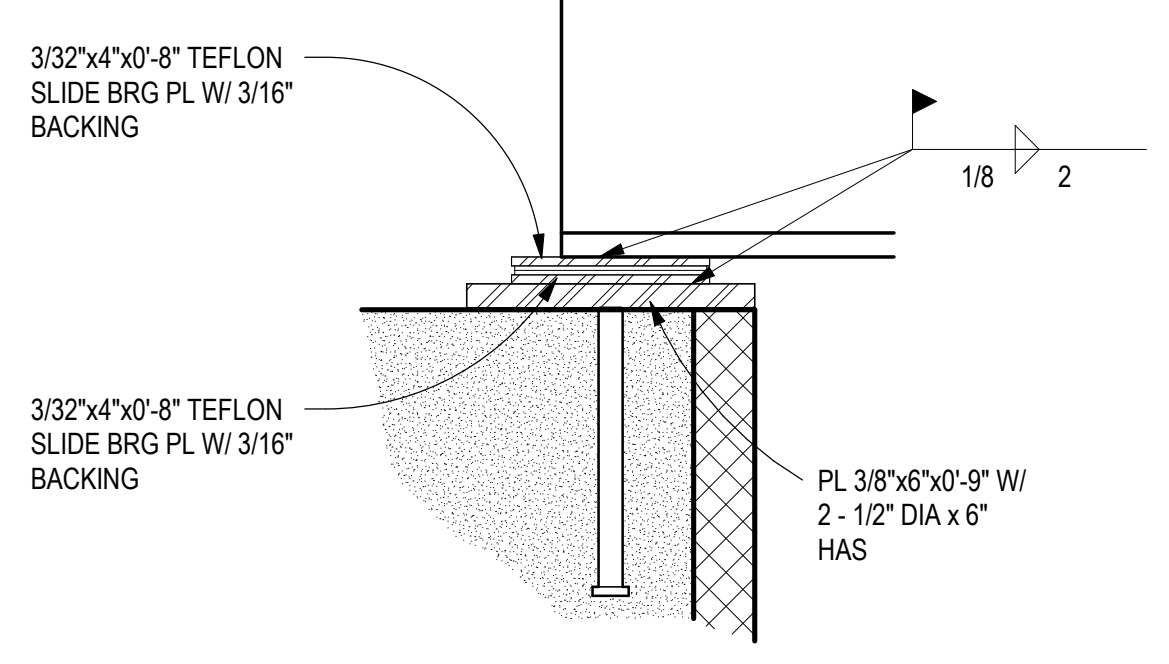


A2 INT CMU BEARING WALL TO DECK
SCALE: 3/4" = 1'-0"

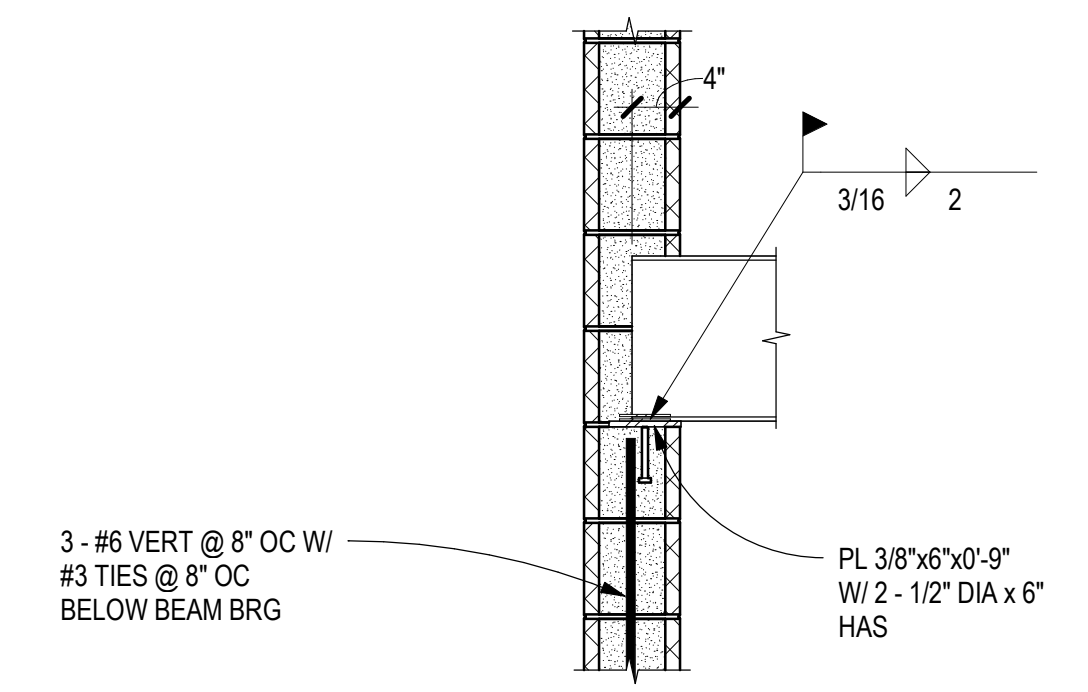


A3 CMU NON-BRG WALL TO DECK
SCALE: 3/4" = 1'-0"

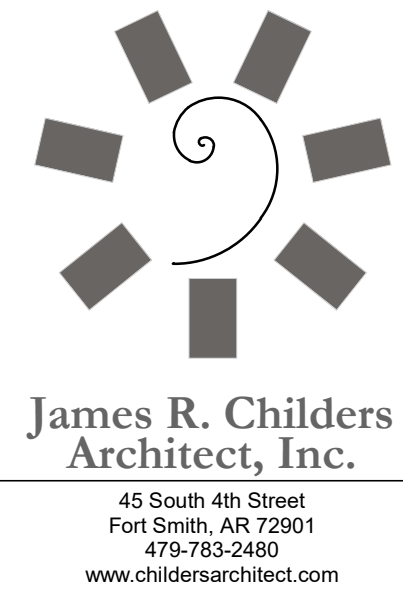
NOTE: INSTALL PLATES TEFLON TO TEFLON



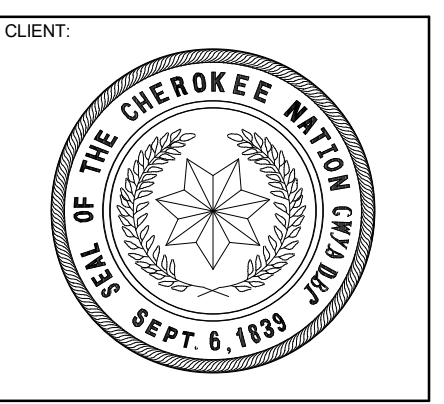
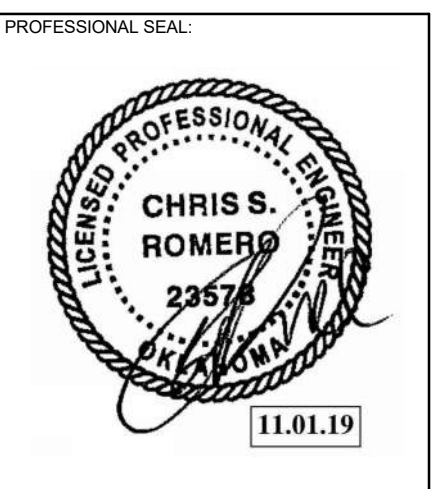
A4 TYPICAL SLIDE BRG CONN
SCALE: 1 1/2" = 1'-0"



A5 TYPICAL GIRDER TO CMU WALL
SCALE: 3/4" = 1'-0"



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**WILMA P. MANKILLER HEALTH CENTER
EXPANSION**
STILWELL, OKLAHOMA

KEY PLAN:

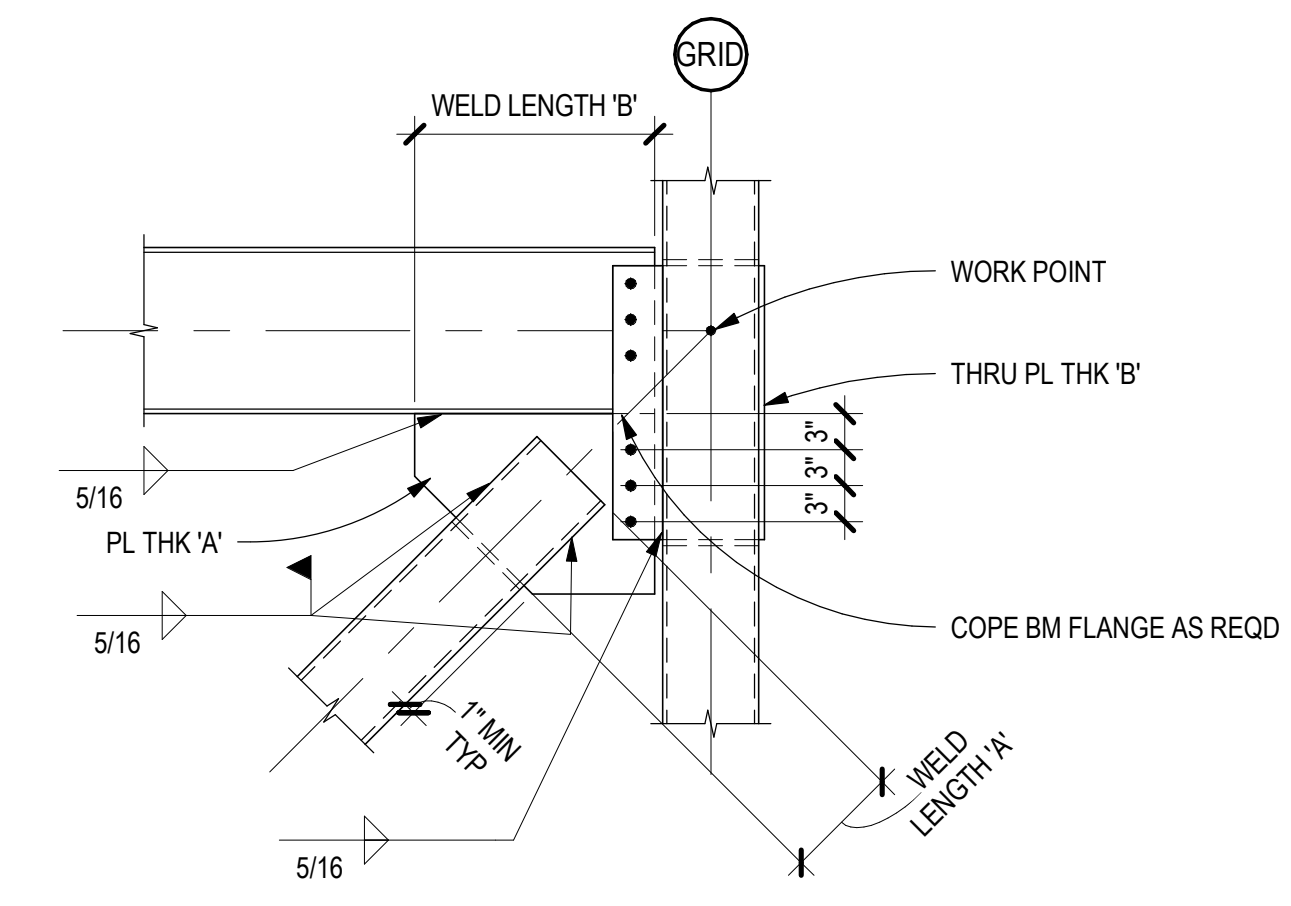
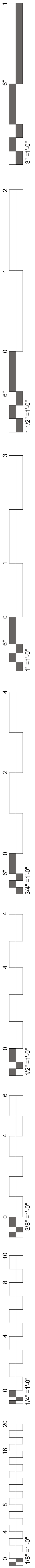
PROJECT PHASE:
BID PACKAGE 01

#	DATE	REVISIONS
1	11/22/19	BID PACKAGE 01 - ADD 01

DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER:
S5.21

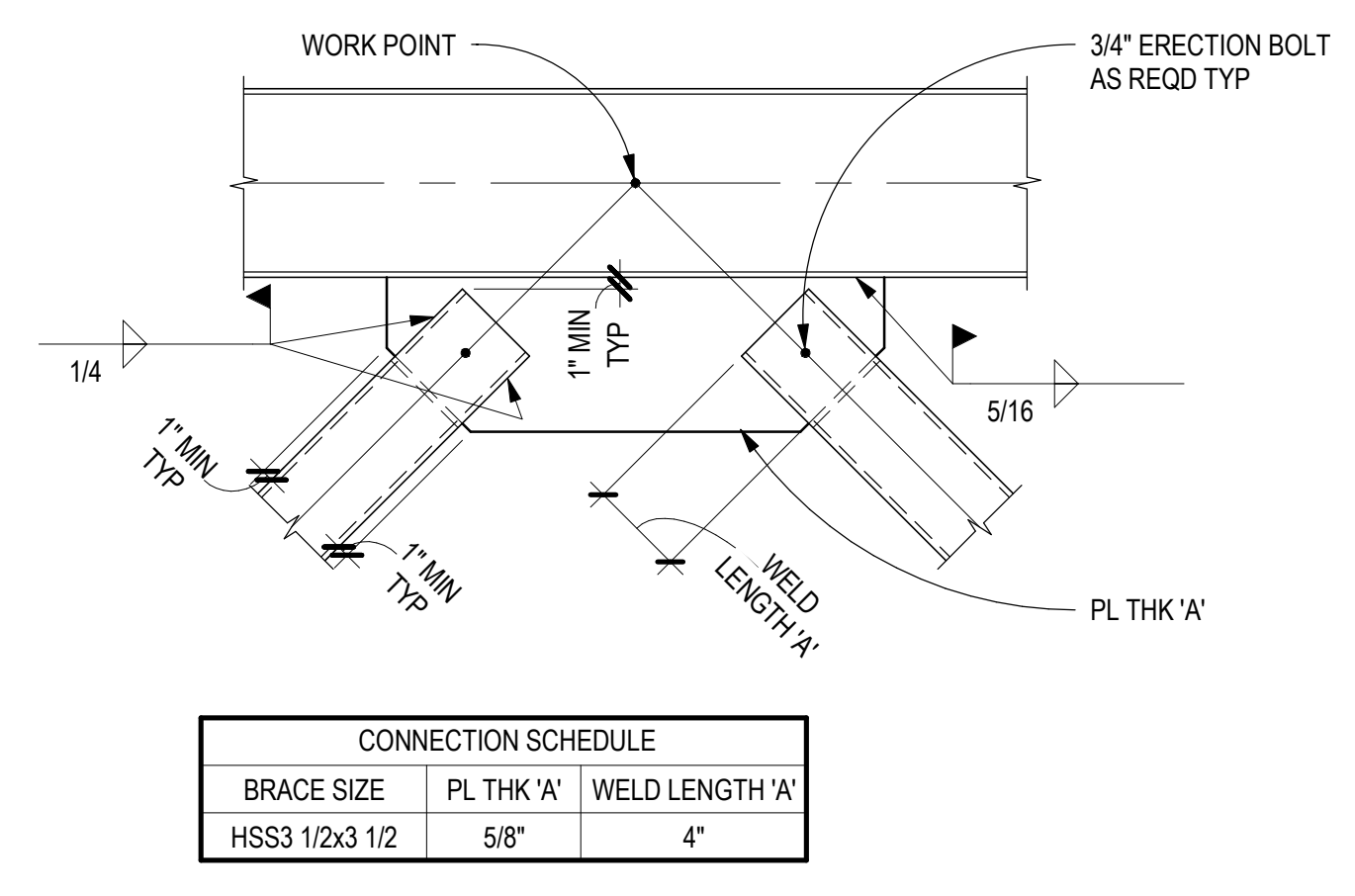
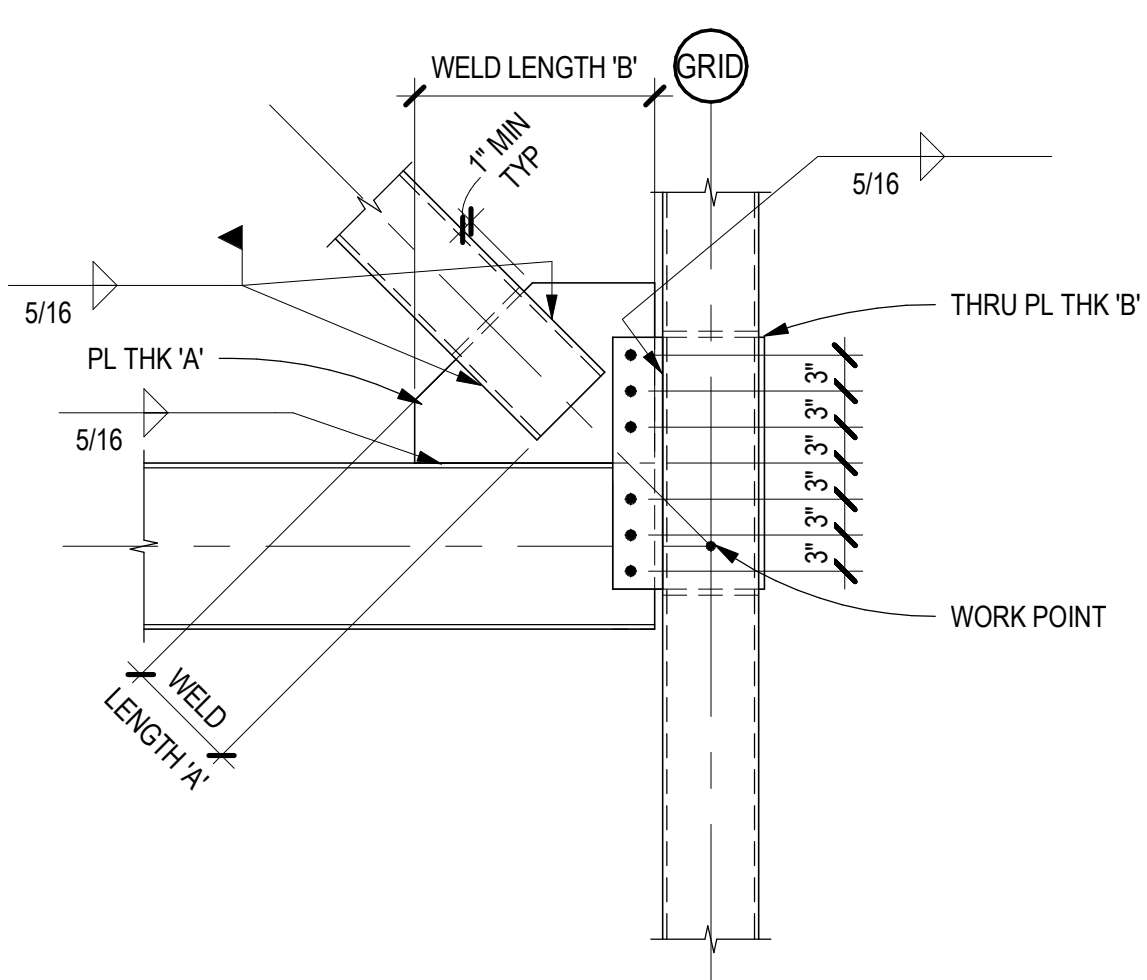
MASONRY FRAMING SECTIONS AND DETAILS



CONNECTION SCHEDULE					
BRACE SIZE	PL THK 'A'	THRU PL THK 'B'	WELD LENGTH 'A'	WELD LENGTH 'B'	GUSSET PL BOLTS
HSS4x4	5/8"	5/8"	4"	20"	3
HSS5x5	5/8"	5/8"	5"	20"	3
HSS6x6	5/8"	5/8"	6"	22"	3
HSS8x8	3/4"	3/4"	8"	24"	4
HSS12x8	1"	1"	12"	28"	5

- NOTES**
- LENGTHS GIVEN ARE SINGLE-SIDE LENGTHS AND MINIMUM LENGTHS
 - LONGER GUSSET/WELD LENGTHS MAY BE REQD WHERE BRACE SLOPE VARIES FROM 1:1
 - ALL BOLTS IN STANDARD HOLES

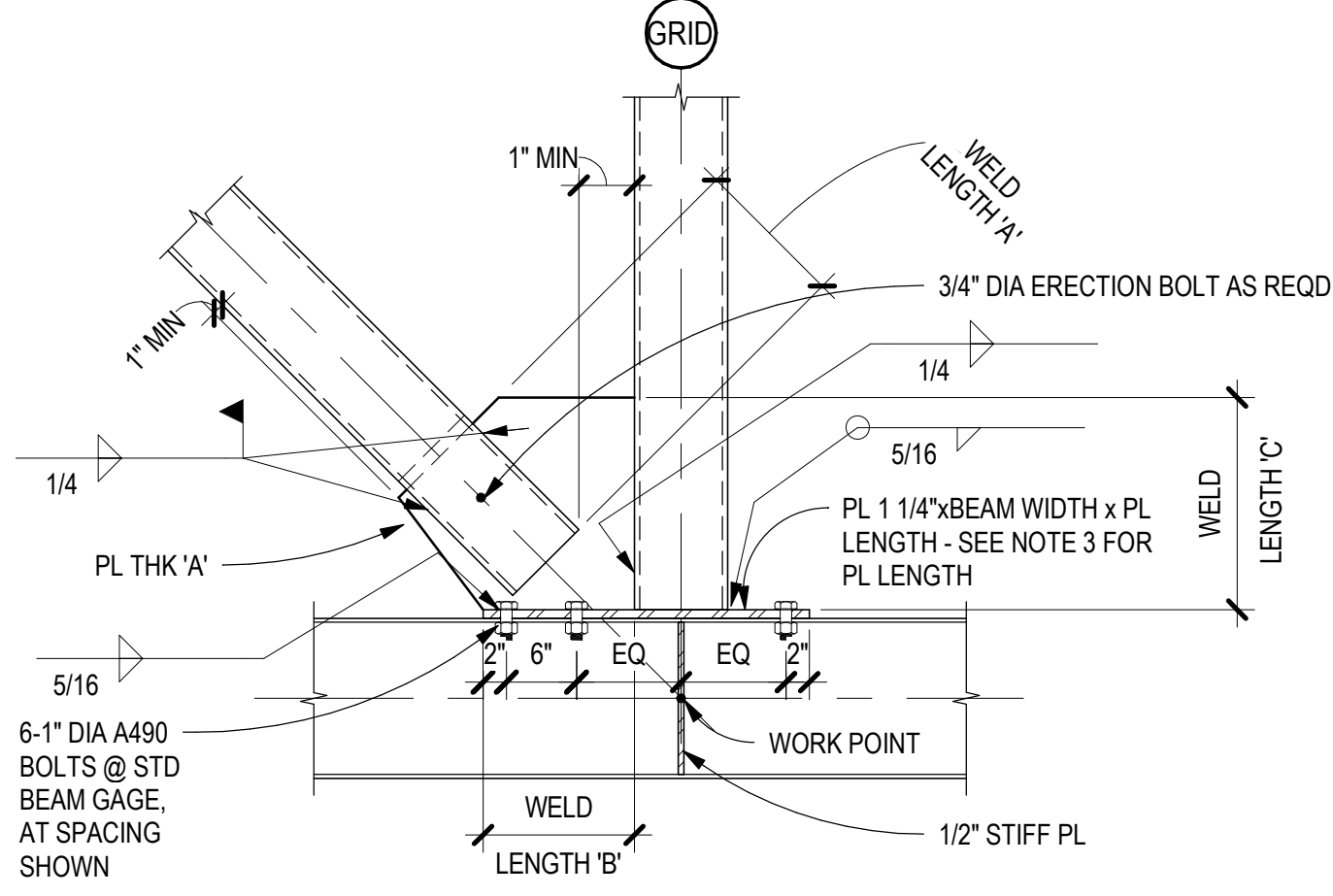
D2 BRACED FRAME CONNECTION
SCALE: 3/4" = 1'-0"



CONNECTION SCHEDULE		
BRACE SIZE	PL THK 'A'	WELD LENGTH 'A'
HSS3 1/2x3 1/2	5/8"	4"

- NOTES**
- LENGTHS GIVEN ARE SINGLE-SIDE LENGTHS AND MINIMUM REQUIRED LENGTHS
 - LONGER GUSSET/WELD LENGTHS MAY BE REQD WHERE BRACE SLOPE VARIES FROM 1:1
 - BRACE AND GUSSET CONNECTION CAN OCCUR ON TOP AND BOTTOM FLANGES

D3 BRACED FRAME CONNECTION
SCALE: 3/4" = 1'-0"



CONNECTION SCHEDULE			
BRACE SIZE	PL THK 'A'	WELD LENGTH 'A'	WELD LENGTH 'C'
HSS3 1/2x3 1/2	5/8"	4"	12"
HSS5x5	1/2"	5"	12"

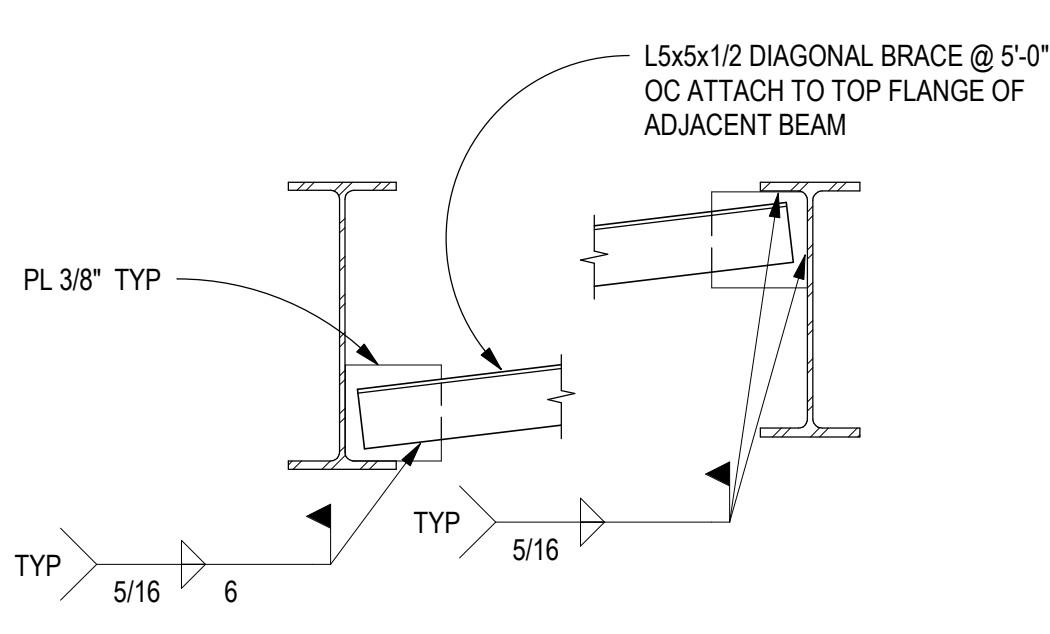
- NOTES**
- LENGTHS GIVEN ARE SINGLE-SIDE LENGTHS AND MINIMUM LENGTHS
 - LONGER GUSSET/WELD LENGTHS MAY BE REQD WHERE BRACE SLOPE VARIES FROM 1:1
 - PLATE LENGTHS: BASED ON POST SIZE
A. HSS5x5 & HSS6x6: 2' - 0"

C3 BRACED FRAME CONNECTION
SCALE: 3/4" = 1'-0"

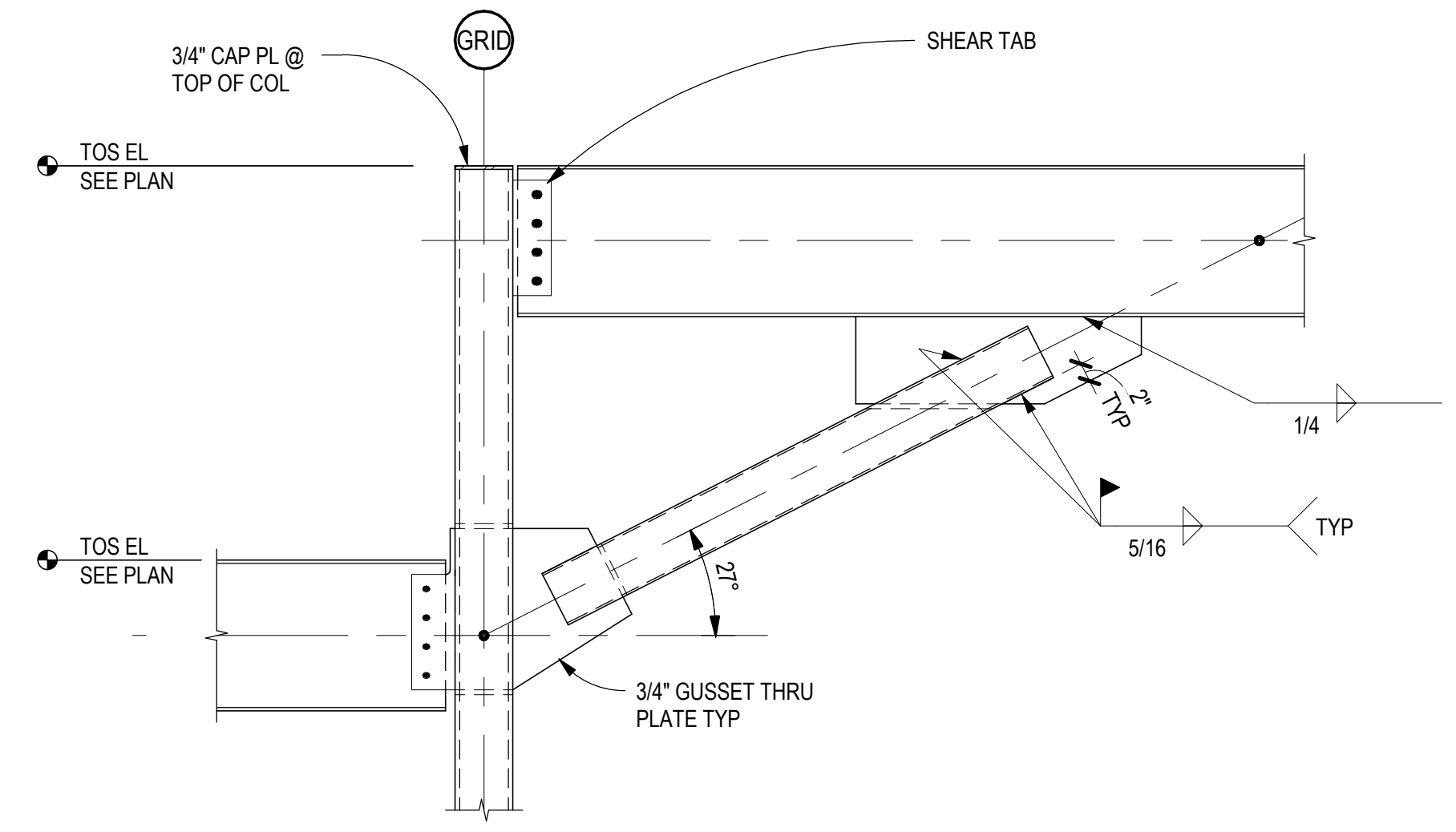
CONNECTION SCHEDULE					
BRACE SIZE	PL THK 'A'	THRU PL THK 'B'	WELD LENGTH 'A'	WELD LENGTH 'B'	GUSSET PL BOLTS
HSS4x4	5/8"	5/8"	4"	20"	3
HSS5x5	5/8"	5/8"	5"	20"	3
HSS6x6	5/8"	5/8"	6"	22"	3
HSS8x8	3/4"	3/4"	8"	24"	4
HSS12x8	1"	1"	12"	28"	5

- NOTES**
- LENGTHS GIVEN ARE SINGLE-SIDE LENGTHS AND MINIMUM LENGTHS
 - LONGER GUSSET/WELD LENGTHS MAY BE REQD WHERE BRACE SLOPE VARIES FROM 1:1
 - ALL BOLTS IN STANDARD HOLES

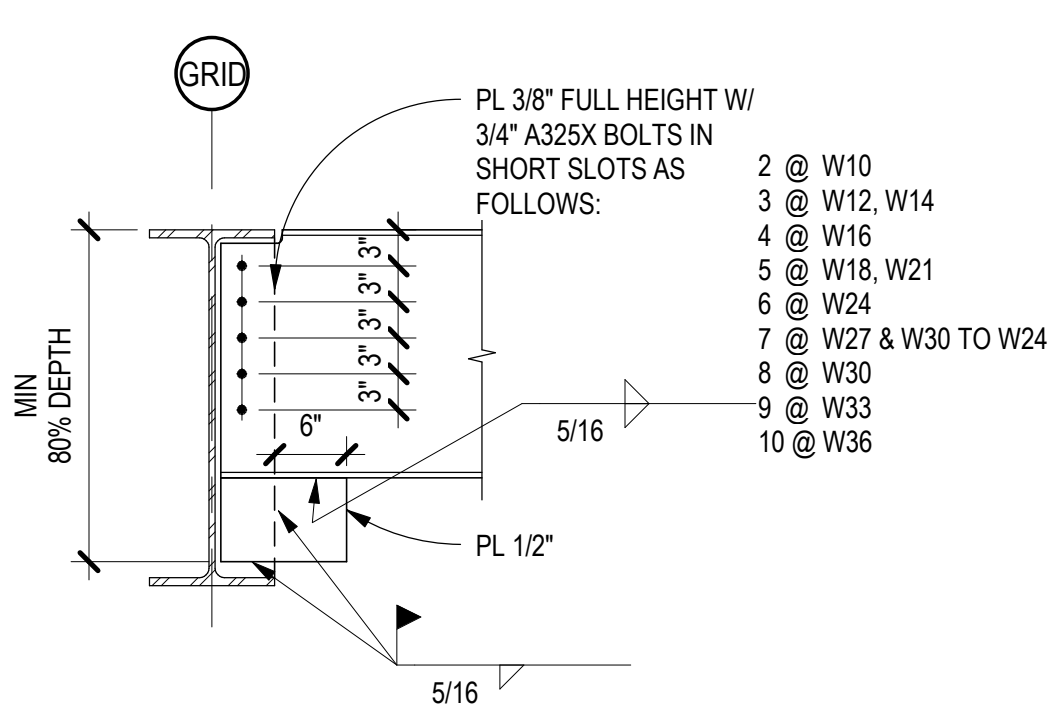
B2 BRACED FRAME CONNECTION
SCALE: 3/4" = 1'-0"



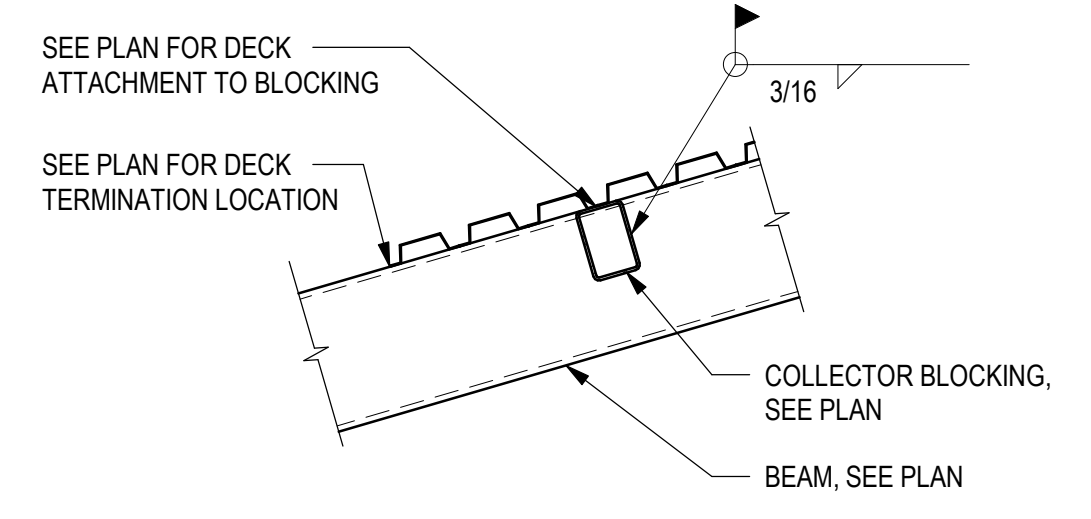
B3 DIAG ANGLE AT MOMENT CONN
SCALE: 3/4" = 1'-0"



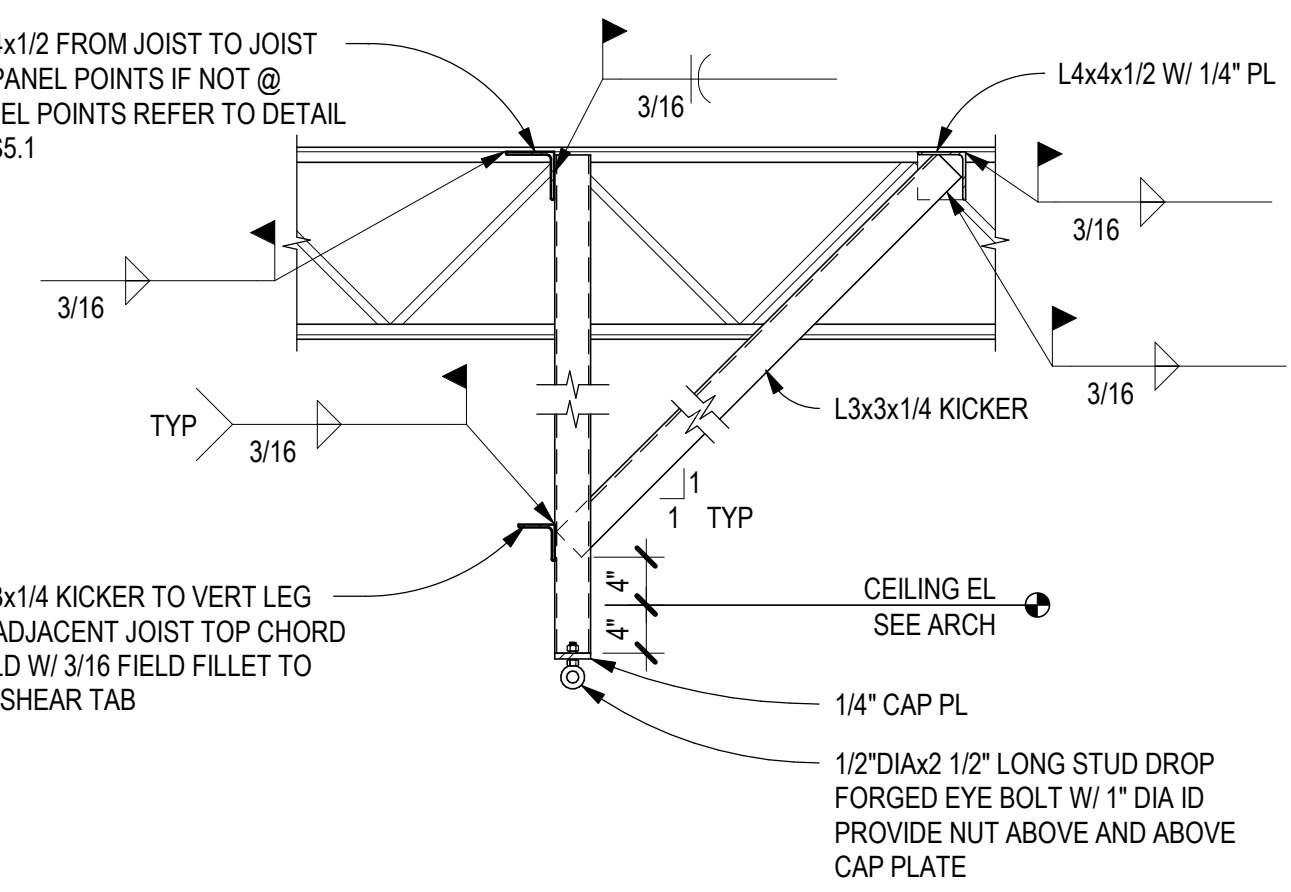
A1 COLLECTOR FRAMING DETAIL
SCALE: 3/4" = 1'-0"



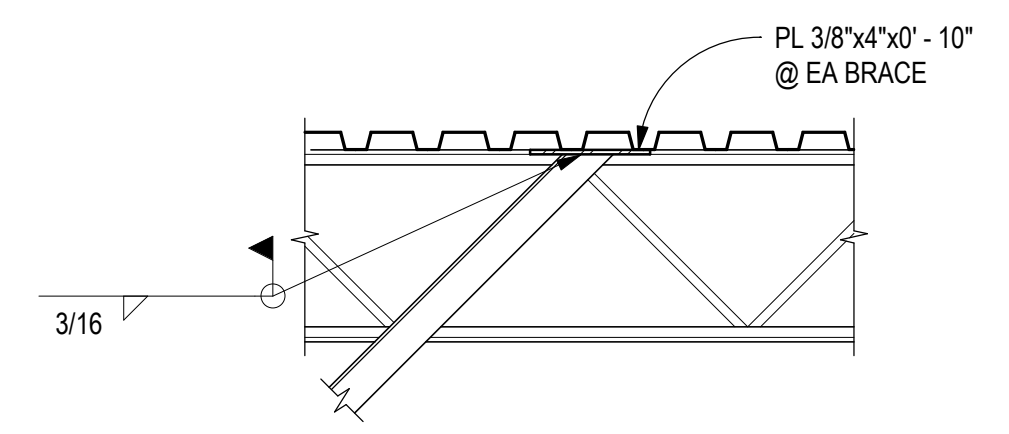
A3 BEAM CONN AT MOMENT (WUF) CONN
SCALE: 3/4" = 1'-0"



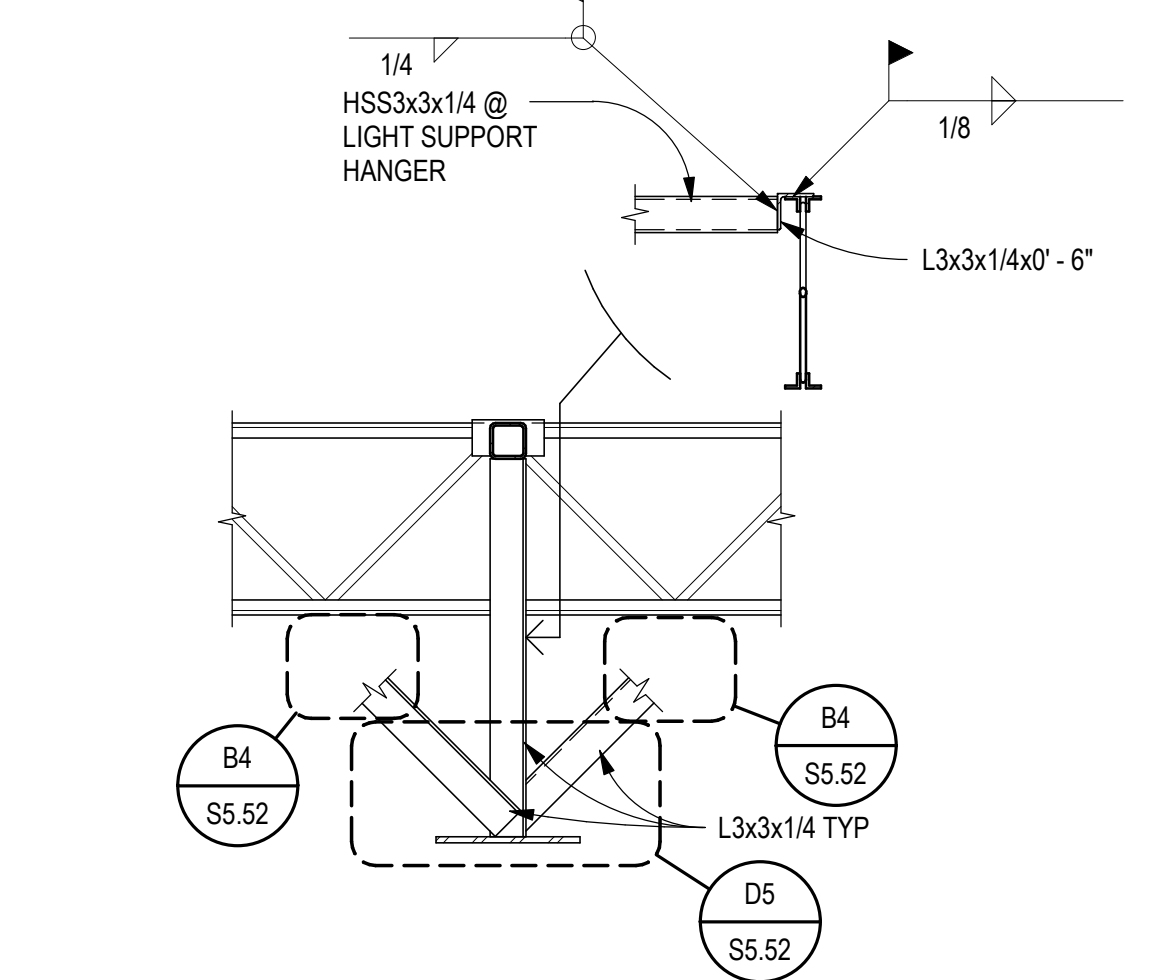
D4 CANOPY COLLECTOR BLOCKING
SCALE: 3/4" = 1'-0"



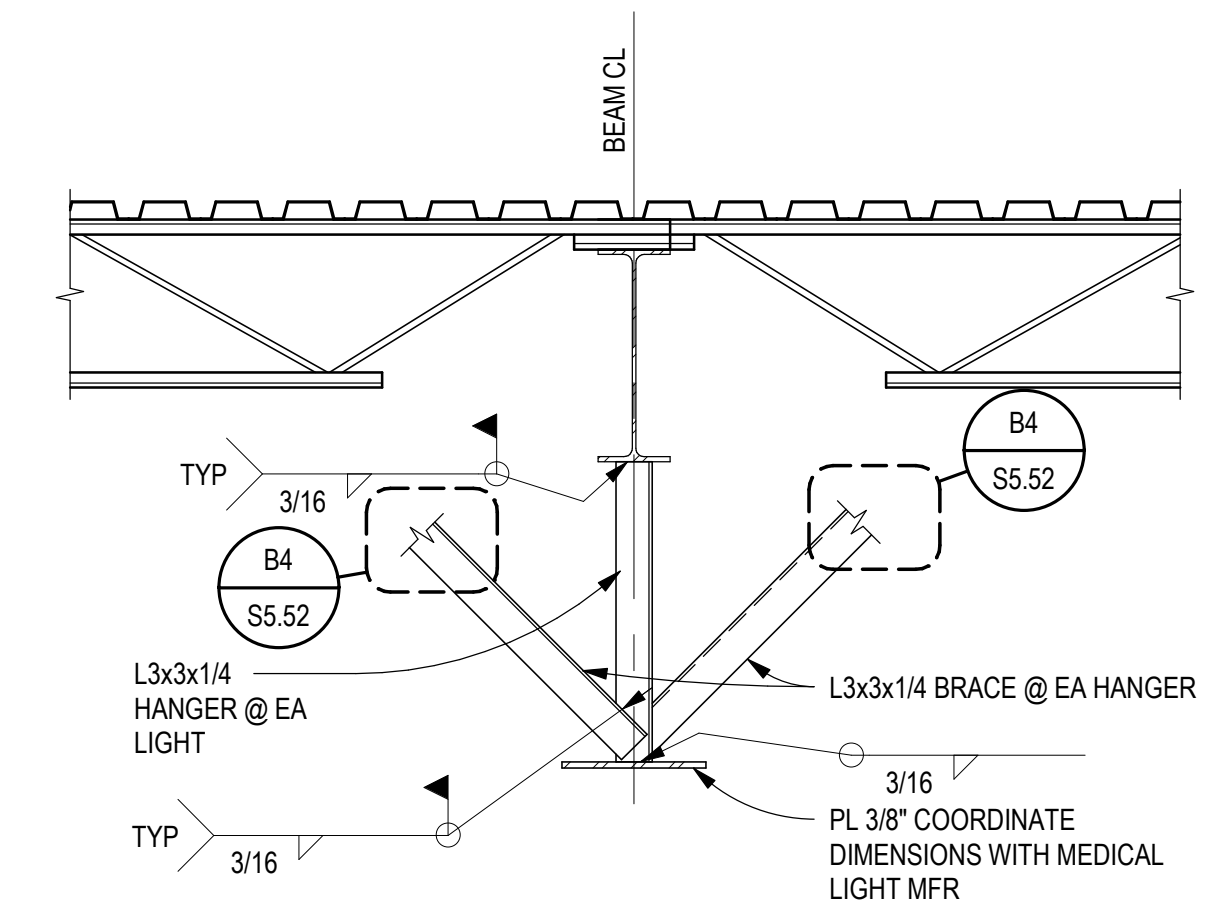
C4 SWING SUPPORT BETWEEN JOISTS
SCALE: 3/4" = 1'-0"



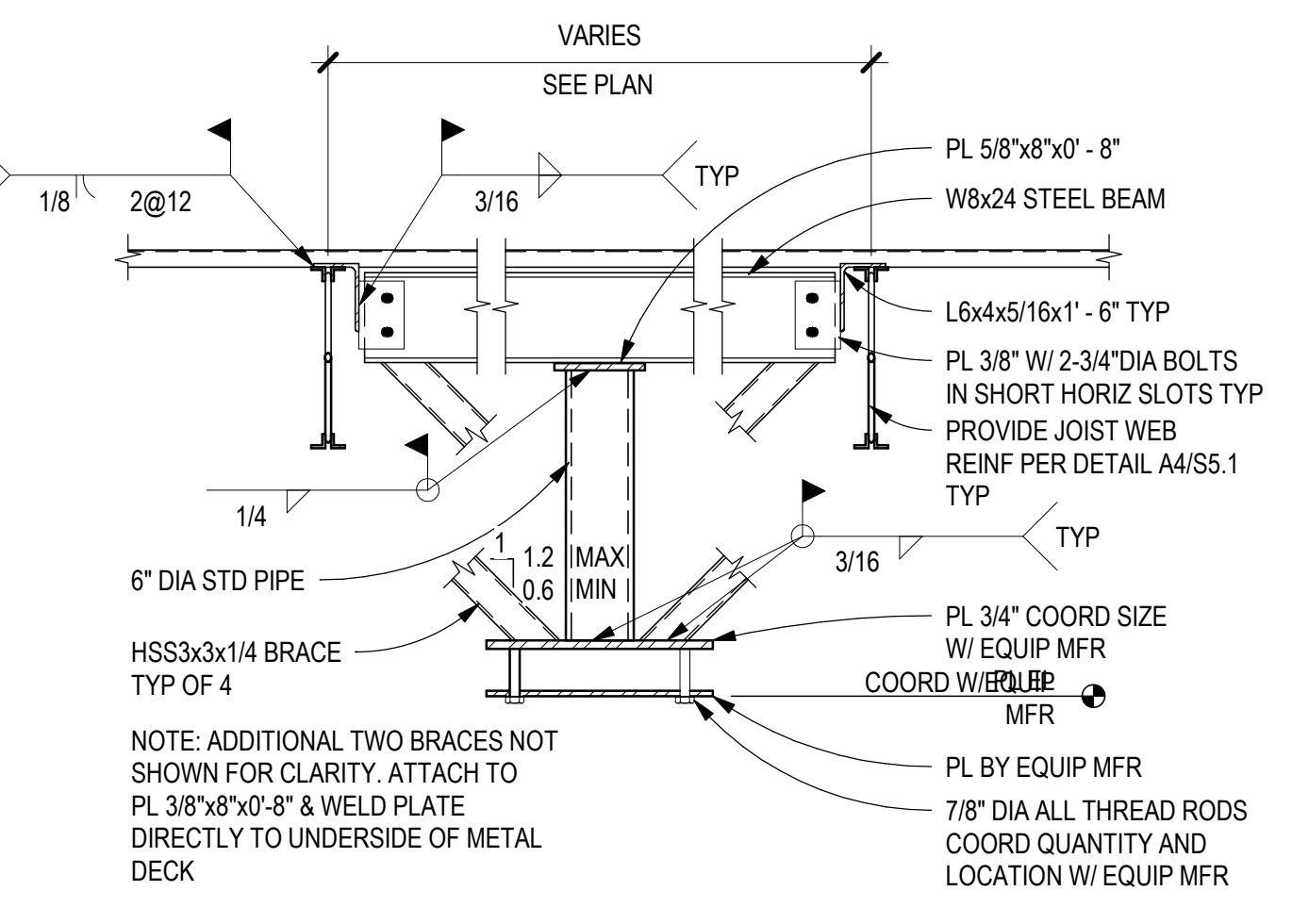
B4 LIGHT SUPPORT BETWEEN JOISTS
SCALE: 3/4" = 1'-0"



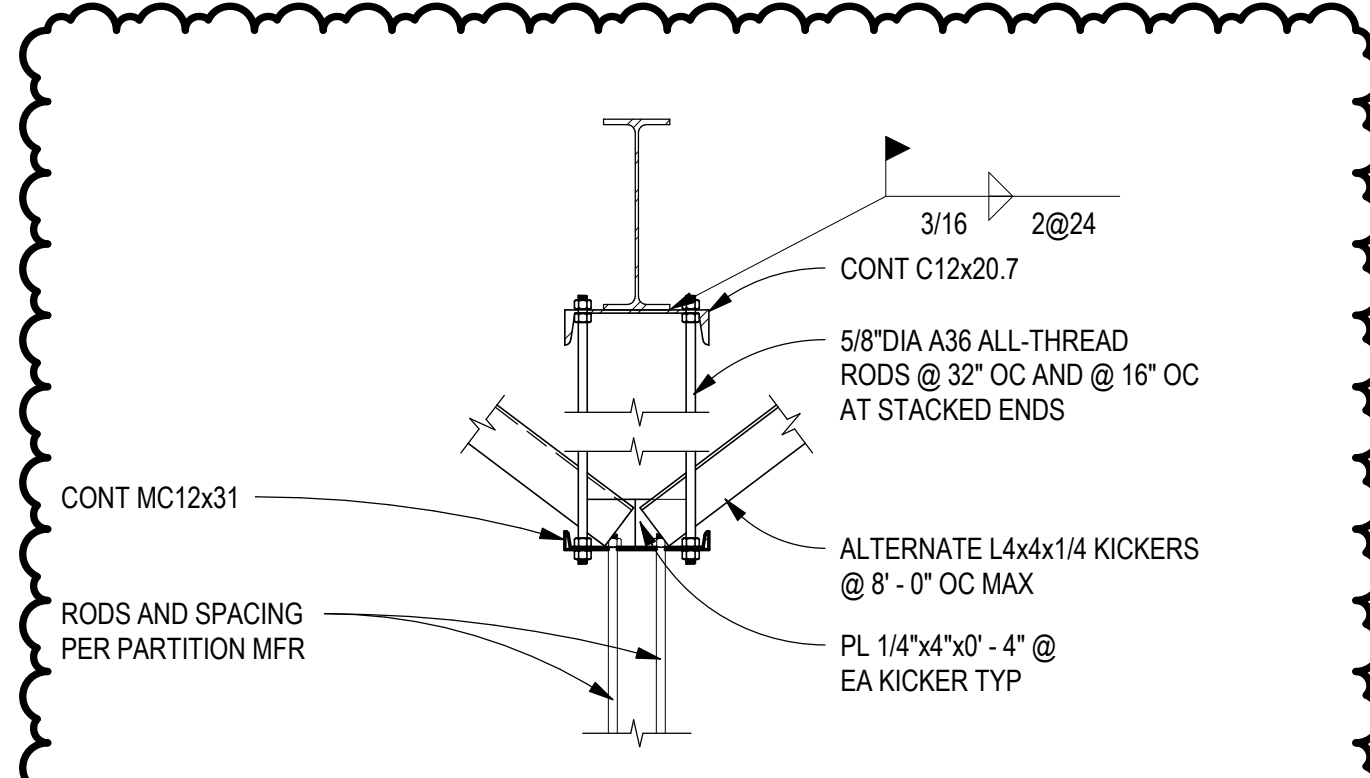
A4 LIGHT SUPPORT BETWEEN JOISTS
SCALE: 3/4" = 1'-0"



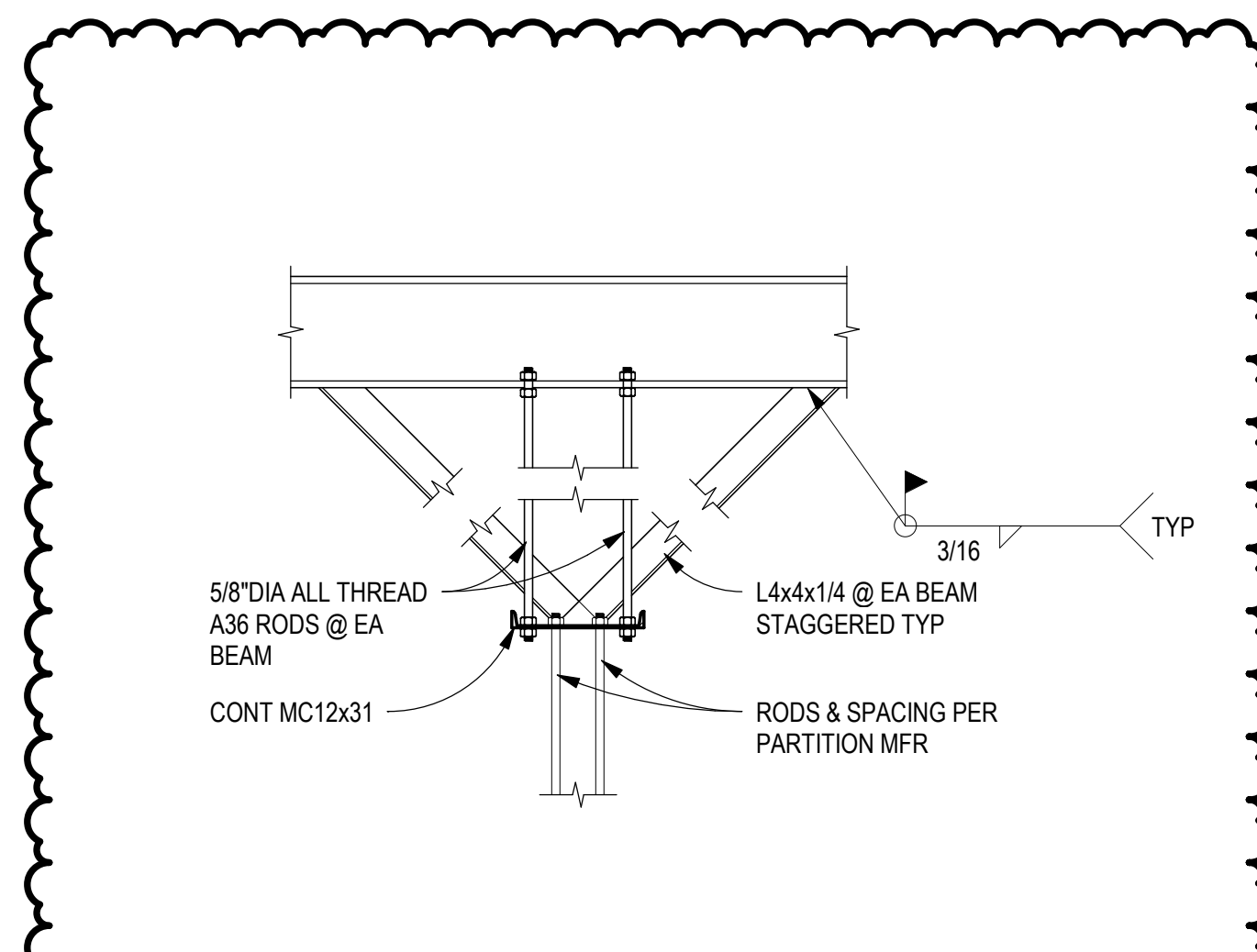
D5 MEDICAL LIGHT SUPPORT
SCALE: 3/4" = 1'-0"



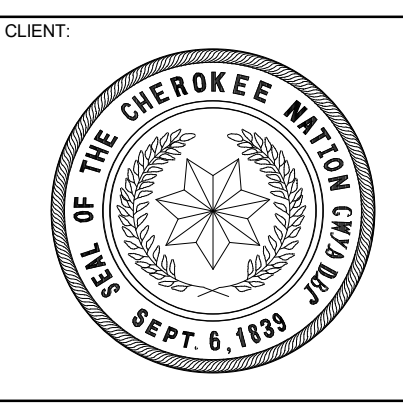
C5 SURGICAL LIGHT SUPPORT
SCALE: 3/4" = 1'-0"



B5 PARTITION SUPPORT SECTION
SCALE: 3/4" = 1'-0"



A5 PARTITION FRAMING SECTION
SCALE: 3/4" = 1'-0"



WILMA P. MANKILLER HEALTH CENTER EXPANSION
STILWELL, OKLAHOMA

KEY PLAN

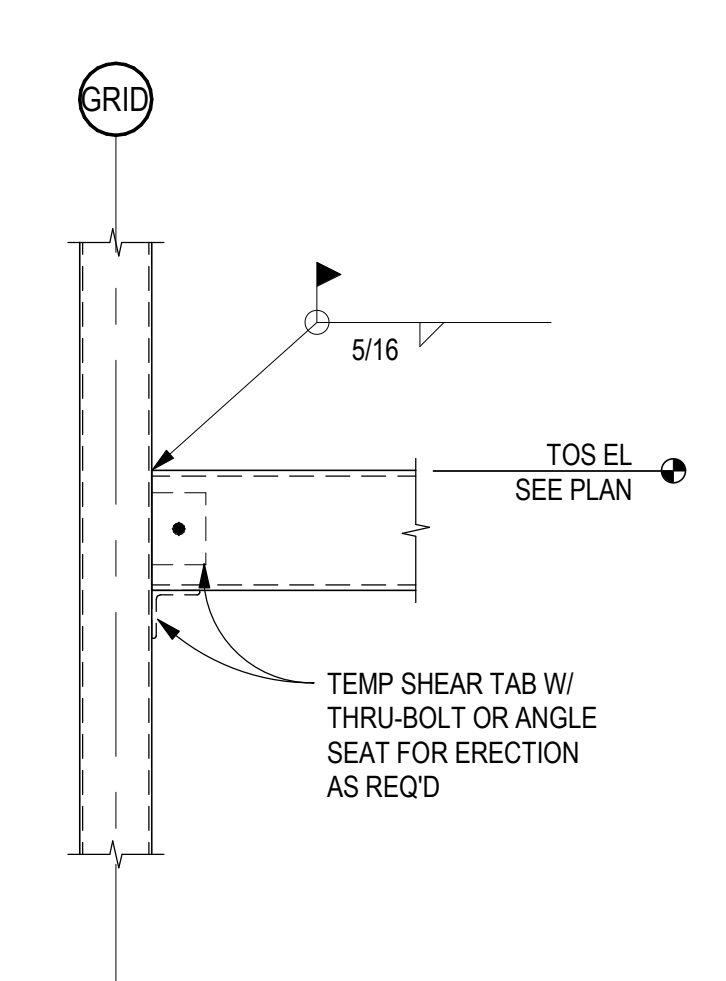
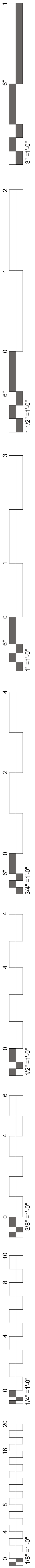
PROJECT PHASE
BID PACKAGE 01

#	DATE	REVISIONS
1	11/02/19	BID PACKAGE 01 - ADD 01

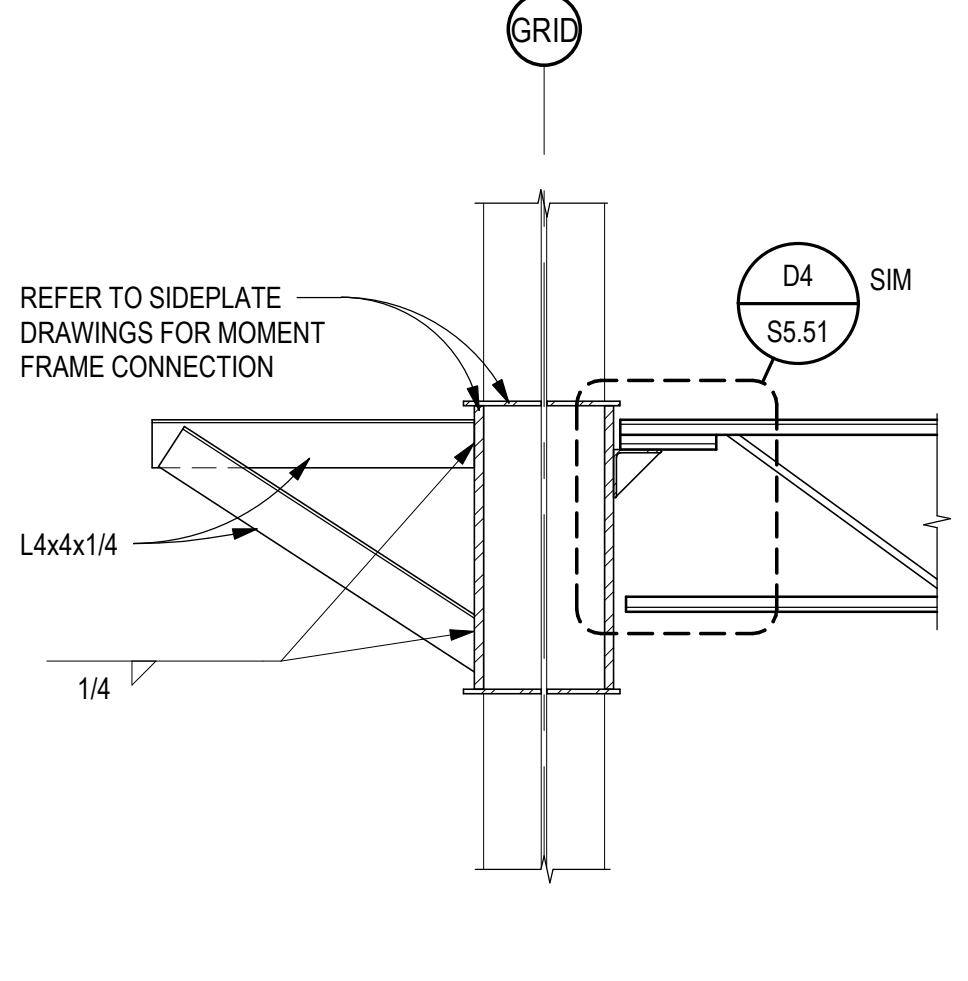
DATE: 11-01-19
JOB NUMBER: 18-01.01

SHEET NUMBER:
S5.52

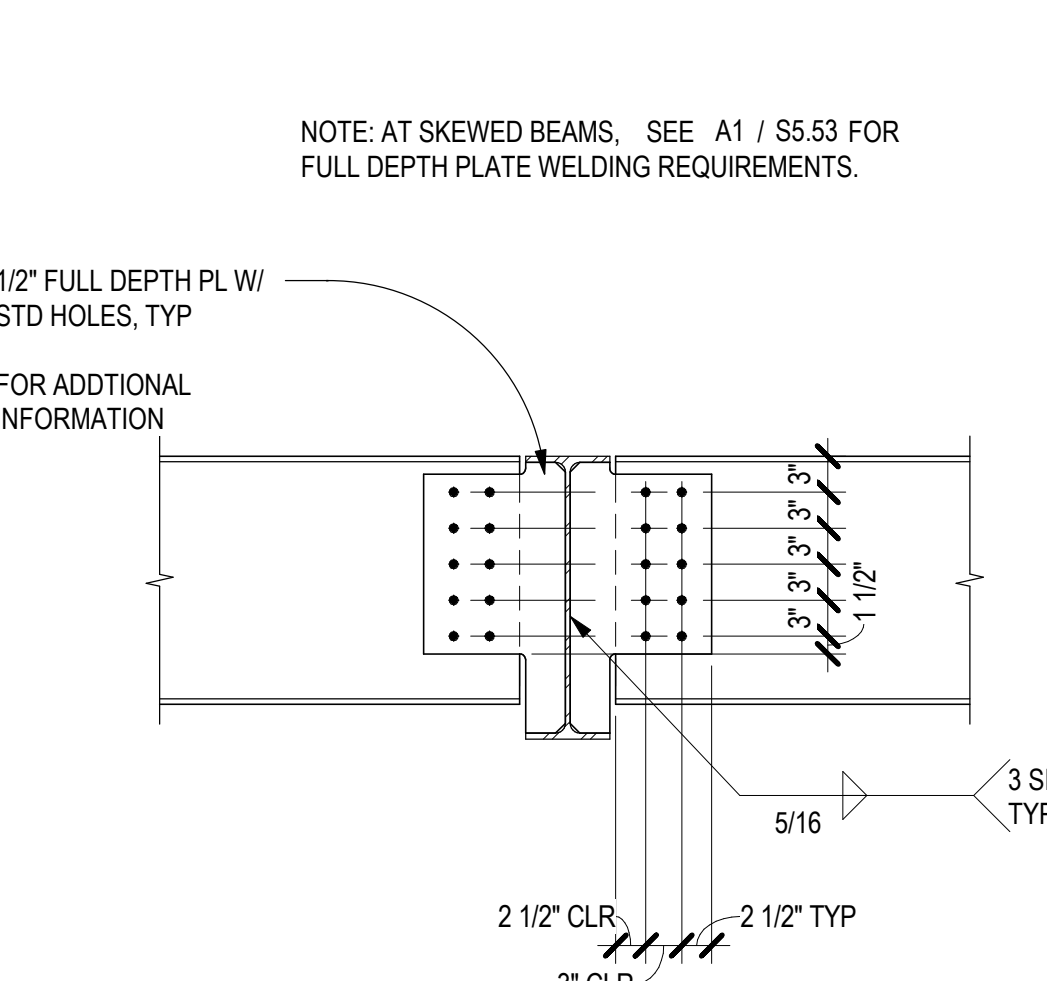
STEEL DETAILS



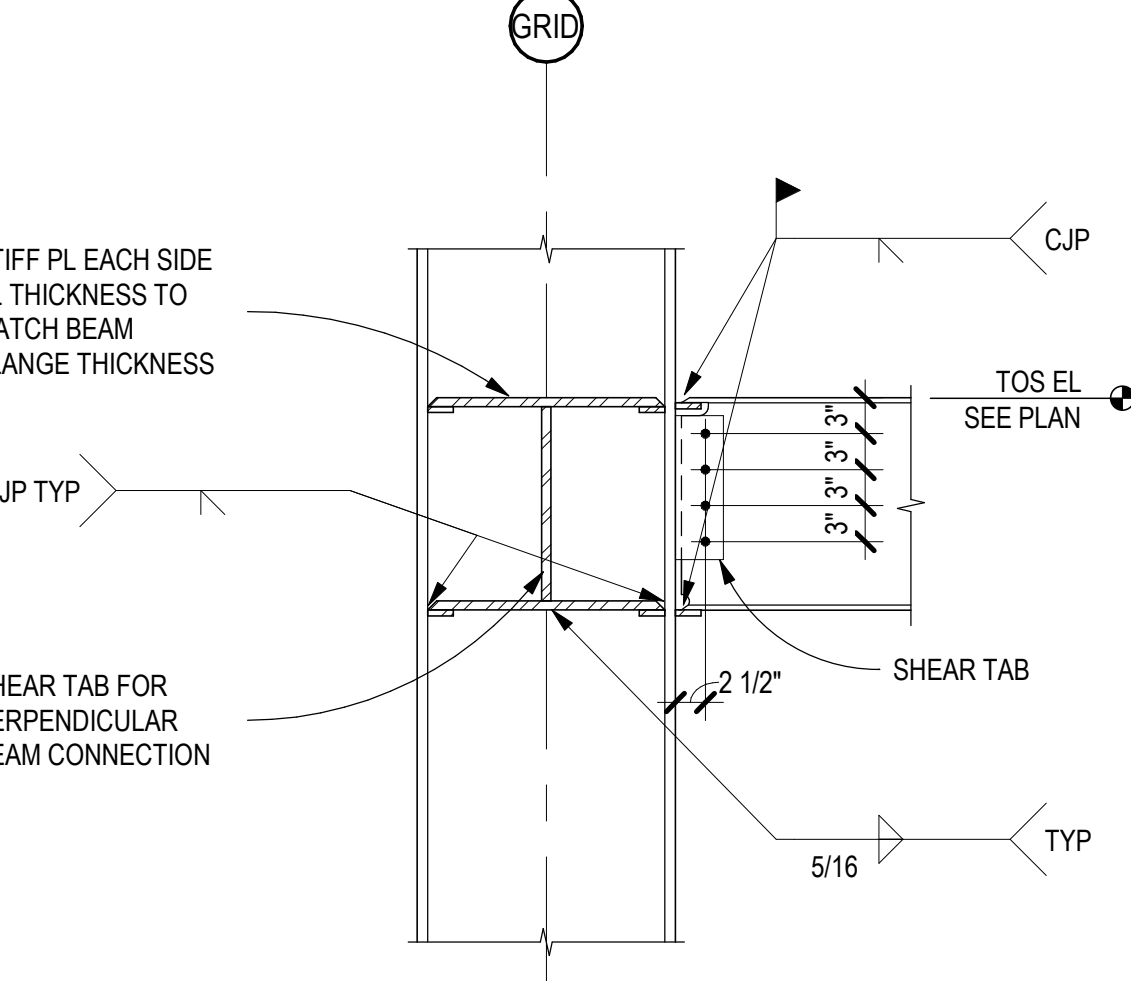
D1 HSS TO COLUMN WITH PARTIAL FIXITY
SCALE: 3/4" = 1'-0"



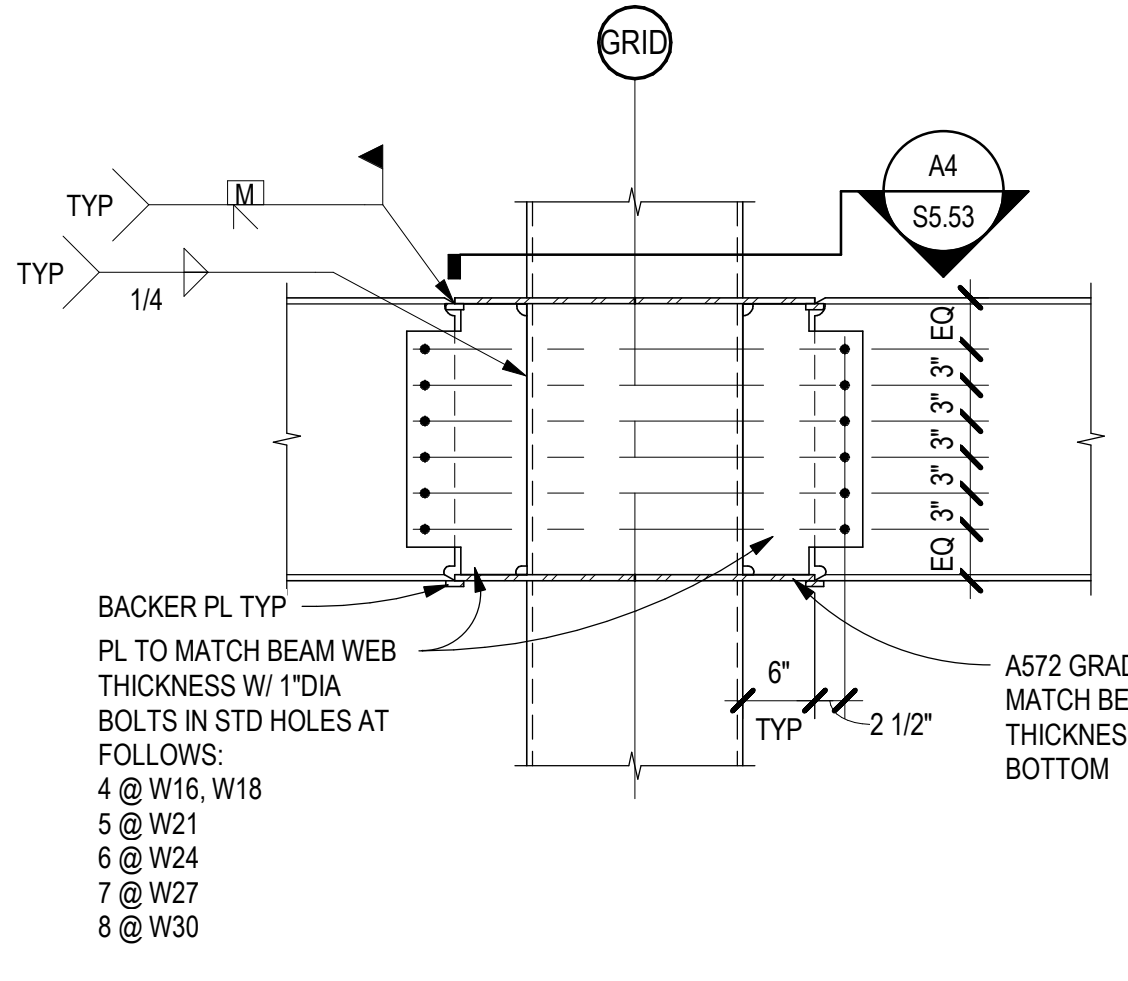
D2 BM TO COL WEB CONN AT SIDEPLATE
SCALE: 3/4" = 1'-0"



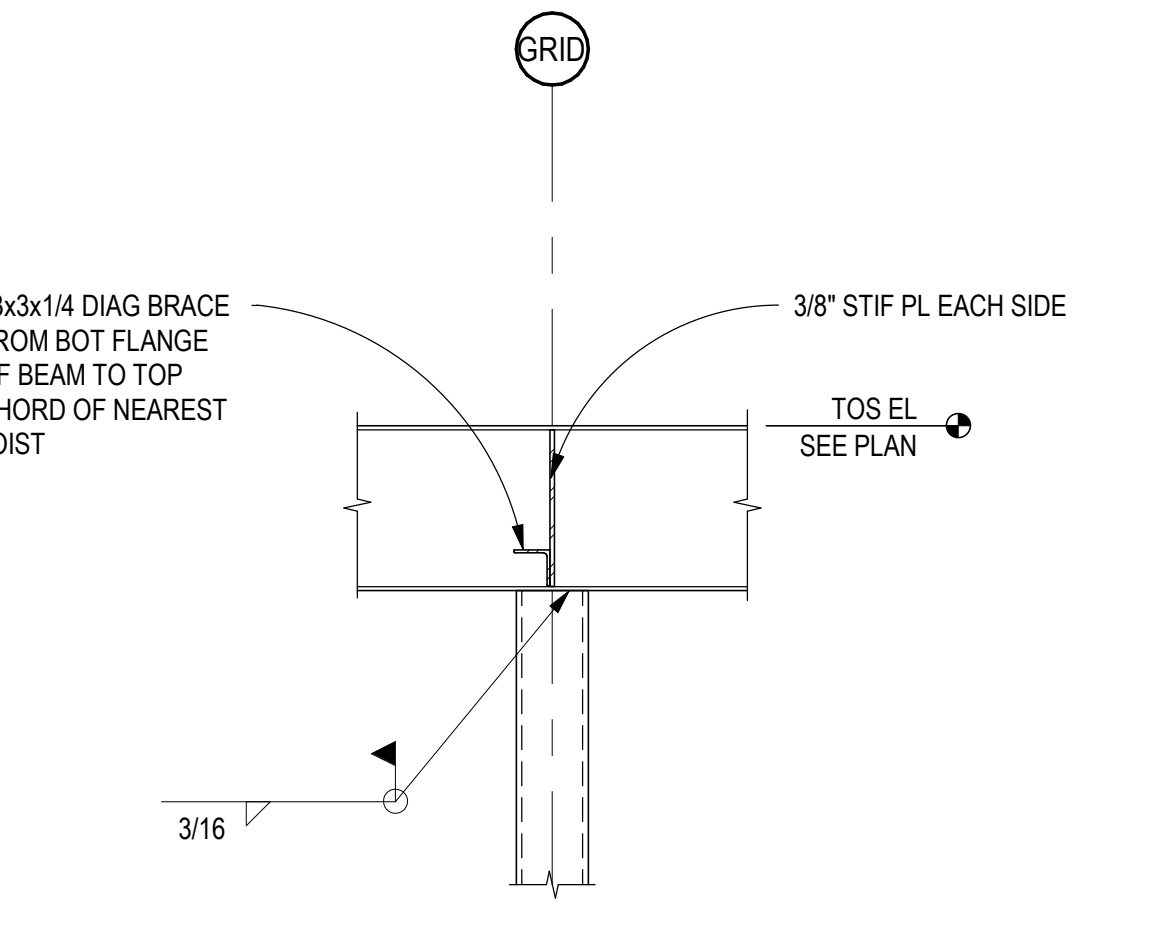
D3 BEAM TO BEAM COLLECTOR DETAIL
SCALE: 3/4" = 1'-0"



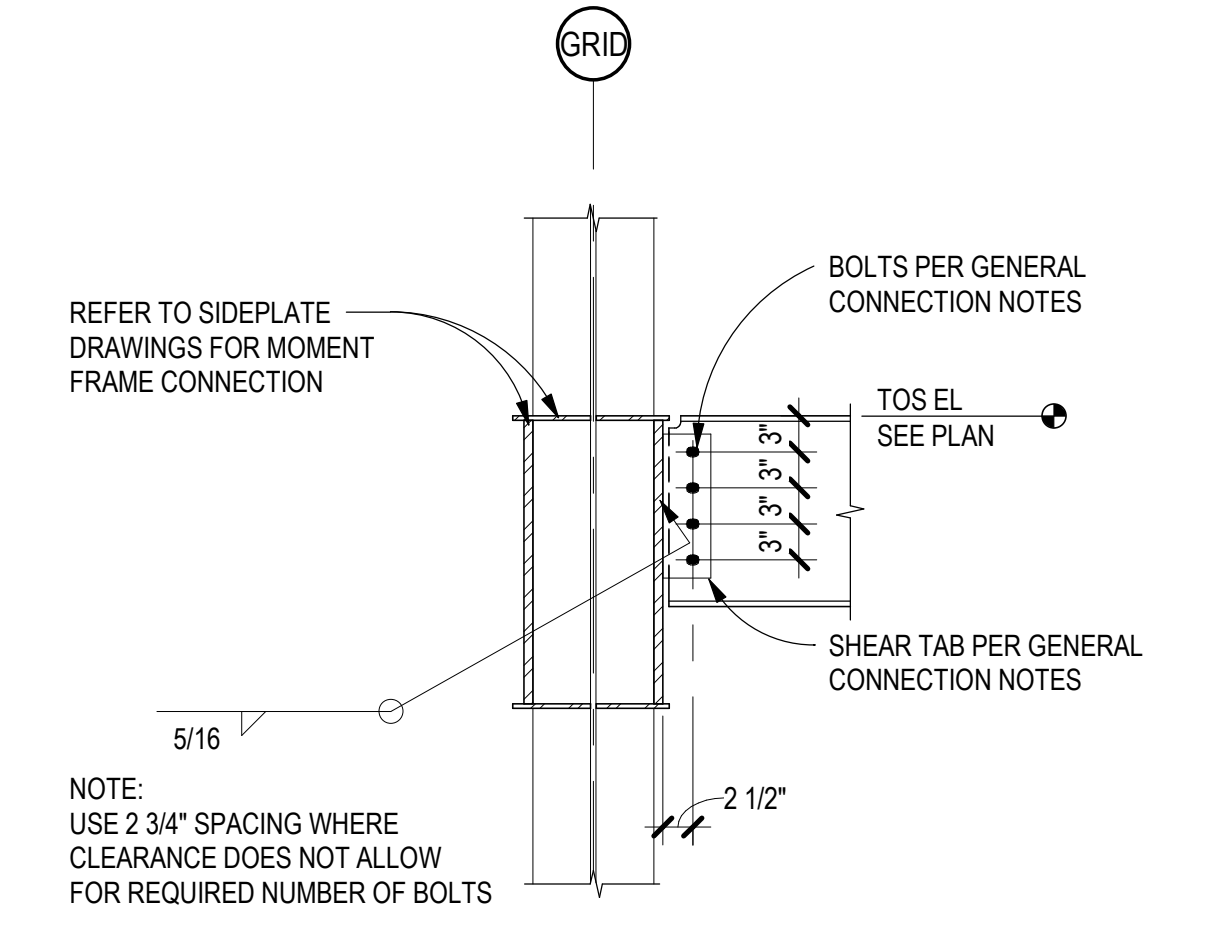
D4 BEAM TO COL FLANGE MOMENT CONN
SCALE: 3/4" = 1'-0"



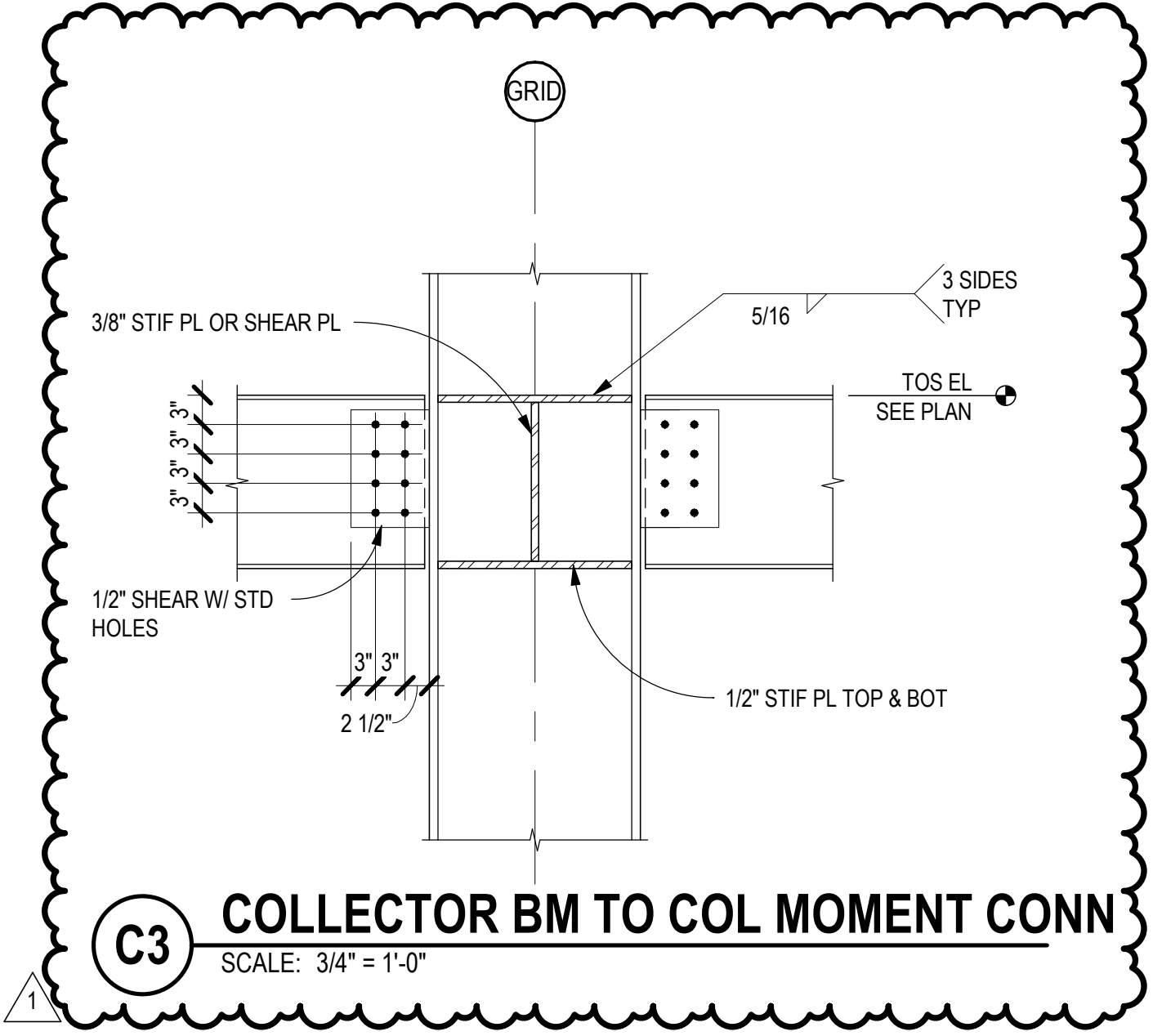
D5 TYPICAL FLANGE PLATE TO COLUMN
SCALE: 3/4" = 1'-0"



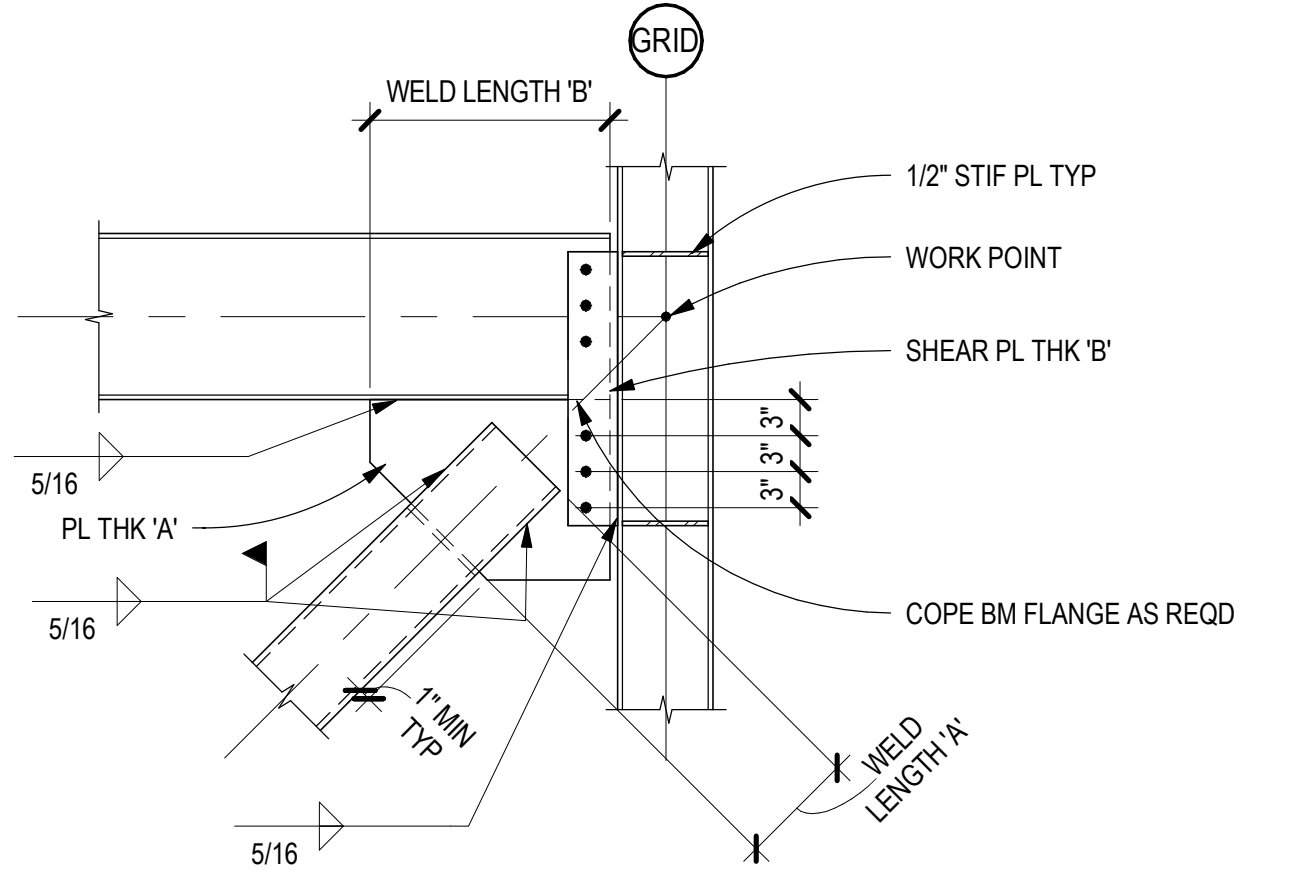
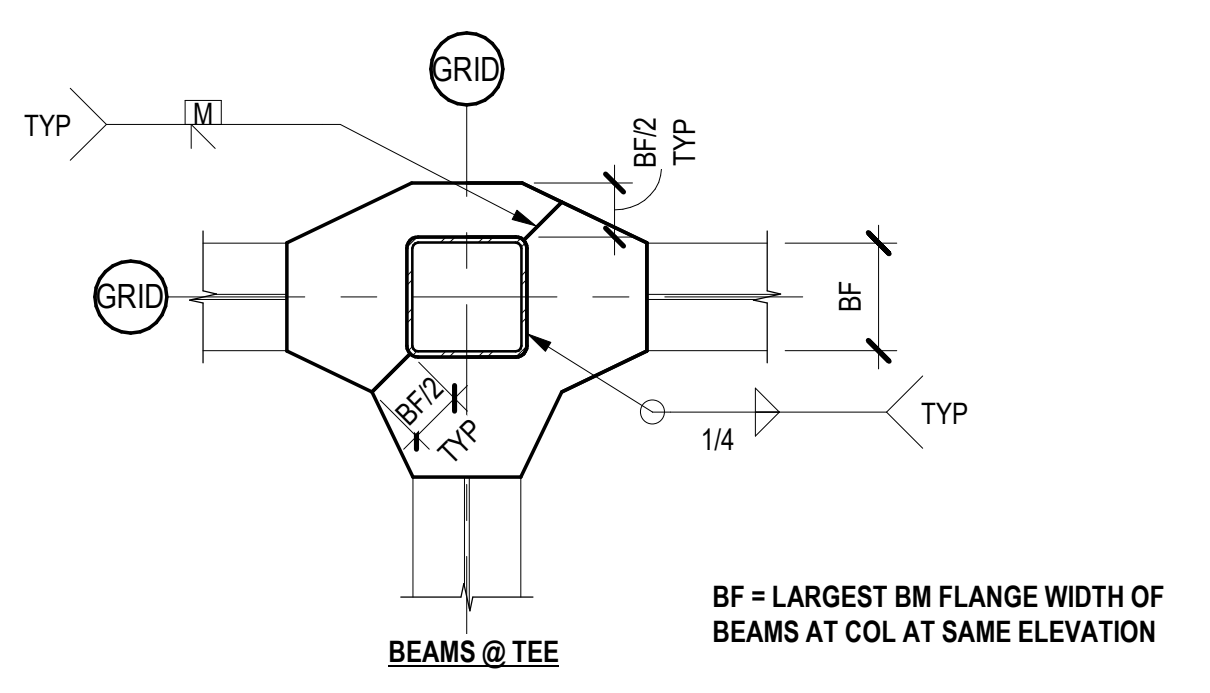
C1 BEAM OVER COLUMN CONN
SCALE: 3/4" = 1'-0"



C2 BM TO COL WEB CONN AT SIDEPLATE
SCALE: 3/4" = 1'-0"



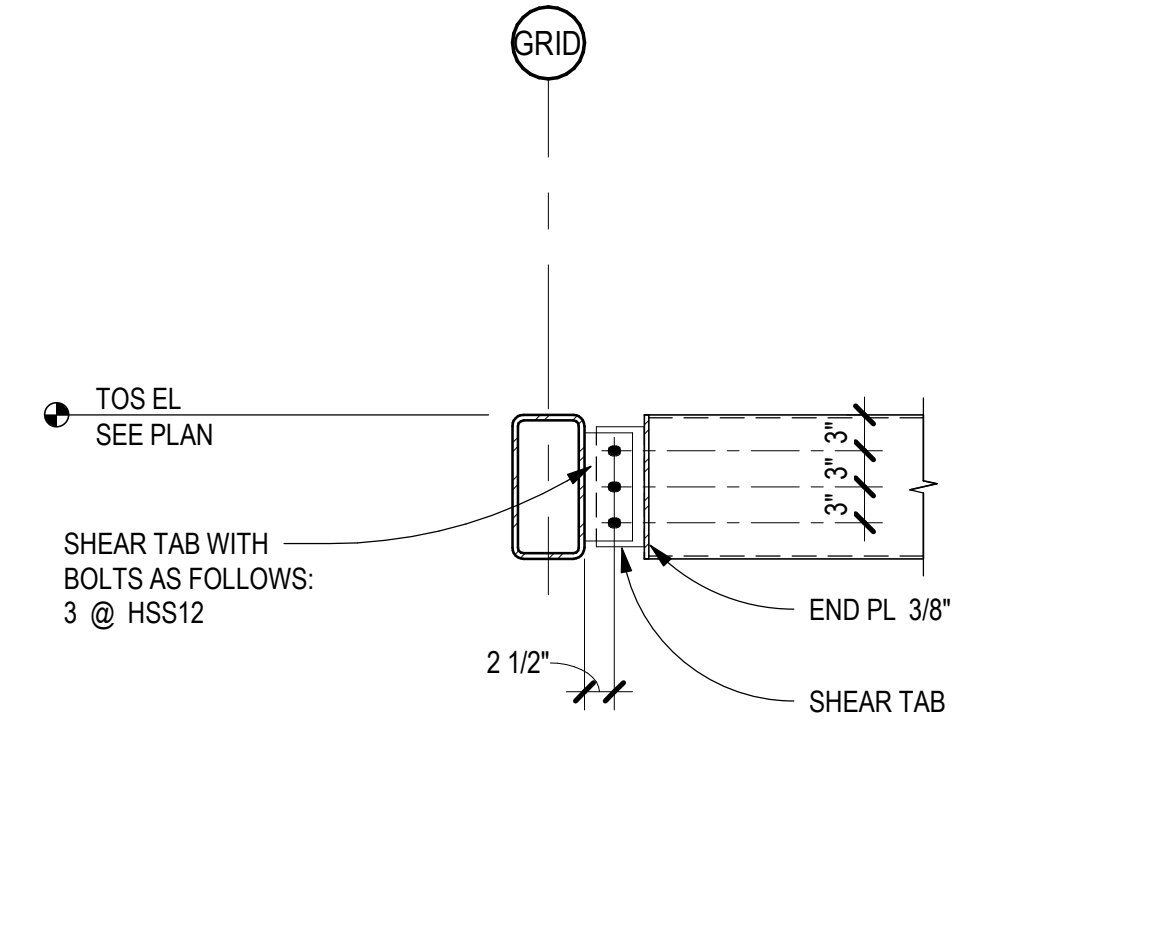
C3 COLLECTOR BM TO COL MOMENT CONN
SCALE: 3/4" = 1'-0"



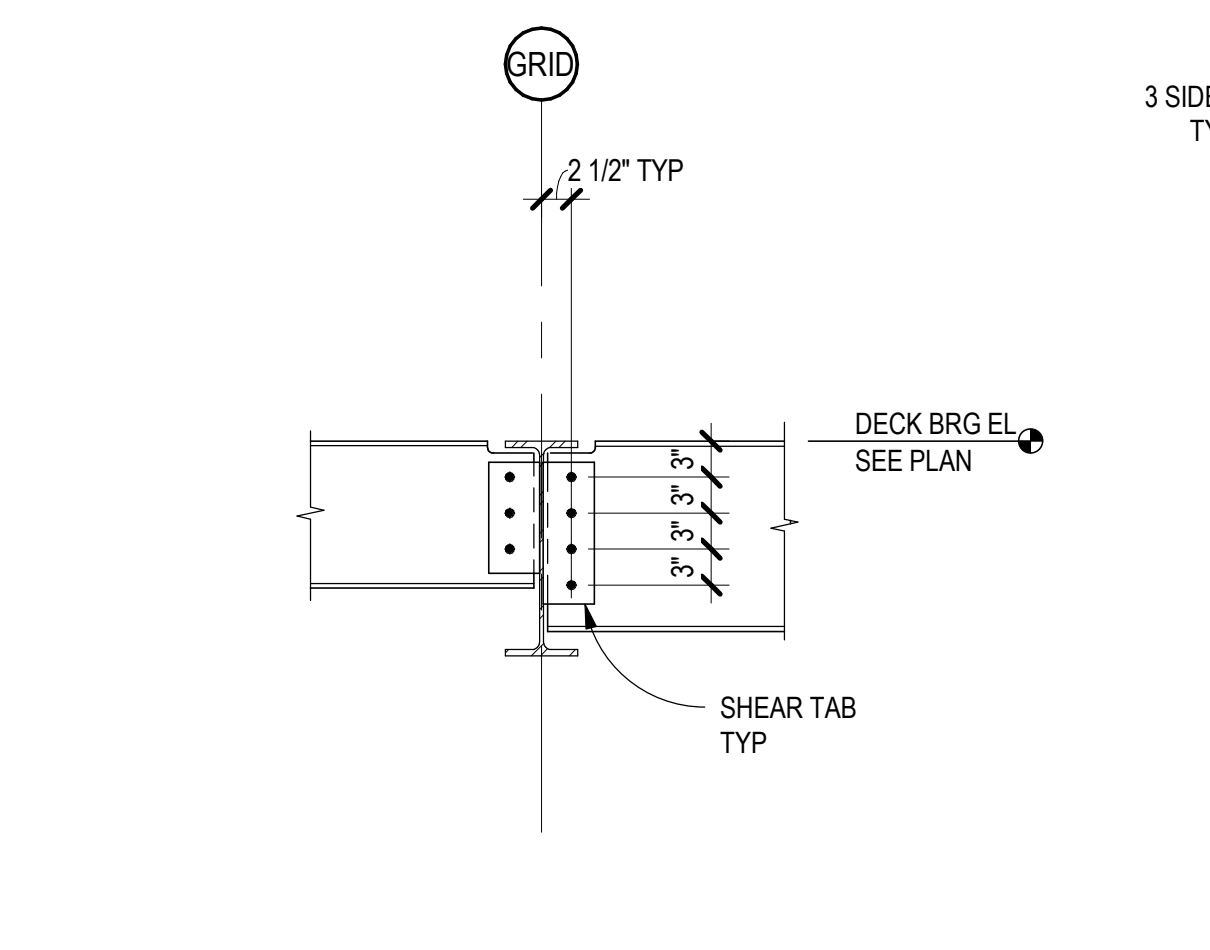
CONNECTION SCHEDULE					
BRACE SIZE	PL THK 'A'	THRU PL THK 'B'	WELD LENGTH 'A'	WELD LENGTH 'B'	GUSSET PL BOLTS
HSS4x4	5/8"	5/8"	4"	20"	3
HSS5x5	5/8"	5/8"	5"	20"	3
HSS6x6	5/8"	5/8"	6"	22"	3
HSS8x8	3/4"	3/4"	8"	24"	4
HSS12x8	1"	1"	12"	28"	5

NOTES:
1. LENGTHS GIVEN ARE SINGLE-SIDE LENGTHS AND MINIMUM LENGTHS
2. LONGER GUSSET/WELD LENGTHS MAY BE REQ'D WHERE BRACE SLOPE VARIES FROM 1:1
3. ALL BOLTS IN STANDARD HOLES

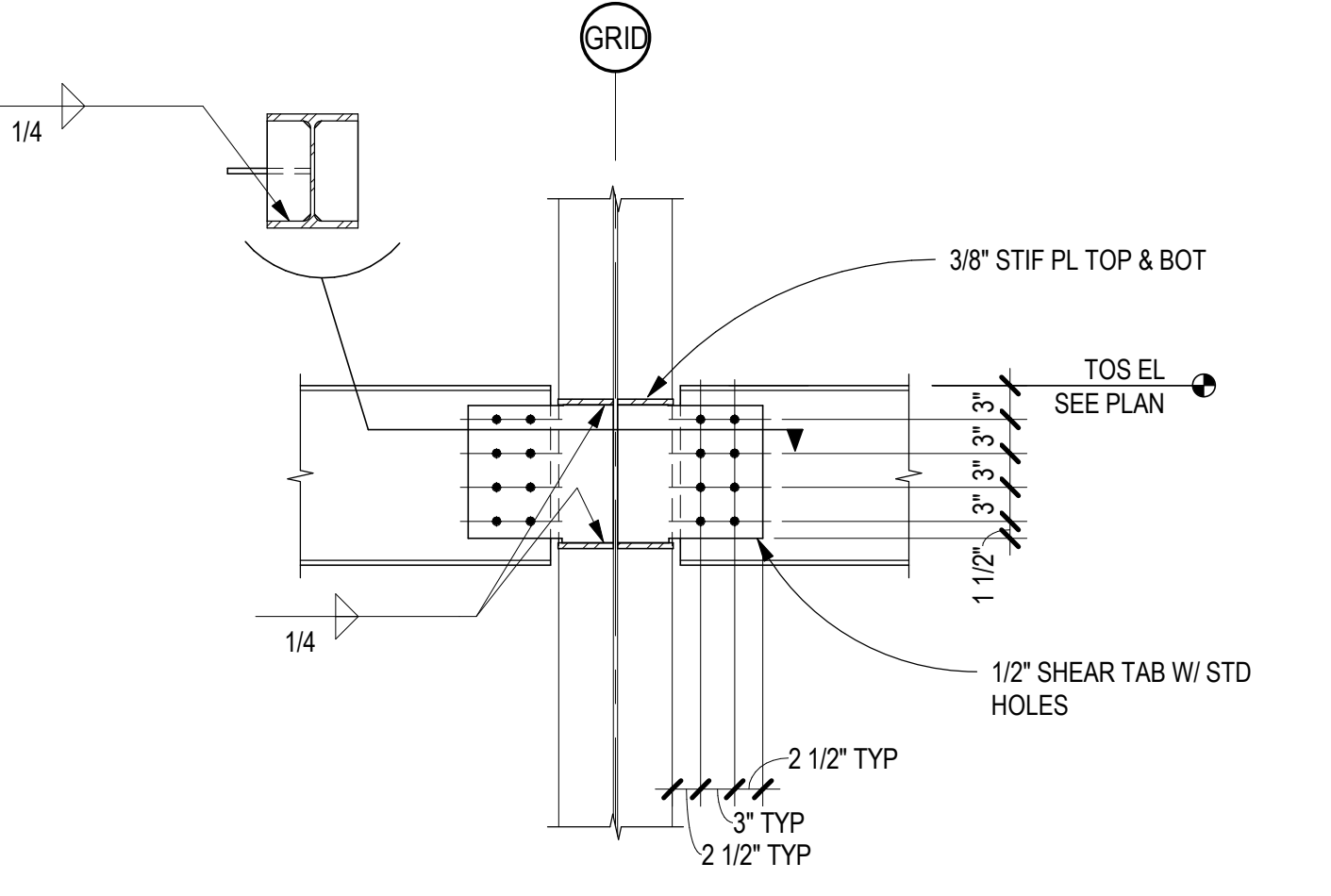
B5 BRACED FRAME CONNECTION
SCALE: 3/4" = 1'-0"



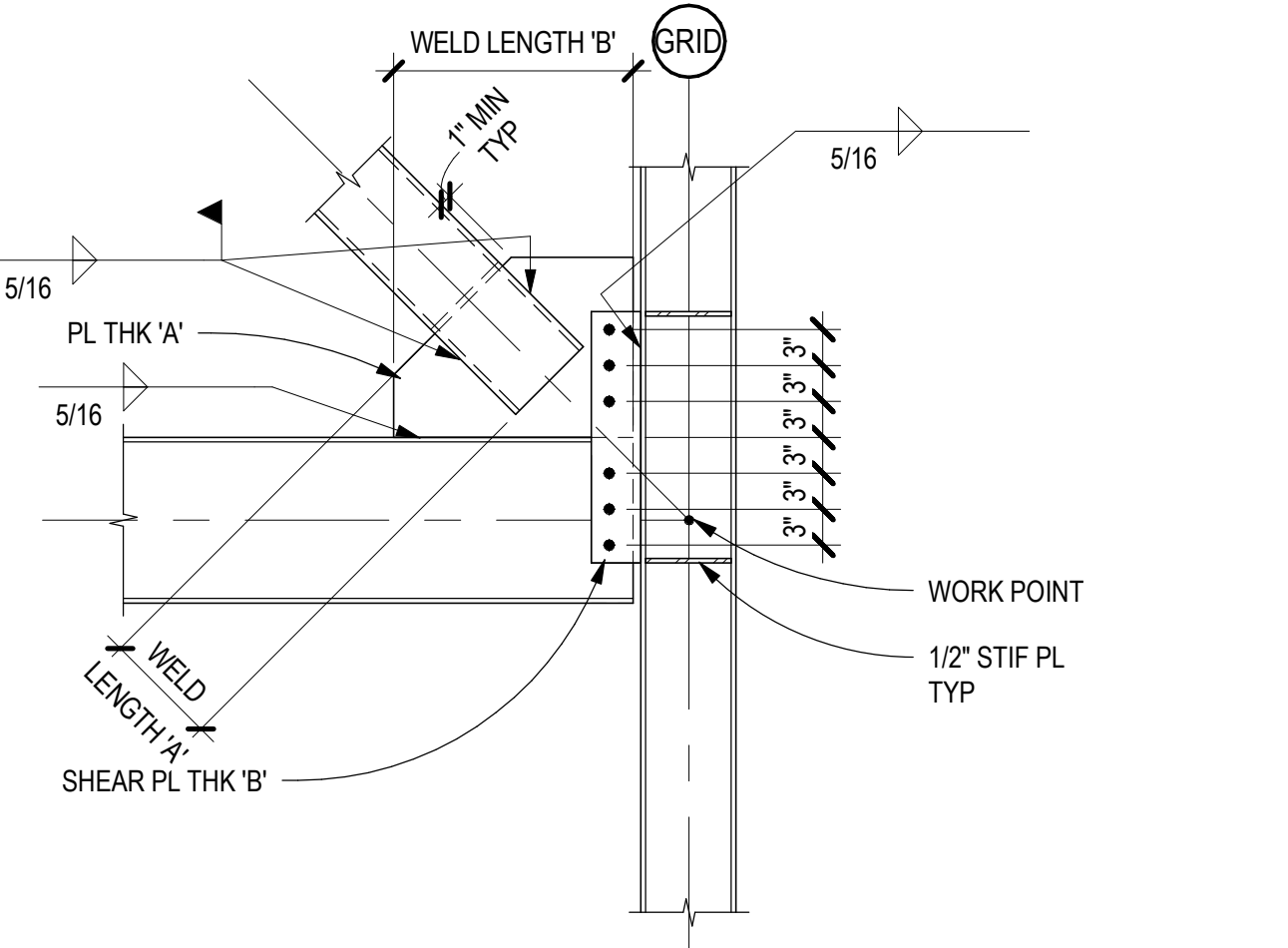
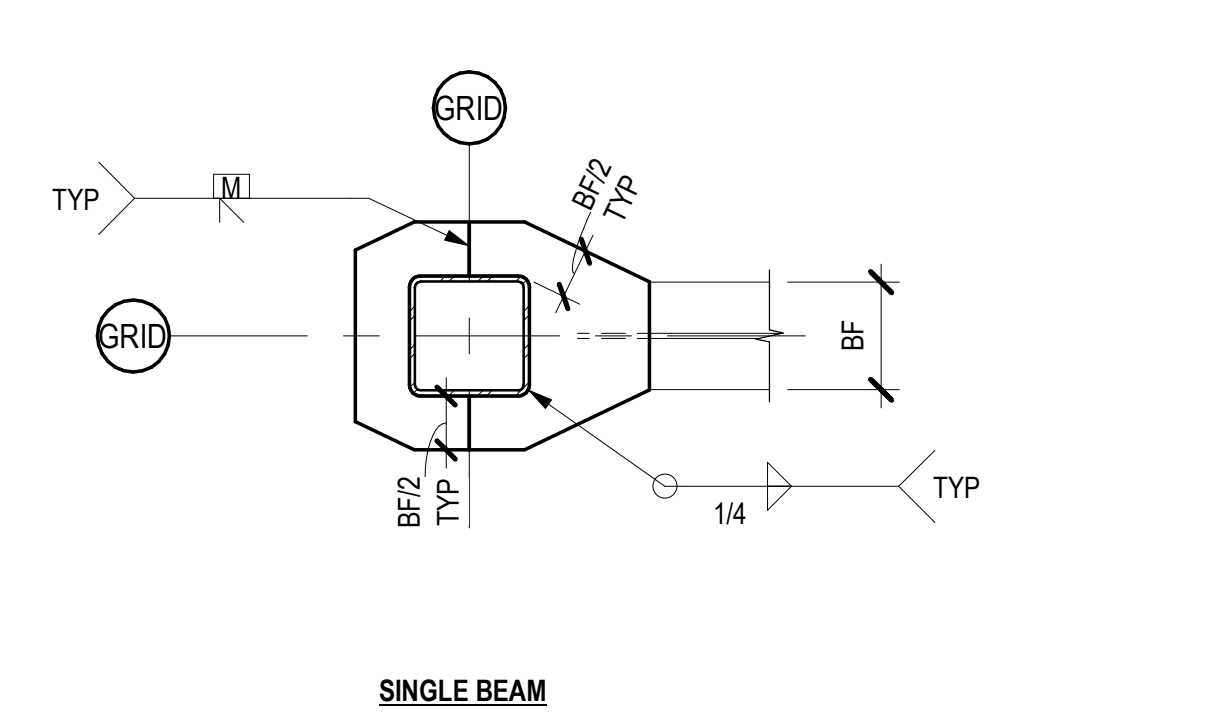
B1 BEAM TO BEAM CONN
SCALE: 3/4" = 1'-0"



B2 TYPICAL BEAM TO BEAM CONN
SCALE: 3/4" = 1'-0"



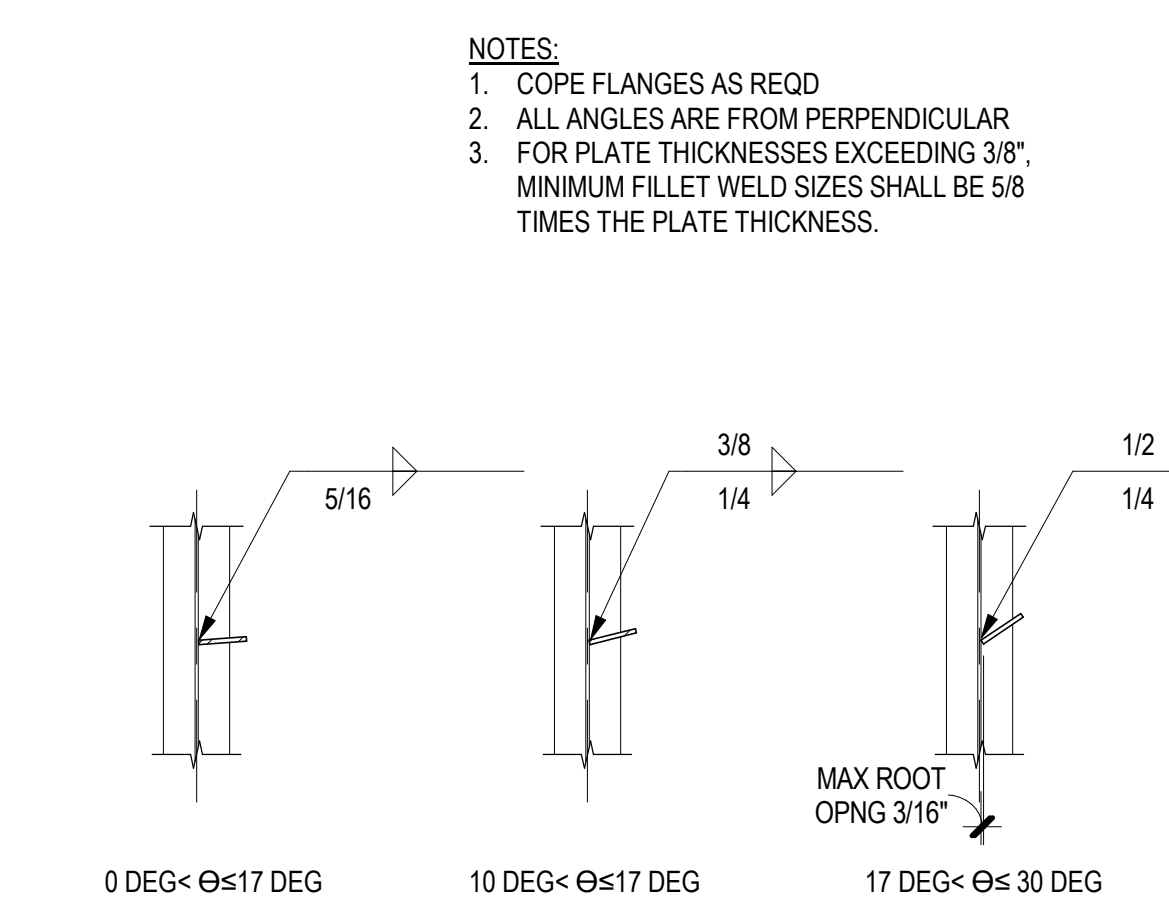
B3 COLLECTOR BM TO COL WEB CONN
SCALE: 3/4" = 1'-0"



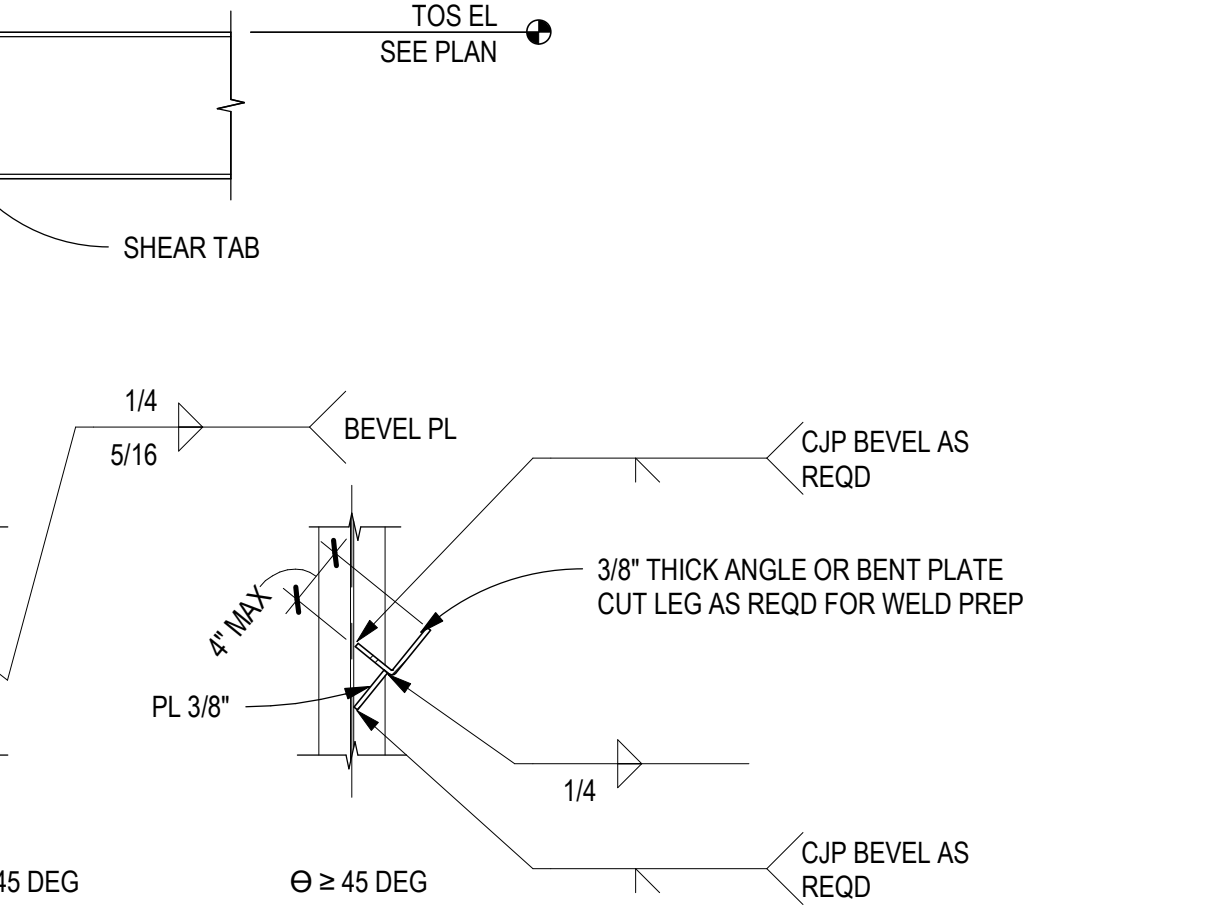
CONNECTION SCHEDULE					
BRACE SIZE	PL THK 'A'	THRU PL THK 'B'	WELD LENGTH 'A'	WELD LENGTH 'B'	GUSSET PL BOLTS
HSS4x4	5/8"	5/8"	4"	20"	3
HSS5x5	5/8"	5/8"	5"	20"	3
HSS6x6	5/8"	5/8"	6"	22"	3
HSS8x8	3/4"	3/4"	8"	24"	4
HSS12x8	1"	1"	12"	28"	5

NOTES:
1. LENGTHS GIVEN ARE SINGLE-SIDE LENGTHS AND MINIMUM LENGTHS
2. LONGER GUSSET/WELD LENGTHS MAY BE REQ'D WHERE BRACE SLOPE VARIES FROM 1:1
3. ALL BOLTS IN STANDARD HOLES

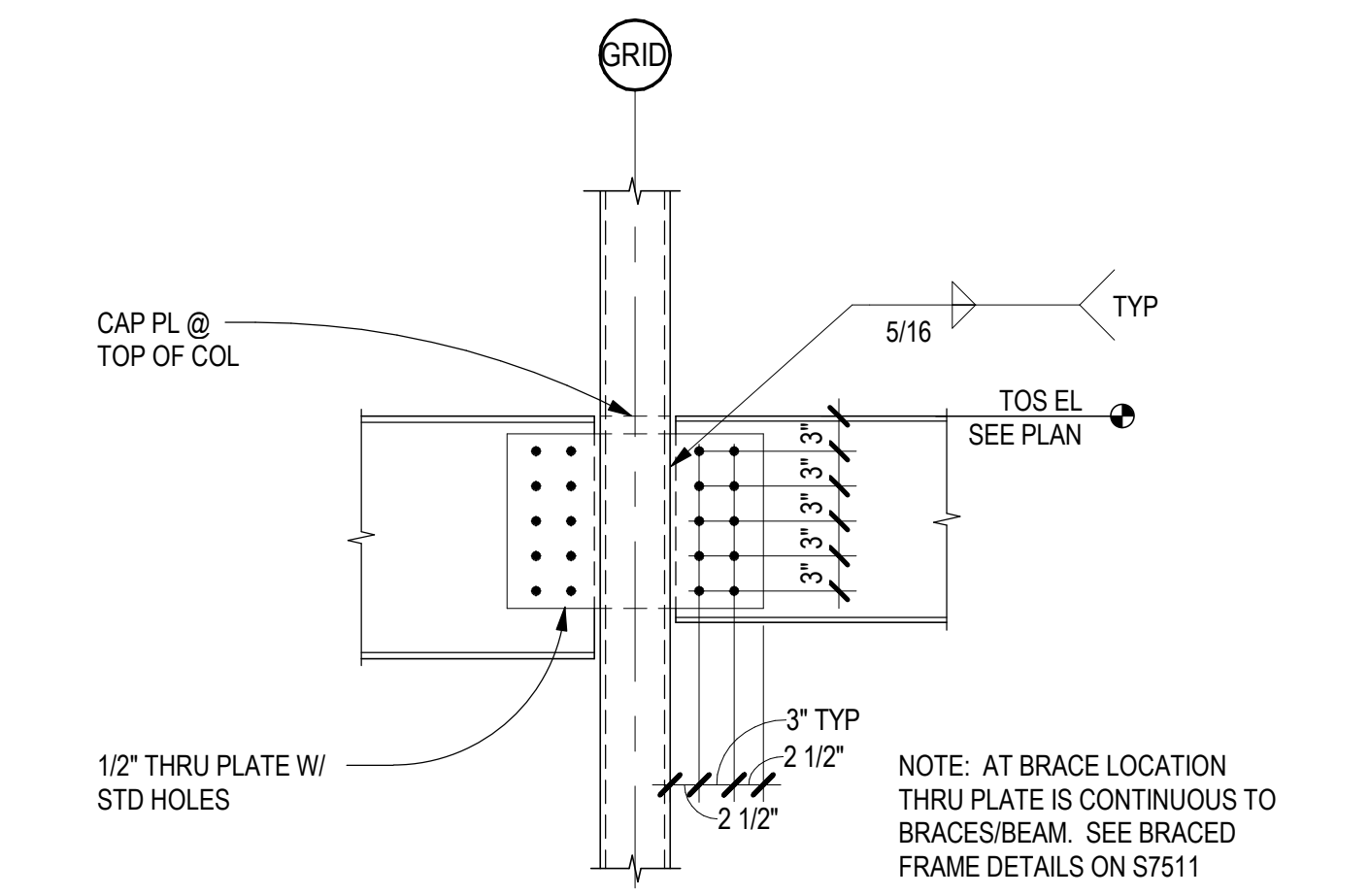
A5 BRACED FRAME CONNECTION
SCALE: 3/4" = 1'-0"



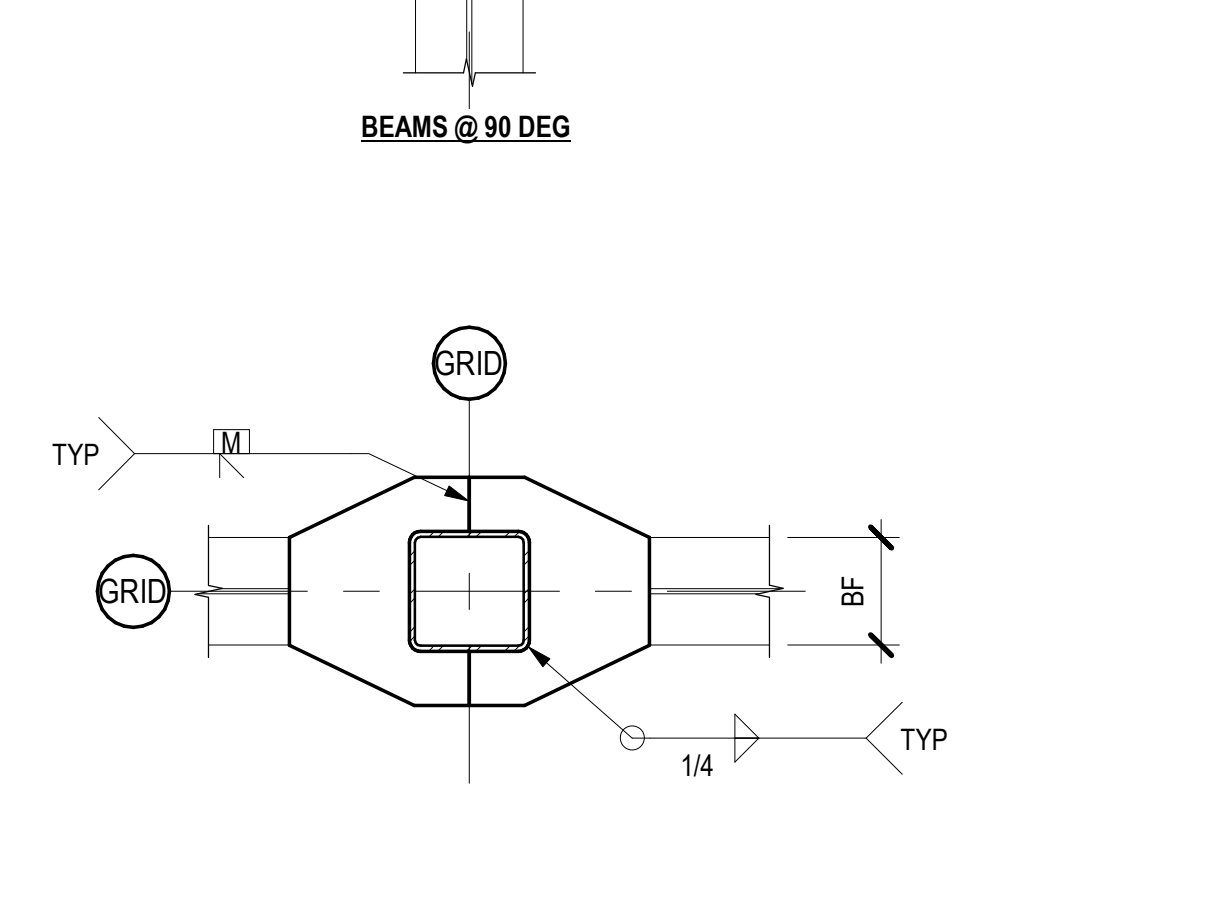
A1 SKEWED BEAM TO BEAM CONN
SCALE: 3/4" = 1'-0"



A3 COLLECTOR BEAM TO COLUMN CONN
SCALE: 3/4" = 1'-0"



A3 COLLECTOR BEAM TO COLUMN CONN
SCALE: 3/4" = 1'-0"

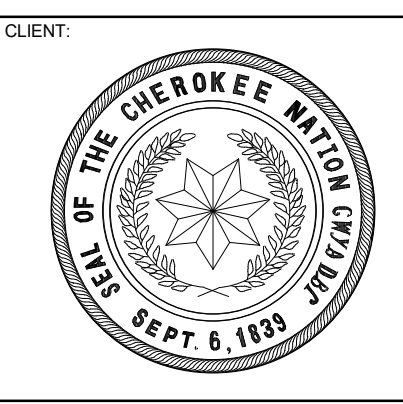


A4 FLANGE PLATE CONN AT HSS COL
SCALE: 3/4" = 1'-0"

NOTES:
1. LENGTHS GIVEN ARE SINGLE-SIDE LENGTHS AND MINIMUM LENGTHS
2. LONGER GUSSET/WELD LENGTHS MAY BE REQ'D WHERE BRACE SLOPE VARIES FROM 1:1
3. ALL BOLTS IN STANDARD HOLES



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45 South 4th Street
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479-783-2460
www.childersarchitect.com

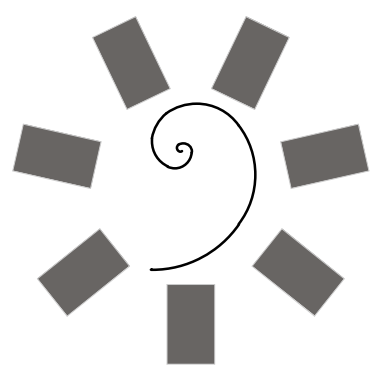
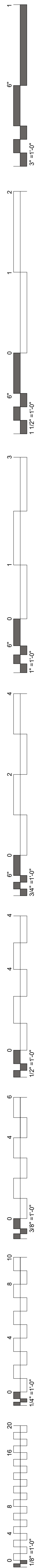


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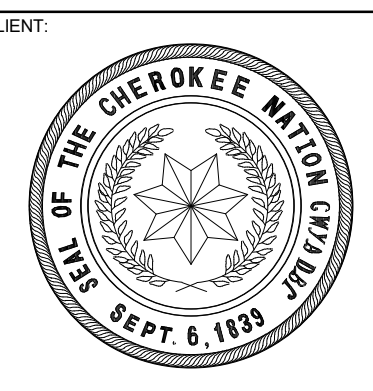
PROJECT PHASE:
BID PACKAGE 01

#	DATE	REVISIONS
1	11/02/19	BID PACKAGE 01 - ADD 01

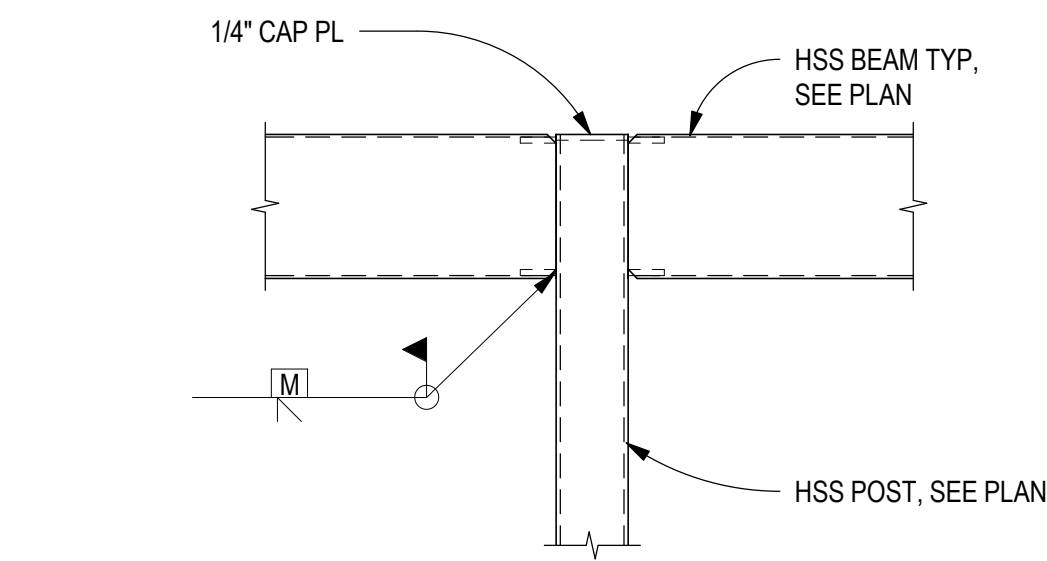
DATE: 11-01-19
JOB NUMBER: 18-01.01
SHEET NUMBER: S5.53
STEEL DETAILS



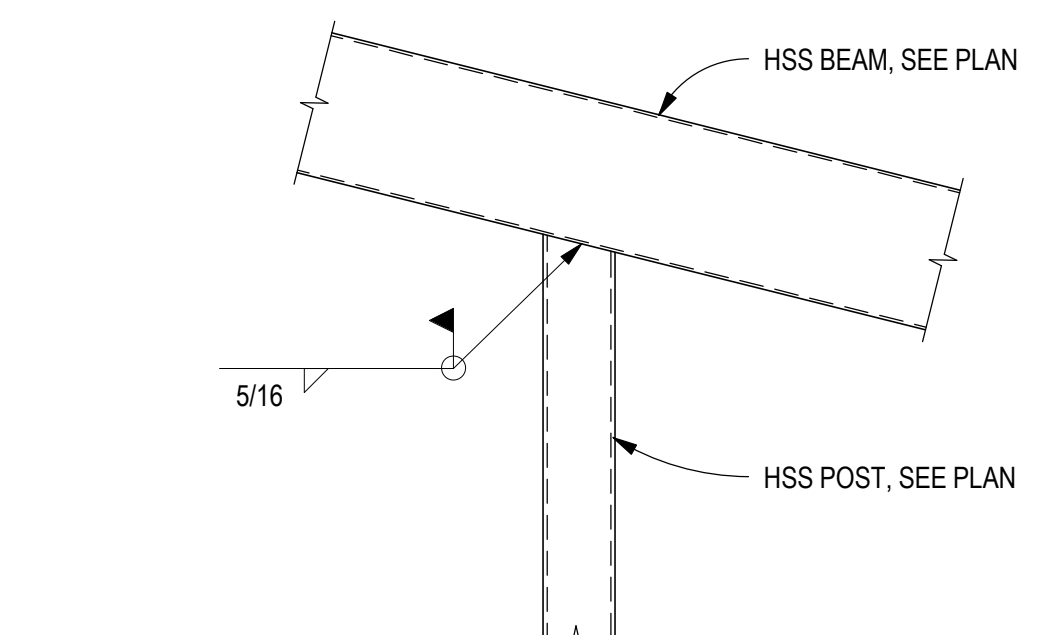
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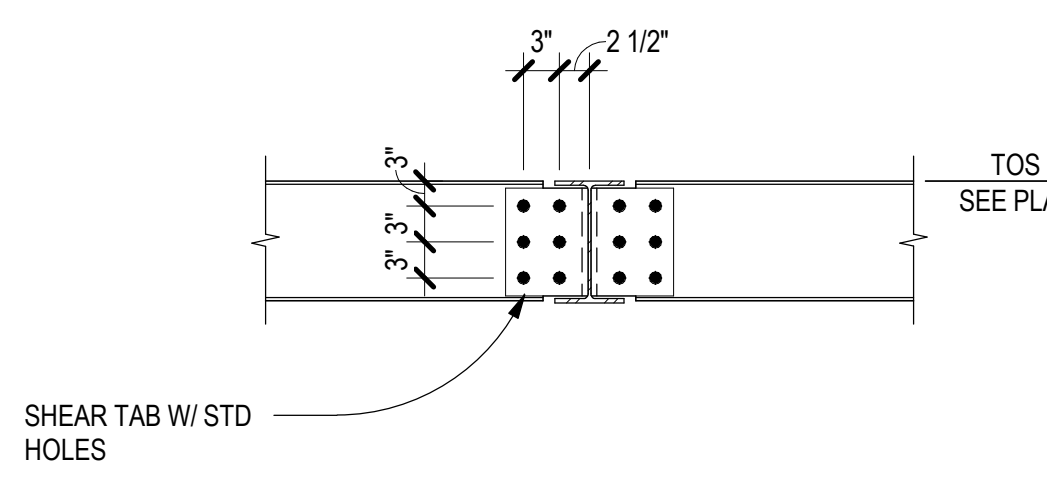
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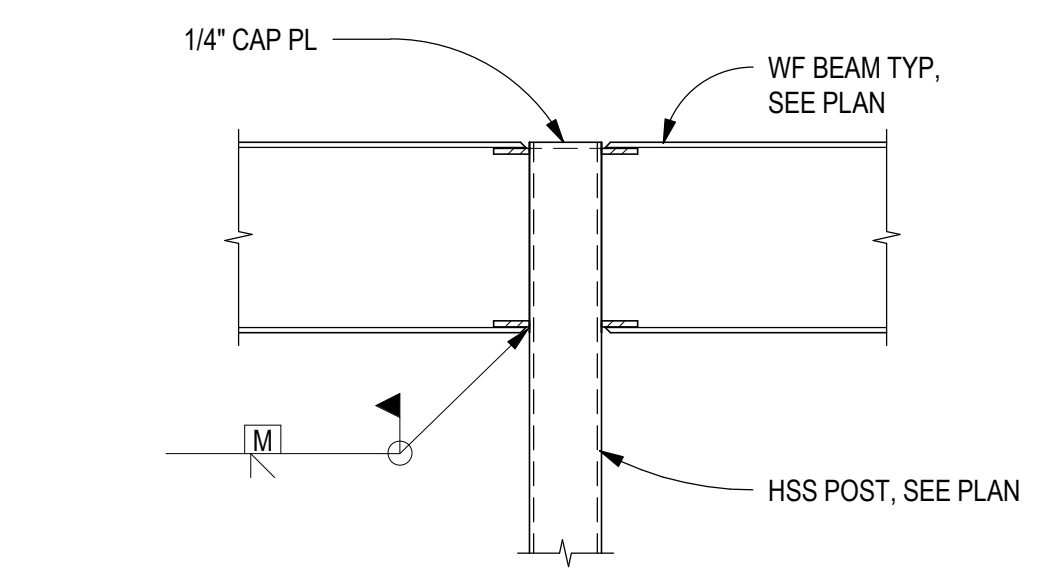
D5 BEAM TO POST MOMENT CONN
SCALE: 3/4" = 1'-0"



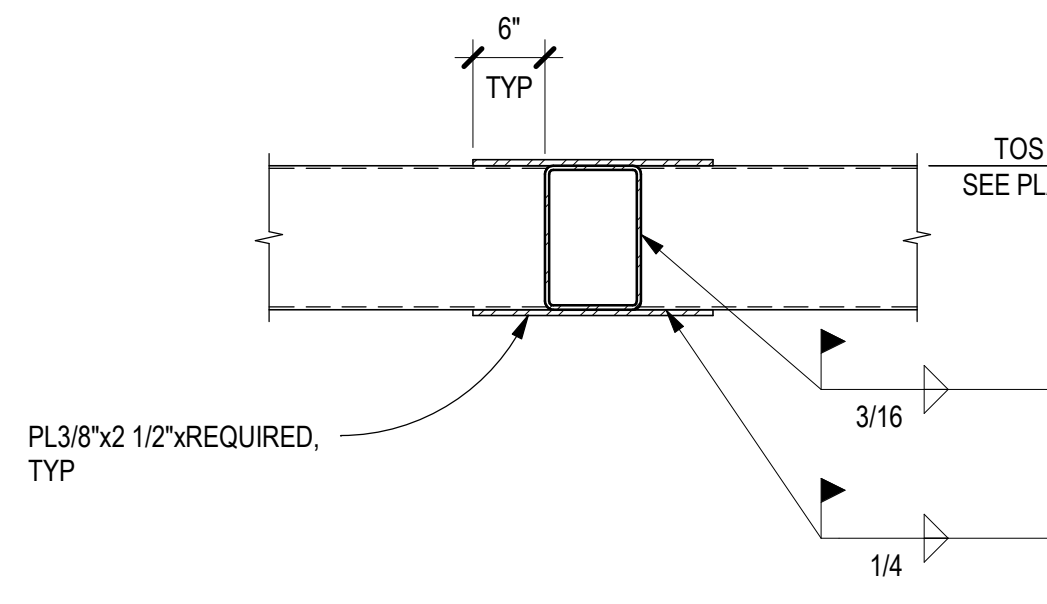
C5 BEAM OVER POST CONNECTION
SCALE: 3/4" = 1'-0"



B5 BEAM TO BEAM CONNECTION
SCALE: 3/4" = 1'-0"



A4 BEAM TO POST MOMENT CONN
SCALE: 3/4" = 1'-0"



A5 HSS TO HSS MOMENT CONNECTION
SCALE: 3/4" = 1'-0"

ENTIRE SHEET MODIFIED

KEY PLAN:

PROJECT PHASE:
BID PACKAGE 01

#	DATE	REVISIONS / DESCRIPTION
1	11/22/19	BID PACKAGE 01 - ADD 01

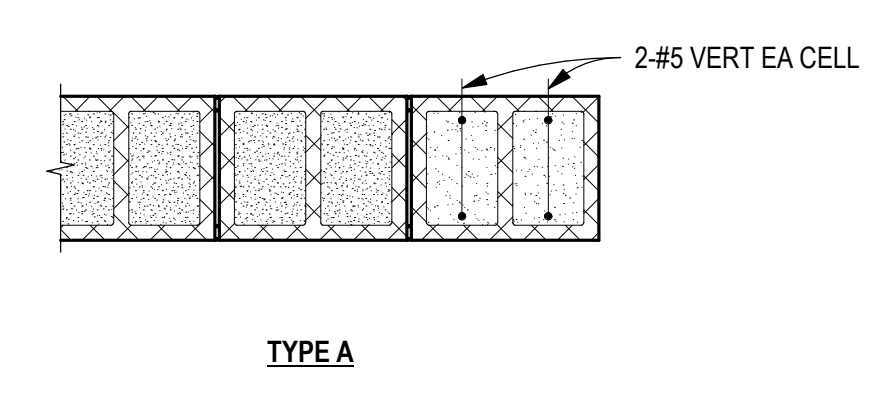
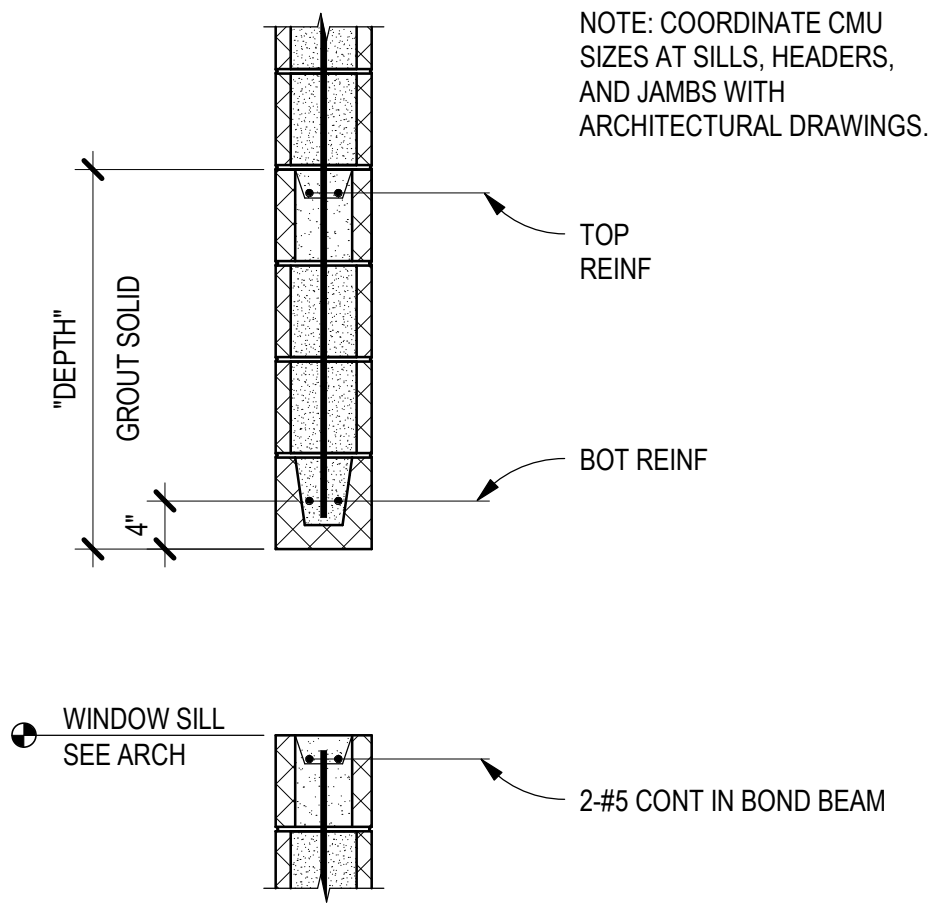
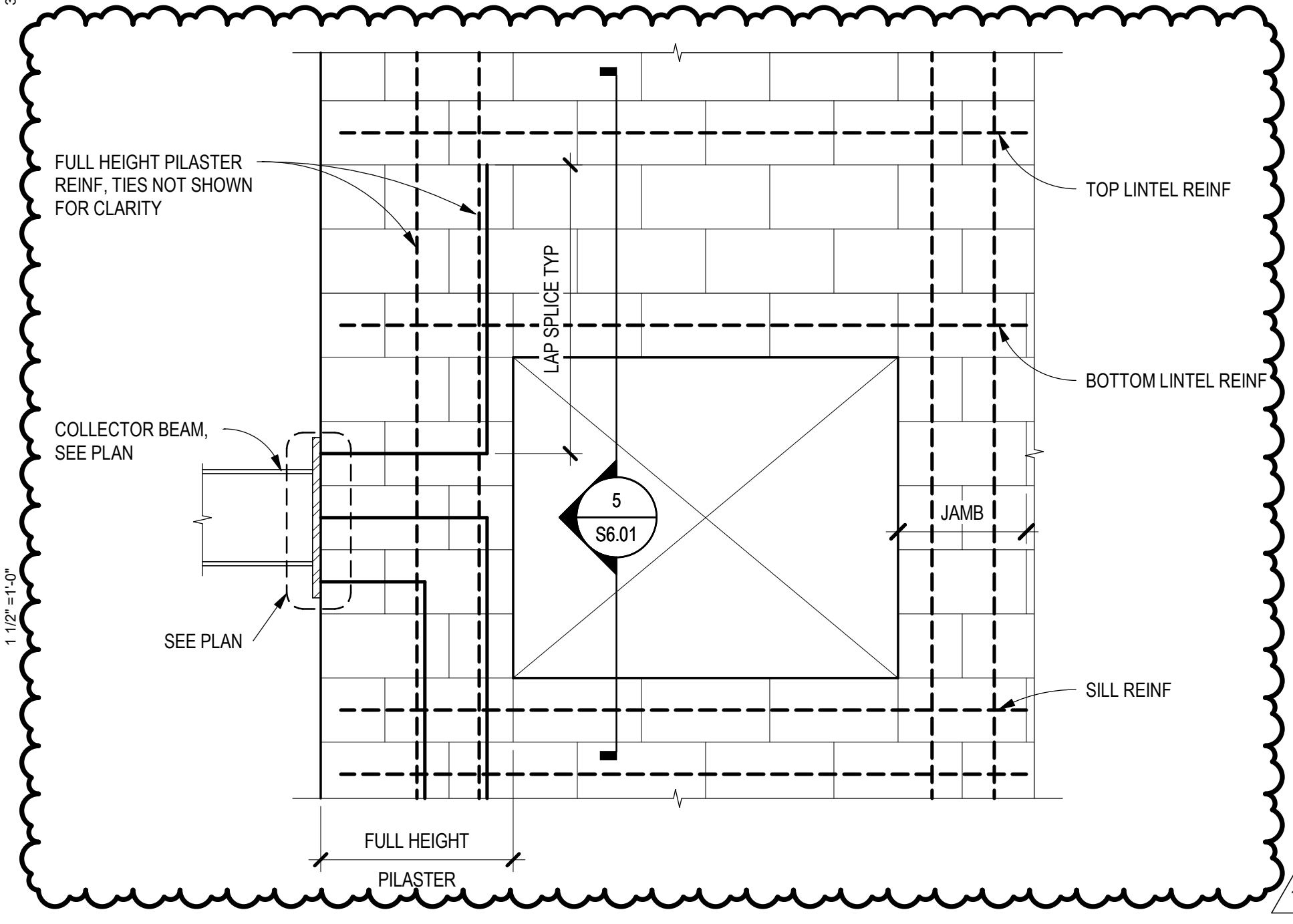
DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER:

S5.54

STEEL DETAILS

CMU LINTEL SCHEDULE						
OPENING WIDTH	WIDTH	DEPTH	LINTEL REINFORCING		SILL REINFORCING	LINTEL JAMB TYPE
			TOP	BOTTOM		
0'-0" - 8'-0"	12"	32"	2-#5	2-#5	8"	2-#5
						TYPE A

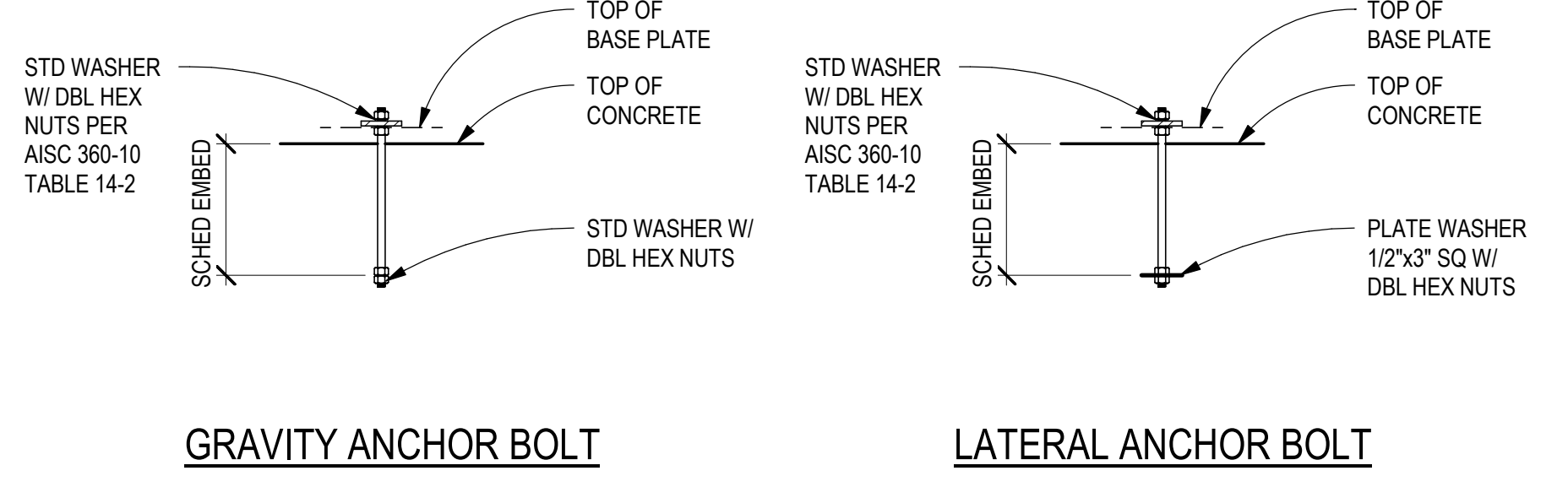
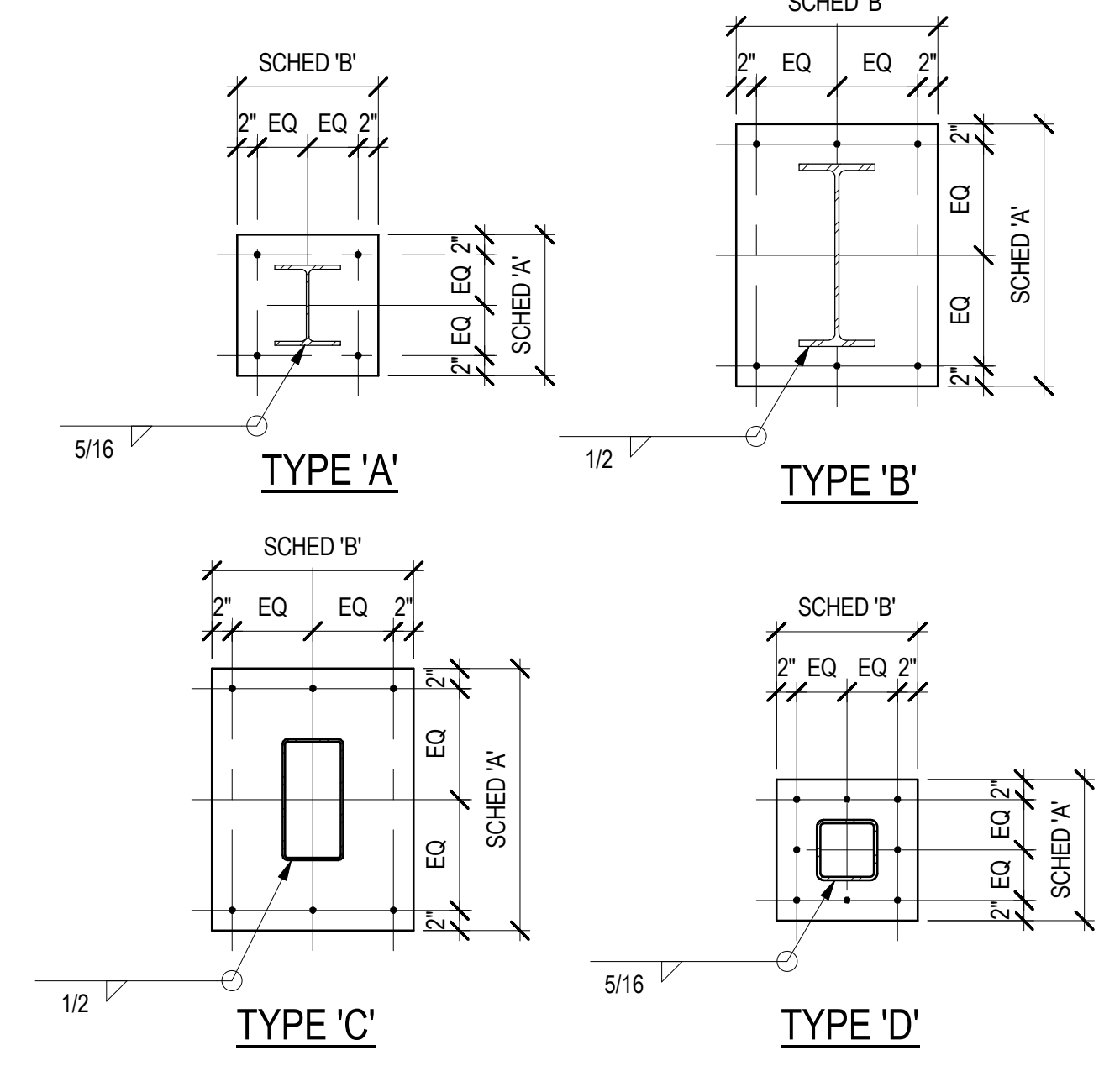


NOTE: SEE TYPICAL CMU PLAN DETAILS SHEET S7.21 FOR TYPICAL HORIZONTAL REINFORCING REQUIREMENTS.

DECK SCHEDULE													
MARK	COMPOSITE SLAB	SLAB			METAL DECK			DECK ATTACHMENTS			TOTAL SLAB / DECK THICKNESS	COMMENTS	
		THICK	MATL	REINF	THICK	TYPE	GAGE	FINISH	ATTACH PERP TO RIBS	ATTACH PARALLEL TO RIBS			ATTACH SIDELAPS
D1.5R	X	3"	NW CONC	6x6 - W2.1xW2.1 WELD WIRE FABRIC IN FLAT SHEETS	3"	VLI	18	GALVANIZED	4-5/8" DIA PUDDLE WELDS PER 36" WIDE SHEET	5/8" DIA PUDDLE WELDS @ 12" OC	#10 SCREWS @ 9" OC	11/2"	

SLAB-ON-GRADE SCHEDULE					
MARK	THICKNESS	MATL	REINFORCING	BEARING STRATA	COMMENTS
S6	6"	CONC	#4 @ 12" OC EA WAY	15 MIL VAPOR RETARDER OVER 1/2" SAND BLOTTER LAYER OVER 4" COMPACTED GRANULAR FILL OVER 14" OF COMPACTED STRUCTURAL FILL OVER COMPACTED SUBGRADE. SUBGRADE WILL BE PLACED WITH LASER LEVEL.	PREPARE SUBGRADE AND STRUCTURAL FILL PER GEOTECHNICAL REPORT

BASE PLATE SCHEDULE					
MARK	TYPE	SIZE	ANCHOR BOLTS		COMMENTS
			F1554	TYPE	
BP1	A	PL 1 1/4"x18"x1'-6"	4-3/4" DIA x 9"	GRAVITY	
BP2	A	PL 1 3/4"x18"x1'-6"	4-3/4" DIA x 9"	GRAVITY	
BP3	A	PL 1 3/4"x20"x1'-8"	4-3/4" DIA x 9"	GRAVITY	
BP4	A	PL 1 3/4"x22"x1'-10"	6-1" DIA x 18"	GRAVITY	
BP5	A	PL 3/4"x14"x1'-2"	4-3/4" DIA x 9"	GRAVITY	
BP6	B	PL 1 3/4"x22"x1'-8"	6-1" DIA x 18"	LATERAL	
BP7	B	PL 1 3/4"x22"x1'-8"	6-1" DIA x 18"	LATERAL	
BP8	C	PL 1 1/2"x20"x1'-4"	6-1" DIA x 18"	LATERAL	
BP9	D	PL 3/4"x14"x1'-2"	4-3/4" DIA x 9"	LATERAL	
BP10	D	PL 1"x18"x1'-6"	8-3/4" DIA x 9"	LATERAL	



WALL SCHEDULE						
MARK	VENEER	WALL	REINFORCING			COMMENTS
			VERTICAL	HORIZONTAL	GRADE	
WC8	--	8" CONC	#4 @ 12" OC	#4 @ 12" OC	A615	
WC12	SEE ARCH	12" CONC	#5 @ 12" OC EA FACE	#5 @ 12" OC EA FACE	A615	
WM12	SEE ARCH	12" CMU	#7 @ 16" OC EA FACE	#5 @ 24" OC EA FACE & STD LADDER TYPE JOINT REINF @ 16" OC	A615	GROUT ALL CELLS SOLID. SEE 7.21 FOR MASONRY DETAILS. D4/S7.21 FOR HORIZONTAL REINFORCEMENT LOCATION

REINFORCEMENT TYPE	REQUIRED LAP SPLICES ACI 318-14/IBC 2015						MINIMUM LENGTH (IN)	COMMENTS
	#6 AND SMALLER (#6)	#7 THROUGH #11 (#6b)	3000PSI	4000PSI	5000PSI	3000PSI		
CONTINUOUS WALL FOOTINGS AND HORIZONTAL REINFORCEMENT IN SITE WALLS	30	30	30	30	30	30	18	
CONCRETE WALLS: ALL VERTICAL REINFORCEMENT	44	38	34	55	48	43	12	
CONCRETE WALLS: ALL HORIZONTAL REINFORCEMENT, EXCLUDING SITE WALLS AND STEM WALLS	57	50	45	72	62	56	12	
CONCRETE COLUMNS	44	38	34	55	48	43	12	
TOP FLEXURAL REINFORCEMENT, INCLUDING BEAMS, GRADE BEAMS, AND COMBINED COLUMN FOOTINGS	57	50	45	72	62	56	12	
BOTTOM FLEXURAL REINFORCEMENT, INCLUDING BEAMS, GRADE BEAMS, AND COMBINED COLUMN FOOTINGS	44	38	34	55	48	43	12	
MINIMUM EMBEDMENT OF STANDARD HOOKS INTO CONCRETE BASE	22	19	17	22	19	17	6	ALLOWED FOR BARS LARGER THAN #11
SLABS ON GRADE	30	30	30	30	30	30	12	
SLABS ON METAL DECK	30	30	30	30	30	30	12	WWF MINIMUM LAP LENGTH = 6 IN

NOTES:

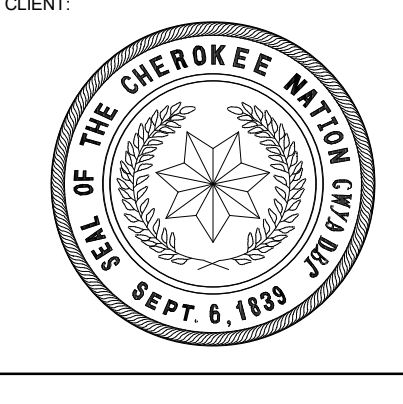
- LAP SPLICES SHALL NOT BE PERMITTED FOR BARS LARGER THAN #11.
- LAP SPLICES FOR BUNDLED BARS SHALL BE IN ACCORDANCE WITH ACI 318-14 SECTION 25.5.1.4
- LAP LENGTHS FOR LIGHTWEIGHT CONCRETE SHALL BE INCREASED BY 33%
- LAP LENGTHS FOR EPOXY COATED BARS SHALL BE INCREASED BY 50%
- FOR INTERMEDIATE OR LARGER VALUES OF F_{CD}, USE THE CLOSEST LOWER VALUE IN THE TABLE. DO NOT INTERPOLATE

MASONRY LAP SPLICES (#6) ACI 530-13/IBC 2015								
	#3	#4	#5	#6	#7	#8	#9	
8" BLOCK WITH 1-LAYER OF REINFORCEMENT	32	40	51	72	N/A	N/A	N/A	
8" BLOCK WITH 2-LAYERS OF REINFORCEMENT	32	29	36	58	68	72	N/A	
12" BLOCK WITH 1-LAYER OF REINFORCEMENT	51	68	72	72	N/A	N/A	N/A	
12" BLOCK WITH 2-LAYERS OF REINFORCEMENT	32	24	23	37	43	57	65	
16" BLOCK WITH 1-LAYER OF REINFORCEMENT	51	68	72	72	72	N/A	N/A	
16" BLOCK WITH 2-LAYERS OF REINFORCEMENT	32	24	23	30	32	42	48	
16" BLOCK WITH 2-LAYERS OF REINFORCEMENT	51	68	72	72	72	72	72	

BARS LARGER THAN #9 SHALL BE SPLICED USING MECHANICAL CONNECTIONS

SPOT FOOTING SCHEDULE						
MARK	WIDTH	SIZE		REINFORCING		COMMENTS
		LENGTH	DEPTH	REINFORCING	GRADE	
F48	4'-0"	4'-0"	1'-0"	4-#5 EA WAY BOT	A615	
F48P	4'-0"	4'-0"	1'-0"	4-#5 EA WAY BOT	A615	
F60	5'-0"	5'-0"	1'-6"	5-#6 EA WAY BOT	A615	
F60A	5'-0"	5'-0"	2'-0"	6-#6 EA WAY TOP & BOT	A615	TOP BARS TO HAVE STD HOOKS AT ENDS
F72	6'-0"	6'-0"	1'-6"	6-#6 EA WAY BOT	A615	
F72A	6'-0"	6'-0"	2'-0"	8-#6 EA WAY TOP & BOT	A615	TOP BARS TO HAVE STD HOOKS AT ENDS
F72P	6'-0"	6'-0"	1'-6"	6-#6 EA WAY TOP & BOT	A615	TOP BARS TO HAVE STD HOOKS AT ENDS
F84	7'-0"	7'-0"	2'-0"	9-#6 EA WAY BOT	A615	
F84A	7'-0"	7'-0"	2'-0"	9-#6 EA WAY TOP & BOT	A615	TOP BARS TO HAVE STD HOOKS AT ENDS
F84P	7'-0"	7'-0"	2'-0"	9-#6 EA WAY TOP & BOT	A615	TOP BARS TO HAVE STD HOOKS AT ENDS
F96	8'-0"	8'-0"	2'-0"	7-#7 EA WAY BOT	A615	
F96A	8'-0"	8'-0"	2'-0"	7-#7 EA WAY TOP & BOT	A615	TOP BARS TO HAVE STD HOOKS AT ENDS
F276	23'-0"	21'-0"	2'-9"	#8 @ 9" OC EA WAY TOP & BOT	A615	TOP BARS TO HAVE STD HOOKS AT ENDS

CONTINUOUS FOOTING SCHEDULE					
MARK	WIDTH	DEPTH	REINFORCING		COMMENTS
			CONTINUOUS	TRANSVERSE	
CF16	1'-4"	1'-0"	3-#4	#4 @ 48" OC	
CF24	2'-0"	1'-0"	3-#4	#4 @ 48" OC	
CF84	7'-0"	2'-9"	8-#8	#8 @ 9" OC	TOP BARS TO HAVE STD HOOKS AT ENDS



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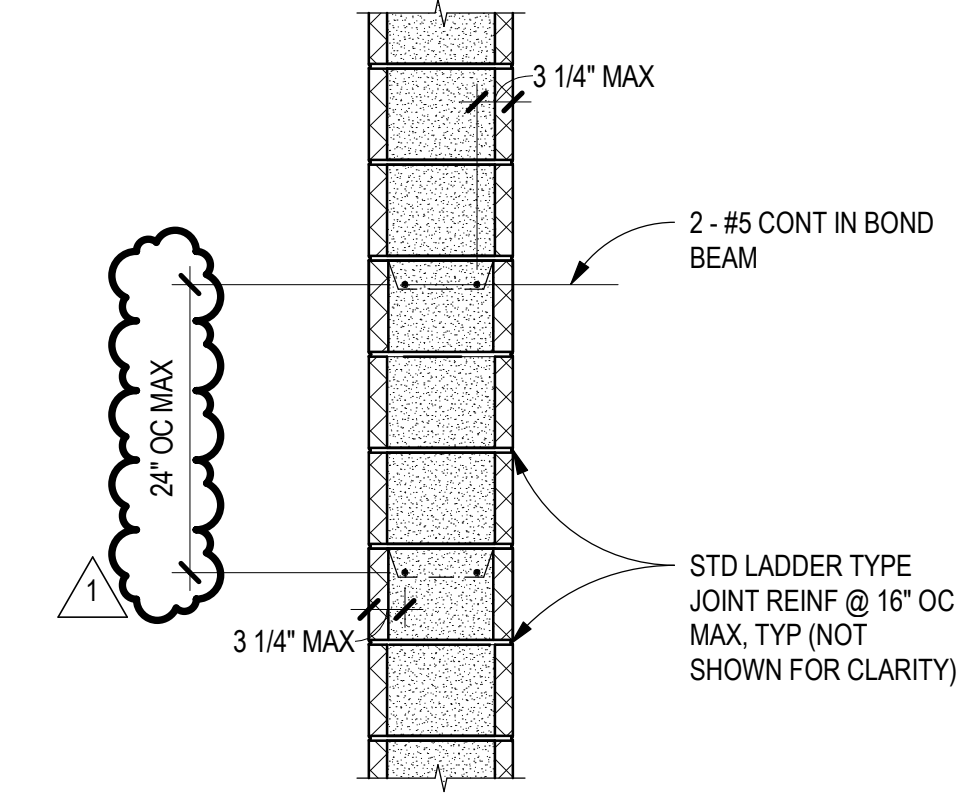
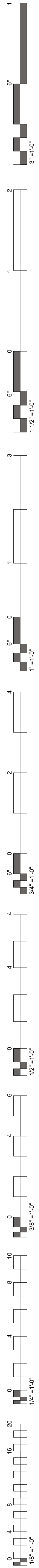
KEY PLAN:
PROJECT PHASE:
BID PACKAGE 01

#	DATE	REVISIONS / DESCRIPTION
1	11/02/19	BID PACKAGE 01 - ADD 01

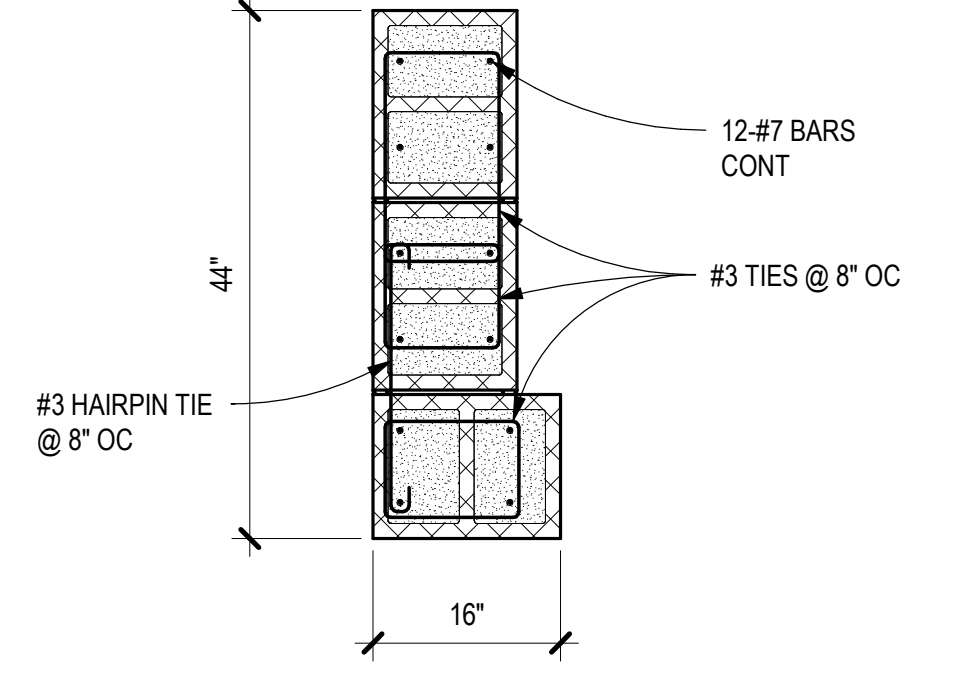
DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER: S6.01

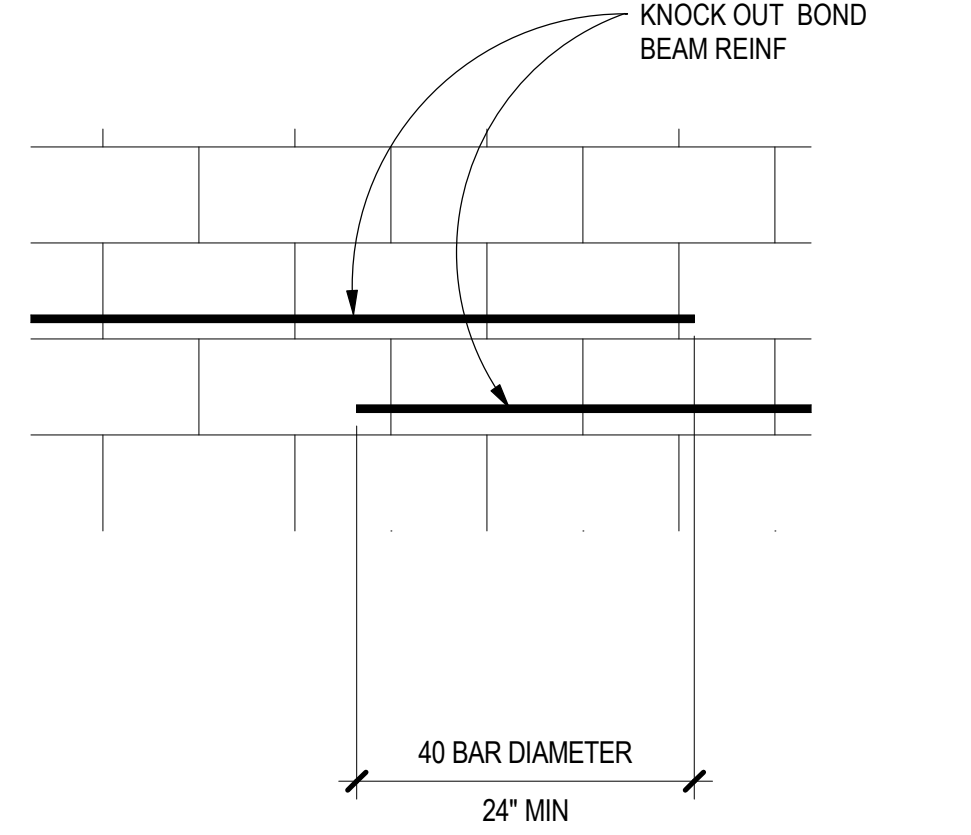
SCHEDULES



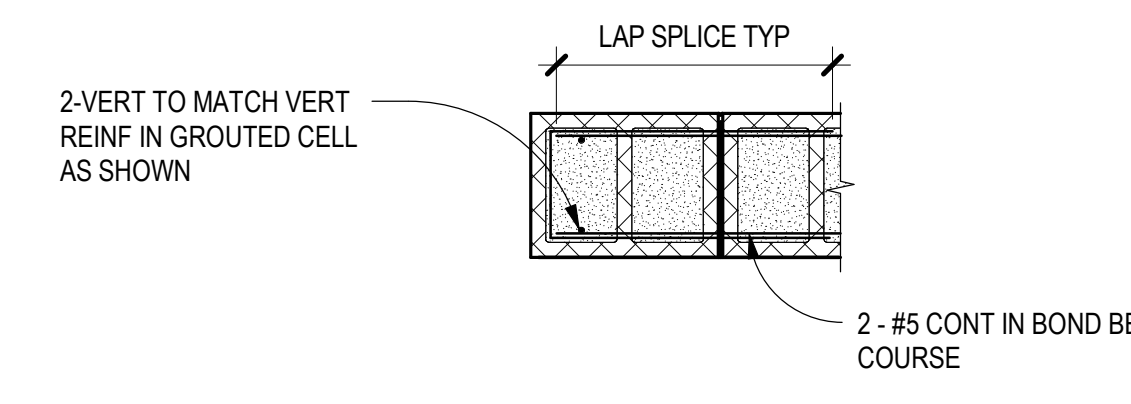
D4 TYPICAL BOND BEAM DETAIL
SCALE: 3/4" = 1'-0"



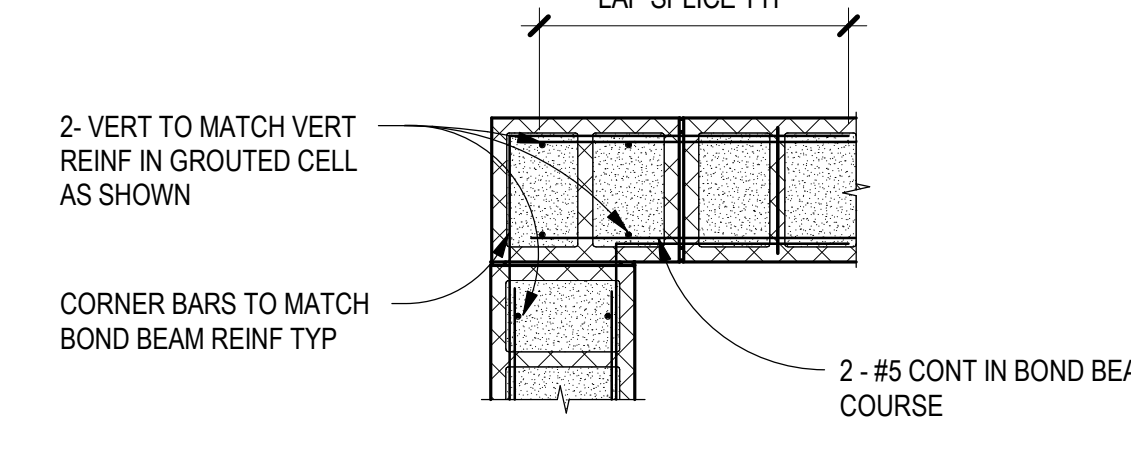
C4 PILASTER DETAIL
SCALE: 3/4" = 1'-0"



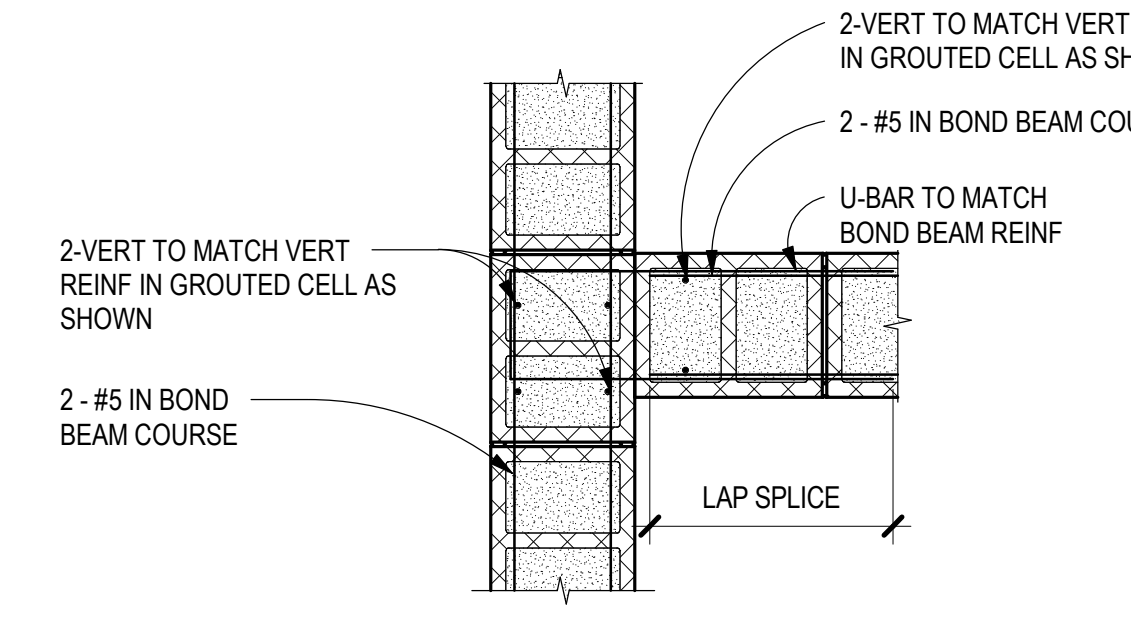
B4 TYPICAL STEP IN BOND BEAM
SCALE: 3/4" = 1'-0"



BOND BEAM AT WALL END

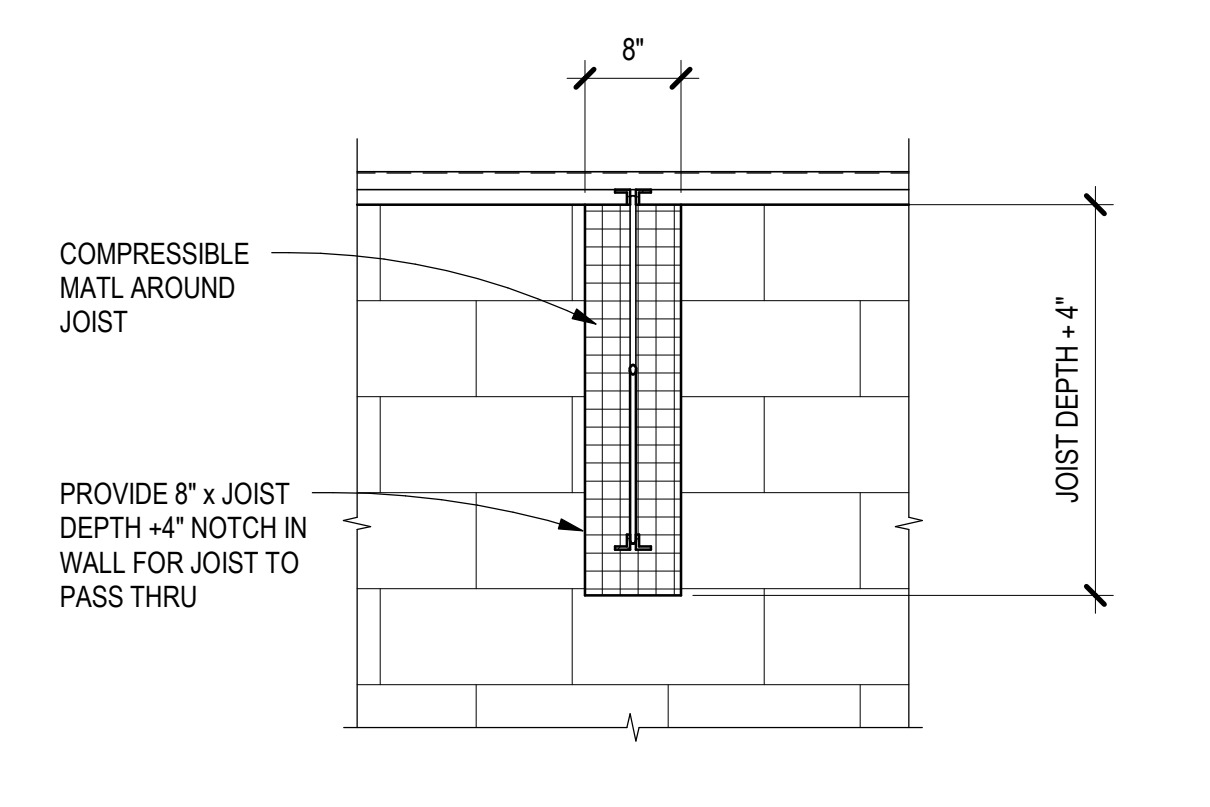


BOND BEAM AT WALL CORNER

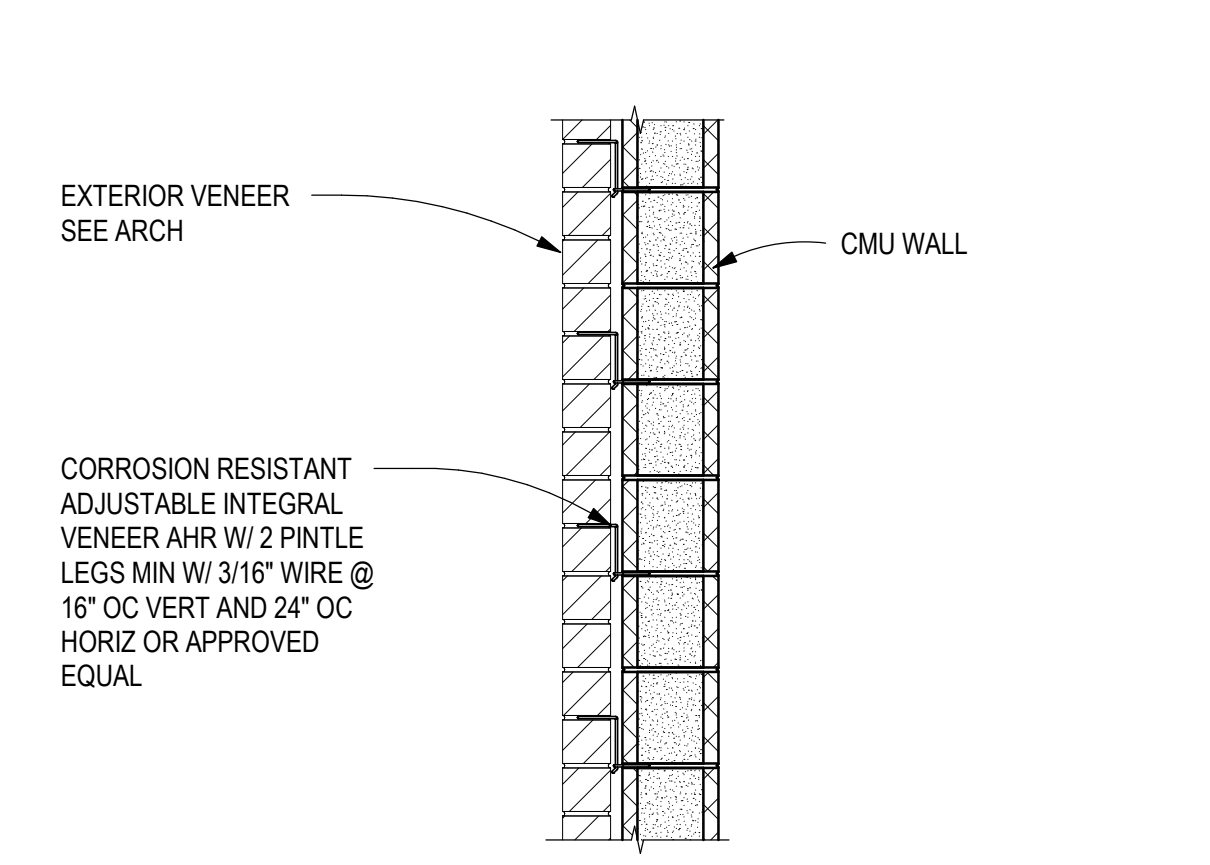


BOND BEAM AT WALL TEE

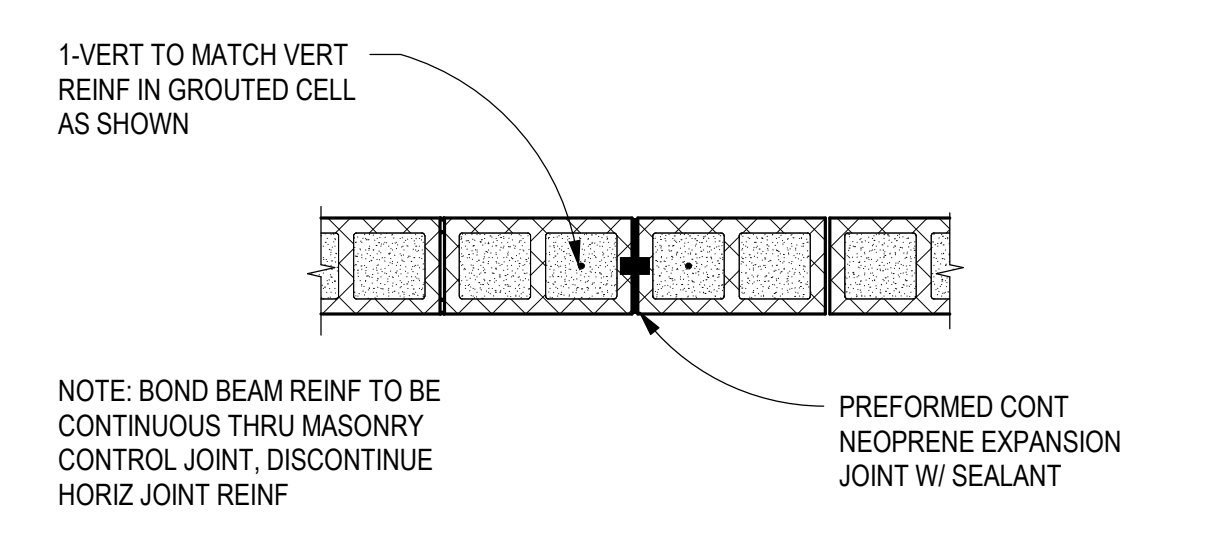
A4 TYPICAL 12" CMU PLAN DETAILS
SCALE: 3/4" = 1'-0"



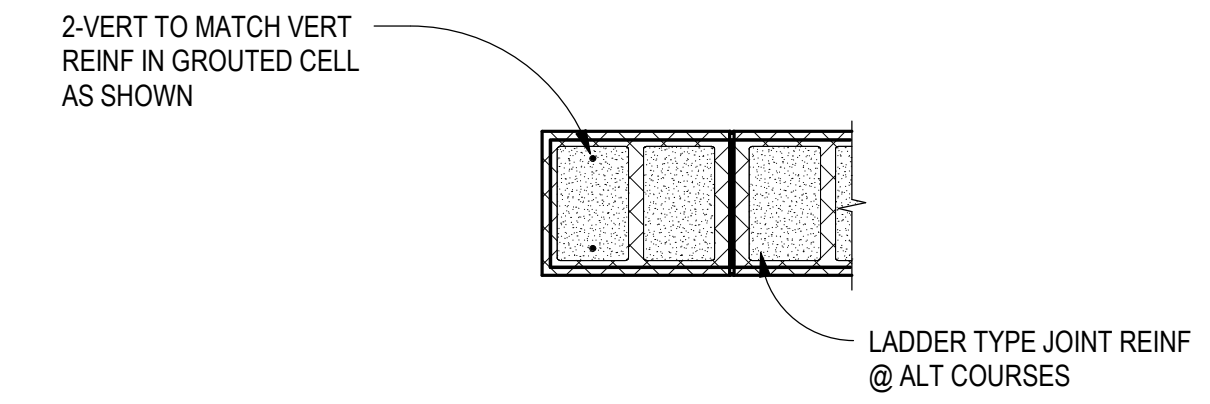
D5 TYPICAL JOIST THRU CMU WALL
SCALE: 3/4" = 1'-0"



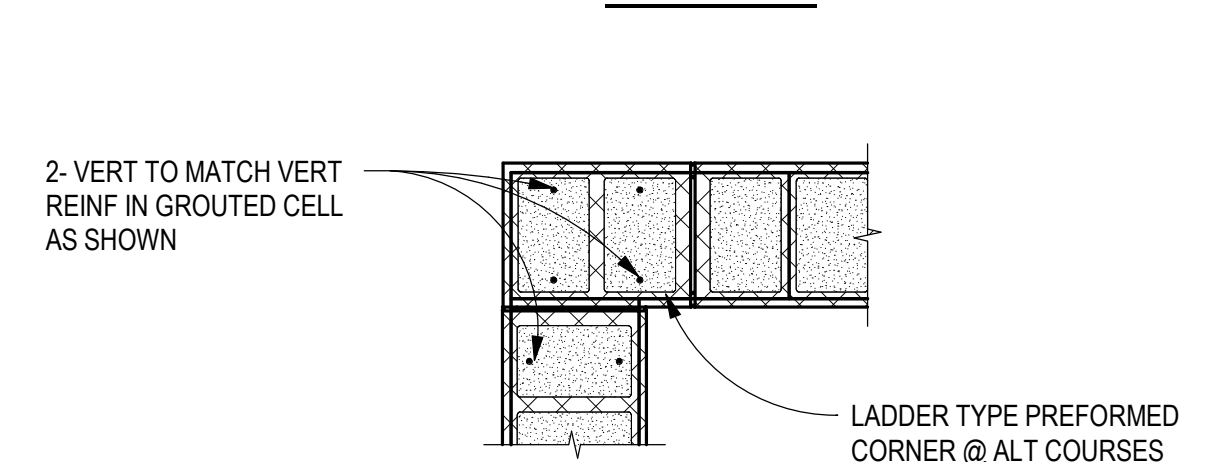
C5 TYPICAL VENEER TO CMU WALL DETAIL
SCALE: 3/4" = 1'-0"



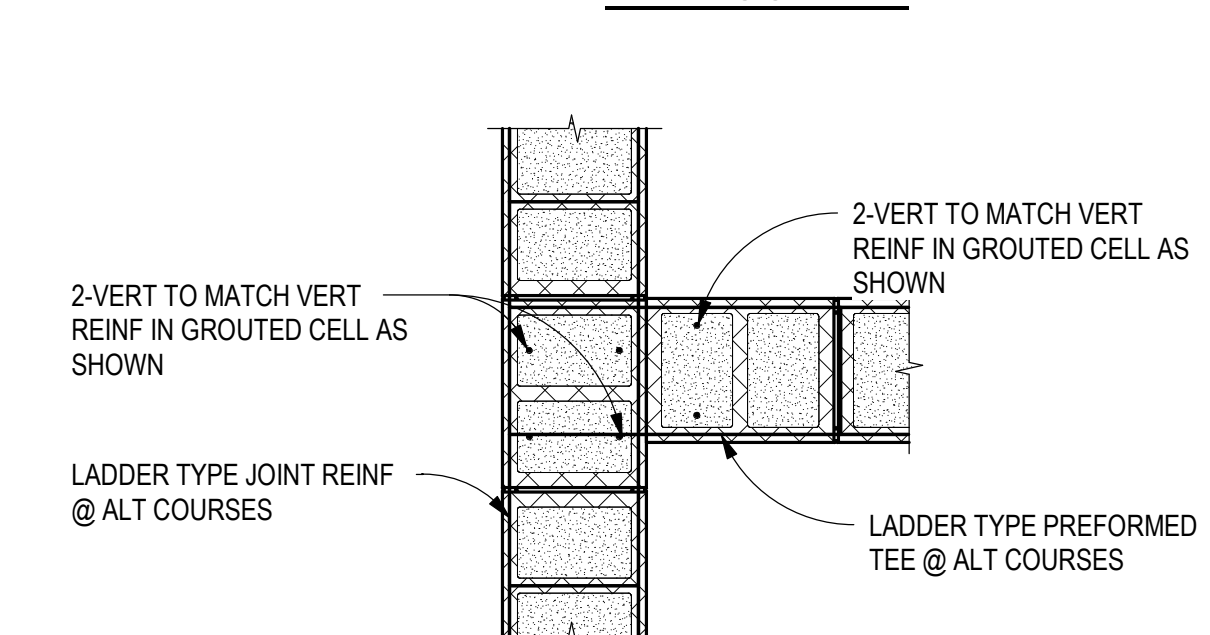
B5 TYPICAL CMU CNRTL JOINT (MCJ)
SCALE: 3/4" = 1'-0"



WALL END

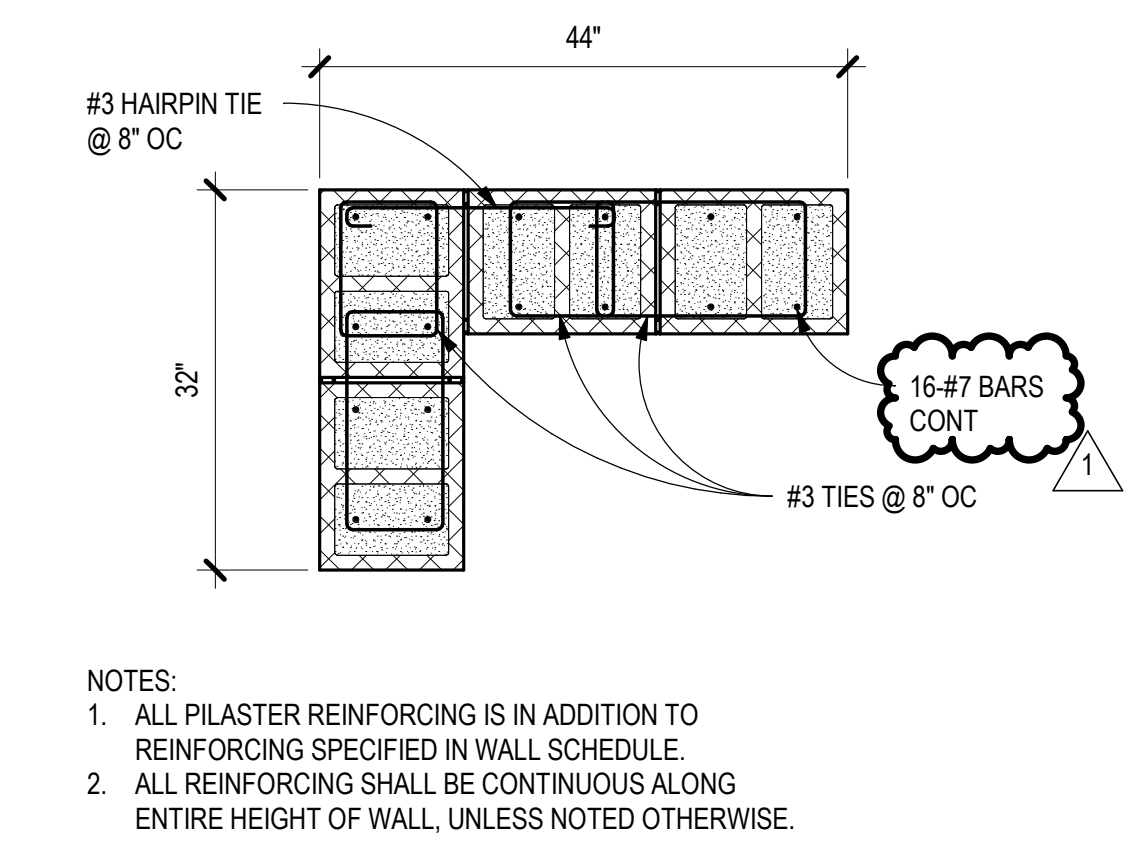


WALL CORNER



WALL TEE

A5 TYPICAL 12" CMU PLAN DETAILS
SCALE: 3/4" = 1'-0"

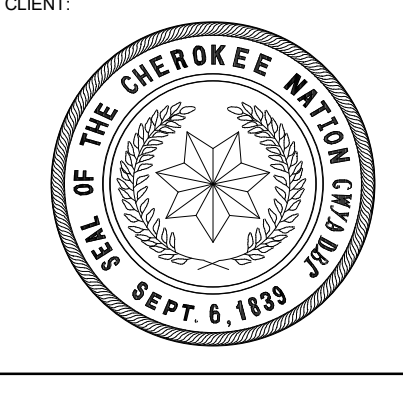


A3 PILASTER DETAIL
SCALE: 3/4" = 1'-0"

NOTES:
1. ALL PILASTER REINFORCING IS IN ADDITION TO REINFORCING SPECIFIED IN WALL SCHEDULE.
2. ALL REINFORCING SHALL BE CONTINUOUS ALONG ENTIRE HEIGHT OF WALL, UNLESS NOTED OTHERWISE.

NOTES:
1. ALL PILASTER REINFORCING IS IN ADDITION TO REINFORCING SPECIFIED IN WALL SCHEDULE.
2. ALL REINFORCING SHALL BE CONTINUOUS ALONG ENTIRE HEIGHT OF WALL, UNLESS NOTED OTHERWISE.

NOTE: BOND BEAM REINF TO BE CONTINUOUS THRU MASONRY CONTROL JOINT, DISCONTINUE HORIZ JOINT REINF



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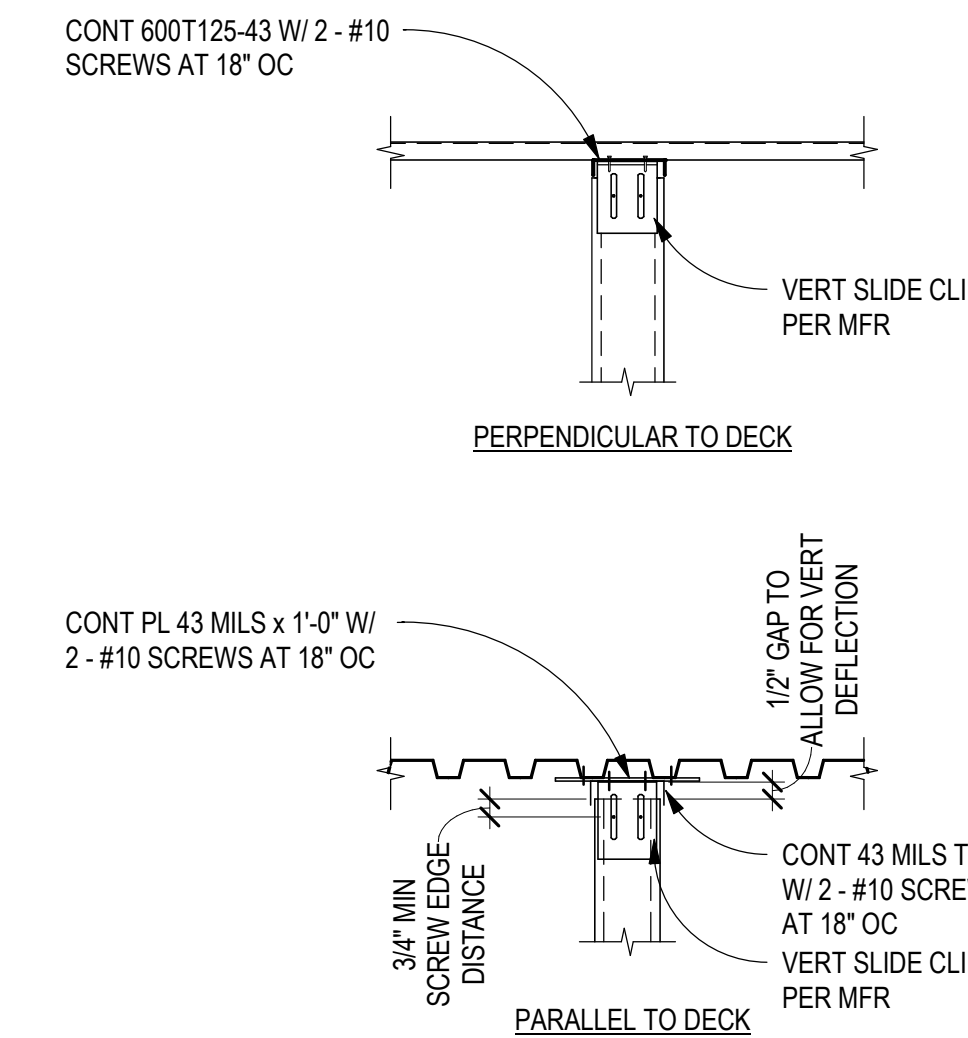
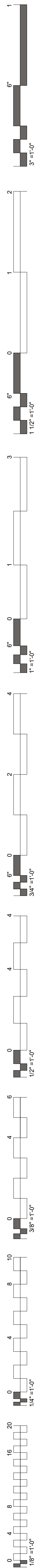
PROJECT PHASE:
BID PACKAGE 01

#	DATE	REVISIONS	DESCRIPTION
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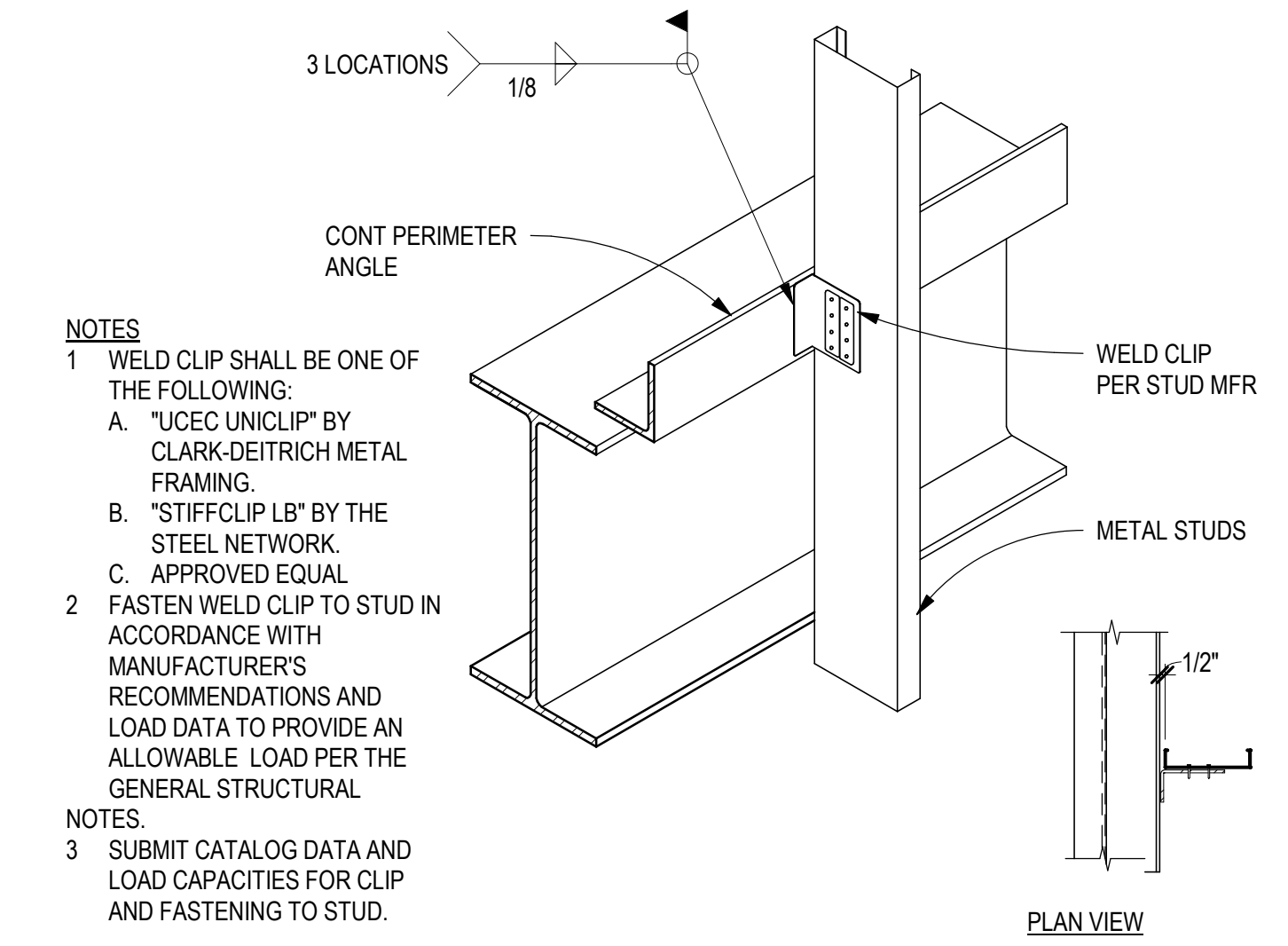
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SHEET NUMBER: S7.21

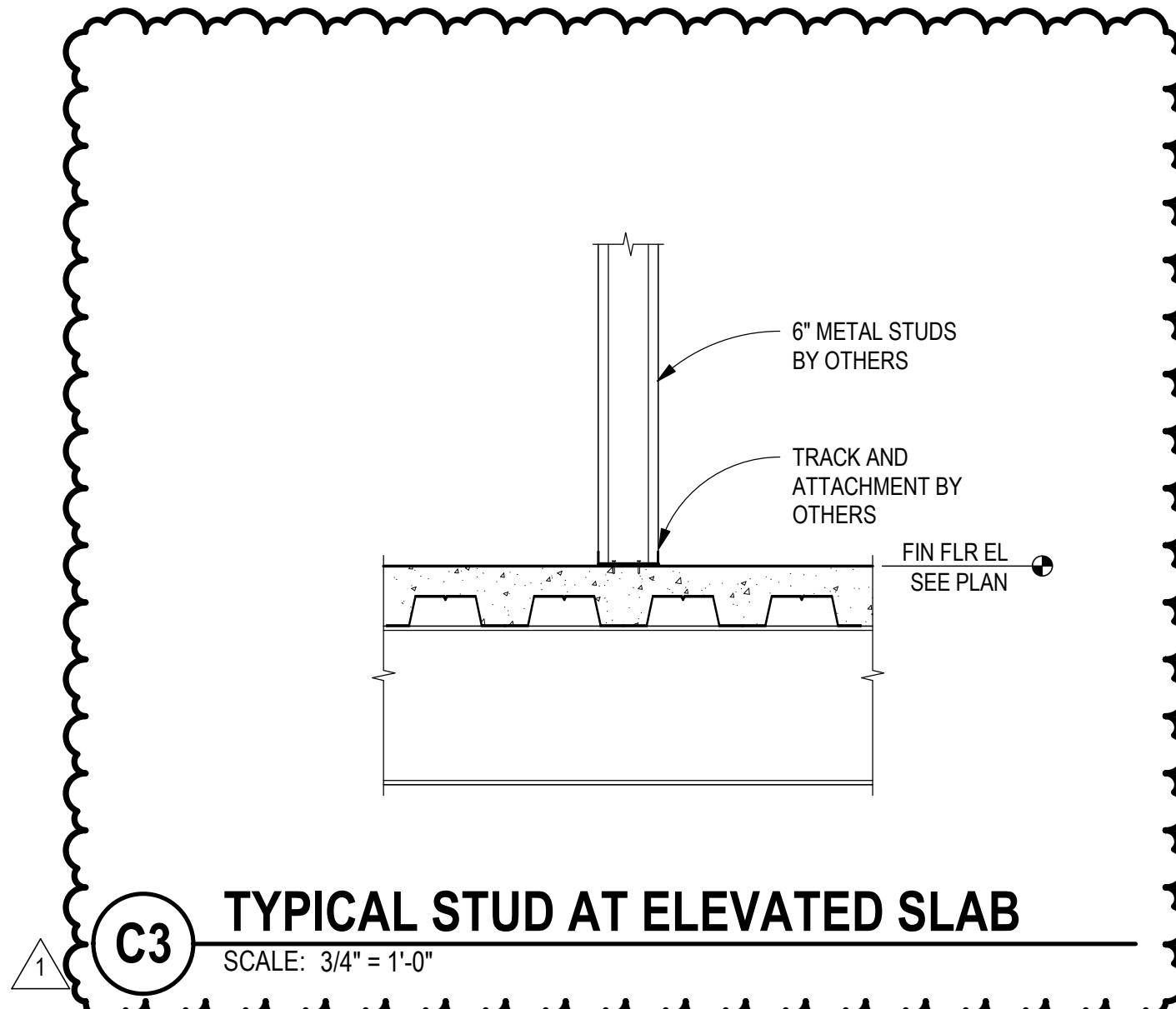
TYPICAL MASONRY DETAILS



D4 TYPICAL SLIP TRACK ASSEMBLY
SCALE: NTS

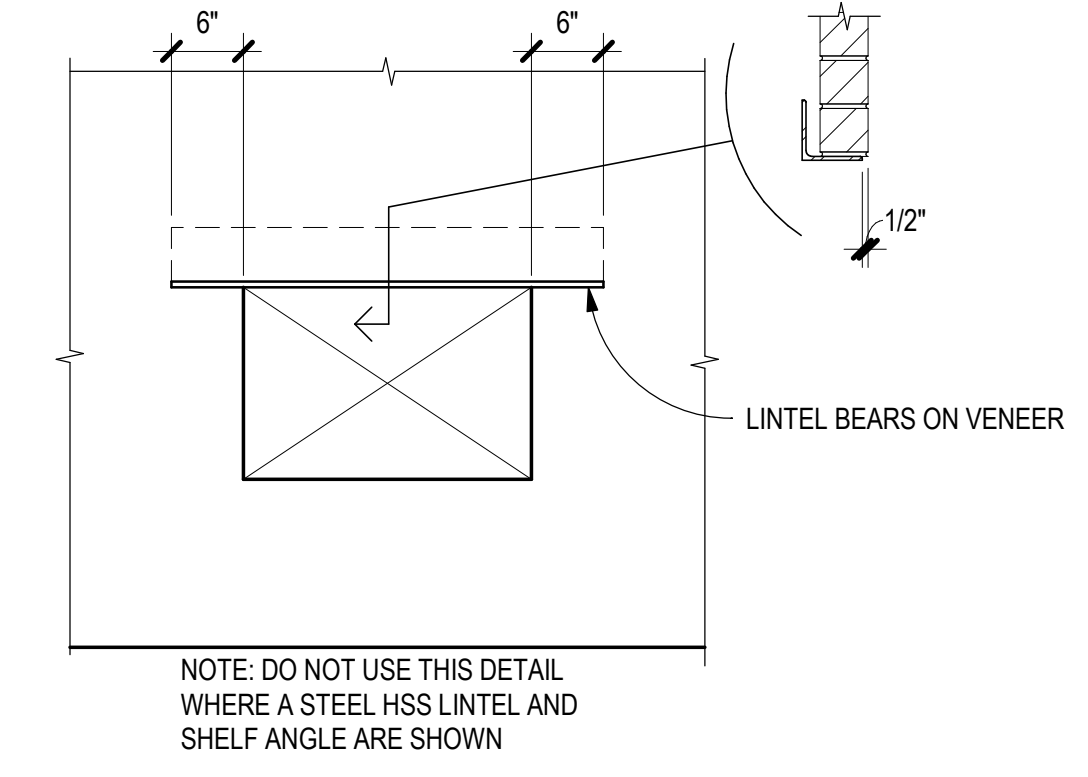


D5 TYPICAL WELD CLIP
SCALE: NTS

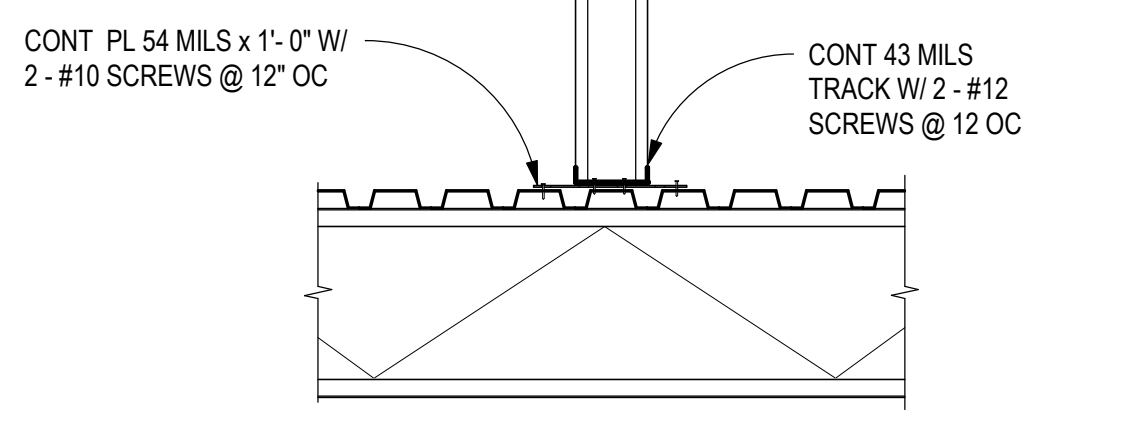


C3 TYPICAL STUD AT ELEVATED SLAB
SCALE: 3/4\"/>

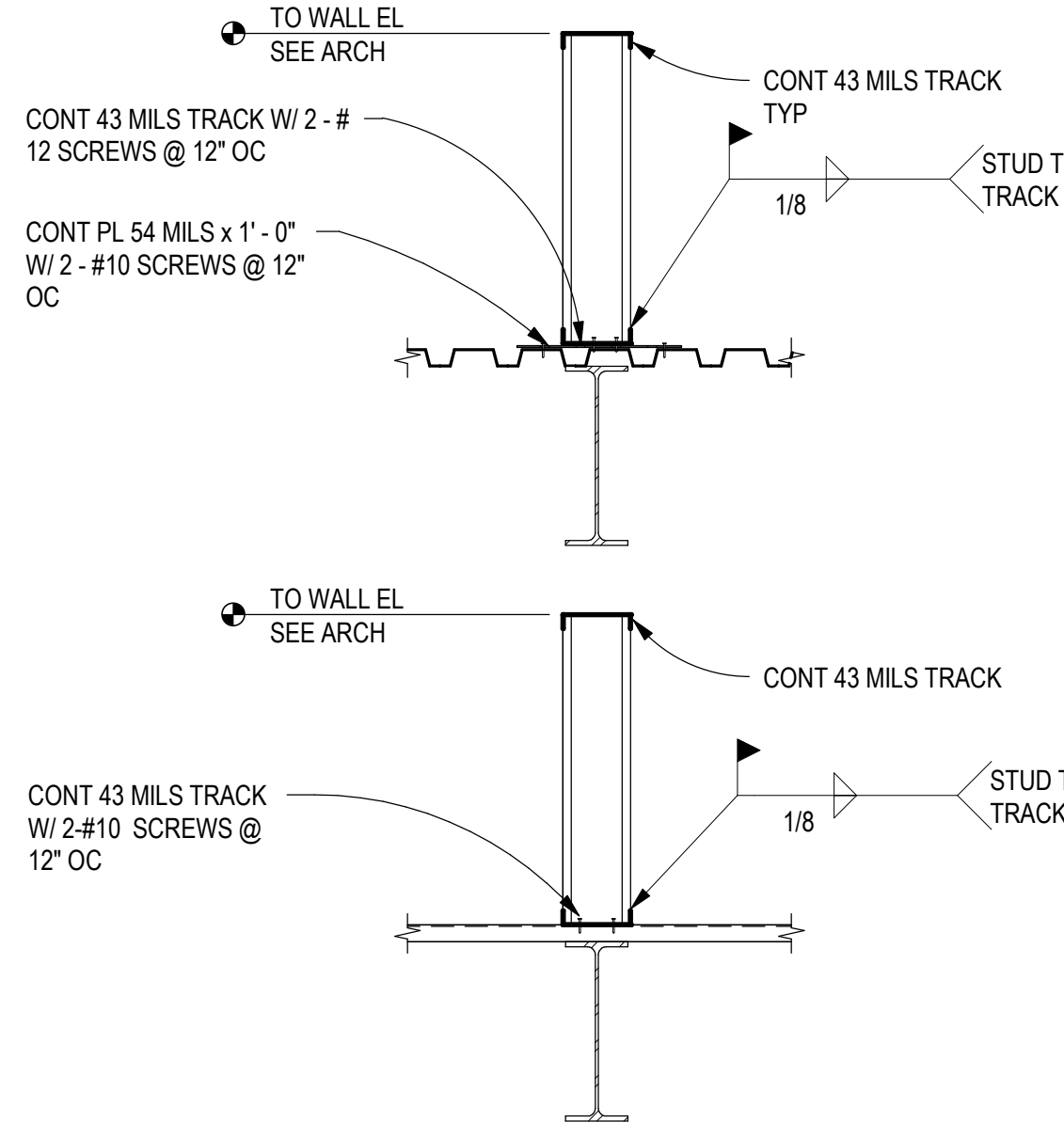
VENEER LINTEL	
OPENING WIDTH	ANGLE
0' - 0" TO 2' - 0"	L5x3x1/4 LLH
2' - 1" TO 3' - 4"	L5x3x5/16 LLH
3' - 5" TO 4' - 0"	L5x3x3/8 LLH
4' - 1" TO 6' - 4"	L6x6x1/2
6' - 5" TO 8' - 0"	L6x6x1/2



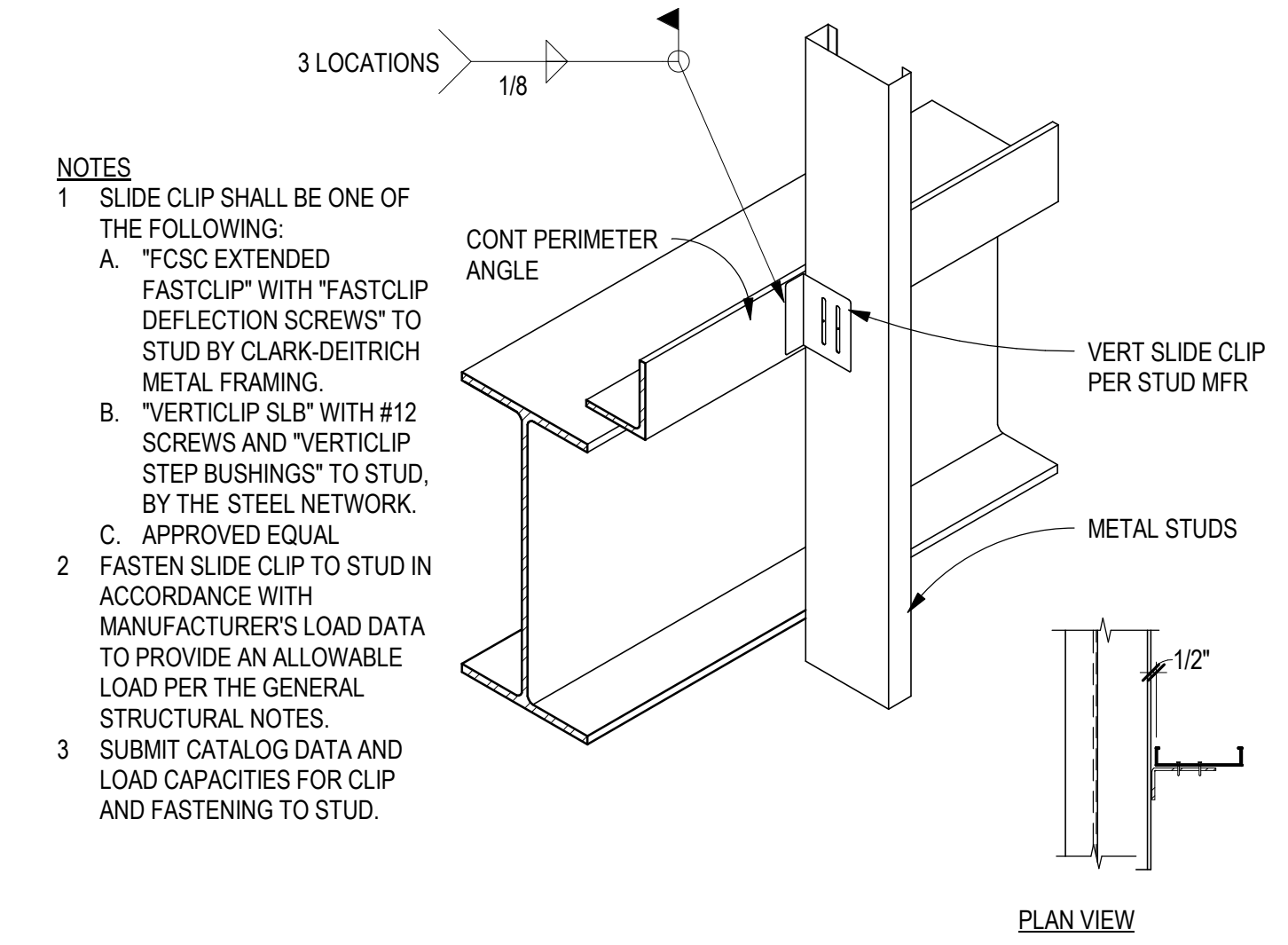
B3 COLD-FORMED LINTEL SECTION @ VENEER
SCALE: 3/4\"/>



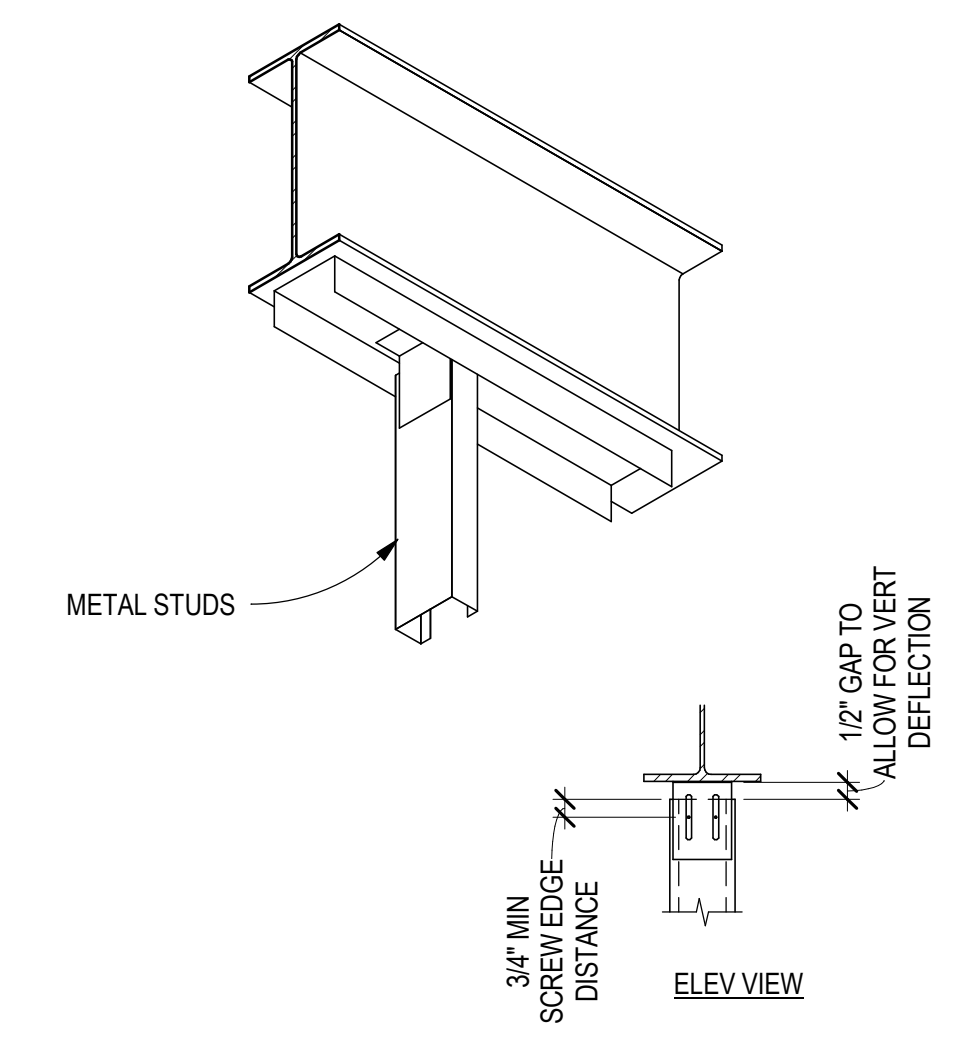
C4 TYPICAL STUD AT METAL DECK
SCALE: 3/4\"/>



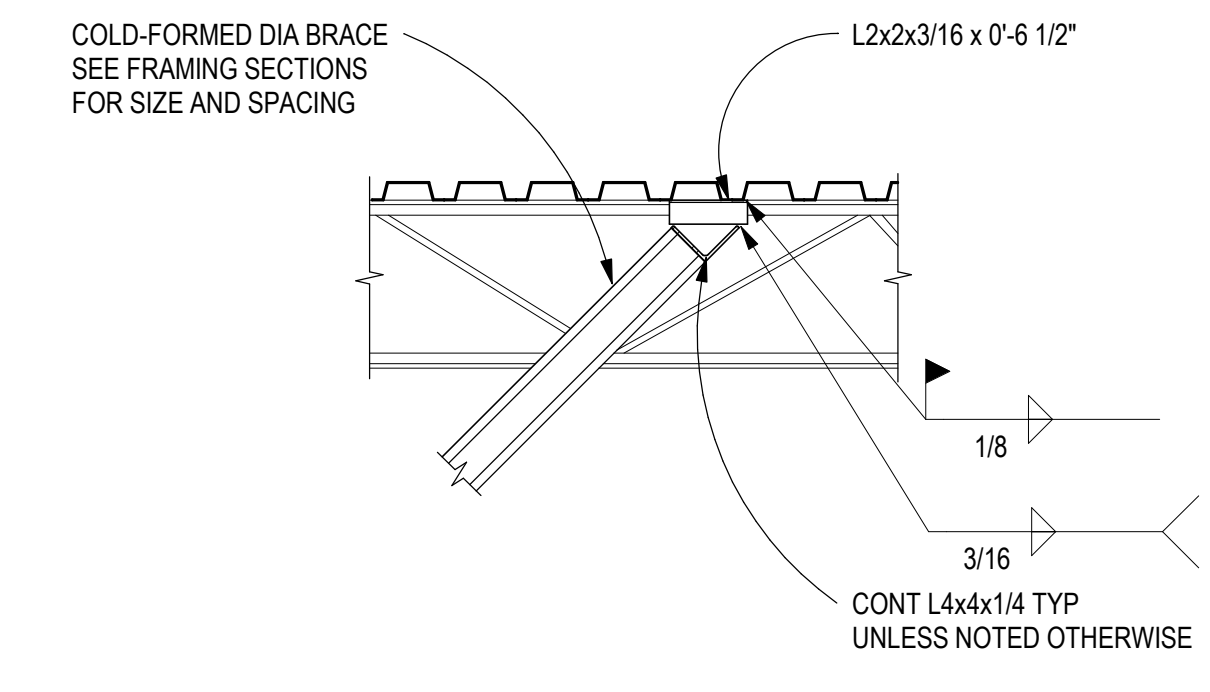
B4 TYPICAL PARAPET TO DECK
SCALE: 3/4\"/>



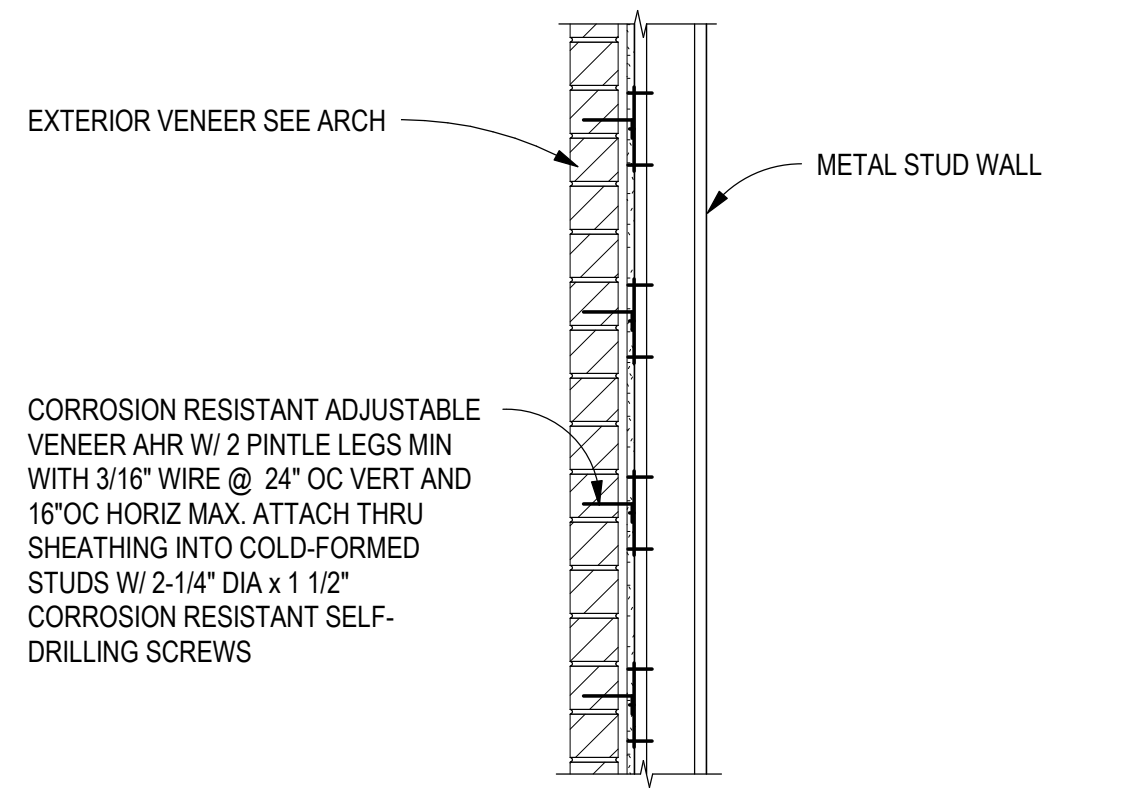
C5 TYPICAL VERTICAL SLIDE CLIP
SCALE: NTS



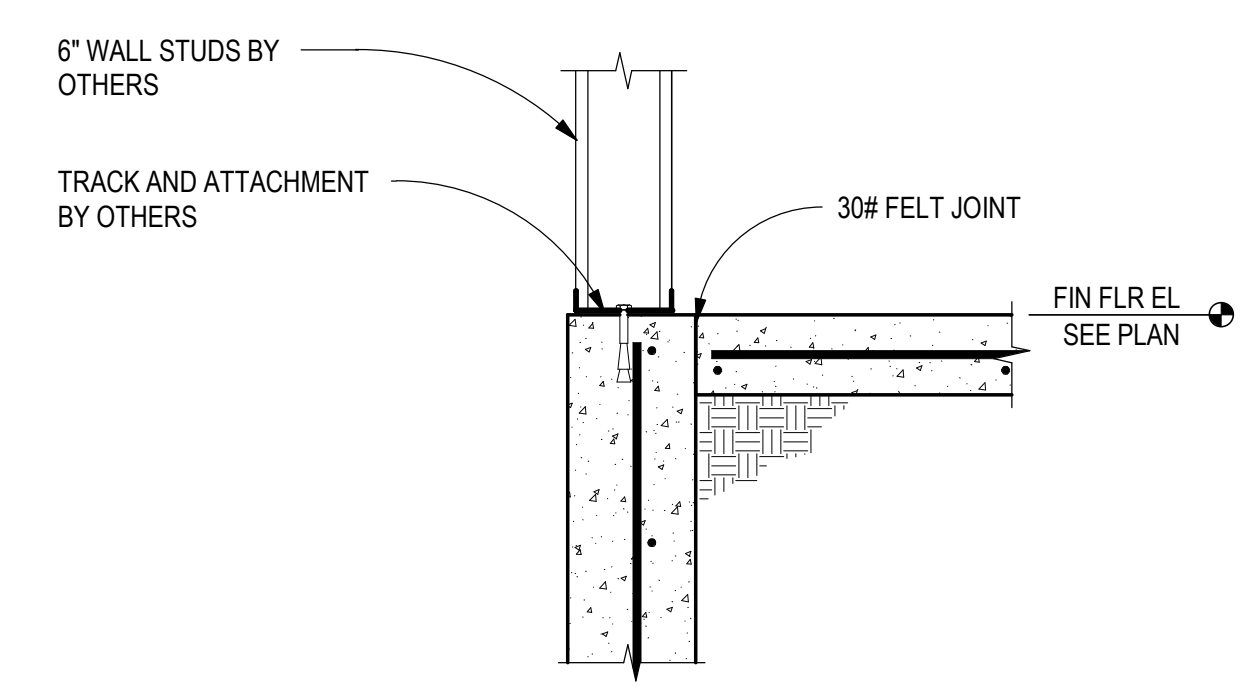
B5 TYPICAL SLIP TRACK ASSEMBLY
SCALE: NTS



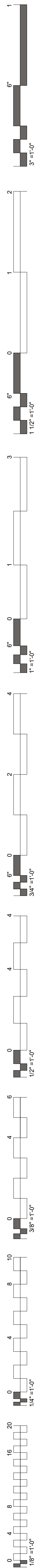
A3 TYPICAL DIAG BRACE TO DECK
SCALE: 3/4\"/>



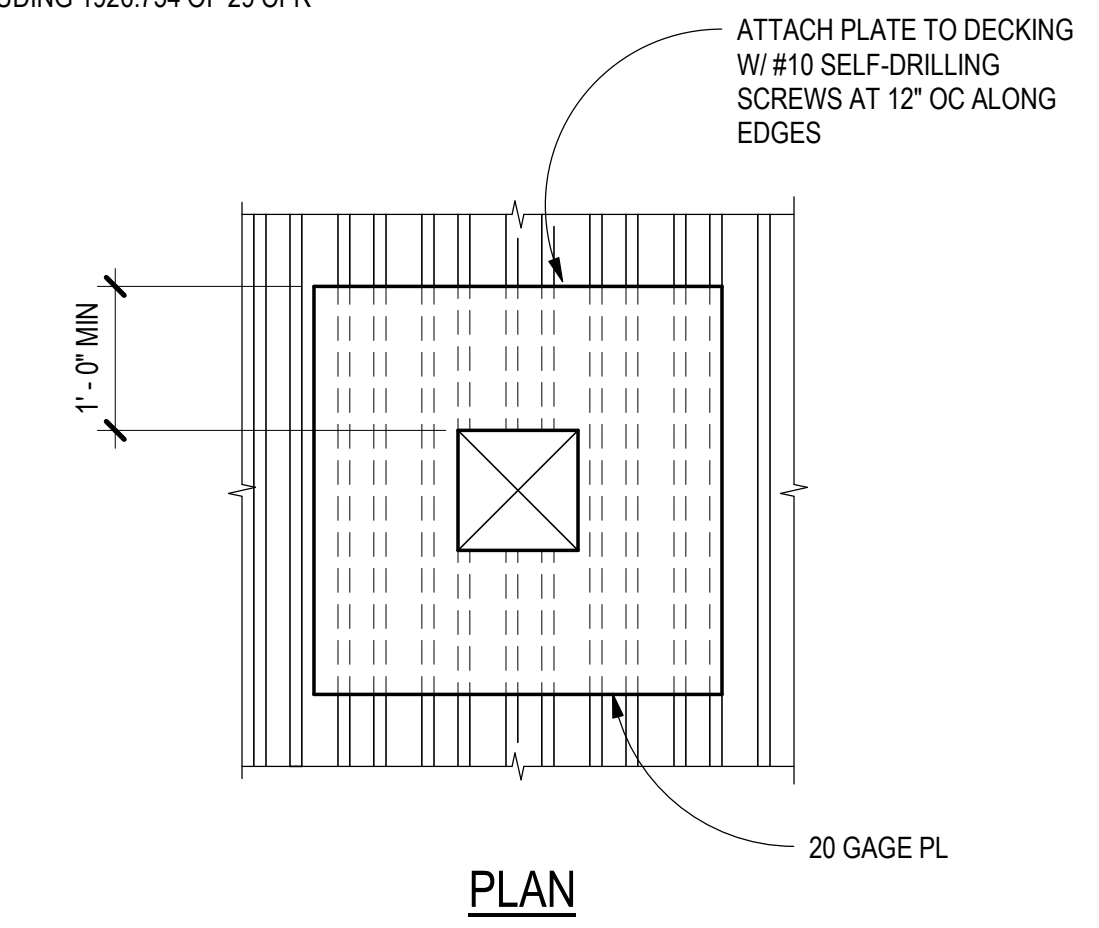
A4 TYPICAL VENEER ON MTL STUDS
SCALE: 3/4\"/>



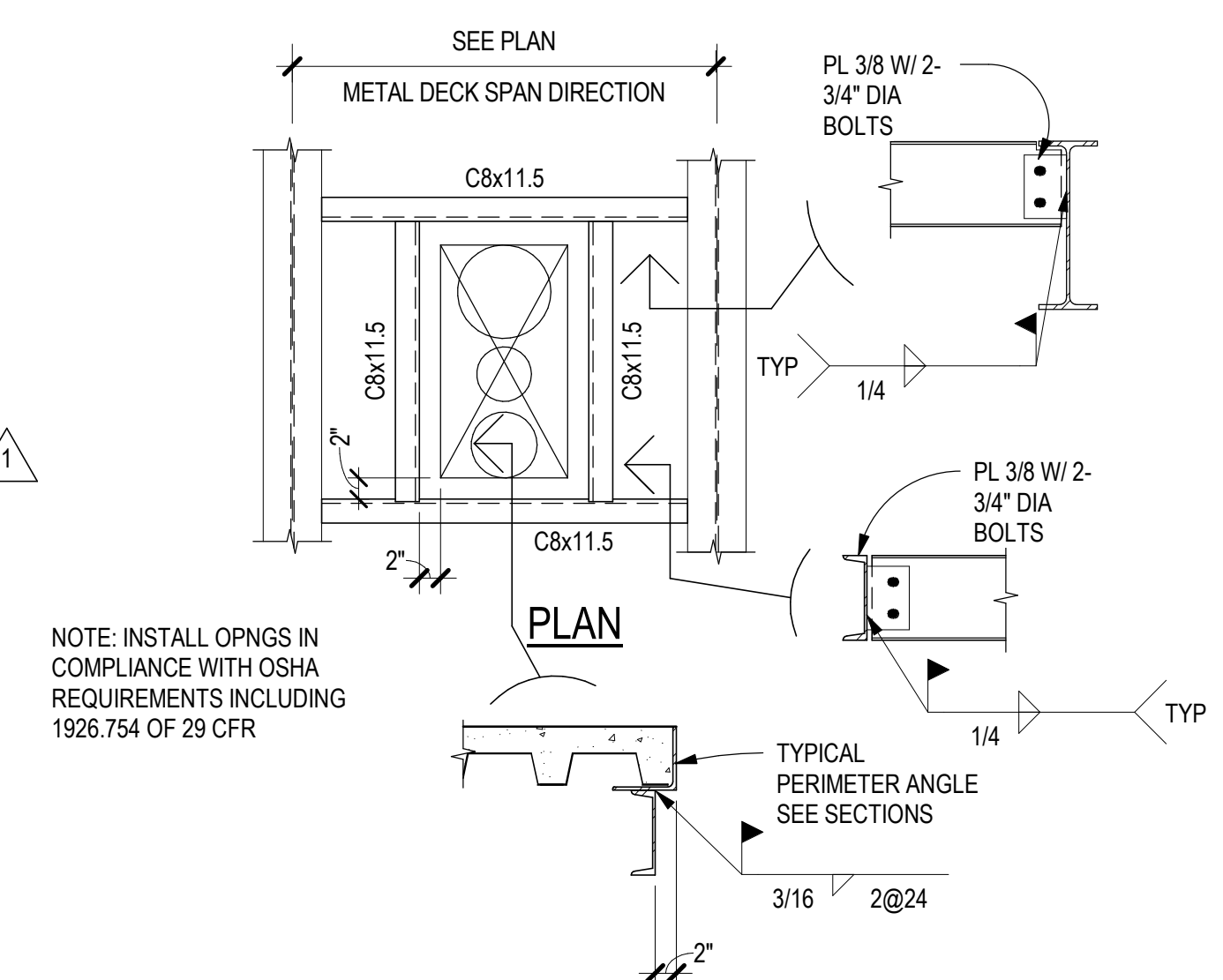
A5 TYPICAL STUDS AT STEMWALL
SCALE: 1\"/>



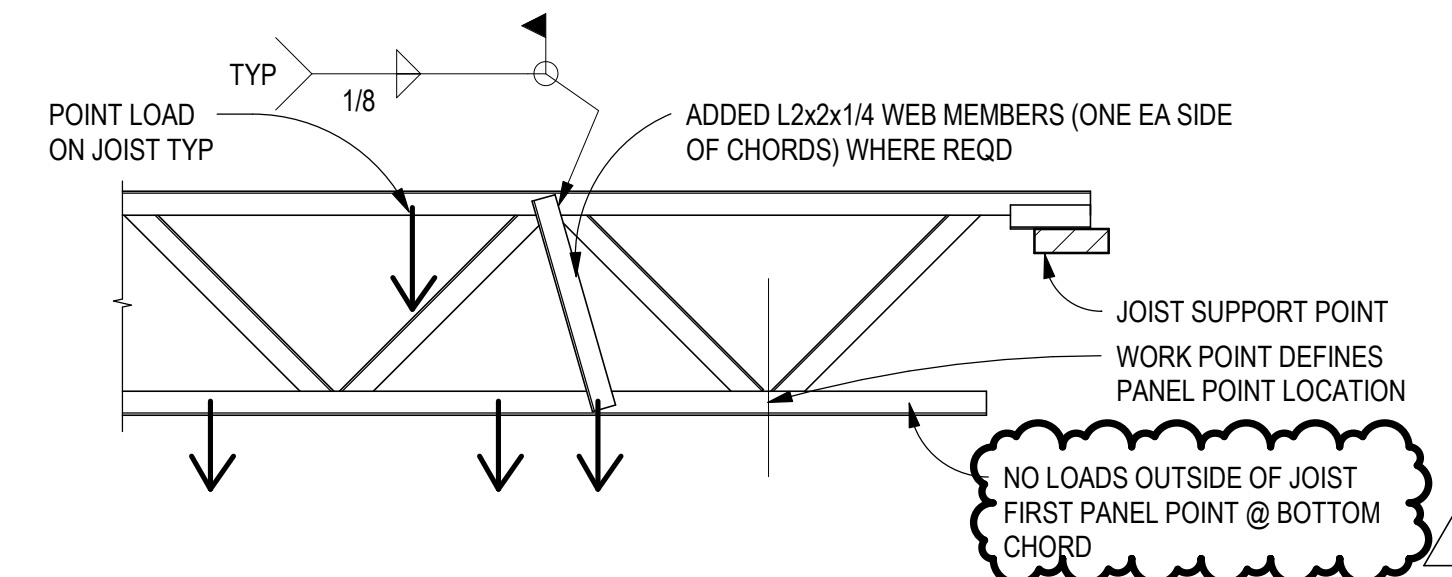
NOTE: INSTALL OPENINGS IN COMPLIANCE WITH OSHA REQUIREMENTS INCLUDING 1926.754 OF 29 CFR



D5 TYPICAL ROOF OPENING 0"-12"
SCALE: 3/4" = 1'-0"



C5 TYPICAL SLAB OPNG > 24"
SCALE: 3/4" = 1'-0"

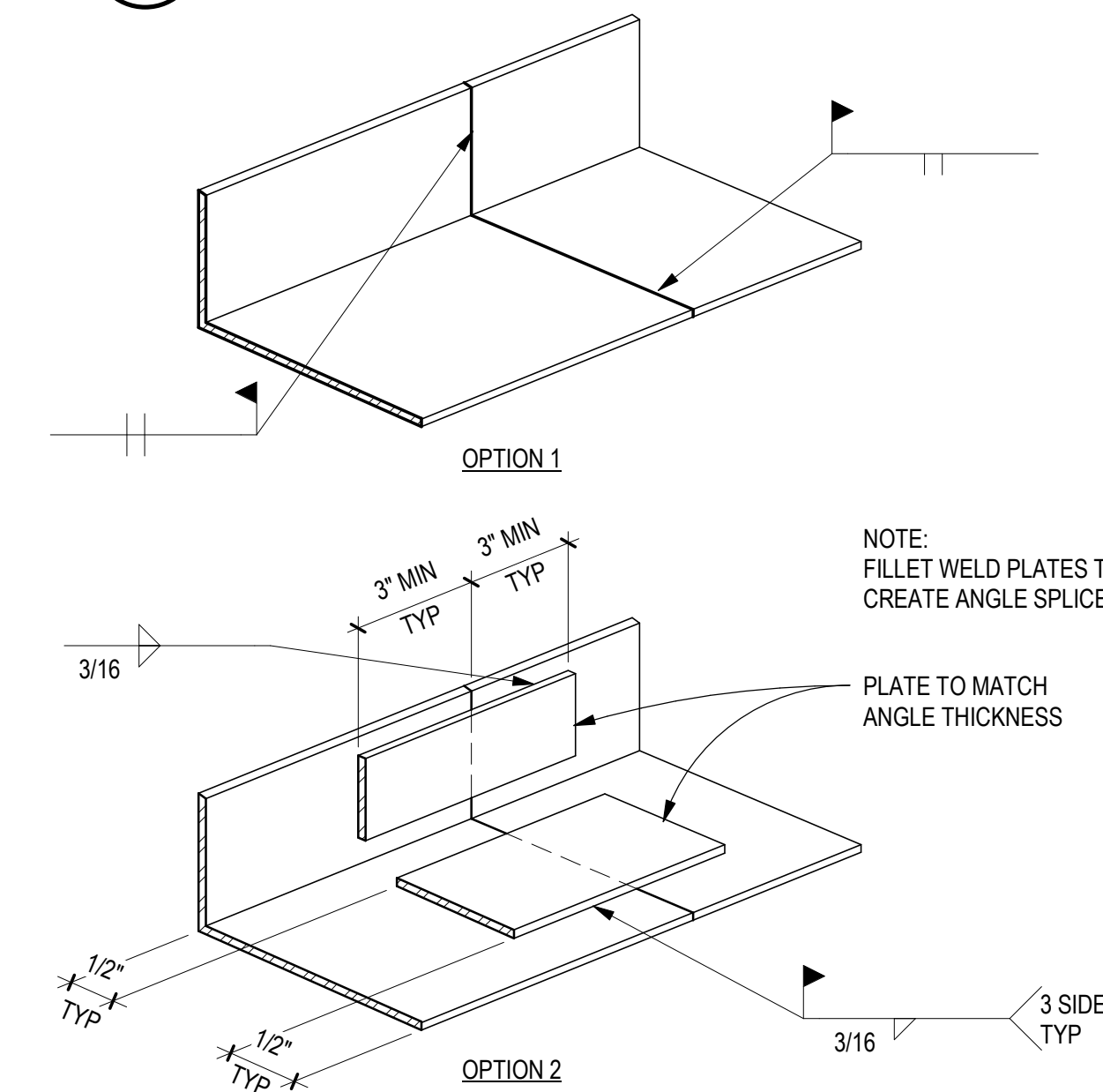


POINT LOADS ON K-SERIES OR LH-SERIES (NON SP JOISTS) SHALL MEET ALL OF THE FOLLOWING REQUIREMENTS:

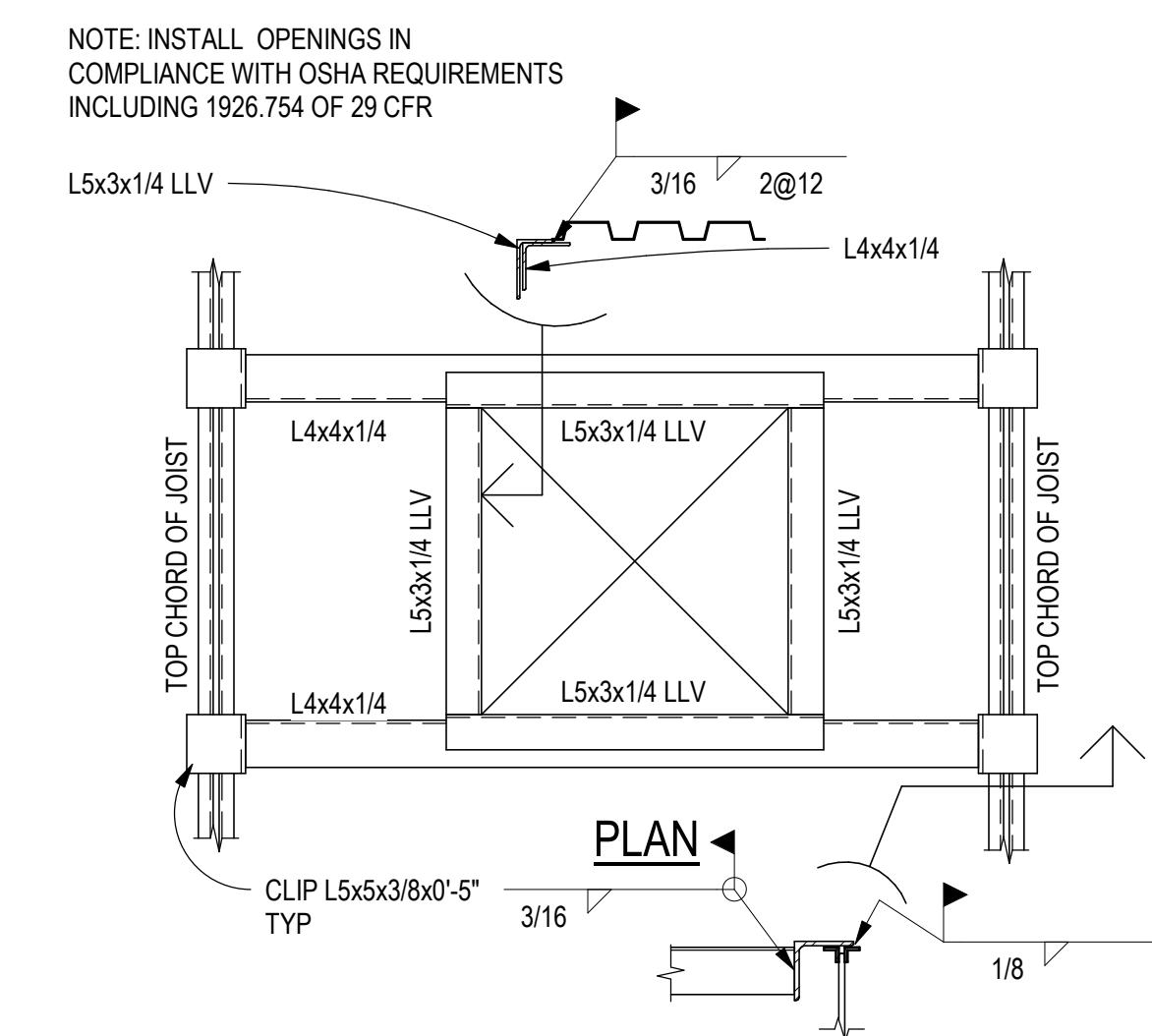
- POINT LOADS ON THE JOIST TOP CHORD OR BOTTOM CHORD THAT EXCEED 100LBS SHALL BE PLACED WITHIN 4" OF A JOIST PANEL POINT, OR ADDITIONAL WEB MEMBERS SHALL BE ADDED AT THE POINT OF LOAD APPLICATION.
- WHERE MULTIPLE POINT LOADS ARE PLACED BETWEEN THE SAME TWO PANEL POINTS, THE SUM OF THOSE LOADS THAT ARE NOT REINFORCED WITH ADDITIONAL WEB MEMBERS SHALL NOT EXCEED 100LBS.
- POINT LOADS SHALL BE CONCENTRIC WITH THE CHORD FROM WHICH IT IS HUNG. BEAM CLAMPS OR OTHER CONNECTIONS THAT INDUCE NON-CONCENTRIC LOADS ARE NOT PERMITTED.
- POINT LOADS SHALL BE SPACED SO THAT THE COMBINED TOP CHORD PLUS BOTTOM CHORD POINT LOADS DO NOT EXCEED AN EQUIVALENT LINE LOAD OF 65PLF AT ANY POINT ALONG THE JOIST.
- LOADS SHALL NOT BE PLACED ON THE BOTTOM CHORD OUTSIDE OF THE FIRST PANEL POINT.
- POINT LOADS IN EXCESS OF 350LBS THAT ARE NOT SPECIFICALLY NOTED ON THE STRUCTURAL DRAWINGS SHALL NOT BE PLACED WITHOUT APPROVAL FROM THE ENGINEER OF RECORD AND THE JOIST MANUFACTURER.

NOTE: JOISTS LABELED AS "SP" HAVE BEEN DESIGNED FOR THE LOADS NOTED ON THE STRUCTURAL PLANS. THOSE LOADS SHALL BE PLACED WITHIN 2" OF A PANEL POINT, OR ADDITIONAL WEB MEMBERS SHALL BE ADDED.

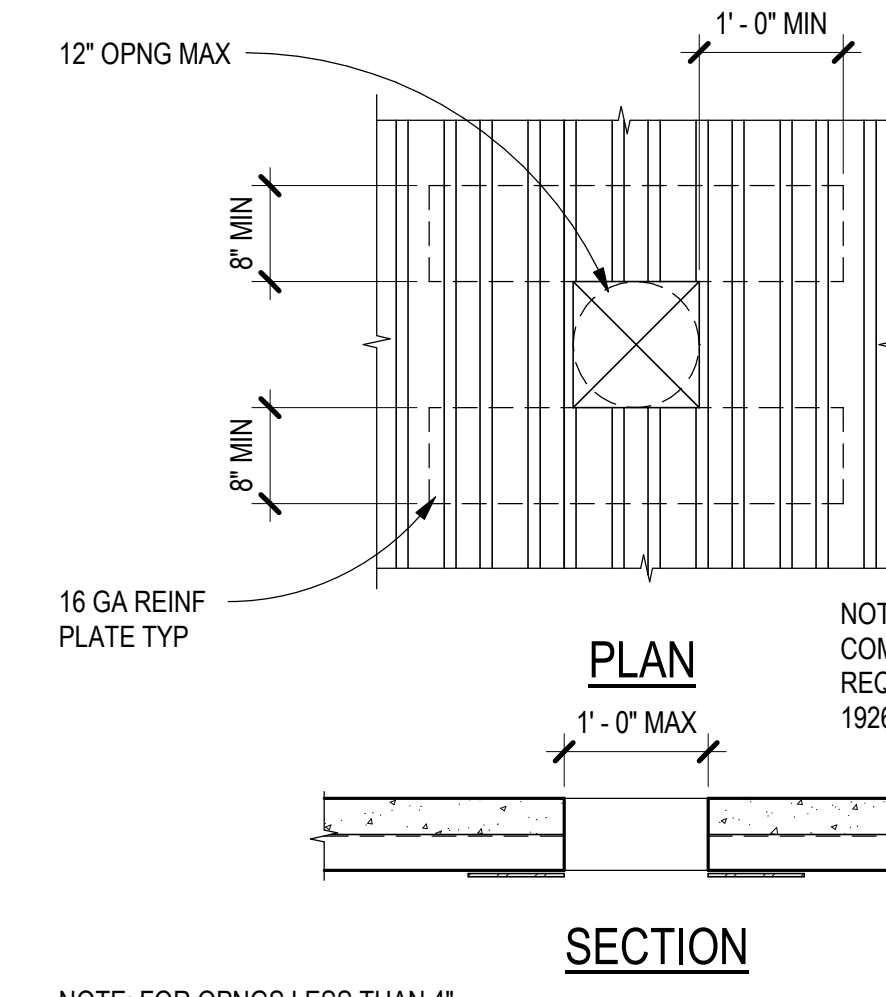
C4 TYPICAL JOIST REINFORCING
SCALE: 1/2" = 1'-0"



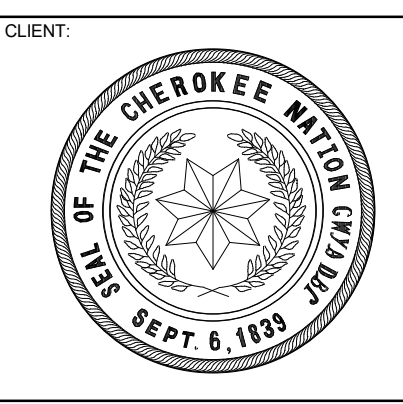
B4 TYPICAL PERIMETER ANGLE SPLICE
SCALE: 3/4" = 1'-0"



A4 TYPICAL ROOF OPENING > 12"
SCALE: 3/4" = 1'-0"



A5 TYPICAL SLAB OPENING 4"-12"
SCALE: 3/4" = 1'-0"



WILMA P. MANKILLER HEALTH CENTER EXPANSION
STILWELL, OKLAHOMA

KEY PLAN:

PROJECT PHASE:
BID PACKAGE 01

#	DATE	REVISIONS / DESCRIPTION
1	11/02/19	BID PACKAGE 01 - ADD 01

DATE: 11-01-19
JOB NUMBER: 18-01.01

SHEET NUMBER:
S7.42
TYPICAL STEEL DETAILS

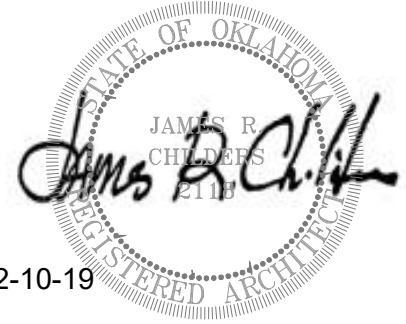


BID PACKAGE 01 – ADDENDUM 02

Date: December 10, 2019

Re: Wilma P Mankiller Health Center Expansion

From: James R Childers Architect, Inc.
45 South 4th Street
Fort Smith, Arkansas 72901



12-10-19

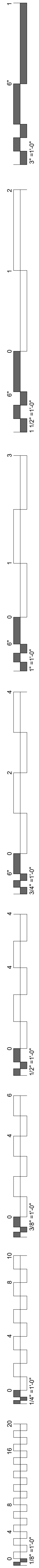
This addendum forms part of the Contract Documents, and modifies the documents as noted below. Acknowledge receipt of this addendum in the space provided on the bid form. Failure to do so may subject the bidder to disqualification.

- Item 01** CS101 /CS102 – Existing 8” sewer line and existing 15” RCP storm line north of building needs to be replaced and lowered.
- Item 02** See attached narrative from Chavez Grieves

Bid Package 01- Addendum 02 – Wilma P. Mankiller Health Center Expansion

Chavez-Grieves would like to incorporate the following revisions into the drawings for the above referenced project.

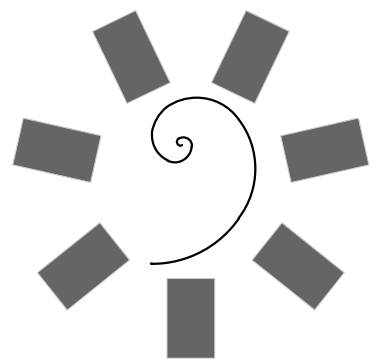
<u>Sheet</u>	<u>Description</u>
S1.02	Rolling door support elevation added near Grid F/1.
S1.12	Rolling door support elevation added near Grid F/1.
S1.13	Stair landing framing adjusted to meet architectural requirements; near Grid E/2. Dimensions added.
S1.21	Sheet keynote 8 added and located on plan.
S1.21	Connection details revised along Grid G, D, and C.
S1.21	Beam sizes near Grid G/8 revised.
S1.21	Bottom flange braces added to beams along Grid G, D and C labeled as "Collector".
S1.22	Sheet keynote 8 added and located on plan.
S1.22	Connection details revised along Grid H, G, and D.
S1.22	Beam size near Grid H/3 revised.
S1.22	Bottom flange braces added to beams along Grid H, G and D labeled as "Collector".
S2.02	Elevations B3 and D4 revised
S3.11	Section D2 revised.
S3.12	Section A3, B3, and D4 added.
S4.01	Plans A3, D2, and D4 revised.
S5.54	Details A2 and B4 added.
S7.41	Detail C2 revised.



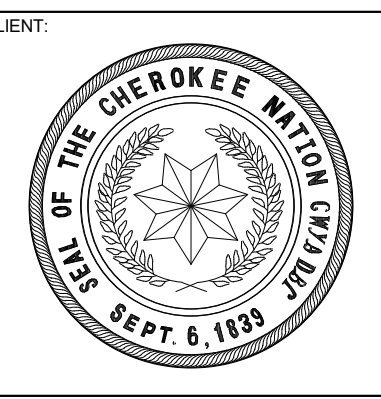
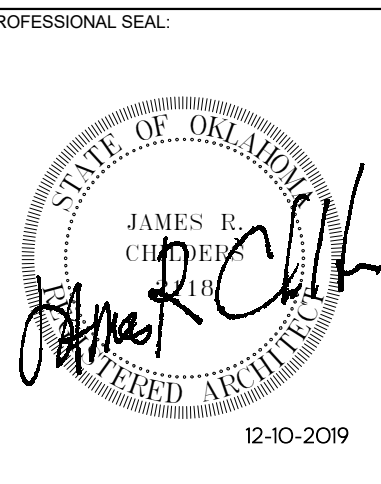
WILMA P. MANKILLER HEALTH CENTER EXPANSION

BID PACKAGE 01 (DEMOLITION / STEEL / FOUNDATIONS)

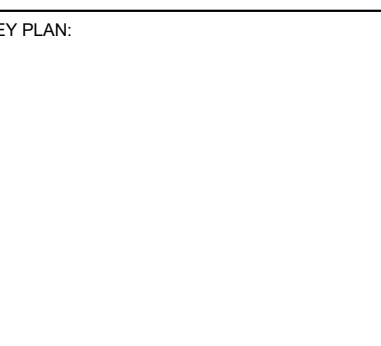
SHEET NUMBER	SHEET NAME	11-01-19 - BID PACKAGE 01	11-22-19 - BID PACKAGE 01 - ADDENDUM 01	12-10-19 - BID PACKAGE 01 - ADDENDUM 02
GENERAL				
G0.01	COVER / INDEX			
CIVIL				
C002	GENERAL NOTES			
CS100	EXISTING SITE PLAN			
CS101	DEMOLITION PLAN			
CS102	DEMOLITION PLAN			
CE100	EROSION CONTROL SITE PLAN			
CE500	EROSION CONTROL DETAILS			
ARCHITECTURAL				
A0.01	OVERALL BUILDING DEMOLITION PLAN			
STRUCTURAL				
S0.01	ABBREVIATIONS AND LEGENDS			
S0.02	GENERAL STRUCTURAL NOTES			
S0.03	GENERAL STRUCTURAL NOTES AND SPECIAL INSPECTIONS			
SD0.01	DEMOLITION GENERAL STRUCTURAL NOTES			
SD1.01	DEMOLITION PLANS - SECTOR 1			
SD2.01	DEMOLITION SECTIONS			
S1.00	OVERALL PLAN - FOUNDATION			
S1.01	FOUNDATION PLAN SECTOR 1			
S1.02	FOUNDATION PLAN SECTOR 2			
S1.10	OVERALL PLAN - FLOOR FRAMING			
S1.11	FLOOR FRAMING PLAN - SECTOR 1			
S1.12	FLOOR FRAMING PLAN - SECTOR 2			
S1.13	LOW ROOF FRAMING PLAN			
S1.20	OVERALL PLAN - ROOF FRAMING			
S1.21	ROOF FRAMING PLAN - SECTOR 1			
S1.22	ROOF FRAMING PLAN - SECTOR 2			
S2.01	MOMENT FRAME ELEVATIONS			
S2.02	MOMENT FRAME AND BRACED FRAME ELEVATIONS			
S3.01	WALL SECTIONS			
S3.02	WALL SECTIONS			
S3.03	WALL SECTIONS			
S3.04	WALL SECTIONS			
S3.11	FOUNDATION SECTIONS			
S3.12	FOUNDATION SECTIONS			
S3.21	FLOOR FRAMING SECTIONS			
S3.31	ROOF FRAMING SECTIONS			
S4.01	ENLARGED PLANS			
S5.21	MASONRY FRAMING SECTIONS AND DETAILS			
S6.41	VERTICAL CIRCULATION DETAILS			
S5.51	STEEL DETAILS			
S5.52	STEEL DETAILS			
S5.53	STEEL DETAILS			
S5.54	STEEL DETAILS			
S6.01	SCHEDULES			
S7.11	TYPICAL CONCRETE DETAILS			
S7.21	TYPICAL MASONRY DETAILS			
S7.31	TYPICAL COLD-FORMED DETAILS			
S7.41	TYPICAL STEEL DETAILS			
S7.42	TYPICAL STEEL DETAILS			
S8.01	SIDEPLATE GENERAL NOTES AND CONSTRUCTION GUIDELINES			
S8.02	SIDEPLATE COLUMN DETAILS, A TYPE			
S8.03	SIDEPLATE COLUMN DETAILS, B TYPE			
S8.04	SIDEPLATE BEAM DETAILS			
S8.05	SIDEPLATE BEAM DETAILS, NARROW			
S8.06	SIDEPLATE FIELD ERECTION DETAILS			
S8.07	SIDEPLATE MISC DETAILS AND COORDINATION ITEMS			
ELECTRICAL				
E0.01	ELECTRICAL DEMOLITION PLAN			
Grand total: 55				



James R. Childers
Architect, Inc.
45 South 4th Street
Fort Smith, AR 72901
479-783-2450
www.childersarchitect.com



**WILMA P. MANKILLER HEALTH CENTER
EXPANSION**
 STILWELL, OKLAHOMA



PROJECT PHASE:
BID PACKAGE 01

#	DATE	REVISIONS / DESCRIPTION
1	11/22/19	BID PACKAGE 01 - ADD 01
2	12/10/19	BID PACKAGE 01 - ADD 02

DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER:
G0.01

COVER / INDEX



1836 SOUTH BALTIMORE AVE.
TULSA, OK 74119
(539) 664-4618

MECHANICAL / ELECTRICAL / PLUMBING ENGINEER



3902 UNIVERSITY BOULEVARD
DURANT, OK 74701
(580) 931-9045

CIVIL ENGINEER



4700 LINCOLN ROAD NE, SUITE 102
ALBUQUERQUE, NM 87109
(505) 344-4080

STRUCTURAL ENGINEER



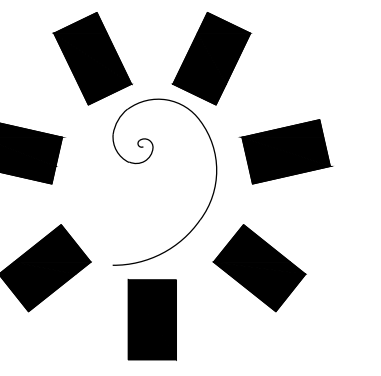
808 TRAVIS STREET, SUITE 200
HOUSTON, TX 77002
(281) 589-5900

FIRE PROTECTION / LIFE SAFETY

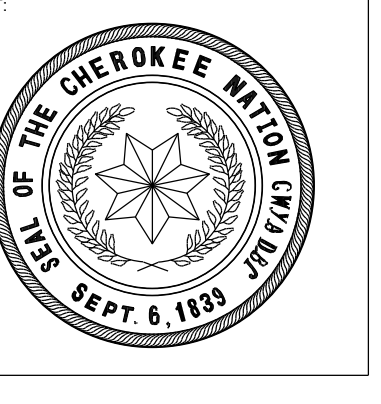
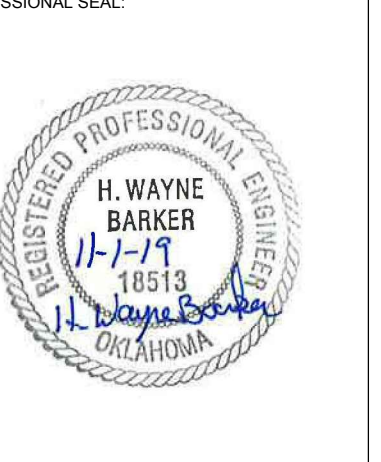


1316 E 35TH PLACE, SUITE 100
TULSA, OK 74105
(918) 382-9120

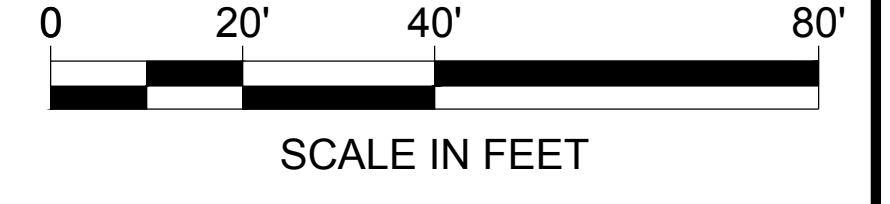
EQUIPMENT PLANNER



James R. Childers
Architect, Inc.
45 South 4th Street
Ft. Smith, AR 72001
479-763-2460
www.childersarchitect.com



**WILMA P. MANKILLER HEALTH CENTER
EXPANSION
STILWELL, OKLAHOMA**

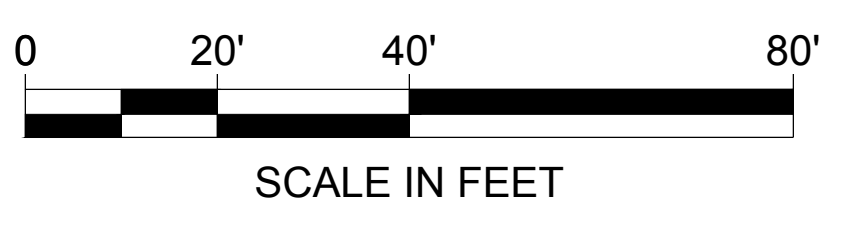


LEGEND

--- APROX. LIMITS OF DEMOLITION.

DEMOLITION LEGEND:

- A 114± LF. OF UNDERGROUND ELECTRIC LINE TO BE REMOVED AND RELOCATED FROM UTILITY CUT OFF POINT TO EDGE OF BUILDING.
- B 106± LF. OF TELEPHONE LINE TO BE REMOVED AND RELOCATED FROM UTILITY CUT OFF POINT TO EDGE OF BUILDING.
- C 131± LF. OF WATER LINE TO BE REMOVED AND RELOCATED FROM UTILITY CUT OFF POINT TO EDGE OF BUILDING.
- D EXISTING INLET TO REMAIN. RE: NOTE 4
- E EXISTING SITE VEGETATION TO BE REMOVED. RE: NOTE 4
- F EXISTING IRRIGATION VALVE TO BE REMOVED. RE: NOTE 4
- G EXISTING STORM MANHOLE TO BE REMAIN. RE: NOTE 4
- H EXISTING HVAC, GENERATOR & ELECTRICAL TRANSFORMER TO BE REMOVED. REFER TO MEP.
- I 173± OF GAS LINE TO BE REMOVED AND RELOCATED FROM UTILITY CUT OFF POINT TO OUT SIDE OF BUILDING.
- J EXISTING CURB TO BE REMOVED.
- K EXISTING YARD LIGHT TO BE REMOVED.
- L CONTRACTOR TO VERIFY IF EXISTING ELECTRIC BOX NEEDS TO BE REMOVED OR RELOCATED.
- M APPROXIMATE LOCATION OF EXISTING FIRE HYDRANT. TO REMOVED ONLY AFTER INSTALLATION OF NEW FIRE HYDRANT.
- N EXISTING COLUMN TO BE REMOVED, WITH CORRESPONDING ROOF SECTION.
- O EXISTING FLUME TO BE REMOVED.
- P EXISTING LIGHT POLE TO BE REMOVED.
- Q EXISTING SITE SIGNAGE TO BE REMOVED/RELOCATE RE: OWNER/ARCH.
- R EXISTING WALL SECTION TO BE REMOVED.
- S EXISTING PAVEMENT TO REMAIN.
- T EXISTING SEWER/STORM MANHOLE TO BE REMAIN AND ADJUSTED TO NEW GRADE IN FUTURE BID PACKAGE.
- U EXISTING STORM INLET TOP TO BE REMOVED FOR FUTURE REPLACEMENT WITH SOLID LID TOP.
- V EXISTING UNDERGROUND UTILITY TO REMAIN IN PLACE DURING CONSTRUCTION. CONTRACTOR TO ENSURE NOT DISTURB OR DAMAGE DURING CONSTRUCTION.
- W 18± LF. OF SEWER LINE TO BE REMOVED AND REPLACED FROM UTILITY CUT OFF POINT TO EDGE OF BUILDING.



NOTE:

- 1 IRRIGATION LINES & EQUIPMENT MAY BE PRESENT THROUGHOUT THE LIMITS OF CONSTRUCTION. CONTRACTOR SHALL VERIFY LIMIT OF IRRIGATION SYSTEM AND COORDINATE WITH OWNER. CUT/PLUG EXISTING LINES TO CLEAR CONSTRUCTION AND MAINTAIN OPERATION OF OTHER ZONES.
- 2 UTILITY SERVICE LINES SUCH AS WATER, AND NATURAL GAS SHALL BE ISOLATED AT UTILITY MAIN CONNECTION, TO ISOLATE CONSTRUCTION SITE WHILE MAINTAINING SERVICE TO OTHERS.
- 3 QUANTITIES ARE SHOWN FOR INFORMATION ONLY. CONTRACTOR TO FIELD VERIFY QUANTITIES.
- 4 EXISTING STORM SYSTEM TO REMAIN IN PLACE AND WILL BE REMOVED IN FUTURE BID PACKAGE. AFTER NEW STORM SYSTEM IS PLACED.
- 5 UTILITY CUT OFF POINT SYMBOL. THE BEGINNING POINT OF ANY UTILITY (GAS, WATER, SEWER, & ELECTRIC) LINE TO BE REMOVED.
- 6 EXISTING 8" SEWER LINE TO BE REPLACED & LOWERED. RE: FUTURE BID PACKAGE.
- 7 EXISTING 15" Ø RCP PIPE TO BE REPLACED & LOWERED. RE: FUTURE BID PACKAGE.

95,152± SQFT. OF EXISTING CONCRETE PAVING TO BE REMOVED.

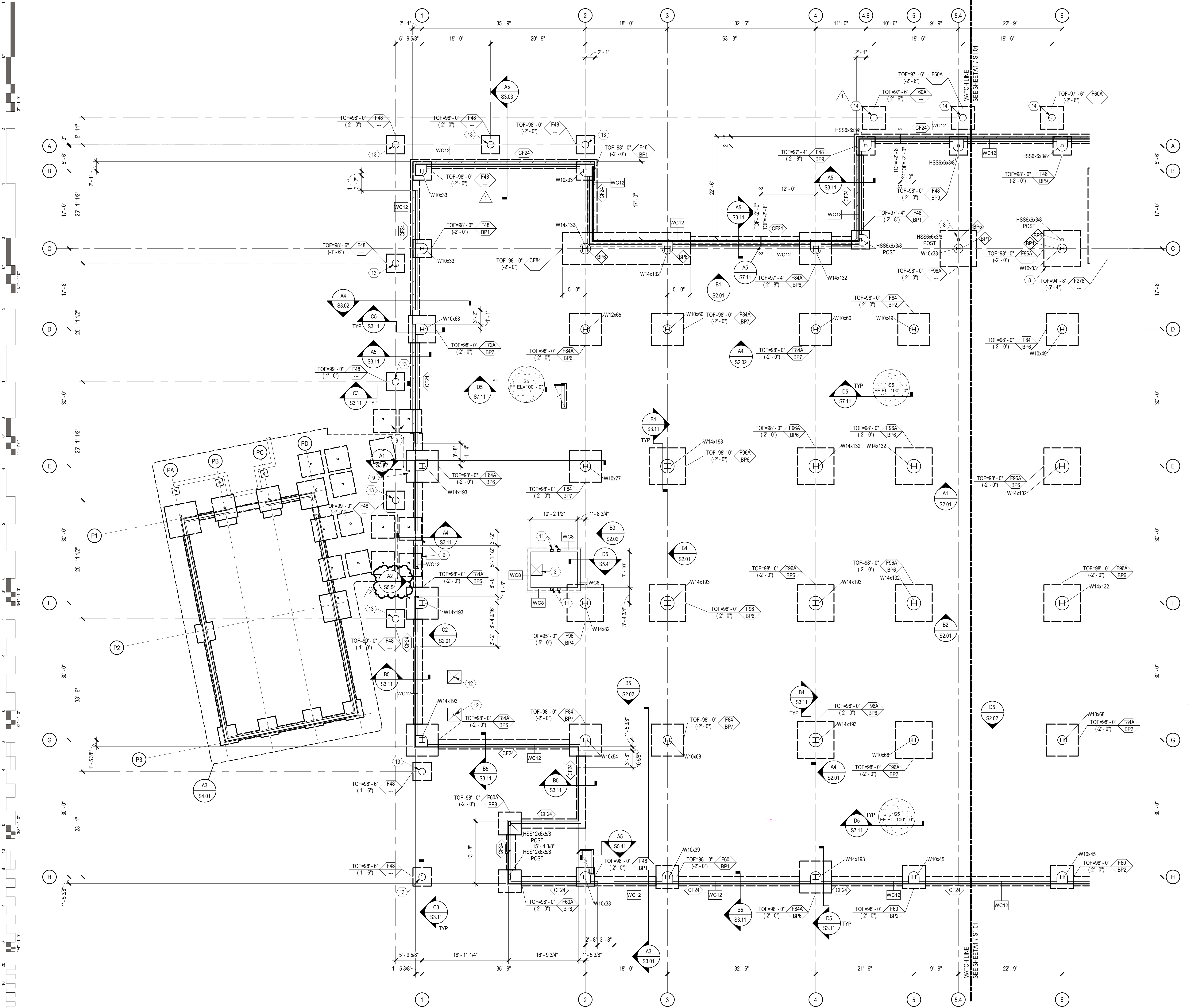
KEY PLAN

PROJECT PHASE:
BID PACKAGE 01

#	DATE	REVISIONS
1	12/10/19	BID PACKAGE 01 - ADD 02

DATE: 11.01.19 JOB NUMBER: 18-01.01

SHEET NUMBER:
CS101
DEMOLITION PLAN



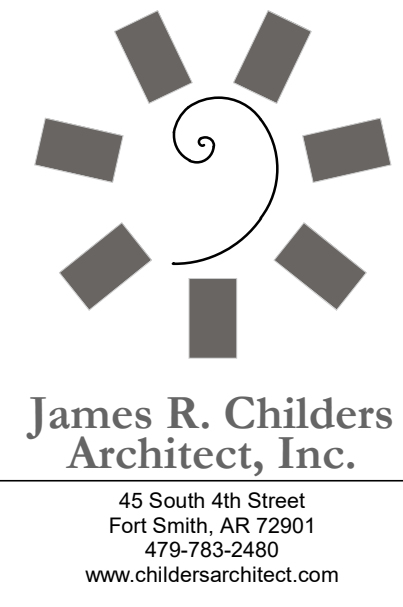
A1 FOUNDATION PLAN - SECTOR 2
SCALE: 1/8" = 1'-0"

GENERAL SHEET NOTES

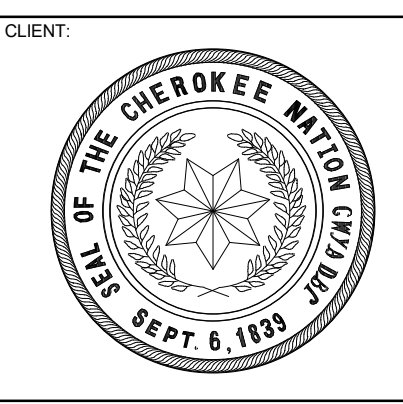
- SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- REFERENCE FINISH FLOOR ELEVATION 100'-0" = MEAN SEA FINISH FLOOR ELEVATION. SEE CIVIL DRAWINGS.
- TOP OF FOOTING ELEVATION = 98'-0" (-2'-0"), UNLESS NOTED OTHERWISE ON PLAN.
- NOTE TO CONTRACTOR: ENLARGED SLAB BLOCKOUTS MAY BE REQUIRED AT FRAME COLUMNS FOR MOMENT FRAME BASE PLATE CLEARANCE.
- NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
- DIMENSIONS ARE TO THE FACE OF STUD UNLESS NOTED OTHERWISE.
- SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
- EXISTING CONSTRUCTION IS PER AVAILABLE EXISTING DRAWINGS. ALL EXISTING CONSTRUCTION AND DIMENSIONS SHALL BE VERIFIED PRIOR TO CONSTRUCTION. SHOULD CONDITIONS VARY FROM THOSE SHOWN, CONTACT ENGINEER BEFORE PROCEEDING.
- PROVIDE SLAB JOINTS AT 10'-0" ON CENTER MAXIMUM. THE AREA OF THE CONTROL JOINT SHALL NOT EXCEED A 2:1 RATIO. CONTROL JOINTS SHALL BE LOCATED AT COLUMN LINES WHERE THE LAYOUT PERMITS. AT RE-ENTRANT CORNERS THAT DO NOT HAVE CONTROL JOINTS, PROVIDE 2#4 x 3'-0" DIAGONAL TO THE RE-ENTRANT CORNER.
- STRUCTURAL COLD FORMED METAL STUDS SHALL BE 6" WIDE UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING BY OTHERS.
- SEE SHEET S7.00 SERIES SHEETS FOR TYPICAL FOUNDATION SECTIONS AND DETAILS.
- SEE SHEET S6.01 FOR SCHEDULES.

SHEET KEYNOTE

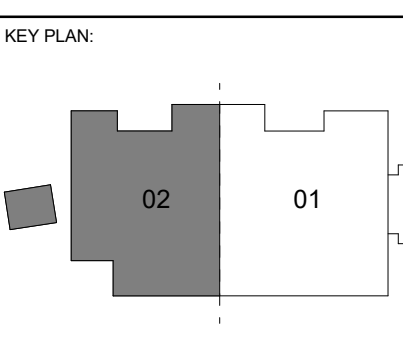
- FLOOR DRAIN, SLOPE SLAB TO DRAIN 1/8" PER FOOT. COORDINATE EXACT SIZE AND LOCATION WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
- POST AND FOUNDATION AS REQUIRED FOR STAIR SUPPORT. STAIR ENGINEER TO PROVIDE REQUIRED LOADS AND LOCATIONS.
- ELEVATOR SUMP PIT. COORDINATE EXACT SIZE AND LOCATION WITH ELEVATOR MANUFACTURER. SEE A4 / S5.41
- HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION AND SPACING WITH ELEVATOR MANUFACTURER. SEE B4 / S5.41
- PRE-ENGINEERED METAL BUILDING STEEL AND ANCHORAGE BY OTHERS. CONTRACTOR TO CONFIRM LOCATIONS OF FOUNDATIONS WITH FINAL PRE-ENGINEERED METAL BUILDING SHOP DRAWINGS.
- NOTCH MASONRY AS REQUIRED TO FACILITATE BASEPLATE INSTALLATION. STEP BOND BEAM AT THIS LOCATION. FILL VOID FROM NOTCH WITH NON-SHRINK GROUT.
- CUT AND REMOVE EXISTING SLAB AS REQUIRED TO PLACE NEW FOOTING. NEW SLAB TO POUR UP TO REMAINING SLAB.
- CENTER FOOTING ON GRID C.
- F60A PRE-MANUFACTURED SUNSHADE CONCRETE FOOTING. TOP OF FOOTING = 99'-0" (-1'-0"). SEE SHEET S6.01 FOR FOOTING SCHEDULE. COORDINATE FINAL LOCATION WITH SUNSHADE MANUFACTURER.
- EXISTING CANOPY. SEE ARCHITECTURAL DEMOLITION PLANS FOR EXTENT OF DEMOLITION.
- HSS6x6x1/2 ELEVATOR SUPPORT POST. COORDINATE EXACT LOCATION AND SPACING WITH ELEVATOR MANUFACTURER. SEE B4 / S5.41, D3 / S5.41, A2 / S5.41, B2 / S5.41, AND C2 / S5.41
- 1 1/2" RECESSED SLAB AT ADA SHOWER. COORDINATE EXACT SIZE, LOCATION, AND SLOPE REQUIREMENTS WITH ARCHITECTURAL DRAWINGS. SEE C4 / S7.11
- 18" DIAMETER PRECAST CONCRETE COLUMN BY OTHERS. SEE C3 / S3.11 AND B1 / S3.31
- 18" DIAMETER PRECAST CONCRETE CANOPY COLUMN BY OTHERS. SEE C3 / S3.11, C4 / S3.12, A1 / S3.31, AND A5 / S3.31
- F60A PRE-MANUFACTURED SUNSHADE CONCRETE FOOTING WITH 18" SQUARE CONCRETE PEDESTAL. TOP OF FOOTING ELEVATION TO MATCH TOP OF FOOTING ELEVATION OF SITE RETAINING WALL. SEE SHEET S6.01 FOR FOOTING SCHEDULE. SEE B3 / S3.12 FOR PEDESTAL DETAIL. ANCHORAGE AND SUPPLEMENTAL ANCHOR REINFORCEMENT FOR PRE-MANUFACTURED CANOPY TO BE PROVIDED BY MANUFACTURER.
- SITE RETAINING WALL. SEE D2 / S3.11
- DOCK LEVELER. SEE ARCHITECTURAL FOR EXACT LOCATION AND DIMENSIONS.



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WILMA P. MANKILLER HEALTH CENTER EXPANSION
STILWELL, OKLAHOMA



PROJECT PHASE:
BID PACKAGE 01

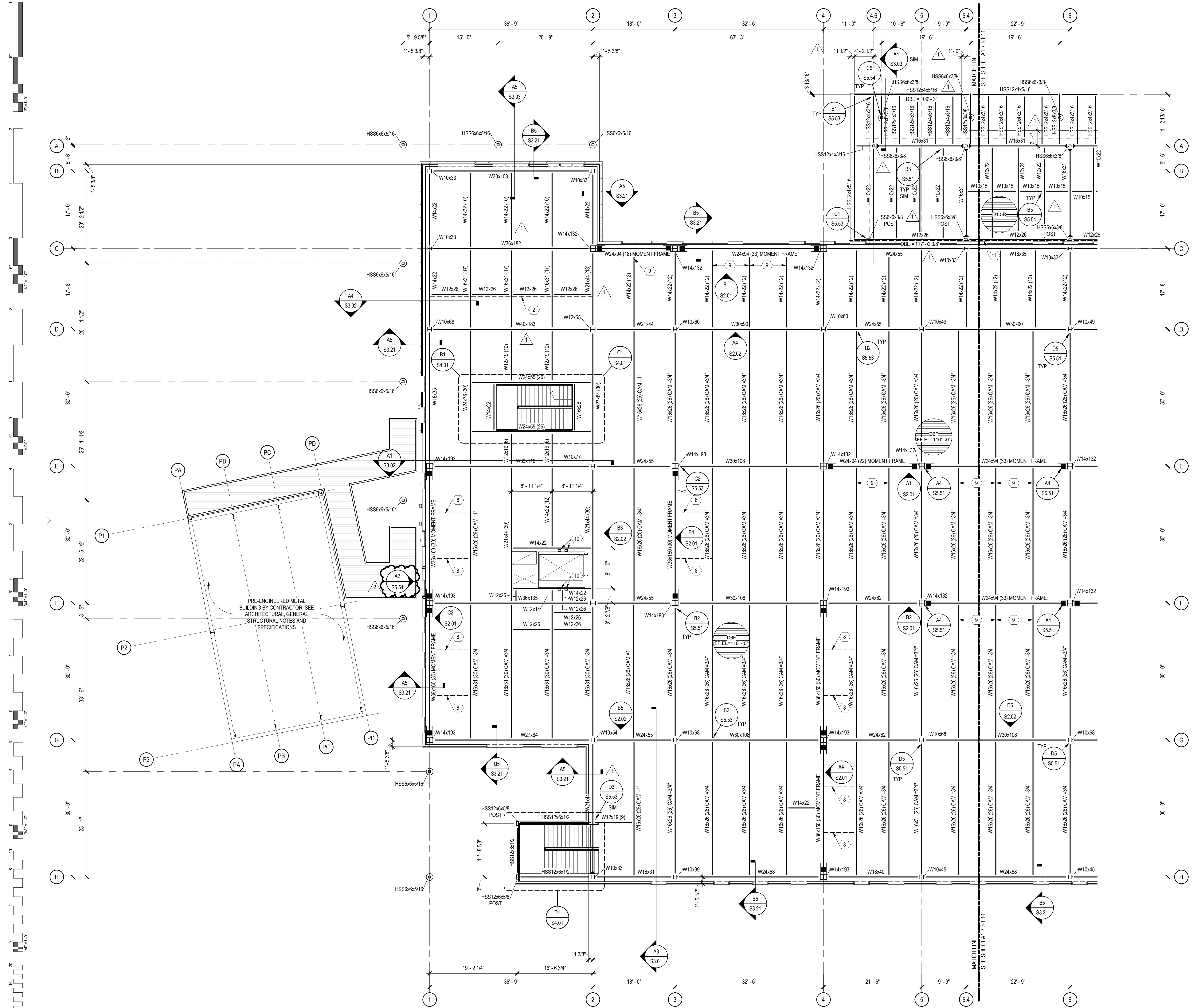
#	DATE	REVISIONS / DESCRIPTION
1	11/22/19	BID PACKAGE 01 - ADD 01
2	12/19/19	BID PACKAGE 01 - ADD 02

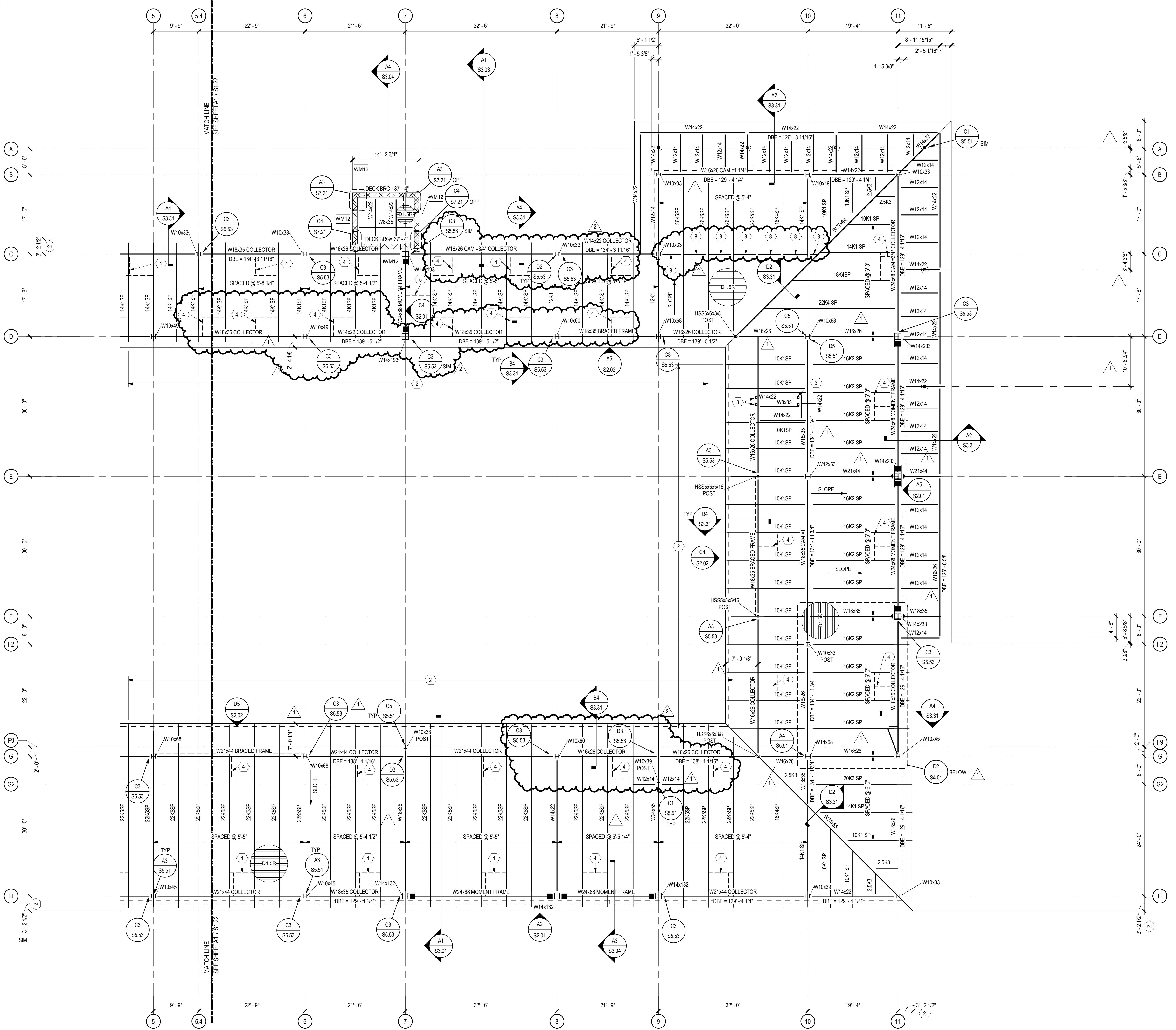
DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER: S1.02

FOUNDATION PLAN SECTOR 2





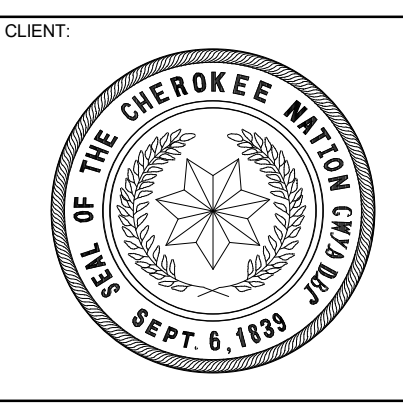


GENERAL SHEET NOTES

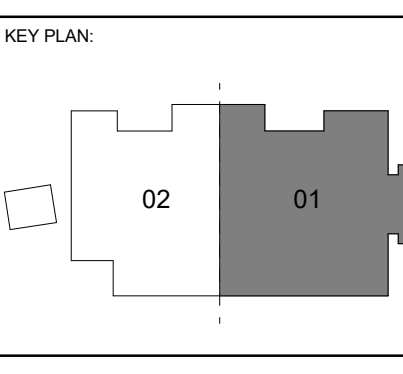
- SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
- DIMENSIONS ARE TO THE FACE OF STUD UNLESS NOTED OTHERWISE.
- SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
- EXISTING CONSTRUCTION IS PER AVAILABLE EXISTING DRAWINGS. ALL EXISTING CONSTRUCTION AND DIMENSIONS SHALL BE VERIFIED PRIOR TO CONSTRUCTION. SHOULD CONDITIONS VARY FROM THOSE SHOWN, CONTACT ENGINEER BEFORE PROCEEDING.
- BEAMS AND JOISTS ARE SPACED EQUALLY BETWEEN GRIDS AND COLUMNS UNLESS NOTED OTHERWISE.
- PROVIDE JOIST BRIDGING PER THE 42ND EDITION OF THE SJI SPECIFICATIONS AND OSHA REQUIREMENTS.
- STEEL JOIST MANUFACTURER SHALL DESIGN ROOF JOISTS AND ROOF JOIST GIRDERS SUPPORTING MECHANICAL UNITS FOR 1.2x MECHANICAL UNIT WEIGHTS SHOWN. USE 28 PSF DEAD LOAD AND 20 PSF LIVE LOAD UNLESS NOTED OTHERWISE. CONTRACTOR SHALL VERIFY ACTUAL MECHANICAL LOADS. NOTIFY STEEL JOIST MANUFACTURER OF ANY DISCREPANCIES.
- STRUCTURAL COLD FORMED METAL STUDS SHALL BE 6" IN WIDTH, UNLESS NOTED OTHERWISE.
- SEE SHEET S7.00 SERIES SHEETS FOR TYPICAL ROOF FRAMING SECTIONS.
- SEE SHEET S6.01 FOR SCHEDULES.
- △ DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
- ◻ DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE DRAWINGS.
- PROVIDE HSSxJOIST SEAT DEPTHx1/4 BETWEEN JOISTS AT ALL BEAMS LABELED AS : MOMENT FRAME, BRACED FRAME, AND COLLECTOR. SEE C2 / S7.41

SHEET KEYNOTE

- MECHANICAL UNIT. COORDINATE EXACT SIZE AND LOCATION WITH MECHANICAL DRAWINGS.
- JOIST EXTENDED END. DESIGN EXTENDED END FOR 20 PSF DEAD LOAD, 20 PSF LIVE LOAD, AND ANY POSITIVE OR NEGATIVE WIND PRESSURES PER ROOF WIND LOADING DIAGRAM ON S0.03. DEPTH OF EXTENDED END PER JOIST MANUFACTURER.
- HSS5x5x1/8 ELEVATOR RAIL SUPPORT POST. COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE A2 / S5.41, B2 / S5.41, C2 / S5.41, AND D3 / S5.41
- BOTTOM FLANGE BRACE AT EQUAL SPACING, UNLESS NOTED OTHERWISE. BRACE TO BE ATTACHED TO BOTTOM FLANGE OF BEAM NOTED AS MOMENT FRAME OR BRACED FRAME AND TO TOP FLANGE OF ADJACENT BEAM OR JOIST. SEE B3 / S5.52. JOISTS TO BE DESIGNED FOR 1,500# VERTICAL (REVERSIBLE) WIND AND SEISMIC LOAD FROM BRACE.
- BOTTOM FLANGE BRACING AT EQUAL SPACING, UNLESS NOTED OTHERWISE. SEE D1 / S5.51. JOISTS TO BE DESIGNED FOR 1,500# VERTICAL (REVERSIBLE) WIND AND SEISMIC LOAD FROM BRACE.
- BOTTOM FLANGE BRACING ANGLE. SEE A3 / S5.52
- R1 JOIST EXTENDED END.
- 14x4x3/8 BETWEEN JOISTS. PROVIDE DECK ATTACHMENTS PER SCHEDULE AT ANGLES.



**WILMA P. MANKILLER HEALTH CENTER
EXPANSION**
STILWELL, OKLAHOMA



PROJECT PHASE:
BID PACKAGE 01

#	DATE	REVISIONS / DESCRIPTION
1	11/22/19	BID PACKAGE 01 - ADD 01
2	12/19/19	BID PACKAGE 01 - ADD 02

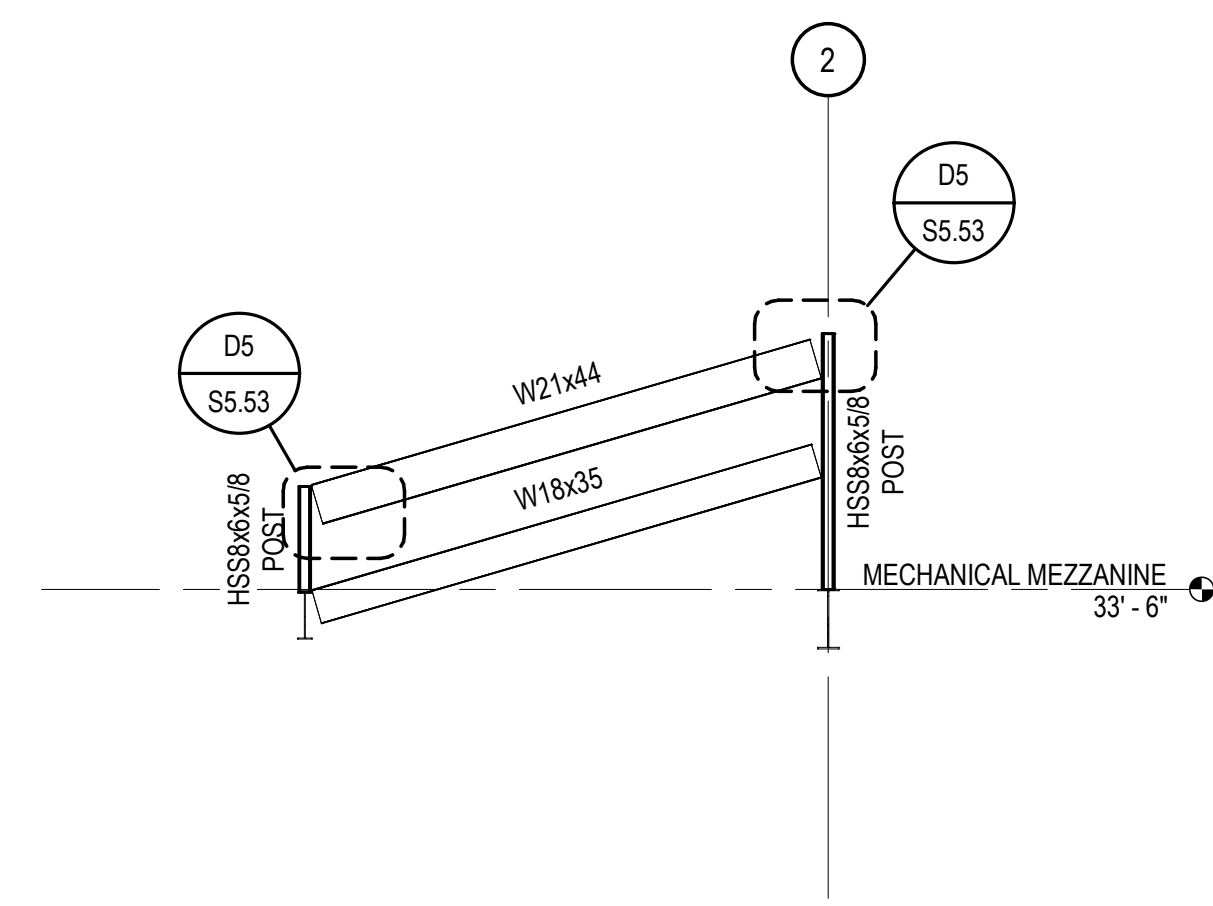
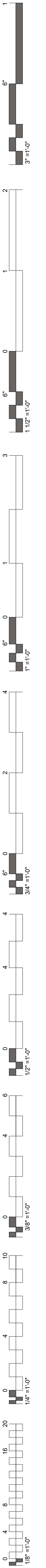
DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER: S1.21

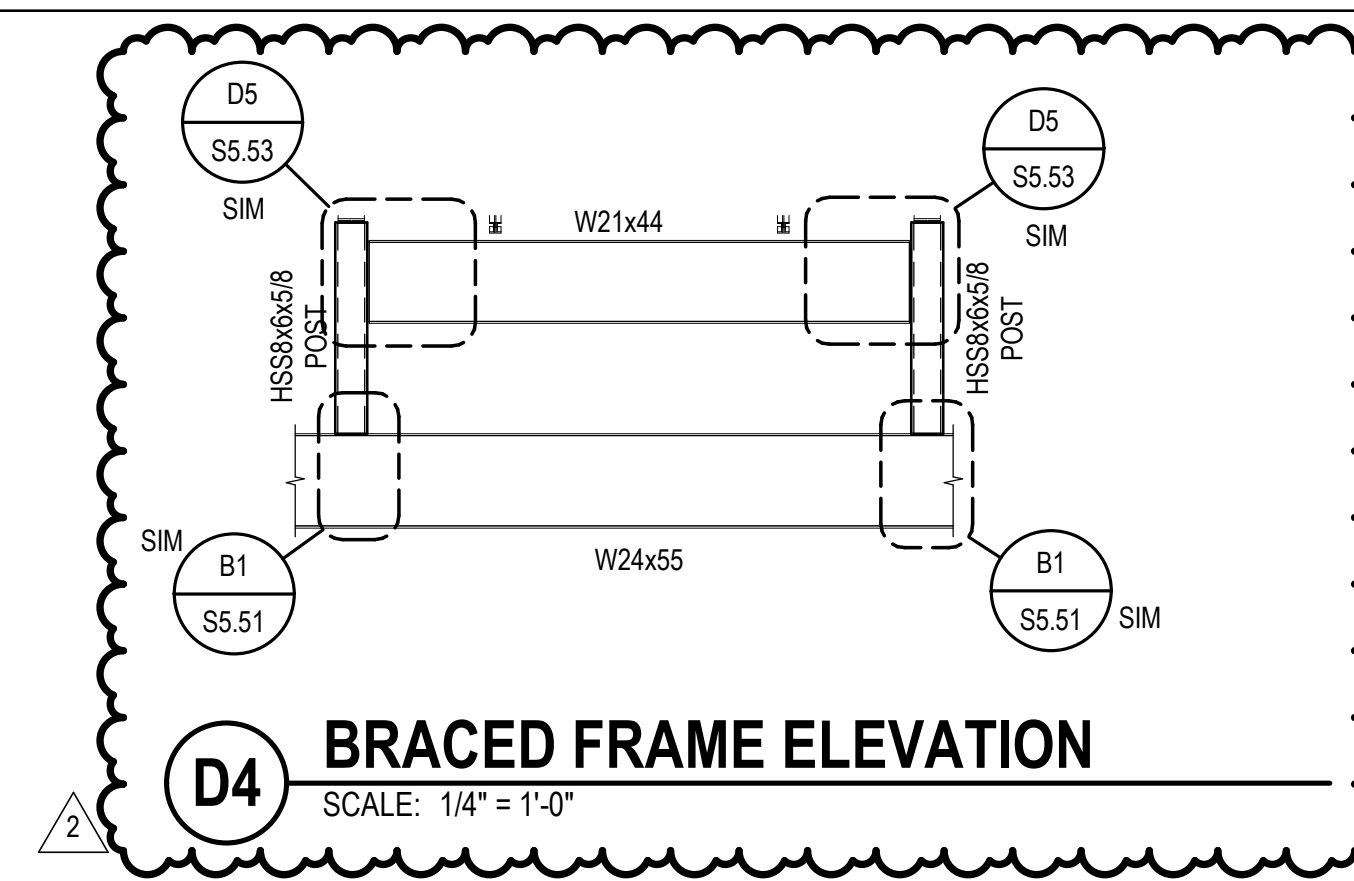
ROOF FRAMING PLAN - SECTOR 1

A1 ROOF FRAMING PLAN - SECTOR 1
SCALE: 1/8" = 1'-0"

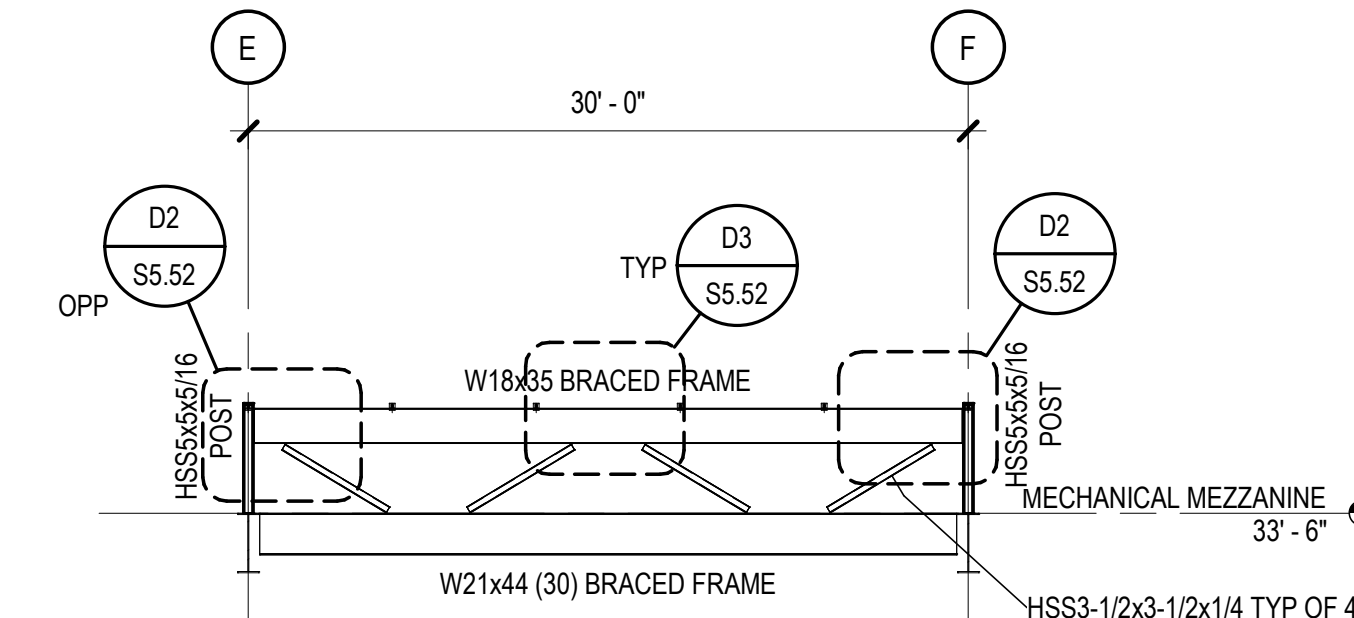




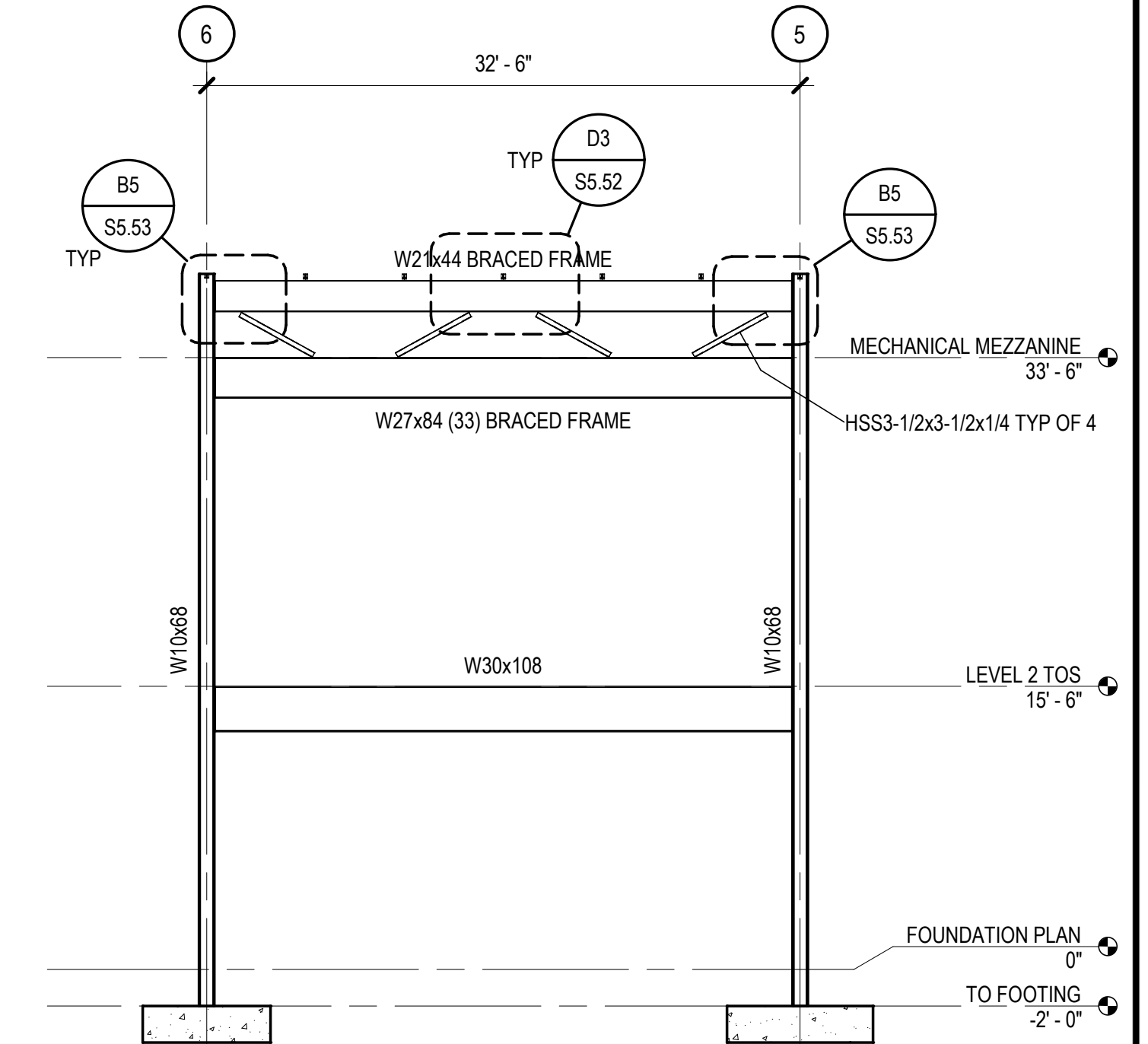
C3 BRACED FRAME ELEVATION
SCALE: 1/8" = 1'-0"



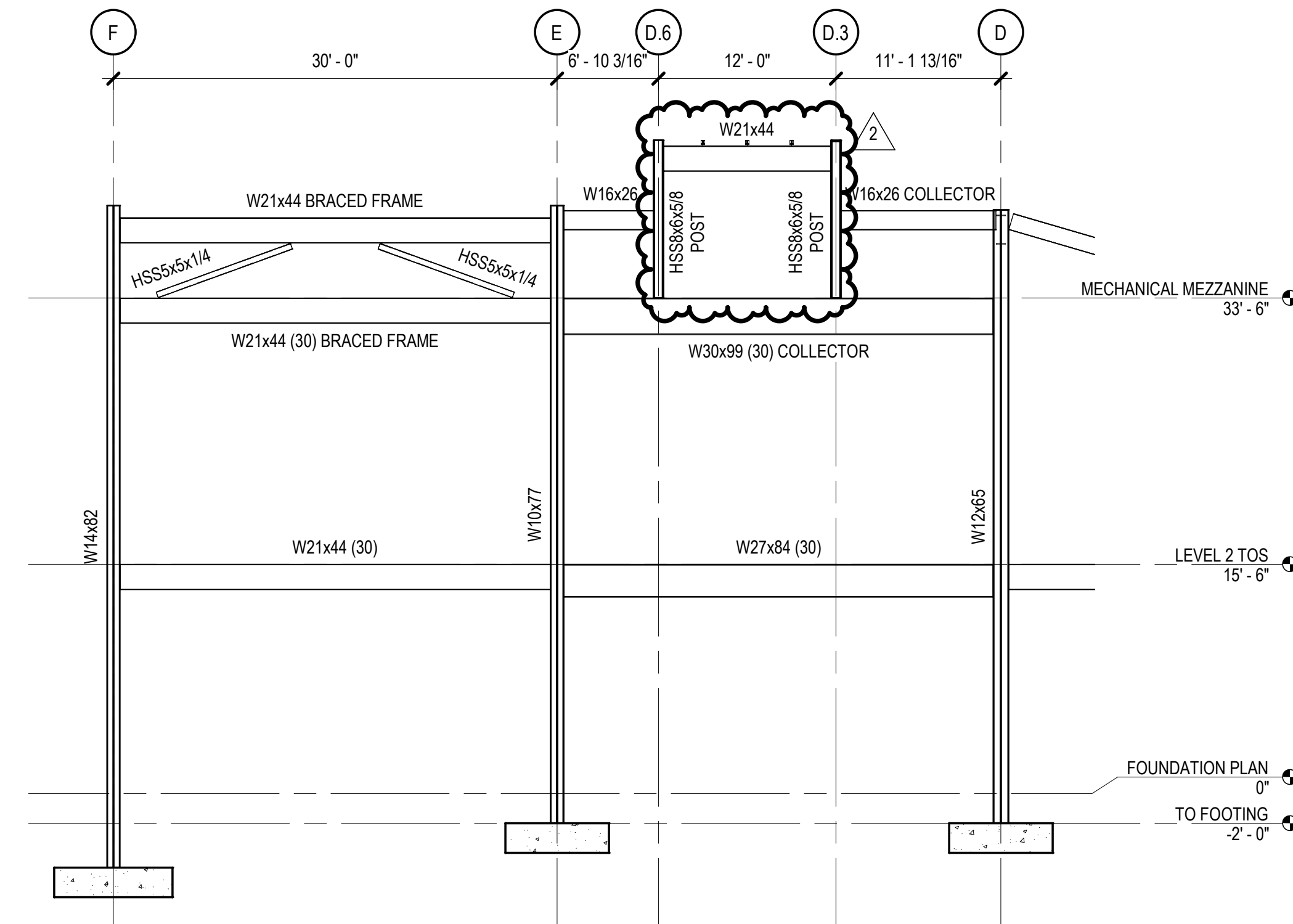
D4 BRACED FRAME ELEVATION
SCALE: 1/4" = 1'-0"



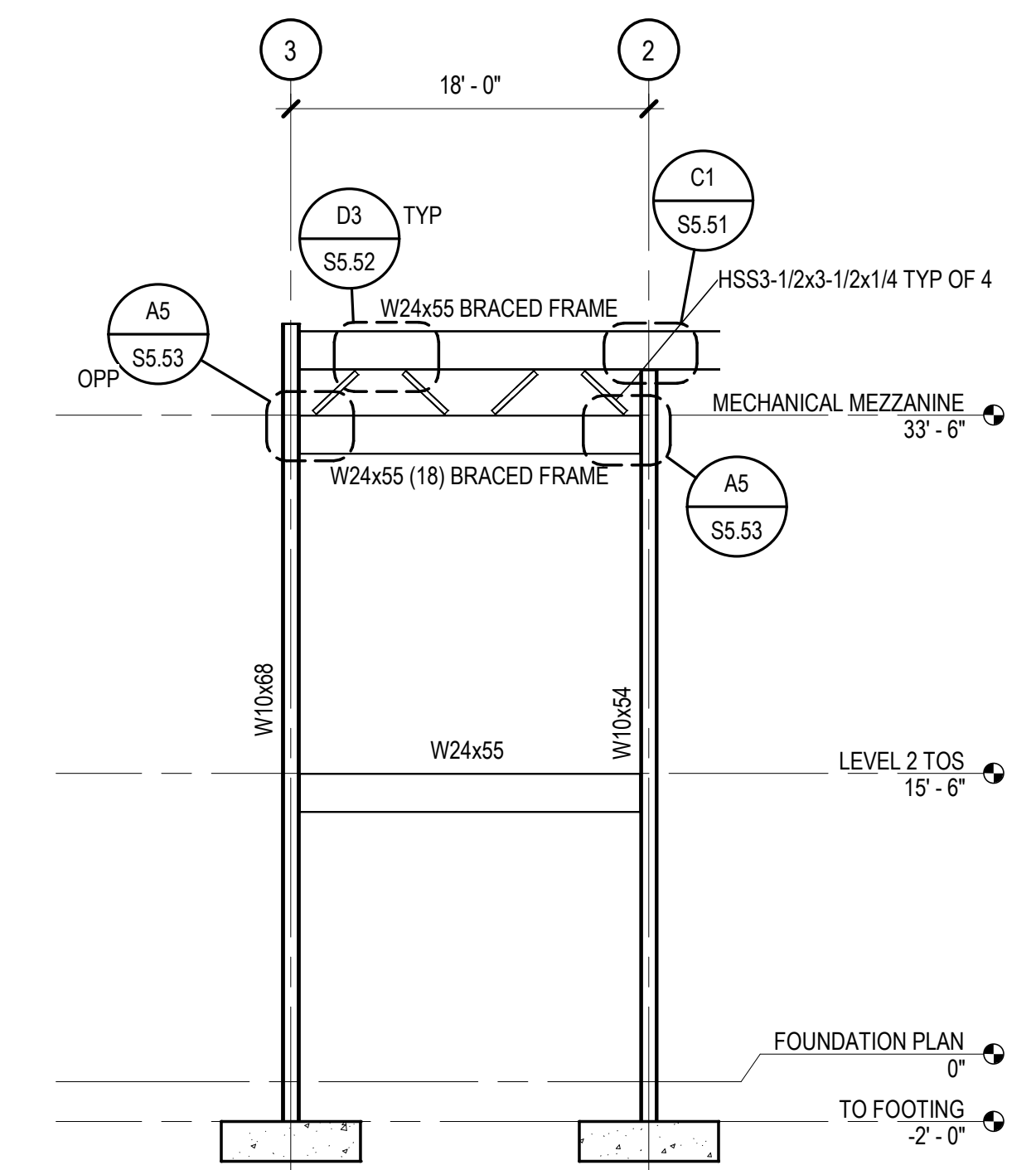
C4 BRACED FRAME ELEVATION
SCALE: 1/8" = 1'-0"



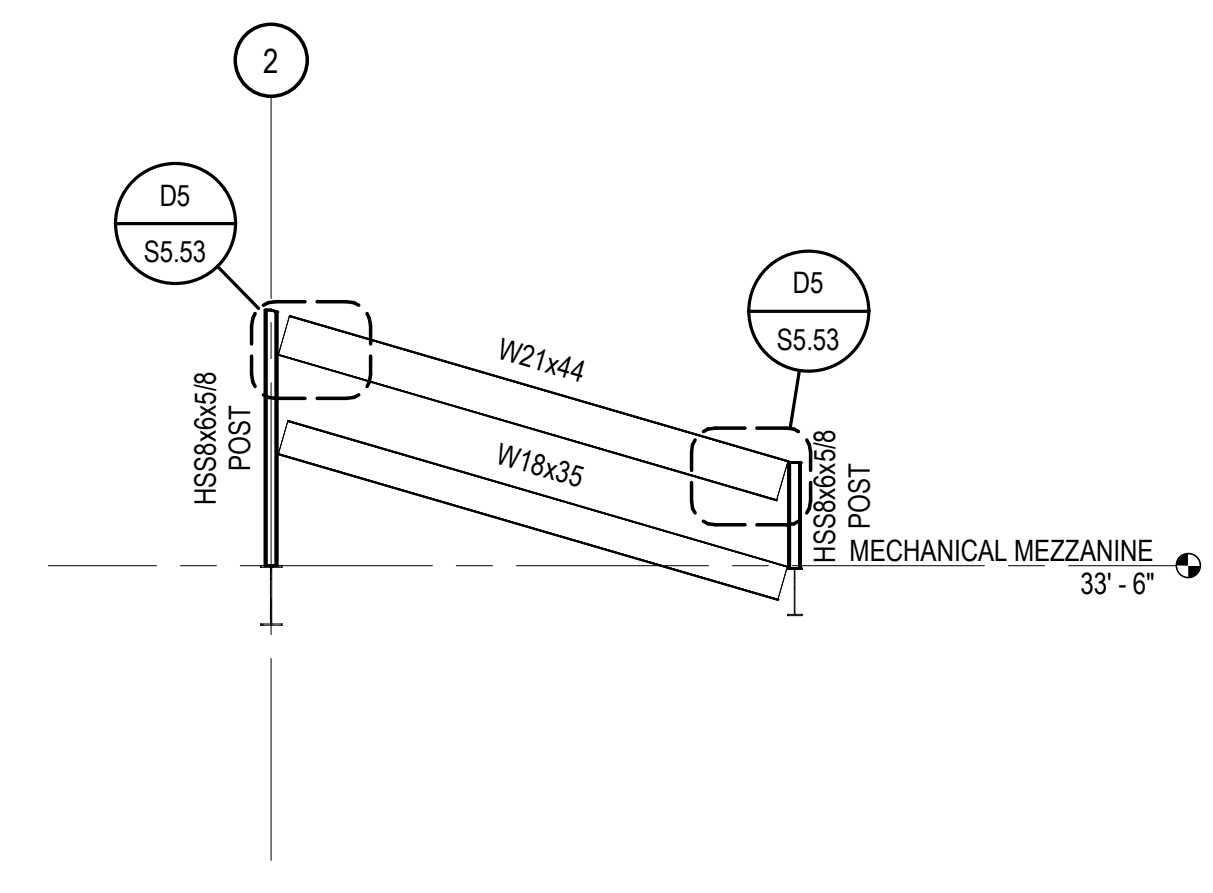
D5 BRACED FRAME ELEVATION
SCALE: 1/8" = 1'-0"



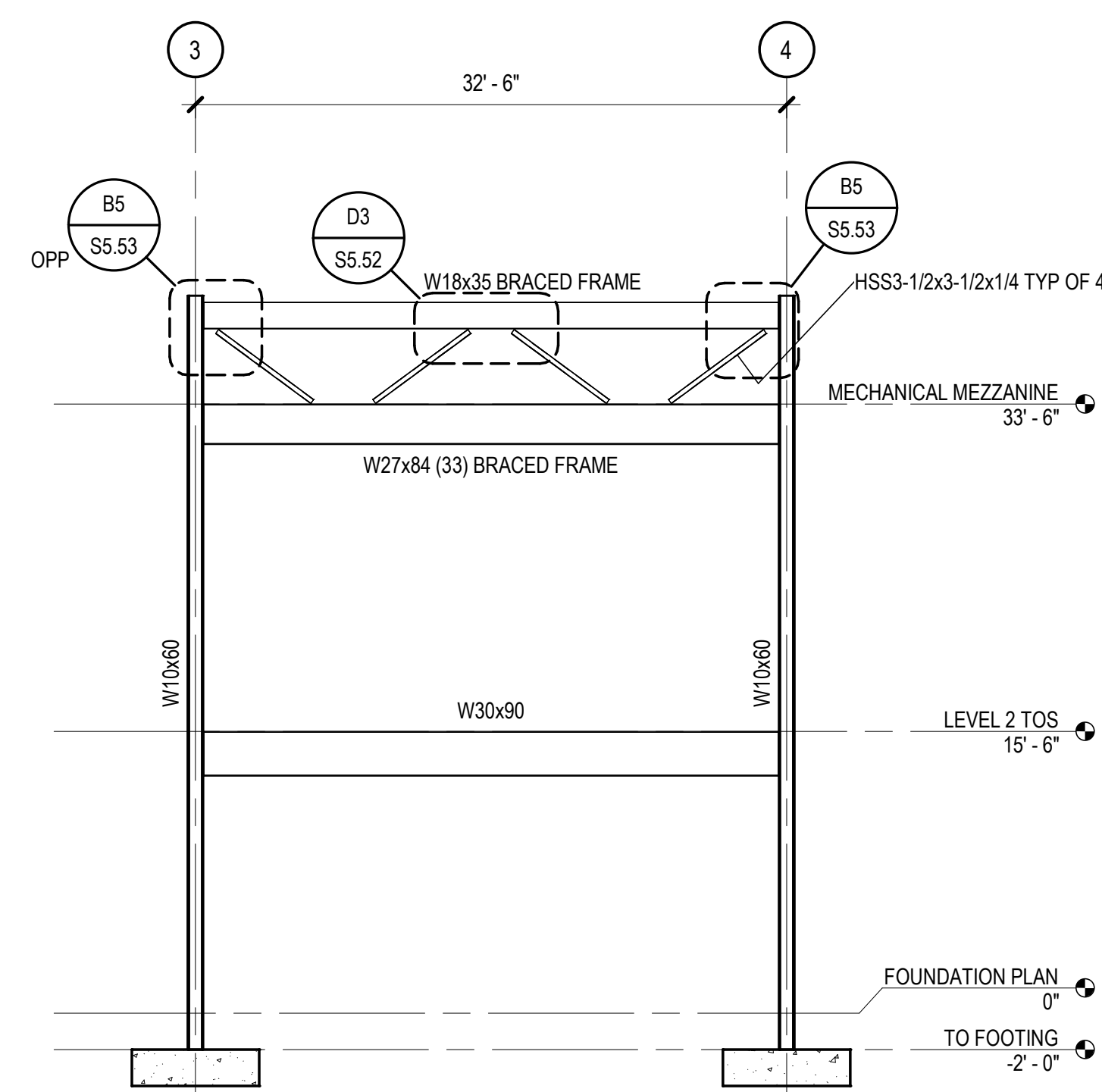
B3 BRACED FRAME ELEVATION
SCALE: 1/8" = 1'-0"



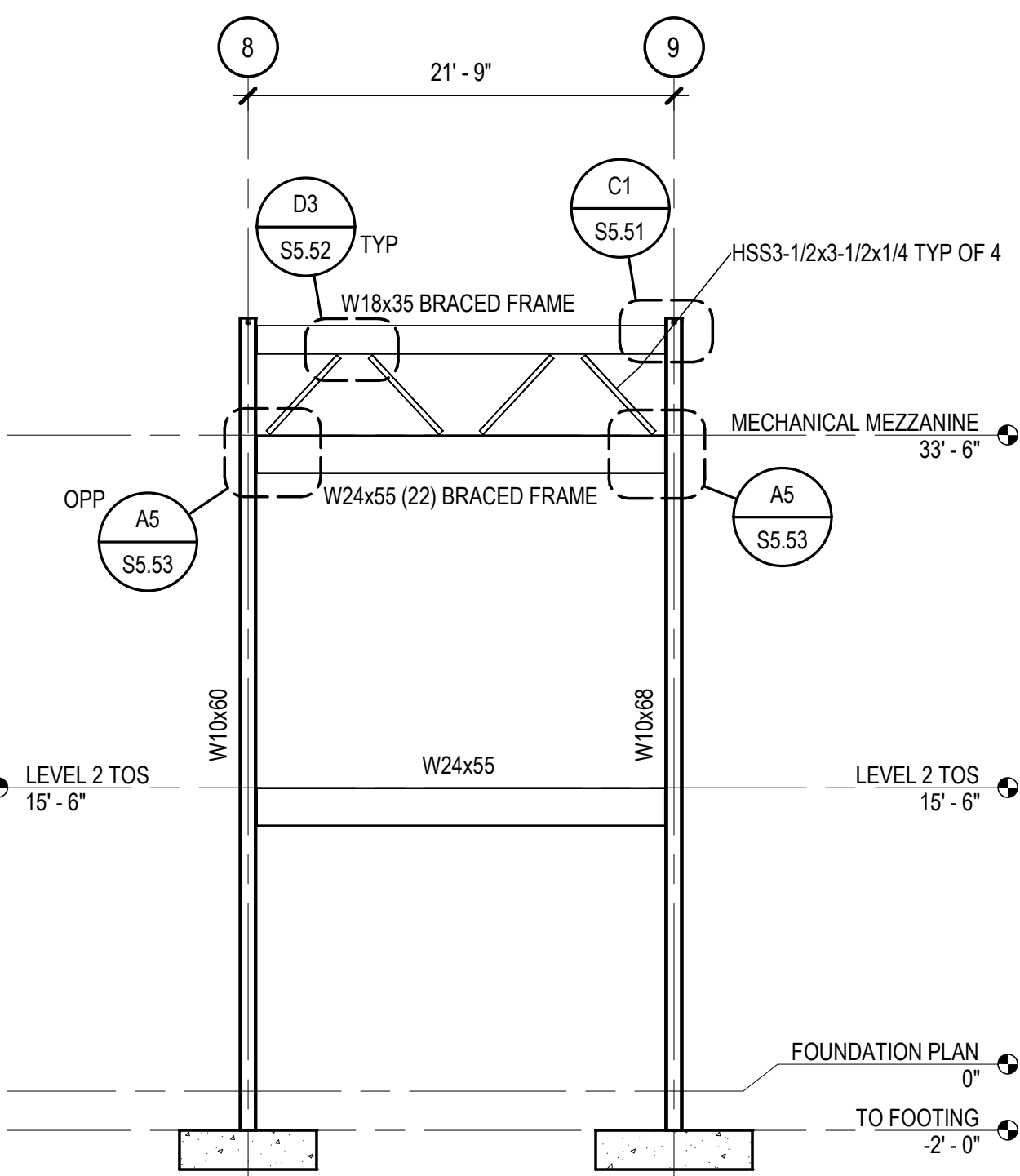
B5 BRACED FRAME ELEVATION
SCALE: 1/8" = 1'-0"



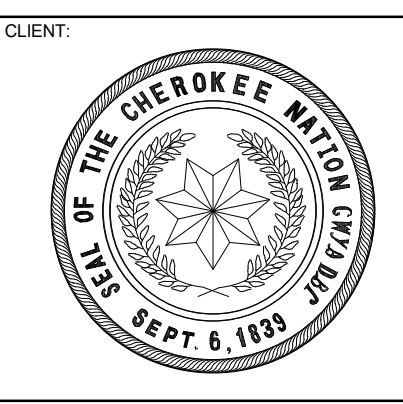
A3 MOMENT FRAME ELEVATION
SCALE: 1/8" = 1'-0"



A4 BRACED FRAME ELEVATION
SCALE: 1/8" = 1'-0"



A5 BRACED FRAME ELEVATION
SCALE: 1/8" = 1'-0"



**WILMA P. MANKILLER HEALTH CENTER
EXPANSION**
STILWELL, OKLAHOMA

KEY PLAN:

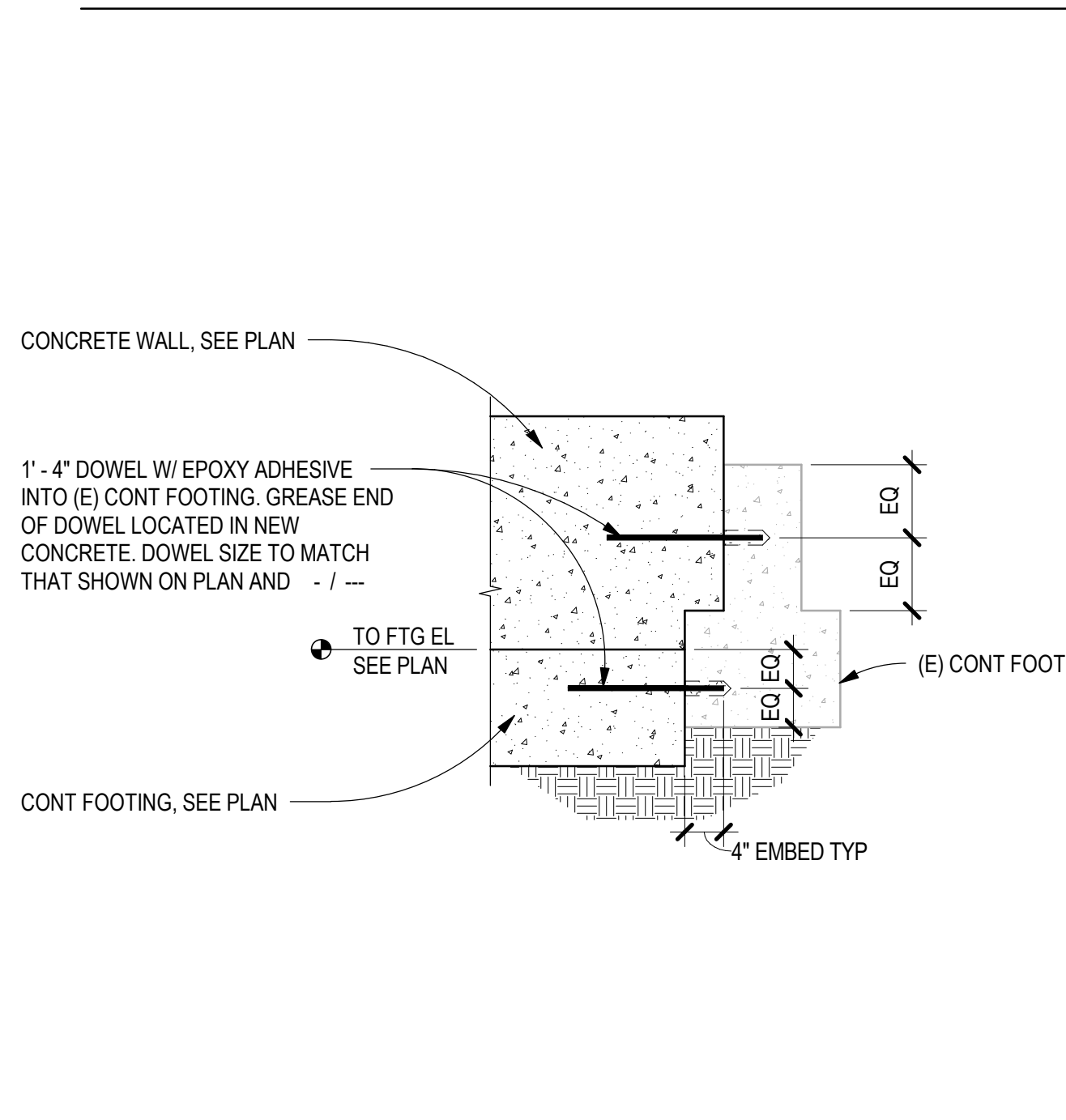
PROJECT PHASE:
BID PACKAGE 01

#	DATE	REVISIONS	DESCRIPTION
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2	12/10/19		BID PACKAGE 01 - ADD 02

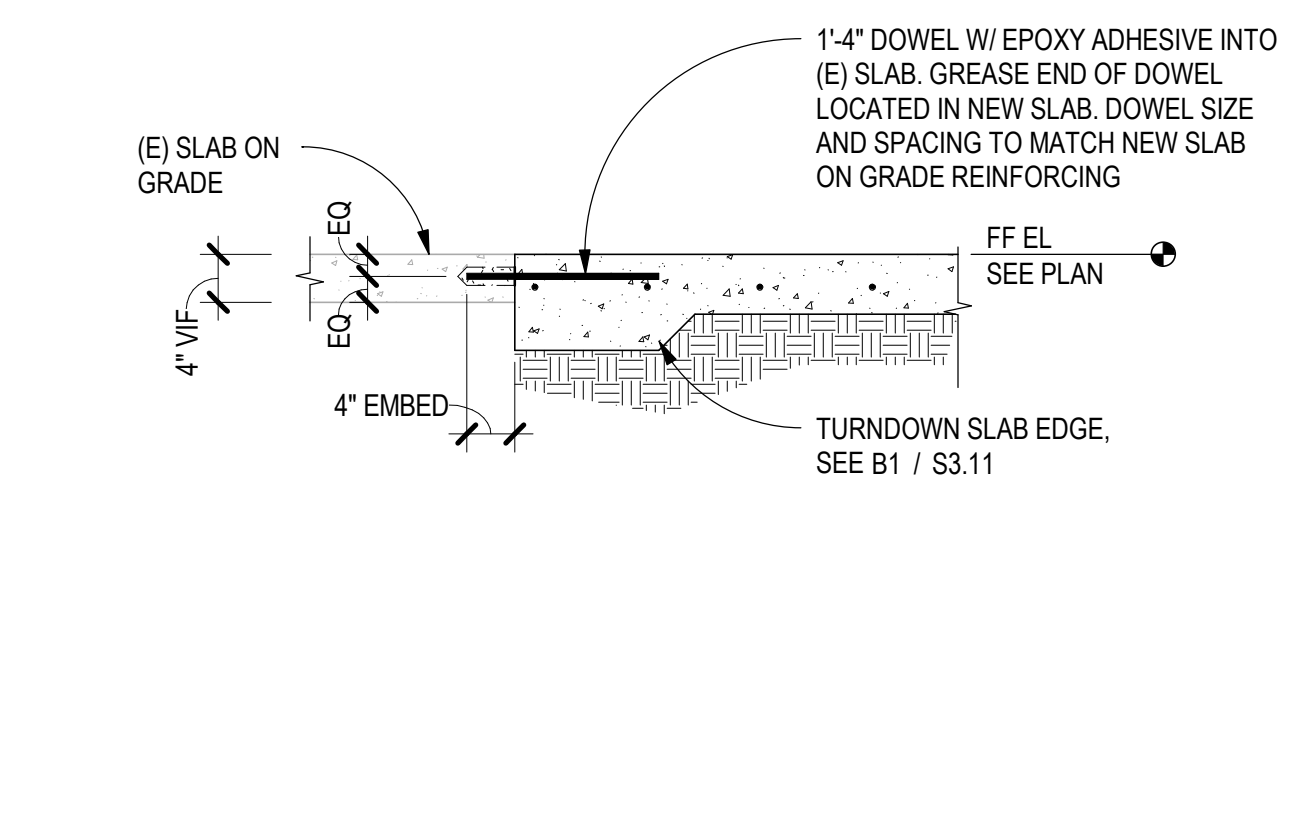
DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER: S2.02

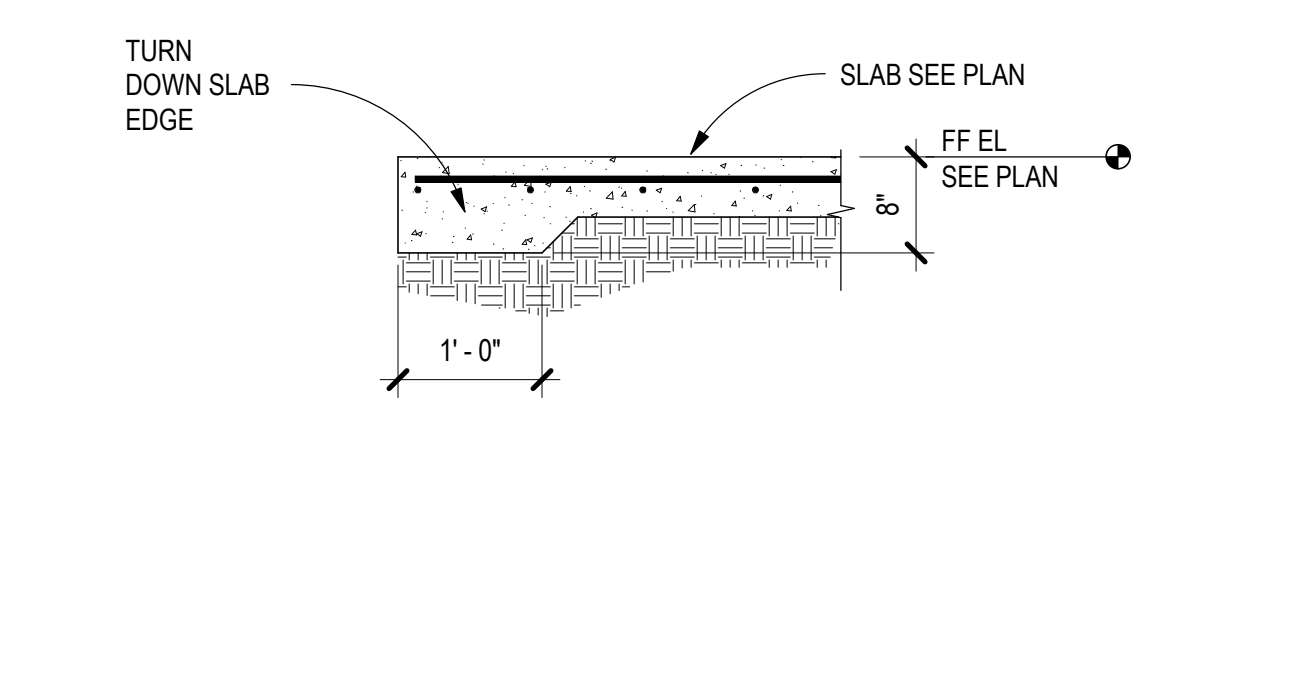
MOMENT FRAME AND BRACED FRAME ELEVATIONS



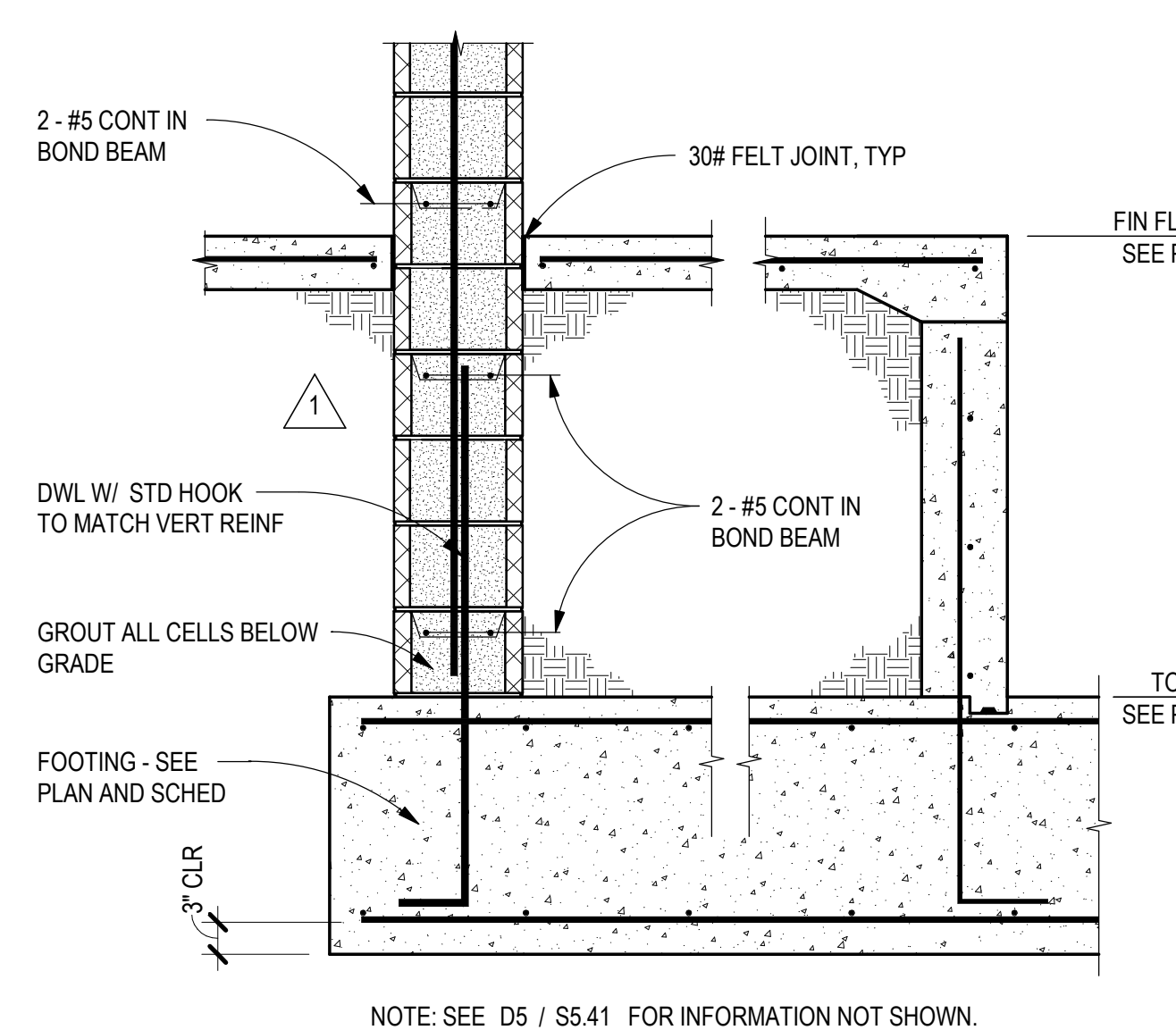
D1 DOWEL INTO (E) CONTINUOUS FOOTING
SCALE: 3/4" = 1'-0"



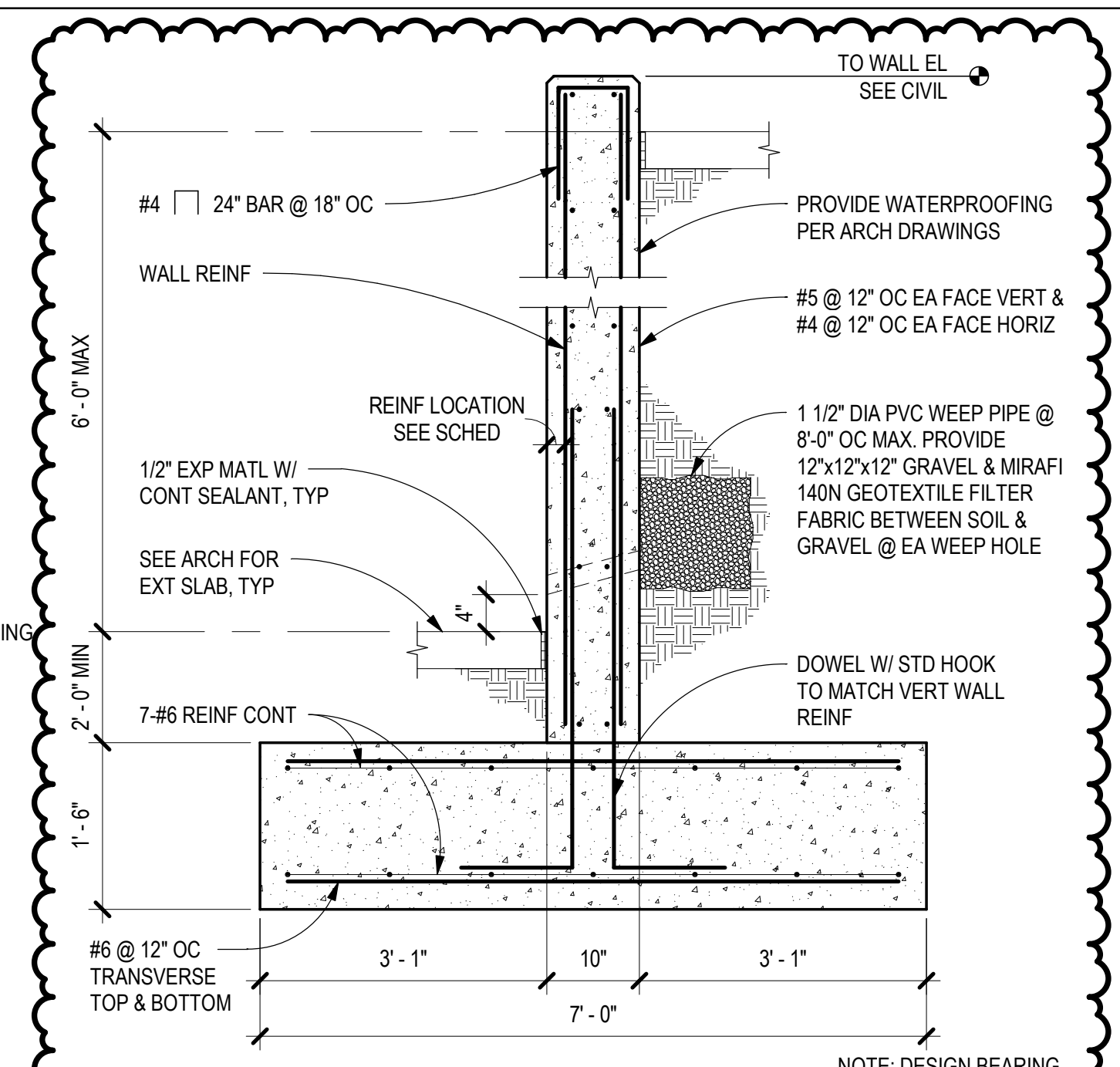
C1 DOWEL INTO (E) SLAB ON GRADE
SCALE: 3/4" = 1'-0"



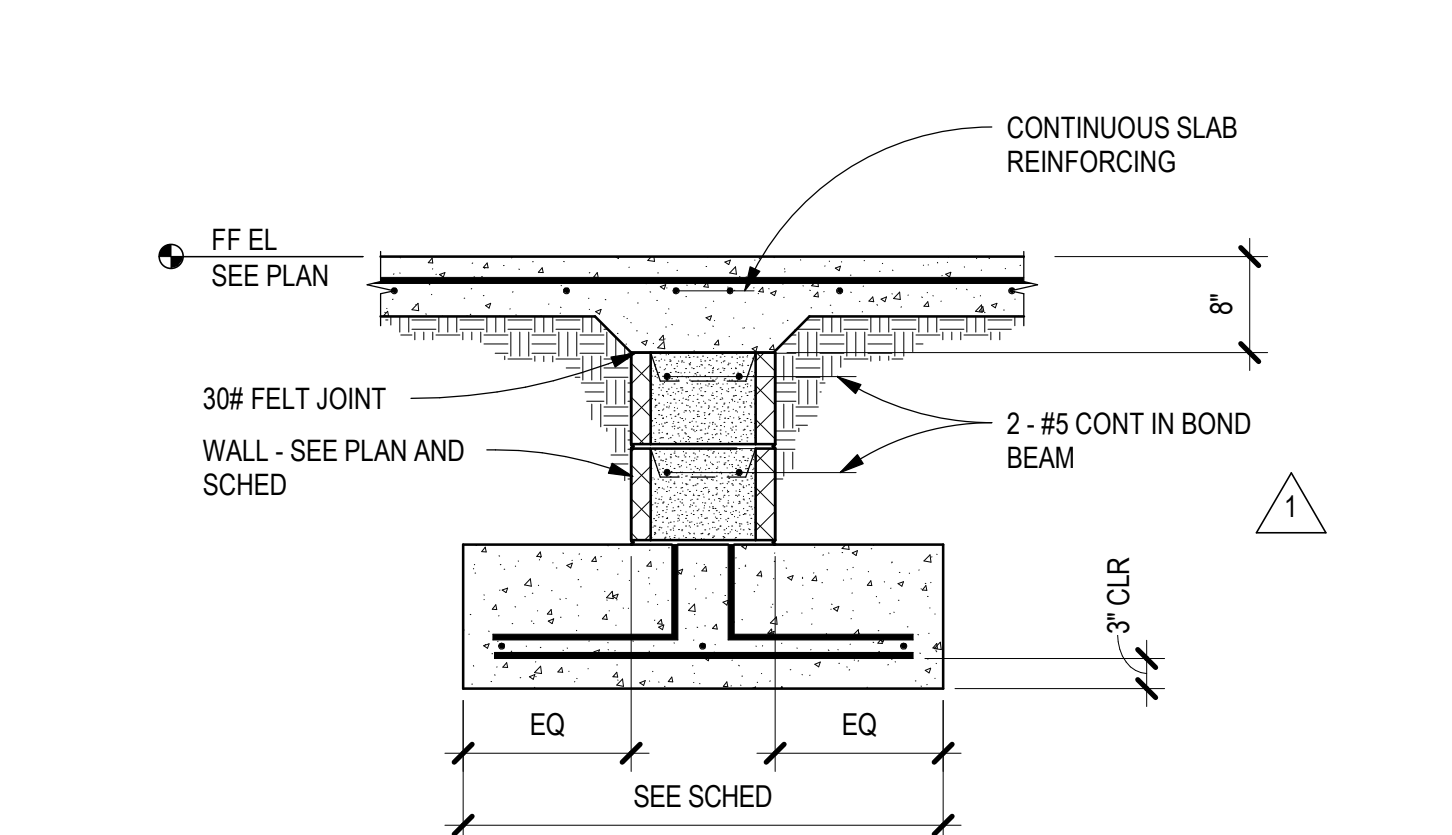
B1 TYP TURN DOWN SLAB EDGE
SCALE: 3/4" = 1'-0"



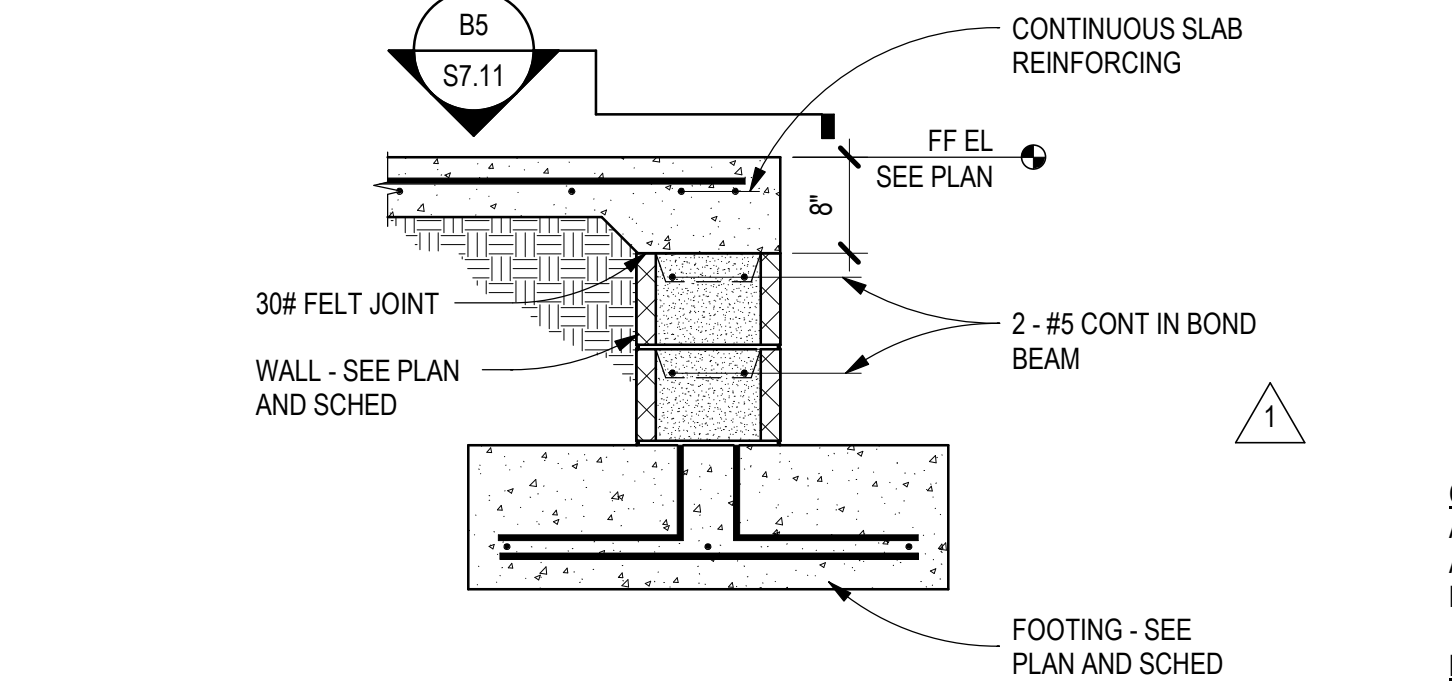
A1 FOUNDATION SECTION @ ELEVATOR
SCALE: 3/4" = 1'-0"



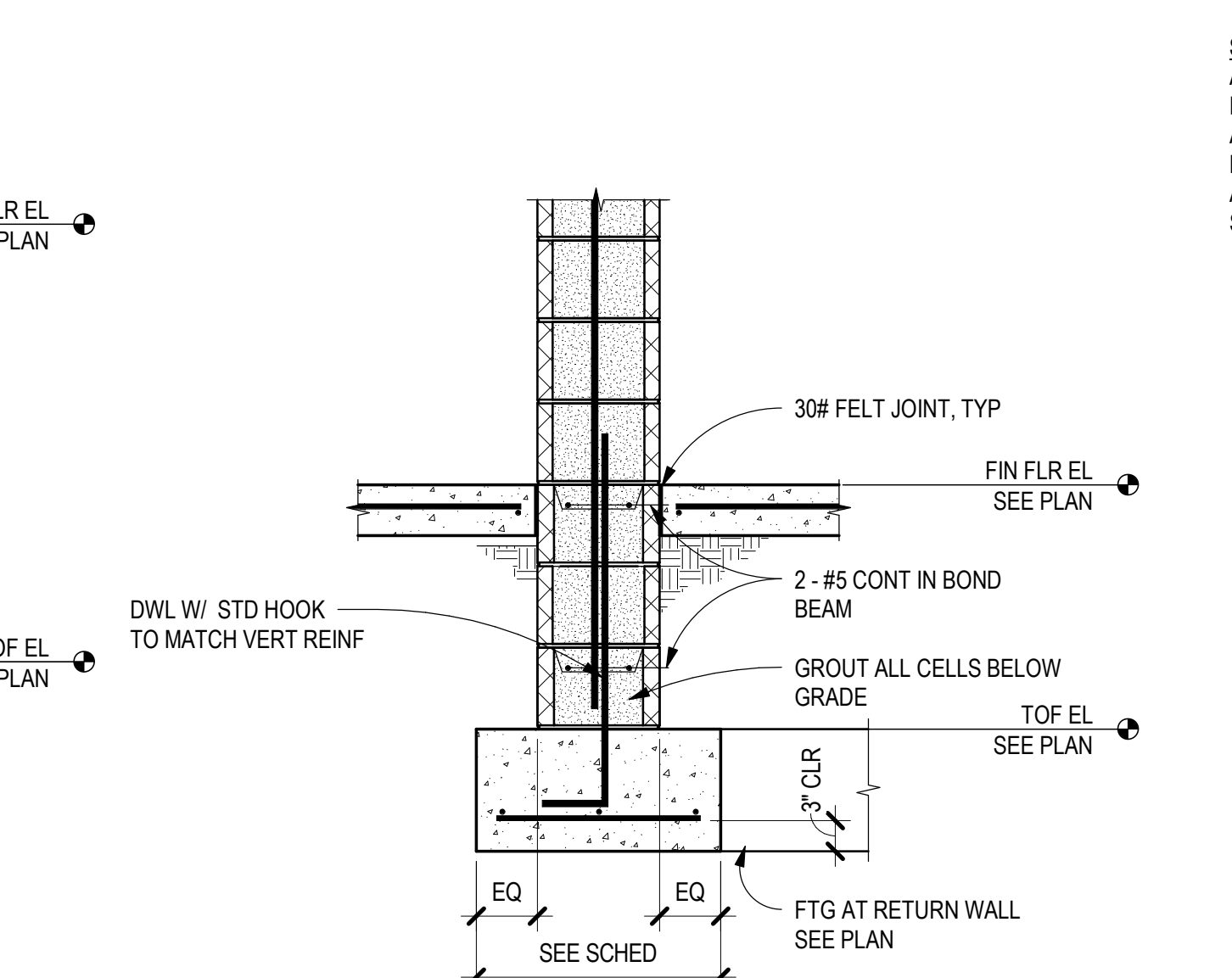
D2 RETAINING WALL SECTION
SCALE: 3/4" = 1'-0"



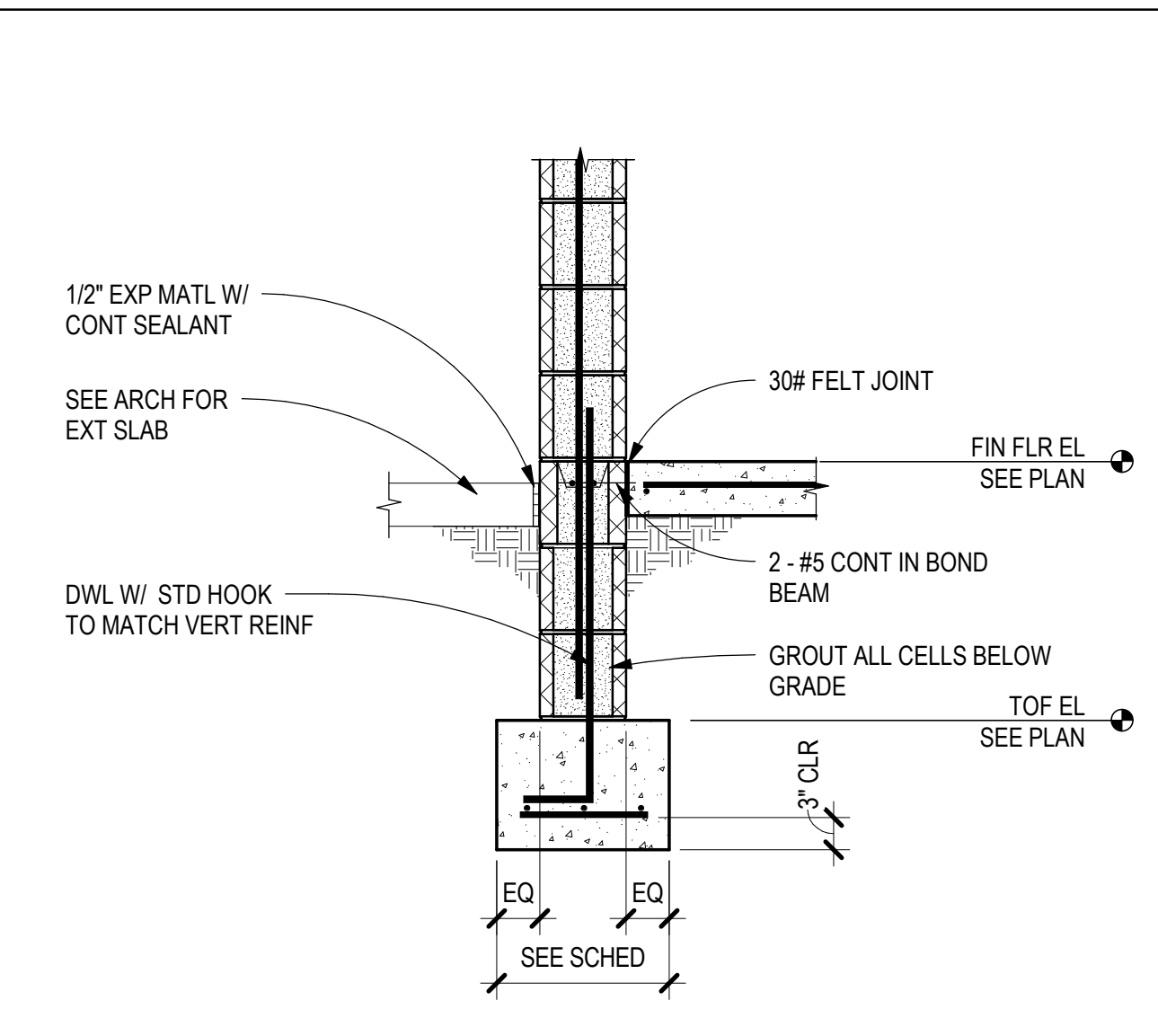
C2 INT SECTION @ DOORWAYS CMU
SCALE: 3/4" = 1'-0"



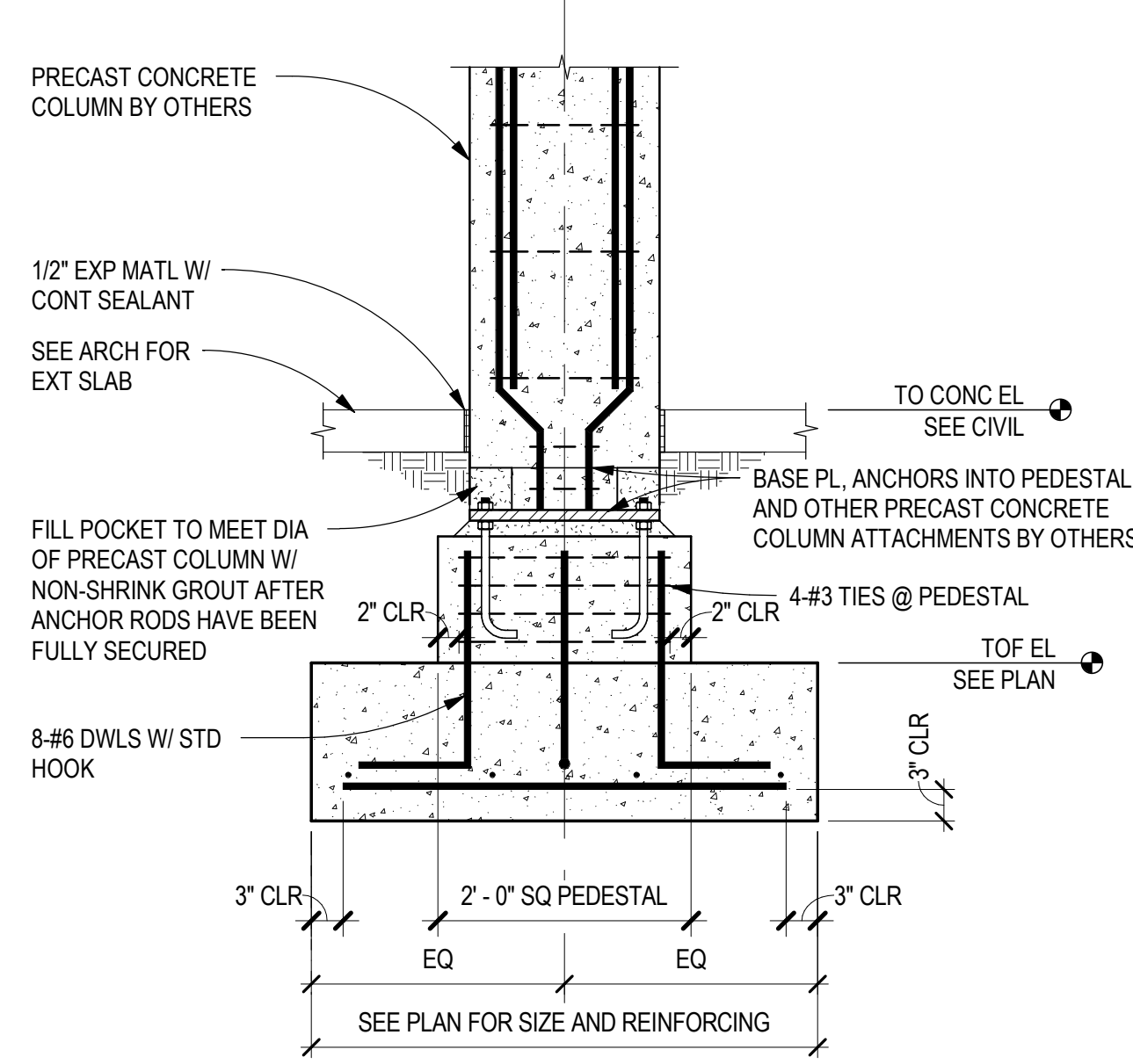
B2 PERIM SECTION AT DOORWAYS CMU
SCALE: 3/4" = 1'-0"



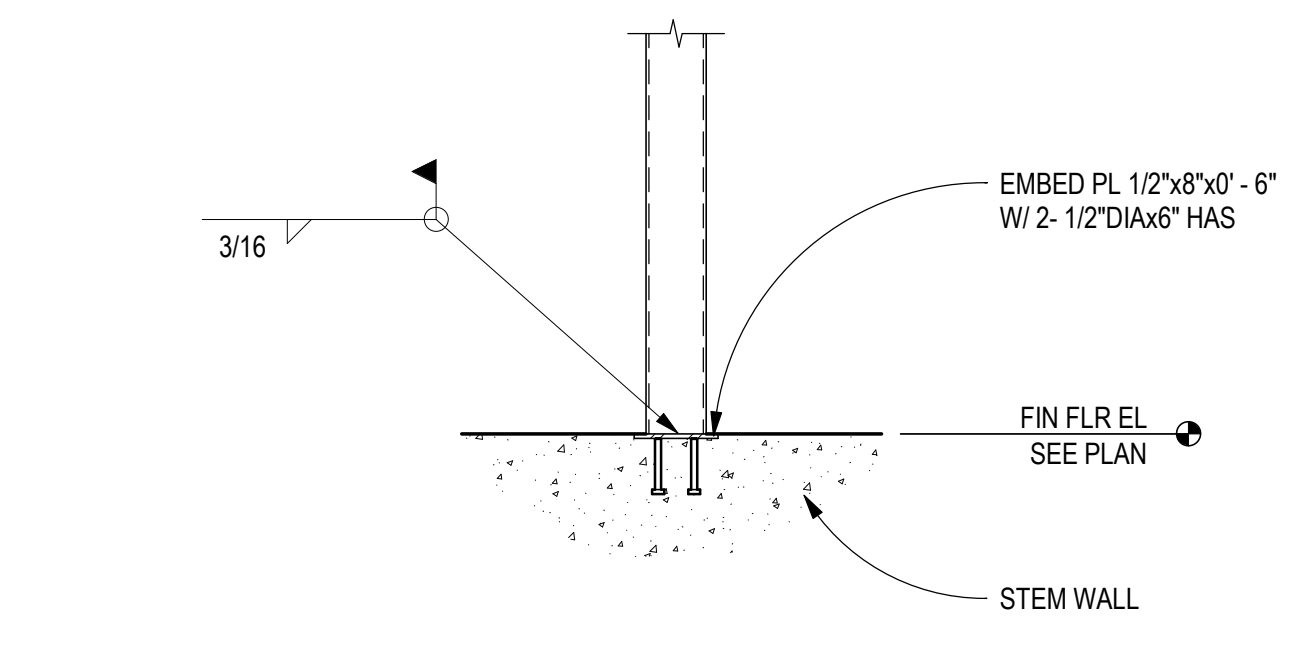
A2 PERIMETER FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



D3 PERIMETER FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



C3 EXT CONC COLUMN SECTION
SCALE: 3/4" = 1'-0"



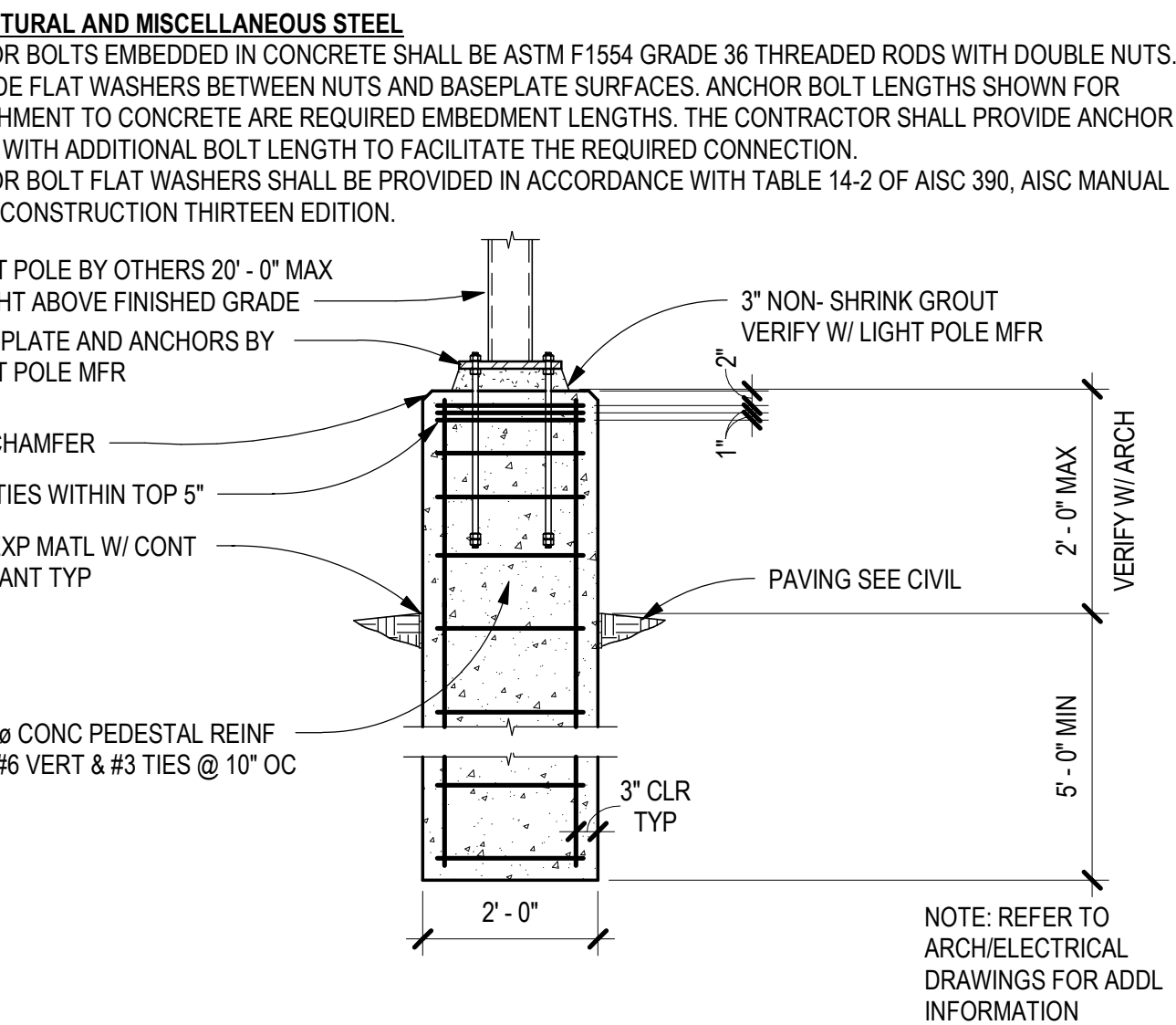
B3 HSS TO STEM WALL SECTION
SCALE: 3/4" = 1'-0"

CAST-IN-PLACE CONCRETE:
ALL CONCRETE SHALL CONFORM TO THE SPECIFICATIONS FOR STRUCTURAL CONCRETE, ACI 301-05. ALL EXPOSED EDGES OF CONCRETE SHALL HAVE A 3/4" CHAMFER UNLESS NOTED OTHERWISE. NORMAL WEIGHT CONCRETE: FC = 4000 PSI @ 28 DAYS

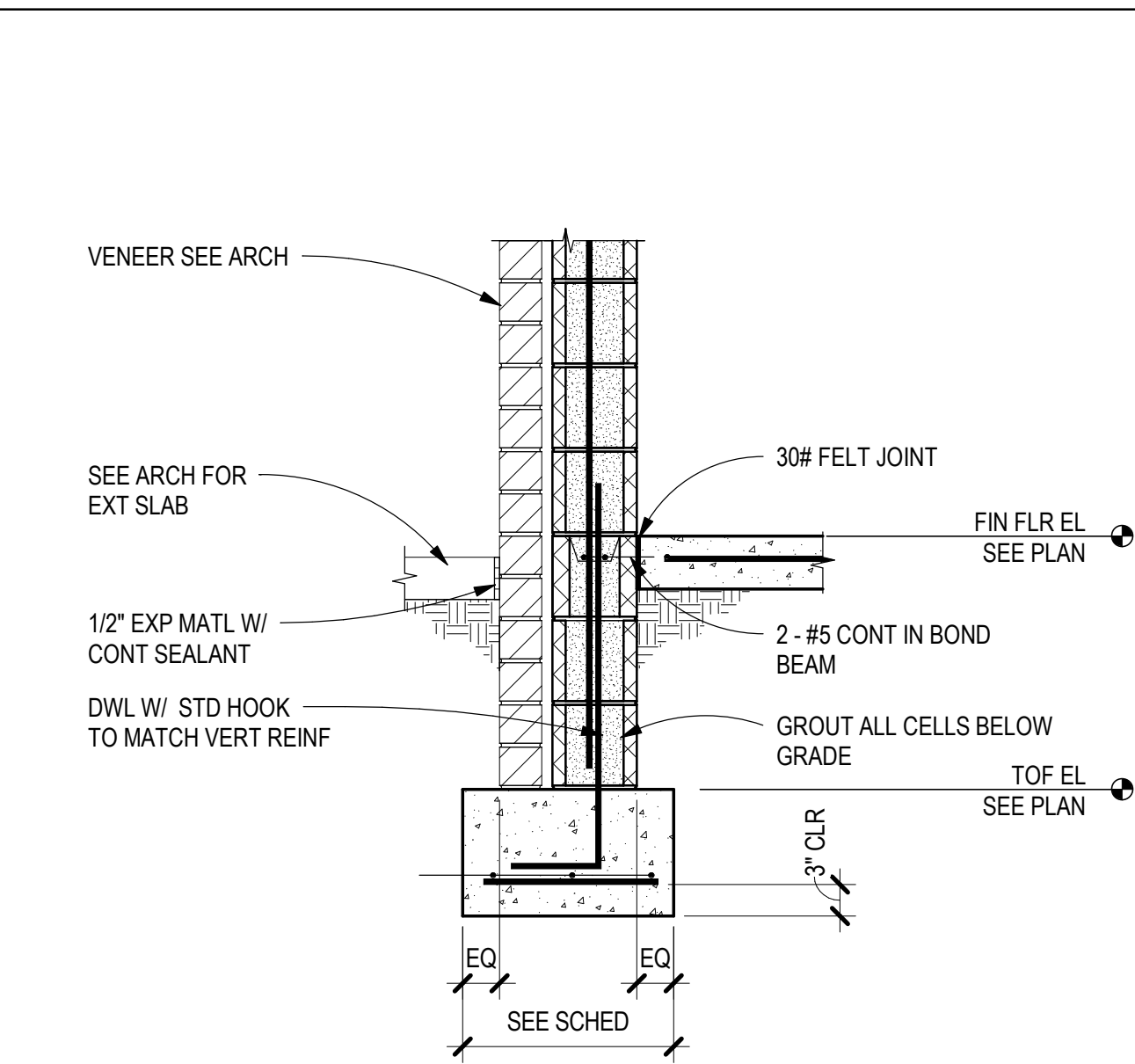
REINFORCING STEEL:
ALL REINFORCING STEEL SHALL BE FABRICATED AND PLACED IN ACCORDANCE WITH THE BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI 318-08), AND DETAILS AND DETAILING OF CONCRETE REINFORCEMENT (ACI 315-99). ALL REINFORCING STEEL SHALL CONFORM TO ASTM A615 GRADE 60; EXCEPT STIRRUPS, TIES AND INDICATED FIELD BARS, WHICH SHALL CONFORM TO ASTM A615 GRADE 40. REINFORCING SHALL NOT BE TACK WELDED OR WELDED IN ANY MANNER UNLESS SPECIFICALLY DETAILED ON THE STRUCTURAL PLANS.

STRUCTURAL AND MISCELLANEOUS STEEL
ANCHOR BOLTS EMBEDDED IN CONCRETE SHALL BE ASTM F1554 GRADE 36 THREADED RODS WITH DOUBLE NUTS. PROVIDE FLAT WASHERS BETWEEN NUTS AND BASEPLATE SURFACES. ANCHOR BOLT LENGTHS SHOWN FOR ATTACHMENT TO CONCRETE ARE REQUIRED EMBEDMENT LENGTHS. THE CONTRACTOR SHALL PROVIDE ANCHOR BOLTS WITH ADDITIONAL BOLT LENGTH TO FACILITATE THE REQUIRED CONNECTION. ANCHOR BOLT FLAT WASHERS SHALL BE PROVIDED IN ACCORDANCE WITH TABLE 14-2 OF AISC 360, AISC MANUAL OF STEEL CONSTRUCTION THIRTEEN EDITION.

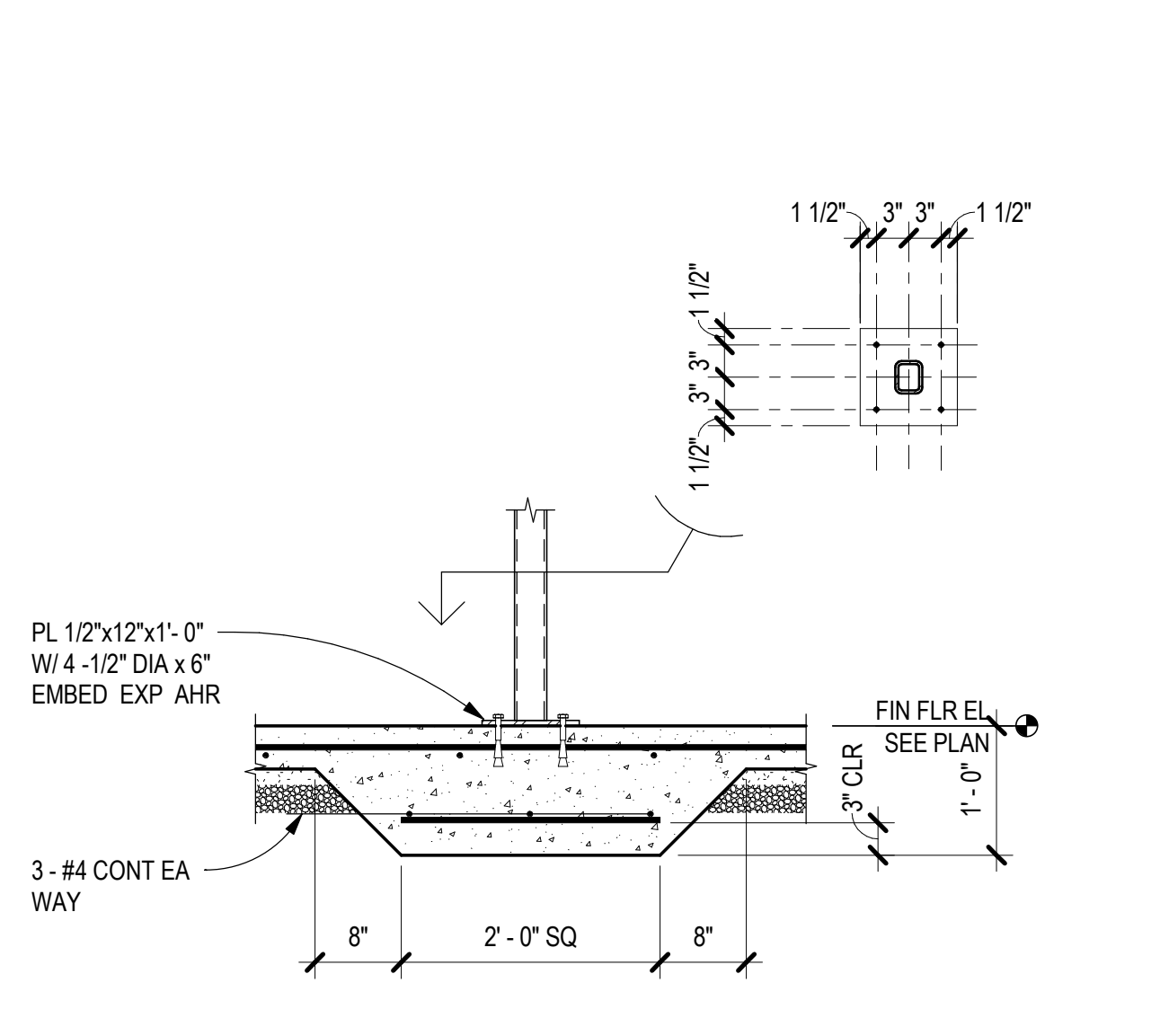
ANCHOR BOLTS EMBEDDED IN CONCRETE SHALL BE ASTM F1554 GRADE 36 THREADED RODS WITH DOUBLE NUTS. PROVIDE FLAT WASHERS BETWEEN NUTS AND BASEPLATE SURFACES. ANCHOR BOLT LENGTHS SHOWN FOR ATTACHMENT TO CONCRETE ARE REQUIRED EMBEDMENT LENGTHS. THE CONTRACTOR SHALL PROVIDE ANCHOR BOLTS WITH ADDITIONAL BOLT LENGTH TO FACILITATE THE REQUIRED CONNECTION. ANCHOR BOLT FLAT WASHERS SHALL BE PROVIDED IN ACCORDANCE WITH TABLE 14-2 OF AISC 360, AISC MANUAL OF STEEL CONSTRUCTION THIRTEEN EDITION.



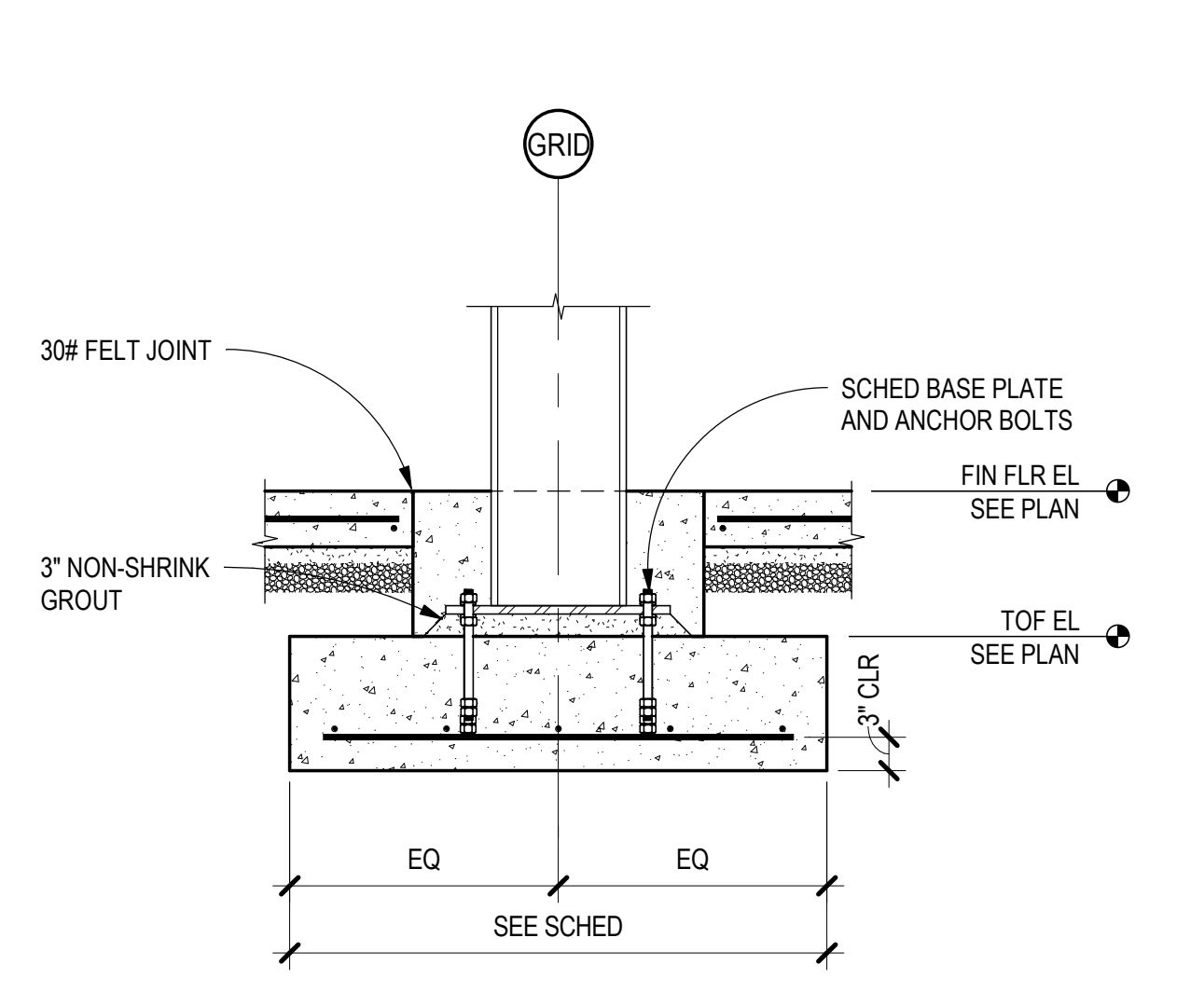
A3 LIGHT POLE BASE
SCALE: 1/2" = 1'-0"



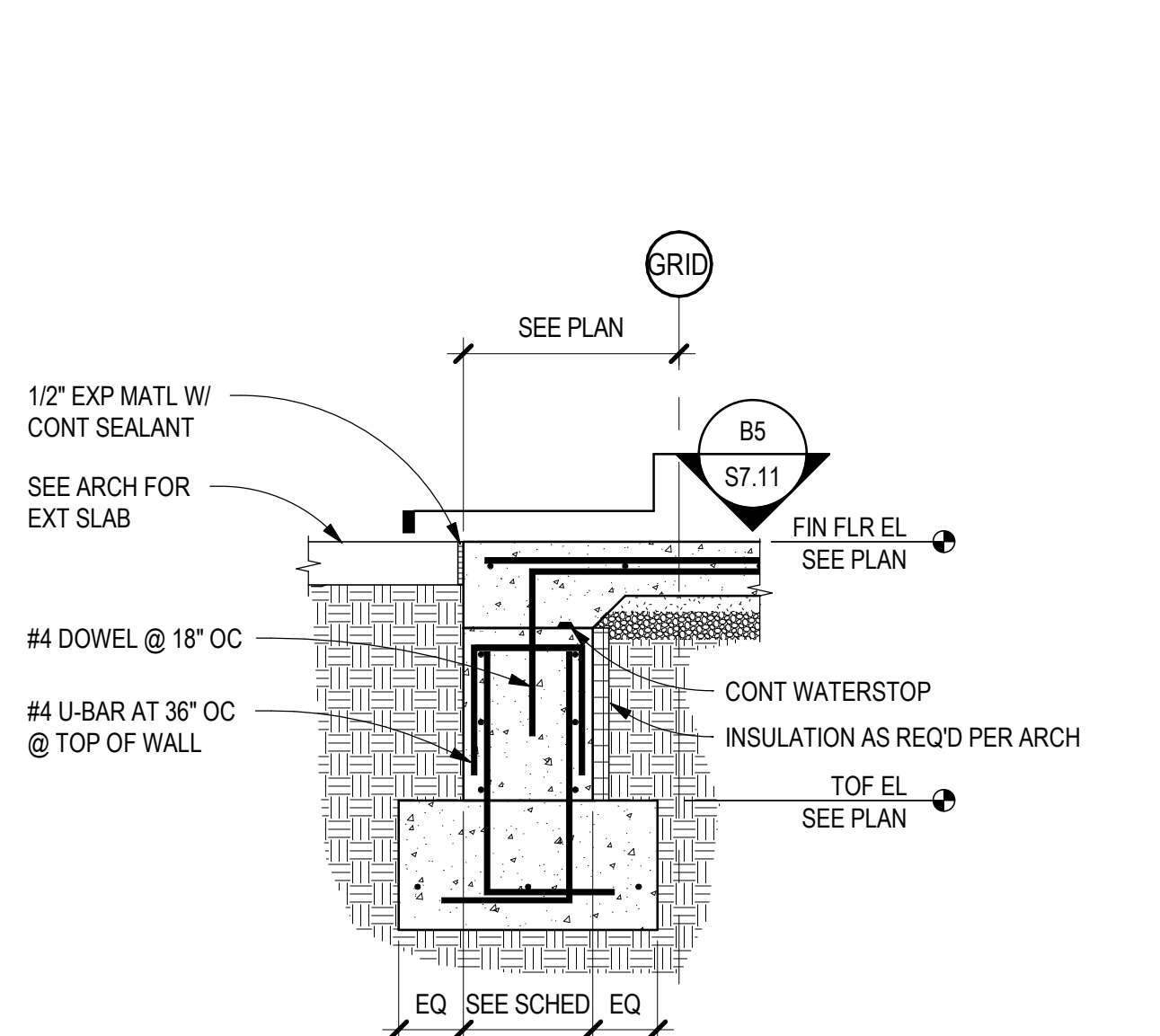
D4 PERIMETER FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



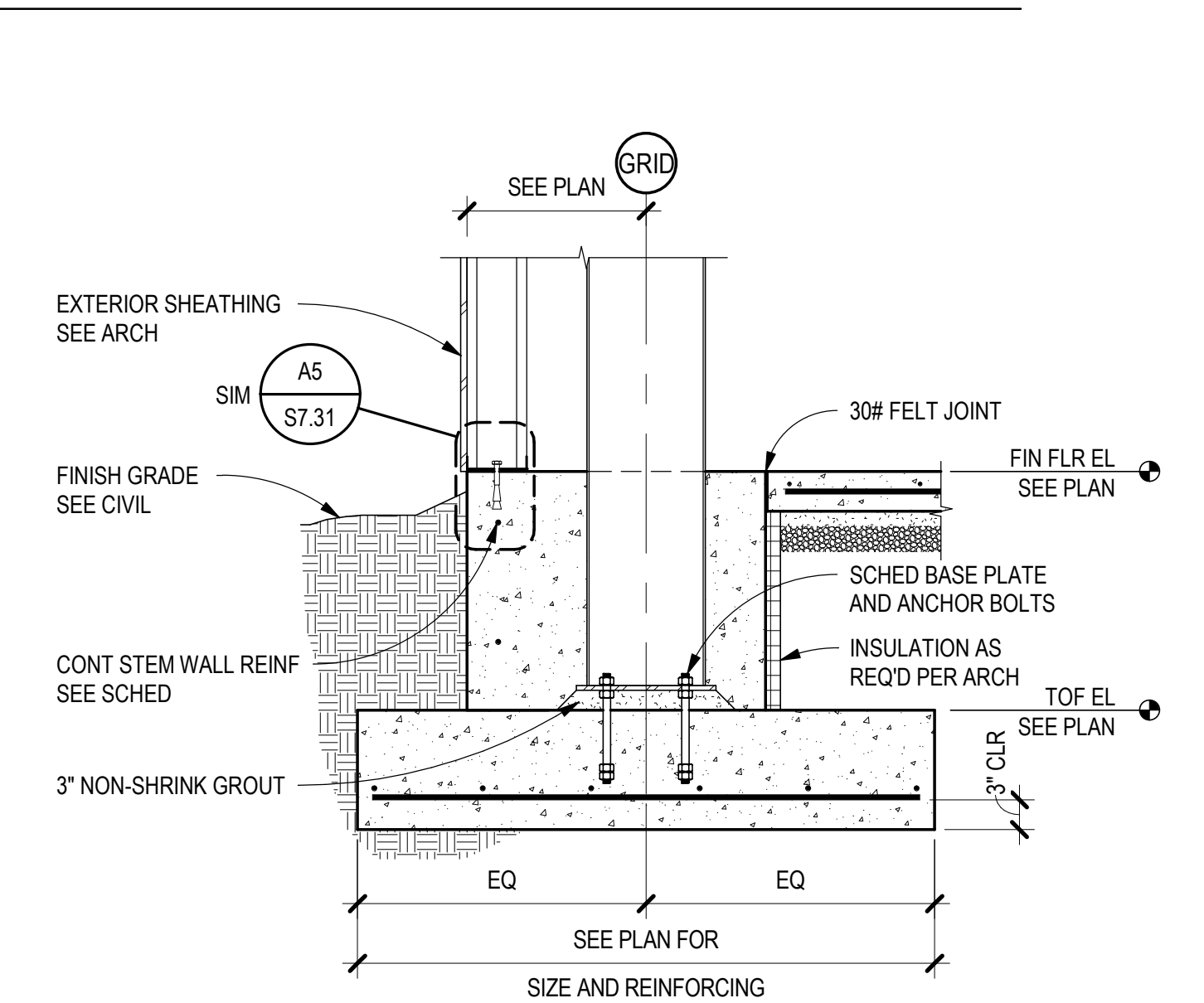
C4 INT COLUMN AT THICKENED SLAB
SCALE: 3/4" = 1'-0"



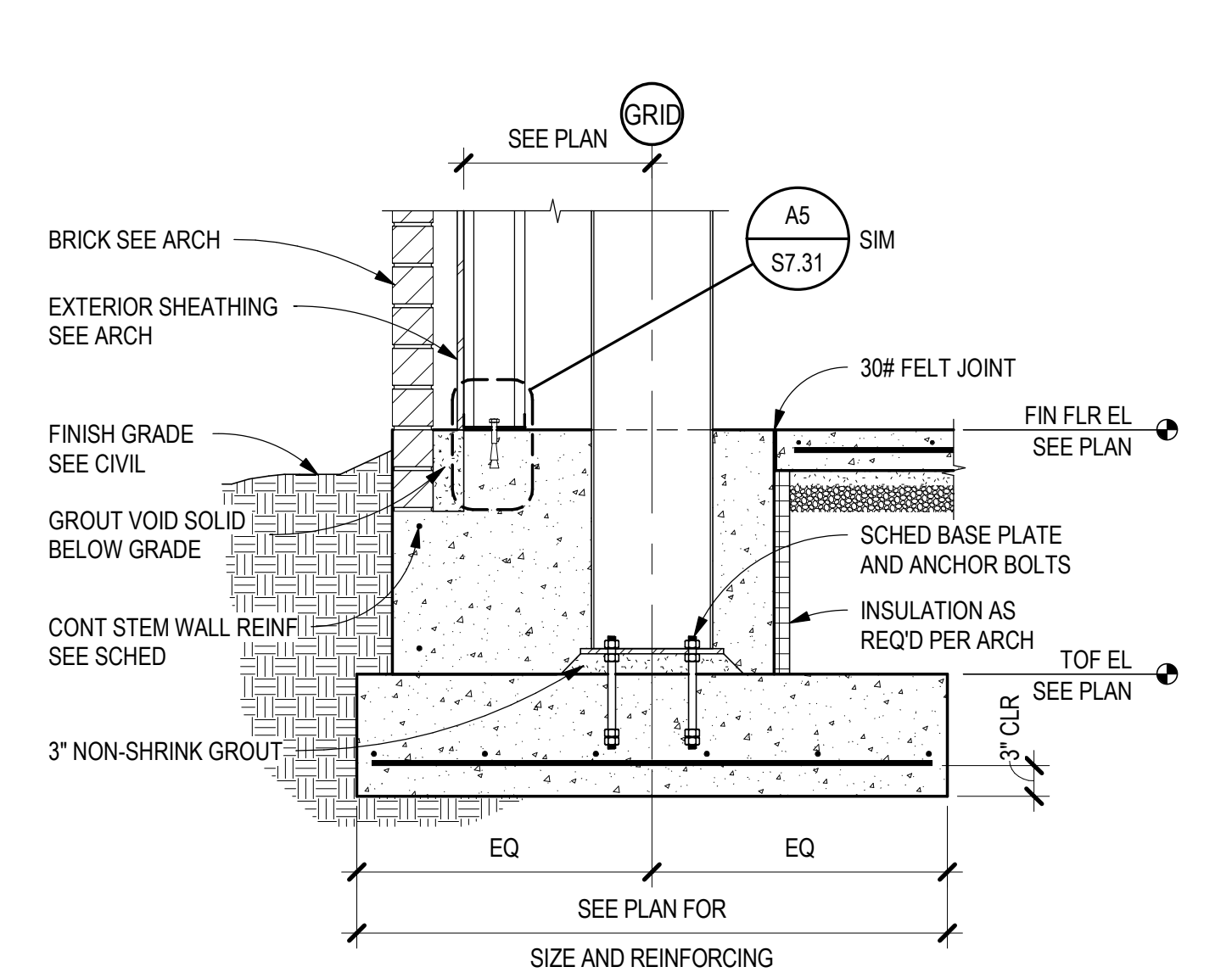
B4 INTERIOR COLUMN SECTION
SCALE: 3/4" = 1'-0"



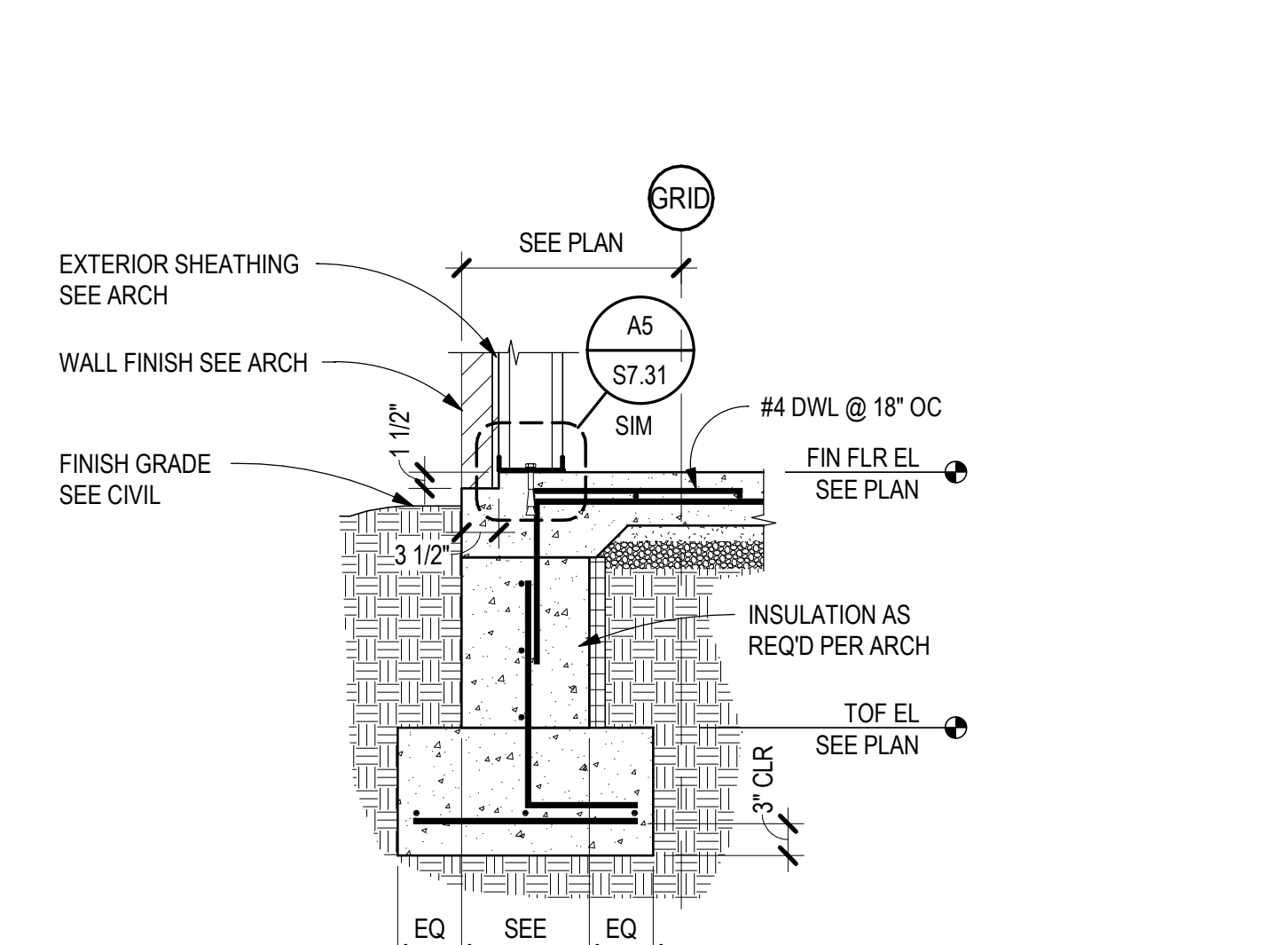
A4 FOUNDATION SECTION AT DOORWAY
SCALE: 3/4" = 1'-0"



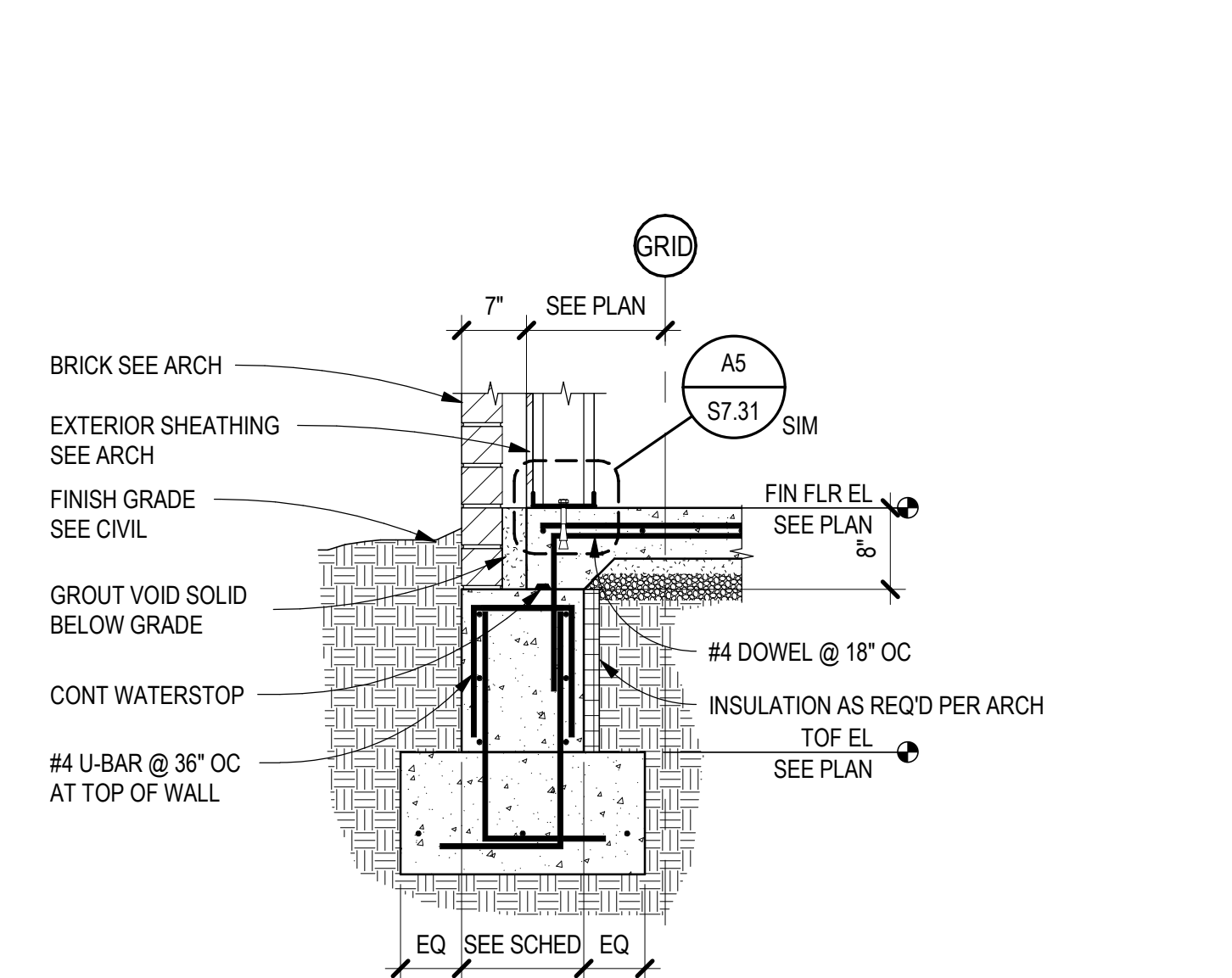
D5 FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



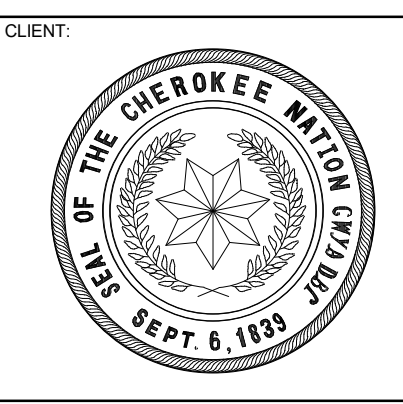
C5 FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



B5 FOUNDATION SECTION NO VENEER
SCALE: 3/4" = 1'-0"



A5 TYPICAL FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



KEY PLAN:

PROJECT PHASE:
BID PACKAGE 01

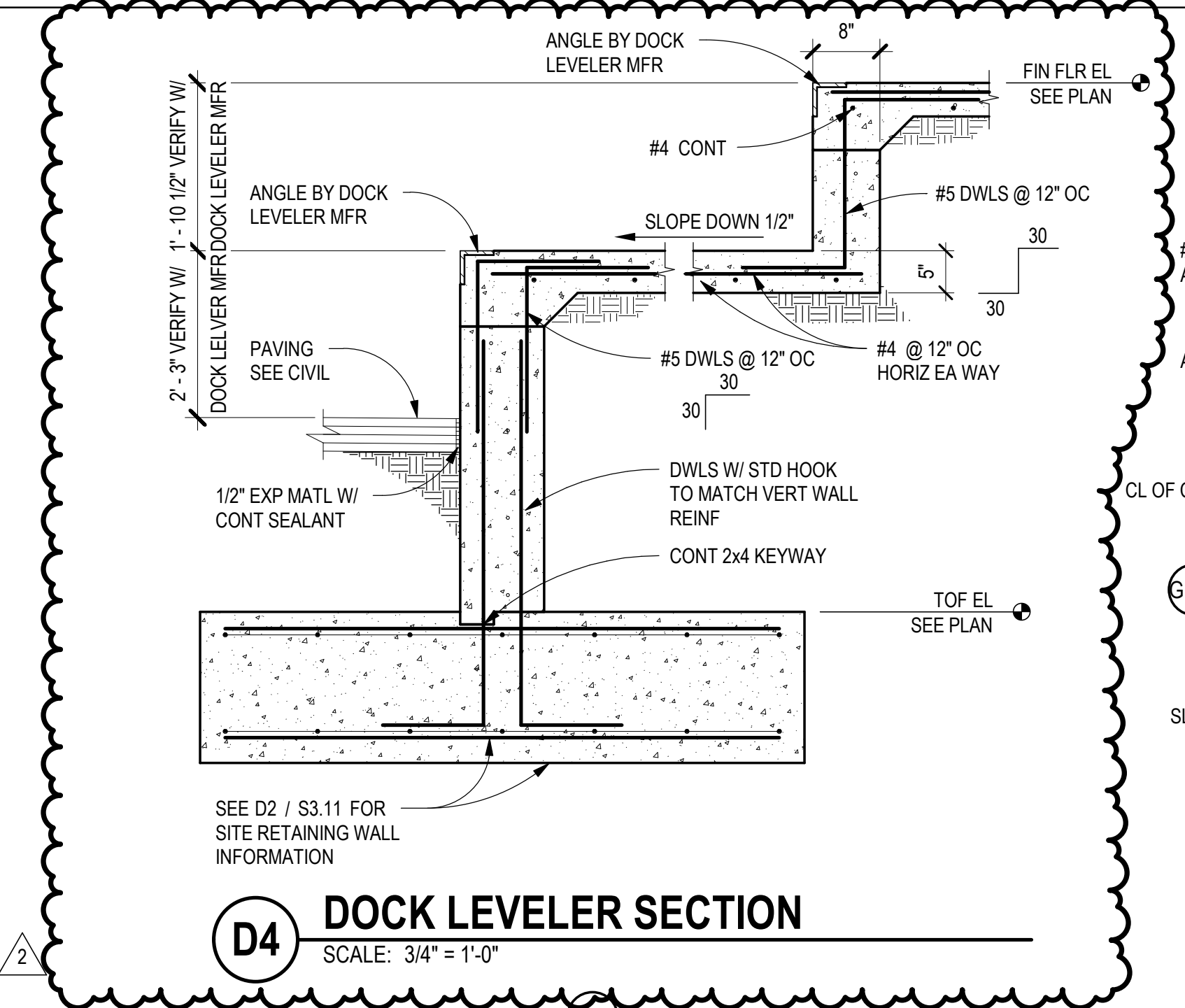
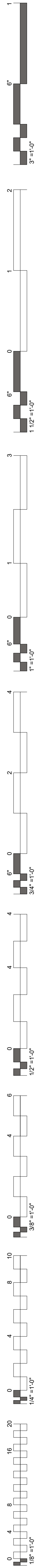
#	DATE	REVISIONS	DESCRIPTION
1	11/22/19	BID PACKAGE 01 - ADD 01	
2	12/19/19	BID PACKAGE 01 - ADD 02	

DATE: 11-01-19 JOB NUMBER: 18-01.01

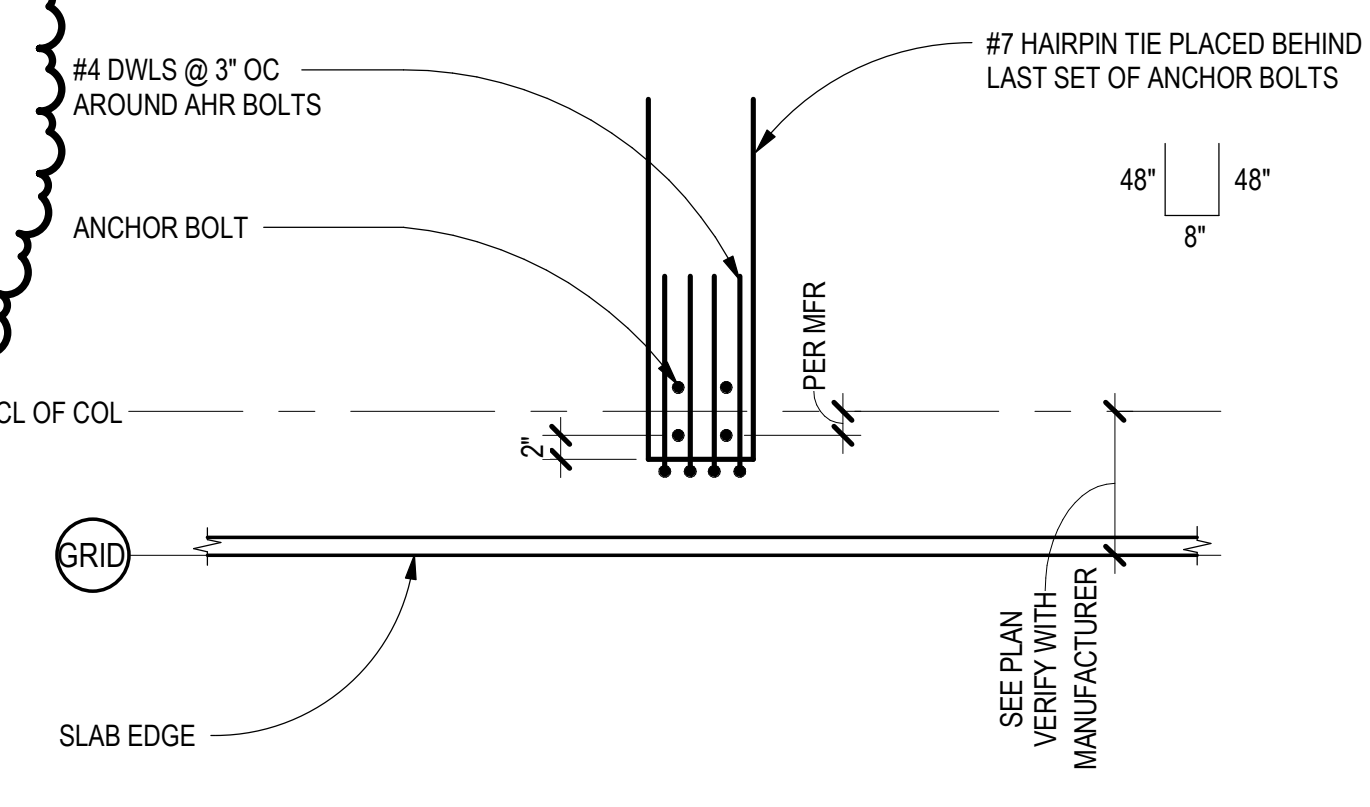
SHEET NUMBER:

S3.11

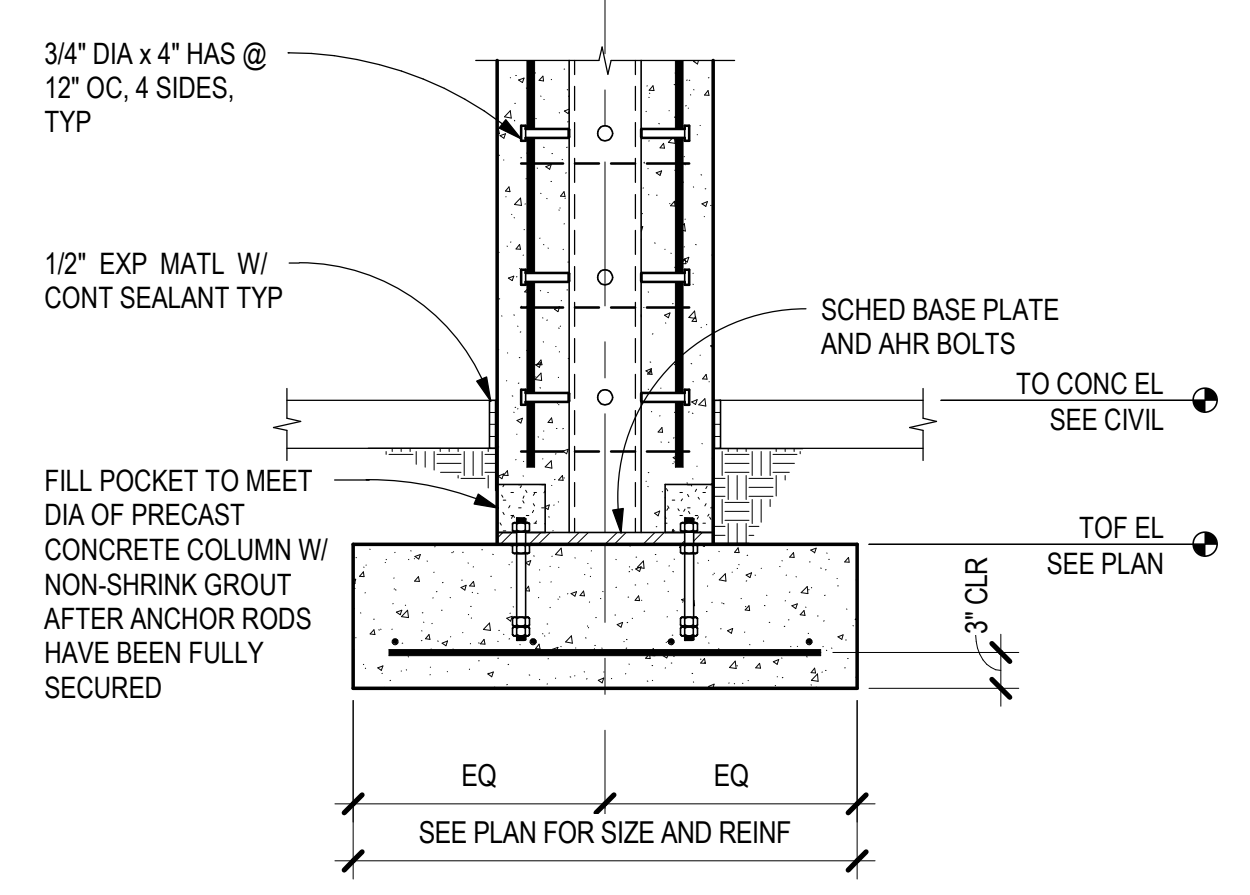
FOUNDATION SECTIONS



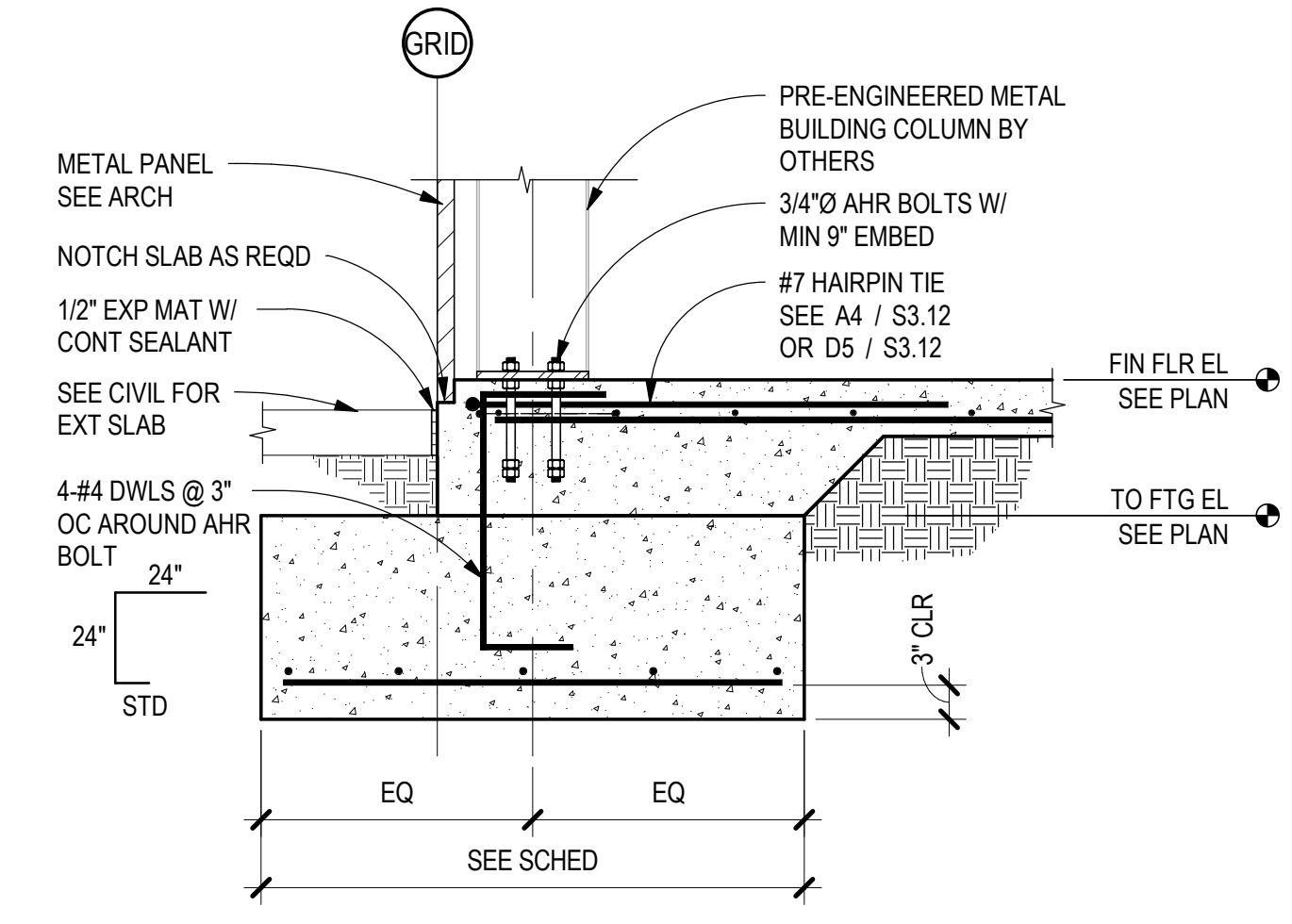
D4 DOCK LEVELER SECTION
SCALE: 3/4" = 1'-0"



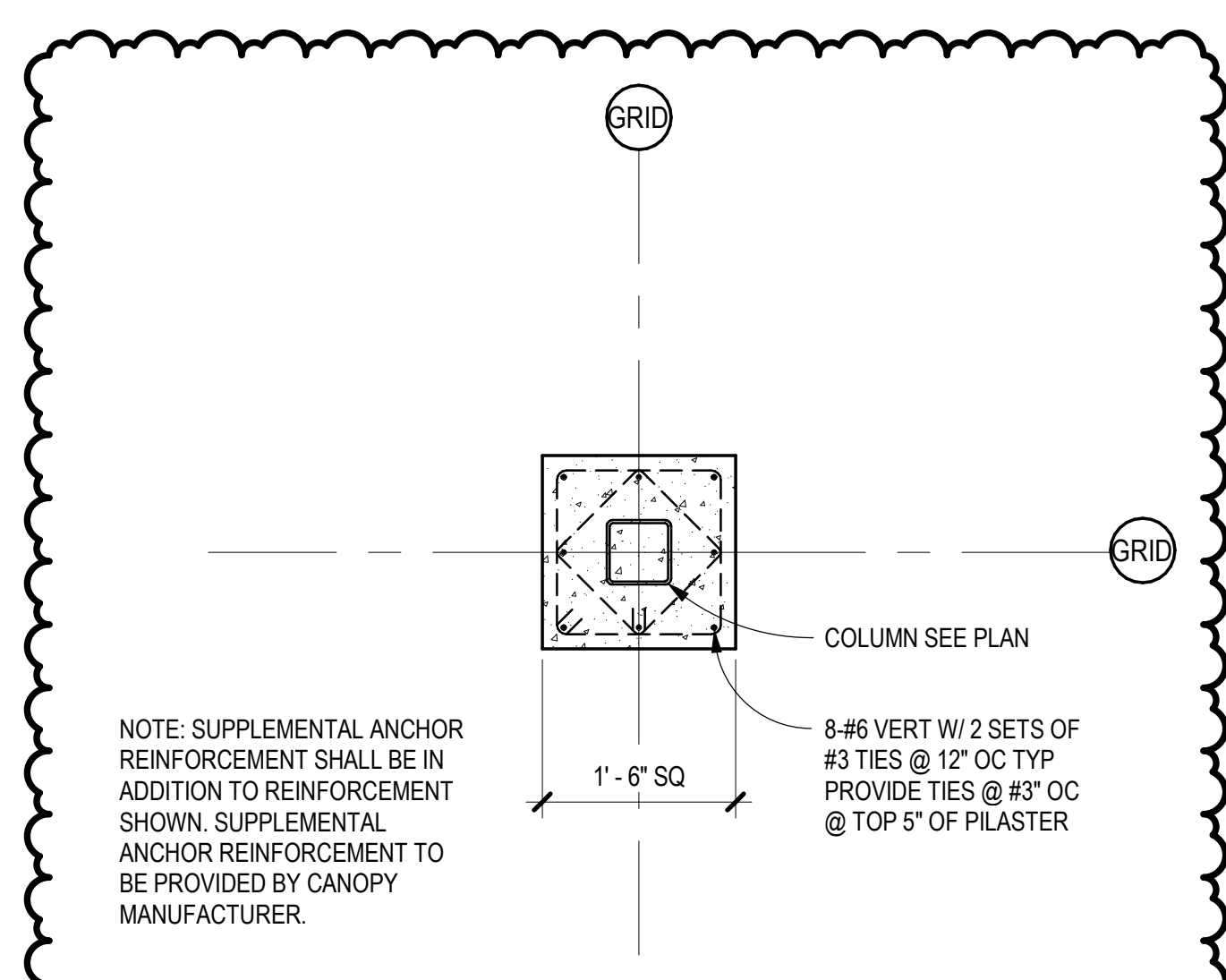
D5 HAIRPIN REINF AT COLUMN
SCALE: 3/4" = 1'-0"



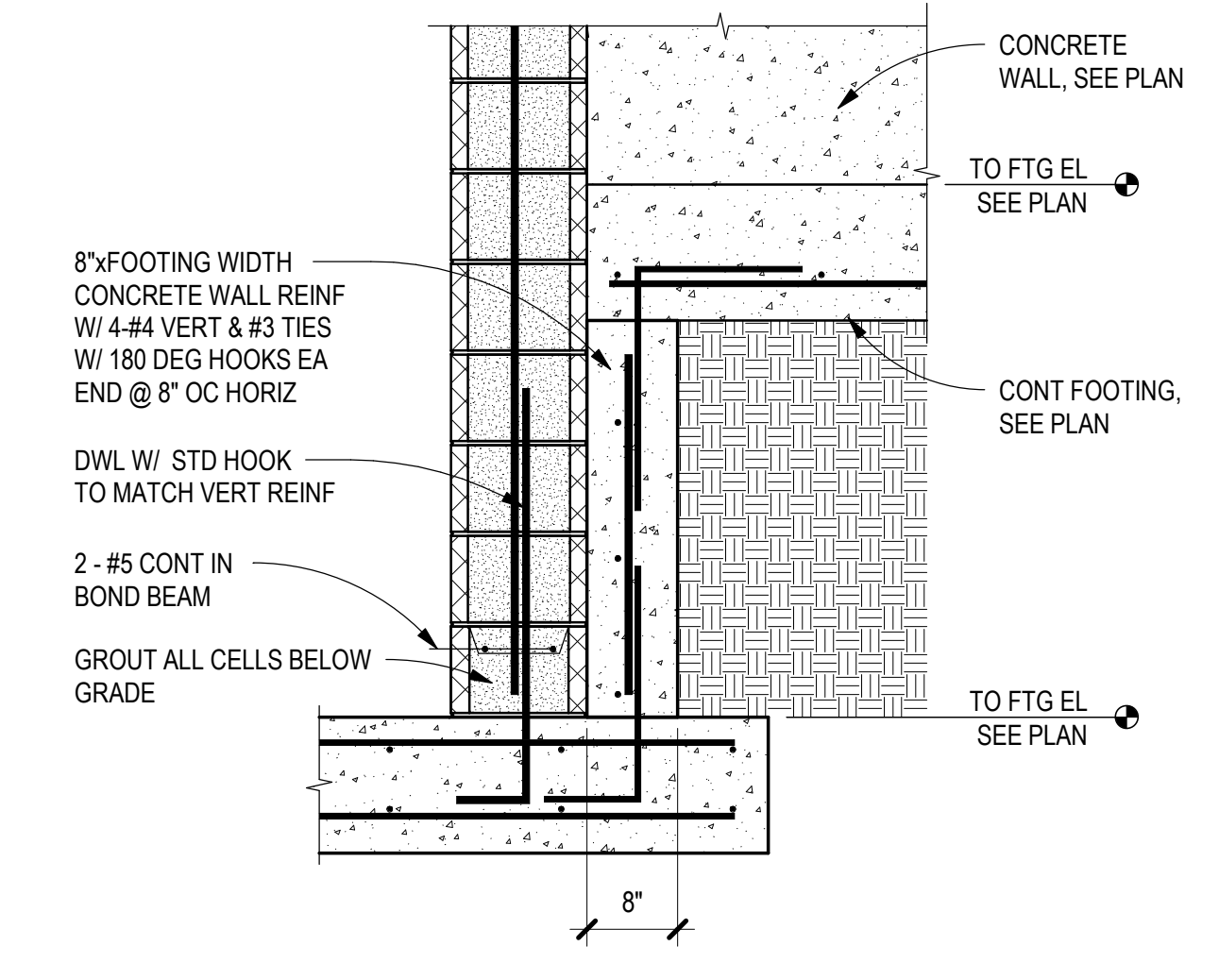
C4 EXTERIOR COLUMN SECTION
SCALE: 3/4" = 1'-0"



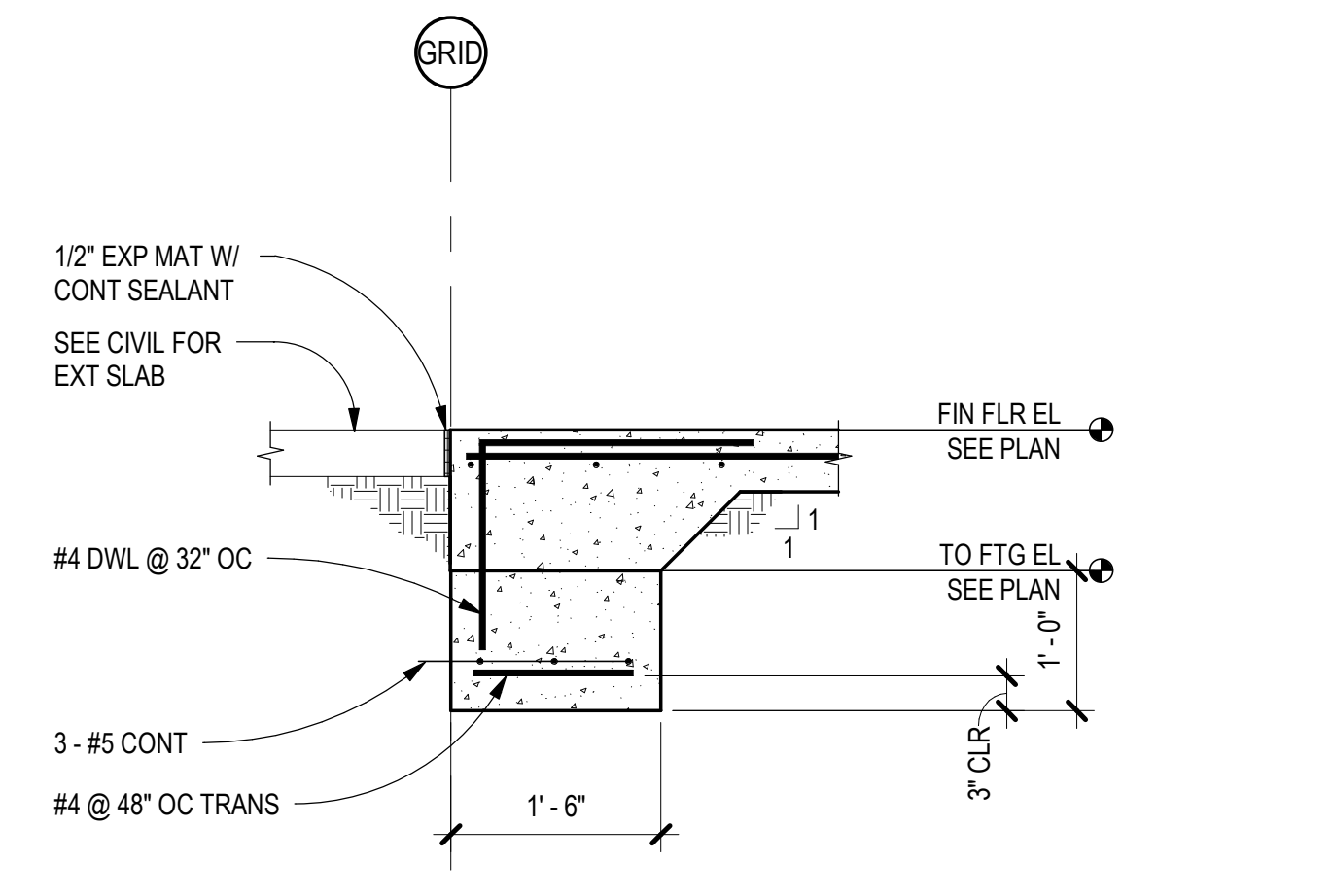
C5 PERIMETER SECTION @ COLUMN
SCALE: 3/4" = 1'-0"



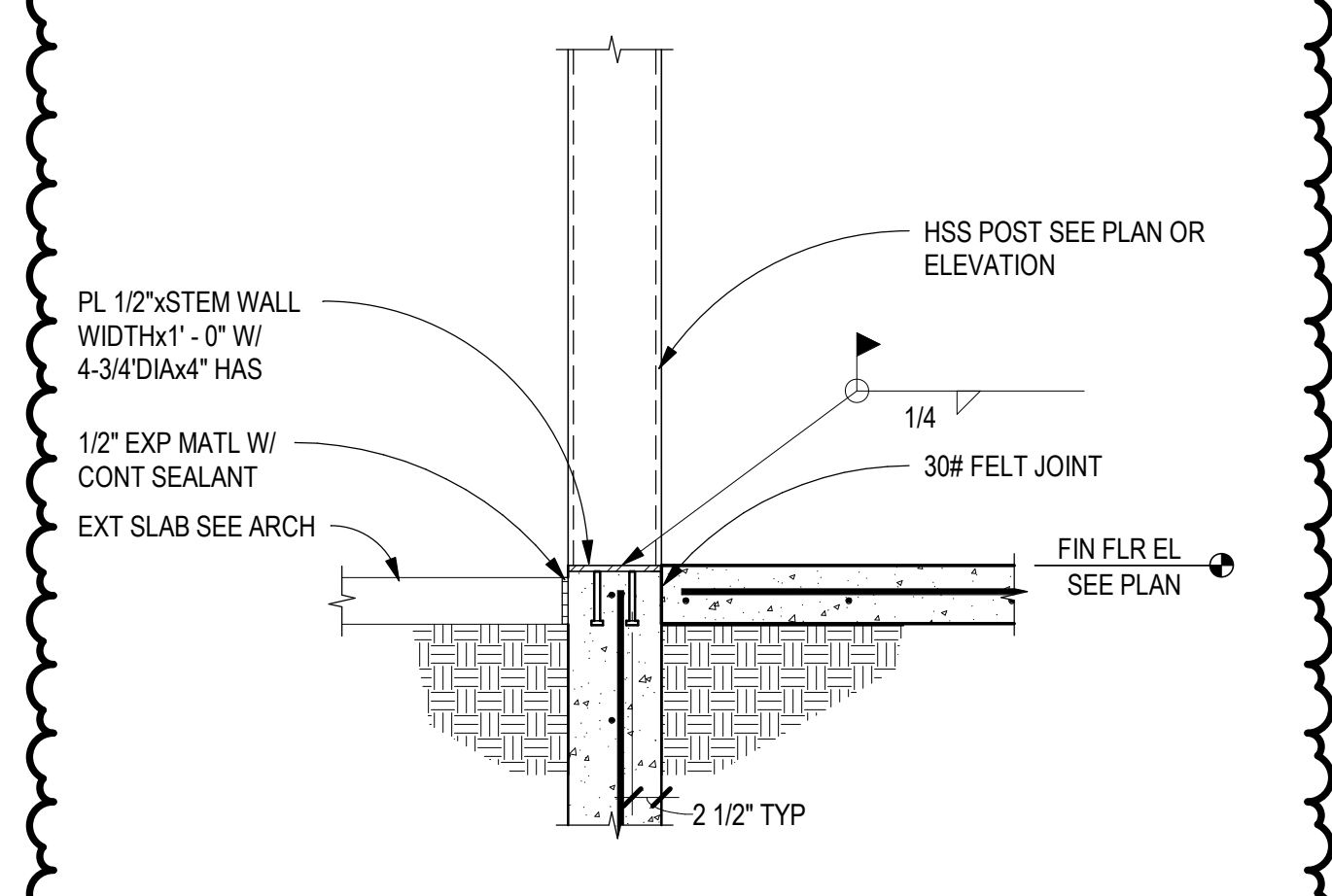
B3 PILASTER AT ELEVATOR SHAFT
SCALE: 3/4" = 1'-0"



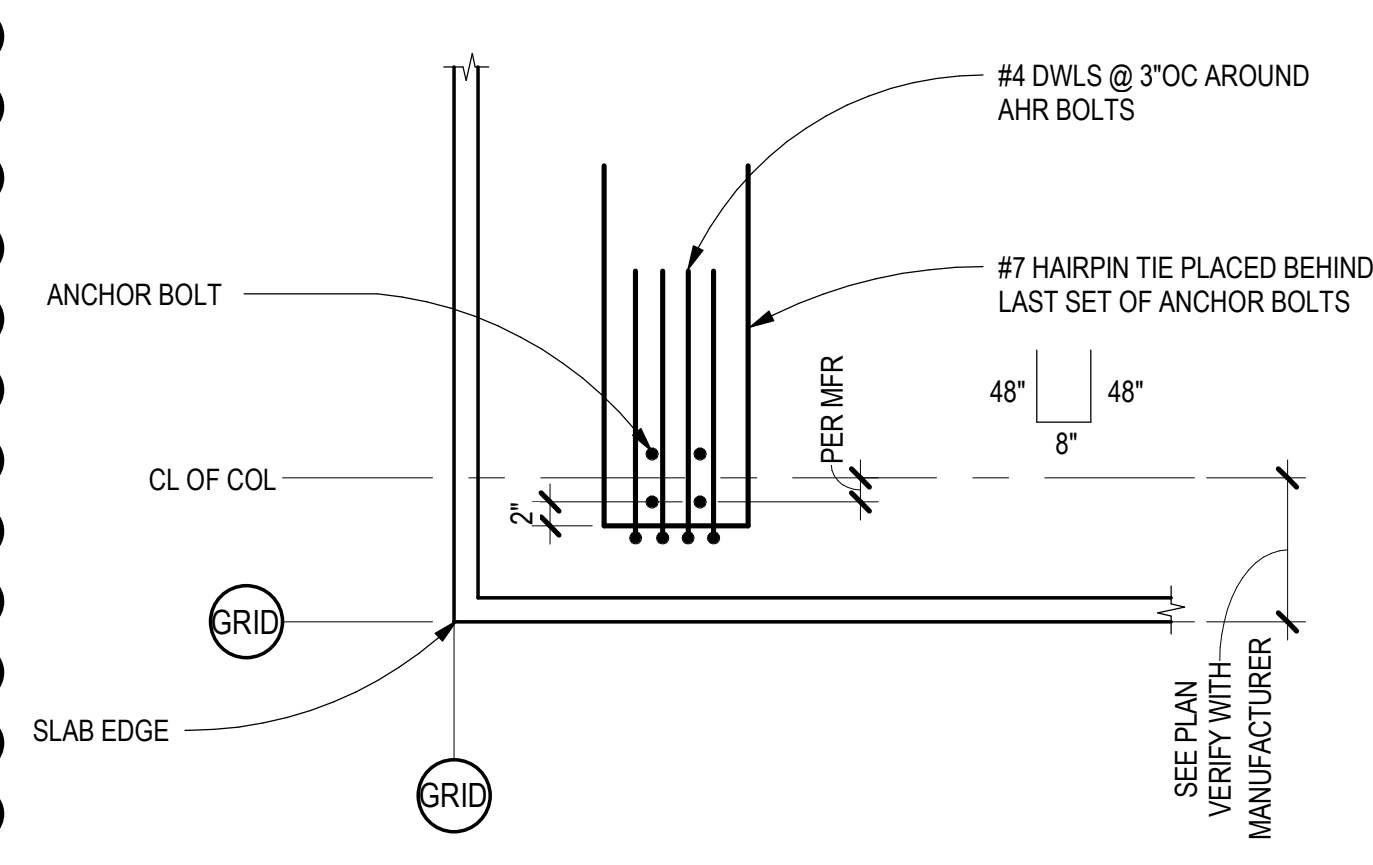
B4 FOUNDATION SECTION @ ELEVATOR
SCALE: 3/4" = 1'-0"



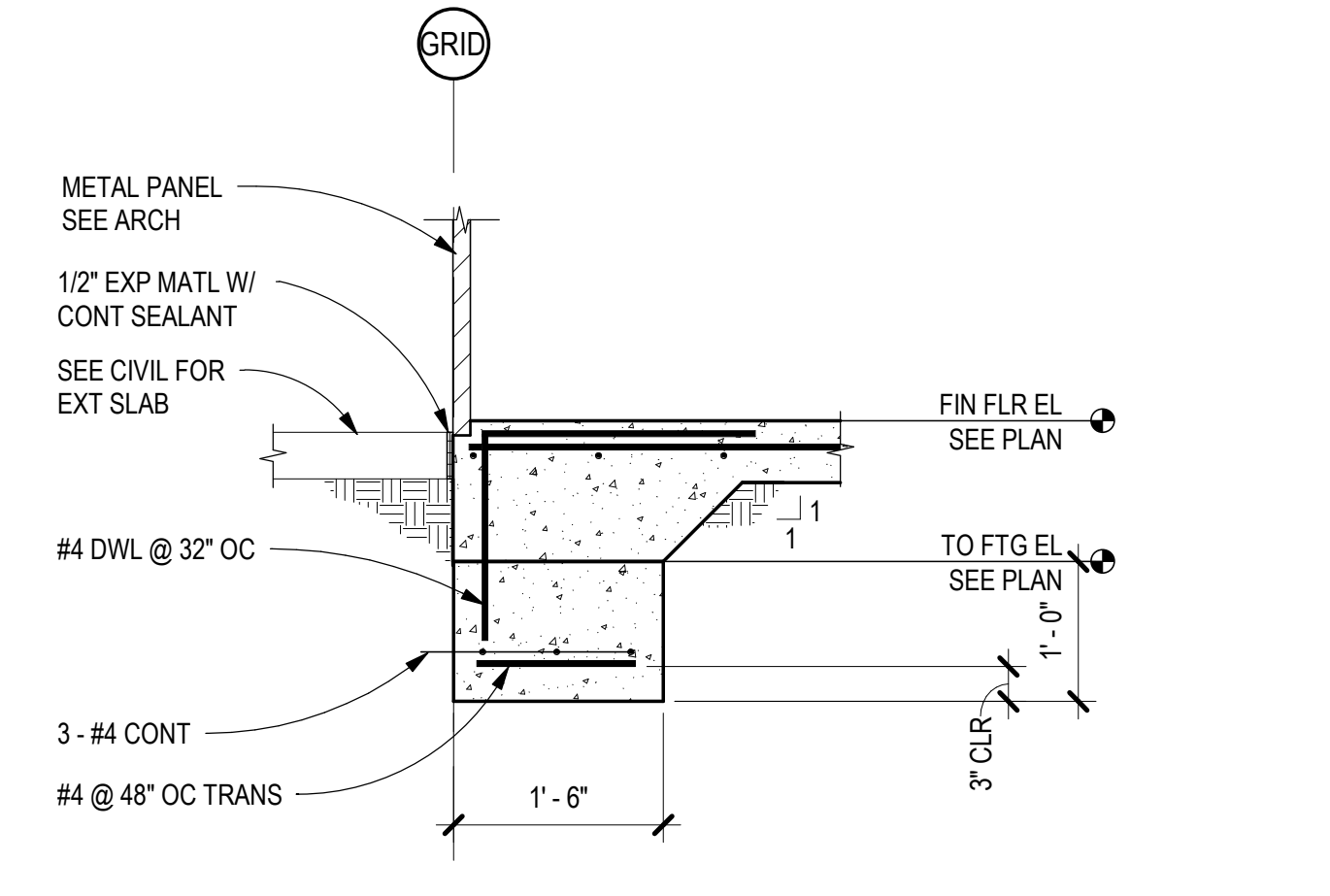
B5 FOUNDATION SECT @ OPENING
SCALE: 3/4" = 1'-0"



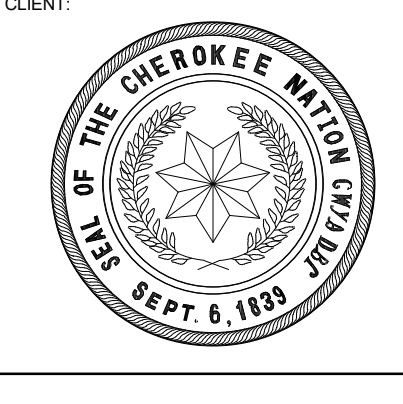
A3 POST TO STEM WALL CONNECTION
SCALE: 3/4" = 1'-0"



A4 HAIRPIN REINF AT CORNER COLUMN
SCALE: 3/4" = 1'-0"



A5 PERIMETER FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



**WILMA P. MANKILLER HEALTH CENTER
EXPANSION**
STILWELL, OKLAHOMA

KEY PLAN:

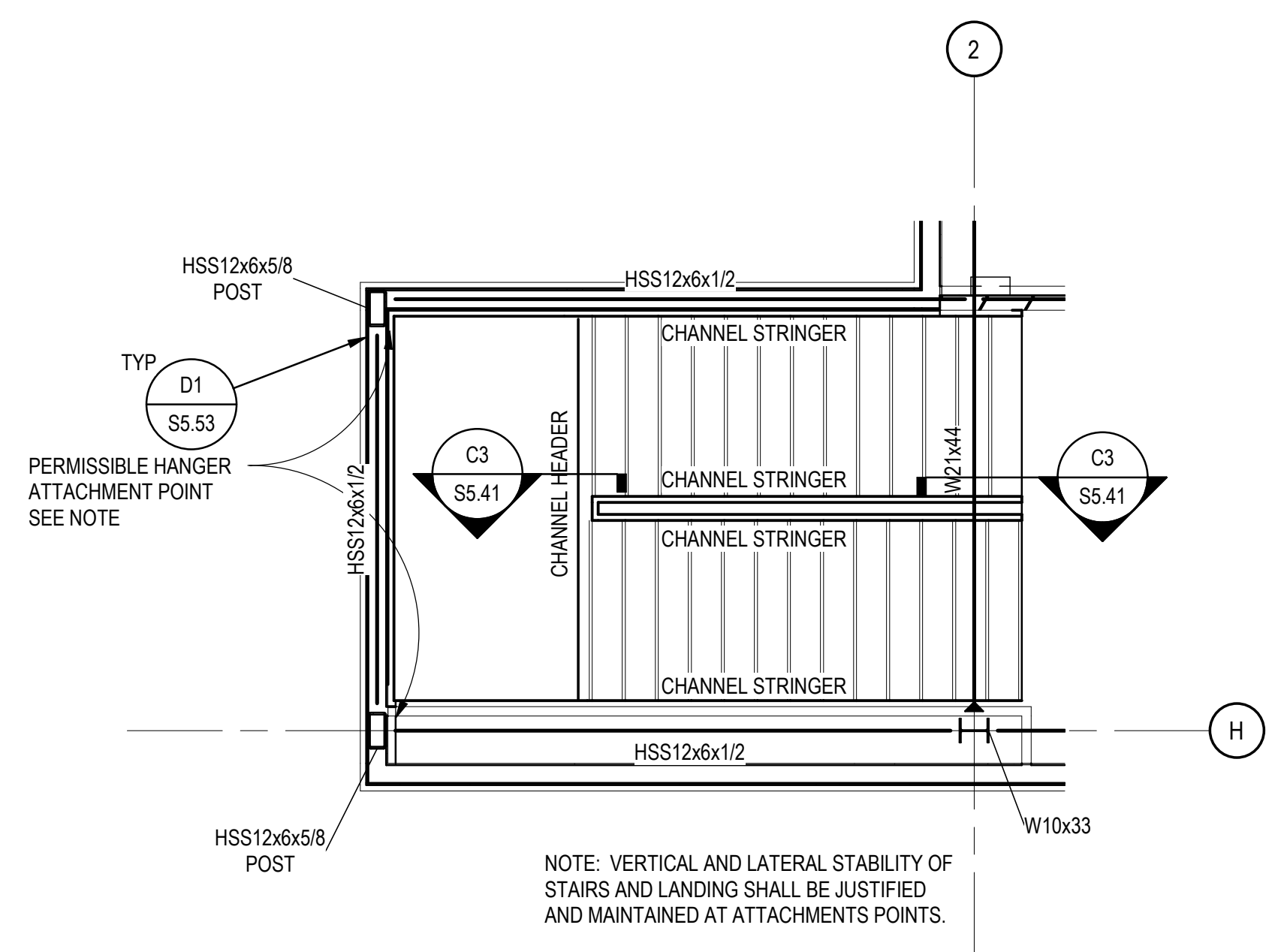
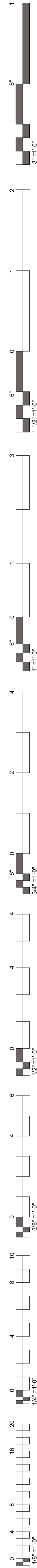
PROJECT PHASE:
BID PACKAGE 01

#	DATE	REVISIONS
1	11/02/19	BID PACKAGE 01 - ADD 01
2	12/10/19	BID PACKAGE 01 - ADD 02

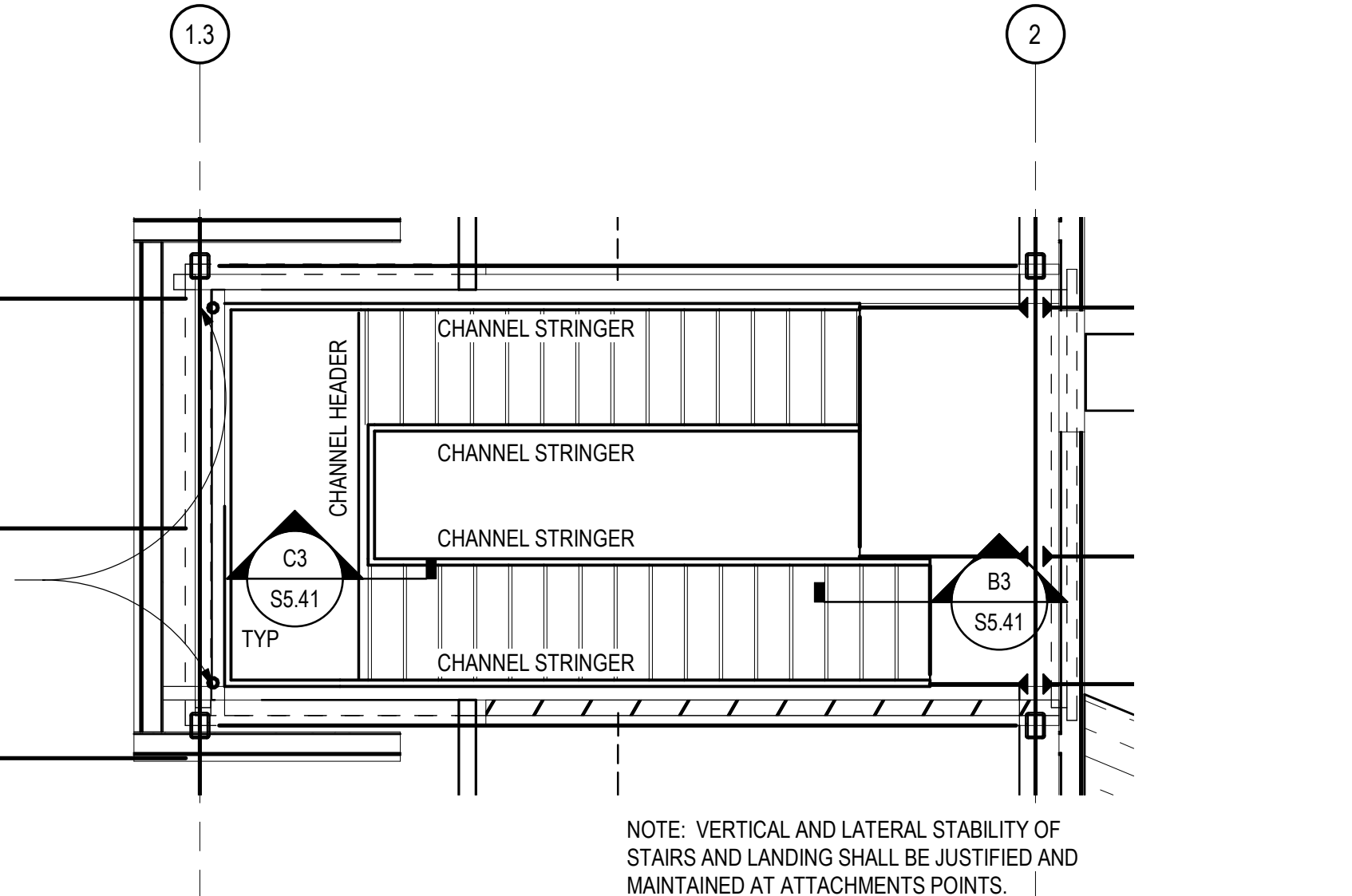
DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER: S3.12

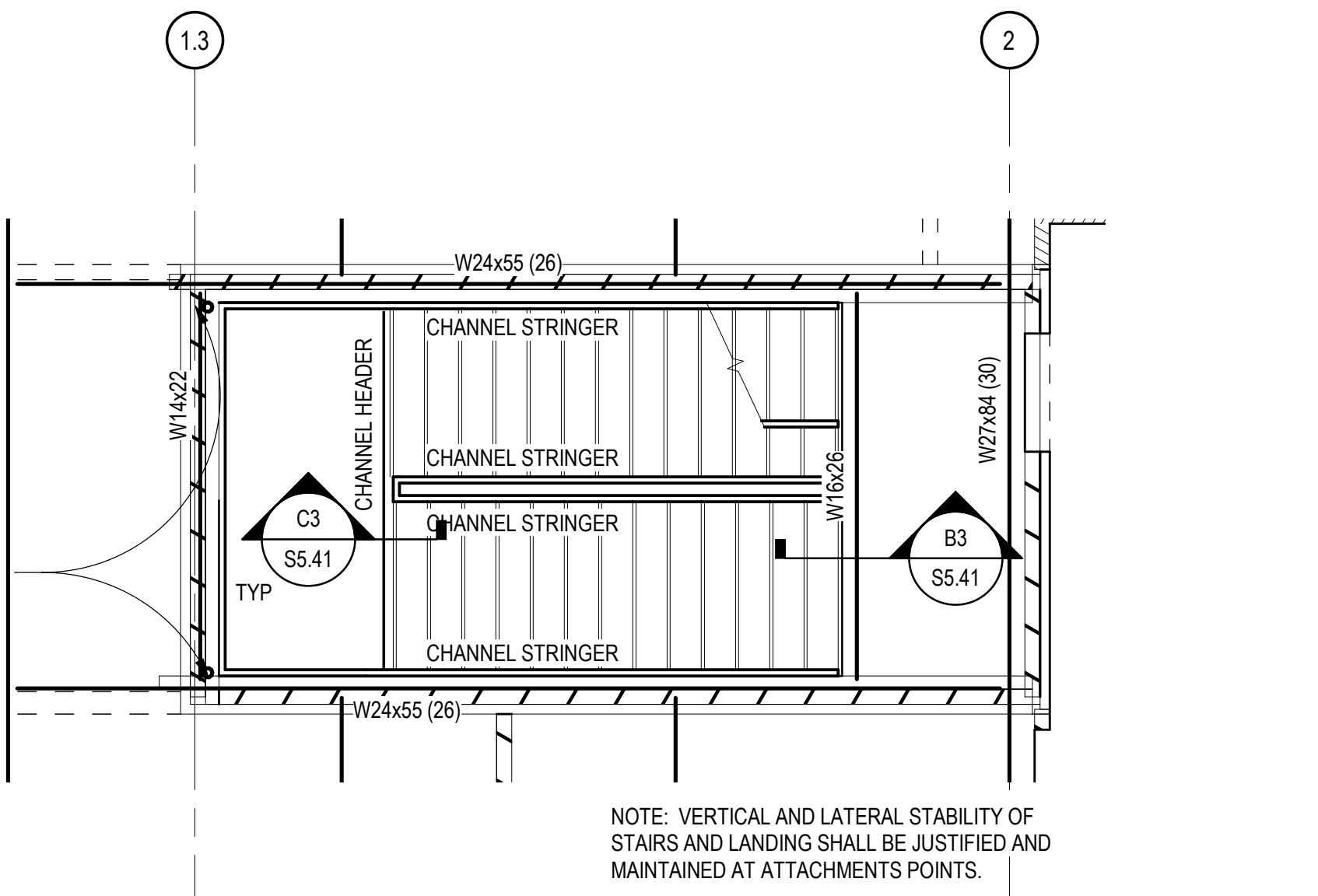
FOUNDATION SECTIONS



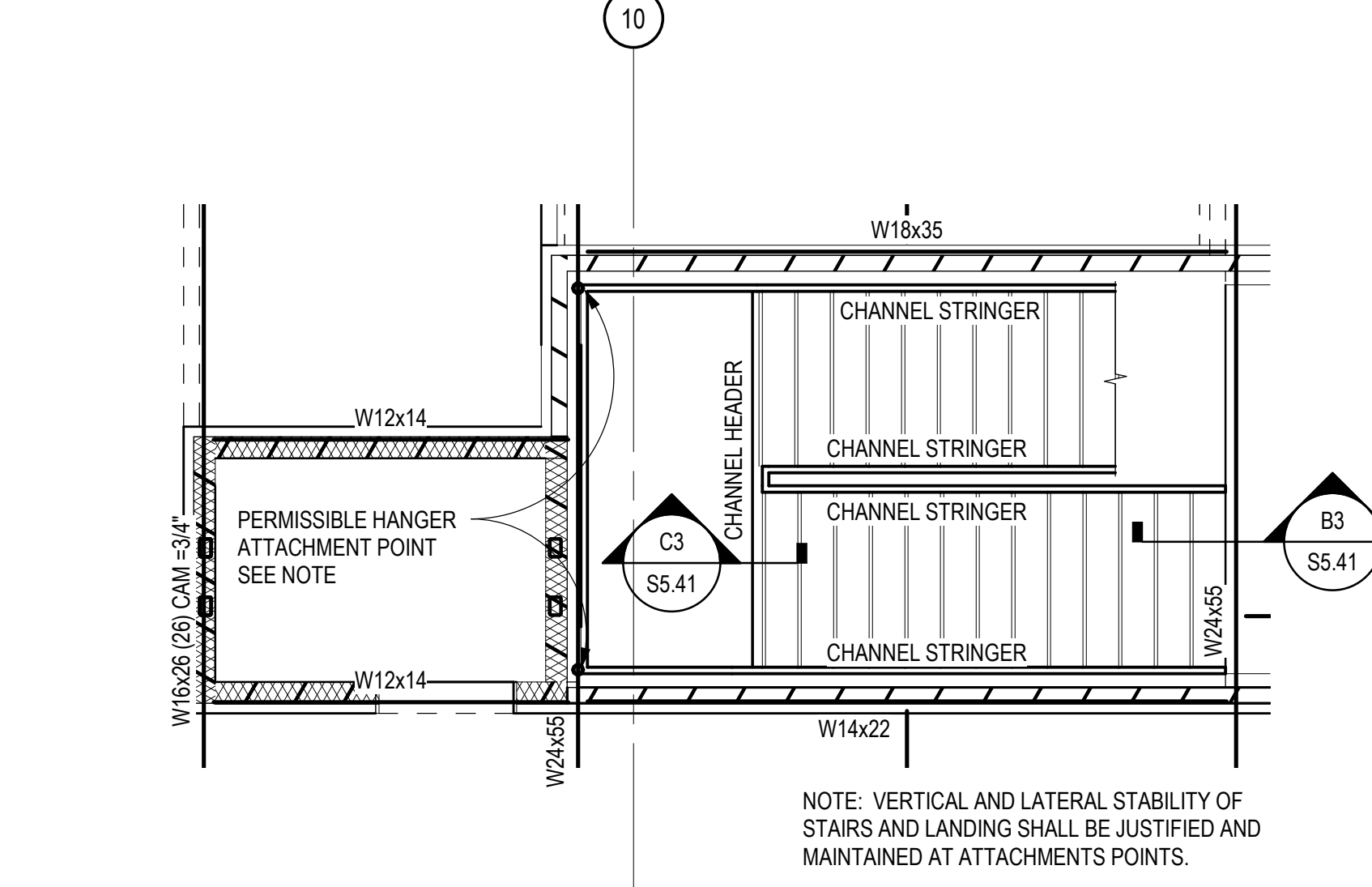
D1 ENLARGED SOUTHWEST STAIR FRAMING PLAN
SCALE: 1/4" = 1'-0"



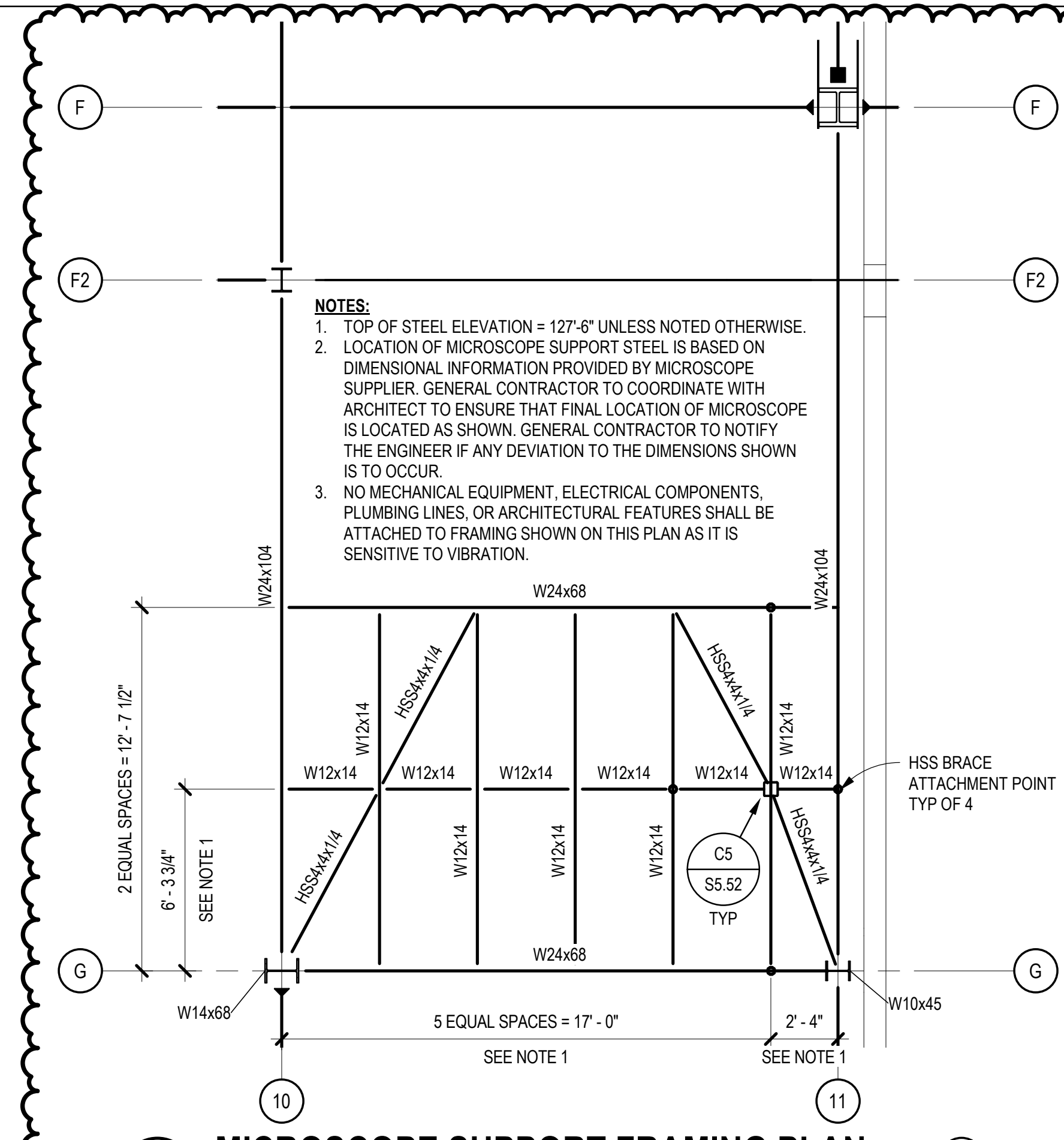
C1 ENLARGED WEST STAIR FRAMING PLAN TO MEZZANINE
SCALE: 1/4" = 1'-0"



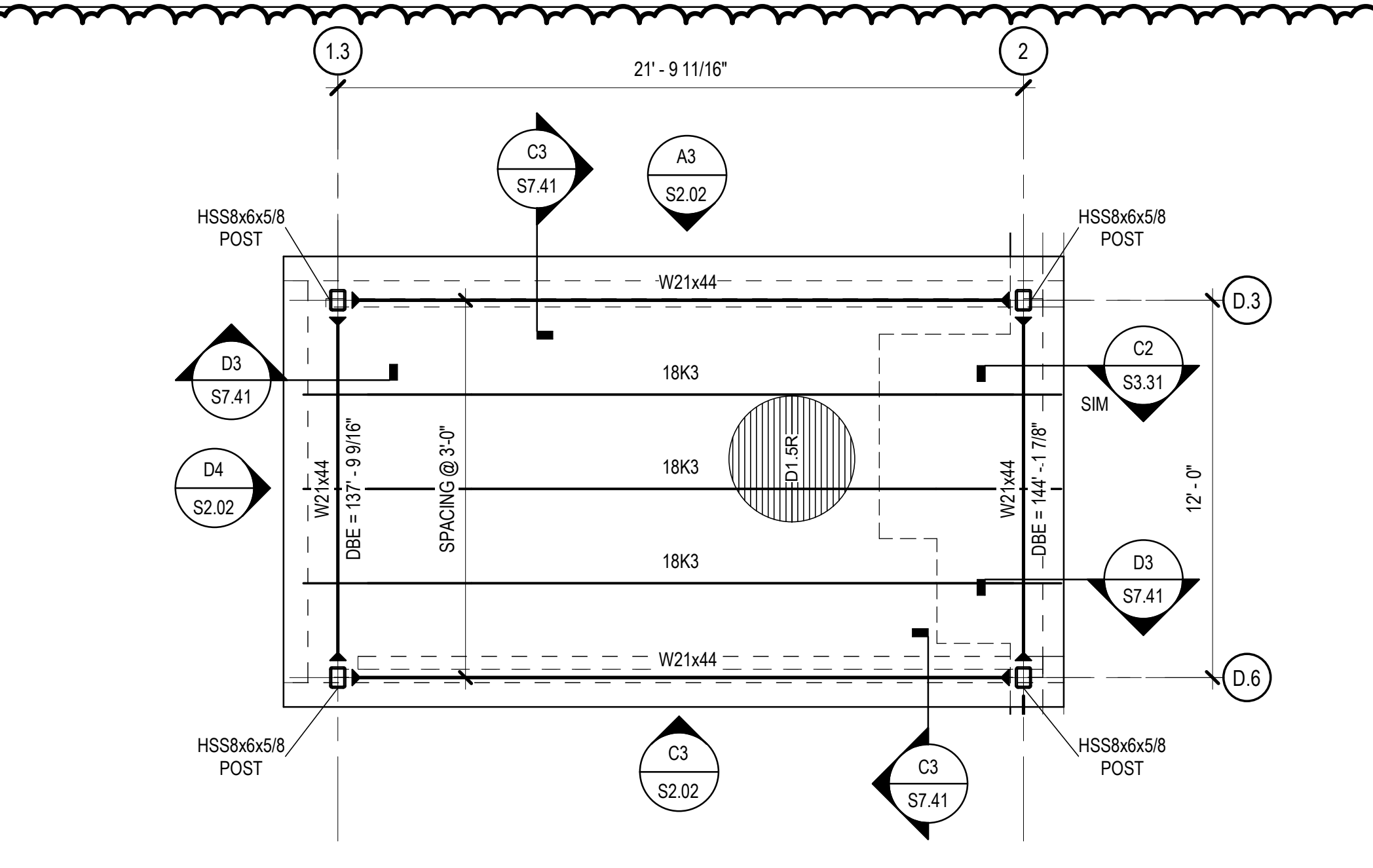
B1 ENLARGED WEST STAIR FRAMING PLAN
SCALE: 1/4" = 1'-0"



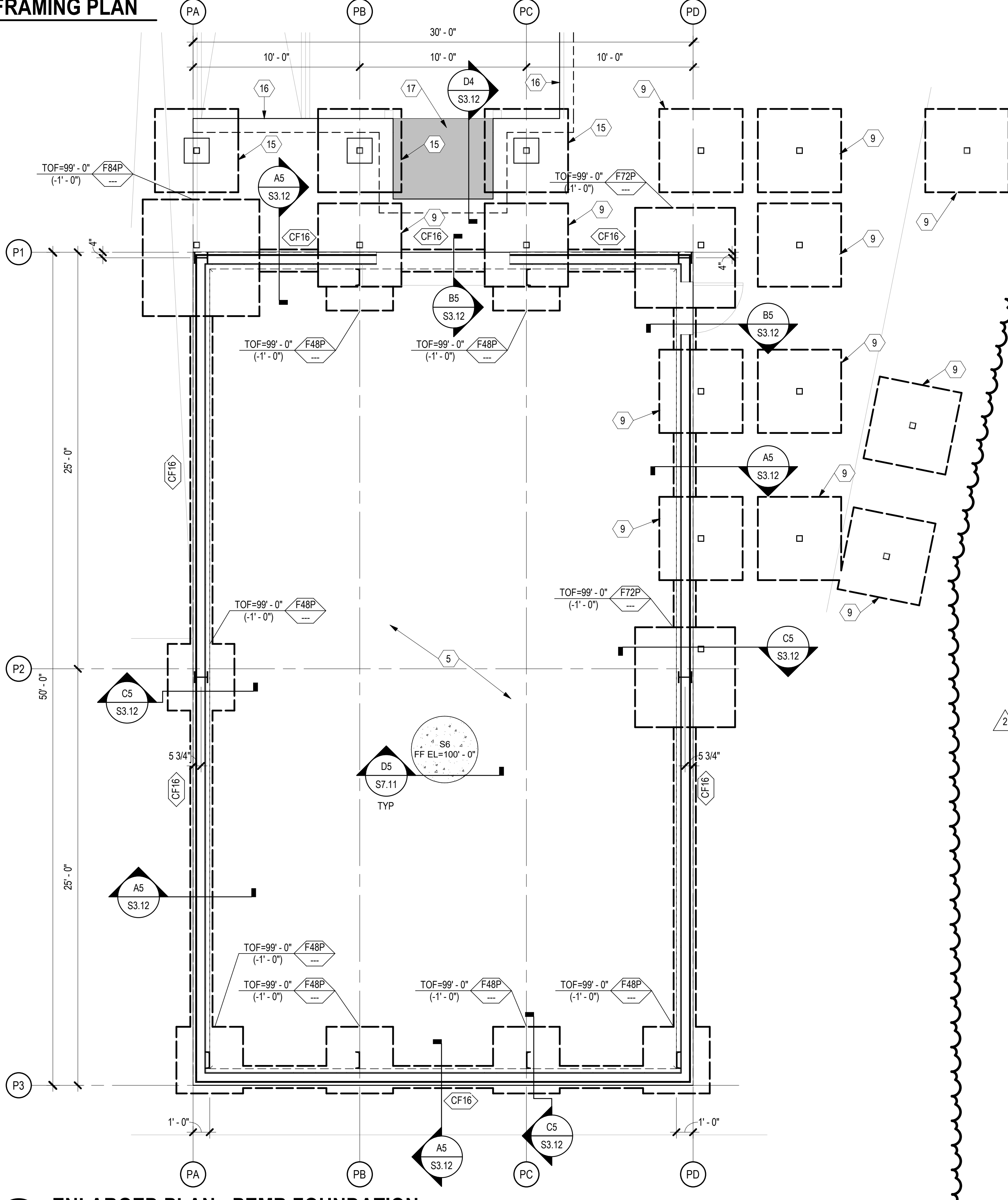
A1 ENLARGED EAST STAIR FRAMING PLAN
SCALE: 1/4" = 1'-0"



D2 MICROSCOPE SUPPORT FRAMING PLAN
SCALE: 1/4" = 1'-0"



D4 ENLARGED PLAN - WEST ROOF POP-UP
SCALE: 1/4" = 1'-0"



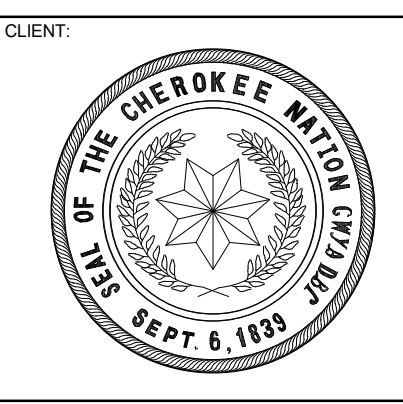
A3 ENLARGED PLAN - PEMB FOUNDATION
SCALE: 1/4" = 1'-0"

GENERAL SHEET NOTES

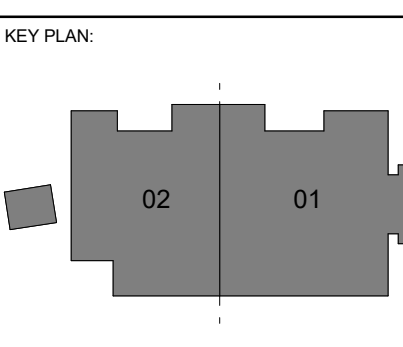
- SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- REFERENCE FINISH FLOOR ELEVATION 100'-0" = MEAN SEA FINISH FLOOR ELEVATION. SEE CIVIL DRAWINGS.
- TOP OF FOOTING ELEVATION = 98'-0" (-2'-0"), UNLESS NOTED OTHERWISE ON PLAN.
- NOTE TO CONTRACTOR: ENLARGED SLAB BLOCKOUTS MAY BE REQUIRED AT FRAME COLUMNS FOR MOMENT FRAME BASE PLATE CLEARANCE.
- NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
- DIMENSIONS ARE TO THE FACE OF STUD UNLESS NOTED OTHERWISE.
- SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
- EXISTING CONSTRUCTION IS PER AVAILABLE EXISTING DRAWINGS. ALL EXISTING CONSTRUCTION AND DIMENSIONS SHALL BE VERIFIED PRIOR TO CONSTRUCTION. SHOULD CONDITIONS VARY FROM THOSE SHOWN, CONTACT ENGINEER BEFORE PROCEEDING.
- PROVIDE SLAB JOINTS AT 10'-0" ON CENTER MAXIMUM. THE AREA OF THE CONTROL JOINT SHALL NOT EXCEED A 2:1 RATIO. CONTROL JOINTS SHALL BE LOCATED AT COLUMN LINES WHERE THE LAYOUT PERMITS. AT RE-ENTRANT CORNERS THAT DO NOT HAVE CONTROL JOINTS, PROVIDE 2-#4 x 3'-0" DIAGONAL TO THE RE-ENTRANT CORNER.
- STRUCTURAL COLD FORMED METAL STUDS SHALL BE 6" WIDE UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING BY OTHERS.
- SEE SHEET S7.00 SERIES SHEETS FOR TYPICAL FOUNDATION SECTIONS AND DETAILS.
- SEE SHEET S6.01 FOR SCHEDULES.

SHEET KEYNOTE

- FLOOR DRAIN, SLOPE SLAB TO DRAIN 1/8" PER FOOT. COORDINATE EXACT SIZE AND LOCATION WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
- POST AND FOUNDATION AS REQUIRED FOR STAIR SUPPORT. STAIR ENGINEER TO PROVIDE REQUIRED LOADS AND LOCATIONS.
- ELEVATOR SUMP PIT. COORDINATE EXACT SIZE AND LOCATION WITH ELEVATOR MANUFACTURER. SEE A4 / S5.41
- HSS8x4x1/2 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION AND SPACING WITH ELEVATOR MANUFACTURER. SEE B4 / S5.41
- PRE-ENGINEERED METAL BUILDING STEEL AND ANCHORAGE BY OTHERS. CONTRACTOR TO CONFIRM LOCATIONS OF FOUNDATIONS WITH FINAL PRE-ENGINEERED METAL BUILDING SHOP DRAWINGS.
- NOTCH MASONRY AS REQUIRED TO FACILITATE BASEPLATE INSTALLATION. STEP BOND BEAM AT THIS LOCATION. FILL VOID FROM NOTCH WITH NON-SHRINK GROUT.
- CUT AND REMOVE EXISTING SLAB AS REQUIRED TO PLACE NEW FOOTING. NEW SLAB TO POUR UP TO REMAINING SLAB.
- CENTER FOOTING ON GRID C.
- F80A PRE-MANUFACTURED SUNSHADE CONCRETE FOOTING. TOP OF FOOTING = 99'-0" (-1'-0"). SEE SHEET S6.01 FOR FOOTING SCHEDULE. COORDINATE FINAL LOCATION WITH SUNSHADE MANUFACTURER.
- EXISTING CANOPY. SEE ARCHITECTURAL DEMOLITION PLANS FOR EXTENT OF DEMOLITION.
- HSS8x6x1/2 ELEVATOR SUPPORT POST. COORDINATE EXACT LOCATION AND SPACING WITH ELEVATOR MANUFACTURER. SEE B4 / S5.41, D3 / S5.41, A2 / S5.41, B2 / S5.41, AND C2 / S5.41
- 1 1/2" RECESSED SLAB AT ADA SHOWER. COORDINATE EXACT SIZE, LOCATION, AND SLOPE REQUIREMENTS WITH ARCHITECTURAL DRAWINGS. SEE C4 / S7.11
- 18" DIAMETER PRECAST CONCRETE COLUMN BY OTHERS. SEE C3 / S3.11 AND B1 / S3.31
- 18" DIAMETER PRECAST CONCRETE CANOPY COLUMN BY OTHERS. SEE C3 / S3.11, C4 / S3.12, A1 / S3.31, AND A5 / S3.31
- F80A PRE-MANUFACTURED SUNSHADE CONCRETE FOOTING WITH 18" SQUARE CONCRETE PEDESTAL. TOP OF FOOTING ELEVATION TO MATCH TOP OF FOOTING ELEVATION OF SITE RETAINING WALL. SEE SHEET S6.01 FOR FOOTING SCHEDULE. SEE B3 / S3.12 FOR PEDESTAL DETAIL. ANCHORAGE AND SUPPLEMENTAL ANCHOR REINFORCEMENT FOR PRE-MANUFACTURED CANOPY TO BE PROVIDED BY MANUFACTURER.
- SITE RETAINING WALL. SEE D2 / S3.11
- DOCK LEVELER. SEE ARCHITECTURAL FOR EXACT LOCATION AND DIMENSIONS.



**WILMA P. MANKILLER HEALTH CENTER
EXPANSION**
STILWELL, OKLAHOMA



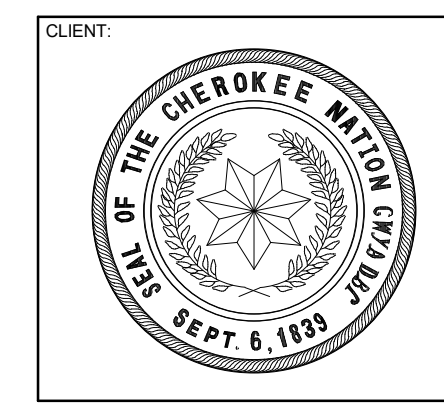
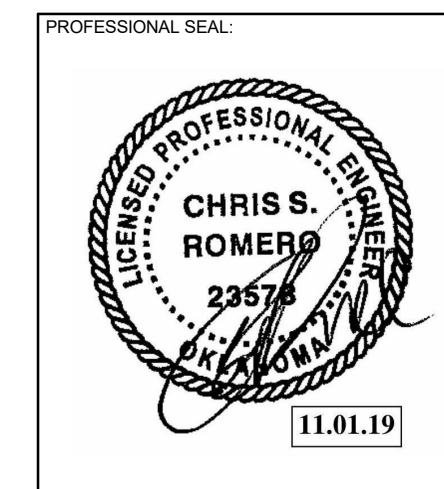
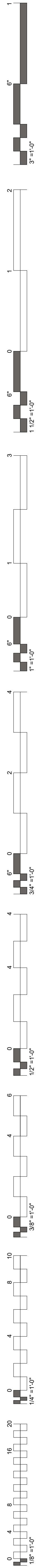
PROJECT PHASE:
BID PACKAGE 01

#	DATE	REVISIONS
1	11/02/19	BID PACKAGE 01 - ADD 01
2	12/10/19	BID PACKAGE 01 - ADD 02

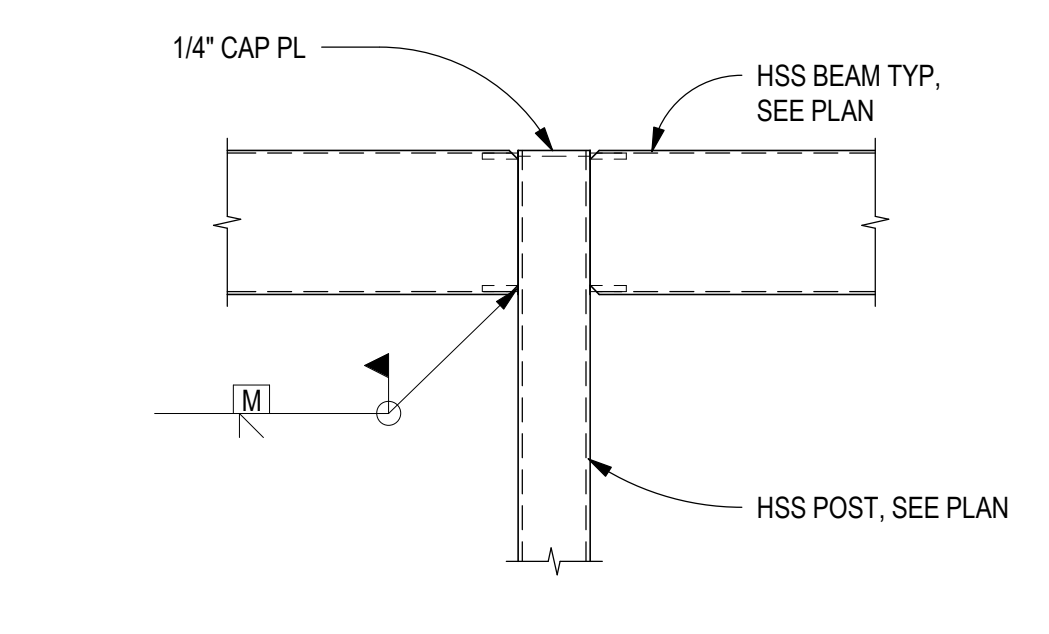
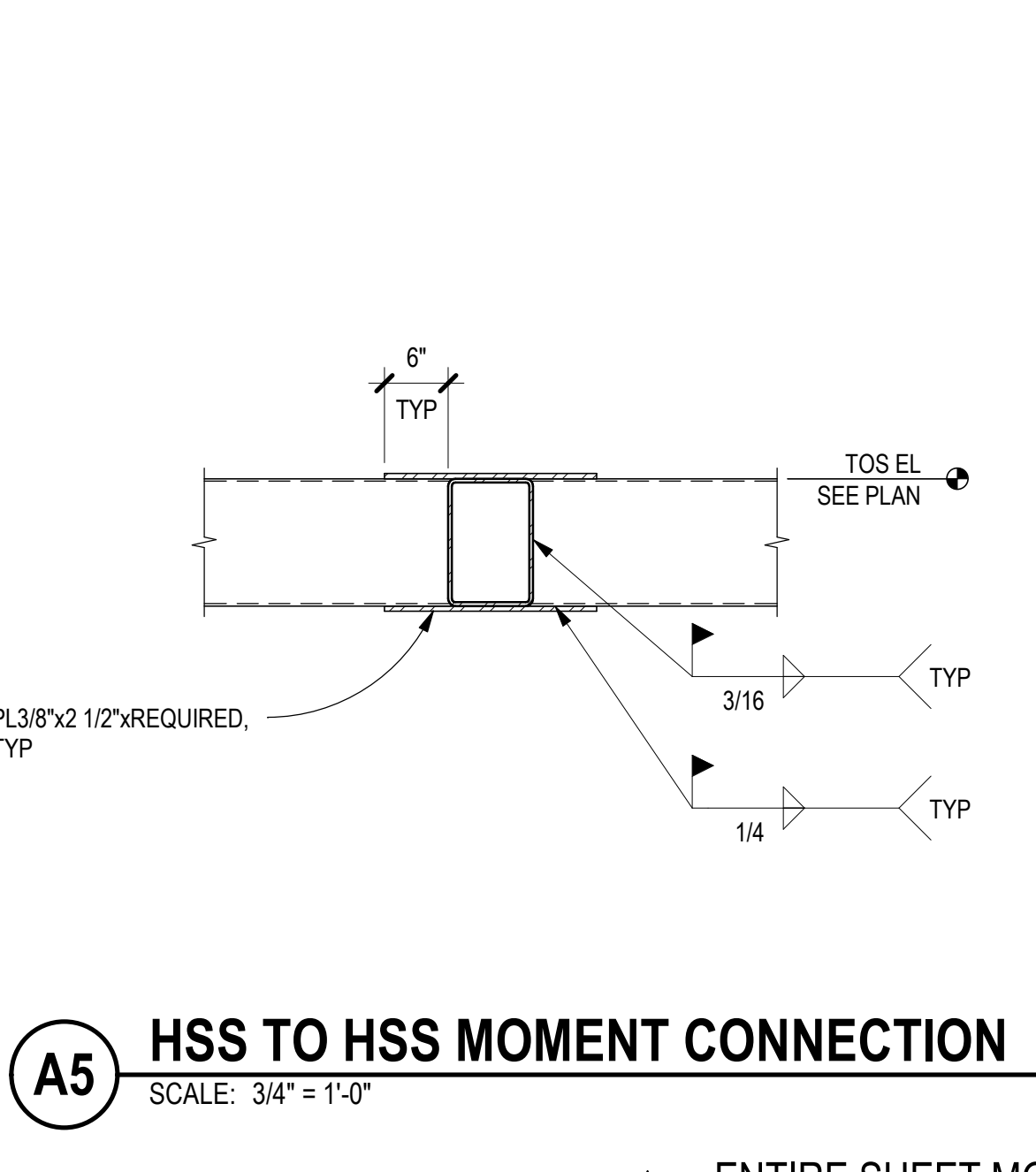
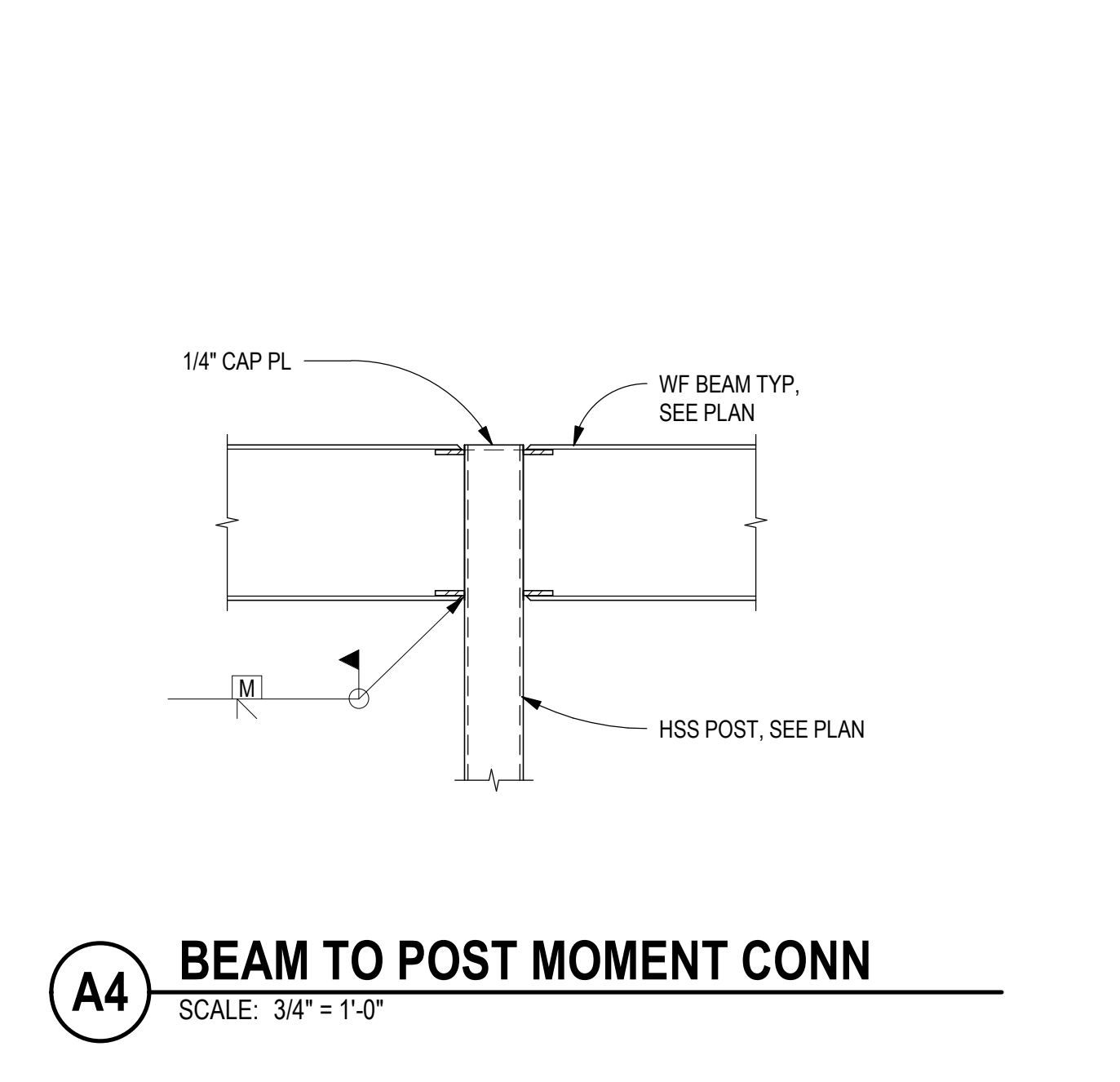
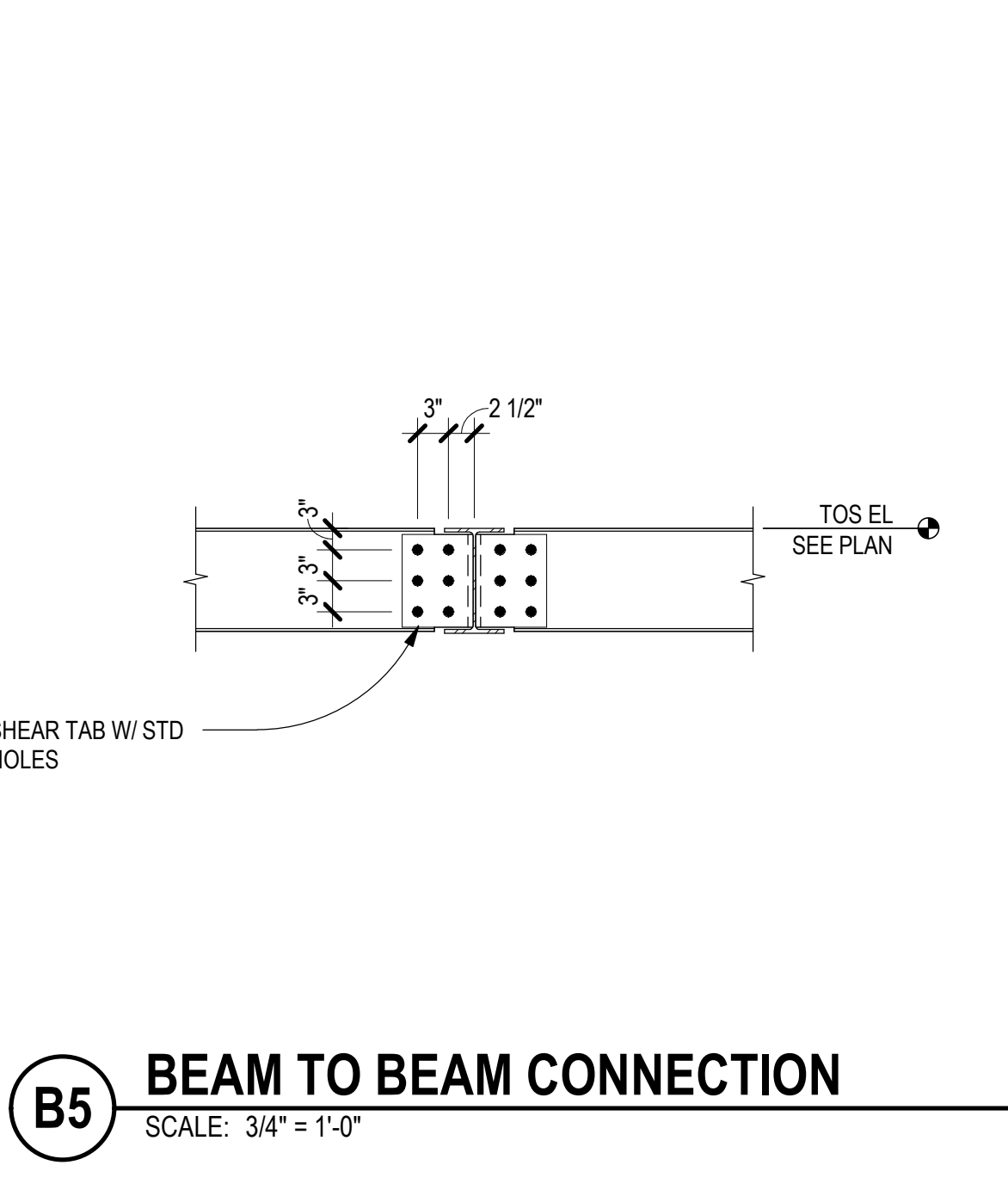
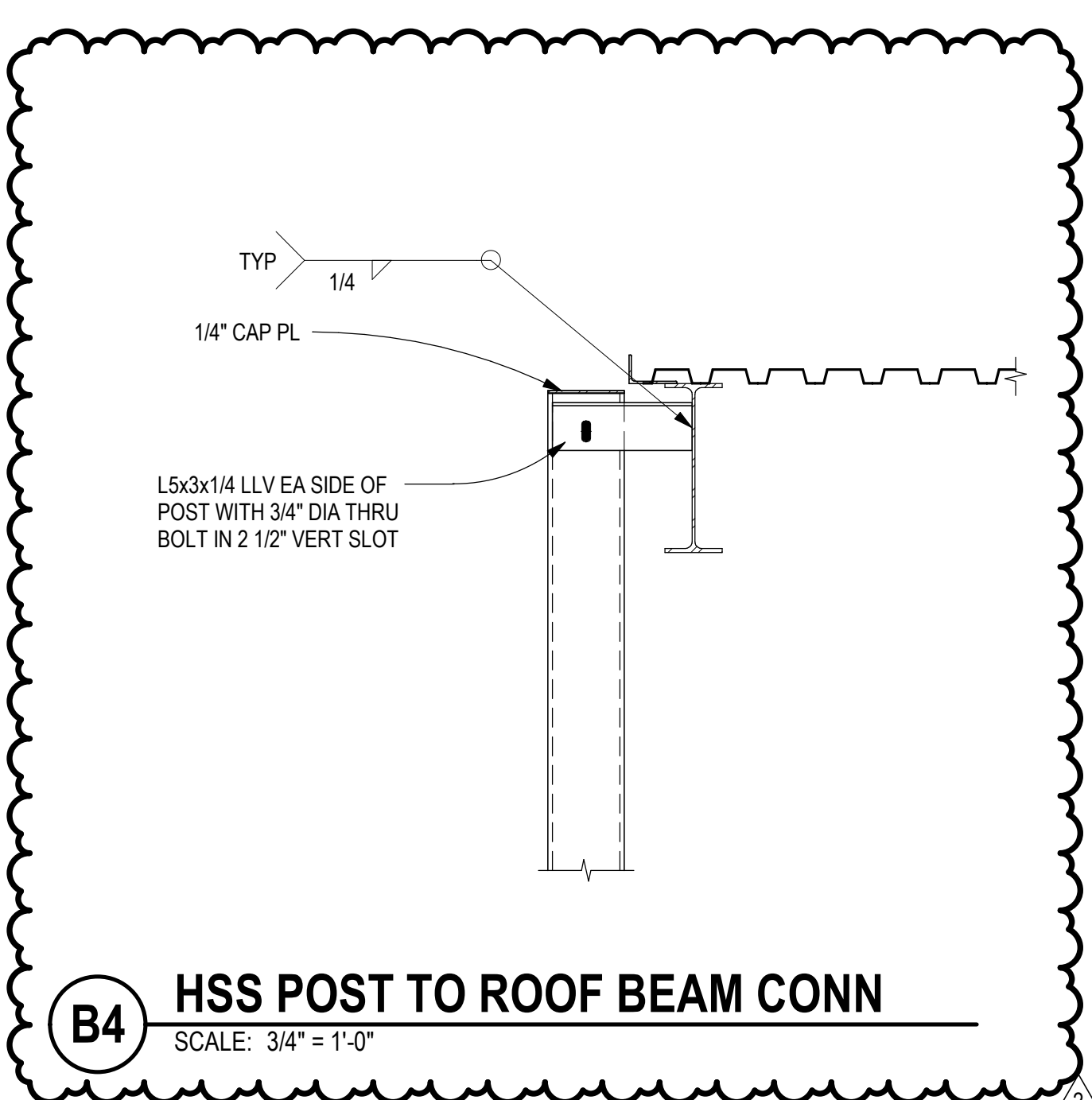
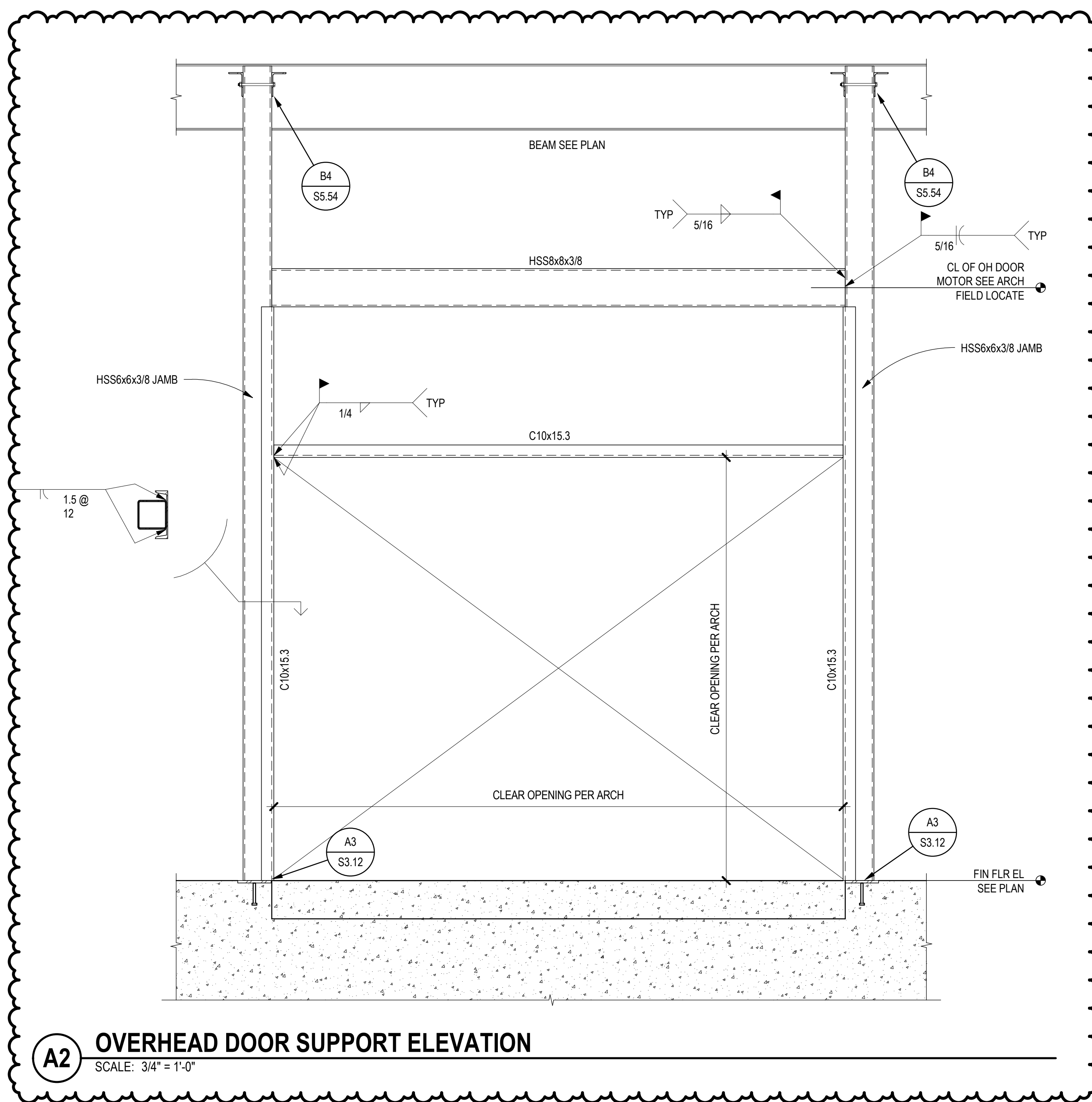
DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER: S4.01

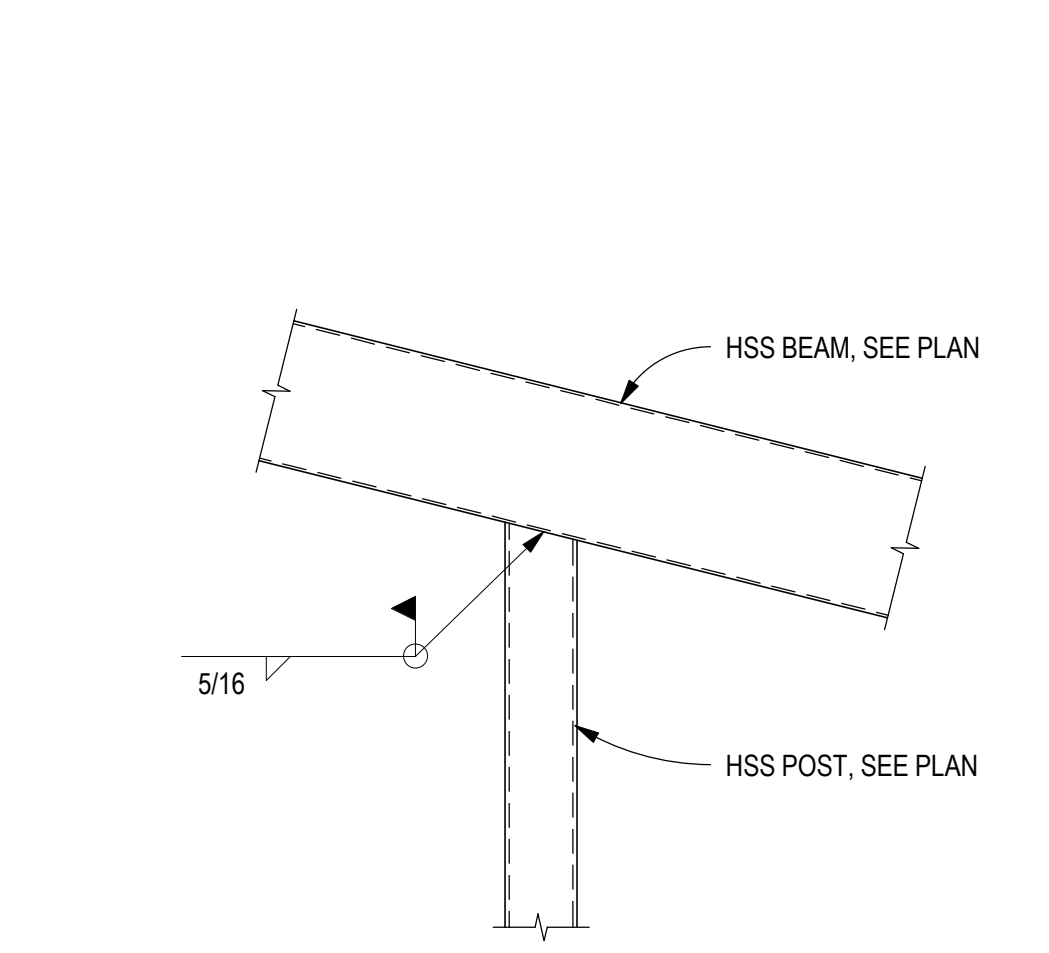
ENLARGED PLANS



**WILMA P. MANKILLER HEALTH CENTER
EXPANSION**
STILWELL, OKLAHOMA



D5 BEAM TO POST MOMENT CONN
SCALE: 3/4" = 1'-0"



C5 BEAM OVER POST CONNECTION
SCALE: 3/4" = 1'-0"

B5 BEAM TO BEAM CONNECTION
SCALE: 3/4" = 1'-0"

B4 HSS POST TO ROOF BEAM CONN
SCALE: 3/4" = 1'-0"

A5 HSS TO HSS MOMENT CONNECTION
SCALE: 3/4" = 1'-0"

A4 BEAM TO POST MOMENT CONN
SCALE: 3/4" = 1'-0"

A2 OVERHEAD DOOR SUPPORT ELEVATION
SCALE: 3/4" = 1'-0"

ENTIRE SHEET MODIFIED

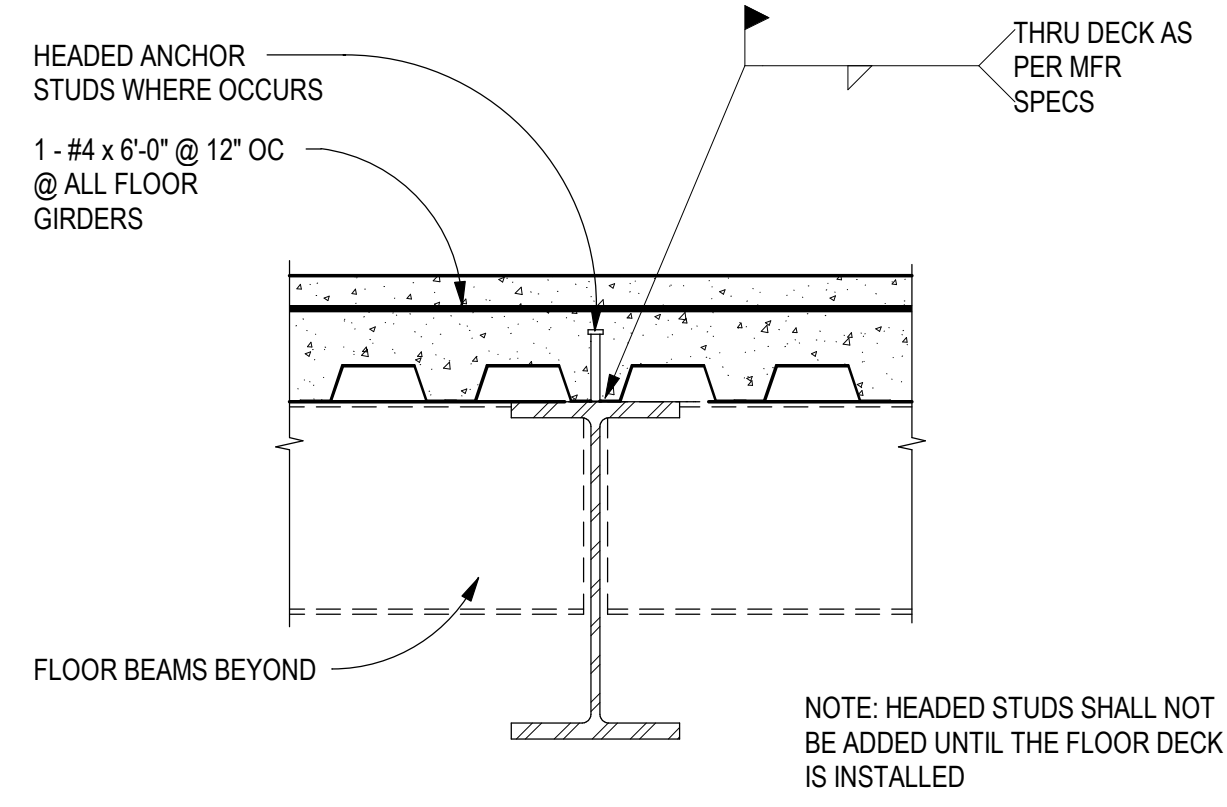
KEY PLAN:

PROJECT PHASE:
BID PACKAGE 01

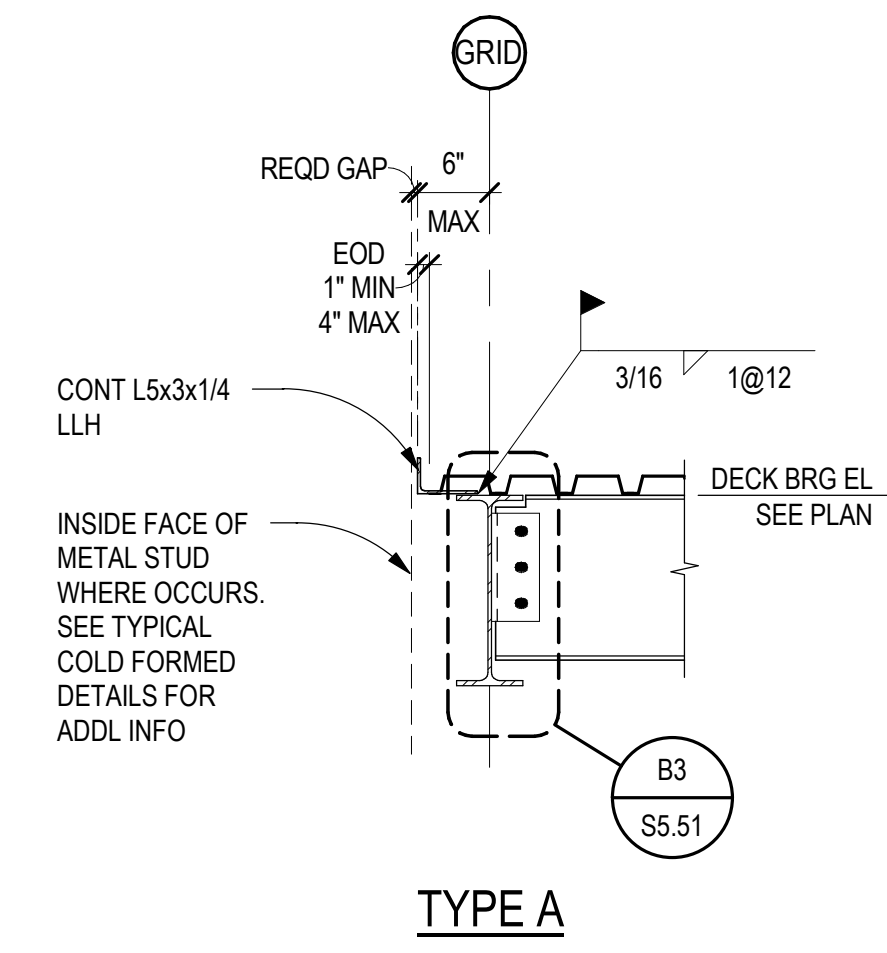
#	DATE	REVISIONS / DESCRIPTION
1	11/02/19	BID PACKAGE 01 - ADD 01
2	12/10/19	BID PACKAGE 01 - ADD 02

DATE: 11-01-19	JOB NUMBER: 18-01.01
-------------------	-------------------------

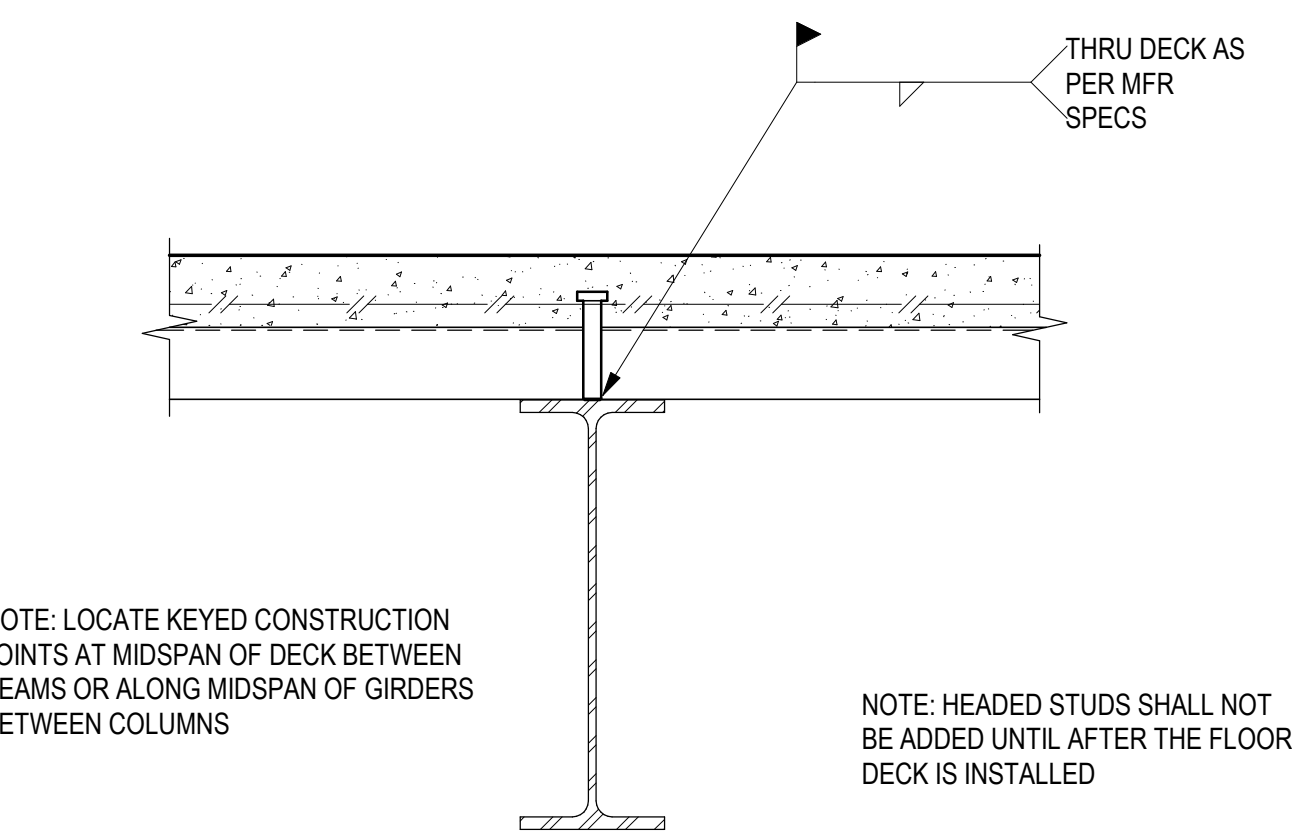
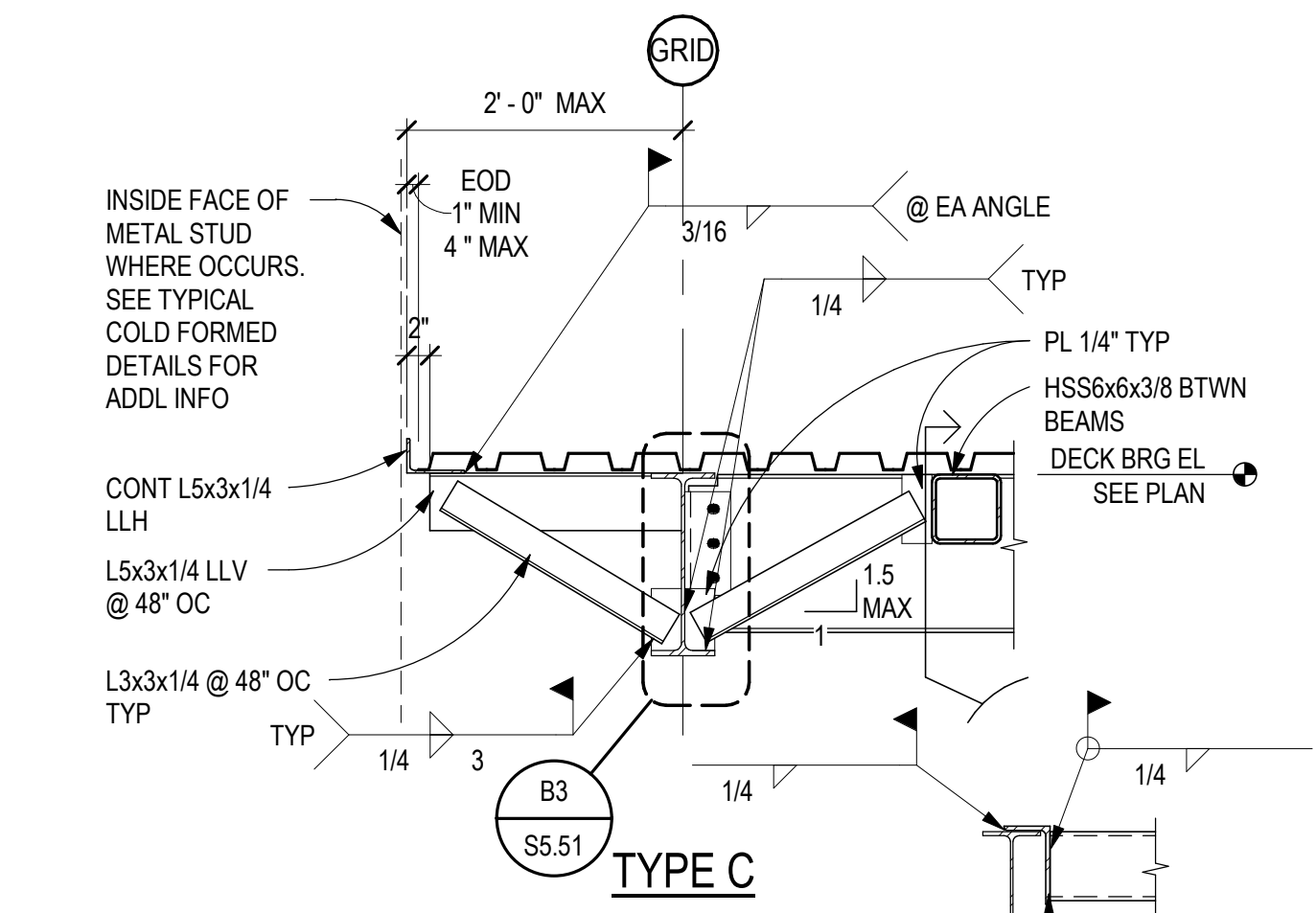
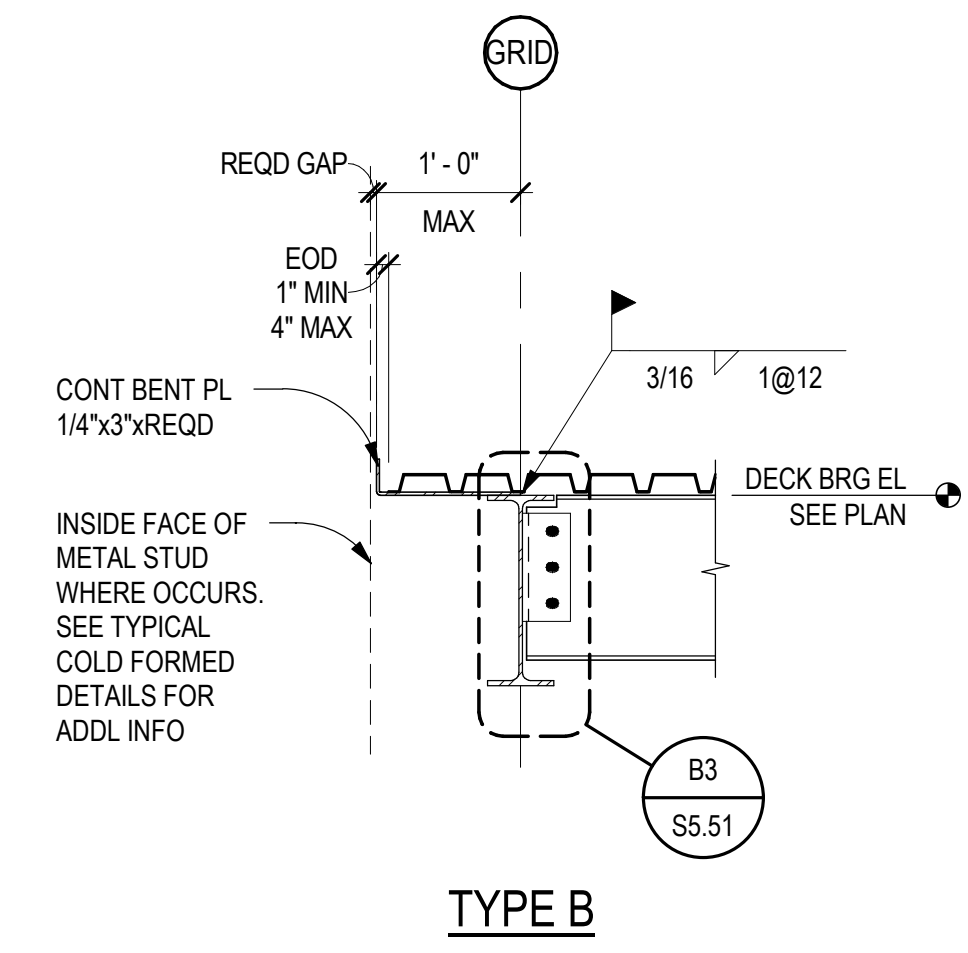
SHEET NUMBER: S5.54
STEEL DETAILS



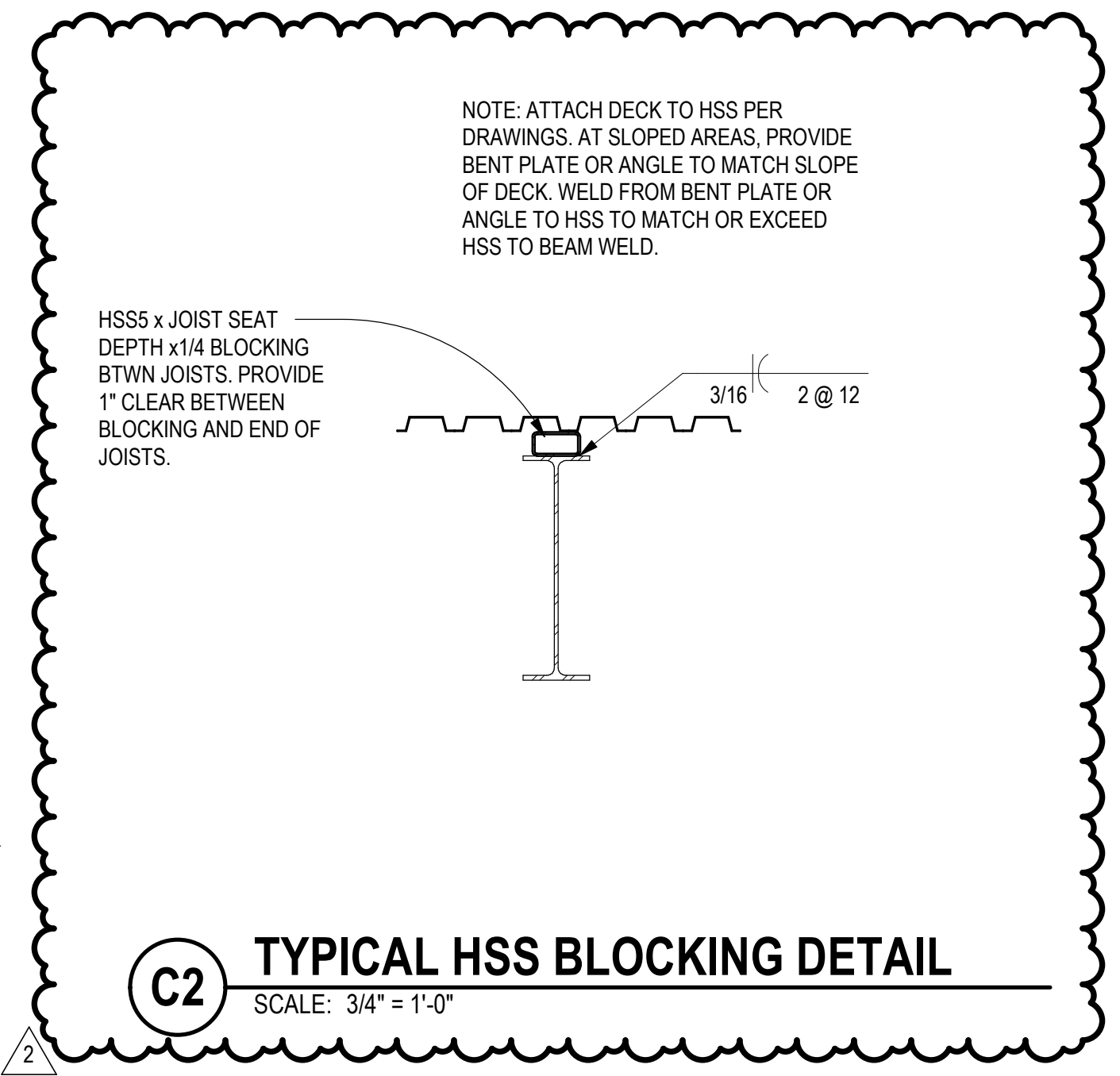
D1 TYPICAL FLOOR GIRDER
SCALE: 3/4" = 1'-0"



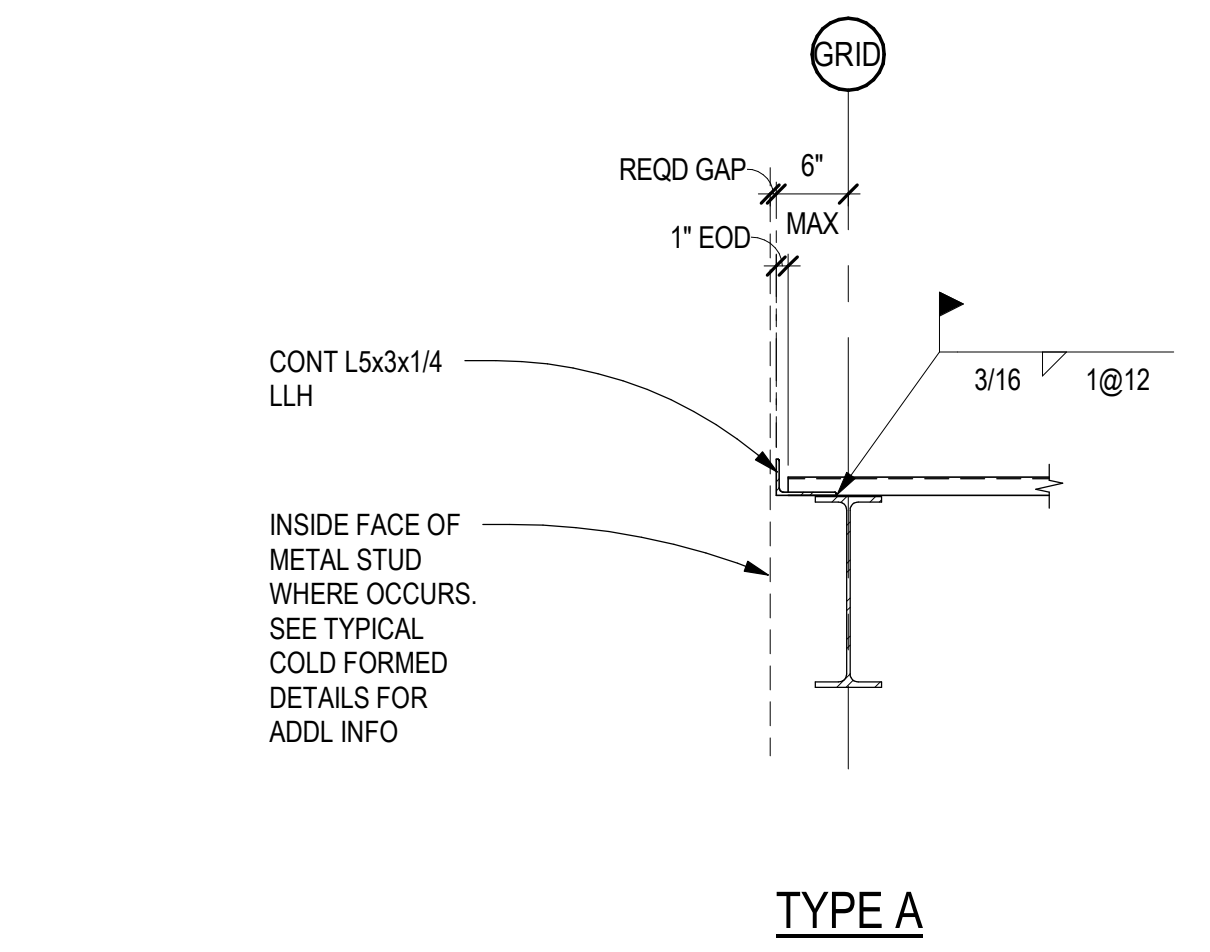
D3 TYPICAL DECK EDGE AT BEARING CONDITION
SCALE: 3/4" = 1'-0"



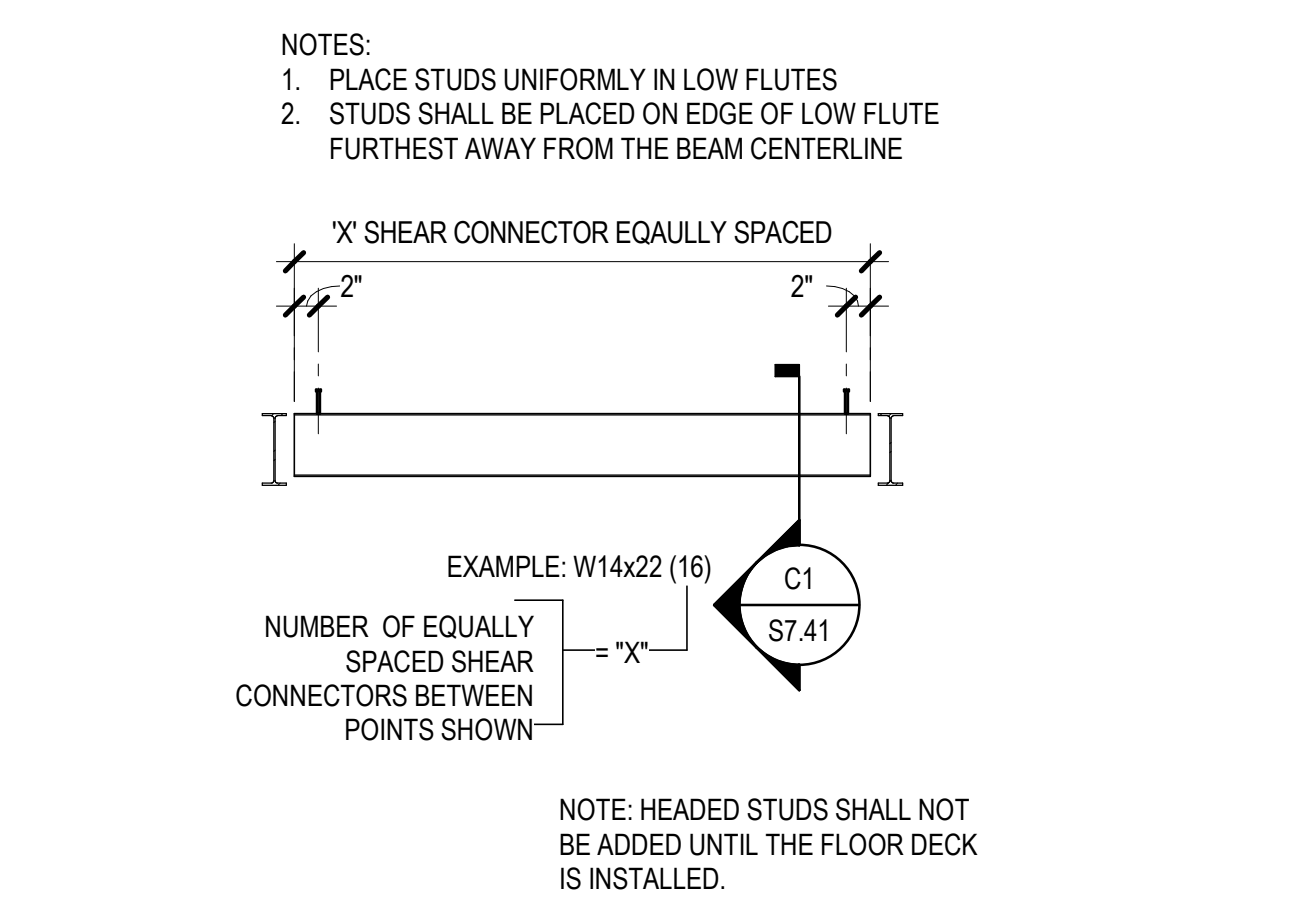
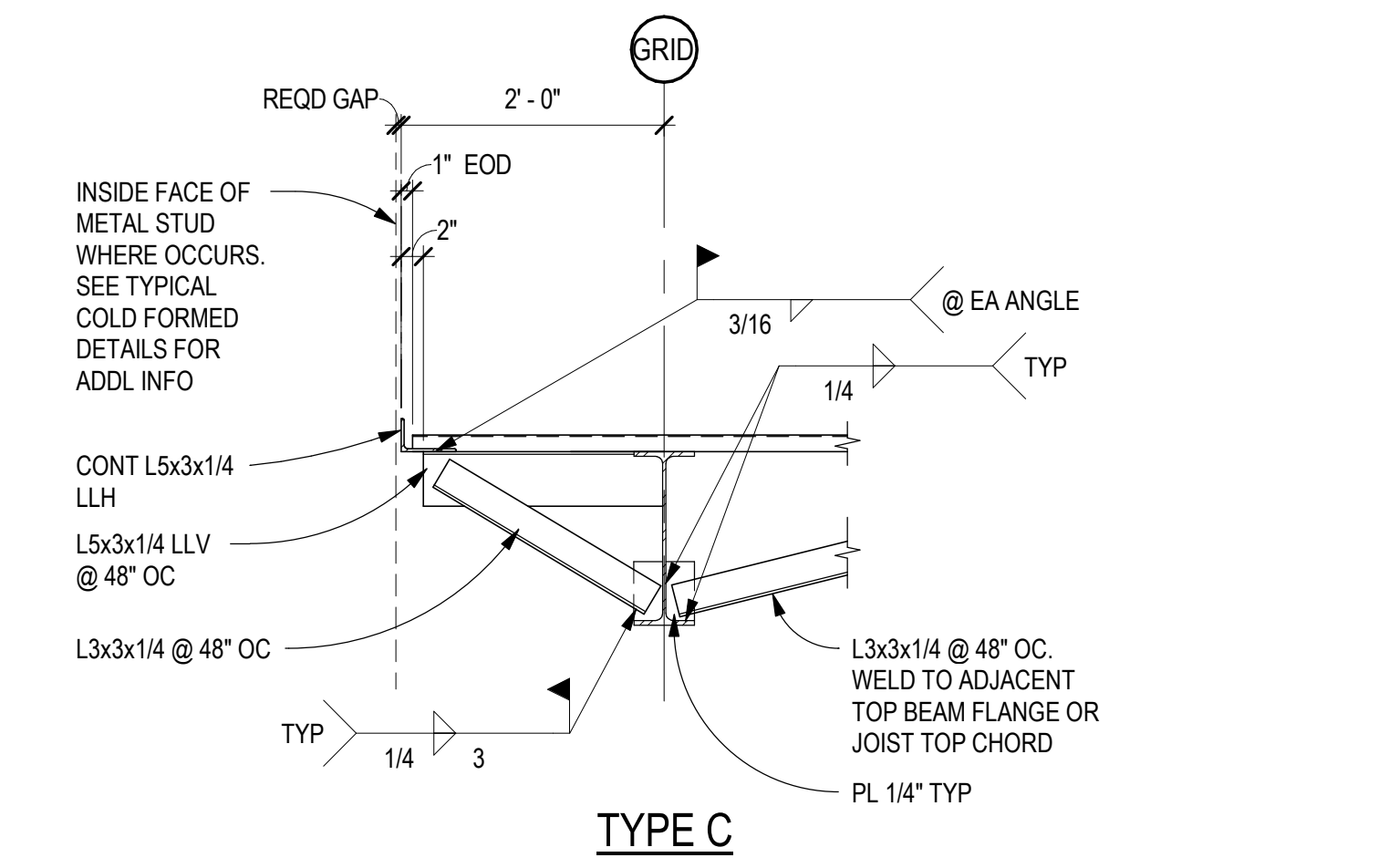
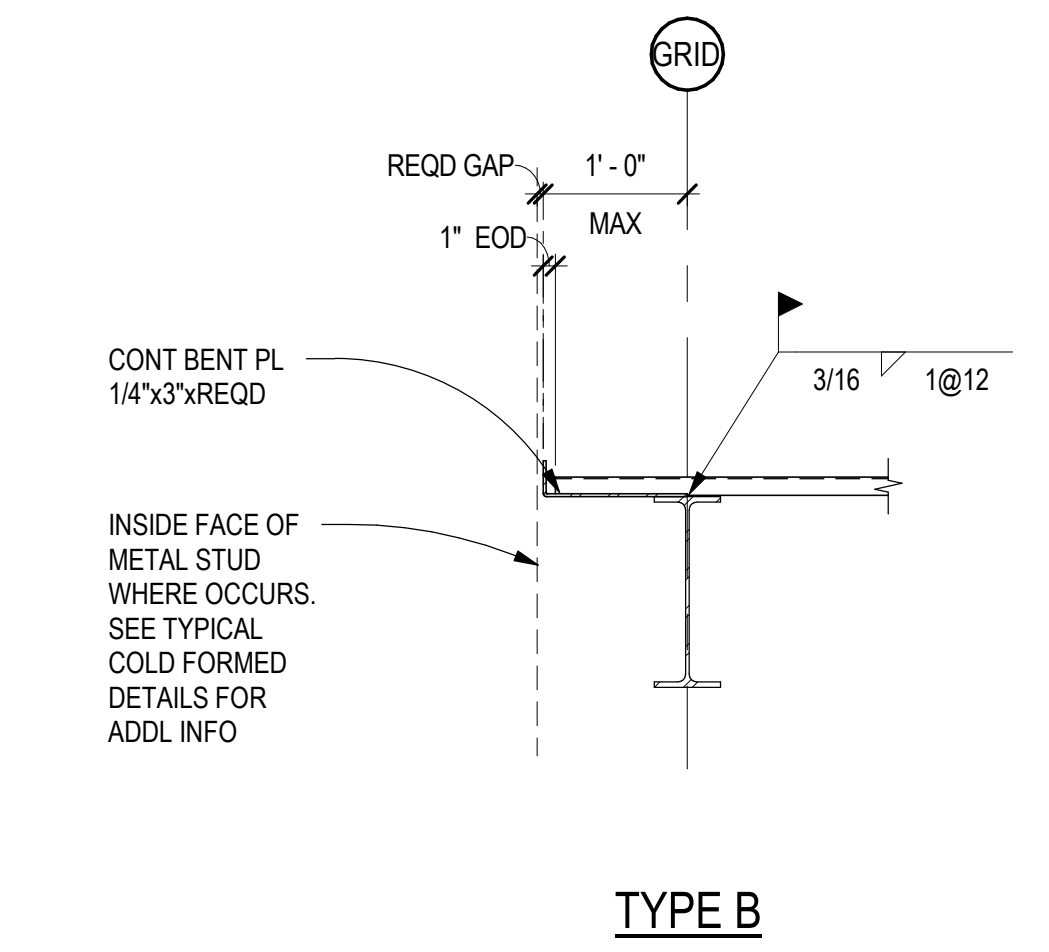
C1 TYPICAL COMPOSITE BEAM
SCALE: 1 1/2" = 1'-0"



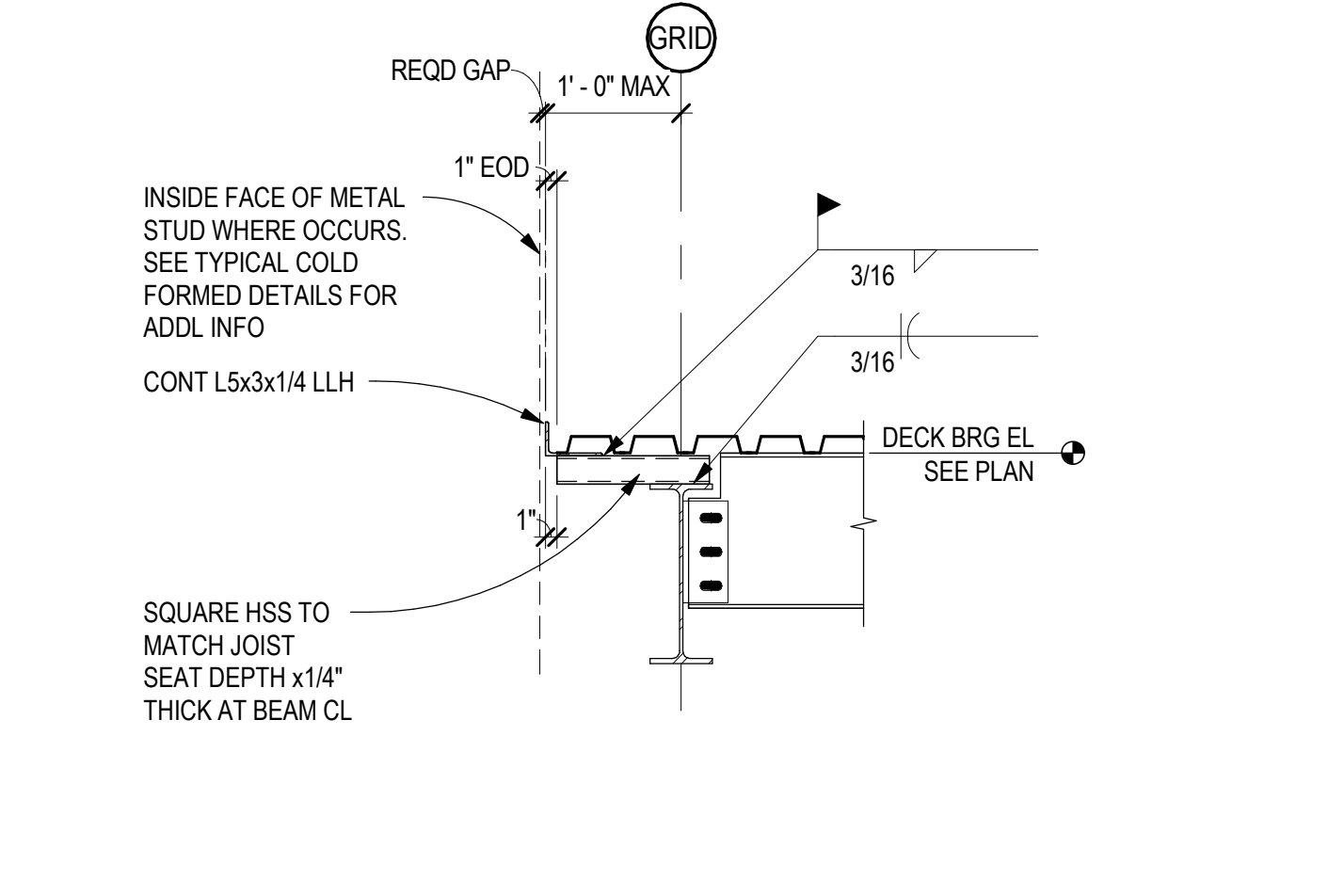
C2 TYPICAL HSS BLOCKING DETAIL
SCALE: 3/4" = 1'-0"



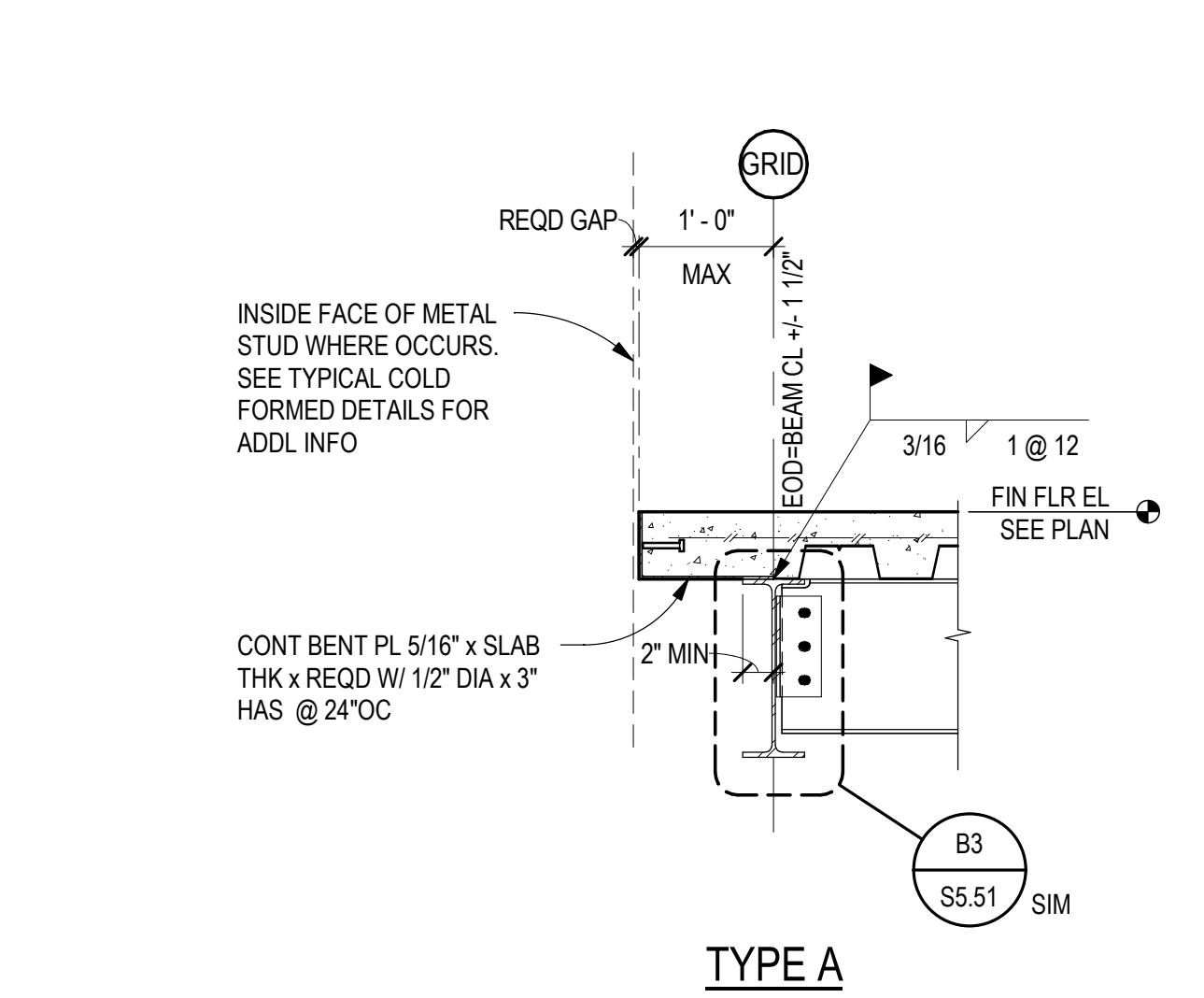
C3 TYPICAL DECK EDGE AT NON-BEARING CONDITION
SCALE: 3/4" = 1'-0"



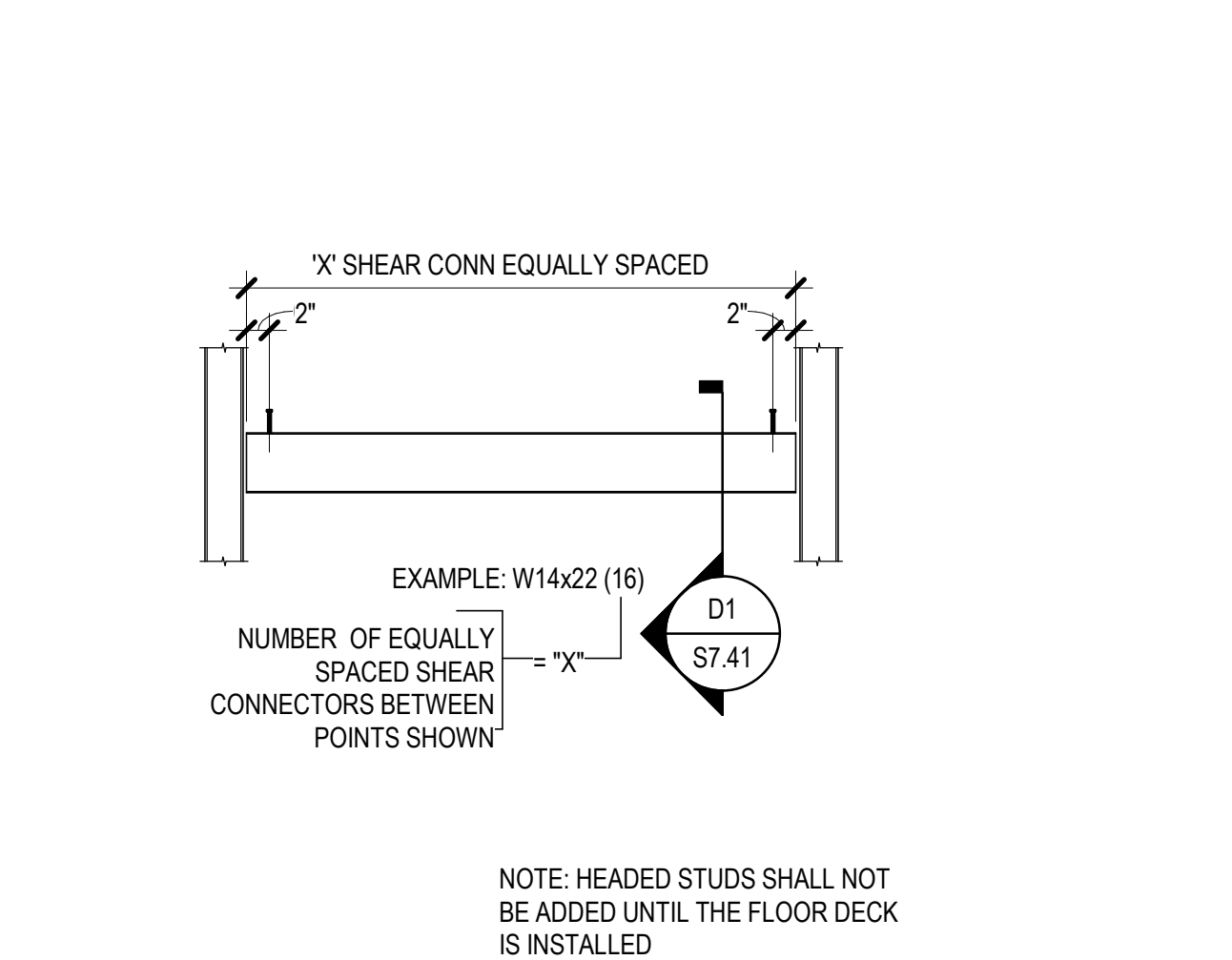
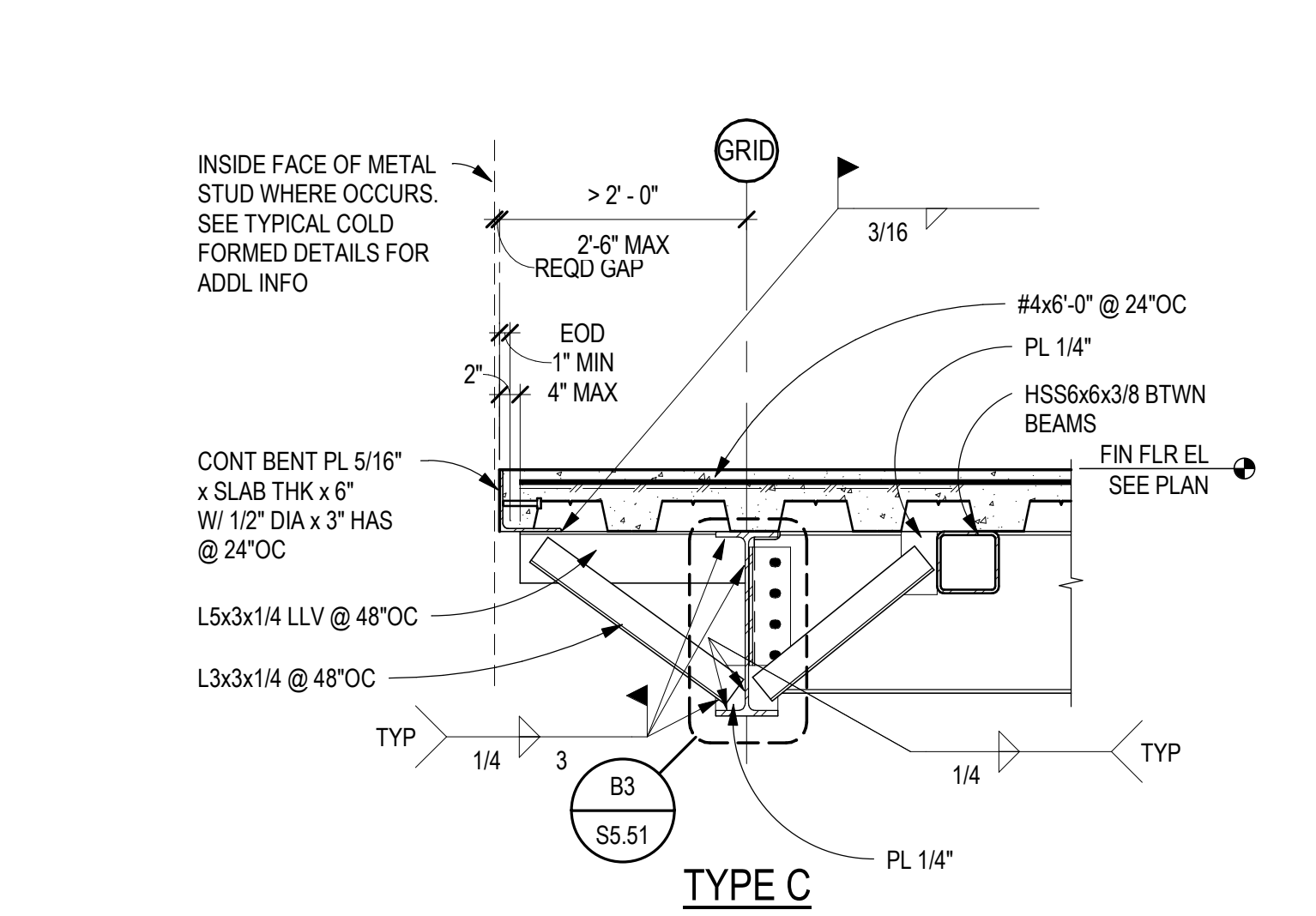
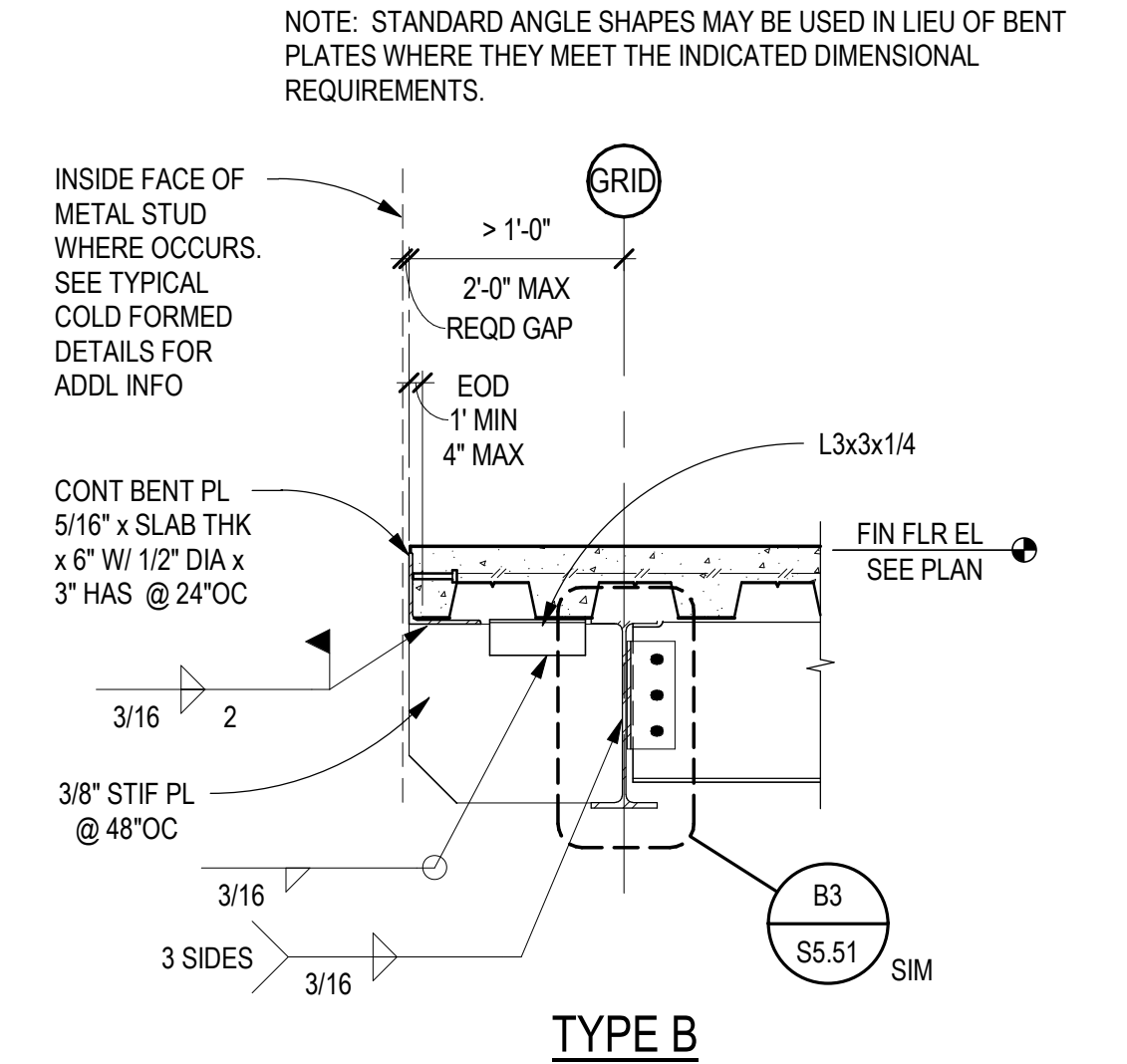
B1 TYPICAL COMPOSITE BEAM ELEV
SCALE: 1/4" = 1'-0"



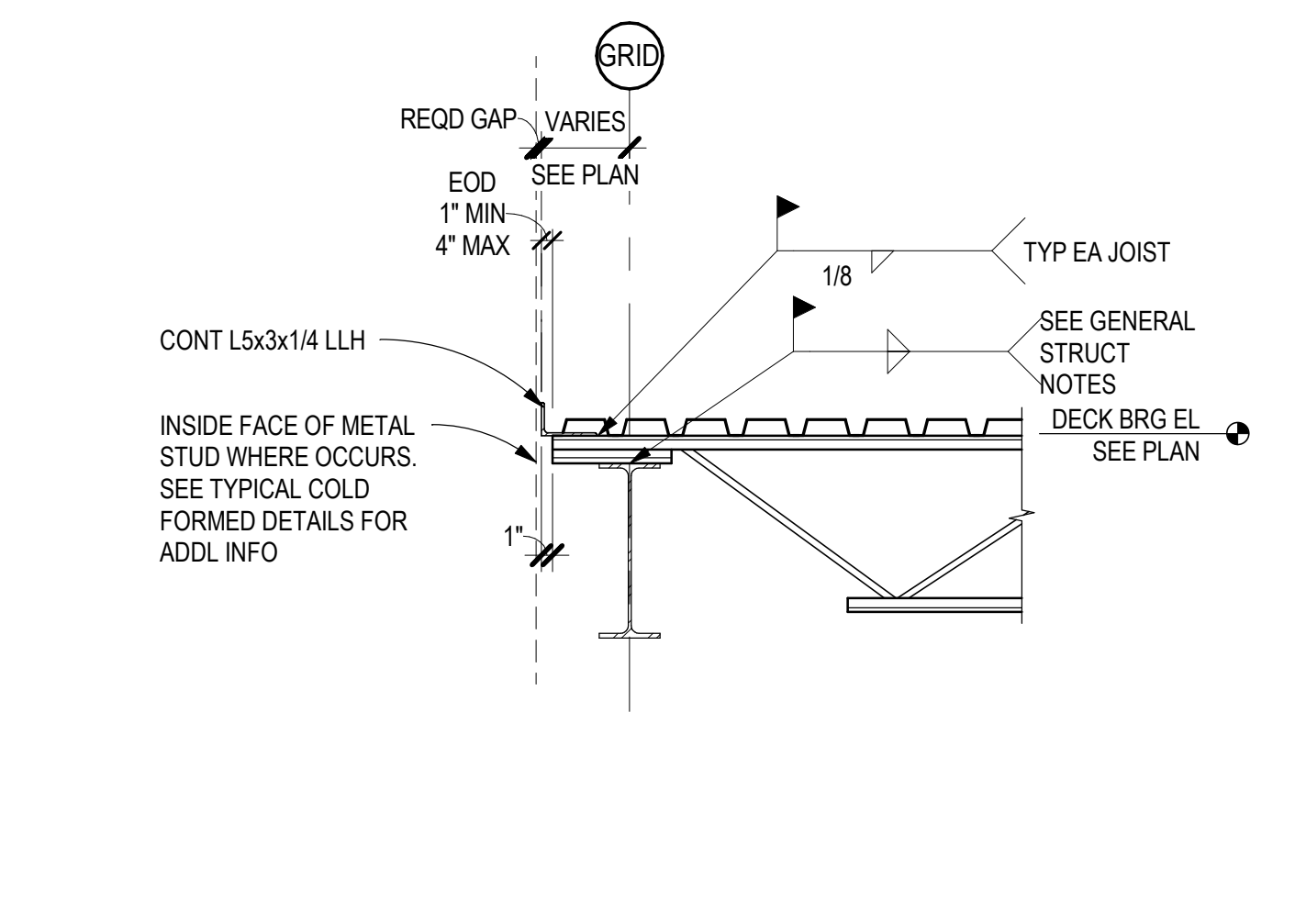
B2 TYPICAL DECK EDGE AT BEAM
SCALE: 3/4" = 1'-0"



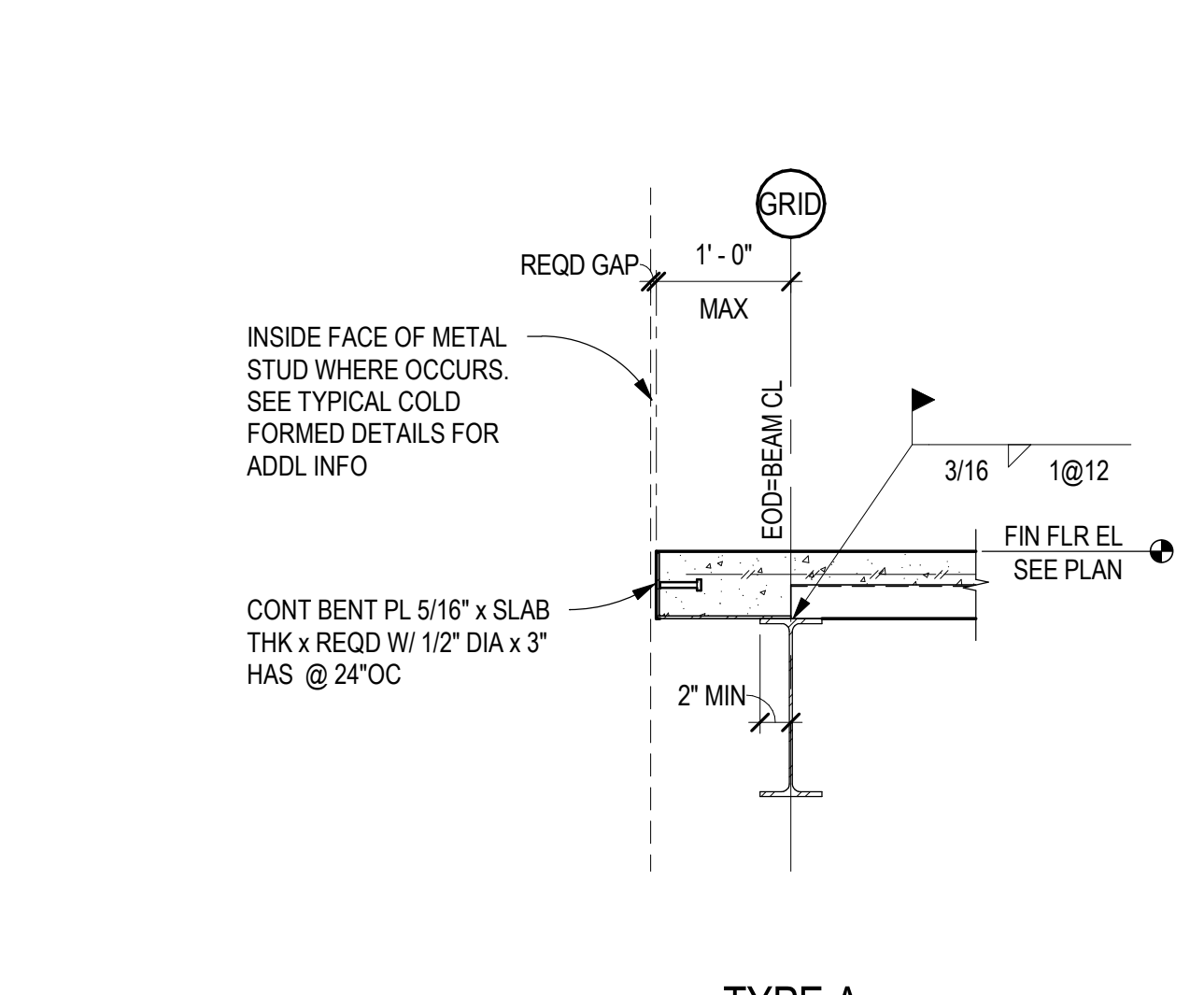
B3 TYPICAL SLAB EDGE AT BEARING CONDITION
SCALE: 3/4" = 1'-0"



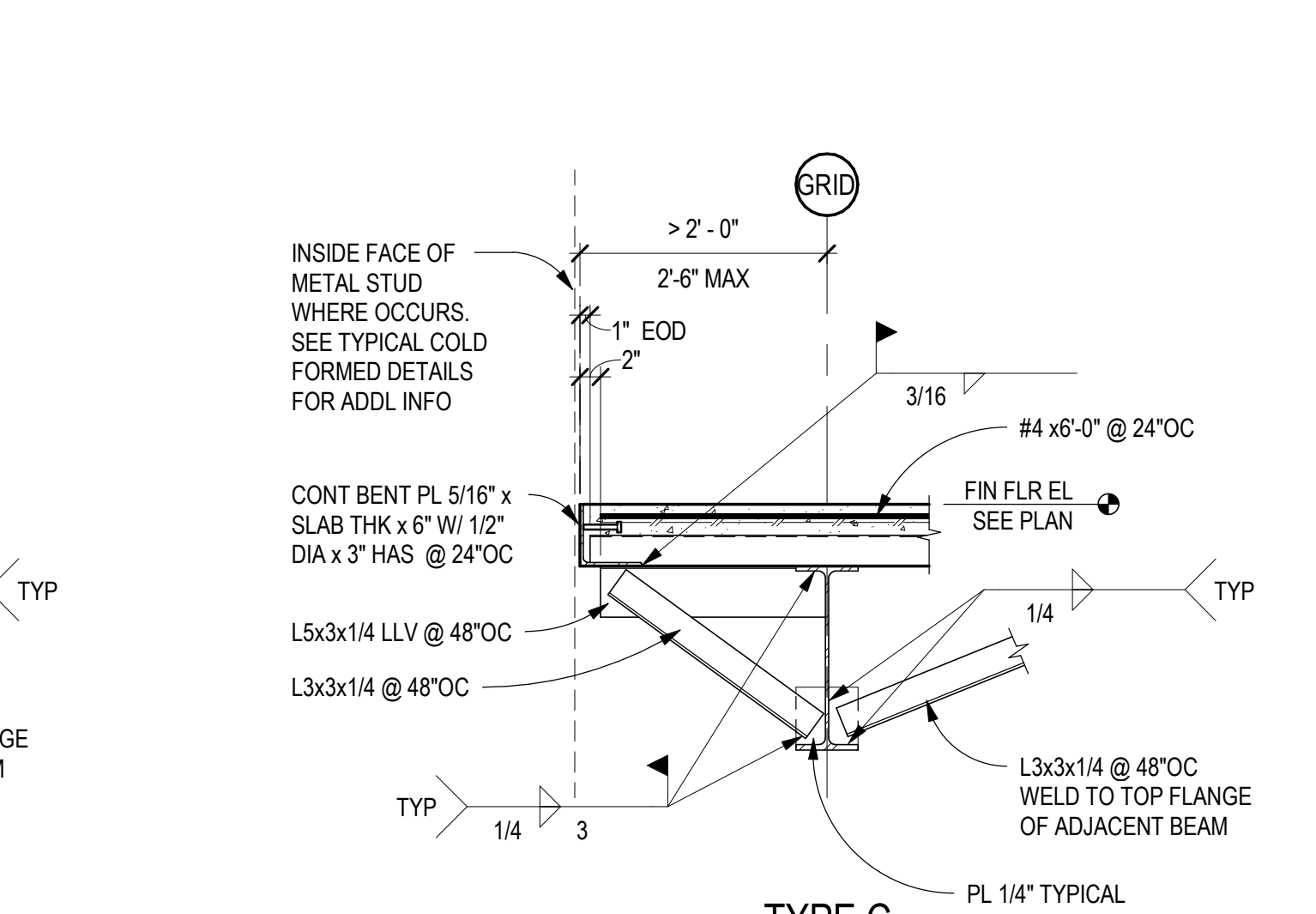
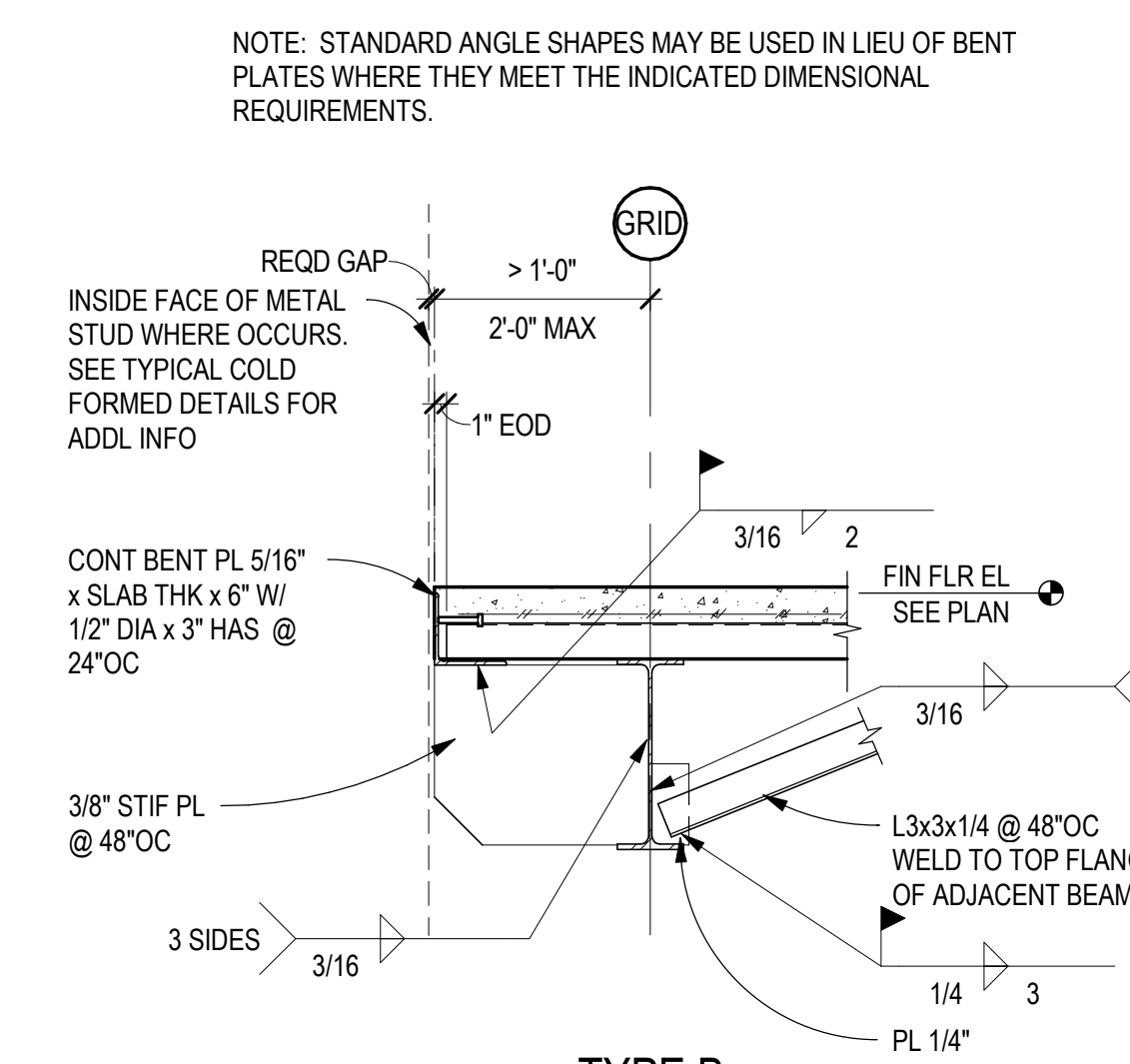
A1 TYPICAL COMPOSITE GIRDER ELEV
SCALE: 1/4" = 1'-0"



A2 TYPICAL ROOF BRG DECK EDGE
SCALE: 3/4" = 1'-0"



A3 TYPICAL SLAB EDGE AT NON-BEARING CONDITION
SCALE: 3/4" = 1'-0"





AIA® Document G710™ – 2017

Architect's Supplemental Instructions

PROJECT: *(name and address)*
Wilma P. Mankiller Health Center
Expansion
Stilwell, OK

CONTRACT INFORMATION:
Contract For: CMAR

Date:

ASI INFORMATION:
ASI Number: Bid Package 01 - ASI 003

Date: 04-17-20

OWNER: *(name and address)*
Cherokee Nation Property Management,
LLC.

ARCHITECT: *(name and address)*
James R. Childers Architect, Inc.
45 South 4th Street
Fort Smith, AR 72901

CONTRACTOR: *(name and address)*
M. Ross, Inc.

The Contractor shall carry out the Work in accordance with the following supplemental instructions without change in Contract Sum or Contract Time. Proceeding with the Work in accordance with these instructions indicates your acknowledgment that there will be no change in the Contract Sum or Contract Time.

(Insert a detailed description of the Architect's supplemental instructions and, if applicable, attach or reference specific exhibits.)

See attached Narrative for Structural Revisions generated and address Owner's third party review comments.

ISSUED BY THE ARCHITECT:

James R. Childers Architect, Inc. _____

ARCHITECT *(Firm name)*

J. Breck Childers _____

SIGNATURE

J. Breck Childers, Managing Principal _____

PRINTED NAME AND TITLE

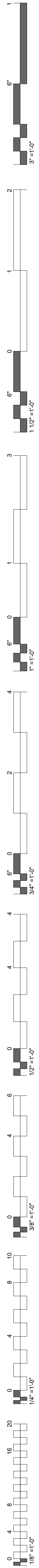
04-17-20 _____

DATE

Bid Package 01- ASI 03 – Wilma P. Mankiller Health Center Expansion

Chavez-Grieves would like to incorporate the following revisions into the drawings for the above referenced project.

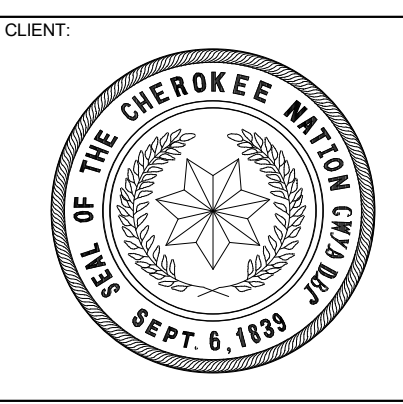
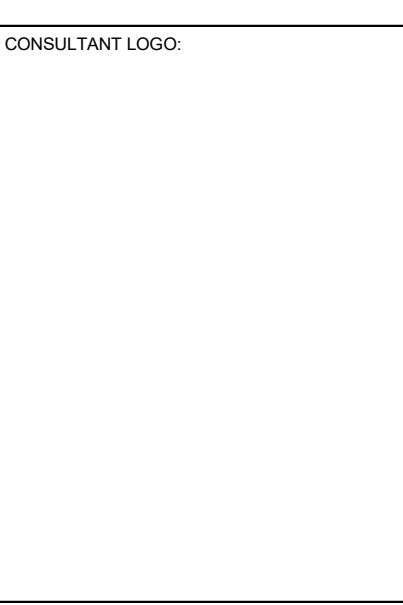
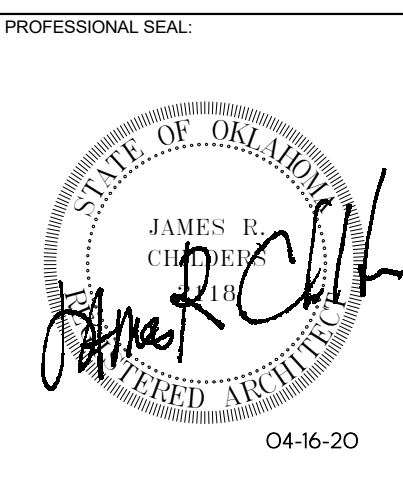
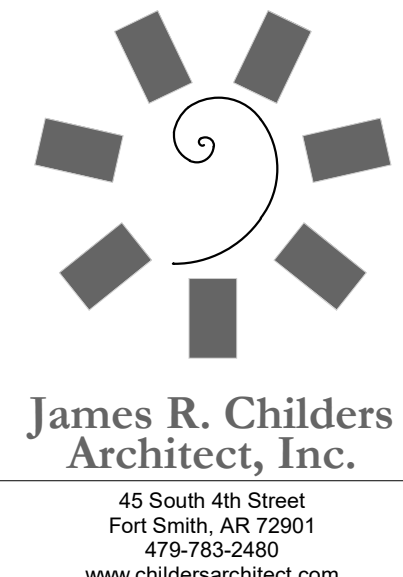
<u>Sheet</u>	<u>Description</u>
S1.01	Keynotes 18, 19, and 20 added.
S1.01	Footing steps added along grids 11 and H to accommodate plumbing lines.
S1.02	Keynotes 18, 19, and 20 added.
S1.02	Footing steps added along grids H, G, and 1 to accommodate plumbing lines.
S1.02	Spread footings dropped along grid 1 to not induce surcharge load on adjacent site retaining wall.
S1.02	Footings supporting precast concrete columns near grid 1 deleted and now supported by site retaining wall pedestal.
S1.02	Site retaining wall outline added for clarity.
S3.11	Sections C3 and D2 updated.
S3.12	Sections A5 and B5 updated.
S3.12	Sections A2, B2, C2, D2, C3, and D3 added.
S4.01	Keynotes 18, 19, and 20 added
S4.01	Spread footings at grid intersections P1/PB and P1/PC updated to not conflict with site retaining wall .
S6.01	Spot footing and continuous footing schedules updated.



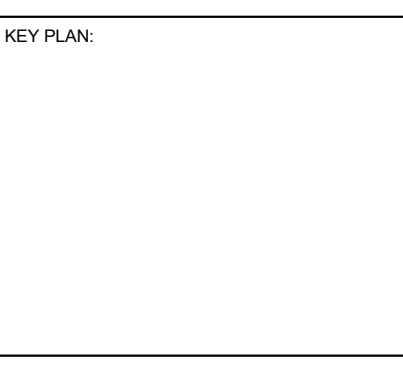
WILMA P. MANKILLER HEALTH CENTER EXPANSION

BID PACKAGE 01 (DEMOLITION / STEEL / FOUNDATIONS)

SHEET NUMBER	SHEET NAME	11-01-19 - BID PACKAGE 01	11-22-19 - BID PACKAGE 01 - ADDENDUM 01	12-10-19 - BID PACKAGE 01 - ADDENDUM 02	01-16-20 - BID PACKAGE 01 - ASB 01	02-10-20 - BID PACKAGE 01 - ASB 02	04-16-20 - BID PACKAGE 01 - ASB 03
GENERAL							
G0.01	COVER / INDEX						
CIVIL							
C002	GENERAL NOTES						
CS100	EXISTING SITE PLAN						
CS101	DEMOLITION PLAN						
CS102	DEMOLITION PLAN						
CE100	EROSION CONTROL SITE PLAN						
CE500	EROSION CONTROL DETAILS						
ARCHITECTURAL							
A0.01	OVERALL BUILDING DEMOLITION PLAN						
STRUCTURAL							
S0.01	ABBREVIATIONS AND LEGENDS						
S0.02	GENERAL STRUCTURAL NOTES						
S0.03	GENERAL STRUCTURAL NOTES AND SPECIAL INSPECTIONS						
SD0.01	DEMOLITION GENERAL STRUCTURAL NOTES						
SD1.01	DEMOLITION PLANS - SECTOR 1						
SD2.01	DEMOLITION SECTIONS						
S1.00	OVERALL PLAN - FOUNDATION						
S1.01	FOUNDATION PLAN SECTOR 1						
S1.02	FOUNDATION PLAN SECTOR 2						
S1.10	OVERALL PLAN - FLOOR FRAMING						
S1.11	FLOOR FRAMING PLAN - SECTOR 1						
S1.12	FLOOR FRAMING PLAN - SECTOR 2						
S1.13	LOW ROOF FRAMING PLAN						
S1.20	OVERALL PLAN - ROOF FRAMING						
S1.21	ROOF FRAMING PLAN - SECTOR 1						
S1.22	ROOF FRAMING PLAN - SECTOR 2						
S2.01	MOMENT FRAME AND BRACED FRAME ELEVATIONS						
S3.01	WALL SECTIONS						
S3.02	WALL SECTIONS						
S3.03	WALL SECTIONS						
S3.04	WALL SECTIONS						
S3.11	FOUNDATION SECTIONS						
S3.12	FOUNDATION SECTIONS						
S3.21	FLOOR FRAMING SECTIONS						
S3.31	ROOF FRAMING SECTIONS						
S4.01	ENLARGED PLANS						
S5.21	MASONRY FRAMING SECTIONS AND DETAILS						
S6.41	VERTICAL CIRCULATION DETAILS						
S5.51	STEEL DETAILS						
S5.52	STEEL DETAILS						
S5.53	STEEL DETAILS						
S5.54	STEEL DETAILS						
S6.01	SCHEDULES						
S7.11	TYPICAL CONCRETE DETAILS						
S7.21	TYPICAL MASONRY DETAILS						
S7.31	TYPICAL COLD-FORMED DETAILS						
S7.41	TYPICAL STEEL DETAILS						
S7.42	TYPICAL STEEL DETAILS						
S8.01	SIDEPLATE GENERAL NOTES AND CONSTRUCTION GUIDELINES						
S8.02	SIDEPLATE COLUMN DETAILS, A TYPE						
S8.03	SIDEPLATE COLUMN DETAILS, B TYPE						
S8.04	SIDEPLATE BEAM DETAILS						
S8.05	SIDEPLATE BEAM DETAILS, NARROW						
S8.06	SIDEPLATE FIELD ERECTION DETAILS						
S8.07	SIDEPLATE COORDINATION ITEMS						
S8.08	SIDEPLATE MISCELLANEOUS DETAILS						
ELECTRICAL							
E0.01	ELECTRICAL DEMOLITION PLAN						
Grand total: 66							



**WILMA P. MANKILLER HEALTH CENTER
EXPANSION**
 STILWELL, OKLAHOMA



PROJECT PHASE:
BID PACKAGE 01

#	DATE	REVISIONS
1	11/22/19	BID PACKAGE 01 - ADD 01
2	12/10/19	BID PACKAGE 01 - ADD 02
3	1/16/20	BID PACKAGE 01 - ASB 01
4	2/5/20	BID PACKAGE 01 - ASB 02
5	4/16/20	BID PACKAGE 01 - ASB 03

DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER:
G0.01

COVER / INDEX



1836 SOUTH BALTIMORE AVE.
TULSA, OK 74119
(539) 664-4618

MECHANICAL / ELECTRICAL / PLUMBING ENGINEER



3802 UNIVERSITY BOULEVARD
DURANT, OK 74701
(580) 931-9045

CIVIL ENGINEER



4700 LINCOLN ROAD NE, SUITE 102
ALBUQUERQUE, NM 87109
(505) 344-4080

STRUCTURAL ENGINEER



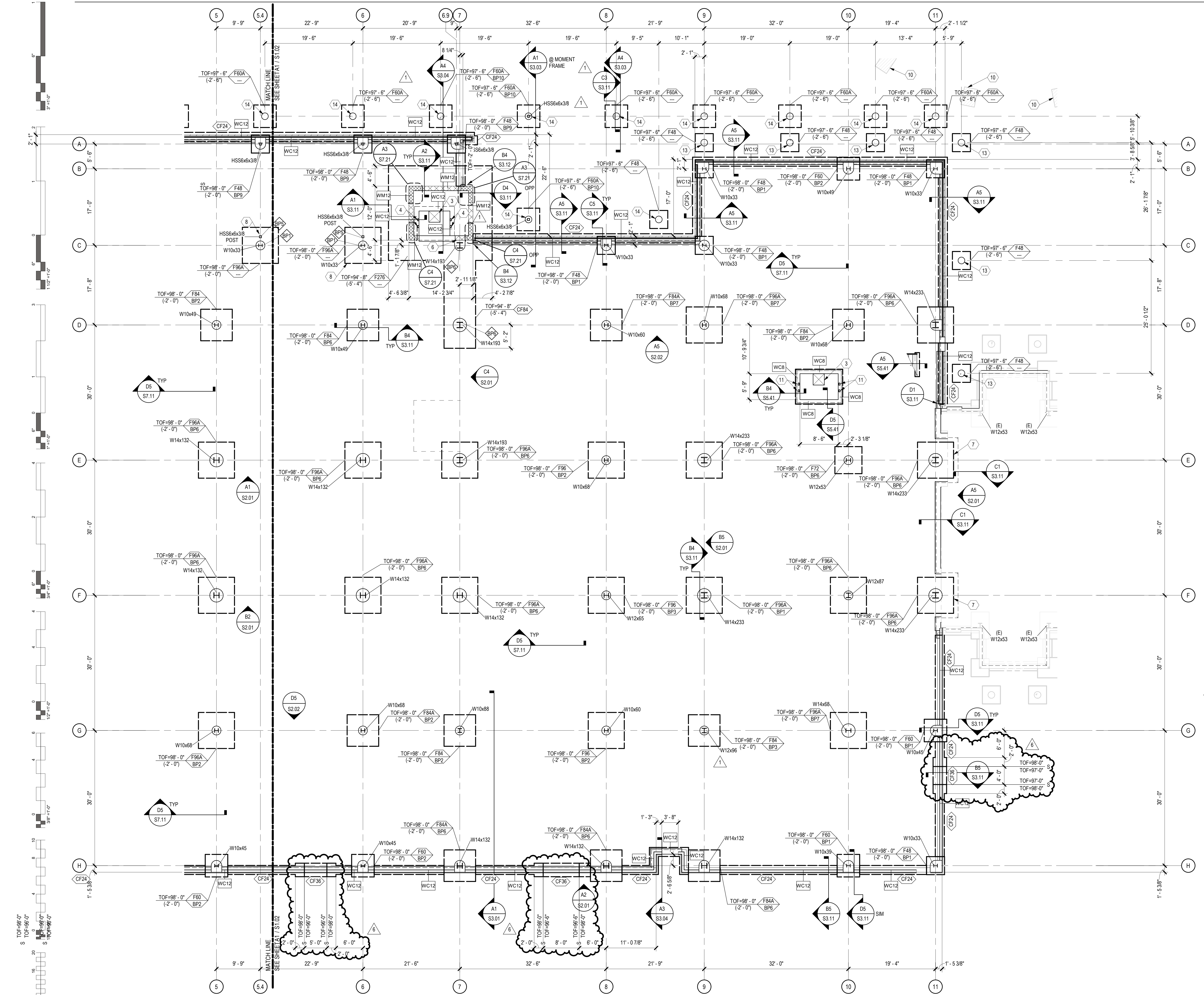
808 TRAVIS STREET, SUITE 200
HOUSTON, TX 77002
(281) 589-5900

FIRE PROTECTION / LIFE SAFETY



1316 E 35TH PLACE, SUITE 100
TULSA, OK 74105
(918) 382-9120

EQUIPMENT PLANNER

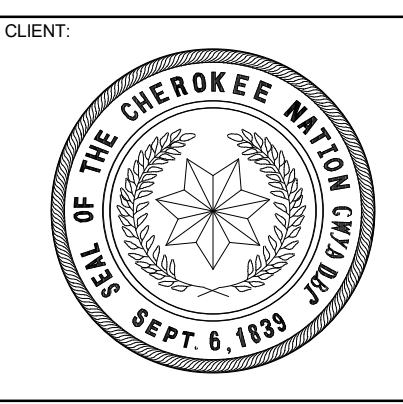
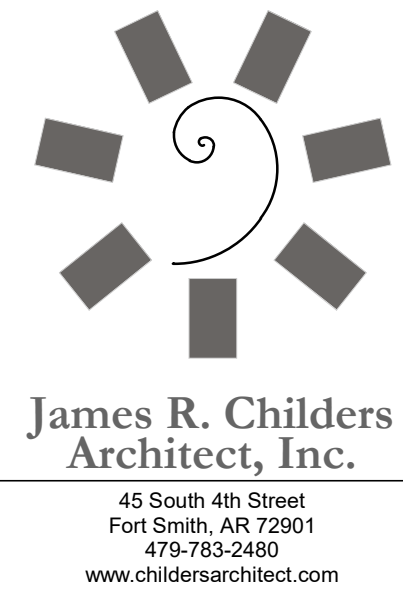


GENERAL SHEET NOTES

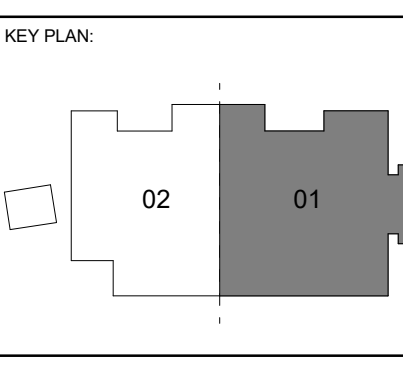
- SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- REFERENCE FINISH FLOOR ELEVATION 100'-0" = MEAN SEA FINISH FLOOR ELEVATION. SEE CIVIL DRAWINGS.
- TOP OF FOOTING ELEVATION = 98'-0" (-2'-0"), UNLESS NOTED OTHERWISE ON PLAN.
- NOTE TO CONTRACTOR: ENLARGED SLAB BLOCKOUTS MAY BE REQUIRED AT FRAME COLUMNS FOR MOMENT FRAME BASE PLATE CLEARANCE.
- NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
- DIMENSIONS ARE TO THE FACE OF STUD UNLESS NOTED OTHERWISE.
- SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
- EXISTING CONSTRUCTION IS PER AVAILABLE EXISTING DRAWINGS. ALL EXISTING CONSTRUCTION AND DIMENSIONS SHALL BE VERIFIED PRIOR TO CONSTRUCTION. SHOULD CONDITIONS VARY FROM THOSE SHOWN, CONTACT ENGINEER BEFORE PROCEEDING.
- PROVIDE SLAB JOINTS AT 10'-0" ON CENTER MAXIMUM. THE AREA OF THE CONTROL JOINT SHALL NOT EXCEED A 2:1 RATIO. CONTROL JOINTS SHALL BE LOCATED AT COLUMN LINES WHERE THE LAYOUT PERMITS. AT RE-ENTRANT CORNERS THAT DO NOT HAVE CONTROL JOINTS, PROVIDE 2#4 x 3'-0" DIAGONAL TO THE RE-ENTRANT CORNER.
- STRUCTURAL COLD FORMED METAL STUDS SHALL BE 6" WIDE UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING BY OTHERS.
- SEE SHEET S7.00 SERIES SHEETS FOR TYPICAL FOUNDATION SECTIONS AND DETAILS.
- SEE SHEET S6.01 FOR SCHEDULES.

SHEET KEYNOTE

- FLOOR DRAIN: SLOPE SLAB TO DRAIN 1/8" PER FOOT. COORDINATE EXACT SIZE AND LOCATION WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
- POST AND FOUNDATION AS REQUIRED FOR STAIR SUPPORT. STAIR ENGINEER TO PROVIDE REQUIRED LOADS AND LOCATIONS.
- ELEVATOR SUMP PIT. COORDINATE EXACT SIZE AND LOCATION WITH ELEVATOR MANUFACTURER. SEE A4 / S5.41
- HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION AND SPACING WITH ELEVATOR MANUFACTURER. SEE B4 / S5.41
- PRE-ENGINEERED METAL BUILDING STEEL AND ANCHORAGE BY OTHERS. CONTRACTOR TO CONFIRM LOCATIONS OF FOUNDATIONS WITH FINAL PRE-ENGINEERED METAL BUILDING SHOP DRAWINGS.
- NOTCH MASONRY AS REQUIRED TO FACILITATE BASEPLATE INSTALLATION. STEP BOND BEAM AT THIS LOCATION. FILL VOID FROM NOTCH WITH NON-SHRINK GROUT.
- CUT AND REMOVE EXISTING SLAB AS REQUIRED TO PLACE NEW FOOTING. NEW SLAB TO POUR UP TO REMAINING SLAB.
- CENTER FOOTING ON GRID P.C.
- F60A PRE-MANUFACTURED SUNSHADE CONCRETE FOOTING. TOP OF FOOTING = 98'-0" (-2'-0"). SEE SHEET S6.01 FOR FOOTING SCHEDULE. COORDINATE FINAL LOCATION WITH SUNSHADE MANUFACTURER.
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- HSS6x1/2 ELEVATOR SUPPORT POST. COORDINATE EXACT LOCATION AND SPACING WITH ELEVATOR MANUFACTURER. SEE B4 / S5.41, D3 / S5.41, A2 / S5.41, B2 / S5.41, AND C2 / S5.41
- 1 1/2" RECESSED SLAB AT ADA SHOWER. COORDINATE EXACT SIZE, LOCATION, AND SLOPE REQUIREMENTS WITH ARCHITECTURAL DRAWINGS. SEE C4 / S7.11
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- 18" DIAMETER PRECAST CONCRETE CANOPY COLUMN BY OTHERS. SEE C3 / S3.11, C4 / S3.12, A1 / S3.31, AND A5 / S3.31
- F60A PRE-MANUFACTURED SUNSHADE CONCRETE FOOTING WITH 18" SQUARE CONCRETE PEDESTAL. TOP OF FOOTING ELEVATION TO MATCH TOP OF FOOTING ELEVATION OF SITE RETAINING WALL. SEE SHEET S6.01 FOR FOOTING SCHEDULE. SEE B3 / S3.12 FOR PEDESTAL DETAIL. ANCHORAGE AND SUPPLEMENTAL ANCHOR REINFORCEMENT FOR PRE-MANUFACTURED CANOPY TO BE PROVIDED BY MANUFACTURER.
- SITE RETAINING WALL. SEE D2 / S3.11
- DOCK LEVELER. SEE ARCHITECTURAL FOR EXACT LOCATION AND DIMENSIONS.
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- SEE C3 / S3.12 FOR SUPPORT OF CONTINUOUS FOOTING.
- SITE RETAINING WALL. SEE B2 / S3.12. SEE CIVIL DRAWING FOR EXTENT.



**WILMA P. MANKILLER HEALTH CENTER
EXPANSION**
 STILLWELL, OKLAHOMA



PROJECT PHASE:
BID PACKAGE 01

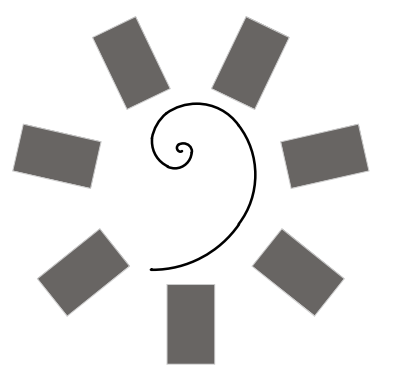
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2	04/16/20	BID PACKAGE 01 - ASB 03	

DATE: 11/01/19 JOB NUMBER: 18-01.01

SHEET NUMBER: S1.01

FOUNDATION PLAN
SECTOR 1

A1 FOUNDATION PLAN - SECTOR 1
SCALE: 1/8" = 1'-0"



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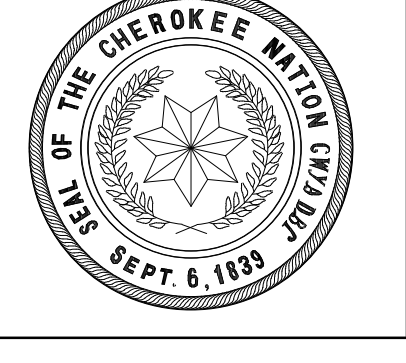
PROFESSIONAL SEAL



CONSULTANT LOGO

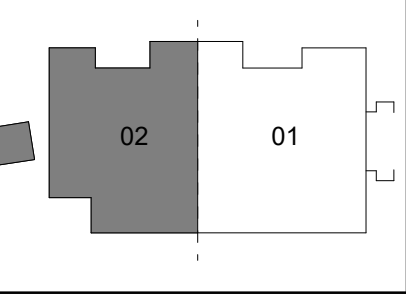


CLEAR



WILMA P. MANKILLER HEALTH CENTER
EXPANSION
STILWELL, OKLAHOMA

KEY PLAN



PROJECT PHASE

BID PACKAGE 01

#	DATE	REVISIONS	DESCRIPTION
1	11/02/19	BID PACKAGE 01 - ADD 01	
2	12/19/19	BID PACKAGE 01 - ADD 02	
3	04/16/20	BID PACKAGE 01 - ADD 03	

DATE: 11/01/19

JOB NUMBER: 18-01.01

SHEET NUMBER:

S1.02

FOUNDATION PLAN

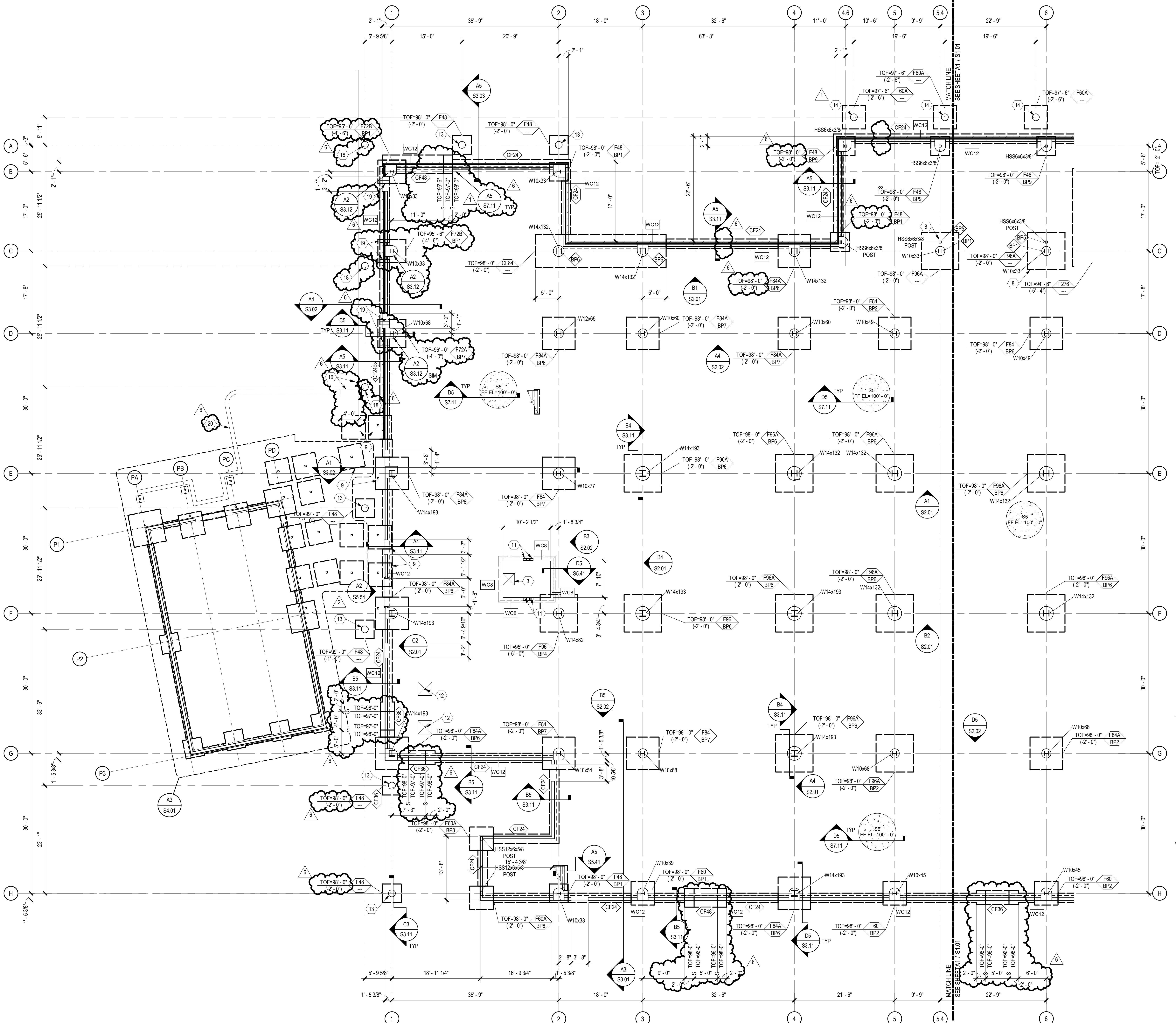
SECTOR 2

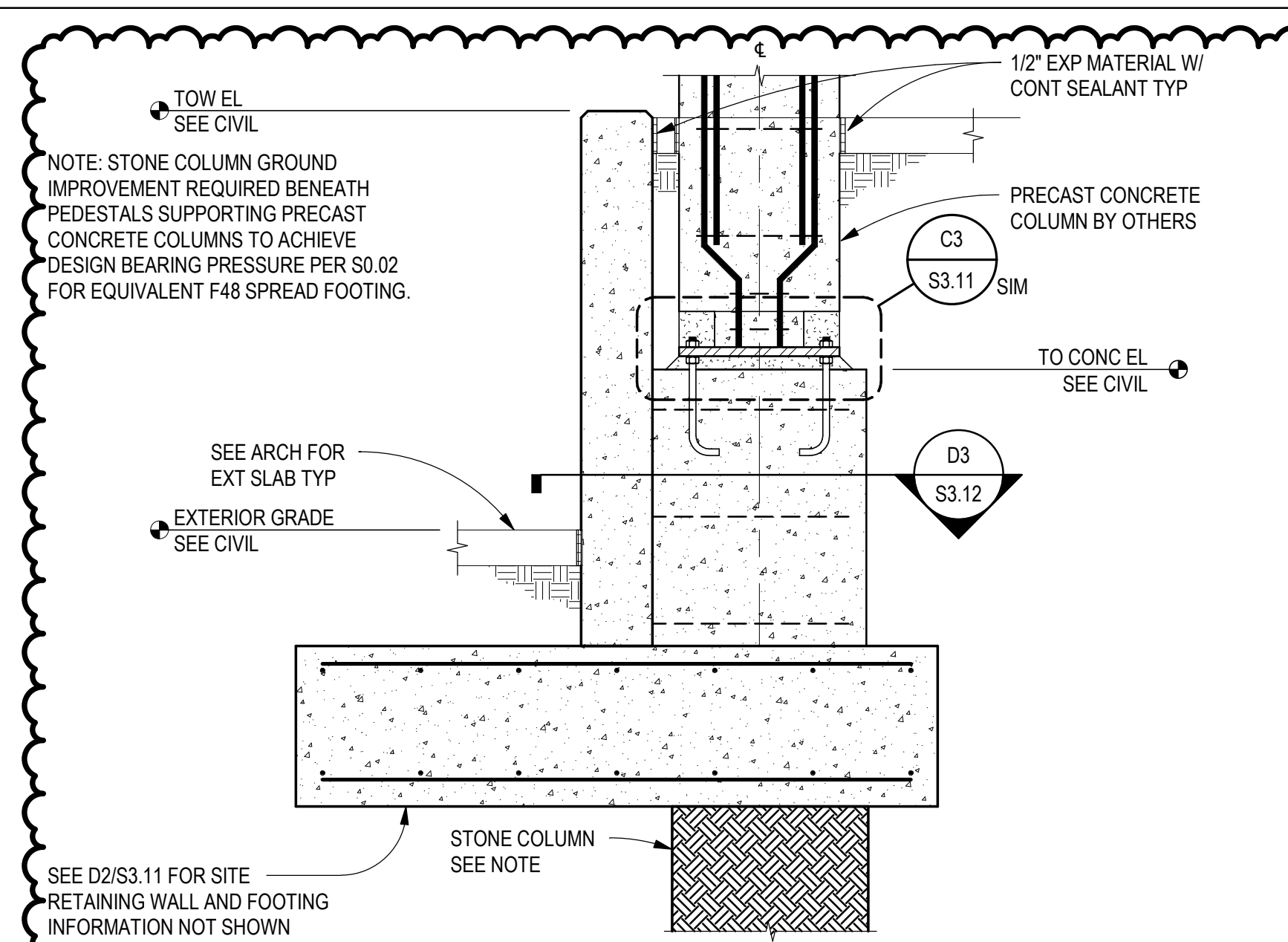
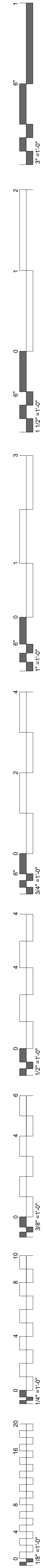
GENERAL SHEET NOTES

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- SEE SHEET S6.01 FOR SCHEDULES.

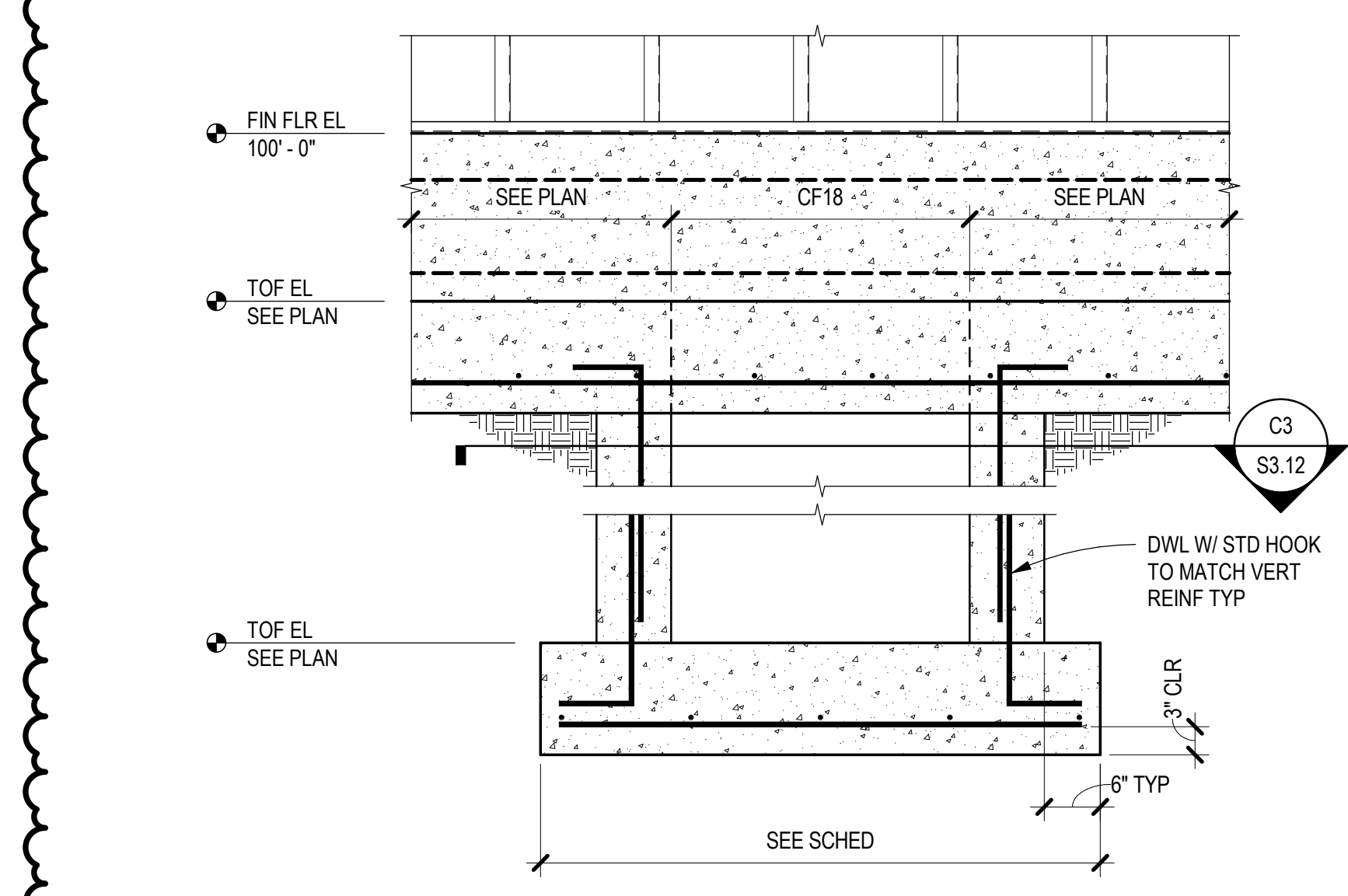
SHEET KEYNOTE

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- HSS6x6x3/8 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION AND SPACING WITH ELEVATOR MANUFACTURER. SEE B4 / S5.41
- PRE-ENGINEERED METAL BUILDING STEEL AND ANCHORAGE BY OTHERS. CONTRACTOR TO CONFIRM LOCATIONS OF FOUNDATIONS WITH FINAL PRE-ENGINEERED METAL BUILDING SHOP DRAWINGS.
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- SITE RETAINING WALL. SEE B2 / S3.12 SEE CIVIL DRAWINGS FOR EXTENT.

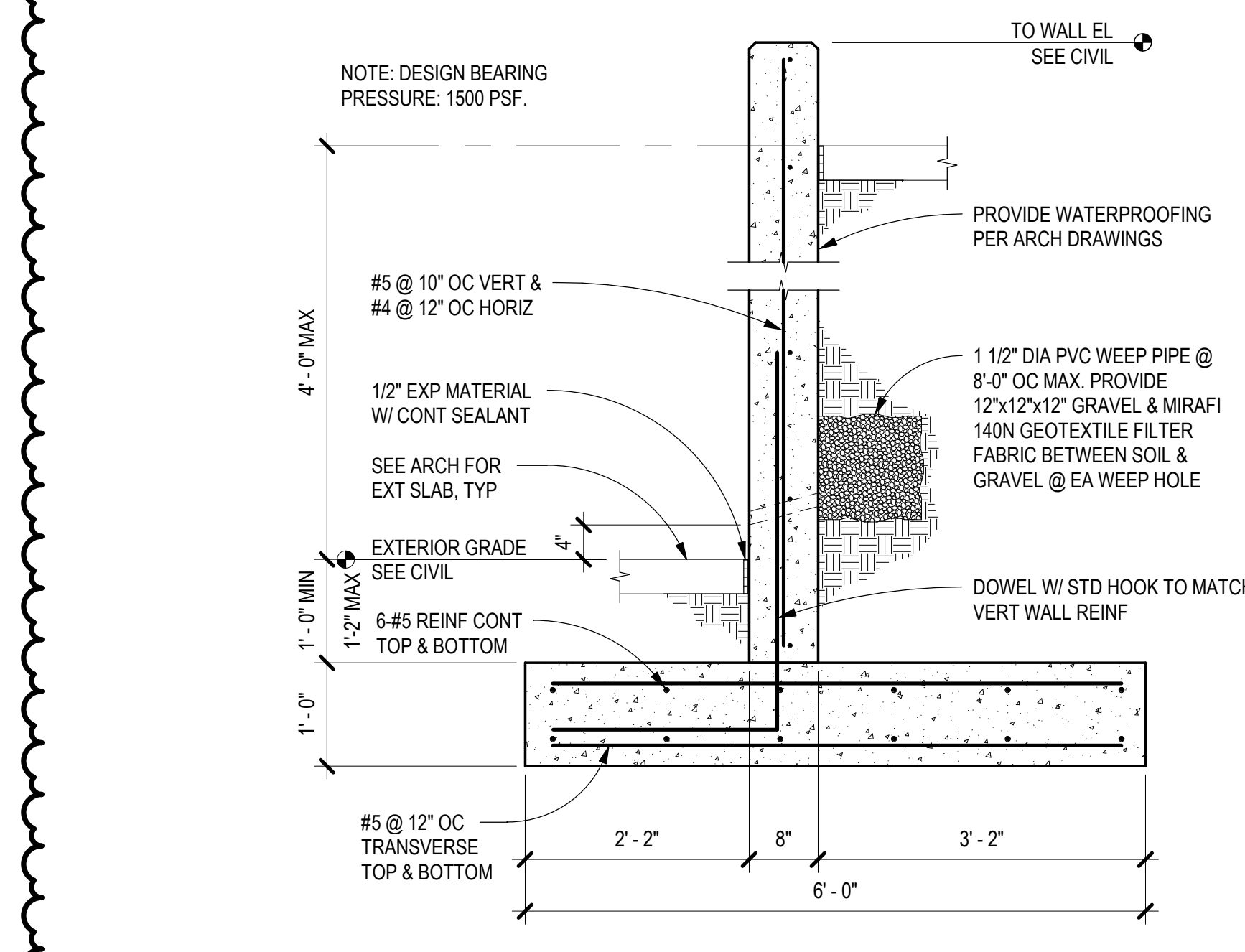




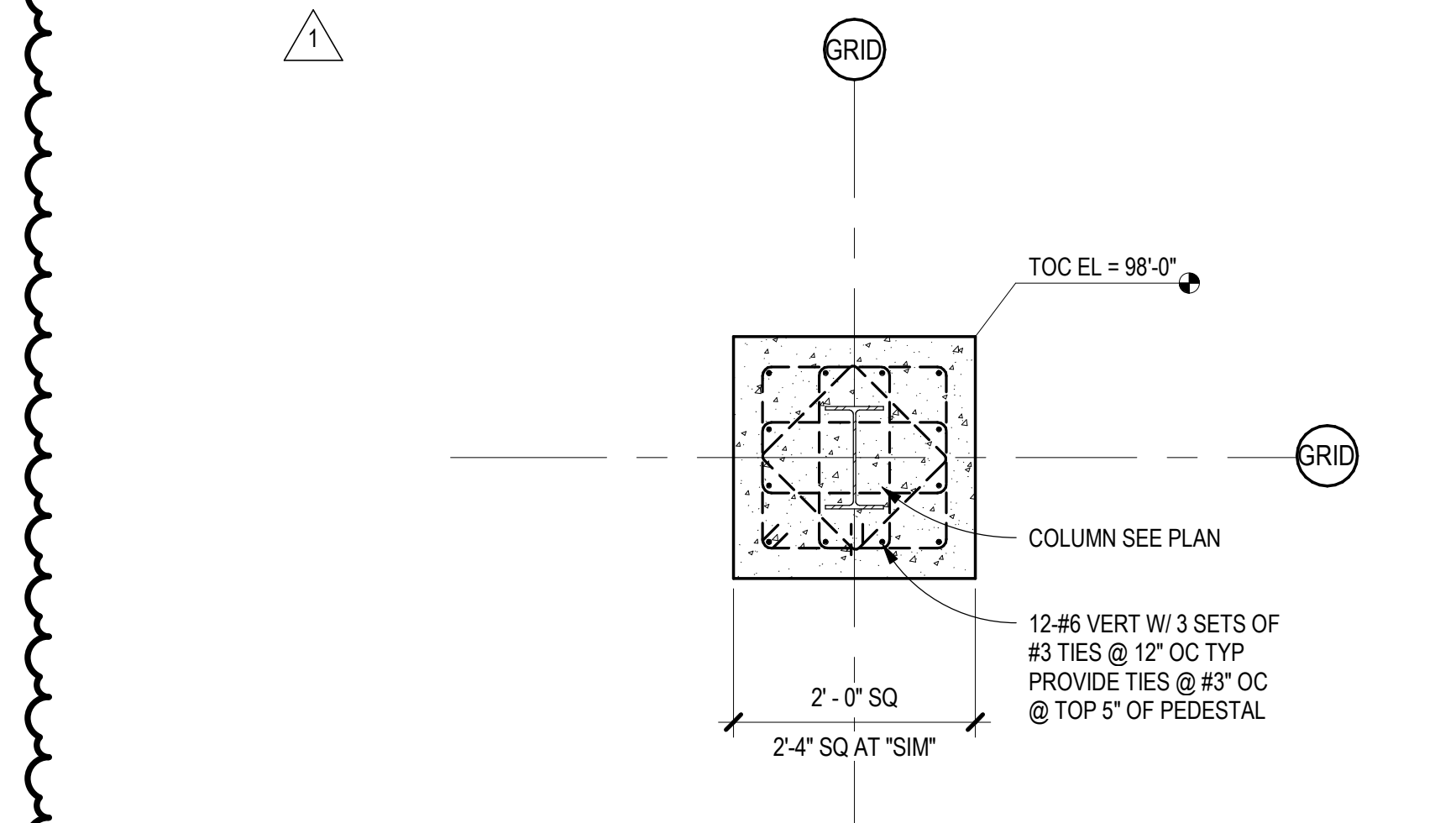
A2 PEDESTAL SECTION
SCALE: 3/4" = 1'-0"



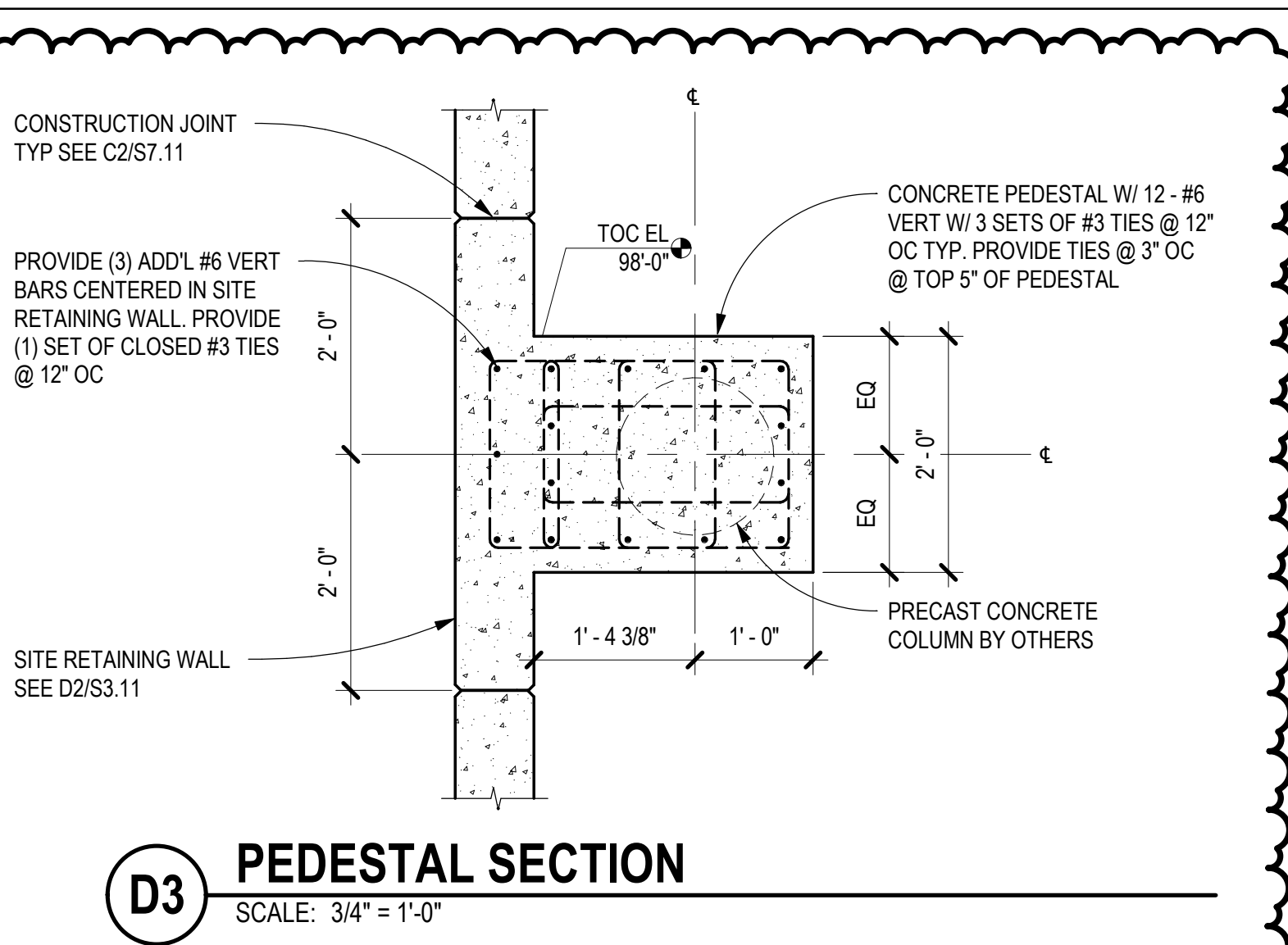
D2 EXT CONC COLUMN SECTION
SCALE: 3/4" = 1'-0"



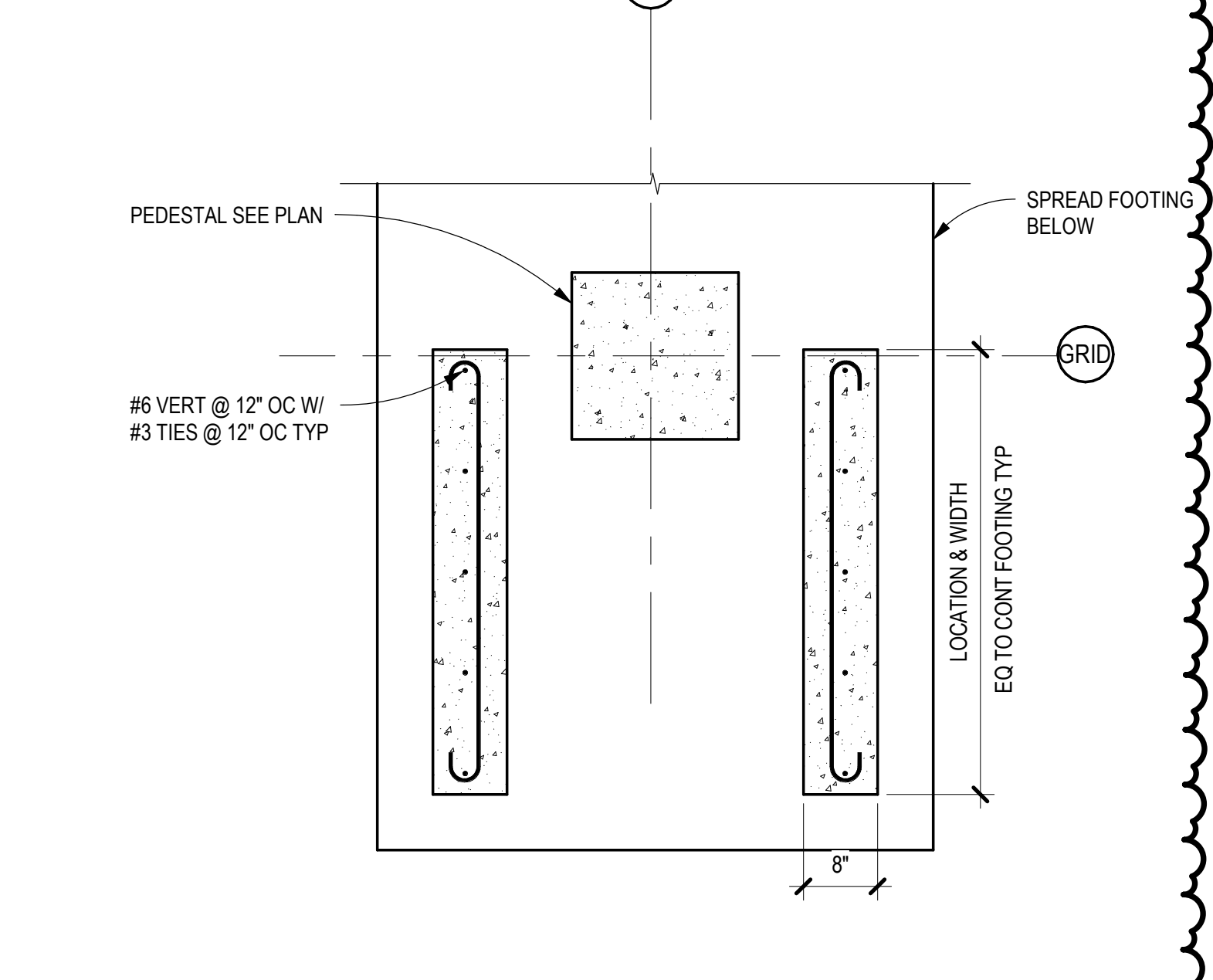
B2 RETAINING WALL SECTION
SCALE: 3/4" = 1'-0"



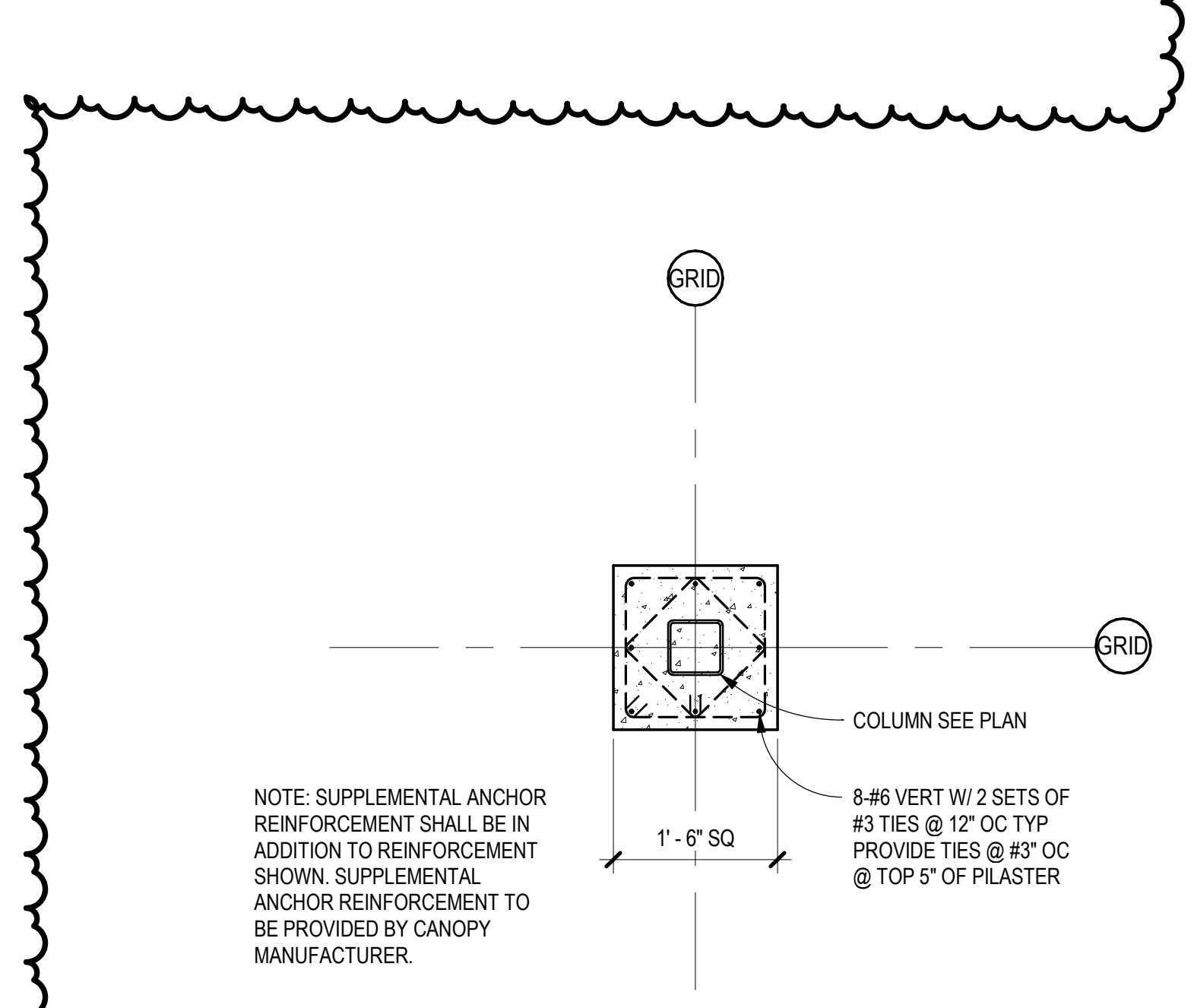
A3 POST TO STEM WALL CONNECTION
SCALE: 3/4" = 1'-0"



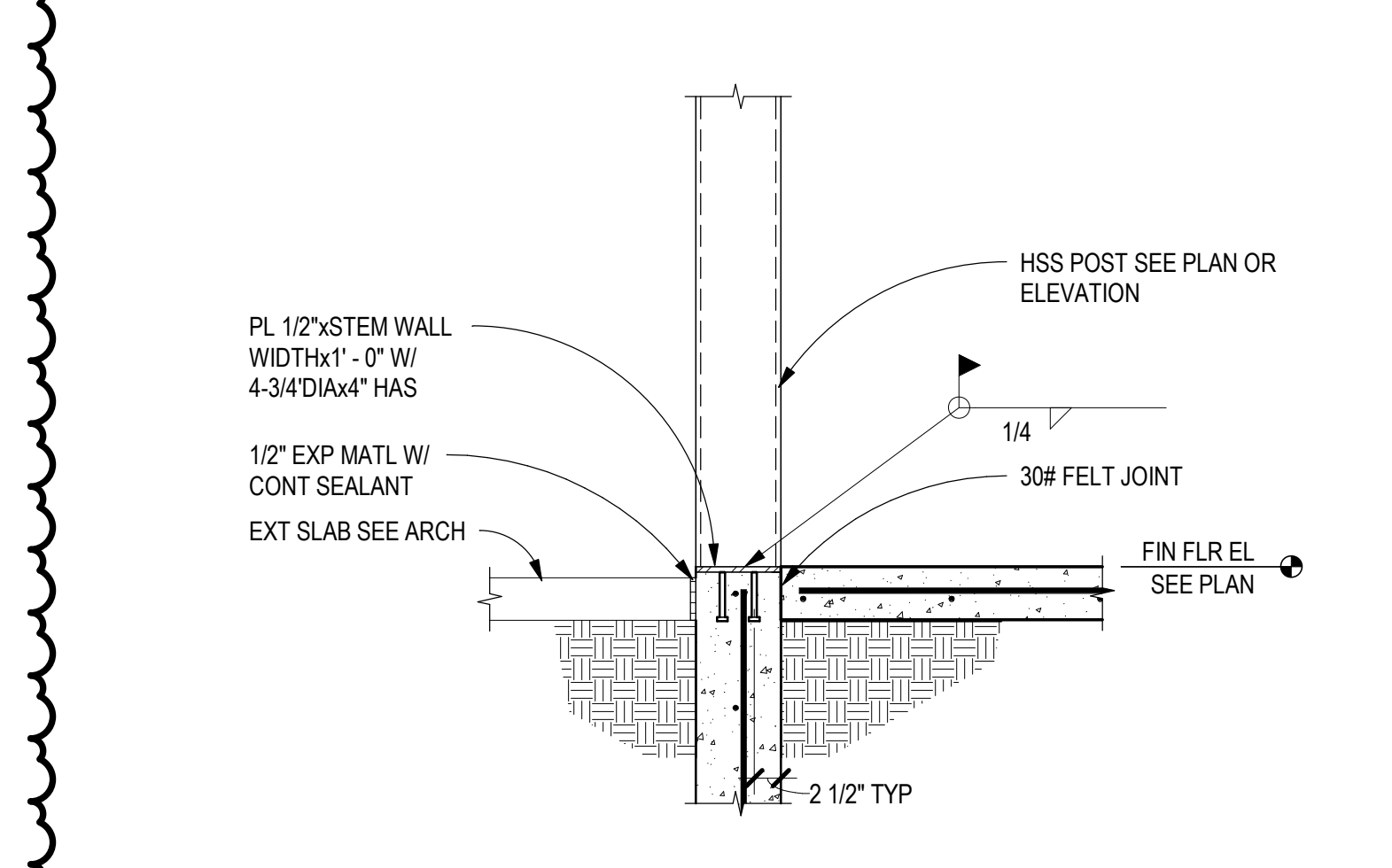
D3 PEDESTAL SECTION
SCALE: 3/4" = 1'-0"



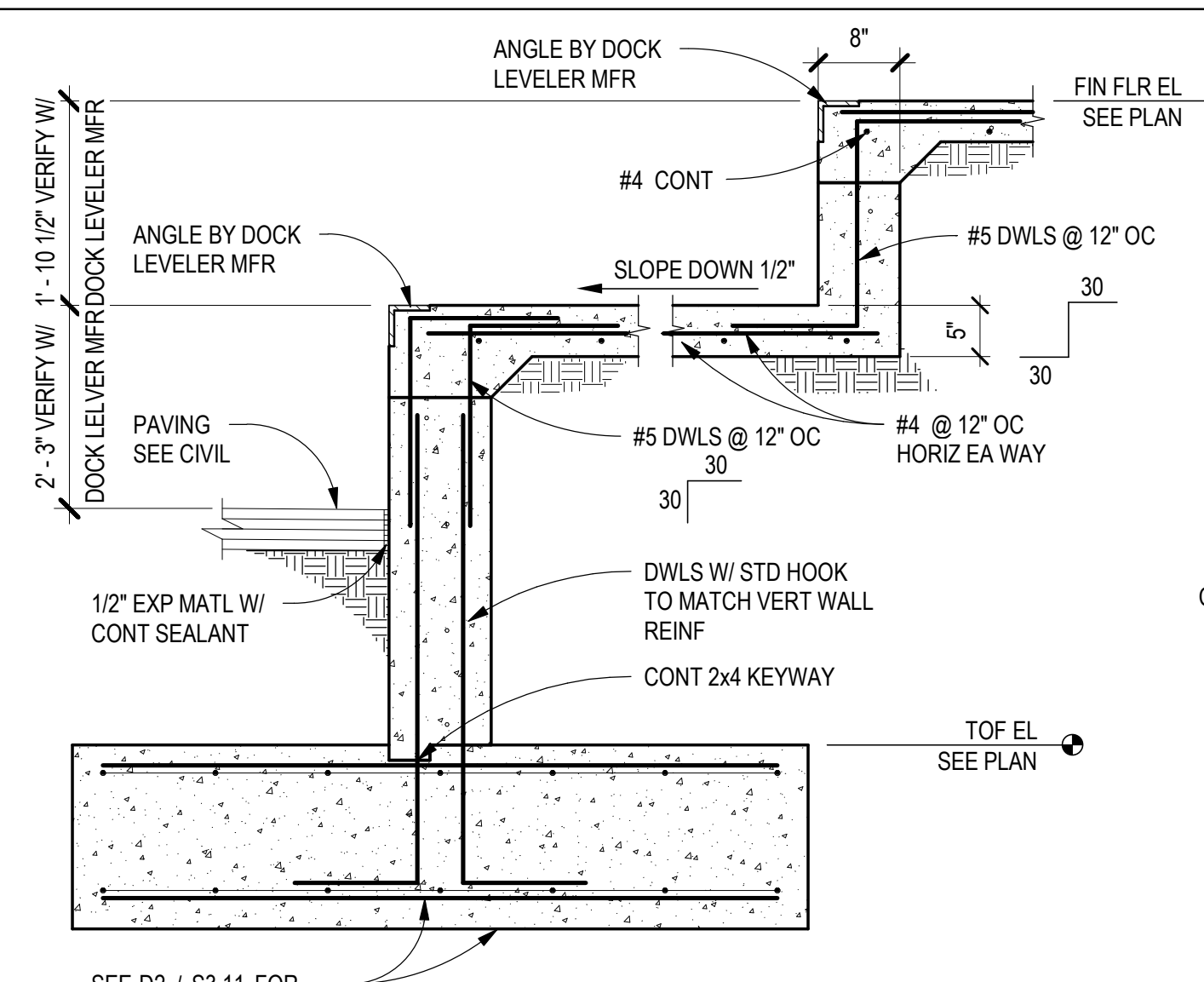
C3 FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



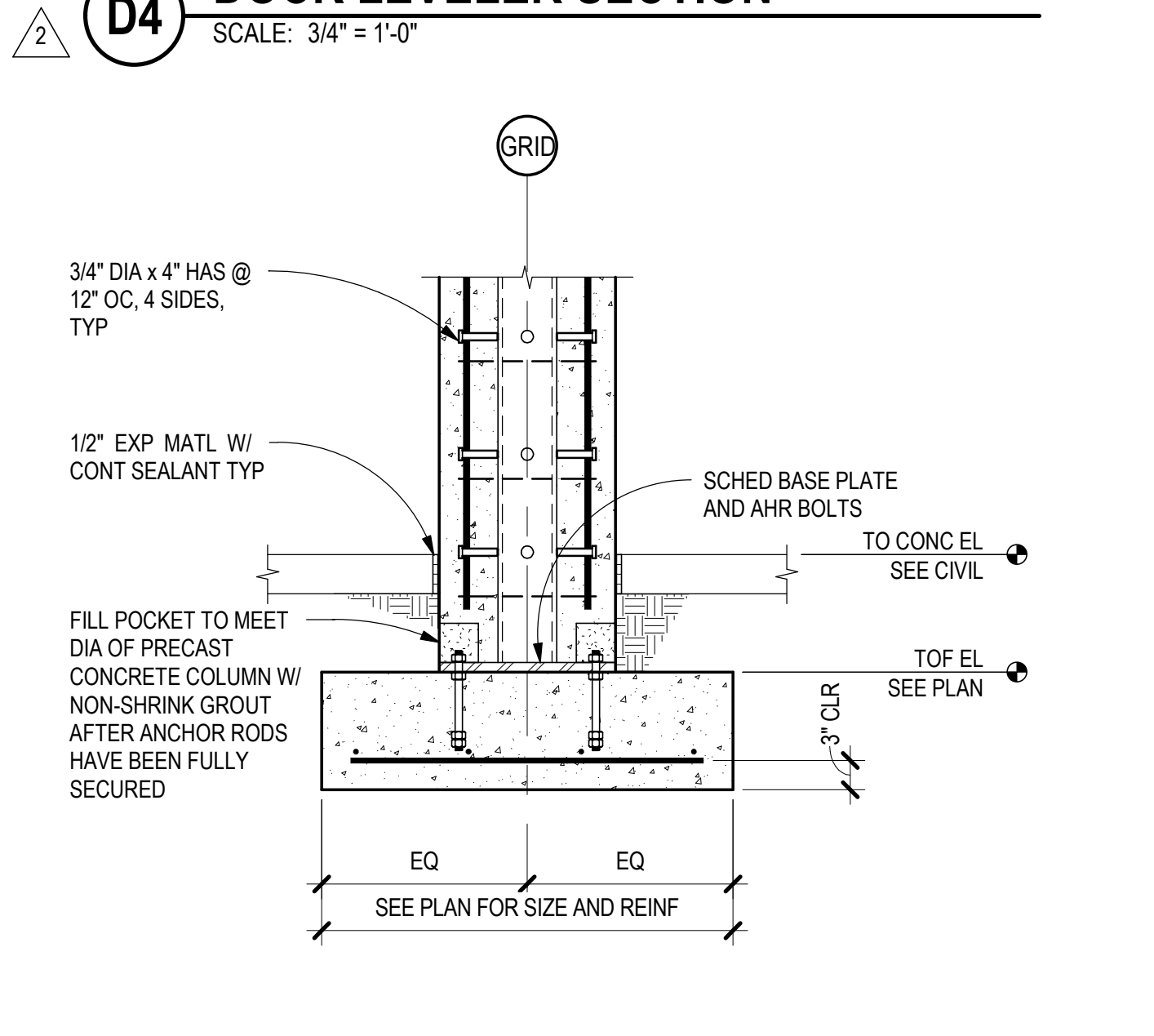
B3 PILASTER AT ELEVATOR SHAFT
SCALE: 3/4" = 1'-0"



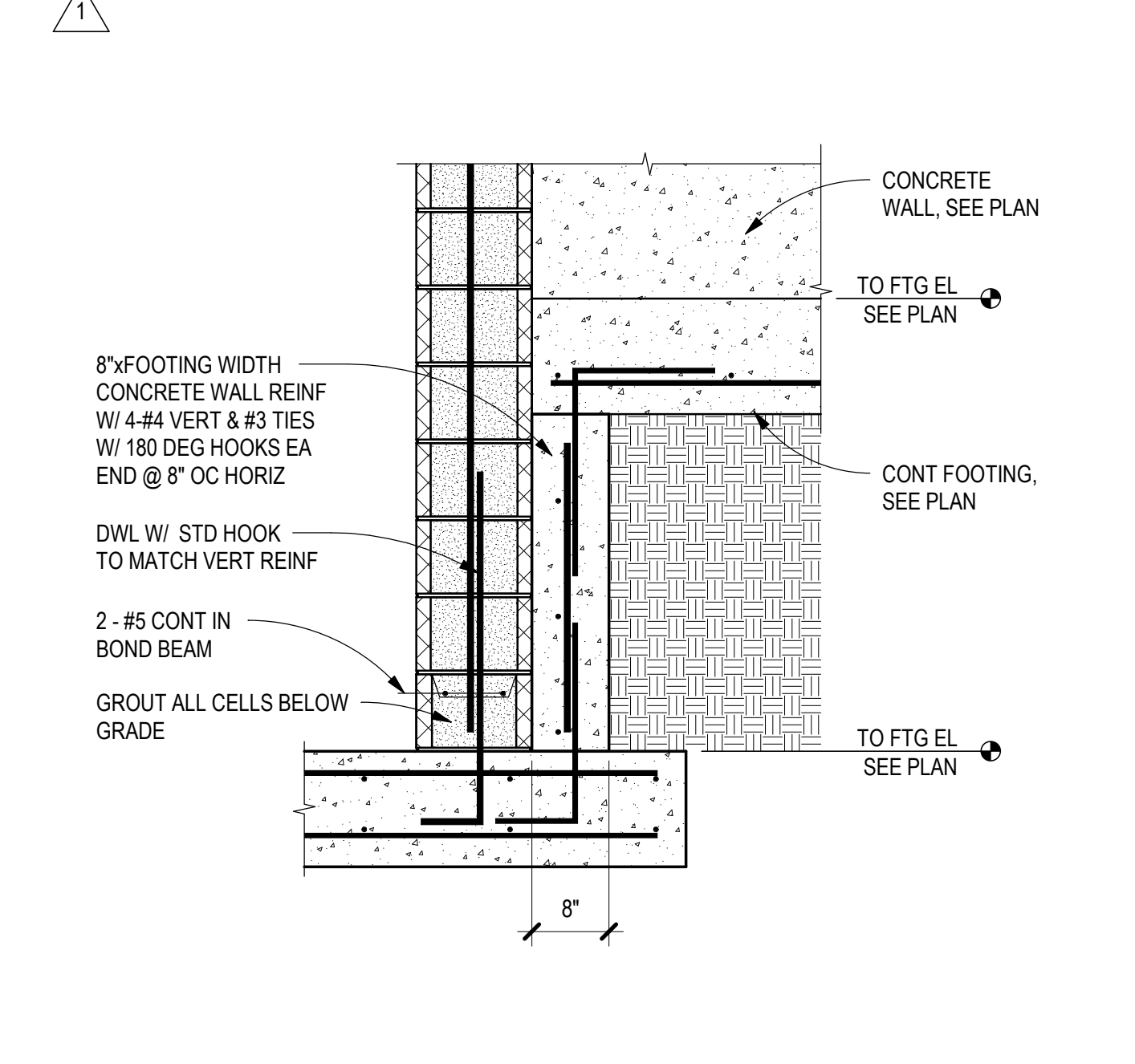
A4 HAIRPIN REINF AT CORNER COLUMN
SCALE: 3/4" = 1'-0"



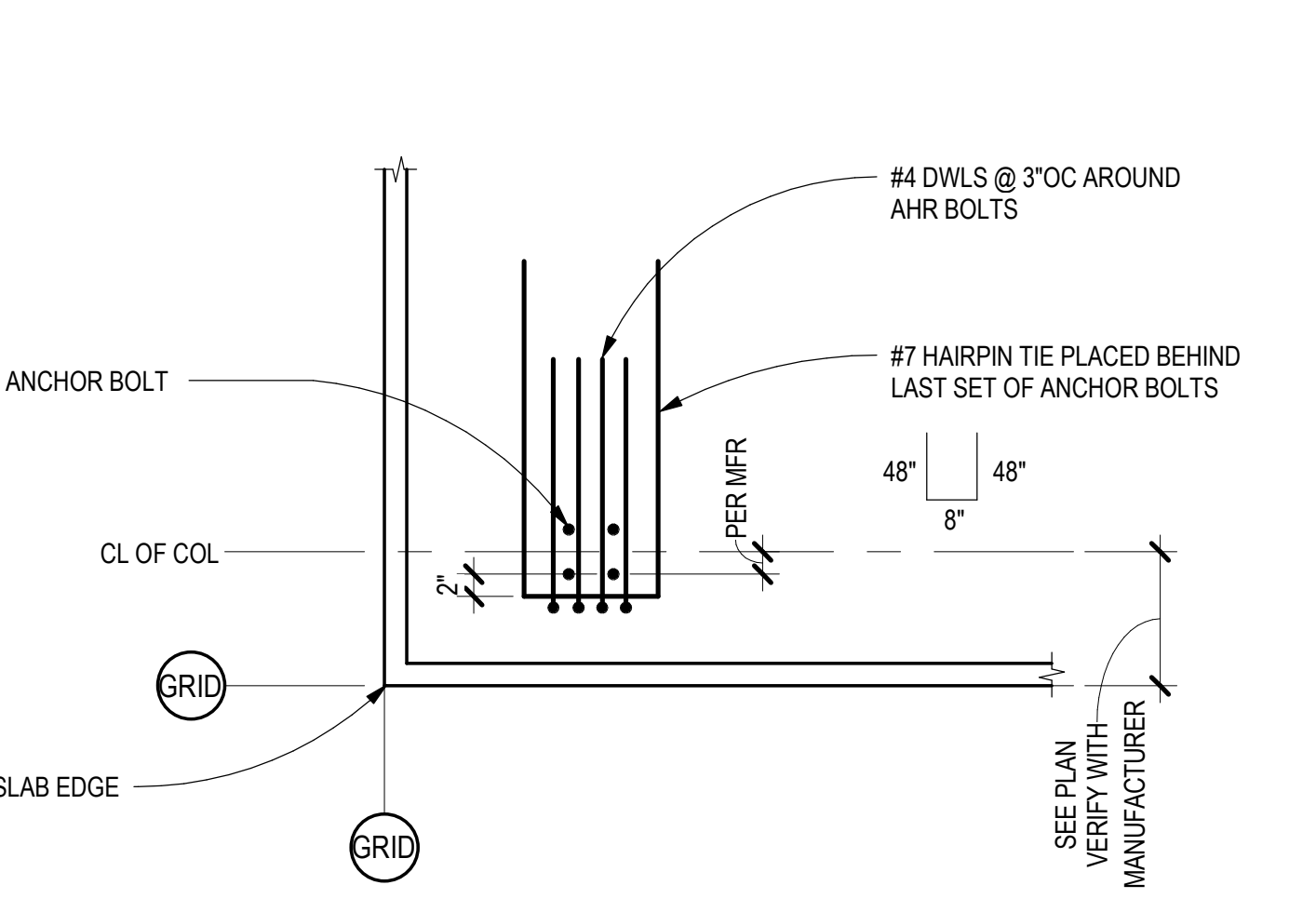
D4 DOCK LEVELER SECTION
SCALE: 3/4" = 1'-0"



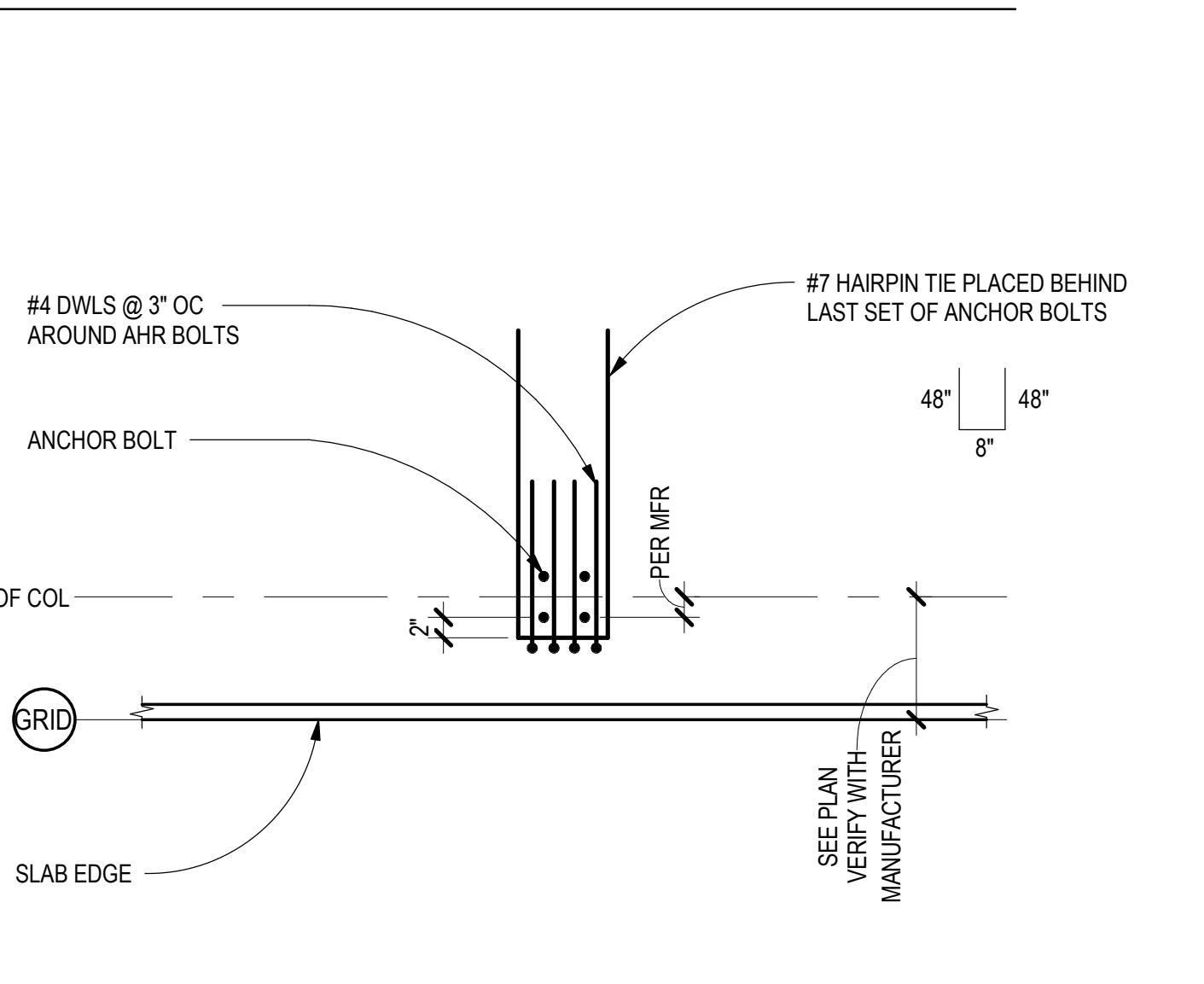
C4 EXTERIOR COLUMN SECTION
SCALE: 3/4" = 1'-0"



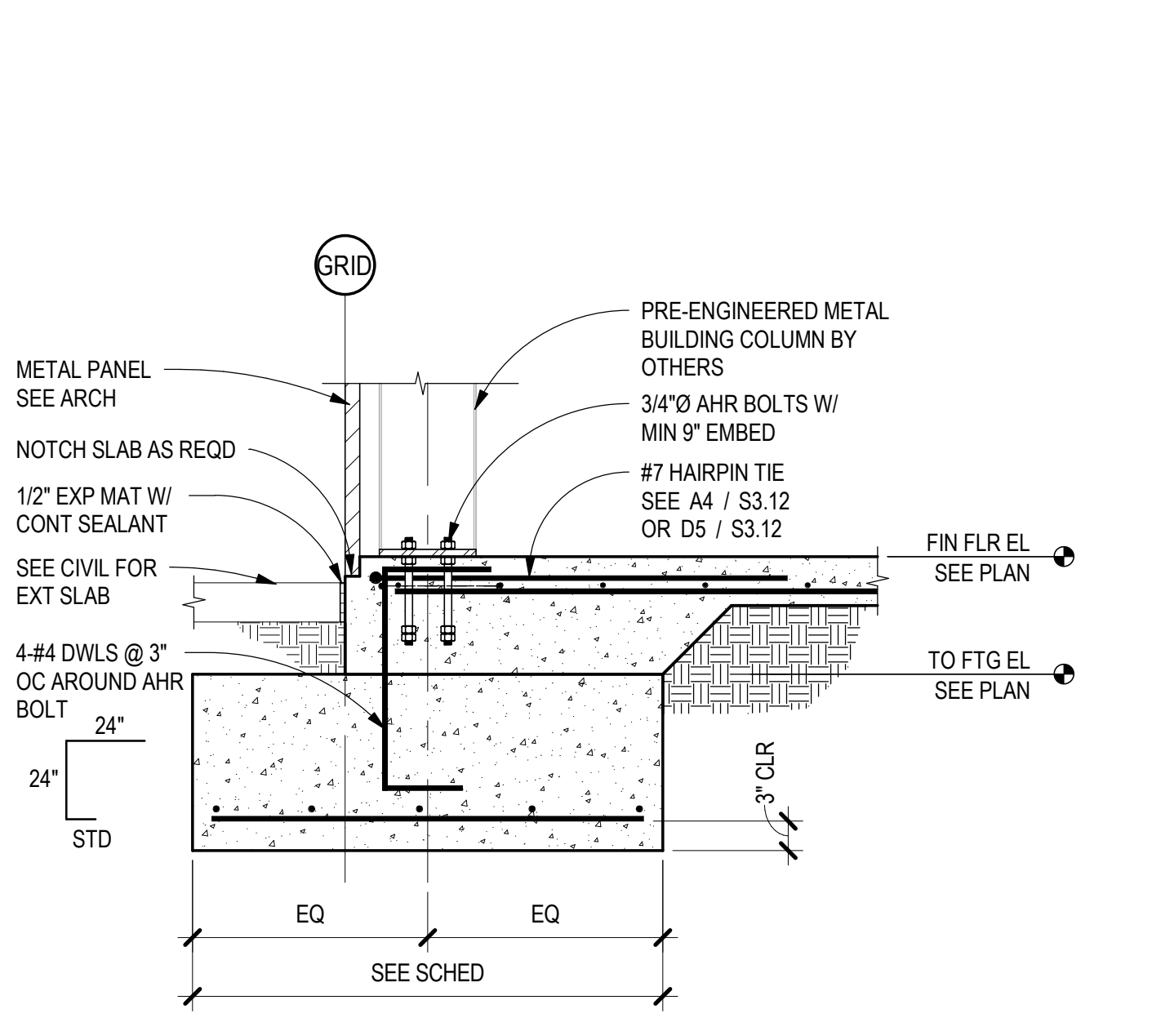
B4 FOUNDATION SECTION @ ELEVATOR
SCALE: 3/4" = 1'-0"



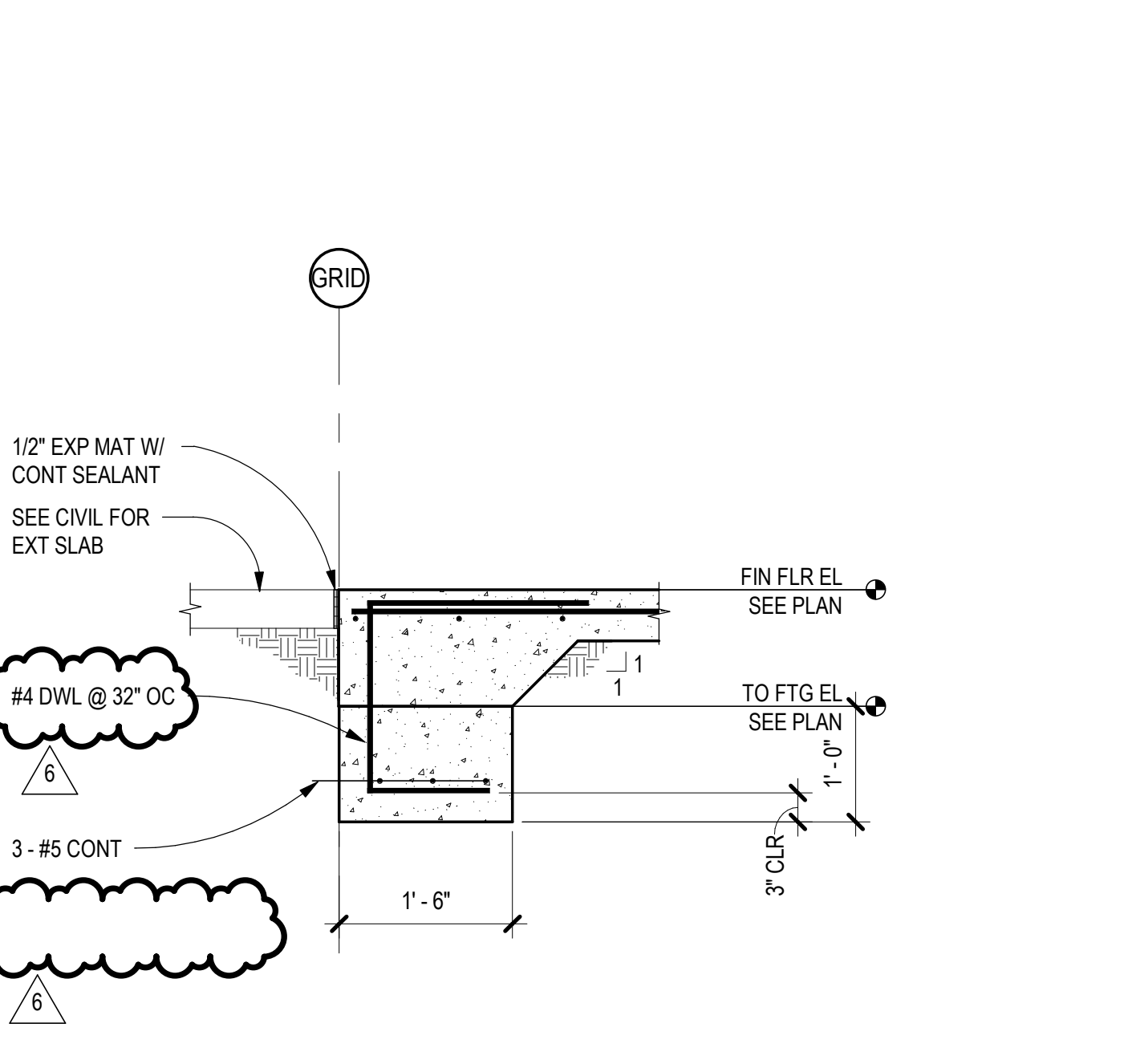
A5 PERIMETER FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



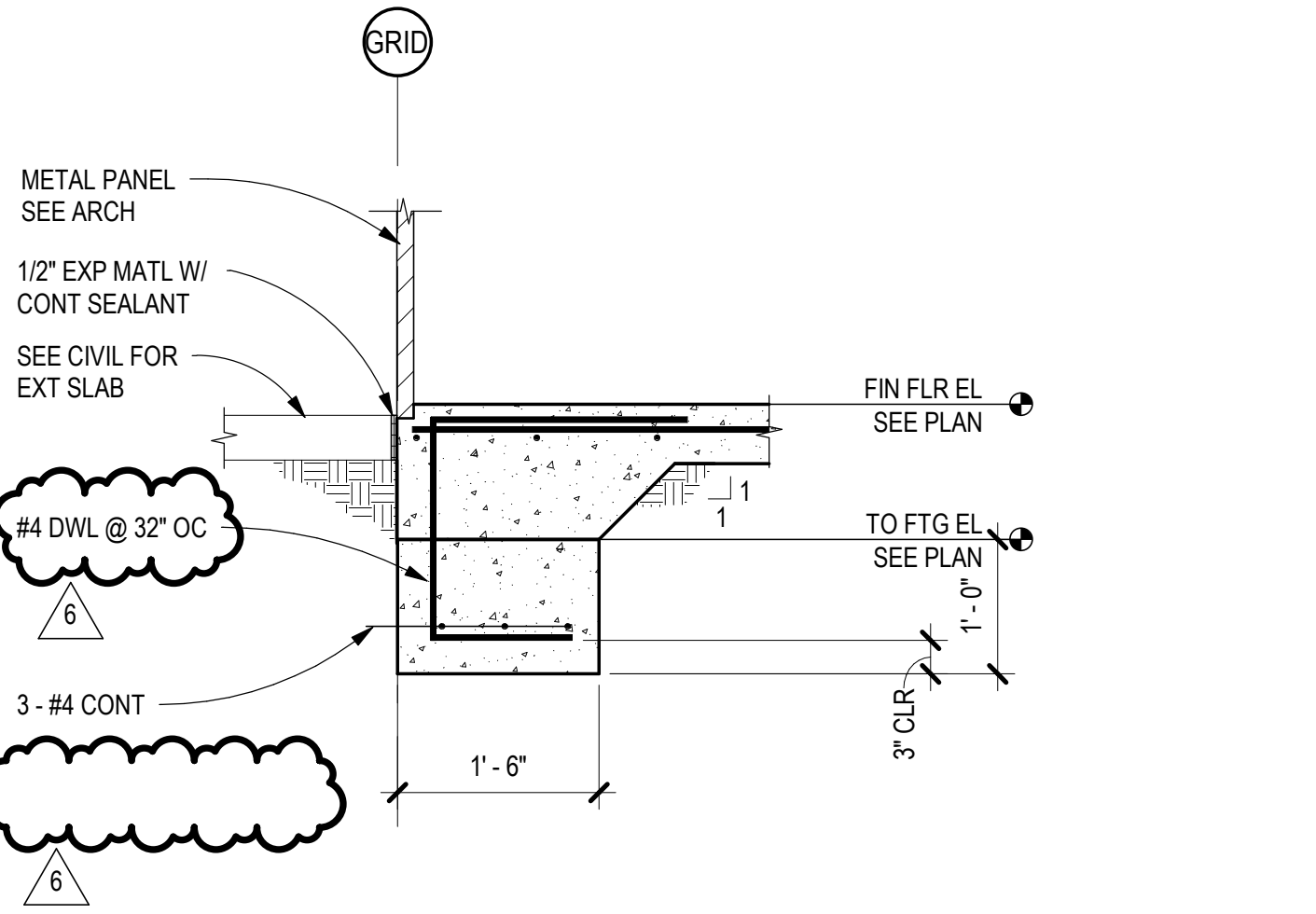
D5 HAIRPIN REINF AT COLUMN
SCALE: 3/4" = 1'-0"



C5 PERIMETER SECTION @ COLUMN
SCALE: 3/4" = 1'-0"



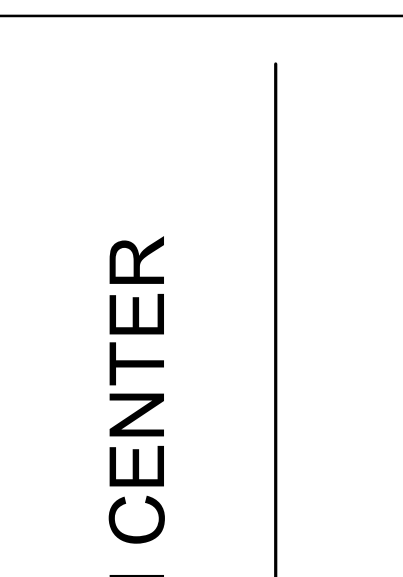
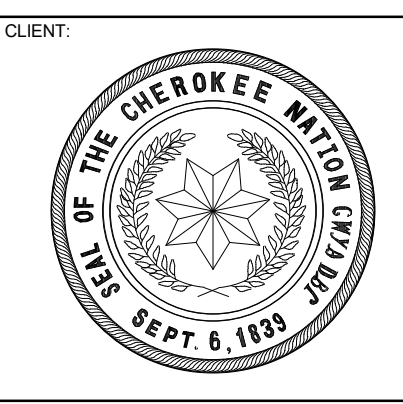
B5 FOUNDATION SECT @ OPENING
SCALE: 3/4" = 1'-0"



A5 PERIMETER FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



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**WILMA P. MANKILLER HEALTH CENTER
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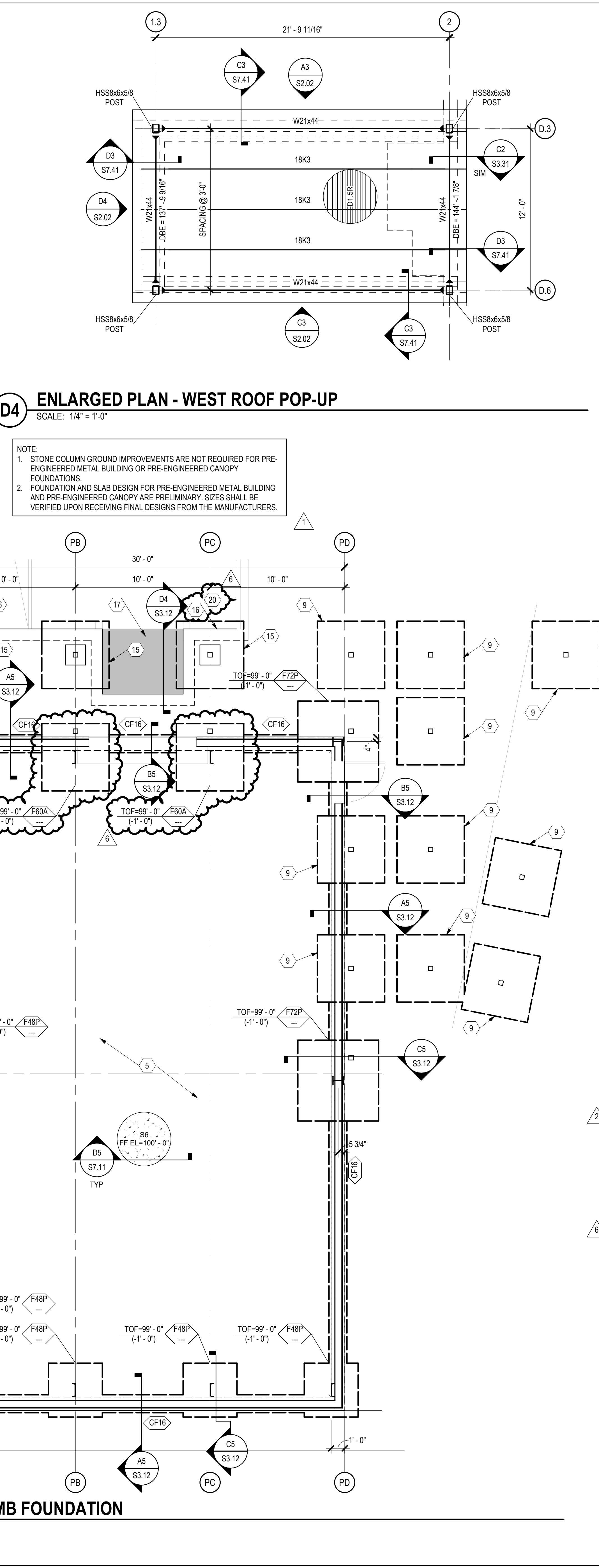
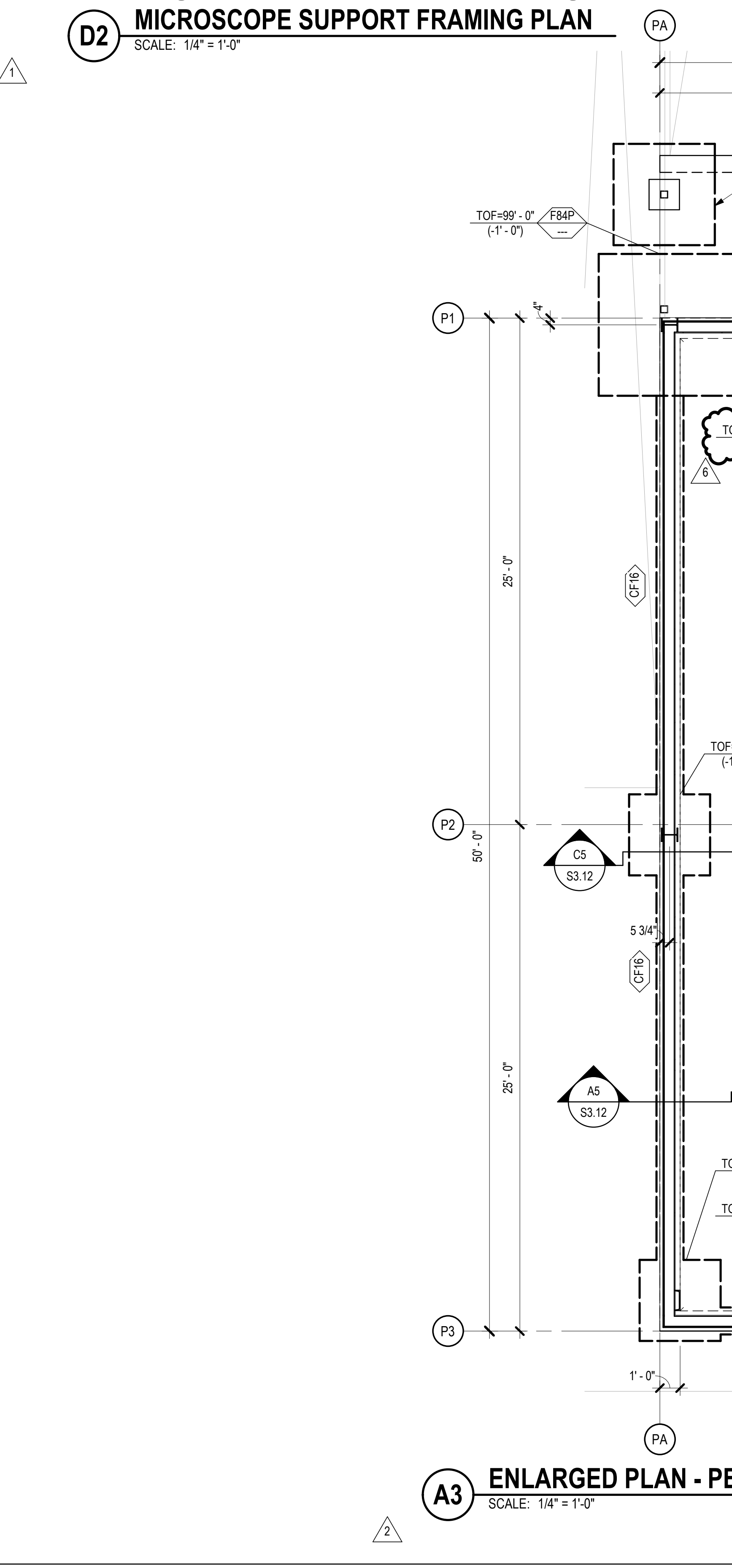
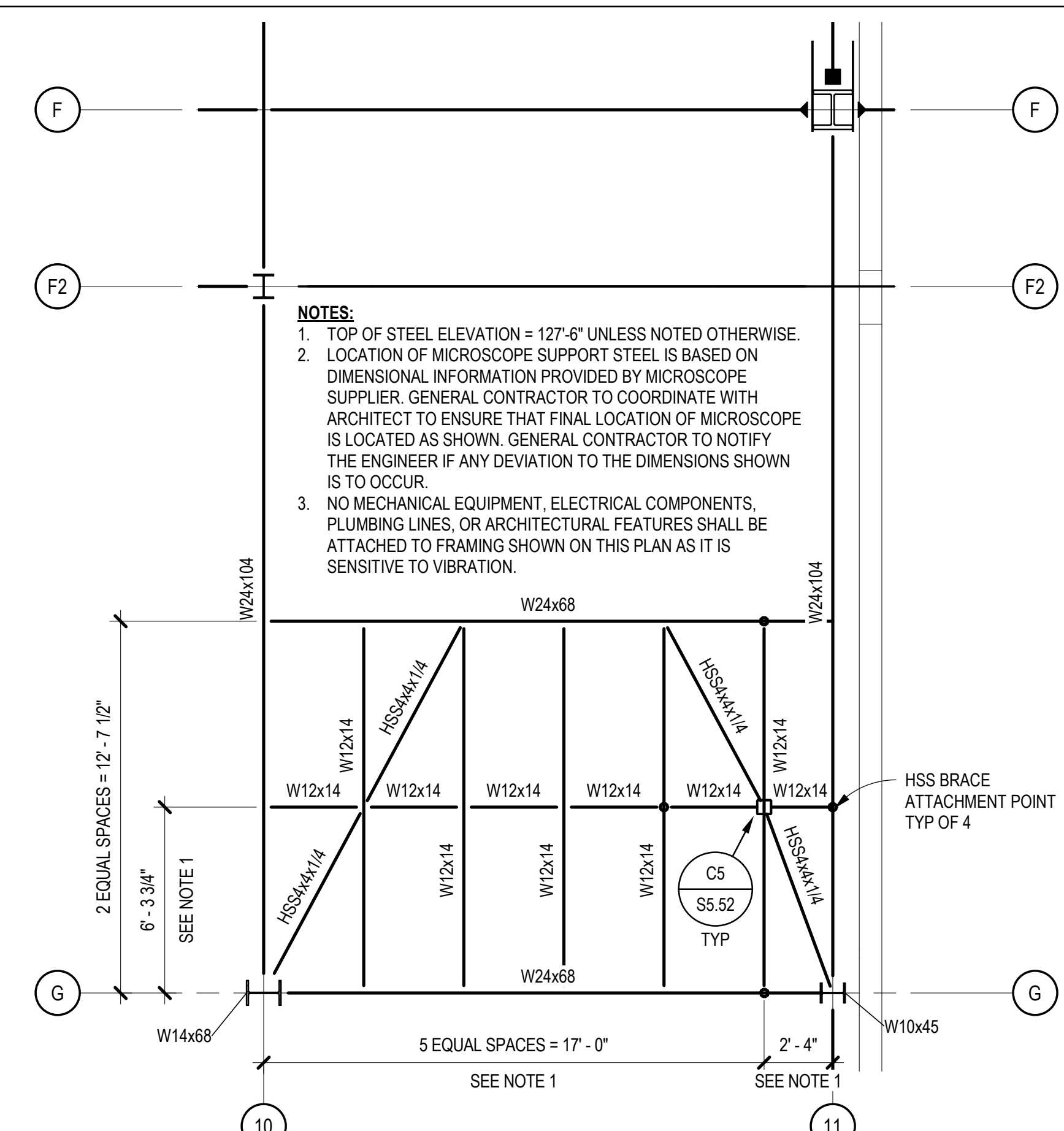
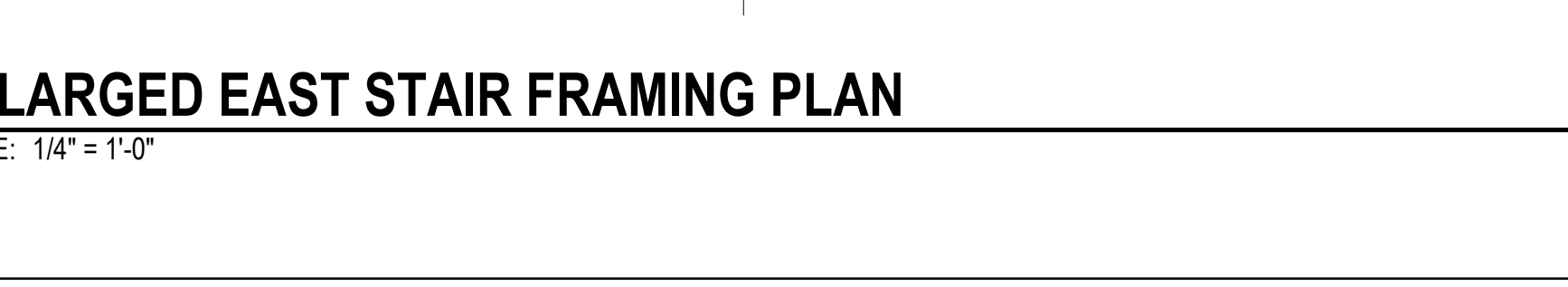
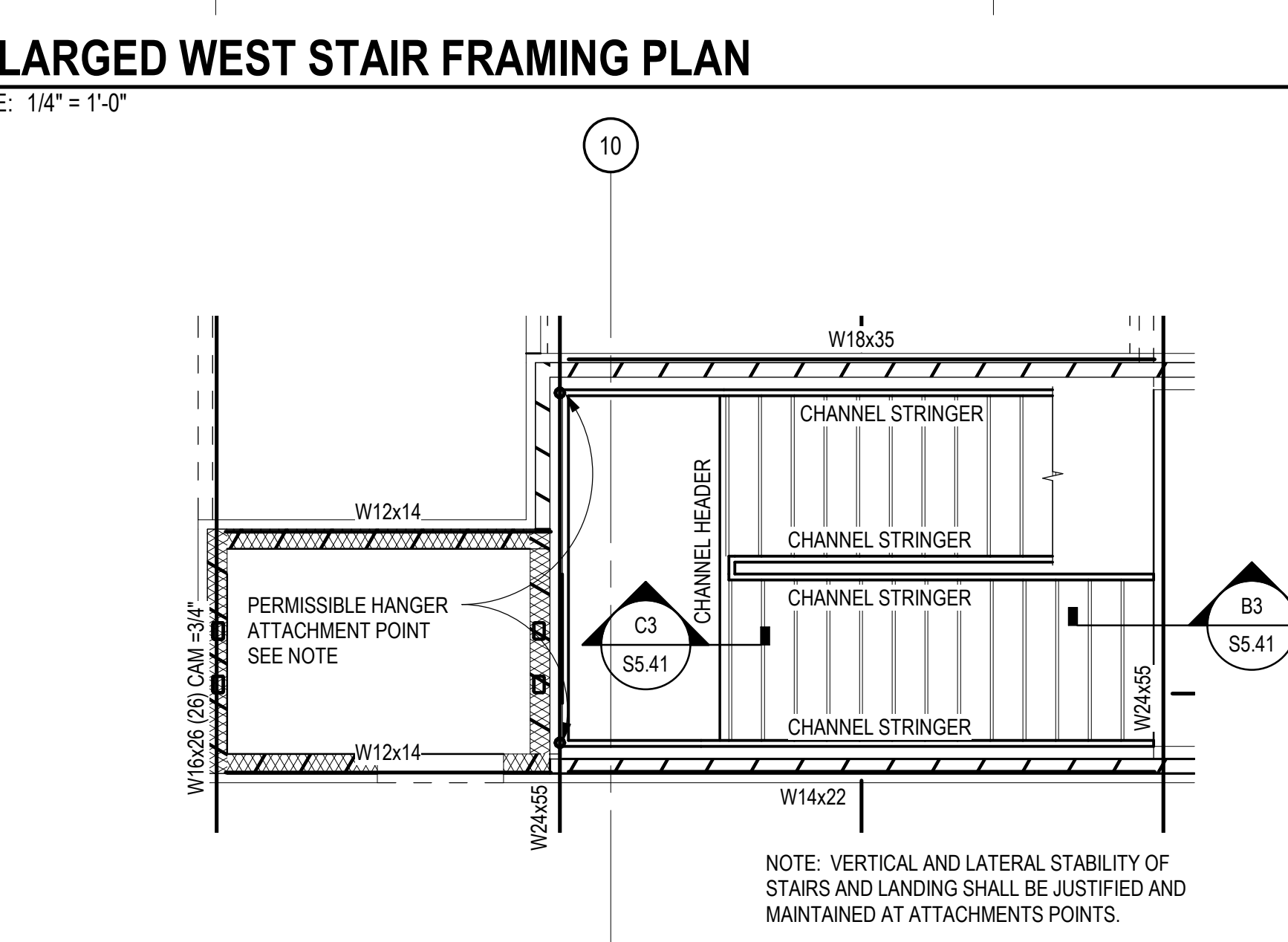
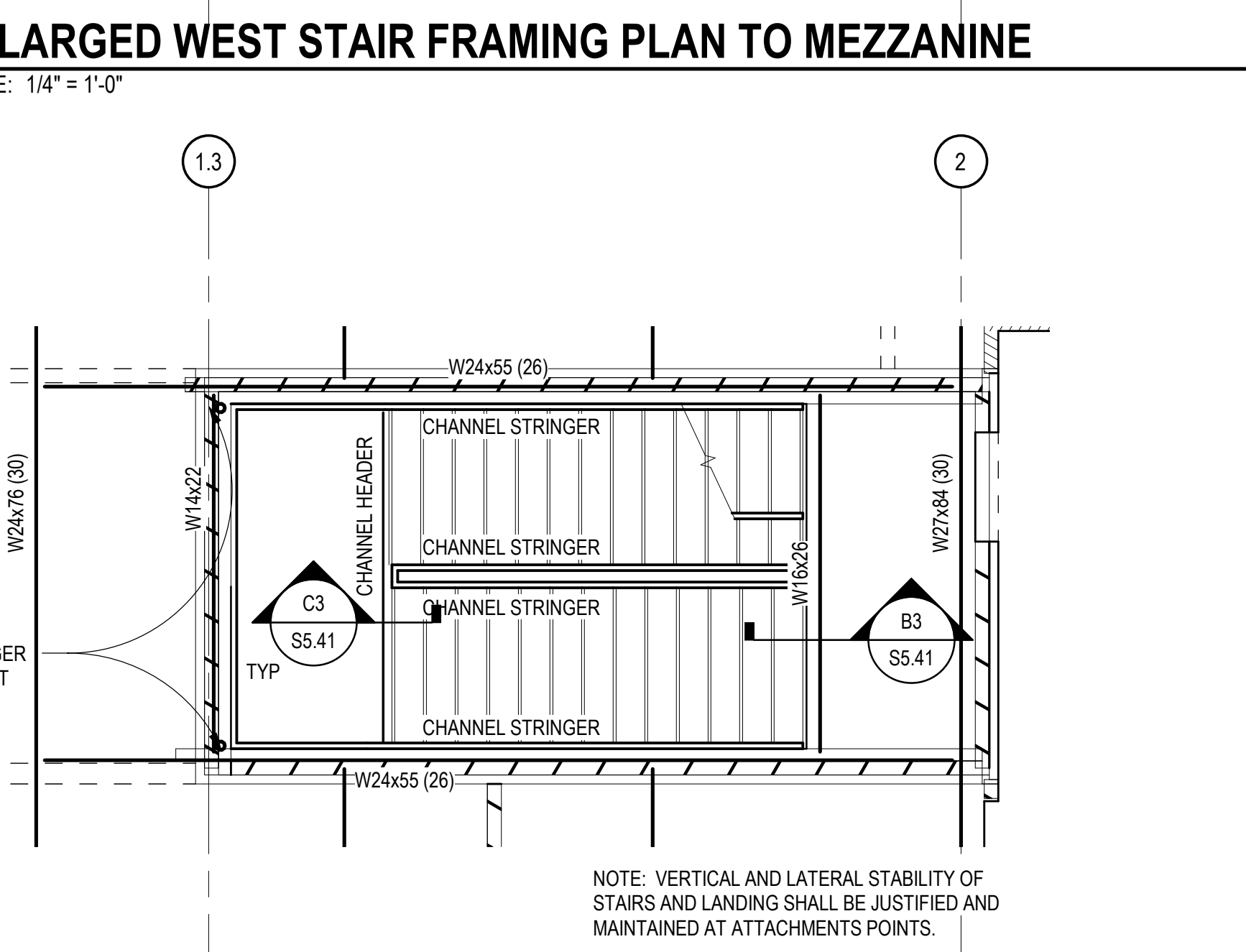
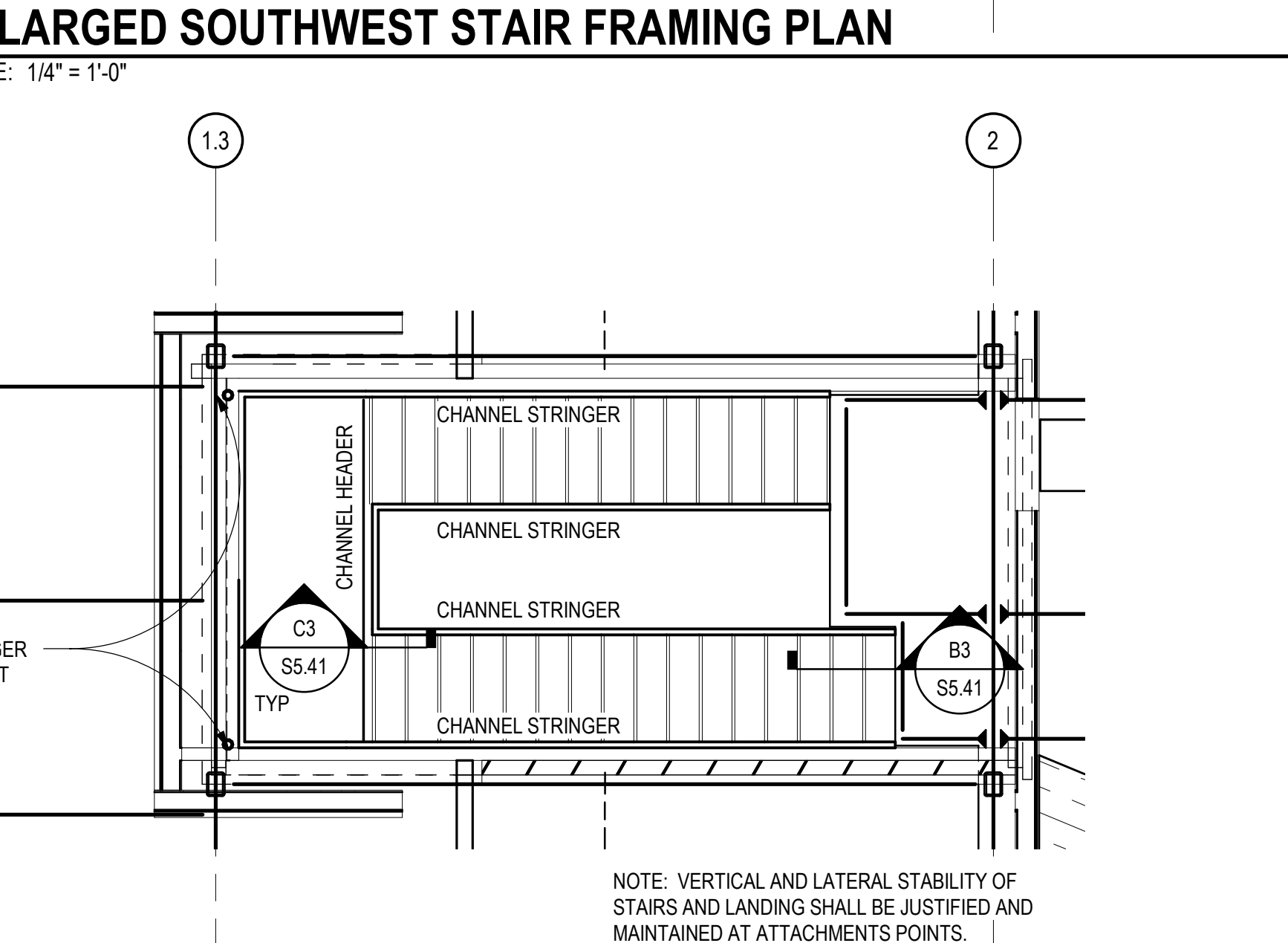
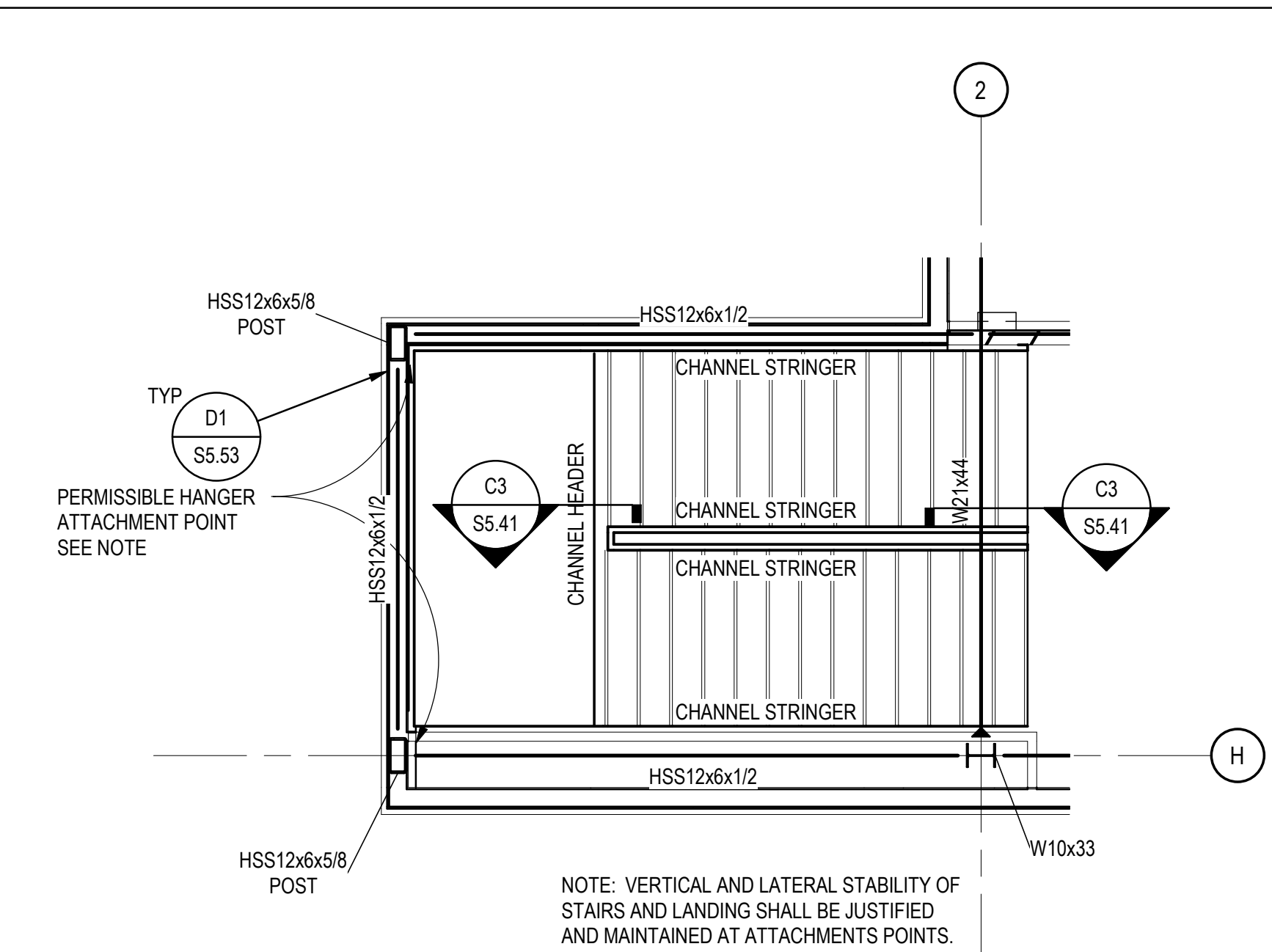
KEY PLAN:

PROJECT PHASE:
BID PACKAGE 01

#	DATE	REVISIONS
1	11/02/19	BID PACKAGE 01 - ADD 01
2	12/16/19	BID PACKAGE 01 - ADD 02
3	04/16/20	BID PACKAGE 01 - ASI 03

DATE: 11/01/19
JOB NUMBER: 18-01.01

SHEET NUMBER:
S3.12
FOUNDATION SECTIONS

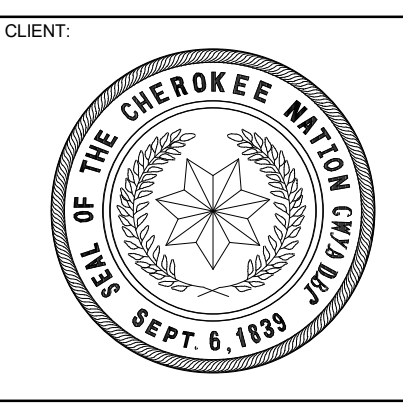


GENERAL SHEET NOTES

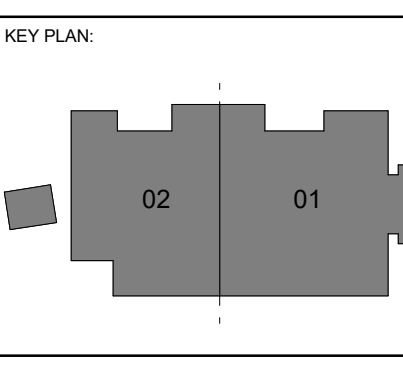
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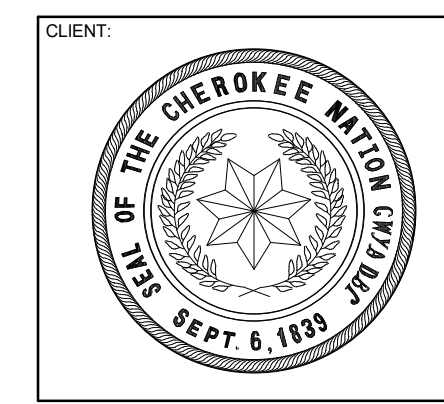
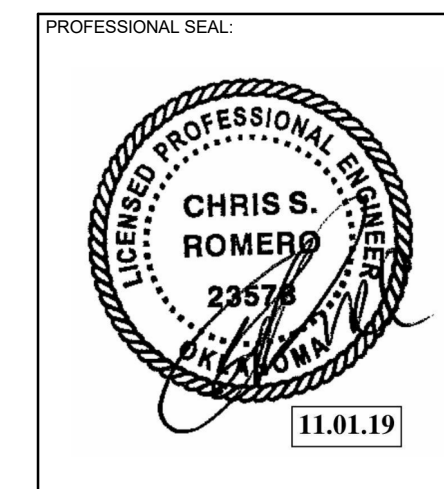
PROJECT PHASE:
BID PACKAGE 01

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3	04/16/20	BID PACKAGE 01 - ASB 03

DATE: 11/01/19
JOB NUMBER: 18-01.01

SHEET NUMBER:
S4.01

ENLARGED PLANS



WILMA P. MANKILLER HEALTH CENTER
EXPANSION
STILWELL, OKLAHOMA

KEY PLAN

PROJECT PHASE
BID PACKAGE 01

#	DATE	REVISIONS	DESCRIPTION
1	11/22/19	BID PACKAGE 01 - ADD 01	
2	04/16/20	BID PACKAGE 01 - ADD 02	

DATE: 11/01/19 JOB NUMBER: 18-01.01

SHEET NUMBER: S6.01

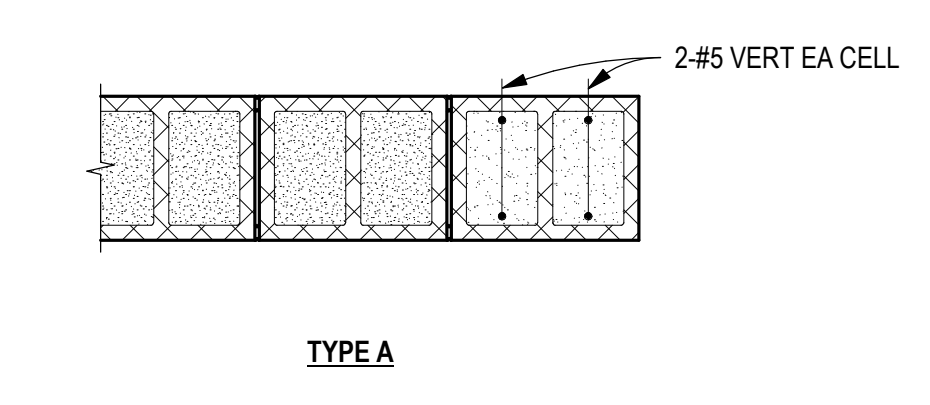
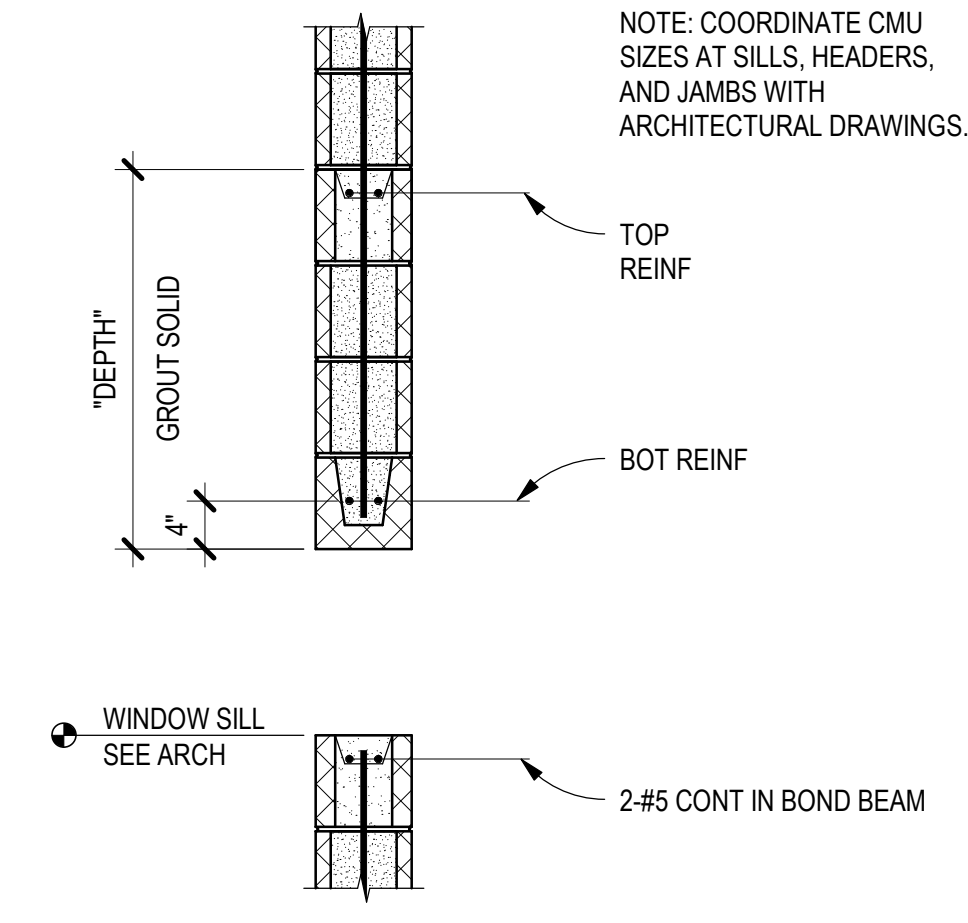
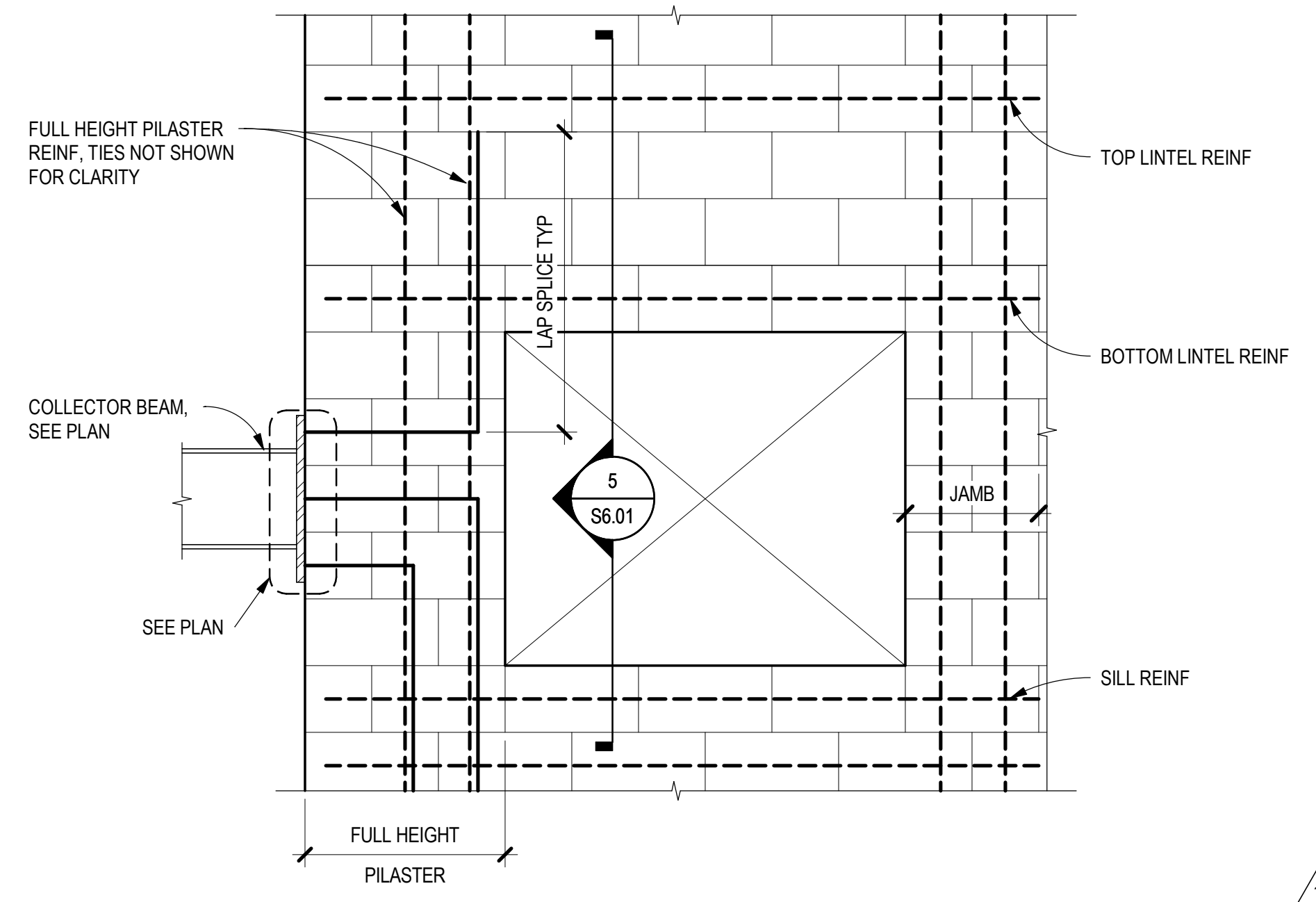
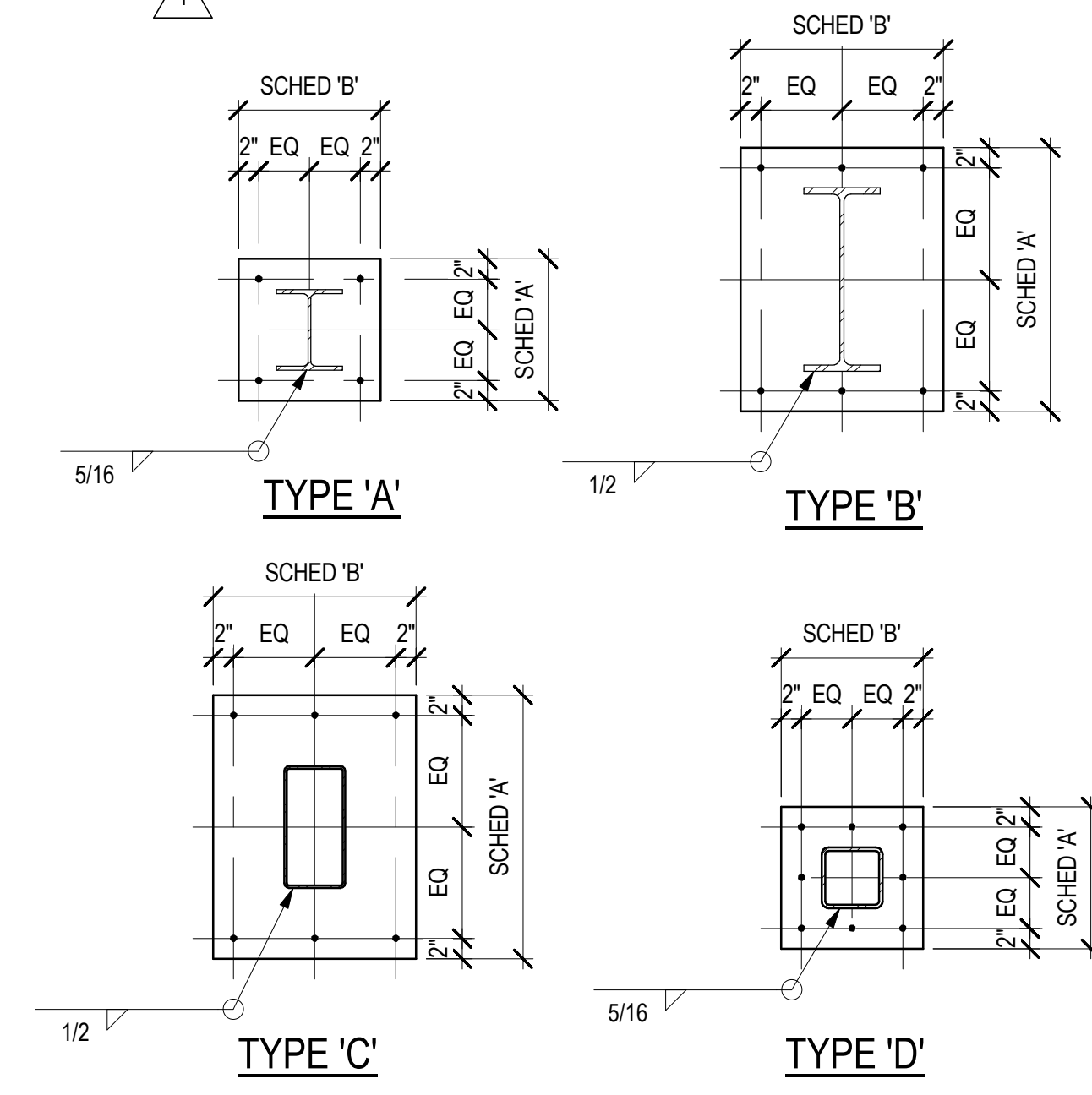
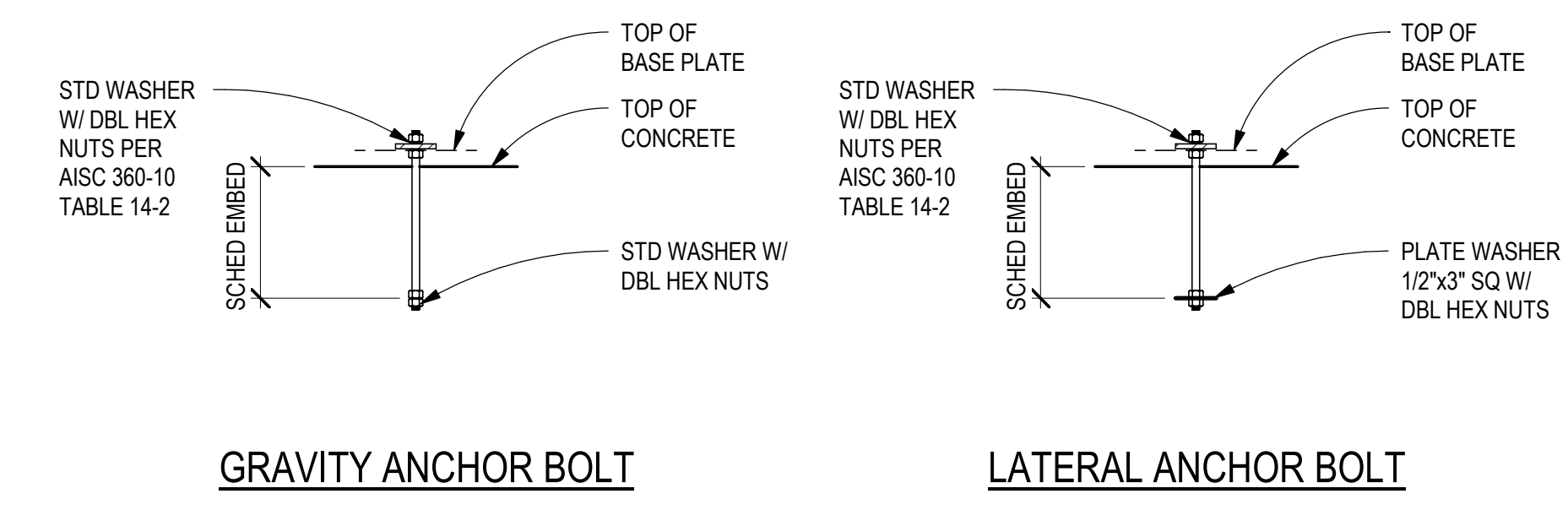
SCHEDULES

OPENING WIDTH	WIDTH	DEPTH	LINTEL REINFORCING		SILL	LINTEL JAMB TYPE
			TOP	BOTTOM		
0' - 0" - 8' - 0"	12"	32"	2 - #5	2 - #5	8"	2 - #5
						TYPE A

MARK	COMPOSITE SLAB	SLAB			METAL DECK			DECK ATTACHMENTS			TOTAL SLAB / DECK THICKNESS	COMMENTS	
		THICK	MATL	REINF	THICK	TYPE	GAGE	FINISH	ATTACH PERP TO RIBS	ATTACH PARALLEL TO RIBS			ATTACH SIDELAPS
D15R	X	3"	NW CONC	6x6 - W2.1xW2.1 WELD WIRE FABRIC IN FLAT SHEETS	1 1/2"	HSB	20	GALVANIZED	7-5/8" DIA PUDDLE WELDS PER 36" WIDE SHEET	5/8" DIA PUDDLE WELDS @ 6" OC	#10 SCREWS @ 9" OC	1 1/2"	
D6F	X	3"	NW CONC	6x6 - W2.1xW2.1 WELD WIRE FABRIC IN FLAT SHEETS	3"	VLI	18	GALVANIZED	4-5/8" DIA PUDDLE WELDS PER 36" WIDE SHEET	5/8" DIA PUDDLE WELDS @ 12" OC	#10 SCREWS @ 9" OC	6"	

MARK	THICKNESS	MATL	REINFORCING	BEARING STRATA	COMMENTS
S6	6"	CONC	#4 @ 12" OC EA WAY	15 MIL VAPOR RETARDER OVER 1/2" SAND BLOTTER LAYER OVER 4" COMPACTED GRANULAR FILL OVER 14" OF COMPACTED STRUCTURAL FILL OVER COMPACTED SUBGRADE. SUBGRADE WILL BE PLACED WITH LASER LEVEL	PREPARE SUBGRADE AND STRUCTURAL FILL PER GEOTECHNICAL REPORT

MARK	TYPE	SIZE	ANCHOR BOLTS	
			F1554	TYPE
BP1	A	PL 1 1/4"x18"x1"-6"	4 - 3/4" DIA x 9"	GRAVITY
BP2	A	PL 1 3/4"x18"x1"-6"	4 - 3/4" DIA x 9"	GRAVITY
BP3	A	PL 1 3/4"x20"x1"-8"	4 - 3/4" DIA x 9"	GRAVITY
BP4	A	PL 1 3/4"x22"x1"-10"	6 - 1" DIA x 18"	GRAVITY
BP5	A	PL 3/4"x14"x1"-2"	4 - 3/4" DIA x 9"	GRAVITY
BP6	B	PL 1 3/4"x20"x1"-8"	6 - 1" DIA x 18"	LATERAL
BP7	B	PL 1 3/4"x22"x1"-8"	6 - 1" DIA x 18"	LATERAL
BP8	C	PL 1 1/4"x20"x1"-4"	6 - 1" DIA x 18"	LATERAL
BP9	D	PL 3/4"x14"x1"-2"	4 - 3/4" DIA x 9"	LATERAL
BP10	D	PL 1"x18"x1"-6"	8 - 3/4" DIA x 9"	LATERAL



NOTE: SEE TYPICAL CMU PLAN DETAILS SHEET S7.21 FOR TYPICAL HORIZONTAL REINFORCING REQUIREMENTS.

MARK	VENEER	WALL	REINFORCING			COMMENTS
			VERTICAL	HORIZONTAL	GRADE	
WC8	--	8" CONC	#4 @ 12" OC	#4 @ 12" OC	A615	
WC10	--	10" CONC	#4 @ 12" OC	#4 @ 12" OC	A615	
WC12	SEE ARCH	12" CONC	#5 @ 12" OC EA FACE	#5 @ 12" OC EA FACE	A615	
WM12	SEE ARCH	12" CMU	#7 @ 16" OC EA FACE	#5 @ 24" OC EA FACE & STD LADDER TYPE JOINT REINF @ 15" OC	A615	GROUT ALL CELLS SOLID. SEE 7.21 FOR MASONRY DETAILS. D4S7.21 FOR HORIZONTAL REINFORCEMENT LOCATION

REINFORCEMENT TYPE	#6 AND SMALLER (#6)					#7 THROUGH #11 (#6b)			MINIMUM LENGTH (IN)	COMMENTS
	3000PSI	4000PSI	5000PSI	3000PSI	4000PSI	5000PSI	3000PSI	4000PSI		
CONTINUOUS WALL FOOTINGS AND HORIZONTAL REINFORCEMENT IN SITE WALLS	30	30	30	30	30	30	30	30	18	
CONCRETE WALLS: ALL VERTICAL REINFORCEMENT	44	38	34	55	48	43	12			
CONCRETE WALLS: ALL HORIZONTAL REINFORCEMENT, EXCLUDING SITE WALLS AND STEM WALLS	57	50	45	72	62	56	12			
CONCRETE COLUMNS	44	38	34	55	48	43	12			
TOP FLEXURAL REINFORCEMENT, INCLUDING BEAMS, GRADE BEAMS, AND COMBINED COLUMN FOOTINGS	57	50	45	72	62	56	12			
BOTTOM FLEXURAL REINFORCEMENT, INCLUDING BEAMS, GRADE BEAMS, AND COMBINED COLUMN FOOTINGS	44	38	34	55	48	43	12			
MINIMUM EMBEDMENT OF STANDARD HOOKS INTO CONCRETE	22	19	17	22	19	17	6		ALLOWED FOR BARS LARGER THAN #11	
SLABS ON GRADE	30	30	30	30	30	30	12			
SLABS ON METAL DECK	30	30	30	30	30	30	12		WWF MINIMUM LAP LENGTH = 6 IN	

NOTES:

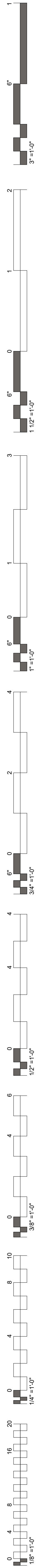
- LAP SPLICES SHALL NOT BE PERMITTED FOR BARS LARGER THAN #11.
- LAP SPLICES FOR BUNDLED BARS SHALL BE IN ACCORDANCE WITH ACI 318-14 SECTION 25.5.1.4
- LAP LENGTHS FOR LIGHTWEIGHT CONCRETE SHALL BE INCREASED BY 33%
- LAP LENGTHS FOR EPOXY COATED BARS SHALL BE INCREASED BY 50%
- FOR INTERMEDIATE OR LARGER VALUES OF F_c, USE THE CLOSEST LOWER VALUE IN THE TABLE. DO NOT INTERPOLATE

	#3	#4	#5	#6	#7	#8	#9
8" BLOCK WITH 1-LAYER OF REINFORCEMENT	32	40	51	72	N/A	N/A	N/A
8" BLOCK WITH 1-LAYER OF REINFORCEMENT	32	29	36	58	68	72	N/A
8" BLOCK WITH 2-LAYERS OF REINFORCEMENT	51	68	72	72	N/A	N/A	N/A
12" BLOCK WITH 1-LAYER OF REINFORCEMENT	32	24	23	37	43	57	65
12" BLOCK WITH 2-LAYERS OF REINFORCEMENT	51	68	72	72	72	N/A	N/A
16" BLOCK WITH 1-LAYER OF REINFORCEMENT	32	24	23	30	32	42	48
16" BLOCK WITH 2-LAYERS OF REINFORCEMENT	51	68	72	72	72	72	72

BARS LARGER THAN #9 SHALL BE SPLICED USING MECHANICAL CONNECTIONS

MARK	WIDTH	SIZE		REINFORCING		COMMENTS
		LENGTH	DEPTH	REINFORCING	GRADE	
F48	4'-0"	4'-0"	1'-0"	4 - #5 EA WAY BOT	A615	
F48P	4'-0"	4'-0"	1'-0"	4 - #5 EA WAY BOT	A615	
F60	5'-0"	5'-0"	1'-6"	5 - #6 EA WAY BOT	A615	
F60A	5'-0"	5'-0"	2'-0"	6 - #6 EA WAY TOP & BOT	A615	
F72	6'-0"	6'-0"	1'-6"	6 - #6 EA WAY BOT	A615	
F72A	6'-0"	6'-0"	2'-0"	8 - #6 EA WAY TOP & BOT	A615	TOP BARS TO HAVE STD HOOKS AT ENDS
F72B	6'-0"	6'-0"	1'-0"	6 - #6 EA WAY BOT	A615	TOP BARS TO HAVE STD HOOKS AT ENDS
F72C	6'-0"	6'-0"	1'-0"	6 - #6 EA WAY TOP & BOT	A615	TOP BARS TO HAVE STD HOOKS AT ENDS
F84	7'-0"	7'-0"	2'-0"	9 - #6 EA WAY BOT	A615	
F84A	7'-0"	7'-0"	2'-0"	9 - #6 EA WAY TOP & BOT	A615	TOP BARS TO HAVE STD HOOKS AT ENDS
F84P	7'-0"	7'-0"	2'-0"	9 - #6 EA WAY TOP & BOT	A615	TOP BARS TO HAVE STD HOOKS AT ENDS
F96	8'-0"	8'-0"	2'-0"	7 - #7 EA WAY BOT	A615	
F96A	8'-0"	8'-0"	2'-0"	7 - #7 EA WAY TOP & BOT	A615	TOP BARS TO HAVE STD HOOKS AT ENDS
F276	23'-0"	21'-0"	2'-9"	#5 @ 9" OC EA WAY TOP & BOT	A615	TOP BARS TO HAVE STD HOOKS AT ENDS

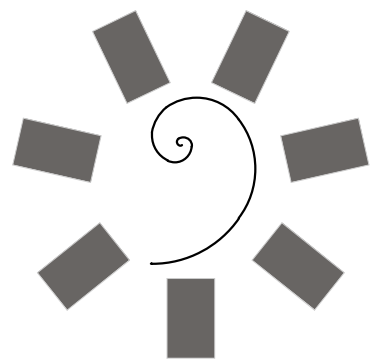
MARK	WIDTH	DEPTH	REINFORCING		COMMENTS
			CONTINUOUS	TRANSVERSE	
CF16	4'-0"	1'-0"	3 - #4	#4 @ 48" OC	
CF18	1'-6"	1'-0"	3 - #4	#4 @ 48" OC	
CF24	2'-0"	1'-0"	3 - #4	#4 @ 48" OC	
CF24B	2'-0"	1'-0"	3 - #4 TOP & BOT	#4 @ 48" OC TOP & BOT	
CF36	3'-0"	1'-0"	4 - #4	#4 @ 24" OC	
CF48	4'-0"	1'-0"	4 - #5 TOP & BOT	#5 @ 12" OC TOP & BOT	
CF60	7'-0"	2'-9"	6 - #6	#6 @ 9" OC	TOP BARS TO HAVE STD HOOKS AT ENDS



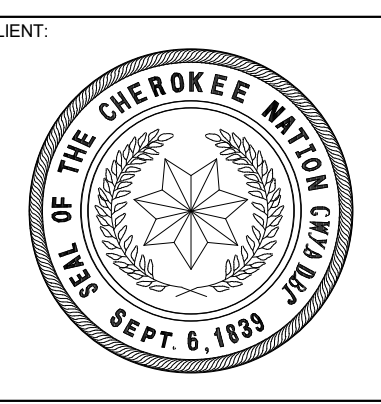
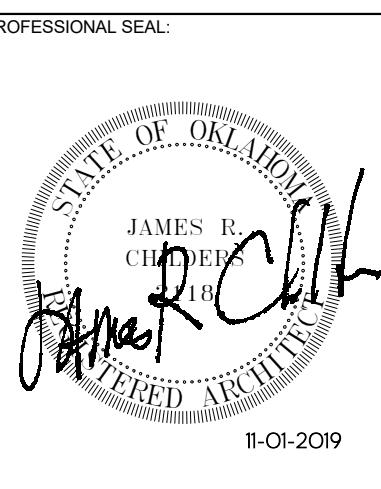
WILMA P. MANKILLER HEALTH CENTER EXPANSION

BID PACKAGE 01 (DEMOLITION / STEEL / FOUNDATIONS)

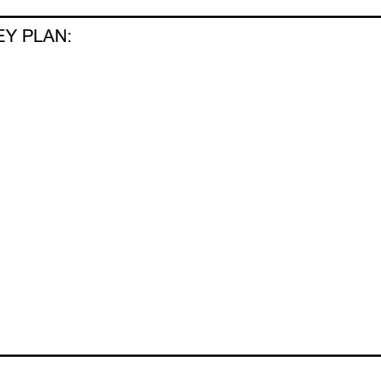
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CS101	DEMOLITION PLAN
CS102	DEMOLITION PLAN
CE100	EROSION CONTROL SITE PLAN
CE500	EROSION CONTROL DETAILS
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A0.01	OVERALL BUILDING DEMOLITION PLAN
STRUCTURAL	
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S0.03	GENERAL STRUCTURAL NOTES AND SPECIAL INSPECTIONS
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S5.41	VERTICAL CIRCULATION DETAILS
S5.51	STEEL DETAILS
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S8.07	SIDEPLATE MISC DETAILS AND COORDINATION ITEMS
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Grand total: 54	



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**WILMA P. MANKILLER HEALTH CENTER
EXPANSION**
 STILWELL, OKLAHOMA



PROJECT PHASE:
BID PACKAGE 01

#	DATE	REVISIONS DESCRIPTION

DATE: 11-01-19 JOB NUMBER: 18-01.01

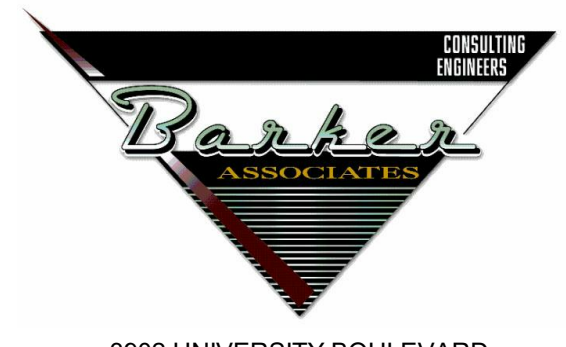
SHEET NUMBER:
G0.01

COVER / INDEX



1836 SOUTH BALTIMORE AVE.
TULSA, OK 74119
(539) 664-4618

MECHANICAL / ELECTRICAL / PLUMBING ENGINEER



3902 UNIVERSITY BOULEVARD
DURANT, OK 74701
(580) 931-9045

CIVIL ENGINEER



4700 LINCOLN ROAD NE, SUITE 102
ALBUQUERQUE, NM 87109
(505) 344-4080

STRUCTURAL ENGINEER



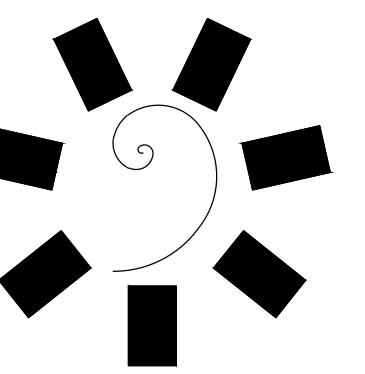
808 TRAVIS STREET, SUITE 200
HOUSTON, TX 77002
(281) 589-5900

FIRE PROTECTION / LIFE SAFETY



1316 E 35TH PLACE, SUITE 100
TULSA, OK 74105
(918) 382-9120

EQUIPMENT PLANNER



**James R. Childers
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45 South 4th Street
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PROFESSIONAL SEAL:



CONSULTANT LOGO:



CLIENT:



**WILMA P. MANKILLER HEALTH CENTER
EXPANSION**
STILWELL, OKLAHOMA

GENERAL NOTES

- THE CONTRACTOR SHALL HAVE EXISTING UTILITIES LOCATED PRIOR TO CONSTRUCTION. CONTRACTOR SHALL CALL "OKIE" 1-800-522-6543 IN ADDITION TO DIRECT NOTIFICATION. CONTRACTOR SHALL BRACE UTILITY POLES AS NECESSARY. UTILITIES DAMAGED BY THE CONTRACTOR SHALL BE REPAIRED TO THE UTILITY OWNER'S SPECIFICATIONS BY THE CONTRACTOR AT NO COST TO THE OWNER.
- THE CONTRACTOR SHALL ESTABLISH, INSTALL, OPERATE, AND MAINTAIN COMPLETE AND ADEQUATE AND SAFE TRAFFIC CONTROLS DURING THE ENTIRE CONSTRUCTION PERIOD. ALL TRAFFIC CONTROL DEVICES SHALL BE APPROVED BY THE ENGINEER.
- ALL DIMENSIONS OR ELEVATIONS WITH ± SHALL BE CONFIRMED BY THE CONTRACTOR.
- ALL DIMENSIONS OF EXISTING STRUCTURES AND EQUIPMENT SHALL BE VERIFIED BY THE CONTRACTOR. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER & OWNER.
- TOPSOIL IN THE DISTURBED AREAS SHALL BE REMOVED, STOCKPILED, AND RESTORED AFTER CONSTRUCTION OPERATIONS, IN ACCORDANCE WITH PROJECT SPECIFICATIONS. ALL EXCESS TOPSOIL SHALL BE CONSIDERED WASTE AND STOCKPILED ON-SITE BY THE CONTRACTOR, UNLESS OTHERWISE NOTED.
- ALL DISTURBED ROADWAY AND DRIVEWAY SURFACES SHALL BE RESTORED TO THEIR PRE-CONSTRUCTION CONDITION.
- FALL PROTECTION AROUND ALL OPENINGS AND EXCAVATION SHALL BE MAINTAINED AT ALL TIMES.
- NORTH ARROWS SHOWN ON DRAWINGS INDICATE LOCAL COORDINATE SYSTEM ESTABLISHED BY THE SURVEYOR, UNLESS OTHERWISE NOTED.
- TRENCH SAFETY AND SHORING IN ACCORDANCE WITH CURRENT OSHA REGULATIONS SHALL BE EMPLOYED BY CONTRACTOR AT ALL TIMES.
- IF AT ANY POINT CONSTRUCTION ACTIVITIES EXPOSE ARCHEOLOGICAL MATERIALS SUCH AS CHIPPED STONE, TOOLS, POTTERY, BONE, HISTORIC CROCKERY, GLASS, METAL ITEMS OR BUILDING MATERIALS, THE OKLAHOMA ARCHEOLOGICAL SURVEY STATE ARCHEOLOGIST, KARY L. STACKELBECK, SHALL BE CONTACTED IMMEDIATELY AT 405-325-7211.
- ALL STATIONS SHOWN ON THE PLANS ARE CENTERLINE STATIONS UNLESS NOTED OTHERWISE.
- THE TOPOGRAPHIC SURVEY WAS COMPLETED BY NATIVE PLAINS SURVEYING & MAPPING, LLC. ALL EXISTING INFORMATION IS SHOWN AS ACCURATELY AS POSSIBLE BASE UPON FIELD RECONNAISSANCE AND RESEARCH. CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL EXISTING INFORMATION. IF CONTRACTOR BELIEVES EXISTING INFORMATION IS INACCURATE, THE CONTRACTOR MAY HAVE A NEW SURVEY COMPLETED AT NO ADDITIONAL COST TO THE OWNER, ARCHITECT, OR ENGINEER.
- DIMENSIONS SHOWN ARE TO BACK OF CURB OR CENTERLINE OF PIPE UNLESS NOTED OTHERWISE.
- CONTRACTOR SHALL REVIEW & COORDINATE W/ ARCHITECTURAL, MECH., ELEC., & PLUMBING DISCIPLINES DRAWINGS, SPEC'S & DETAILS. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER. IN THE EVENT THAT THE ARCH. &/OR CONTRACTOR DEVIATES CONSTRUCTION FROM THESE PLANS W/O THE EXPRESS WRITTEN APPROVAL OF THE ENGINEER, THE ARCH. &/OR CONTRACTOR ASSUMES FULL RESPONSIBILITY FOR THOSE MODIFICATIONS.

EROSION CONTROL NOTES:

- SILT FENCE SHALL BE MAINTAINED AND SEDIMENT BUILDUP REGULARLY REMOVED UNTIL PAVING OPERATIONS ARE COMPLETE AND/OR SEEDING IS IN PLACE AND 75% VEGETATION STABILIZATION IS OBTAINED.
- ALL TREES, BRUSH, AND OTHER DEBRIS THAT MIGHT INTERFERE WITH THE FLOW OF WATER IS TO BE CLEANED OUT TO THE RIGHT-OF-WAY LINE AT EACH STRUCTURE, IN A MANNER APPROVED BY THE ENGINEER.
- ALL FLOW LINES THAT ARE TO BE FILLED SHALL BE THOROUGHLY COMPACTED TO 95% STANDARD PROCTOR DENSITY BEFORE CONSTRUCTION OR EXTENSION OF DRAINAGE STRUCTURES.
- IN ORDER TO ALLEVIATE DUST CONDITIONS DURING GRADING OPERATIONS, AND AFTER GRADING OPERATIONS ARE COMPLETED, BUT BEFORE PAVEMENT AND/OR PERMANENT EROSION CONTROL WORK IS COMPLETED, THE CONTRACTOR SHALL SPRINKLE GRADING AT INTERVALS APPROVED BY THE OWNERS REPRESENTATIVE.
- ALL UNPAVED DISTURBED AREAS SHALL RECEIVE SLAB SOD FOR PERMANENT EROSION CONTROL, UNLESS NOTED OTHERWISE. THIS SHALL INCLUDE FERTILIZER, WATERING & MOWING AS REQUIRED TO ESTABLISH A VIABLE TURF.
- AT THE BEGINNING OF THE TURF OPERATIONS, ANY AREAS INCLUDED IN PLANNED QUANTITIES THAT HAVE GROWN A SATISFACTORY VOLUNTEER TURF OF PERENNIAL GRASS AS DETERMINED BY THE OWNER'S REPRESENTATIVE, SHALL BE FERTILIZED AND WATERED BUT SHALL NOT BE SEEDED, SODDED OR SPRIGGED.
- VEGETATIVE MULCH AND SEEDING SHALL BE UTILIZED FOR TEMPORARY EROSION CONTROL.
- SEED: THE FOLLOWING KINDS OF SEEDS, AT ACRES-RATES INDICATED BELOW, SHALL BE PLANTED ON THE AREAS DESIGNATED FOR SEEDING.

TEMPORARY SEEDING	
KINDS OF SEED TO BE FURNISHED	QUANTITY PER ACRE
COOL SEASON MIX:	
PERENNIAL RYEGRASS (LOLIUM PERENNE)	20 LBS. OF SEED
CRIMSON CLOVER (TRIFOLIUM INCARNATUM)	12 LBS. OF SEED
WARM SEASON MIX:	
KOREAN LESPEDEZA (LESPEDEZA STRIATA)	12 LBS. OF SEED
CRIMSON CLOVER (TRIFOLIUM INCARNATUM)	20 LBS. OF SEED
LITTLE BLUESTEM (ANDROPOGON SCOPARIUS)	12 LBS. OF SEED
COMMON BERMUDA (CYNODON DACTYLON)	4 LBS. OF SEED

VEGETATIVE MULCHING: THE VEGETATIVE MULCH SHALL BE ANCHORED IN ACCORDANCE WITH THE "ADHESIVE SPRAY METHOD", AS SPECIFIED IN 233.04(b) OF THE ODOT STANDARD SPECIFICATIONS.

SEASONAL PLANTING RESTRICTIONS

THE PLANTING OF SPRIGGING SHALL BE RESTRICTED TO THE PERIOD FROM APRIL 1ST TO JUNE 30TH.

THE PLANTING OF TEMPORARY SEEDS (COOL SEASON MIX) SHALL BE RESTRICTED TO THE PERIOD FROM SEPTEMBER 1ST TO NOVEMBER 15TH.

THE PLANTING OF TEMPORARY SEEDS (WARM SEASON MIX) SHALL BE RESTRICTED TO THE PERIOD FROM MARCH 15TH TO JUNE 30TH.

AREAS ON WHICH SALVAGED TOPSOIL IS TO BE REPLACED SHALL HAVE 0-46-0 FERTILIZER APPLIED, AT THE RATE OF 150 LBS. PER ACRE, JUST PRIOR TO THE REPLACEMENT OF SALVAGED TOPSOIL.

SITE WORK NOTES

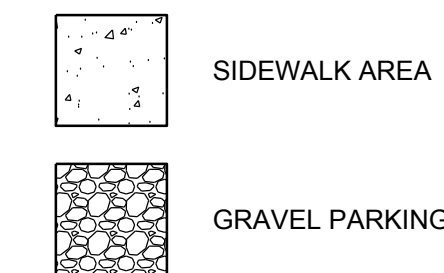
- ALL EARTHWORK & PAVING MATERIALS & METHODS SHALL CONFORM WITH OKLAHOMA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION, LATEST REVISION, UNLESS OTHERWISE NOTED.
- CONTRACTOR SHALL REVIEW GEOTECHNICAL REPORT PREPARED BY BUILDING & EARTH, DATED AUGUST 30, 2018. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER. THIS REPORT SHOULD BE CONSIDERED A PART OF THESE CONSTRUCTION DOCUMENTS.
- ONLY REMOVE TREES THAT DIRECTLY INTERFERE WITH CONSTRUCTION. CONTRACTOR SHALL LIMIT CLEARING & GRUBBING TO BUILDING & PARKING AREA FOOTPRINT, AS MUCH AS POSSIBLE.
- CONTRACTOR SHALL DISPOSE OF TREES, STUMPS, DEBRIS, ETC. OFF SITE IN A MANNER APPROVED BY THE OWNER.
- ALL AREAS TO RECEIVE PAVING SHALL BE STRIPPED OF VEGETATION, TOPSOIL, SOFT OR OTHERWISE SUITABLE MATERIAL. THIS WOULD INCLUDE AREAS IDENTIFIED FOR UNDERCUT. AREA SHALL BE SCARIFIED TO A DEPTH OF 8 INCHES, MOISTURE CONDITIONED TO A RANGE OF 1% BELOW TO 3% ABOVE THE MATERIAL'S OPTIMUM MOISTURE CONTENT, & COMPACTED TO A DENSITY OF AT LEAST 95% OF THE STANDARD PROCTOR (ASTM D 698) MAXIMUM DRY DENSITY. SUBGRADE SHALL BE PROOF ROLLED WITH A ROLLER OR TRUCK (GROSS WEIGHT OF 25 TONS OR MORE). SOFT AREAS SHALL BE EXCAVATED & REPLACED WITH SUITABLE MATERIAL. PROOF ROLLING SHALL BE WITNESSED BY OWNER'S REPRESENTATIVE. OWNER SHALL DETERMINE SUITABILITY OF SUBGRADE. REFER TO GEOTECHNICAL RECOMMENDATIONS.
- WHERE LIMESTONE IS EXPOSED AT FINISHED SUBGRADE, IT IS RECOMMENDED TO UNDERCUT THE LIMESTONE ROCK UNTIL A LEVEL THAT WILL ALLOW FOR PLACEMENT OF AT LEAST 8" OF STRUCTURAL FILL TO PROVIDE FOR UNIFORM SUBGRADE CONDITIONS ACROSS PAVEMENT AREAS.
- REMOVE ANY STUMPS, ROOTS LARGER THAN 2 INCHES IN DIAMETER, ROCKS LARGER THAN 3 INCHES AND ANY MATTED ROOTS, TO A DEPTH OF 18 INCHES BELOW ORIGINAL GROUND SURFACE.
- SELECT FILL SHALL BE COMPOSED OF MATERIAL WITH MAXIMUM DRY DENSITY IN EXCESS OF 100 POUNDS PER CUBIC FOOT, PLASTICITY INDEX (PI) LESS THAN 18, AND A LIQUID LIMIT (LL) LESS THAN 40. STRUCTURAL FILL SHOULD BE FREE OF ANY ORGANICS, SHOULD NOT CONTAIN ROCK FRAGMENTS GREATER THAN 3 INCHES IN ANY DIMENSION, AND SHOULD BE PROPERLY MOISTURE CONDITIONED PRIOR TO USE AS SELECT FILL. SELECT FILL SHOULD BE COMPACTED TO A MINIMUM OF 95% OF THE STANDARD PROCTOR MAXIMUM DRY DENSITY AND WITHIN ±2% OF THE OPTIMUM MOISTURE AS DETERMINED BY ASTM D 698. THE FILL MATERIAL SHOULD BE SPREAD IN HORIZONTAL LIFTS STARTING AT THE LOWEST ELEVATION. THE LIFTS SHOULD NOT EXCEED 8 TO 12 INCHES IN LOOSE LIFT THICKNESS.
- REUSE OF ON SITE SOILS AS FILL IS NOT RECOMMENDED BELOW PLANNED BUILDING OR PAVEMENT AREAS.
- ALL DISTURBED AREAS THAT ARE NOT PAVED ARE TO RECEIVE SLAB SODDING.
- EARTHWORKS SHALL BE PERFORMED IN SUCH A MANNER TO MINIMIZE PONDING WATER ON THE SUBGRADE. SITE SHALL MAINTAIN DRAINAGE AT ALL TIMES. MOISTURE CONTENT OF SOIL SHOULD BE MAINTAINED NEAR OPTIMUM DURING CONSTRUCTION.
- ALL ROCKS AND DEBRIS SHALL BE REMOVED FROM ALL DRAINS PRIOR TO FINAL INSPECTION.
- ROADSIDE HAZARDS SHALL BE COMPLETELY BARRICADED AROUND THEIR PERIMETER FOR THE SAFETY OF PEDESTRIANS AND VEHICLES.
- ONLY THE AMOUNT OF TRENCH THAT CAN BE BACK FILLED OR SURFACED IN (2) DAYS SHALL BE ALLOWED OPEN UNLESS APPROVED BY OWNERS REPRESENTATIVE.
- ANY EXISTING FOUNDATIONS OR FOOTINGS SHALL BE REMOVED FULL DEPTH, & BACKFILLED WITH APPROVED COMPACTED MATERIAL.
- THIS BID PACKAGE IS INTENDED TO INCLUDE ONLY DEMOLITION RELATED TASKS INCLUDING BUT NOT LIMITED TO EROSION CONTROL ITEMS, STRIPPING/SALVAGING TOP SOIL, DEMOLITION, SUBGRADE MAINTENANCE.

UTILITY NOTES

- THE CONTRACTOR SHALL PROTECT ALL EXISTING UTILITIES AND SERVICES FROM DAMAGE. UTILITIES SHALL REMAIN IN SERVICE AT ALL TIMES, AND ANY DISRUPTION OF SERVICE SHALL BE AT THE CONTRACTOR'S SOLE EXPENSE.
- CONTRACTOR SHALL VERIFY EXISTING PIPE SIZE, TYPE AND LOCATION TO INSURE PROPER CONNECTION.
- THE PLANS HAVE BEEN PREPARED TO SHOW THE APPROXIMATE LOCATION OF EXISTING KNOWN UTILITIES. THE CONTRACTOR SHALL CONTACT OKIE, EACH RESPECTIVE UTILITY COMPANY AND THE PROJECT OWNER TO DETERMINE THE EXACT LOCATION OF UNDERGROUND UTILITIES PRIOR TO EXCAVATION. ANY CHANGE IN ALIGNMENT OR GRADE CAUSED BY INTERFERING UTILITIES SHALL BE MADE BY THE CONTRACTOR AT NO COST TO THE OWNER AND THE ENGINEER NOTIFIED.
- DEPTHS OF ANY EXISTING UTILITIES SHOWN ON THE PLANS ARE APPROXIMATE. CONTRACTOR SHALL AT HIS OWN EXPENSE UNCOVER AND VERIFY THE LOCATION AND ELEVATION OF EXISTING UTILITIES IN ADVANCE OF THE CONSTRUCTION.

PROPOSED SITE LEGEND

- SD — SD — STORM WATER LINE
- W — W — WATER LINE
- UGE — UGE — UNDERGROUND ELECTRIC LINE
- GAS — GAS — GAS LINE
- SS — SS — SANITARY SEWER LINE
- — — — — CENTER LINE OF DRIVES
- xxx — — — — PROPOSED CONTOURS
- - - - - CONTOURS
- (WM) WATER METER
- ⬮ GATE VALVE



KEY PLAN:

PROJECT PHASE:

BID PACKAGE 01

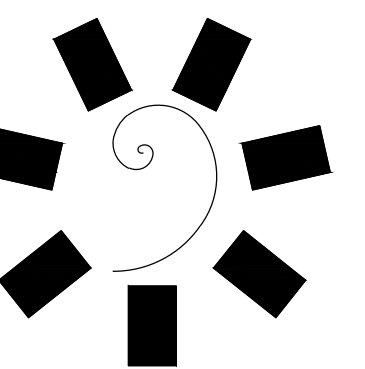
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#	DESCRIPTION

DATE: 11.01.19 JOB NUMBER: 18-01.01

SHEET NUMBER:

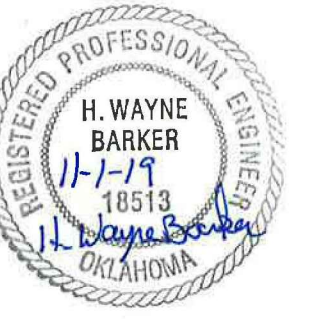
C002

**GENERAL
NOTES**



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PROFESSIONAL SEAL



CONSULTANT LOGO



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580.931.9045
OK, CA, MO, KS
EXP. 05/30/2020

CLIENT



**WILMA P. MANKILLER HEALTH CENTER
EXPANSION**
STILWELL, OKLAHOMA

KEY PLAN

PROJECT PHASE

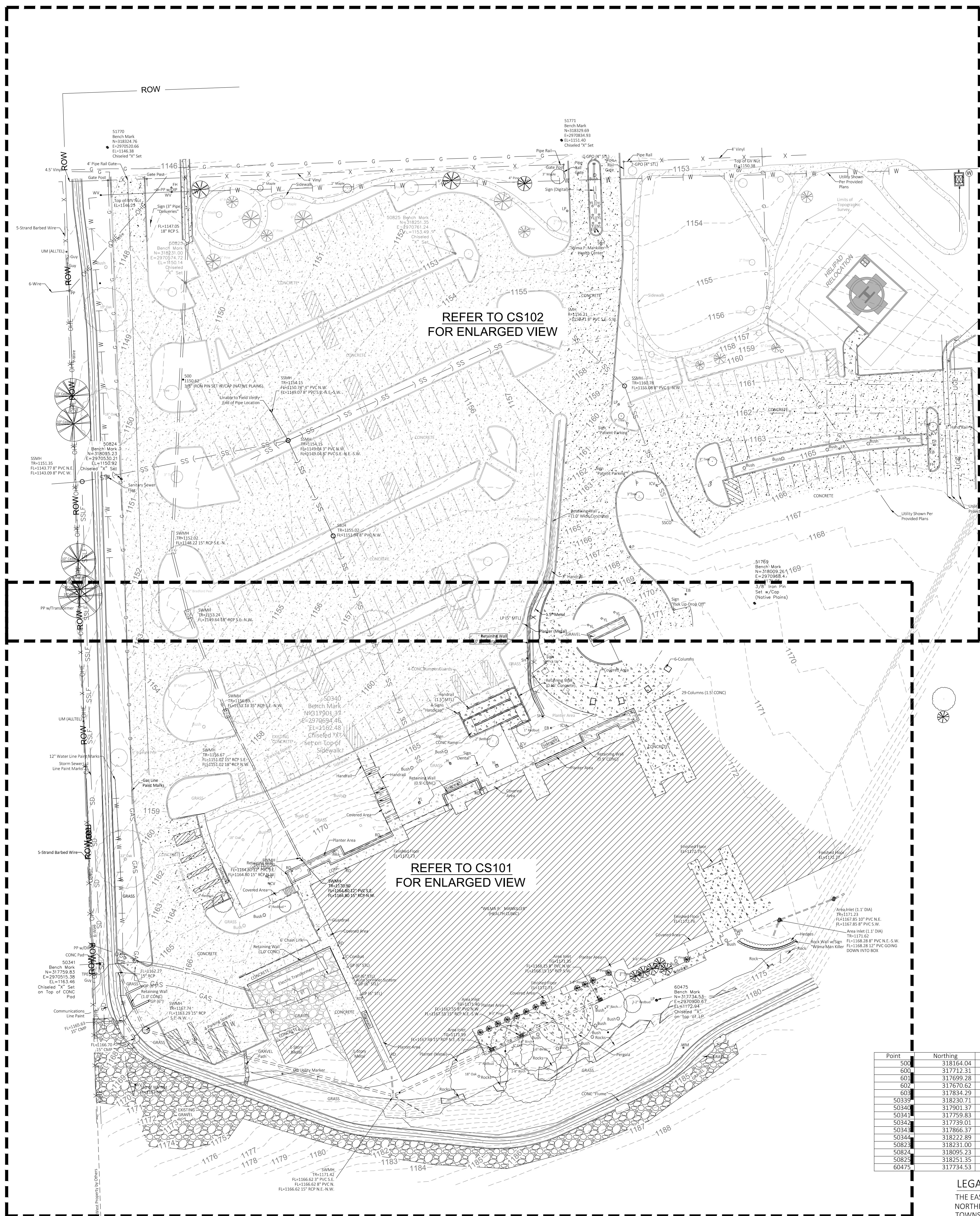
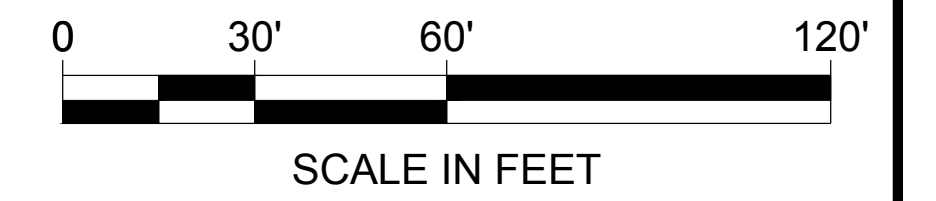
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#	DATE	REVISIONS	DESCRIPTION

DATE: 11.01.19 JOB NUMBER: 18-01.01

SHEET NUMBER:

CS100
EXISTING SITE PLAN



SYMBOL LEGEND

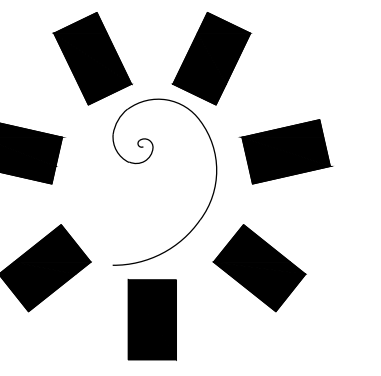
- ⊕ - Column (All Types)
- ⊖ - Electric Box
- ⊕ - Electric Meter
- ⊕ - Electric Transformer
- ⊕ - Fire Hydrant
- ⊕ - Gas Meter
- ⊕ - Gas Line Paint Marks
- ⊕ - Guy Anchor
- ⊕ - Handicap Parking
- ⊕ - Irrigation Control Valve
- ⊕ - Light Pole (All Types)
- ⊕ - Power Pole
- ⊕ - Roof Drain
- ⊕ - Sign (All Types)
- ⊕ - Sanitary Sewer Cleanout
- ⊕ - Sanitary Sewer Lamphole
- ⊕ - Sanitary Sewer Manhole
- ⊕ - Sprinkler Head
- ⊕ - Storm Sewer Area Inlet
- ⊕ - Storm Sewer Manhole
- ⊕ - Telephone Pedestal
- ⊕ - Underground Utility Marker
- ⊕ - Water Meter
- ⊕ - Water Valve
- ⊕ - Yard Light (All Types)
- ⊕ - Deciduous Tree (All Types)
- ⊕ - Coniferous Tree (All Types)
- ⊕ - Bush (All Types)
- X - X - X - Fence Line (All Types)
- GAS - GAS - GAS - Gas Line Paint Marks
- OHE - OHE - OHE - Overhead Electric
- SS - SS - SS - Sanitary Sewer Line
- SD - SD - SD - Storm Sewer Line
- SD - SD - SD - Storm Sewer Line
- T - T - T - Communication Line Paint Marks
- UGE - UGE - UGE - Electric Line Paint Marks
- W - W - W - Water Line Paint Marks
- ⊕ - Bench Mark
- ⊕ - Survey Control Point Found
- ⊕ - Survey Control Point Set
- ⊕ - Sprinkler Head
- ⊕ - Corrugated Metal Pipe
- ⊕ - Polyurethane Pipe
- ⊕ - Reinforced Concrete Pipe

SURVEY CONTROL

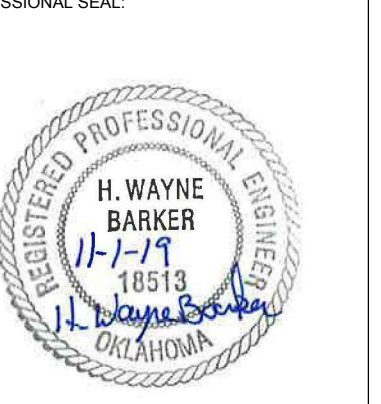
Point	Northing	Easting	Elevation	Description
500	318164.04	2970573.01	1150.62	3/8" Iron Pin Set w/Cap (NP Control)
600	317712.31	2970867.05	1172.51	60d Nail Set
601	317699.28	2970795.96	1171.75	60d Nail Set
602	317670.62	2970722.82	1171.56	60d Nail Set
603	317634.29	2970648.60	1170.44	60d Nail Set
50339	318230.71	2970515.56	1148.96	3/8" Iron Pin Set w/Cap (NP Control)
50340	317901.37	2970694.46	1162.48	Bench Mark
50341	317759.83	2970515.38	1163.46	Bench Mark
50342	317739.01	2970792.89	1180.63	3/8" Iron Pin Set w/Cap (NP Control)
50343	317866.37	2971153.74	1172.60	3/8" Iron Pin Set w/Cap (NP Control)
50344	318222.89	2971150.00	1157.02	3/8" Iron Pin Set w/Cap (NP Control)
50823	318231.00	2970574.72	1150.14	Bench Mark
50824	318095.23	2970530.21	1150.92	Bench Mark
50825	318251.35	2970761.24	1153.49	Bench Mark
60475	317734.53	2970900.67	1172.94	Bench Mark

POINT	NORTHING	EASTING	ELEVATION	DESCRIPTION
500	318164.04	2970573.01	1150.62	3/8" Iron Pin Set w/Cap (Native Plants)
50339	318230.71	2970515.56	1148.96	Bench Mark (Native Plants)
51769	318095.23	2970530.21	1150.92	Bench Mark (Native Plants)
51775	318251.35	2970761.24	1153.49	Bench Mark (Native Plants)

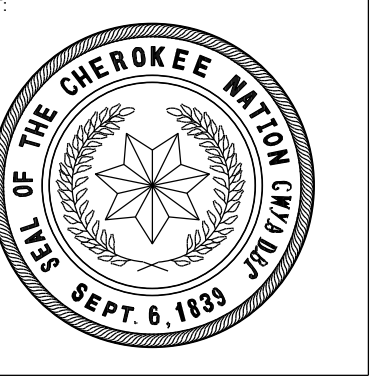
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NORTHEAST QUARTER (E/2 NW/4 NE/4 OF SECTION 35,
TOWNSHIP 16 NORTH, RANGE 25 EAST OF THE INDIAN BASE
AND MERIDIAN, ADAIR COUNTY, STATE OF OKLAHOMA.



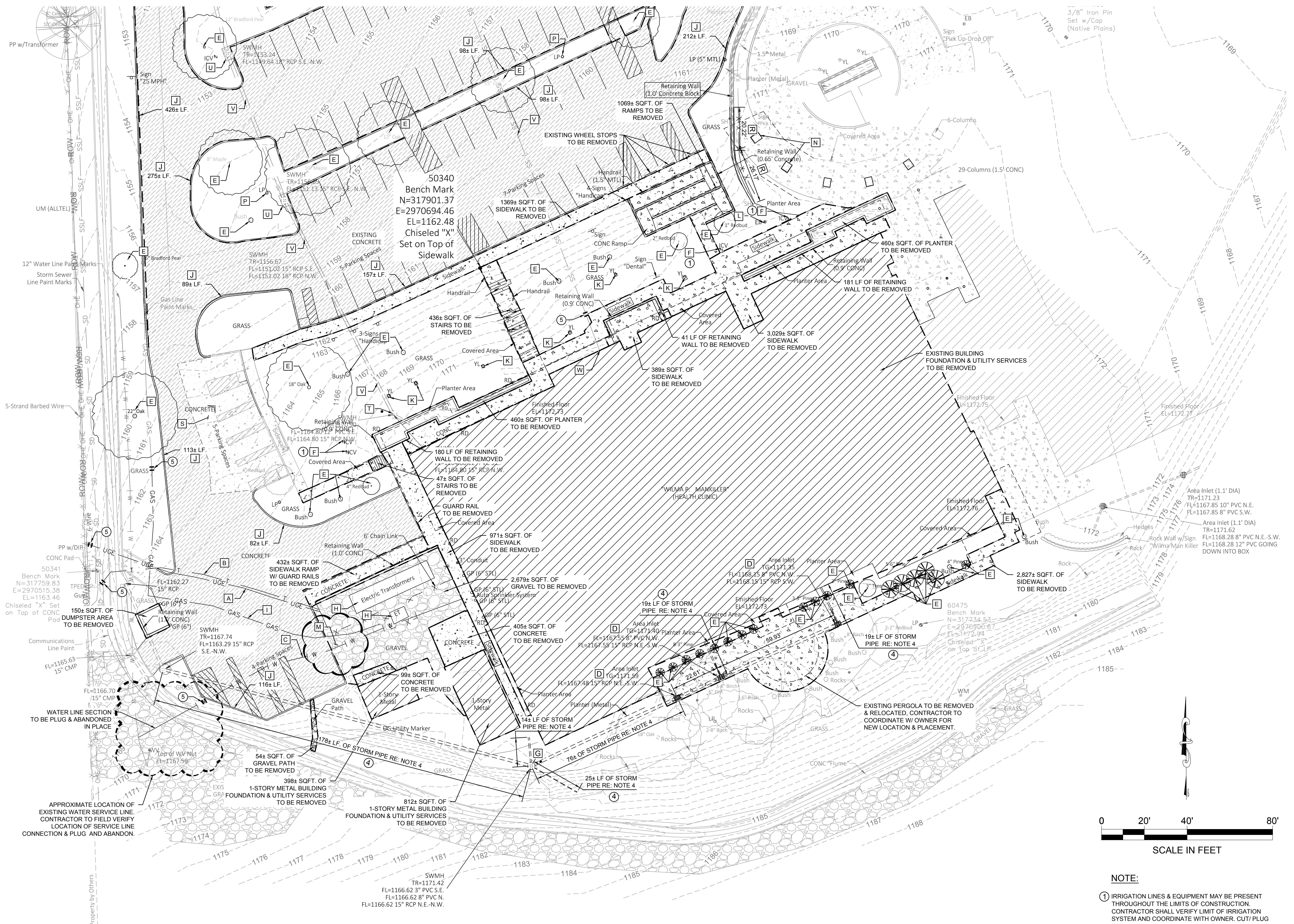
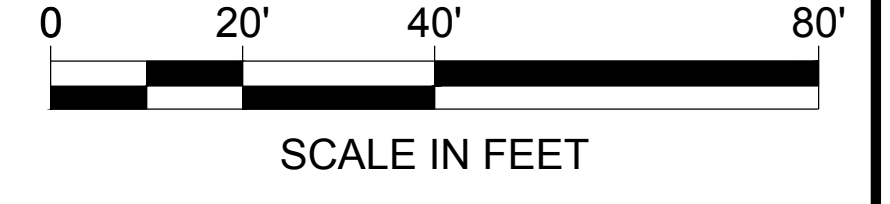
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EXP. 05/30/2020



**WILMA P. MANKILLER HEALTH CENTER
EXPANSION
STILWELL, OKLAHOMA**



LEGEND

- APROX. LIMITS OF DEMOLITION.
- DEMOLITION LEGEND:**
- A 114± LF. OF UNDERGROUND ELECTRIC LINE TO BE REMOVED AND RELOCATED FROM UTILITY CUT OFF POINT TO EDGE OF BUILDING.
 - B 106± LF. OF TELEPHONE LINE TO BE REMOVED AND RELOCATED FROM UTILITY CUT OFF POINT TO EDGE OF BUILDING.
 - C 131± LF. OF WATER LINE TO BE REMOVED AND RELOCATED FROM UTILITY CUT OFF POINT TO EDGE OF BUILDING.
 - D EXISTING INLET TO REMAIN. RE: NOTE 4
 - E EXISTING SITE VEGETATION TO BE REMOVED. RE: NOTE 4
 - F EXISTING IRRIGATION VALVE TO BE REMOVED. RE: NOTE 4
 - G EXISTING STORM MANHOLE TO BE REMAIN. RE: NOTE 4
 - H EXISTING HVAC, GENERATOR & ELECTRICAL TRANSFORMER TO BE REMOVED. REFER TO MEP.
 - I 173± OF GAS LINE TO BE REMOVED AND RELOCATED FROM UTILITY CUT OFF POINT TO OUT SIDE OF BUILDING.
 - J EXISTING CURB TO BE REMOVED.
 - K EXISTING YARD LIGHT TO BE REMOVED.
 - L CONTRACTOR TO VERIFY IF EXISTING ELECTRIC BOX NEEDS TO BE REMOVED OR RELOCATED.
 - M APPROXIMATE LOCATION OF EXISTING FIRE HYDRANT. TO REMOVED ONLY AFTER INSTALLATION OF NEW FIRE HYDRANT.
 - N EXISTING COLUMN TO BE REMOVED, WITH CORRESPONDING ROOF SECTION.
 - O EXISTING FLUME TO BE REMOVED.
 - P EXISTING LIGHT POLE TO BE REMOVED.
 - Q EXISTING SITE SIGNAGE TO BE REMOVED/RELOCATE RE: OWNER/ARCH.
 - R EXISTING WALL SECTION TO BE REMOVED.
 - S EXISTING PAVEMENT TO REMAIN.
 - T EXISTING SEWER/STORM MANHOLE TO BE REMAIN AND ADJUSTED TO NEW GRADE IN FUTURE BID PACKAGE.
 - U EXISTING STORM INLET TOP TO BE REMAIN AND ADJUSTED TO NEW GRADE IN FUTURE BID PACKAGE.
 - V EXISTING UNDERGROUND UTILITY TO REMAIN IN PLACE DURING CONSTRUCTION. CONTRACTOR TO ENSURE NOT DISTURB OR DAMAGE DURING CONSTRUCTION.
 - W 18± LF. OF SEWER LINE TO BE REMOVED AND REPLACED FROM UTILITY CUT OFF POINT TO EDGE OF BUILDING.
- 95,152± SQFT. OF EXISTING CONCRETE PAVING TO BE REMOVED.

NOTE:

- 1 IRRIGATION LINES & EQUIPMENT MAY BE PRESENT THROUGHOUT THE LIMITS OF CONSTRUCTION. CONTRACTOR SHALL VERIFY LIMIT OF IRRIGATION SYSTEM AND COORDINATE WITH OWNER, CUT/PLUG EXISTING LINES TO CLEAR CONSTRUCTION AND MAINTAIN OPERATION OF OTHER ZONES.
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- 5 UTILITY CUT OFF POINT SYMBOL. THE BEGINNING POINT OF ANY UTILITY (GAS, WATER, SEWER, & ELECTRIC) LINE TO BE REMOVED.

KEY PLAN

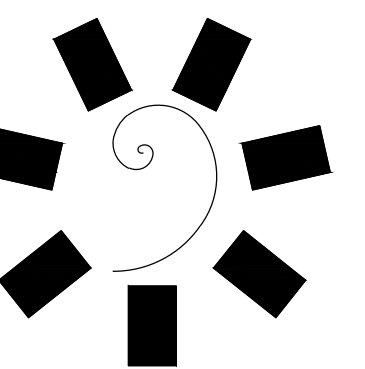
PROJECT PHASE:
BID PACKAGE 01

#	DATE	REVISIONS	DESCRIPTION

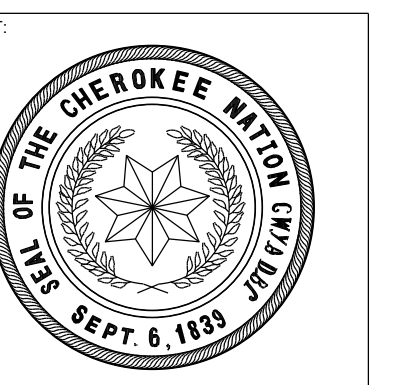
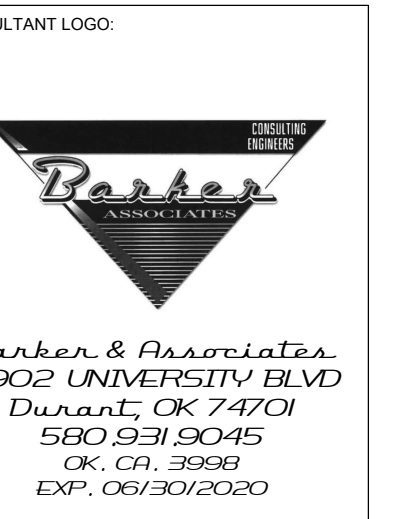
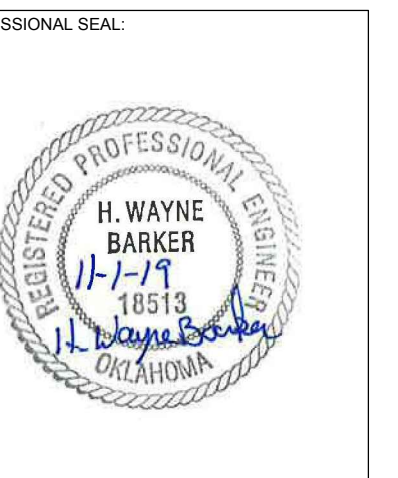
DATE: 11.01.19 JOB NUMBER: 18-01.01

SHEET NUMBER: CS101

DEMOLITION PLAN



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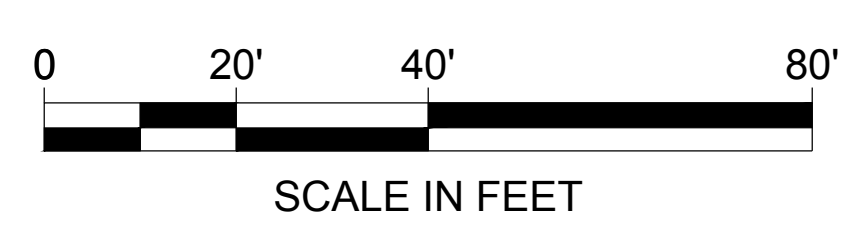
**WILMA P. MANKILLER HEALTH CENTER
EXPANSION**
STILWELL, OKLAHOMA



APPROXIMATE LOCATION OF EXISTING VALVE VAULT & WATER METER. CONTRACTOR TO FIELD VERIFY LOCATIONS & PROTECT DURING CONSTRUCTION.

LEGEND

- APROX. LIMITS OF DEMOLITION.
- DEMOLITION LEGEND:**
- A** 114± LF. OF UNDERGROUND ELECTRIC LINE TO BE REMOVED AND RELOCATED FROM UTILITY CUT OFF POINT TO EDGE OF BUILDING.
- B** 106± LF. OF TELEPHONE LINE TO BE REMOVED AND RELOCATED FROM UTILITY CUT OFF POINT TO EDGE OF BUILDING.
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- H** EXISTING HVAC, GENERATOR & ELECTRICAL TRANSFORMER TO BE REMOVED. REFER TO MEP.
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- 95,152± SQFT. OF EXISTING CONCRETE PAVING TO BE REMOVED.



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- 5 UTILITY CUT OFF POINT SYMBOL, THE BEGINNING POINT OF ANY UTILITY (GAS, WATER, SEWER, & ELECTRIC) LINE TO BE REMOVED.

KEY PLAN

PROJECT PHASE:
BID PACKAGE 01

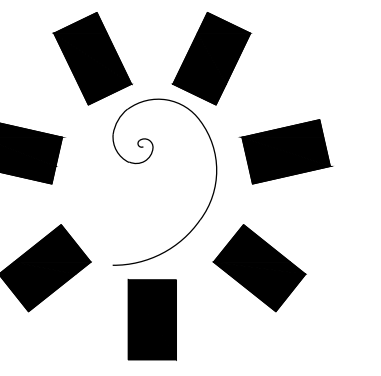
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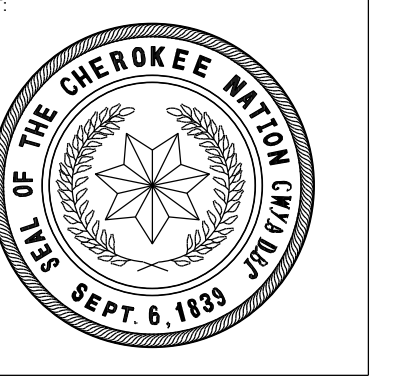
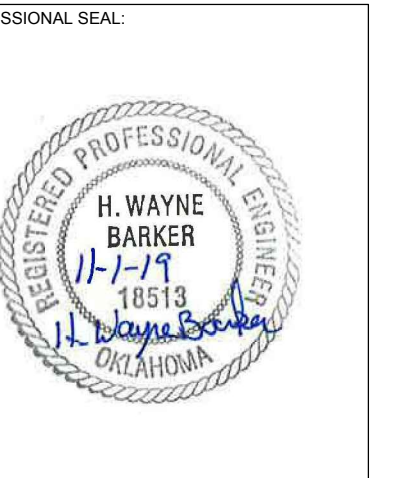
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CS102

DEMOLITION PLAN

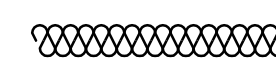
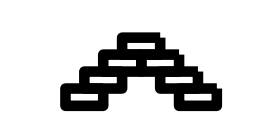
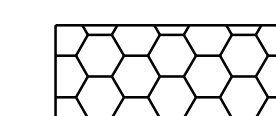




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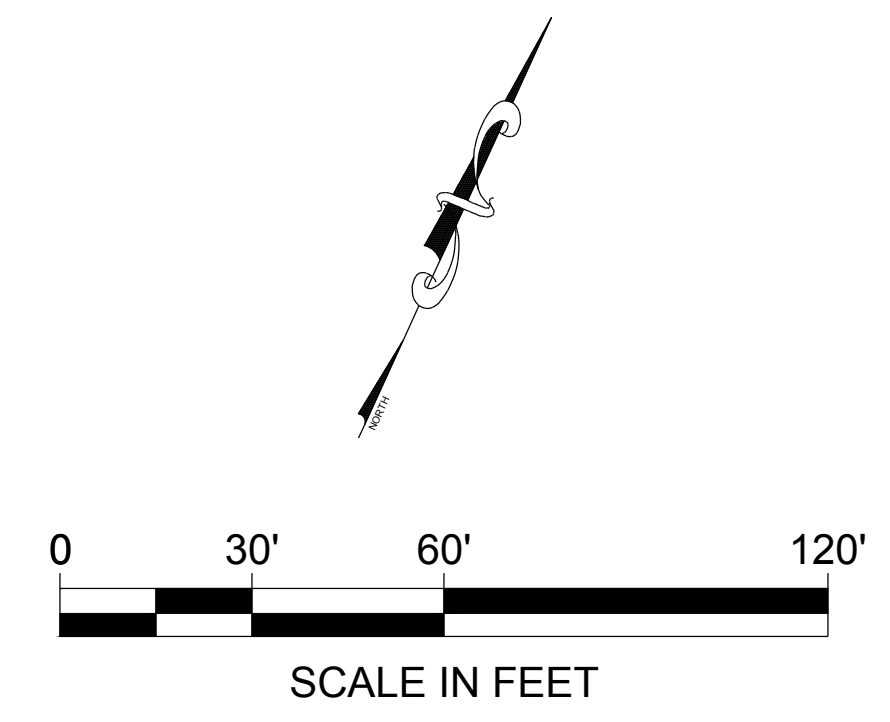
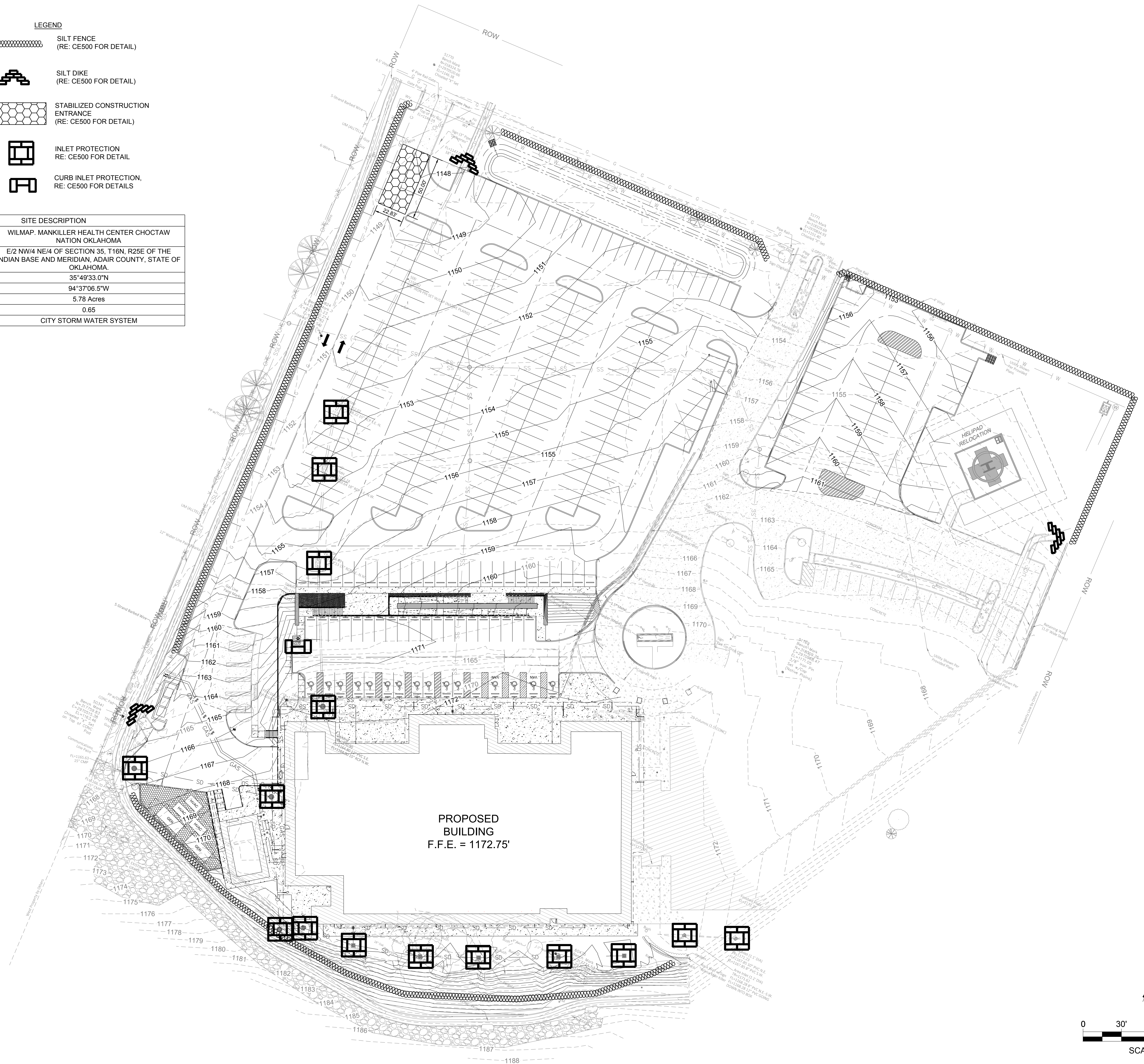


**WILMA P. MANKILLER HEALTH CENTER
EXPANSION**
STILWELL, OKLAHOMA

LEGEND

-  SILT FENCE
(RE: CE500 FOR DETAIL)
-  SILT DIKE
(RE: CE500 FOR DETAIL)
-  STABILIZED CONSTRUCTION
ENTRANCE
(RE: CE500 FOR DETAIL)
-  INLET PROTECTION
RE: CE500 FOR DETAIL
-  CURB INLET PROTECTION
RE: CE500 FOR DETAILS

SITE DESCRIPTION	
PROJECT:	WILMAP. MANKILLER HEALTH CENTER CHOCTAW NATION OKLAHOMA
LEGAL DESCRIPTION	E/2 NW/4 NE/4 OF SECTION 35, T16N, R25E OF THE INDIAN BASE AND MERIDIAN, ADAIR COUNTY, STATE OF OKLAHOMA.
LATITUDE	35°49'33.0"N
LONGITUDE	94°37'06.5"W
DISTURBED AREA	5.78 Acres
RUNOFF CO. EFF.	0.65
RECEIVING WATERS	CITY STORM WATER SYSTEM



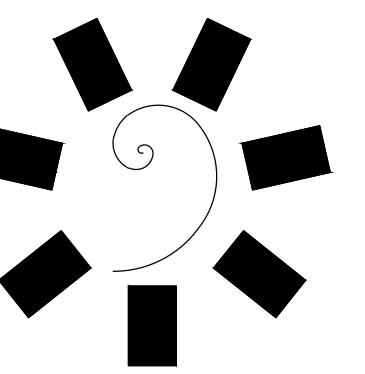
KEY PLAN

PROJECT PHASE:
BID PACKAGE 01

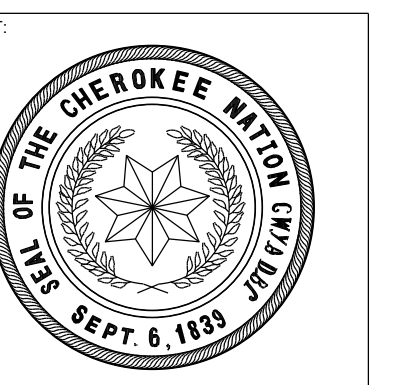
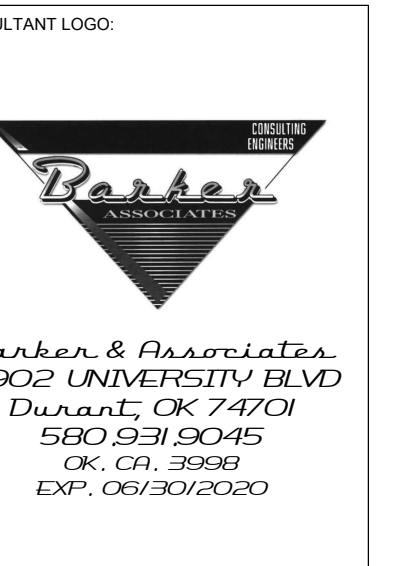
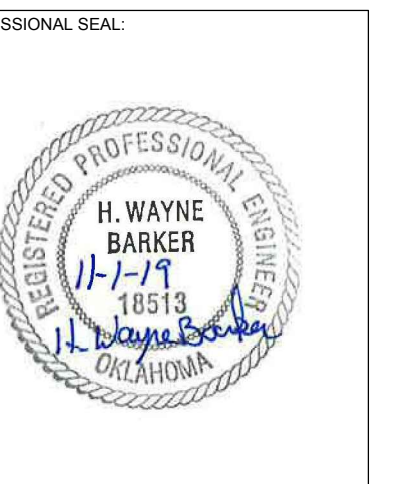
#	DATE	REVISIONS DESCRIPTION

DATE: 11.01.19 JOB NUMBER: 18-01.01

SHEET NUMBER:
CE100
EROSION CONTROL SITE PLAN



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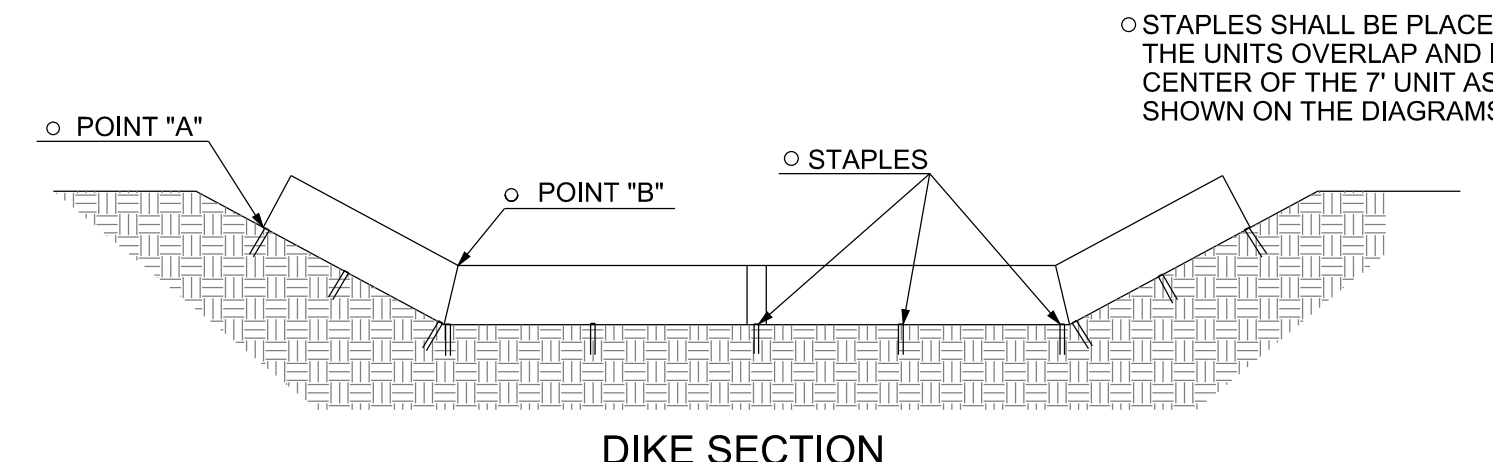
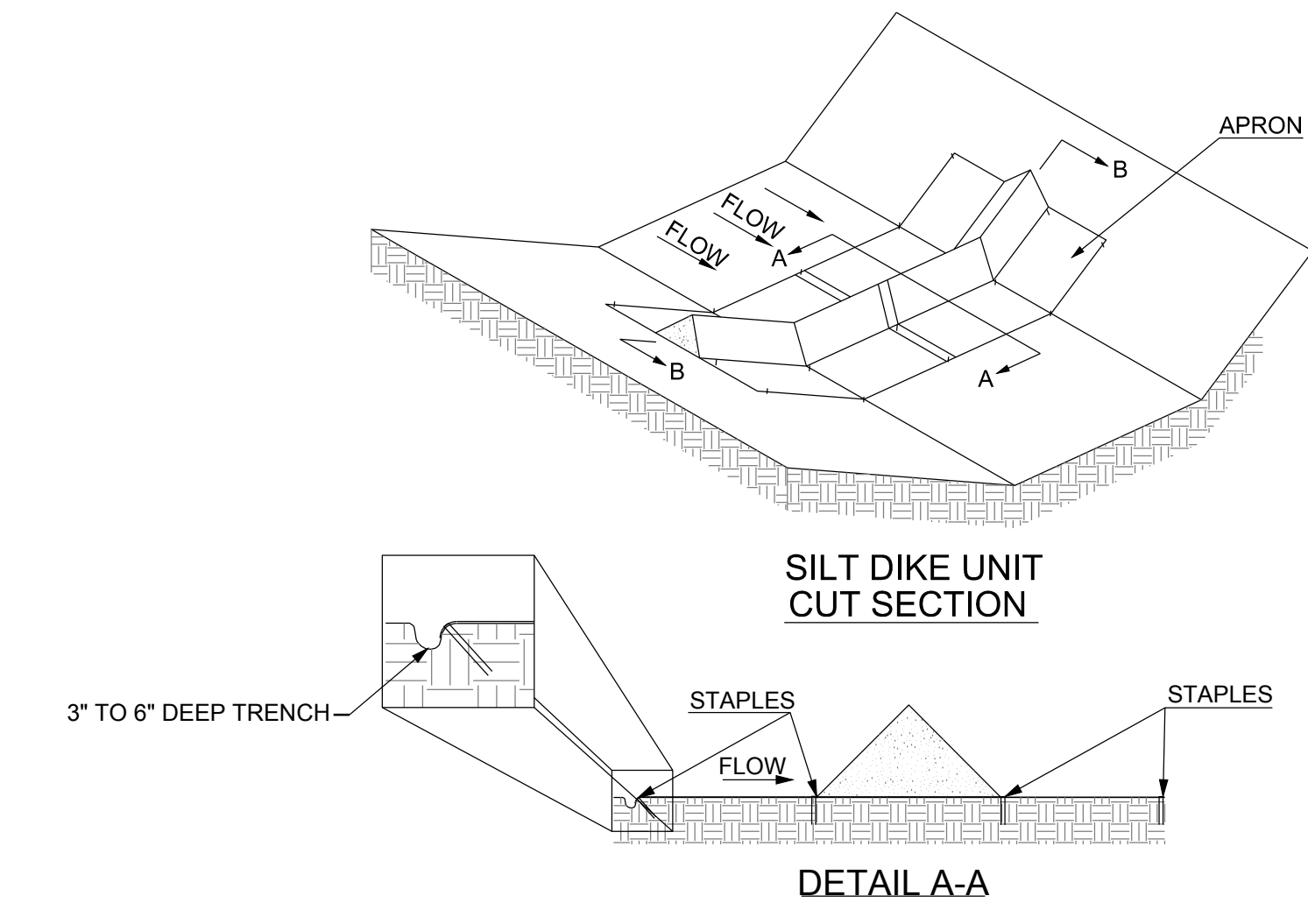
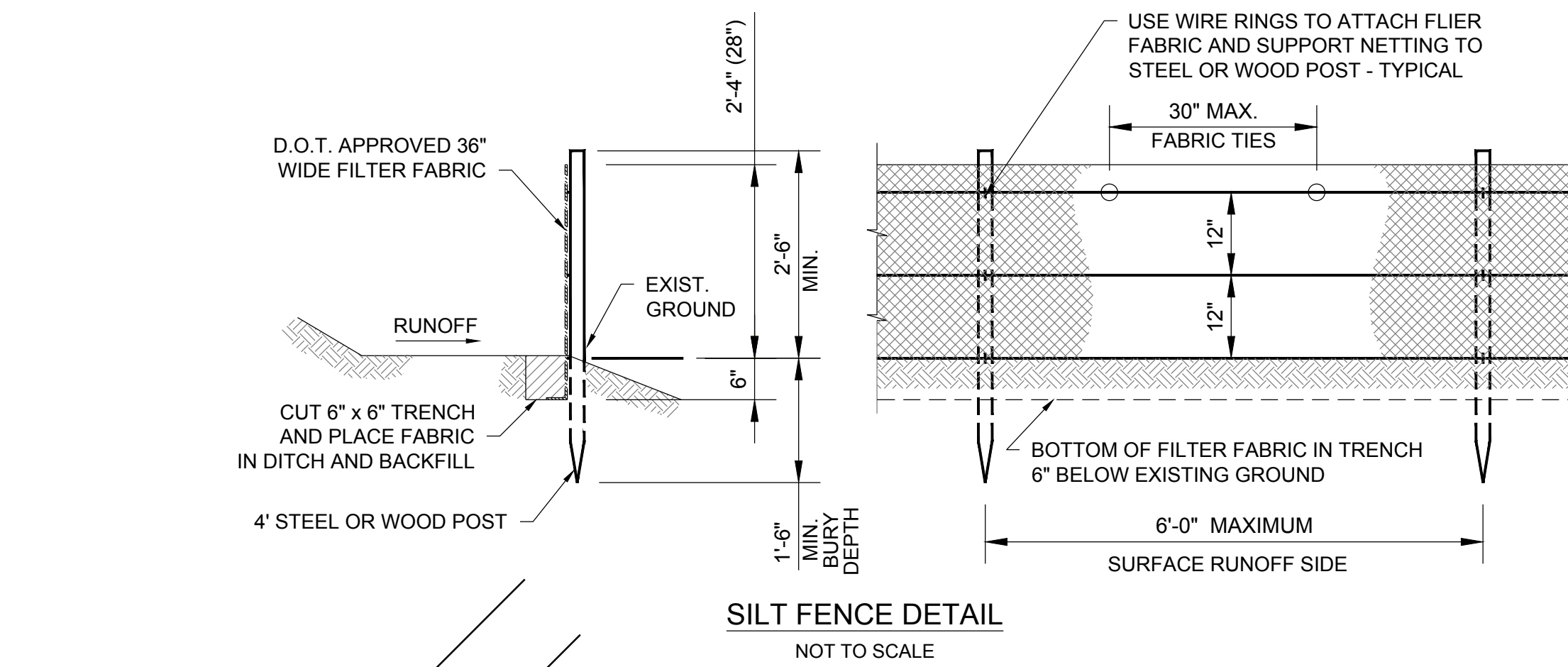
WILMA P. MANKILLER HEALTH CENTER
EXPANSION
STILWELL, OKLAHOMA

GENERAL EROSION NOTES

- THE STORM WATER POLLUTION PREVENTION PLAN IS COMPRISED OF THIS DRAWING (SITE MAP), THE STANDARD DETAILS, PLUS THE PERMIT AND ALL SUBSEQUENT REPORTS AND RELATED DOCUMENTS.
- ALL CONTRACTORS AND SUBCONTRACTORS INVOLVED WITH STORM POLLUTION PREVENTION SHALL OBTAIN A COPY OF THE STORM WATER POLLUTION PREVENTION PLAN AND THE STATE NATIONAL POLLUTION DISCHARGE ELIMINATION SYSTEM GENERAL PERMIT (NPDES PERMIT) AND BECOME FAMILIAR WITH THEIR CONTENTS.
- CONTRACTOR SHALL IMPLEMENT BEST MANAGEMENT PRACTICES AS REQUIRED BY THE SWPPP. ADDITIONAL BEST MANAGEMENT PRACTICES SHALL BE IMPLEMENTED AS DICTATED BY CONDITIONS AT NO ADDITIONAL COST OF OWNER THROUGHOUT ALL PHASES OF CONSTRUCTION.
- BEST MANAGE PRACTICES (BMP'S) AND CONTROLS SHALL CONFORM TO FEDERAL, STATE, OR LOCAL REQUIREMENTS OR MANUAL OF PRACTICE, AS APPLICABLE. CONTRACTOR SHALL IMPLEMENT ADDITIONAL CONTROLS AS DIRECTED BY PERMITTING AGENCY OR OWNER.
- SITE MAP MUST CLEARLY DELINEATE ALL STATE WATERS, PERMITS FOR ANY CONSTRUCTION ACTIVITY IMPACTING STATE WATER OR REGULATED WETLANDS MUST BE MAINTAINED ON SITE AT ALL TIMES.
- CONTRACTOR SHALL MINIMIZE CLEARING TO THE MAXIMUM EXTENT PRACTICAL OR AS REQUIRED BY THE GENERAL PERMIT.
- GENERAL CONTRACTOR SHALL DENOTE ON PLAN THE TEMPORARY PARKING AND STORAGE AREA WHICH SHALL ALSO BE USED AS THE EQUIPMENT MAINTENANCE AND CLEANING AREA, EMPLOYEE PARKING AREA, AND AREA FOR LOCATING PORTABLE FACILITIES, OFFICE TRAILERS, AND TOILET FACILITIES. CONTRACTOR SHALL CONSTRUCT TEMPORARY BERM ON DOWN STREAM SIDES.
- ALL WASH WATER (CONCRETE TRUCKS, VEHICLE CLEANING, EQUIPMENT CLEANING, ETC.) SHALL BE DETAINED AND PROPERLY TREATED OR DISPOSED.
- SUFFICIENT OIL AND GREASE ABSORBING MATERIALS AND FLOTATION BOOMS SHALL BE MAINTAINED ON SITE OR READILY AVAILABLE TO CONTAIN AND CLEAN-UP FUEL OR CHEMICAL SPILLS AND LEAKS.
- DUST ON THE SITE SHALL BE CONTROLLED. THE USE OF MOTOR OILS AND OTHER PETROLEUM BASED OR TOXIC LIQUIDS FOR DUST SUPPRESSION OPERATIONS IS PROHIBITED.
- RUBBISH, TRASH, GARBAGE, LITTER, OR OTHER SUCH MATERIALS SHALL BE DEPOSITED INTO SEALED CONTAINERS, MATERIALS SHALL BE PREVENTED FROM LEAVING THE PREMISES THROUGH THE ACTION OF WIND OR STORM WATER DISCHARGE INTO DRAINAGE DITCHES OR WATERS OF THE STATE.
- ALL STORM WATER POLLUTION PREVENTION MEASURES PRESENTED ON THIS SITE MAP, AND IN THE STORM WATER POLLUTION PREVENTION PLAN, SHALL BE INITIATED AS SOON AS PRACTICABLE.
- ALL DENUDED AREAS THAT WILL BE INACTIVE FOR 14 DAYS OR MORE, MUST BE STABILIZED TEMPORARY WITH THE USE OF FAST-GERMINATING ANNUAL GRASS/GRAIN VARIETIES, STRAW/HAY MULCH, WOOD CELLULOSE FIBERS, TACKIFIERS NETTING OR BLANKETS AS SHOWN ON SITE MAP.
- DISTURBED PORTIONS OF THE SITE WHERE CONSTRUCTION ACTIVITY HAS PERMANENTLY STOPPED SHALL BE PERMANENTLY STABILIZED AS SHOWN ON THE PLANS. THESE AREAS SHALL BE SEEDED, SODDED, AND/OR VEGETATED NO LATER THAN 14 DAYS AFTER THE LAST CONSTRUCTION ACTIVITY OCCURRING IN THESE AREAS. REFER TO THE GRADING PLAN AND/OR LANDSCAPE PLAN.
- IF THE ACTION OF VEHICLES TRAVELING OVER THE GRAVEL CONSTRUCTION ENTRANCES IS NOT SUFFICIENT TO REMOVE THE MAJORITY OF DIRT OR MUD, THEN THE TIRES MUST BE WASHED BEFORE THE VEHICLES ENTER A PUBLIC ROAD. IF WASHING IS USED, PROVISION MUST BE MADE TO INTERCEPT THE WASH WATER AND TRAP THE SEDIMENT BEFORE IT IS CARRIED OFF THE SITE ONLY USE INGRESS/EGRESS LOCATIONS AS PROVIDED.
- ALL MATERIALS SPILLED, DROPPED, WASHED, OR TRACKED FROM VEHICLES ONTO ROADWAY OR INTO STORM DRAINS MUST BE REMOVED IMMEDIATELY.
- CONTRACTORS OR SUBCONTRACTORS WILL BE RESPONSIBLE FOR REMOVING SEDIMENT IN DETENTION POND AND ANY SEDIMENT THAT MAY HAVE COLLECTED IN THE STORM SEWER DRAINAGE SYSTEMS IN CONJUNCTION WITH THE STABILIZATION OF THE SITE.
- ON-SITE AND OFF-SITE SOIL STOCKPILE AND BORROW AREAS SHALL BE PROTECTED FROM EROSION AND SEDIMENTATION THROUGH IMPLEMENTATION OF BEST MANAGEMENT PRACTICES. STOCKPILE AND BORROW AREA LOCATIONS SHALL BE NOTED ON THE SITE MAP AND PERMITTED IN ACCORDANCE WITH GENERAL PERMIT REQUIREMENTS.
- SLOPES SHALL BE LEFT IN A ROUGHENED CONDITION DURING THE GRADING PHASE TO REDUCE RUNOFF VELOCITIES AND EROSION.
- DUE TO THE GRADE CHANGES DURING THE DEVELOPMENT OF THE PROJECT, THE CONTRACTOR SHALL BE RESPONSIBLE FOR ADJUSTING THE EROSION AND SEDIMENT CONTROL MEASURES (SILT FENCES, ETC.) TO PREVENT EROSION AND POLLUTANT DISCHARGE.
- GENERAL CONTRACTOR IS TO DESIGNATE/IDENTIFY AREAS ON THE SITE MAPS, INSIDE OF THE LIMITS OF DISTURBANCE, FOR WASTE DISPOSAL AND DELIVERY AND MATERIAL STORAGE.
- CONTRACTOR SHALL BE RESPONSIBLE FOR PREPARING & SUBMITTING NOTICE OF INTENT (N.O.I.) & NOTICE OF TERMINATION (N.O.T.).
- CONTRACTOR TO LIMIT DISTURBANCE OF SITE IN STRICT ACCORDANCE WITH EROSION CONTROL SEQUENCING SHOWN ON THIS PLAN. NO UNNECESSARY OR IMPROPERLY SEQUENCED CLEARING AND/OR GRADING SHALL BE PERMITTED.

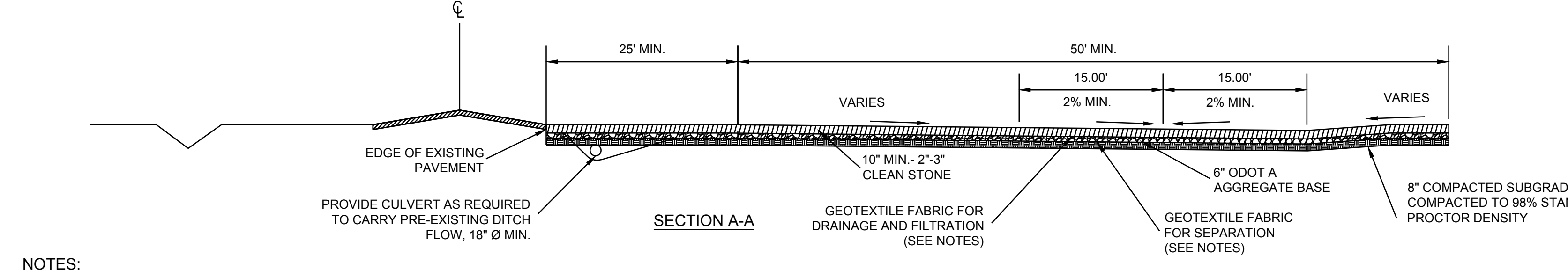
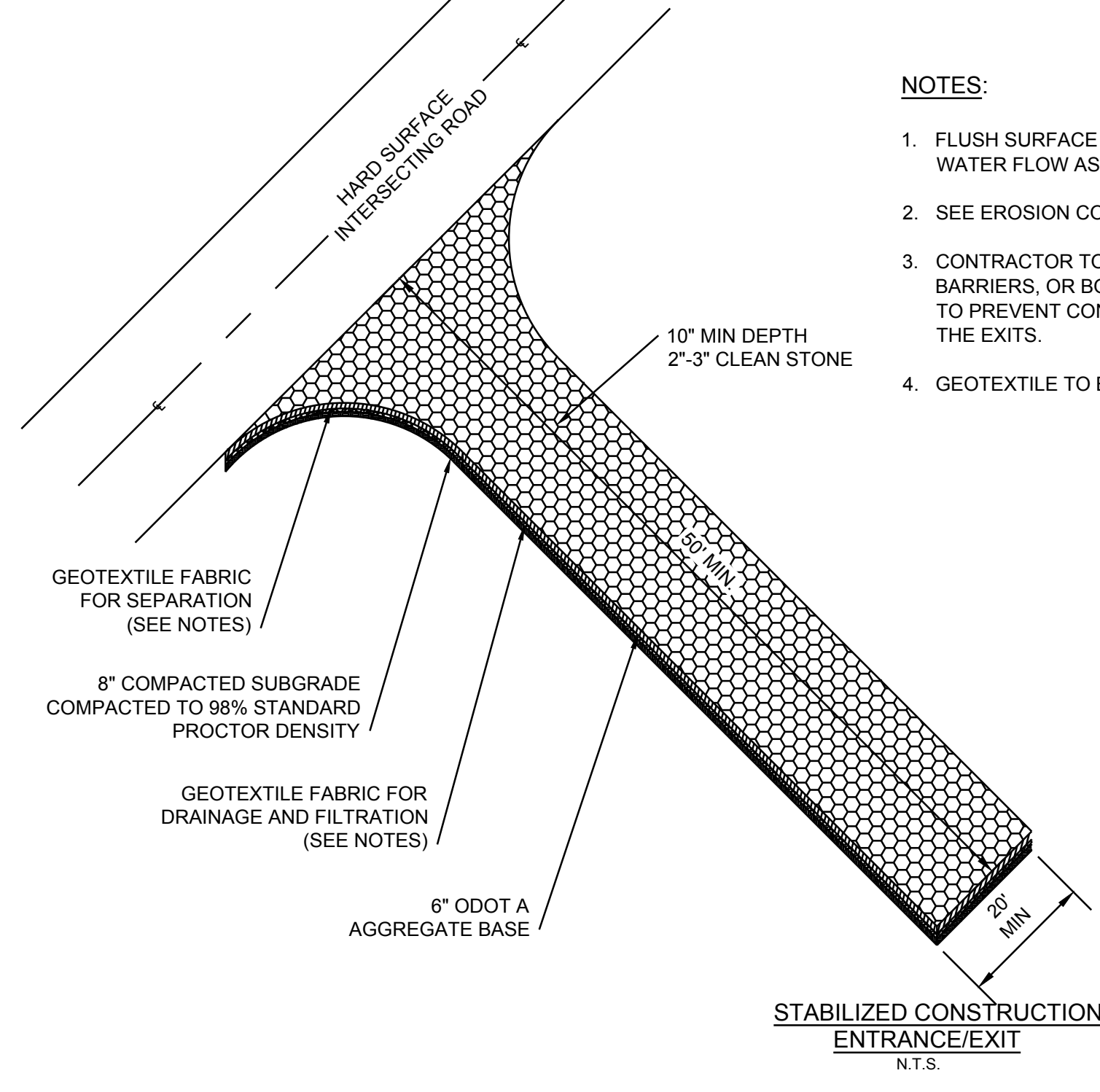
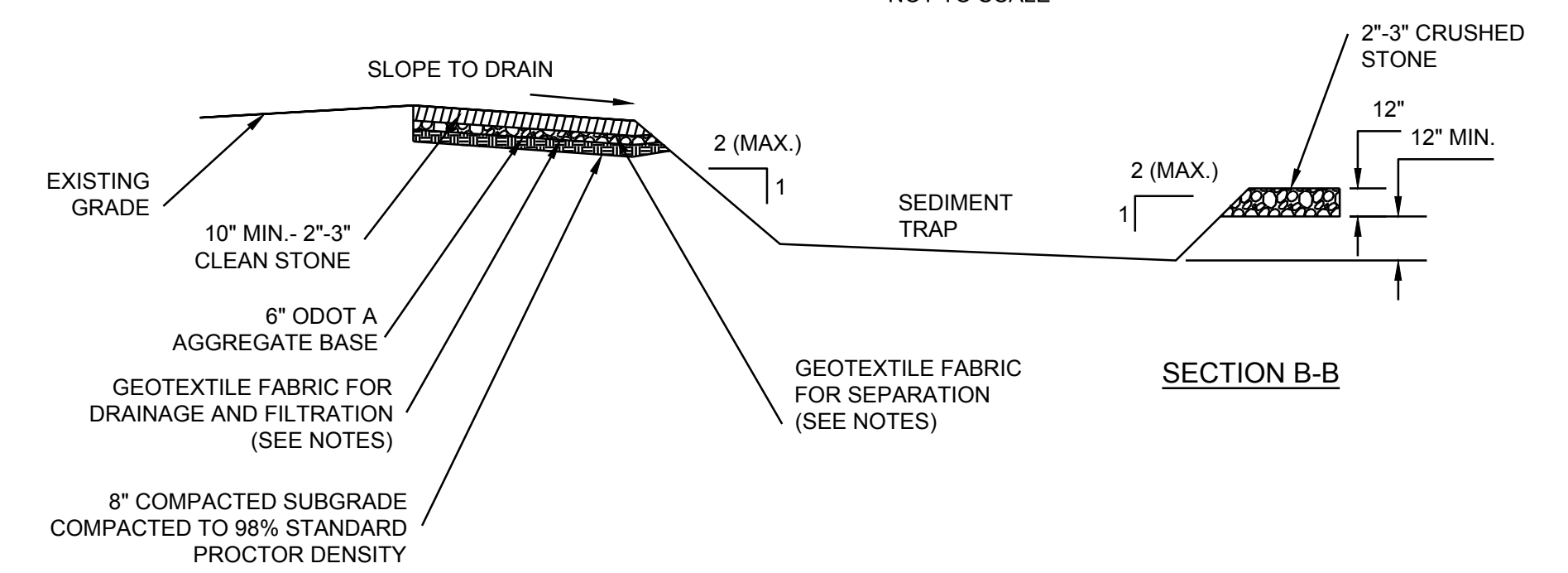
BMP MAINTENANCE EROSION NOTES

- ALL MEASURES STATED ON THIS SITE MAP, AND IN THE STORM WATER POLLUTION PREVENTION PLAN, SHALL BE MAINTAINED IN FULLY FUNCTIONAL CONDITION UNTIL NO LONGER REQUIRED FOR A COMPLETED PHASE OF WORK OR FINAL STABILIZATION OF THE SITE. ALL EROSION AND SEDIMENTATION CONTROL MEASURES SHALL BE CHECKED BY A QUALIFIED PERSON IN ACCORDANCE WITH THE CONTRACT DOCUMENTS OR THE APPLICABLE PERMIT, WHICHEVER IS MORE STRINGENT, AND REPAIRED IN ACCORDANCE WITH THE FOLLOWING:
- INLET PROTECTION DEVICES AND BARRIERS SHALL BE REPAIRED OR REPLACED IF THEY SHOW SIGNS OF UNDERMINING OR DETERIORATION.
 - ALL SEEDED AREAS SHALL BE CHECKED REGULARLY TO SEE THAT A GOOD STAND IS MAINTAINED. AREAS SHOULD BE FERTILIZED, WATERED, AND RESEEDED AS NEEDED.
 - SILT FENCES SHALL BE REPAIRED TO THEIR ORIGINAL CONDITIONS IF DAMAGED. SEDIMENT SHALL BE REMOVED FROM THE SILT FENCES WHEN IT REACHES ONE-HALF THE HEIGHT OF THE SILT FENCE.
 - THE CONSTRUCTION EXITS SHALL BE MAINTAINED IN A CONDITION WHICH WILL PREVENT TRACKING OR FLOW OF MUD ONTO PUBLIC RIGHT-A-WAY. THIS MAY REQUIRE PERIODIC TOP DRESSING OF THE CONSTRUCTION EXITS AS CONDITIONS DEMAND.
 - THE TEMPORARY PARKING AND STORAGE AREA SHALL BE KEPT IN GOOD CONDITION (SUITABLE FOR PARKING AND STORAGE). THIS MAY REQUIRE PERIODIC TOP DRESSING OF THE TEMPORARY PARKING AREA AS CONDITIONS DEMAND.
 - OUTLET STRUCTURES IN THE SEDIMENTATION BASINS SHALL BE MAINTAINED IN OPERATIONAL CONDITIONS AT ALL TIMES. SEDIMENT SHALL BE REMOVED FROM SEDIMENT BASINS OR TRAPS WHEN THE DESIGN CAPACITY HAS BEEN REDUCED BY 50%.
 - PRIOR TO LEAVING THE SITE, ALL VEHICLES SHALL BE CLEANED OF DEBRIS. ANY DEBRIS AND/OR SEDIMENT REACHING THE PUBLIC STREET SHALL BE CLEANED IMMEDIATELY BY A METHOD OTHER THAN FLUSHING.

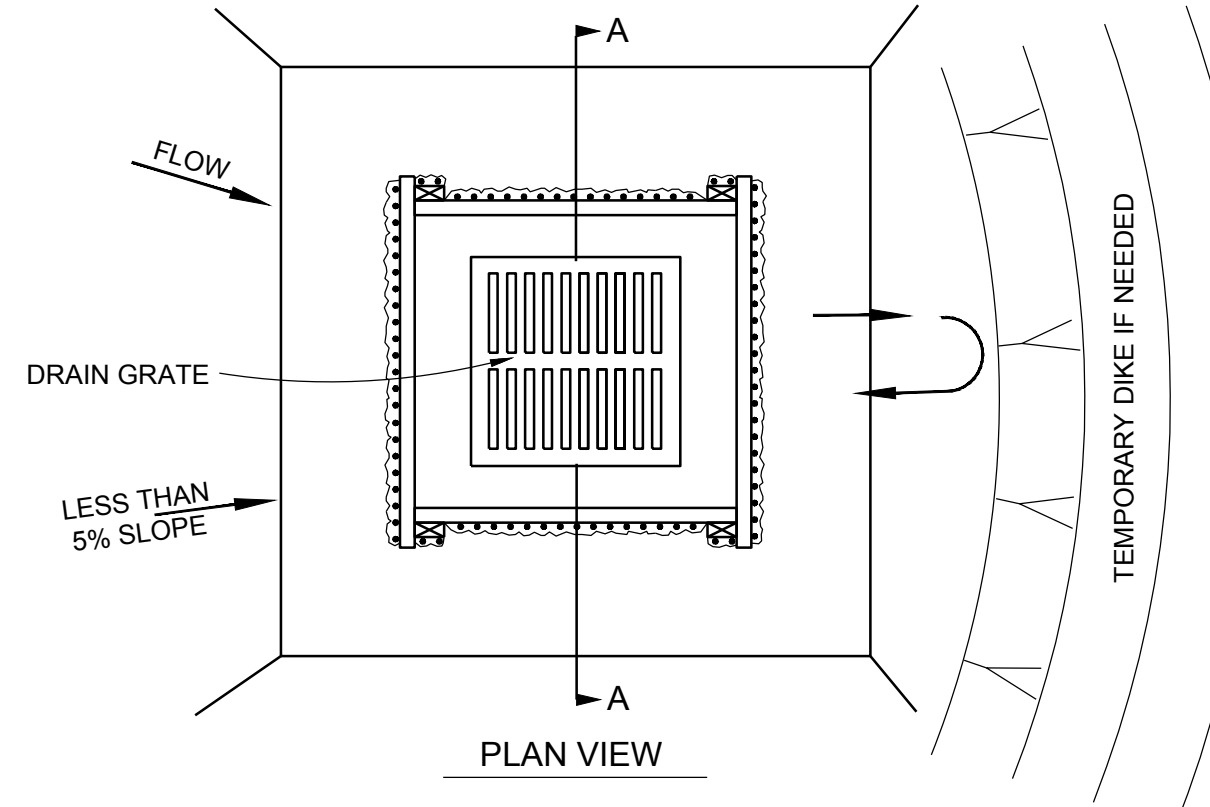
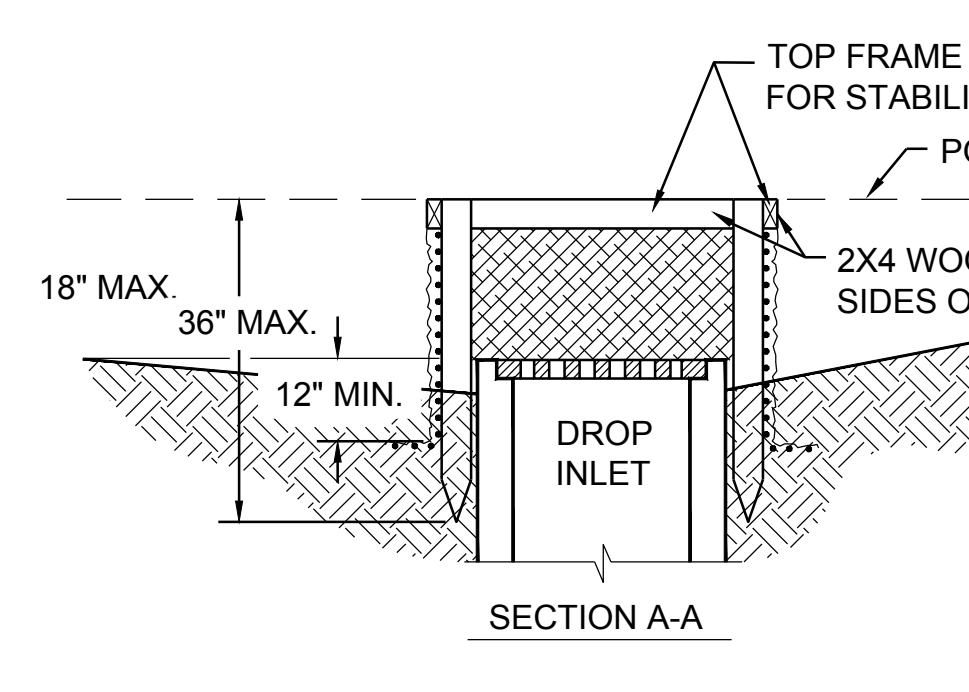


NOTES:
1. FLUSH SURFACE STONE TOWARD SEDIMENT TRAP WITH HIGH VOLUME WATER FLOW AS NEEDED TO MAINTAIN CLEAN SURFACE STONE.
2. SEE EROSION CONTROL FOR LOCATION.
3. CONTRACTOR TO LOCATED TEMPORARY CONSTRUCTION FENCING, JERSEY BARRIERS, OR BOTH ALONG THE SIDES OF THE CONSTRUCTION EXITS TO PREVENT CONSTRUCTION TRAFFIC FROM SHORT CIRCUITING/BYPASSING THE EXITS.
4. GEOTEXTILE TO BE AASHTO M 288-97 CLASS-2

TRIANGULAR SILT DIKE INSTALLATION FOR ROADWAY DITCH OR DRAINAGE DITCH
NOT TO SCALE



- NOTES:
1. DROP INLET SEDIMENT BARRIERS ARE TO BE USED FOR SMALL, NEARLY LEVEL DRAINAGE AREAS. (LESS THAN 5%).
2. USE 2X4 WOOD OR EQUIVALENT METAL STAKES, 3' MINIMUM LENGTH.
3. INSTALL 2X4 WOOD TOP FRAME TO INSURE STABILITY.
4. THE TOP OF THE FRAME (PONDING HEIGHT), MUST BE WELL BELOW THE GROUND ELEVATION DOWNSLOPE TO PREVENT RUNOFF FROM BY-PASSING THE INLET. A TEMPORARY DIKE MAY BE NECESSARY ON THE DOWNSLOPE SIDE OF THE STRUCTURE.



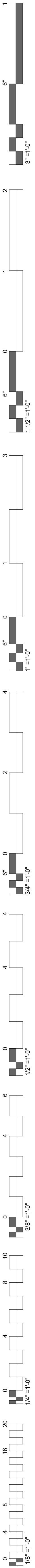
KEY PLAN

PROJECT PHASE:
BID PACKAGE 01

#	DATE	REVISIONS DESCRIPTION

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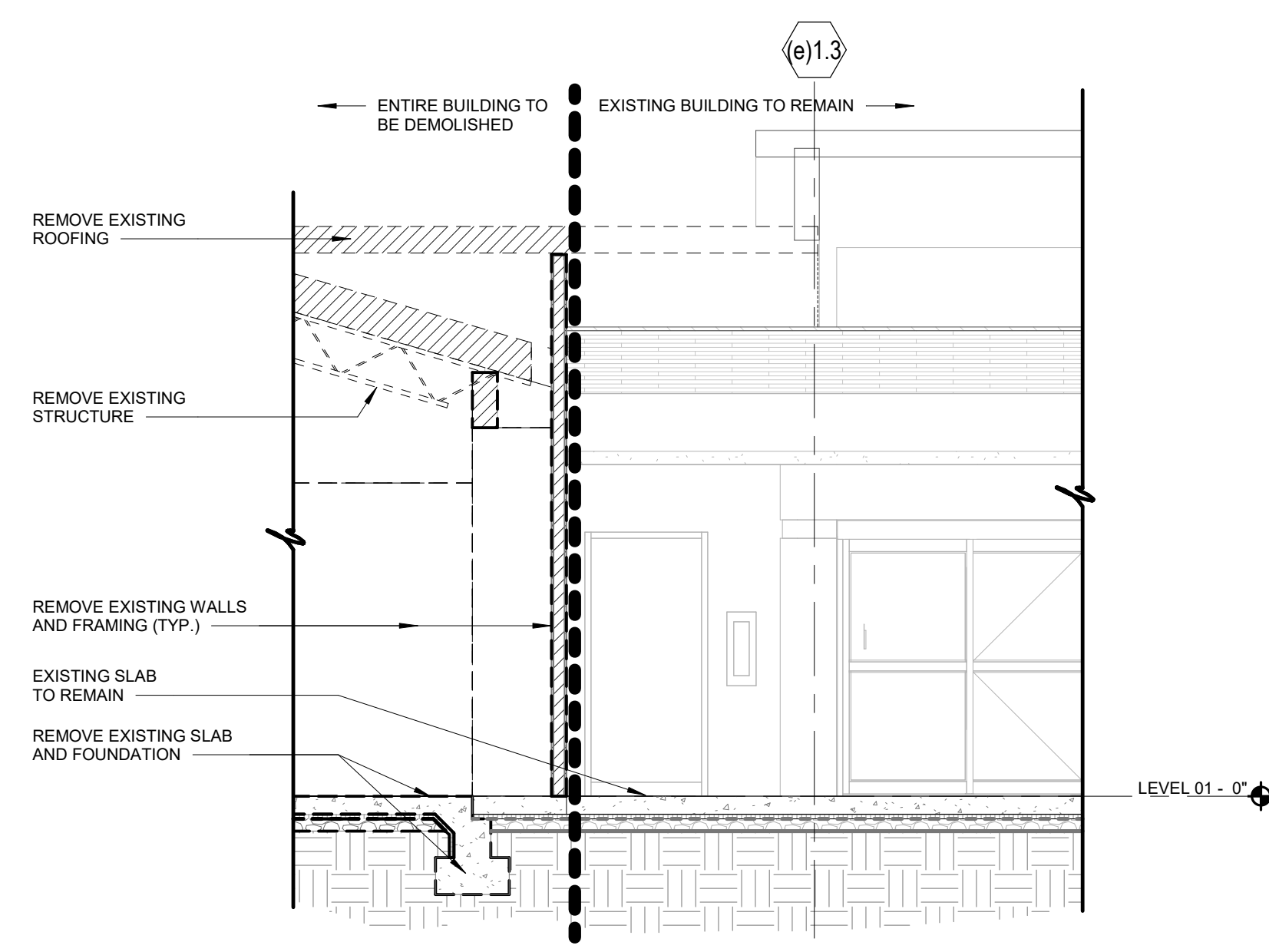
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EROSION CONTROL DETAILS



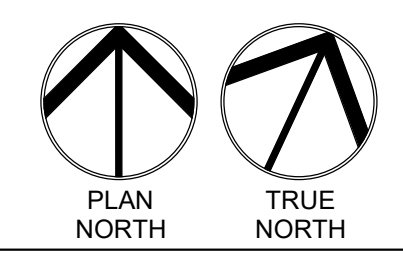
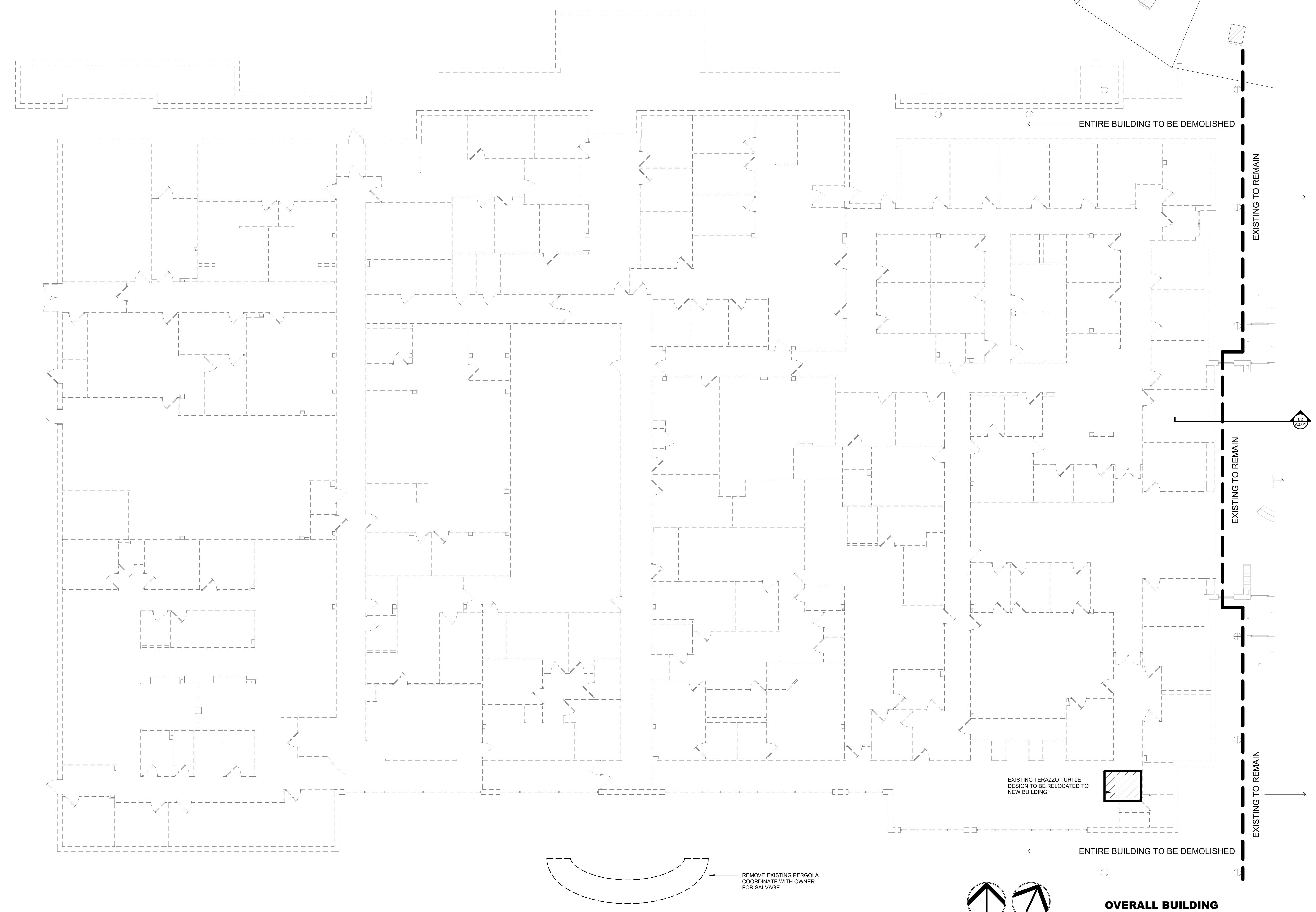
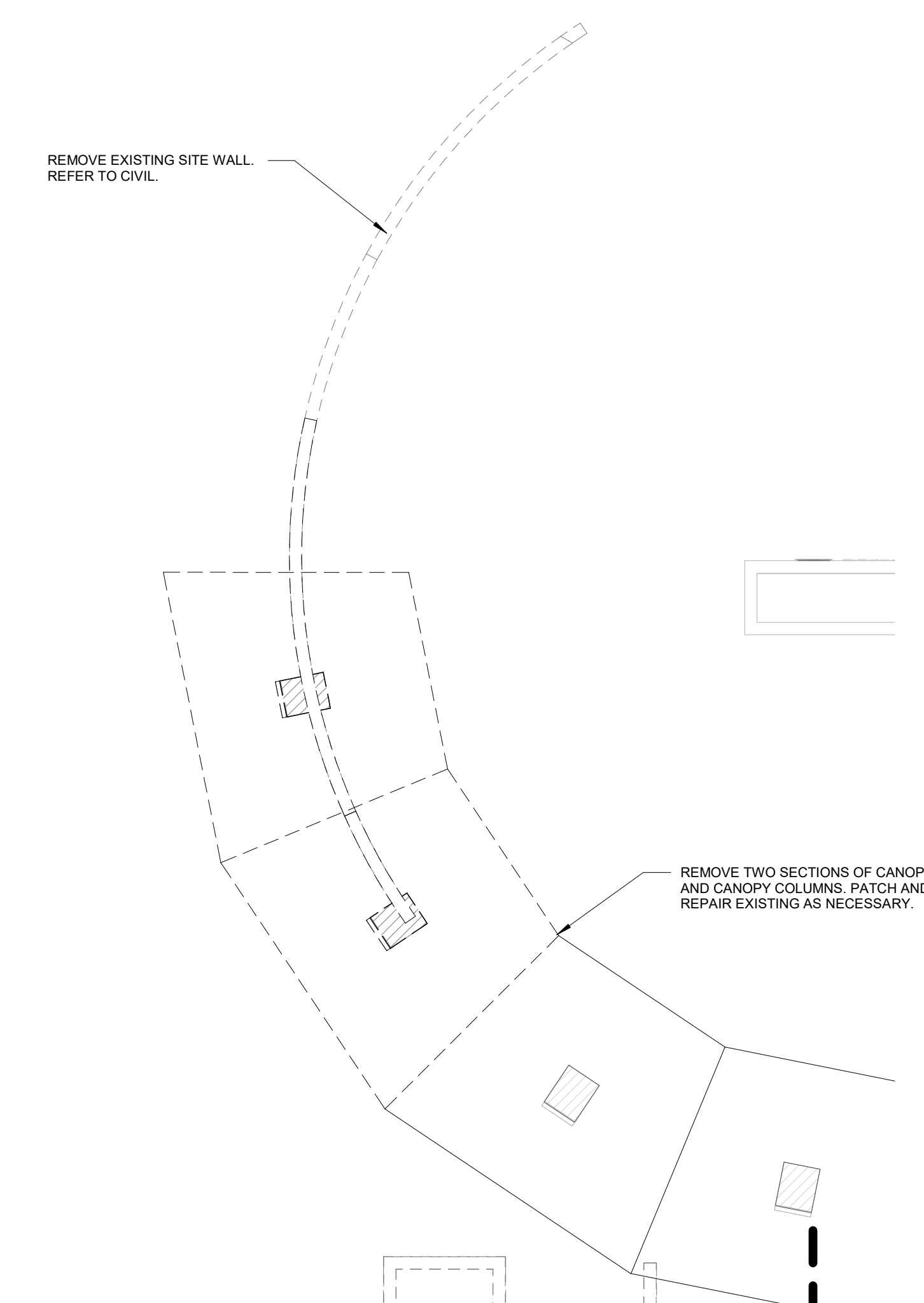
DEMOLITION LEGEND

	ITEMS SHOWN DASHED ARE TO BE REMOVED.
	ITEMS SHOWN SCREENED ARE TO REMAIN.
	ITEMS SHOWN SOLID ARE NEW CONSTRUCTION.

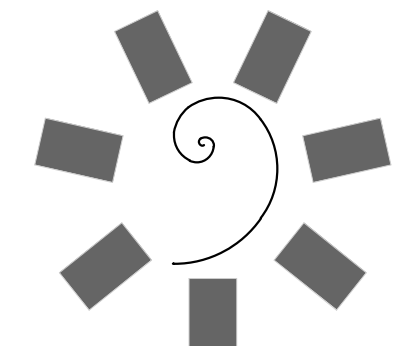
- GENERAL DEMOLITION NOTES**
- ENTIRE BUILDING SHOWN IN DASHED LINE TO BE DEMOLISHED. REFER TO SPECIFICATION DIVISION 02 FOR SPECIFIC PROJECT DEMOLITION REQUIREMENTS.
 - REFER TO DEMOLITION SYMBOL LEGEND ON DRAWINGS.
 - CONTRACTOR TO COORDINATE DEMOLITION WORK SEQUENCE. DEMOLITION DRAWINGS REPRESENT EXISTING CONDITIONS BASED ON LIMITED EXISTING DRAWINGS AND SITE OBSERVATIONS. CONTRACTOR SHALL FIELD VERIFY ALL EXISTING BUILDING AND SITE CONDITIONS. DEMOLITIONS DRAWINGS GENERALLY INDICATE EXISTING SCOPE OF WORK TO BE DEMOLISHED AND ARE NOT INTENDED TO LIMIT OR FULLY DEFINE THE SCOPE OF WORK TO BE REMOVED IN ORDER TO ACCOMPLISH SCOPE OF NEW CONSTRUCTION. WHERE THESE CONDITIONS OCCUR OUTSIDE OF THE DEMOLITION LIMITS, AREAS SHALL BE RETURNED TO THEIR ORIGINAL CONDITION AS PART OF THE NEW CONSTRUCTION SCOPE OF WORK.
 - CONTRACTOR SHALL NOTIFY ARCHITECT OF ANY CONFLICTS BETWEEN EXISTING CONSTRUCTION AND CONSTRUCTION DOCUMENTS. REFERENCE STRUCTURAL, CIVIL, AND MEP DRAWINGS FOR OTHER DISCIPLINE DEMOLITION SCOPE OF WORK.
 - WHERE EXISTING WALL MOUNTED DEVICES, FIXTURES OR OTHER WALL MOUNTED ITEMS ARE SCHEDULED TO BE SALVAGED, REFERENCE CONSTRUCTION DRAWINGS FOR NEW LOCATIONS OR COORDINATE WITH OWNER FOR STORAGE LOCATION.
 - PARTITIONS SCHEDULED TO BE REMOVED. DEMOLITION SHOULD INCLUDE MISCELLANEOUS BRACING, TRACK, ETC. TO BOTTOM OF STRUCTURE.
 - CONTRACTOR SHALL MAINTAIN ALL REQUIRED EXITS UNOBSTRUCTED, ILLUMINATED AND PROTECTED FROM CONSTRUCTION ACTIVITIES. CONTRACTOR TO CLEAN AREAS ADJACENT TO DEMOLITION AREA OF DUST, DIRT AND DEBRIS CAUSED BY DEMOLITIONS OPERATIONS. PROMPTLY DISPOSE OF DEMOLISHED MATERIALS. DO NOT ALLOW DEMOLISHED MATERIALS TO ACCUMULATE ON-SITE. TRANSPORT DEMOLISHED MATERIALS AND LEGALLY DISPOSE OF THEM.
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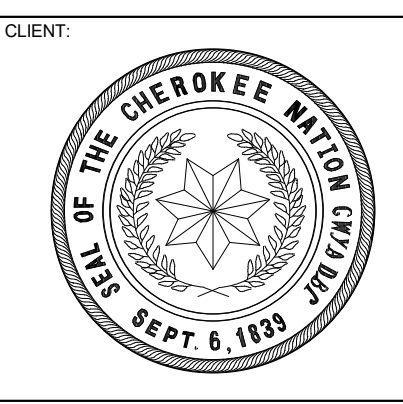
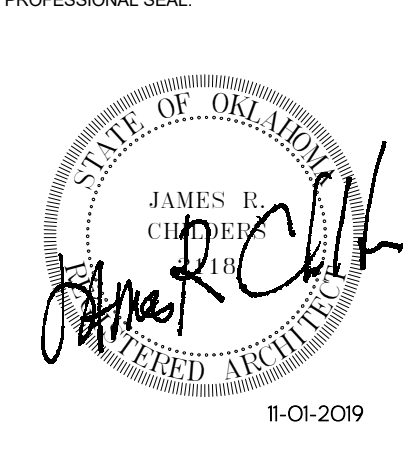
02 DEMOLITION SECTION
1/4" = 1'-0"



01 OVERALL BUILDING DEMOLITION PLAN
1" = 10'-0"



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**WILMA P. MANKILLER HEALTH CENTER
EXPANSION**
STILWELL, OKLAHOMA

KEY PLAN:

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PROJECT PHASE:
BID PACKAGE 01

#	DATE	REVISIONS DESCRIPTION

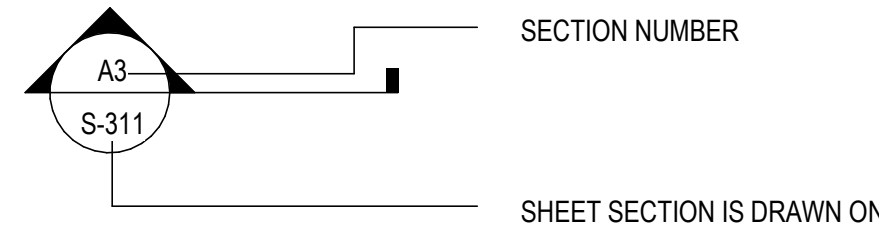
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SHEET NUMBER:
A0.01

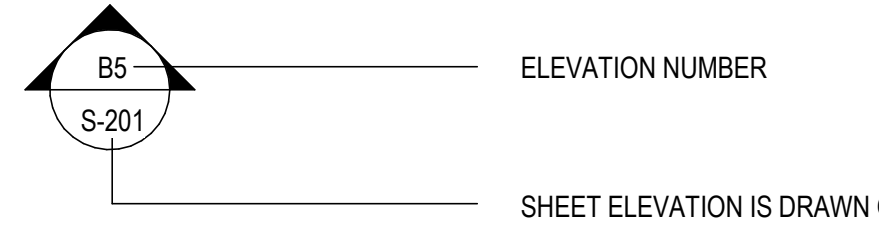
OVERALL BUILDING DEMOLITION PLAN

STRUCTURAL GRAPHIC SYMBOLS

SECTION CROSS-REFERENCE SYMBOL



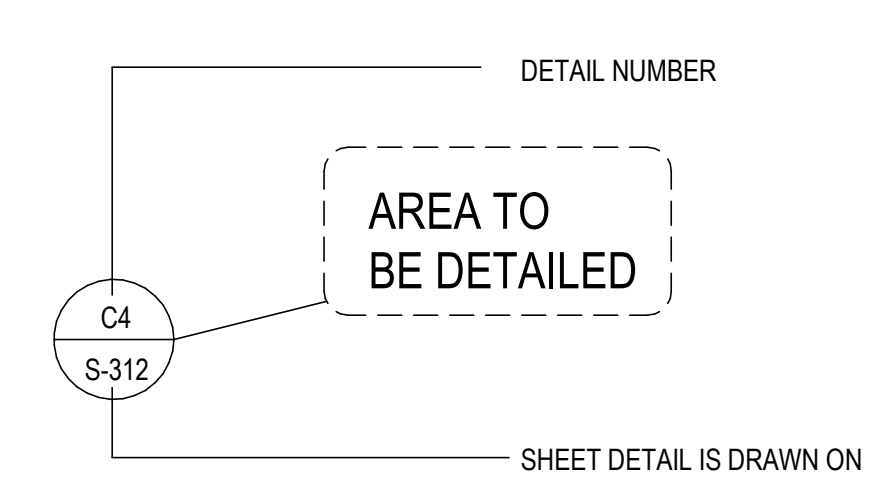
EXTERIOR ELEVATION CROSS-REFERENCE SYMBOL



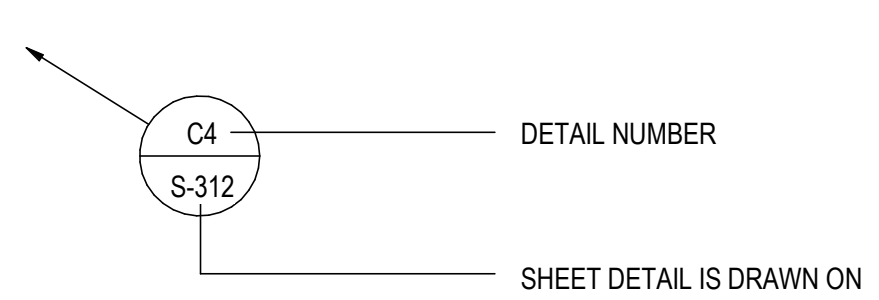
INTERIOR ELEVATION CROSS-REFERENCE SYMBOL



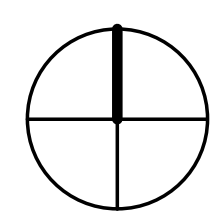
PLAN DETAIL CROSS-REFERENCE SYMBOL



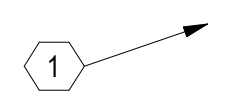
DETAIL CROSS-REFERENCE SYMBOL



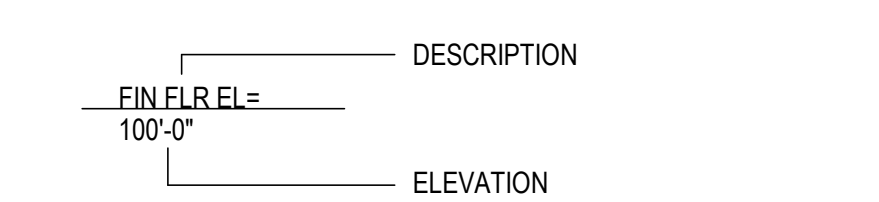
NORTH ARROW SYMBOL



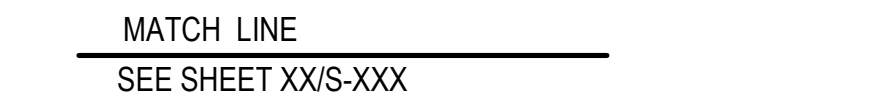
KEYNOTE SYMBOL



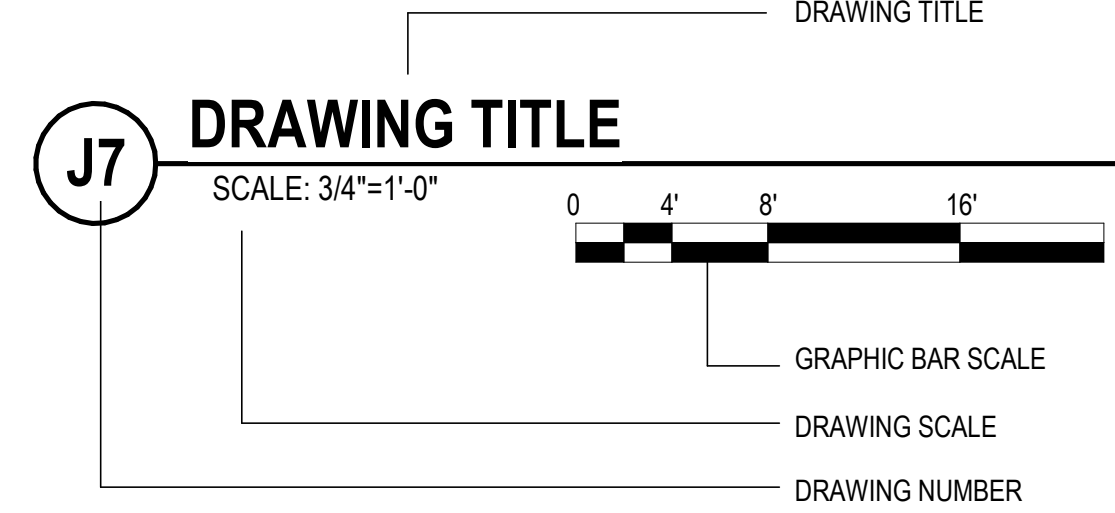
ELEVATION TARGET SYMBOL



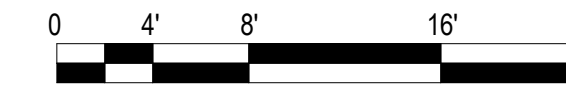
MATCH LINE SYMBOL



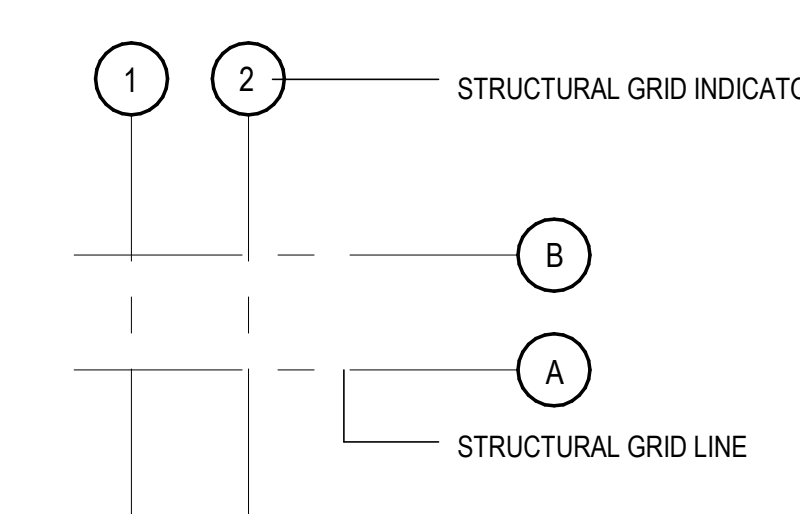
DRAWING TITLE SYMBOL



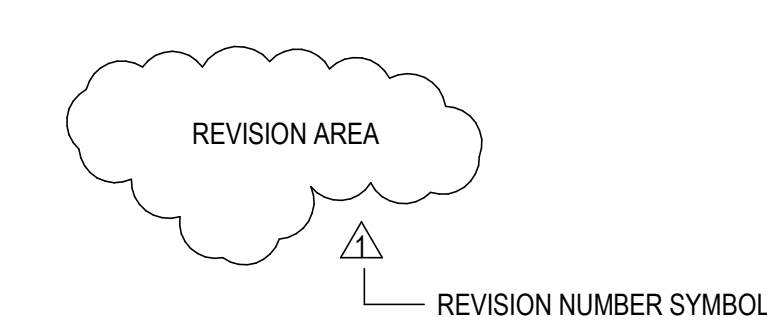
GRAPHIC BAR SCALE SYMBOL



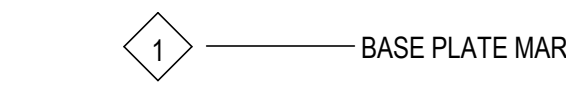
STRUCTURAL GRID REFERENCE SYMBOL



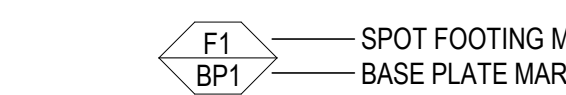
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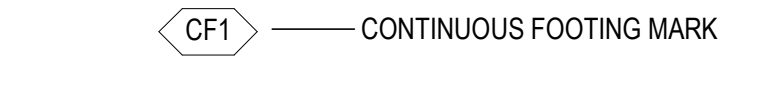
BASE PLATE MARK SYMBOL



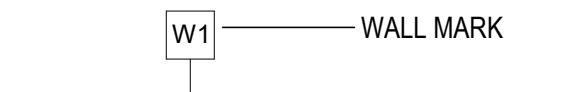
SPOT FOOTING MARK SYMBOL



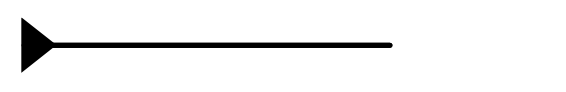
CONTINUOUS FOOTING MARK SYMBOL



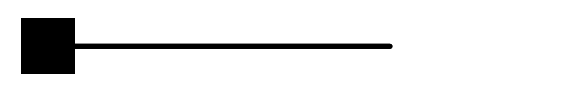
WALL MARK SYMBOL



MOMENT CONNECTION SYMBOL



SIDEPLATE MOMENT CONNECTION SYMBOL, SEE SIDEPLATE DRAWINGS



STRUCTURAL MATERIALS LEGEND

	STEEL		MASONRY GROUT		CMU (IN SECTION)
	CAST-IN-PLACE CONCRETE		GRATING (IN SECTION)		CMU (IN PLAN)
	BAR GRATING		COMPACTED EARTH		RIGID INSULATION
			UN-COMPACTED EARTH		

ABBREVIATIONS

A/E	ARCHITECT/ENGINEER
AB	ANCHOR BOLT
ABAN	ABANDON
ABBRV	ABBREVIATION
AC	ASPHALTIC CONCRETE
ACI	AMERICAN CONCRETE INSTITUTE
ACP	ASPHALTIC CONCRETE PAVING
ACR	ACROSS
ACST	ACOUSTIC
AD	AREA DRAIN
ADA	AMERICANS WITH DISABILITIES ACT
ADDL	ADDITIONAL
ADDM	ADDDUM
ADJ	ADJACENT/ADJOINING
ADMIN	ADMINISTRATION
AFF	ABOVE FINISHED FLOOR
AFG	ABOVE FINISHED GRADE
AFS	ABOVE FINISHED SLAB
AGGR	AGGREGATE
AHR	ANCHOR
AIA	AMERICAN INSTITUTE OF ARCHITECTS
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION
AISI	AMERICAN IRON AND STEEL INSTITUTE
AITC	AMERICAN INSTITUTE OF TIMBER CONSTRUCTION
ALNMT	ALIGNMENT
ALT	ALTERNATE, ALTERNATIVE
ALUM	ALUMINUM
AMT	AMOUNT
ANSI	AMERICAN NATIONAL STANDARDS INSTITUTE
APA	AMERICAN PLYWOOD ASSOCIATION
APPD	APPROVED
APPROX	APPROXIMATE
APPY	APPENDIX
AR	AS REQUIRED
ARCH	ARCHITECT
ASCE	AMERICAN SOCIETY OF CIVIL ENGINEERS
ASPH	ASPHALT
ASI	ARCHITECT'S SUPPLEMENTAL INSTRUCTIONS
ASSN	ASSOCIATION
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS
ATCH	ATTACHMENT
ATTN	ATTENTION
AWS	AMERICAN WELDING SOCIETY
AZ	AZIMUTH
B&F	BELL AND FLANGE
BAL	BALANCE
BB	BACK TO BACK
BC	BOTTOM CHORD
BD	BOARD
BDRY	BOUNDARY
BEVEL	BEVEL
BFF	BELOW FINISH FLOOR
BKG	BACKING
BKGD	BACKGROUND
BLD	BUILD
BLDG	BUILDING
BLK	BLOCK/BLOCKING
BLT	BUILT
BLVD	BOULEVARD
BLVD	BELOW
BM	BEAM
BO	BOTTOM OF
BOS	BOTTOM OF STEEL
BOT	BOTTOM
BP	BASE PLATE
BRCG	BRACING
BRDG	BRIDGING
BRG	BEARING
BRG PL	BEARING PLATE
BS	BOTH SIDES
BSMT	BASEMENT
BT WLD	BUTT WELD
BTWN	BETWEEN
C	CHANNEL
C/C	CENTER TO CENTER
CAM	CAMBER
CAN	CANOPY
CD	CONSTRUCTION DOCUMENTS, CONTRACT DOCUMENTS
CEM	CEMENT
CHFR	CHAMFER
CHKD	CHECKED/CHECKERED
CI	CAST IRON
CIP	CAST IN PLACE
CJ	CONSTRUCTION JOINT
CJ	CONTRACTION JOINT
CJ	CONTROL JOINT
CL	CENTER LINE
CLG	CEILING
CLR	CLEAR
cm	CENTIMETER
CMU	CONCRETE MASONRY UNIT
CO	COMPANY
COA	CITY OF ALBUQUERQUE
COL	COLUMN
COM	COMMON
CONC	CONCRETE
CONN	CONNECTION
CONSTR	CONSTRUCTION
CONT	CONTINUOUS, CONTINUE
CONTR	CONTRACTOR
COORD	COORDINATE
CRSI	CONCRETE REINFORCING STEEL INSTITUTE
CSI	CONSTRUCTION SPECIFICATIONS INSTITUTE
CTR	CENTER
CTRL	CONTROL
CU	CUBIC
CU YD	CUBIC YARD
D	DEEP, DEPTH
D-B	DESIGN-BUILD
DAT	DATUM
DBE	DECK BEARING ELEVATION
DBL	DOUBLE
DEG	DEGREE
DEL	DELETE
DEMO	DEMOLITION
DET	DETAIL
DEV	DEVELOPMENT
DFTG	DRAFTING
DIA	DIAMETER
DIAG	DIAGONAL
DIFF	DIFFERENCE, DIFFERENTIAL
DM	DIMENSION
DIST	DISTANCE
DIV	DIVIDE
DJ	DOUBLE JOIST
DL	DEAD LOAD
DOC	DOCUMENT
DOUG FIR	DOUGLAS FIR
DSGN	DESIGN
DWG	DRAWING
DWL/DWLS	DOWELS
E	EAST, MODULUS OF ELASTICITY
EA	EACH
EE	EACH END
EF	EACH FACE
EFS	EXTERIOR INSULATION AND FINISH SYSTEM
EJ	EXPANSION JOINT
EL	ELEVATION
ELAST	ELASTOMERIC
ELEC	ELECTRIC
ELEM	ELEMENTARY
ELEV	ELEVATOR
EMBED	EMBEDDED / EMBEDMENT
ENCL	ENCLOSURE
ENGR	ENGINEER
ESD	EDGE OF SLAB
EPA	ENVIRONMENTAL PROTECTION AGENCY
EQ	EQUAL
EQUIP	EQUIPMENT
EQUIV	EQUIVALENT
ESCAL	ESCALATOR
ESMT	EASEMENT
EST	ESTIMATE
ETC	ET CETERA

ABBREVIATIONS

EW	EACH WAY
EX	EXAMPLE
EXC	EXCAVATE
EXCL	EXCLUDE
EXIST	EXISTING
EXP	EXPANSION
EXT	EXTERIOR
F/F	FACE TO FACE
FAB	FABRIC
FACIL	FACILITY
FAT	FLAT BAR
FD	FLOOR DRAIN
FDTN	FOUNDATION
FF	FAR FACE
FF EL	FINISH FLOOR ELEVATION
FIN GR	FINISH GRADE
FN	FINISH
FIN	FINISH
FIN FLR	FINISH FLOOR
FLG	FLANGE
FLR	FLOOR
FLR SK	FLOOR SINK
FOC	FACE OF CONCRETE
FOF	FACE OF FINISH
FOM	FACE OF MASONRY
FOS	FACE OF STUD
FOS	FACE OF SLAB
FOW	FACE OF WALL
FR	FRAME
FRMG	FRAMING
FS	FAR SIDE
FSTNR	FASTENER
FT	FOOT / FEET
FTLB	FOOT/POUND
FTLBF	FOOT/POUND FORCE
FTG	FOOTING
FUT	FUTURE
G	GRID
GAGE	GAGE
GALV	GALVANIZED
GALV STL	GALVANIZED STEEL
GR BM	GRADE BEAM
GC	GENERAL CONTRACTOR
GEN	GENERAL
GLU LAM	GLUED LAMINATED WOOD
GLZ	GLAZING
GOVT	GOVERNMENT
GRTG	GRATING
GT	GROUT
H	HIGH
HAS	HEADED ANCHOR STUD
HC	HOLLOW-CORE
HCP	HANDICAPPED
HD	HEAVY DUTY
HGR	HANGER
HLDN	HOLD/DOWN
HORIZ	HORIZONTAL
HS	HIGH STRENGTH
HSPG	HOUSEKEEPING
HSS	HOLLOW STRUCTURAL SECTIONS
HST	HOIST
HT	HEIGHT
IBC	INTERNATIONAL BUILDING CODE
ID	INSIDE DIAMETER
IF	INSIDE FACE
IFS	INSIDE FACE OF STUD
IN	INCH
INCL	INCLUDED
INFO	INFORMATION
IN-LB	INCH-POUND
IN-LBF	INCH-POUND FORCE
INSTL	INSTALL
INSUL	INSULATION
INT	INTERIOR
IR	INSIDE RADIUS
K	KIP
K	THOUSAND
K	KNEE BRACE
KCJ	KEYED CONTROL JOINT
KIP	THOUSAND POUNDS
KIP FT	THOUSAND FOOT/POUNDS
KLF	KIPS PER LINEAL FOOT
KO	KNOCK OUT
KOP	KNOCK OUT PANEL
KSF	KIPS PER SQUARE FOOT
KSI	KIPS PER SQUARE INCH
L	ANGLE
LAM	LAMINATE
LATL	LATERAL
LBF	POUND-FORCE
LBR	LUMBER
LBS	POUND
LD BRG	LOAD BEARING
LF	LINEAR FEET (FOOT)
LN	LINEAR
LL	LIVE LOAD
LLBB	LONG LEG BACK TO BACK
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LONG	LONGITUDINAL
LT GA	LIGHT GAGE
LT WT	LIGHT WEIGHT
LVR	LOUVER
LWC	LIGHTWEIGHT CONCRETE
M	MOMENT
MAINT	MAINTENANCE
MATL	MATERIAL
MAX	MAXIMUM
MB	MACHINE BOLT
MC	MOMENT CONNECTION
MCJ	MASONRY CONTROL JOINT
MD	METAL DECK
ME	MECHANICAL ENGINEER
MECH	MECHANICAL
MEZZ	MEZZANINE
MFR	MANUFACTURER
MID	MIDDLE
MIN	MINIMUM
MISC	MISCELLANEOUS
ML	MICRO-LAMINATED
MONOL	MONOLITHIC
MO	MASONRY OPENING
MS	MACHINE SCREW
MSL	MEAN SEA LEVEL
MTL	METAL
N	NORTH
NA	NOT APPLICABLE
NF	NEAR FACE
NIC	NOT IN CONTRACT
NM	NEW MEXICO
NO	NUMBER
NOM	NOMINAL
NS	NEAR SIDE
NTS	NOT TO SCALE
O/D	OUT TO OUT
OA	OVERALL
OC	ON CENTER
OD	OUTSIDE DIAMETER
OF	OUTSIDE FACE
OFS	OUTSIDE FACE OF STUD
OPH	OPPOSITE HAND
OPNG	OPENING
OPP	OPPOSITE
OPT	OPTIONAL
OR	OUTSIDE RADIUS
PAR	PARALLEL, PARAPET
PART	PARTIAL
PC	PIECE, PORTLAND CEMENT
PCC	PRECAST CONCRETE
PCF	POUNDS PER CUBIC FOOT
PCIP	PRECAST/PRESTRESSED CONCRETE
INSTITUTE	INSTITUTE
PED	PEDESTAL
PEN	PENETRATE
PERIM	PERIMETER
PERP	PERPENDICULAR
PH	PHASE
PIL	PLASTER

ABBREVIATIONS

PL	PLATE
PLAT	PLATFORM
PLBG	PLUMBING
PLF	POUNDS PER LINEAR FOOT
PLM	PARALLAM
PL YWD	PLYWOOD
POS	POSITION
PP	PANEL POINT
PRCST	PRECAST
PREFAB	PREFABRICATE
PRELIM	PRELIMINARY
PREV	PREVIOUS
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PT	POST-TENSIONED
PT CONC	POST-TENSIONED CONCRETE
PTN	PARTITION
PVQ	PAVING
QTY	QUANTITY
QUAD	QUAD
R	RADIUS, RISER
RC	REINFORCED CONCRETE
RD	ROAD, ROOF DRAIN
REC	RECESSED
REF	REFERENCE
REIN	REINFORCE/REINFORCEMENT
REPL	REPLACE
REQ	REQUIRED
REQD	REQUIRED
REV	REVISION
RGD INS	RIGID INSULATION
RFI	REQUEST FOR INFORMATION
RND	ROUND
RO	ROUGH OPENING
RT	RIGHT
RVL	REVEAL
S	SCHEDULE
SCHM	SCHEMATIC
SCHED	SCHEDULE
SPEC	SPECIFICATIONS
SDI	STEEL DECK INSTITUTE
SDD	SADDLE
SE	STRUCTURAL ENGINEER
SECT	SECTION
SEN	SQUARE FEET (FOOT)
SHT	SHEET, SHAFT
SHTHG	SHEATHING
SM	SMILAR
SJ	STEEL JOIST INSTITUTE
SLNT	SEALANT
SP	SMOOTH
SP	SUMP PIT
SPA	SPACE/SPACES
SPEC	SPECIFICATION
SPRT	SUPPORT
SQ	SQUARE
SQ IN	SQUARE INCH
SQ YD	SQUARE YARD
SSPC	STRUCTURAL STEEL PAINTING COUNCIL
ST	STAIRS
STAG	STAGGERED
STD	STANDARD
STIF	STIFFENER
STR	STRIP
STR	STRIPUP
STL	STEEL
STL LNTL	STEEL LINTEL
STL PL	JST STEEL JOIST
STL PL	STEEL PLATE
STL RF DK	STEEL ROOF DECK
STR	STRINGERS
STRUCT	STRUCTURAL
SUB	SUBSTITUTE
SUF	SUFFICIENT
SUP	SUPPLEMENTARY
SUPPL	SUPPLEMENT
SYM	SYMBOL
SYMM	SYMMETRICAL
SYS	SYSTEM
T	TREAD
T&B	TOP AND BOTTOM
T&G	TONGUE AND GROOVE
TAN	TANGENT
TEMP	TEMPORARY
THD	THREAD
THK	THICKNESS
THRU	THROUGH
TJ	TRUSS JOIST INSTITUTE
TOF	TOP OF
TOB	TOP OF BEAM
TOC	TOP OF CONCRETE
TOC FTG	TOP OF CONCRETE FOOTING
TOC WALL	TOP OF CONCRETE WALL
TOF	TOP OF FOOTING
TOG	TOP OF GRATE
TOJ	TOP OF JOIST
TOL	TOLERANCE
TOM	TOP OF MASONRY
TOP	TOP OF PARAPET
TOS	TOP OF SLAB
TOS	TOP OF STEEL
TOW	TOP OF WALL
TRANS	TRANSVERSE
TRNBKLE	TURNBUCKLE
TYP	TYPICAL
UBC	UNIFORM BUILDING CODE
UNO	UNLESS NOTED OTHERWISE
VAR	VARIES
VERT	VERTICAL
VIF	VERIFY IN FIELD
VNR	VENEER
VR	VAPOR RETARDER
VRFY	VERIFY
W	WEST, WIDE
W	WITH
W/O	WITHOUT
WBL	WOOD BLOCKING
WD	WOOD
WF	WIDE FLANGE
WF BM	WIDE FLANGE BEAM
WLD	WIND LOAD
WLD	WELD
WM	WIRE MESH
WP	WATERPROOFING
WSCOT	WAINSCOT
WT	WEIGHT
WVF	WELDED WIRE FABRIC
WWM	WELDED WIRE MESH
X BRACE	CROSS BRACING
XXH	DOUBLE EXTRA HEAVY
YO	YARD

GENERAL FOUNDATION NOTES:

FOUNDATION NOTES

GENERAL:

A SUBSURFACE SOIL INVESTIGATION HAS BEEN MADE BY BUILDING AND EARTH, PROJECT NO. 0K180172.

A REPORT OF THAT INVESTIGATION DATED AUGUST 30, 2018 IS AVAILABLE FOR VIEWING AT THE OFFICE OF THE ARCHITECT

THE FOUNDATION SYSTEM FOR THIS PROJECT IS SPREAD FOOTINGS OVER AGGREGATE PIERS / STONE COLUMNS FOR THE MAIN BUILDING. THE FOUNDATION SYSTEM FOR THE PRE-ENGINEERED METAL BUILDING IS SPREAD FOOTINGS OVER OVER-EXCAVATED SOILS AND COMPACTED STRUCTURAL FILL.

ADDITIONAL INFORMATION CONCERNING SPECIFIC SOIL CONDITIONS TO BE ENCOUNTERED IS AVAILABLE IN THE SOILS REPORTS AND SHALL BE REVIEWED BY THE CONTRACTOR.

FIELD OBSERVATION AND TESTS:

THE OWNER SHALL EMPLOY THE SERVICES OF A REGISTERED, LICENSED GEOTECHNICAL ENGINEER TO OBSERVE ALL CONTROLLED EARTHWORK. THE GEOTECHNICAL ENGINEER SHALL PROVIDE CONTINUOUS ON-SITE OBSERVATION BY EXPERIENCED PERSONNEL DURING CONSTRUCTION OF CONTROLLED EARTHWORK. THE CONTRACTOR SHALL NOTIFY THE GEOTECHNICAL ENGINEER AT LEAST TWO WORKING DAYS IN ADVANCE OF ANY FIELD OPERATIONS OF THE CONTROLLED EARTHWORK.

TESTS OF MATERIALS SHALL BE MADE AT THE FOLLOWING MINIMUM RATES. THE ON-SITE GEOTECHNICAL ENGINEER SHALL DETERMINE THE ACTUAL TESTING RATES:

GENERAL STRUCTURAL NOTES

CODES AND MANUALS:
IBC-15 INTERNATIONAL BUILDING CODE 2015
ASCE/SEI 3-91 STRUCTURAL DESIGN OF COMPOSITE SLABS
ASCE/SEI 7-10 MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES
AISC 360-10 SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS
AISC 341-10 SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS
AISC MANUAL OF STEEL CONSTRUCTION 14TH EDITION
SJI-K-1.1-10 STANDARD SPECIFICATION FOR OPEN WEB STEEL JOISTS, K-SERIES
SJI-LH/DLH-1.1-10 STANDARD SPECIFICATION FOR LONGSPAN STEEL JOISTS, LH-SERIES AND DEEP LONGSPAN STEEL JOISTS, DLH-SERIES
SJI-JG-1.1-10 STANDARD SPECIFICATION FOR JOIST GIRDERS
SJI-CJ-1.0-10 STANDARD SPECIFICATION FOR COMPOSITE STEEL JOIST, CJ-SERIES
SDI DIAPHRAGM DESIGN MANUAL, 3RD EDITION
ANSI/SKI NC1 0-06 STANDARD FOR NONCOMPOSITE STEEL FLOOR DECK
ANSI/SKI R01 0-06 STANDARD FOR STEEL ROOF DECK
ANSI/SKI C1 0-06 STANDARD FOR COMPOSITE STEEL FLOOR DECK
AISI S100-12 NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS
AISI S200-12 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - GENERAL PROVISIONS
AISI S210-12 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - FLOOR AND ROOF SYSTEM DESIGN
AISI S211-07 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - WALL STUD DESIGN WITH 2012 SUPPLEMENT
AISI S212-12 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - HEADER DESIGN
AISI S213-12 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - LATERAL DESIGN WITH 2010 SUPPLEMENT
AISI S214-12 NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - TRUSS DESIGN, WITH SUPPLEMENT 2, DATED 2008
MBMA METAL BUILDING SYSTEMS MANUAL, 2007 EDITION
ACI 318-14 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE
PCI DESIGN HANDBOOK - MANUAL OF DESIGN, 27th EDITION, 2010
ACI 530-13 BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES
ACI 530.1-13 SPECIFICATIONS FOR MASONRY STRUCTURES
AWS D1.1-04 STRUCTURAL WELDING CODE - STEEL
AWS D1.3-98 STRUCTURAL WELDING CODE - SHEET STEEL
AWS D1.4-11 STRUCTURAL WELDING CODE - REINFORCING STEEL

DESIGN CRITERIA:
VERTICAL:
LIVE LOAD
FLOOR 65 PSF
STAIRS AND EXIT-WAYS* 100 PSF
MINIMUM CONCENTRATED LOAD 300 LBS
ADDITIONAL SUPERIMPOSED LOADS
PARTITIONS 15 PSF
SUSPENDED EQUIPMENT 10 PSF
CONCENTRATED LOAD 2000 LBS (PER IBC 1607.4)
ROOF LIVE LOAD: LR = 20'R1'R2 20 PSF
REDUCTION FACTOR BASED ON TRIB AREA R1=1.0
REDUCTION FACTOR BASED ON ROOF SLOPE R2=1.0
SNOW LOAD
GROUND SNOW LOAD PG=10 PSF
FLAT ROOF SNOW LOAD** PF=12 PSF
SNOW EXPOSURE FACTOR CE=1.0
SNOW LOAD IMPORTANCE FACTOR IS=1.0
THERMAL FACTOR CT=1.0
**INCLUDES 5 PSF RAIN-ON SNOW SURCHARGE LOAD
HORIZONTAL:
WIND
ULTIMATE DESIGN WIND SPEED 115 MPH
RISK CATEGORY II
INTERNAL PRESSURE COEFFICIENT GCPI=0.18
NATURAL FREQUENCY 0.685
STRUCTURE IS FLEXIBLE
SEISMIC
SEISMIC IMPORTANCE FACTOR IS = 1.0
MAPPED SPECTRAL RESPONSE ACCELERATIONS
SHORT PERIOD SS=0.161G
1 SECOND PERIOD S1=0.086G
SITE CLASS C
SPECTRAL RESPONSE COEFFICIENTS
SHORT PERIOD SDS=0.129G
1 SECOND PERIOD SD1=0.097G
SEISMIC DESIGN CATEGORY C
BASIC SEISMIC FORCE RESISTING SYSTEM: STRUCTURAL STEEL SYSTEMS
STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE
SEISMIC RESPONSE COEFFICIENT CS=0.043
RESPONSE MODIFICATION FACTOR R = 3
DESIGN BASE SHEAR V = 0.043W
ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE
BASIC SEISMIC FORCE RESISTING SYSTEM: REINFORCED MASONRY SYSTEMS
INTERMEDIATE REINFORCED MASONRY SHEAR WALLS
SEISMIC RESPONSE COEFFICIENT CS=0.037
RESPONSE MODIFICATION FACTOR R = 3.12
DESIGN BASE SHEAR V = 0.037W
ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE
ALLOWABLE SOIL BEARING PRESSURE = 6000 PSF (STONE COLUMNS REQUIRED TO ACHIEVE BEARING PRESSURE)
FROST DEPTH = 24 INCHES
FUTURE BUILDING EXPANSION: NONE
GENERAL:
STRUCTURAL DRAWINGS ARE NOT STAND-ALONE DOCUMENTS AND ARE INTENDED TO BE USED IN CONJUNCTION WITH CIVIL, ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND DRAWINGS FROM OTHER DISCIPLINES. THE CONTRACTOR SHALL COORDINATE ALL REQUIREMENTS OF THE CONTRACT DOCUMENTS INTO THE SHOP DRAWINGS AND FIELD WORK.
COORDINATE DIMENSIONS OF ALL OPENINGS, DEPRESSIONS, BLOCKOUTS, ETC. WITH ARCHITECTURAL DRAWINGS, DRAWINGS FROM OTHER DISCIPLINES, PROJECT SHOP DRAWINGS, AND FIELD CONDITIONS PRIOR TO SHOP DRAWING SUBMITTAL. THE STRUCTURAL DRAWINGS ONLY REPRESENT A PORTION OF THE REQUIREMENTS FOR THE PROJECT.
SEE ARCHITECTURAL PLANS FOR INTERIOR NON-BEARING PARTITION WALLS. PARTITION FRAMING SHALL BE CONNECTED TO THE PRIMARY STRUCTURE TO ALLOW FOR VERTICAL LIVE LOAD DEFLECTIONS OF SPAN/360 FOR FLOOR FRAMING AND SPAN/240 FOR ROOF FRAMING.
CONTRACTOR'S CONSTRUCTION AND/OR ERECTION SEQUENCES SHALL RECOGNIZE AND CONSIDER THE EFFECTS OF THERMAL MOVEMENTS OF STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PERIOD.
THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS IN THE FIELD.
SHOP DRAWINGS SHALL BE FURNISHED AND REVIEWED BEFORE ANY FABRICATION OR ERECTION IS STARTED. THE CONTRACTOR SHALL REVIEW AND APPROVE SHOP DRAWINGS PRIOR TO SUBMITTAL TO THE ARCHITECT FOR REVIEW. POORLY EXECUTED SHOP DRAWINGS WILL BE REJECTED AND SHALL BE RESUBMITTED.
THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING SAFE AND ADEQUATE SHORING FOR ALL PARTS OF THE STRUCTURE DURING CONSTRUCTION.
TEMPORARY PROVISIONS SHALL BE MADE FOR STRUCTURAL STABILITY DURING CONSTRUCTION. THE STRUCTURE SHOWN ON THE DRAWINGS HAS BEEN DESIGNED FOR STABILITY UNDER FINAL CONFIGURATION.
NOTCHING OR CUTTING ANY STRUCTURAL MEMBER IN THE FIELD IS PROHIBITED.
THE CONTRACTOR SHALL VERIFY THE SIZE AND LOCATION OF FOUNDATIONS UNDER MECHANICAL AND ELECTRICAL EQUIPMENT AS REQUIRED. NO CONCRETE PADS SHALL BE LOCATED ON ROOF UNLESS SHOWN ON THE STRUCTURAL DRAWINGS.
BACKFILL SHALL NOT BE PLACED BEHIND RETAINING WALLS UNTIL CONCRETE HAS ATTAINED 100 PERCENT OF DESIGN STRENGTH.
BACKFILL SHALL NOT BE PLACED BEHIND BASEMENT WALLS UNTIL THE CONCRETE HAS ATTAINED 100 PERCENT OF DESIGN STRENGTH AND THE ELEVATED FLOOR PROVIDING LATERAL SUPPORT AT THE TOP OF THE WALL IS COMPLETELY CONSTRUCTED, OR TEMPORARY BRACING/SHORING OF THE WALL IS PROVIDED. DESIGN OF ANY TEMPORARY WALL BRACING/SHORING IS THE RESPONSIBILITY OF THE CONTRACTOR.
REMOVAL OF FORMS AND SHORING SHALL BE IN ACCORDANCE WITH ACI 347. WHERE CONCRETE MUST SUPPORT SUPERIMPOSED LOADS PRIOR TO ATTAINING THE SPECIFIED DESIGN STRENGTH, RESHORE CONCRETE IN ACCORDANCE WITH ACI 347. RESHORING SHALL NOT BE REMOVED SOONER THAN 28 DAYS FROM THE DATE OF POUR OR UNTIL CONCRETE HAS ATTAINED THE SPECIFIED DESIGN STRENGTH.
THE CONTRACTOR SHALL SUBMIT FOR PRIOR APPROVAL THE END OF POUR LOCATIONS FOR CONCRETE GRADE BEAMS, CONCRETE COLUMNS, AND CONCRETE BEAMS.

GENERAL STRUCTURAL NOTES

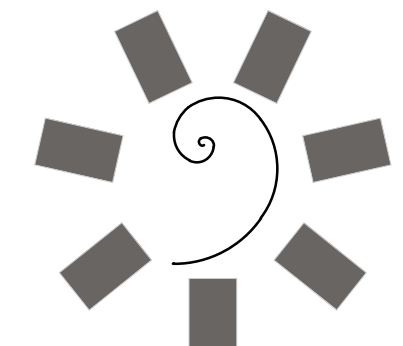
THE CONTRACTOR SHALL BE RESPONSIBLE FOR ADHERING TO ALL APPLICABLE STANDARDS SET FORTH BY OSHA, INCLUDING THE FOLLOWING REQUIREMENTS FROM STANDARDS - 29 CFR, SECTION 1926, SUBPART R:
A. THE STEEL ERECTION CONTRACTOR SHALL NOT ERECT STEEL UNLESS THEY HAVE RECEIVED WRITTEN NOTIFICATION FROM THE CONTRACTOR THAT THE CONCRETE IN THE FOOTINGS, PIERS AND WALLS OR THE MORTAR IN THE MASONRY PIERS AND WALLS HAS ATTAINED, ON THE BASIS OF AN APPROPRIATE ASTM STANDARD TEST METHOD OF FIELD-CURED SAMPLES, EITHER 75 PERCENT OF THE INTENDED MINIMUM COMPRESSIVE DESIGN STRENGTH OR SUFFICIENT STRENGTH TO SUPPORT THE LOADS IMPOSED DURING STEEL ERECTION.
PROVIDE STRUCTURAL ENGINEER A COPY OF WRITTEN NOTIFICATION WHEN IT IS PROVIDED TO THE STEEL ERECTOR.
B. ANCHOR RODS (ANCHOR BOLTS) SHALL NOT BE REPAIRED, REPLACED OR FIELD-MODIFIED WITHOUT THE APPROVAL OF THE PROJECT STRUCTURAL ENGINEER OF RECORD.
PRIOR TO ERECTION OF COLUMNS, THE CONTRACTOR SHALL PROVIDE WRITTEN NOTIFICATION TO THE STEEL ERECTOR IF THERE HAS BEEN ANY REPAIR, REPLACEMENT OR MODIFICATION OF THE ANCHOR RODS (ANCHOR BOLTS).
PROVIDE STRUCTURAL ENGINEER A COPY OF WRITTEN NOTIFICATION WHEN IT IS PROVIDED TO THE STEEL ERECTOR.
C. NO MODIFICATION THAT AFFECTS THE STRENGTH OF A STEEL JOIST OR STEEL JOIST GIRDER SHALL BE MADE WITHOUT THE APPROVAL OF THE PROJECT STRUCTURAL ENGINEER OF RECORD.
D. METAL DECKING HOLES AND OPENINGS SHALL NOT BE CUT UNTIL IMMEDIATELY PRIOR TO BEING PERMANENTLY FILLED WITH THE EQUIPMENT OR STRUCTURE, OR SHALL BE IMMEDIATELY COVERED.
PROTECTION: PROPER PRECAUTIONS SHALL BE TAKEN AT ALL TIMES TO PROTECT VEHICULAR AND PEDESTRIAN TRAFFIC FROM ANY DAMAGE OR INJURY WHICH MAY BE CAUSED, EITHER DIRECTLY OR INDIRECTLY, BY THE WORK INCLUDED ON THESE DRAWINGS. SUCH PRECAUTIONS SHALL INCLUDE THE ERECTION AND MAINTENANCE OF FENCES, BARRICADES, RAILINGS, GUARDS, SIGNS, COVERINGS, LIGHTS, AND OTHER PRECAUTIONS AS MAY BE REQUIRED. IF AT ANY TIME THE OWNER OR THE OWNER'S REPRESENTATIVE, PROPER PRECAUTIONS ARE NOT BEING TAKEN TO SECURE THIS PROTECTION, THE CONTRACTOR SHALL AT NO ADDITIONAL COST TO THE OWNER, INSTALL AND MAINTAIN SUCH ADDITIONAL PROTECTION AS MAY BE DIRECTED BY THE OWNER.
POLLUTION CONTROLS: USE WATER SPRINKLING, TEMPORARY ENCLOSURES, AND OTHER SUITABLE METHODS TO LIMIT DUST AND DIRT RISING AND SCATTERING IN THE AIR TO LOWEST PRACTICAL LEVEL. COMPLY WITH GOVERNING REGULATIONS PERTAINING TO ENVIRONMENTAL PROTECTION.
TYPICAL DETAIL SHEETS:
THE S7.00 SERIES SHEETS IN THESE DRAWINGS CONTAIN TYPICAL STRUCTURAL DETAILS FOR VARIOUS BUILDING MATERIALS. SOME OF THESE DETAILS MAY NOT BE PART OF THIS PROJECT.
DRAWINGS:
DO NOT SCALE DRAWINGS.
WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL NOTES, AND SPECIFICATIONS, THE MORE STRINGENT REQUIREMENTS SHALL GOVERN. DETAILS ON DRAWINGS TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS. DETAILS NOTED "TYPICAL" APPLY TO ALL SIMILAR CONDITIONS. WHERE NO SPECIFIC DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ELSEWHERE ON THE PROJECT.
CAST-IN-PLACE CONCRETE:
ALL CONCRETE SHALL CONFORM TO THE SPECIFICATIONS FOR STRUCTURAL CONCRETE, ACI 301-10.
ALL EXPOSED EDGES OF CONCRETE SHALL HAVE A 3/4" CHAMFER UNLESS NOTED OTHERWISE.
NORMALWEIGHT CONCRETE:
A. FC = 4500 PSI @ 28 DAYS - ALL CONCRETE EXPOSED TO FREEZE/THAW CYCLES AND OCCASIONAL MOISTURE, INCLUDING CONCRETE FLAT WORK, EXPOSED BUILDING STEM WALLS, SITE WALLS, ETC...
EXTERIOR CONCRETE SHALL MEET EXPOSURE CATEGORY AND CLASS F1 ACCORDING TO ACI 318 TABLE 19.3.1.1.
B. FC = 3000 PSI @ 28 DAYS - ALL INTERIOR CONCRETE (I.E. FOOTINGS, PEDESTALS, THE BEAMS, GRADE BEAMS, RETAINING WALLS, ETC.).
C. FC = 3000 PSI @ 28 DAYS - ALL INTERIOR SLABS ON GRADE, UNLESS NOTED OTHERWISE.
D. FC = 3500 PSI @ 28 DAYS - ALL CONCRETE FILL OVER METAL DECK, UNLESS NOTED OTHERWISE.
CONCRETE MIX DESIGNS (INCLUDING AIR CONTENT, WATER TO CEMENT RATIOS, AND OTHER CRITERIA) SHALL CONFORM TO THE REQUIREMENTS SET FORTH IN ACI 318 TABLE 4.3.1, BASED ON THE EXPOSURE CATEGORIES AND CLASSES DEFINED IN ACI 318 TABLE 4.2.1. USE AIR ENTRAINING ADMIXTURE IN ALL EXTERIOR CONCRETE. AIR CONTENT IN FIRE RATED SLABS SHALL ALSO COMPLY WITH THE REQUIREMENTS IN THE SPECIFIED UL LISTING.
COLD WEATHER CONCRETING: PROTECT CONCRETE WORK FROM PHYSICAL DAMAGE OR REDUCED STRENGTH CAUSED BY FROST, FREEZING OR LOW TEMPERATURES. COMPLY WITH ACI 306.1.
HOT WEATHER CONCRETING: WHEN HOT WEATHER CONDITIONS EXIST THAT WOULD IMPAIR THE QUALITY AND STRENGTH OF THE CONCRETE, REDUCE DELIVERY TIME OF READY MIX CONCRETE, LOWER THE TEMPERATURE OF MATERIALS, OR ADD RETARDER TO ENSURE THAT THE CONCRETE IS PLASTIC. RETEMPERING WITH WATER IS NOT ALLOWED. COMPLY WITH ACI 305R.
SLAB CURING: ALL INTERIOR CONCRETE SLABS, EXCEPT EXPOSED INTEGRALLY COLORED SLABS, ARE TO BE CURED WITH A MOISTURE RETAINING COVER FOR THE FIRST 7 DAYS (MINIMUM) AFTER PLACEMENT.
THE CONTRACTOR IS ALLOWED TO CAST FOUNDATIONS AGAINST EXCAVATED SOIL SURFACES, PROVIDED THE FOLLOWING IS ADHERED TO:
A. THE SIDE SLOPES OF THE EXCAVATION SHALL BE ABLE TO MAINTAIN VERTICAL SOIL SURFACE WITHOUT SOIL SLOUGHAGE.
B. THE BOTTOM WIDTH OF THE EXCAVATION SHALL BE ONE INCH WIDER MINIMUM ON EACH SIDE THAN THE SPECIFIED FOOTING WIDTH.
C. THE SIDE WALLS OF THE EXCAVATION SHALL BE BATTERED A MINIMUM OF ONE INCH HORIZONTAL TO TWELVE INCHES VERTICAL.
D. IF SANDY OR LOOSE MATERIALS ARE ENCOUNTERED, THE FOOTING MUST BE FORMED.
E. THE CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVAL OF ANY SOIL SLOUGHAGE FROM THE WET CONCRETE DURING THE CASTING OPERATION.
F. THE CONTRACTOR AGREES TO REMOVE AND RECAST ANY FOOTING WHERE THE ABOVE CONDITIONS ARE NOT MET.
EXPOSED SITE WALLS, RETAINING WALLS, AND STEM WALLS GREATER THAN 30 FEET IN LENGTH SHALL HAVE CONTROL JOINTS INSTALLED AT THE FOLLOWING MAXIMUM SPACING:
12'-0" ON CENTER FOR WALLS 6'-0" MAXIMUM HEIGHT
18'-0" ON CENTER FOR WALLS 10'-0" MAXIMUM HEIGHT
20'-0" ON CENTER FOR WALLS GREATER THAN 10'-0" IN HEIGHT
ALL CONCRETE EXPOSED TO GROUND SHALL BE MANUFACTURED WITH PORTLAND CEMENT TYPE II OR TYPE V.
SEE THE S7.00 SERIES SHEETS FOR TYPICAL CONCRETE DETAILS.
REINFORCING STEEL:
ALL REINFORCING STEEL SHALL BE FABRICATED AND PLACED IN ACCORDANCE WITH THE BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI 318-14), AND DETAILS AND DETAILING OF CONCRETE REINFORCEMENT (ACI 315-99).
ALL REINFORCING STEEL SHALL CONFORM TO ASTM A615 GRADE 60, EXCEPT STIRRUPS, TIES AND INDICATED FIELD-BENT BARS, WHICH SHALL CONFORM TO ASTM A615 GRADE 40.
ALL WELDED WIRE FABRIC SHALL BE DEFORMED AND SHALL CONFORM TO ASTM A479. PROVIDE IN FLAT SHEETS ONLY.
TENSION AND COMPRESSION LAPS IN REINFORCING SHALL CONFORM TO THE LAP SPlice SCHEDULE ON SHEET S-601 AND BE IN ACCORDANCE WITH ACI 318, CHAPTER 12, UNLESS NOTED OTHERWISE.
ALL HORIZONTAL REINFORCING IN FOOTINGS, WALLS AND BEAMS SHALL BE CONTINUOUS AROUND CORNERS OR HAVE BENT (CORNER) BARS OF THE SAME SIZE AND SPACING AS THE HORIZONTAL BARS AND LAP 30 BAR DIAMETERS (24" MINIMUM).
CONCRETE COVER FOR REINFORCING SHALL BE AS FOLLOWS UNLESS OTHERWISE NOTED:
A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3"
B. CONCRETE CAST AGAINST FORMS BUT EXPOSED TO EARTH OR WEATHER:
1. BARS LARGER THAN NO. 5: 2"
2. BARS NO. 5 OR SMALLER: 1 1/2"
C. CONCRETE NOT EXPOSED TO WEATHER OR NOT IN CONTACT WITH GROUND:
1. COLUMNS, GIRDERS AND BEAMS: 1 1/2"
2. STRUCTURAL SLABS, WALLS AND JOISTS (NO. 11 AND SMALLER): 3/4"
D. SLAB ON GRADE: 1 1/2" FROM TOP OF SLAB
E. STRUCTURAL SLABS ON METAL DECK: 1" FROM TOP OF SLAB
FORM TIES SHALL BE EITHER OF THE THREADED OR SNAP-OFF TYPE SO THAT NO METAL WILL BE LEFT WITHIN 1 INCH OF THE SURFACE OF THE WALL. FOLLOWING REMOVAL OF FORM TIES, RECESSES ARE TO BE CAREFULLY FILLED AND POINTED WITH MORTAR.
REINFORCING SHALL NOT BE TACK WELDED OR WELDED IN ANY MANNER UNLESS SPECIFICALLY DETAILED ON THE STRUCTURAL PLANS.
BAR SUPPORTS AND SPACERS FOR REINFORCING SHALL BE PROVIDED IN ACCORDANCE WITH ACI 315-99. REINFORCING SHALL BE SECURELY TIED TO SUPPORTS.
CHAIRS WITH 22 GAGE SAND PLATES OR PRECAST BLOCKS SHALL BE PROVIDED FOR ALL REINFORCING OF CONCRETE IN CONTACT WITH GRADE.
DECK CHAIRS SHALL BE PROVIDED FOR ALL WELDED WIRE FABRIC IN SLABS OVER METAL DECK.

GENERAL STRUCTURAL NOTES

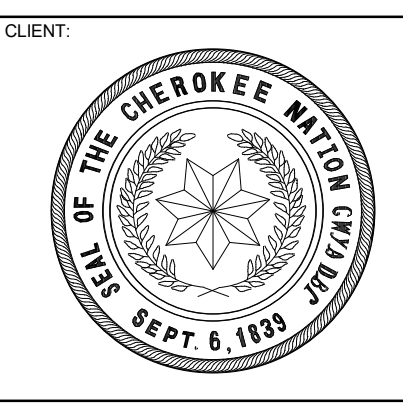
POST INSTALLED ANCHORS:
THE STRUCTURAL DESIGN IS BASED ON THE POST INSTALLED ANCHORING SYSTEMS NOTED BELOW. SINCE ANCHOR CAPACITIES VARY BY MANUFACTURER, THE CONTRACTOR SHALL USE ONLY THE SYSTEMS NOTED BELOW UNLESS AN ALTERNATE IS APPROVED BY THE ENGINEER OF RECORD. ALTERNATE ANCHORING SYSTEMS MAY REQUIRE RE-DESIGN TO VERIFY ANCHOR QUANTITIES, SPACING, AND EMBED DEPTHS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY ADDITIONAL CONSTRUCTION AND RE-DESIGN COSTS ASSOCIATED WITH THE ALTERNATE ANCHORING SYSTEM.
ALL ADHESIVE (EPOXY) FOR POST INSTALLED ANCHORS AND/OR REBAR INTO CONCRETE SHALL BE HILTI HIT-RE 500 V3 OR HIT-HY 200 EPOXY ADHESIVE ANCHORING SYSTEM, HILTI HIT-RE 100 OR HIT-HY 200 EPOXY ADHESIVE SYSTEM OR APPROVED EQUAL. INSTALLATION SHALL BE PER MANUFACTURER'S RECOMMENDATIONS.
ALL ADHESIVE (EPOXY) FOR POST INSTALLED ANCHORS AND/OR REBAR INTO GROUT FILLED MASONRY SHALL BE HILTI HIT HY 70 ADHESIVE ANCHORING SYSTEM OR APPROVED EQUAL. INSTALLATION SHALL BE PER MANUFACTURER'S RECOMMENDATIONS.
ANCHOR LENGTHS (EPOXY) FOR POST INSTALLED ANCHORS AND/OR REBAR INTO HOLLOW MASONRY AND/OR BRICK SHALL BE HILTI HIT HY 70 ADHESIVE ANCHORING SYSTEM OR APPROVED EQUAL. INSTALLATION SHALL BE PER MANUFACTURER'S RECOMMENDATIONS.
ALL POST INSTALLED MECHANICAL ANCHORS INTO CONCRETE SHALL BE HILTI KWIK BOLT TZ EXPANSION ANCHOR OR APPROVED EQUAL. INSTALLATION SHALL BE PER MANUFACTURER'S RECOMMENDATIONS.
ALL POST INSTALLED MECHANICAL ANCHORS INTO GROUT FILLED MASONRY SHALL BE HILTI KWIK BOLT 3 EXPANSION ANCHOR OR APPROVED EQUAL. INSTALLATION SHALL BE PER MANUFACTURER'S RECOMMENDATIONS.
ANCHOR LENGTHS SHOWN FOR ATTACHMENT TO CONCRETE AND/OR MASONRY ARE REQUIRED EMBEDMENT LENGTHS. THE CONTRACTOR SHALL PROVIDE ANCHORS WITH ADDITIONAL LENGTH TO FACILITATE THE REQUIRED CONNECTION.
SUBMIT ALL PROPOSED ANCHORING SYSTEMS INCLUDING ICC-ES REPORTS TO STRUCTURAL ENGINEER FOR REVIEW PRIOR TO INSTALLATION. THE ICC-ES FORMS SHALL MEET THE REQUIREMENTS OF THE IBC REFERENCED IN THESE NOTES.
STRUCTURAL AND MISCELLANEOUS STEEL:
ALL STRUCTURAL STEEL SHALL BE DETAILED AND FABRICATED IN ACCORDANCE WITH THE AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS".
ALL WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A992, GRADE 50, UNLESS NOTED OTHERWISE.
ALL MISCELLANEOUS STEEL MEMBERS, SUCH AS CHANNELS, ANGLES, FLAT BARS, AND PLATES SHALL CONFORM TO ASTM A36 UNLESS NOTED OTHERWISE.
ALL RECTANGULAR AND SQUARE STRUCTURAL TUBING SHALL CONFORM TO ASTM A500, GRADE B, FY = 46 KSI OR ASTM 1085, GRADE B, FY = 50 KSI.
ALL ROUND STRUCTURAL TUBING SHALL CONFORM TO ASTM A500, GRADE B, FY = 42 KSI OR ASTM 1085, GRADE B, FY = 50 KSI.
ALL STRUCTURAL PIPE SHALL CONFORM TO ASTM A53, TYPE E OR S, GRADE B, FY = 35 KSI.
BOLTS SHALL CONFORM TO ASTM A325N TENSION CONTROL BOLTS UNLESS NOTED OTHERWISE, WITH SIZES AS SHOWN ON THE DRAWINGS. WHERE CLEARANCE WITHIN A CONNECTION DOES NOT PERMIT THE USE OF TENSION CONTROL BOLTS, STANDARD A325N BOLTS SHALL BE USED AND INSPECTED IN ACCORDANCE WITH THE AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS".
ALL BOLTS SHALL BE INSTALLED IN A SNUG TIGHT CONDITION EXCEPT AT MOMENT CONNECTIONS, BRACED FRAME CONNECTIONS, AND AT CONNECTIONS DETAILED WITH A325SC BOLTS. AT THESE LOCATIONS, THE BOLTS SHALL BE TIGHTENED SO AS TO SHEAR THE SPLINE OFF THE BOLT.
ANCHOR BOLTS EMBEDDED IN CONCRETE SHALL BE ASTM F1554 GRADE 36 THREADED RODS WITH DOUBLE NUTS. PROVIDE FLAT WASHERS BETWEEN NUTS AND BASEPLATE SURFACES. ANCHOR BOLT LENGTHS SHOWN FOR ATTACHMENT TO CONCRETE AND/OR MASONRY ARE REQUIRED EMBEDMENT LENGTHS. THE CONTRACTOR SHALL PROVIDE ANCHOR BOLTS WITH ADDITIONAL BOLT LENGTH TO FACILITATE THE REQUIRED CONNECTION.
ANCHOR BOLT FLAT WASHERS SHALL BE PROVIDED IN ACCORDANCE WITH TABLE 14-2 OF AISC 360, AISC MANUAL OF STEEL CONSTRUCTION LATEST EDITION.
ALL WELDING SHALL BE DONE IN ACCORDANCE WITH THE LATEST STANDARDS OF THE AWS STRUCTURAL WELDING CODE.
ALL BOLT HOLES THAT ARE REQUIRED TO BE FIELD DRILLED SHALL BE DRILLED WITH A MAG DRILL. FLAME CUTTING OF HOLES OR ENLARGING OF MISALIGNED HOLES WILL NOT BE ALLOWED.
HEADED CONCRETE ANCHORS AND SHEAR CONNECTORS SHALL BE MADE FROM STEEL CONFORMING TO ASTM A108 AND MEET THE MECHANICAL PROPERTIES OF TYPE B, AS REQUIRED BY CHAPTER 7 OF AWS D1.1 "STRUCTURAL WELDING CODE-STEEL", LATEST EDITION. STRUCTURAL STEEL TO RECEIVE SHEAR CONNECTORS SHALL BE FREE OF PAINT. WELDING PREQUALIFICATION REQUIRED.
PROVIDE A SLIDE BEARING CONNECTION FOR STEEL BEAMS BEARING ON MASONRY WALLS UNLESS NOTED OTHERWISE. SEE SHEET S7.41 FOR TYPICAL CONNECTION DETAIL.
SEE S7.00 SERIES SHEETS FOR TYPICAL STEEL DETAILS.
COMPOSITE FLOORS:
THE METAL DECK FOR COMPOSITE FLOORS SHALL BE UNSHORED UNLESS NOTED OTHERWISE.
THE SHEAR CONNECTORS SHALL BE 3/4" DIAMETER X 4 1/2" AT 3" DEEP DECK UNLESS NOTED OTHERWISE. THE SHEAR CONNECTORS SHALL BE MADE FROM STEEL CONFORMING TO ASTM A108 AND MEET THE MECHANICAL PROPERTIES OF TYPE B, AS REQUIRED BY CHAPTER 7 OF AWS D1.1 "STRUCTURAL WELDING CODE-STEEL", LATEST EDITION. STRUCTURAL STEEL TO RECEIVE SHEAR CONNECTIONS SHALL BE FREE OF PAINT. WELDING PREQUALIFICATION REQUIRED.
THE SHEAR CONNECTIONS SHALL NOT BE ADDED UNTIL THE METAL FLOOR DECK IS INSTALLED.
WHERE SHEAR CONNECTIONS AND PUDDLE WELDS COINCIDE, THE SHEAR CONNECTOR MAY REPLACE THE PUDDLE WELD.
CAMBERED BEAMS SHALL HAVE THE CAMBER PUT IN AT 1/3 POINTS OR ALONG A PARABOLIC CURVE.
THE CONTRACTOR SHALL SURVEY THE CAMBER OF THE BEAMS AFTER THE BEAMS HAVE BEEN ERECTED. THE CONTRACTOR SHALL SUBMIT THE SURVEY TO THE ENGINEER FOR REVIEW. THE CONTRACTOR SHALL NOT POUR THE SLAB UNTIL THE ENGINEER HAS REVIEWED AND APPROVED THE BEAM CAMBERS.
CONTRACTOR SHALL SHORE BEAMS WITH A CAMBER MORE THAN 1/2" LOWER THAN SPECIFIED. THE BEAM SHALL BE ALLOWED TO DEFLECT TO LEVEL.
THE CONCRETE FOR THE SLAB SHALL BE POURED AND PLACED TO THE ELEVATION INDICATED ON THE DRAWINGS WHILE MAINTAINING THE MINIMUM THICKNESS. SPREAD CONCRETE OVER AREA OF INFLUENCE TO ROUGH DEPTH IN ORDER TO LOAD BEAMS AND GIRDERS PRIOR TO SETTING SCREED ELEVATIONS.
THE WEIGHT OF THE WET CONCRETE WILL CAUSE DEFLECTIONS OF THE STEEL FRAMING. THEREFORE, CONCRETE OVERRUNS ARE TO BE ANTICIPATED BY THE CONTRACTOR.
CONTRACTOR SHALL CONTINUOUSLY MONITOR THE THICKNESS AND ELEVATIONS DURING CONCRETE PLACING OPERATIONS.
PROVIDE #4 X 6'-0" AT 12" ON CENTER OVER ALL GIRDERS OF COMPOSITE FLOORS.
PROVIDE #4 X 6'-0" AT 12" ON CENTER OVER SHORED BEAMS THAT ARE NOT ALLOWED TO DEFLECT TO LEVEL.
PROVIDE WELDED WIRE FABRIC AS INDICATED ON DRAWINGS IN FLAT SHEETS ONLY.
PROVIDE DECK CHAIRS FOR ALL WELDED WIRE FABRIC IN SLABS OVER METAL DECK.
STEEL JOISTS:
STEEL JOISTS SHALL BE MANUFACTURED BY A MEMBER OF SJI.
STEEL JOISTS SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE STEEL JOIST INSTITUTE STANDARD SPECIFICATIONS.
NO CONSTRUCTION LOADS OF ANY KIND SHALL BE PLACED ON UNBRIDGED JOISTS.
WHERE COLUMNS ARE NOT FRAMED IN AT LEAST TWO DIRECTIONS WITH STRUCTURAL STEEL MEMBERS, JOISTS AT OR CLOSEST TO COLUMN LINES SHALL BE FIELD BOLTED TO ADD LATERAL STABILITY DURING CONSTRUCTION.
PROVIDE BRIDGING IN ACCORDANCE WITH THE LATEST EDITION OF THE SJI STANDARD SPECIFICATIONS AND OSHA REQUIREMENTS.
THE STRUCTURAL DRAWINGS ARE NOT STAND-ALONE DOCUMENTS. THE CONTRACTOR SHALL COORDINATE THE LOCATIONS AND WEIGHTS OF ALL MECHANICAL, PLUMBING, ELECTRICAL AND OTHER EQUIPMENT WITH THE APPLICABLE DRAWINGS. THE JOIST SUPPLIER SHALL ACCOUNT FOR THE LOADS IN THEIR DESIGN.
THE STEEL JOIST MANUFACTURER SHALL DESIGN ROOF JOISTS SUPPORTING MECHANICAL UNITS, INDICATED AS SP JOISTS ON THE DRAWINGS. FOR 1.2x MECHANICAL UNIT WEIGHTS SHOWN, USE 25 PSF DEAD LOAD AND 20 PSF LIVE LOAD (NON-REDUCIBLE) UNLESS NOTED OTHERWISE. CONTRACTOR SHALL VERIFY ACTUAL MECHANICAL LOADS. NOTIFY STEEL JOIST MANUFACTURER OF ANY DISCREPANCIES.
JOIST DEFLECTIONS SHALL BE LIMITED PER SJI REQUIREMENTS AND SHALL NOT BE LESS THAN THE FOLLOWING:
L/360 FOR ROOF LIVE LOAD WITH STANDARD SJI CAMBER WHERE PLASTER OR STUCCO CEILINGS ARE SUPPORTED.
L/240 FOR ROOF LIVE LOAD WITH STANDARD SJI CAMBER WHERE NONPLASTER CEILINGS ARE SUPPORTED.
L/180 FOR ROOF LIVE LOAD WITH STANDARD SJI CAMBER WHERE NO CEILINGS ARE SUPPORTED AND PROVISIONS FOR FUTURE CEILINGS ARE NOT REQUIRED.
STEEL ROOF JOISTS SHALL BE DESIGNED FOR A NET WIND UPLIFT LOAD OF 15 PSF UNLESS NOTED OTHERWISE. THE DEAD LOAD OF MISCELLANEOUS ROOFTOP ITEMS, INCLUDING SCREEN WALLS, SKYLIGHTS, FIRE SUPPRESSION SYSTEM, SOLAR PHOTOVOLTAIC SYSTEM, ETC., SHALL BE ACCOUNTED FOR IN THE DESIGN OF THE STEEL ROOF JOISTS. THE CONTRACTOR SHALL COORDINATE THE MISCELLANEOUS LOADS WITH THE STEEL JOIST MANUFACTURER.

GENERAL STRUCTURAL NOTES

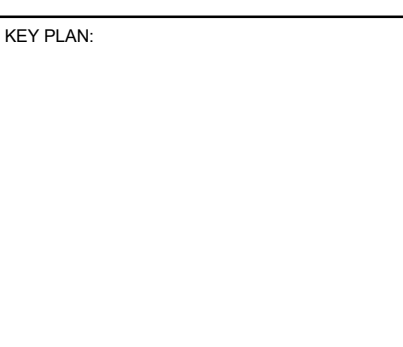
JOIST BEARING SEATS SHALL BEAR ON STEEL SUPPORTS AND SHALL BE CONNECTED AS FOLLOWS UNLESS NOTED OTHERWISE:
KKCS/E: TWO 1/8" x 2 1/2" LONG FILLET WELDS
LH02-06 (OR 2 1/2" AND SMALLER TOP CHORD ANGLE LEG): TWO 3/16" x 2 1/2" LONG FILLET WELDS
LH01H07-17 (OR 3 1/2" AND SMALLER TOP CHORD ANGLE LEG): TWO 1/4" x 2 1/2" LONG FILLET WELDS
LH01H 18-25 (OR 4" AND LARGER TOP CHORD ANGLE LEG): TWO 1/4" x 4" LONG FILLET WELDS
ALL WELDS SHALL MEET CURRENT MINIMUM SJI REQUIREMENTS.
STEEL DECK:
ALL STEEL DECK SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE STEEL DECK INSTITUTE SPECIFICATIONS.
SEE PLANS FOR STEEL DECK TYPE, GAGE, FINISH AND CONNECTIONS.
PROVIDE A MINIMUM OF 1 1/2" BEARING FOR ALL STEEL DECK.
ALL SPLICES AND LAPS SHALL BE A MINIMUM OF 2" IN LENGTH AND SHALL BE LOCATED DIRECTLY ABOVE SUPPORTS.
ALL DECKING SHALL BE CONTINUOUS OVER TWO OR MORE SPANS.
POWDER DRIVEN FASTENERS SHALL BE EQUIVALENT TO:
HILTI X-HSN 24 FOR STEEL BASE MATERIAL 1/4" UP TO 3/8".
HILTI ENP-19 FOR STEEL BASE MATERIAL 1/4" OR THICKER.
MASONRY:
ALL MASONRY UNITS SHALL COMPLY WITH ASTM C 90 WITH A COMPRESSIVE STRENGTH OF 2000 PSI (NET AREA).
FM = 1900 PSI
MORTAR SHALL BE TYPE S.
GROUT - FC = 2000 PSI, MINIMUM.
CELLS CONTAINING REBAR SHALL BE GROUTED SOLID FROM THE BOTTOM TO THE TOP OF THE WALL IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE.
ALL CELLS BELOW GRADE SHALL BE GROUTED SOLID UP TO GRADE.
CELLS CONTAINING EXPANSION ANCHORS SHALL BE GROUTED SOLID.
ALL VERTICAL REBAR SHALL BE IN PLACE AND SECURED WITH REBAR POSITIONERS PRIOR TO GROUTING.
COVER FOR REINFORCING SHALL BE AS FOLLOWS UNLESS OTHERWISE NOTED:
A. MASONRY FACE NOT EXPOSED TO EARTH OR WEATHER: 1 1/2"
B. MASONRY FACE EXPOSED TO EARTH OR WEATHER:
1. BARS LARGER THAN NO. 5: 2"
2. BARS NO. 5 OR SMALLER: 1 1/2"
UNLESS OTHERWISE NOTED MASONRY CELLS SHALL BE GROUTED IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE (MAXIMUM 5 FOOT GROUT LIFTS).
LAP REBAR PER THE SCHEDULE ON S6.01.
WHERE REBAR LAP SPLICES EXCEED 5 FT GROUT LIFTS, 8 FT GROUT LIFTS MAY BE USED WITH CLEANOUTS PROVIDED AT THE BOTTOM OF EACH VERTICALLY REINFORCED CELL. SOLID GROUTED WALLS SHALL HAVE CLEANOUTS AT 32" ON CENTER MAXIMUM.
ALL HORIZONTAL REINFORCING IN BOND BEAMS SHALL BE CONTINUOUS AROUND CORNERS OR HAVE BENT (CORNER) BARS OF THE SAME SIZE AND A LAP AS NOTED ABOVE. VERTICAL STEEL SHALL CONTINUE THROUGH BOND BEAMS.
PROVIDE STANDARD LADDER TYPE JOINT REINFORCING AT 16" ON CENTER (ALTERNATE COURSES) UNLESS NOTED OTHERWISE IN THE PROJECT DOCUMENTS. USE PREFABRICATED CORNERS AND TEES AT ALL WALL CORNERS AND INTERSECTIONS RESPECTIVELY.
PROVIDE A SLIDE BEARING CONNECTION FOR STEEL BEAMS BEARING ON MASONRY WALLS UNLESS NOTED OTHERWISE. SEE SHEET S7.41 FOR TYPICAL CONNECTION DETAIL.
SEE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR INFORMATION REGARDING MASONRY COLORS, FINISHES, BOND, ETC. AT ALL EXPOSED MASONRY WALLS.
ALL MASONRY WALL CONFIGURATIONS INCLUDING WALL OPENINGS SHALL BE COORDINATED WITH CIVIL, MECHANICAL, PLUMBING, ELECTRICAL AND DRAWINGS FROM ALL OTHER DISCIPLINES.
EXPOSED MASONRY SITE WALLS AND RETAINING WALLS GREATER THAN 16 FEET IN LENGTH SHALL HAVE MASONRY CONTROL JOINTS INSTALLED AT THE FOLLOWING MINIMUM SPACING:
12'-0" ON CENTER FOR WALLS 6'-0" MAXIMUM HEIGHT
18'-0" ON CENTER FOR WALLS 10'-0" MAXIMUM HEIGHT
20'-0" ON CENTER FOR WALLS GREATER THAN 10'-0" IN HEIGHT
SEE S7.00 SERIES SHEETS FOR TYPICAL MASONRY DETAILS.
VENEER:
FOR CMU OR BRICK VENEER (5" MAXIMUM, 3" MINIMUM THICKNESS) ATTACHMENT TO STRUCTURAL MASONRY, PROVIDE ADJUSTABLE INTEGRAL ANCHOR TIES. ADJUSTABLE INTEGRAL ANCHOR TIES SHALL BE CORROSION RESISTANT AND HAVE TWO PINTLE LEGS MINIMUM WITH W2.8 (3/16)" WIRE OR APPROVED EQUAL. PROVIDE DUR-O-WALL DA37N ADJUSTABLE INTEGRAL ANCHOR TIES OR APPROVED EQUAL.
FOR CMU OR BRICK VENEER (5" MAXIMUM, 3" MINIMUM THICKNESS) ATTACHMENT TO STRUCTURAL CONCRETE, PROVIDE ADJUSTABLE ANCHOR TIES. ADJUSTABLE ANCHOR TIES SHALL BE CORROSION RESISTANT AND HAVE A TWO PINTLE LEGS MINIMUM WITH A MINIMUM W2.8 (3/16)" WIRE. ATTACH TO CONCRETE WITH 2-1/4" DIAMETER CONCRETE SCREWS, HILTI KWIKCON 11 x 1 1/2" OR APPROVED EQUAL.
FOR CMU OR BRICK VENEER (5" MAXIMUM, 3" MINIMUM THICKNESS) ATTACHMENT TO STRUCTURAL COLD FORMED METAL STUDS, PROVIDE ADJUSTABLE ANCHOR TIES. ADJUSTABLE ANCHOR TIES SHALL BE CORROSION RESISTANT AND HAVE TWO PINTLE LEGS MINIMUM W2.8 (3/16)" WIRE. PROVIDE DUR-O-WALL DA213 ADJUSTABLE ANCHOR TIE OR APPROVED EQUAL. ATTACH THROUGH SHEATHING TO STUDS WITH 2-1/4" x 1 1/2" CORROSION RESISTANT TEK SCREWS.
SEE TYPICAL DETAILS ON SHEET S7.31 FOR VENEER TIE SPACING.
PROVIDE ADDITIONAL ANCHORS AROUND ALL OPENINGS LARGER THAN 16" IN EITHER DIMENSION. SPACE ANCHORS WITHIN 12" OF OPENING PERIMETER AND MATCH HORIZONTAL OR VERTICAL ANCHOR TIE SPACING.
COORDINATE VENEER LOCATION, TYPE, BOND PATTERN, ETC. WITH ARCHITECTURAL DRAWINGS.
PRE-ENGINEERED METAL BUILDING:
FOUNDATION CONFIGURATION AND SIZES SHOWN ON THESE DRAWINGS ARE BASED ON PRELIMINARY DESIGN CALCULATIONS. THESE SIZES MAY REQUIRE MODIFICATIONS PER THE METAL BUILDING MANUFACTURER'S FINAL GRAVITY AND LATERAL DESIGN CALCULATIONS.
THE METAL BUILDING MANUFACTURER SHALL PROVIDE FINAL GRAVITY AND LATERAL DESIGN CALCULATIONS FOR APPROVAL PRIOR TO THE COMMENCEMENT OF FOUNDATION EARTHWORK.
THE BUILDING SHALL BE A MANUFACTURER'S STANDARD PREFABRICATED METAL STRUCTURE OF THE APPROXIMATE INSIDE AREA SHOWN, EXCEPT AS NOTED. RIGID FRAMES SHALL BE SPACED AS SPECIFIED ON THE DRAWINGS, BUT OVERALL DIMENSIONS AND CONSTRUCTION DETAILS MAY VARY TO SUIT MANUFACTURER'S STANDARD DESIGN. MINIMUM WEB THICKNESS OF RIGID FRAMES SHALL BE 3/16".
THE BUILDING SHALL BE DESIGNED AND FABRICATED ACCORDING TO AISC, MBMA AND AISI SPECIFICATIONS. THE DIMENSIONAL TOLERANCES APPLICABLE TO ROLLED FORM STEEL UNDER THE LATEST EDITION OF THE AISC "STANDARD MILL PRACTICE" SECTION SHALL BE REQUIRED IN THE FABRICATION OF THE STEEL BUILDING FRAMES.
THE BUILDING FRAME SHALL BE DESIGNED TO LIMIT THE LATERAL DEFLECTION TO H/240 INCH AT THE BUILDING EAVE FOR THE SPECIFIED BASIC WIND SPEED.
THE BUILDING SHALL BE DESIGNED TO SUPPORT ALL MECHANICAL EQUIPMENT INCLUDING HEATERS, SPRINKLERS, EXHAUST SYSTEMS AND ALL OTHER DEVICES. ADDITIONAL GIRTS OR PURLINS SHALL BE PLACED IN CONVENIENT LOCATIONS FOR ATTACHMENT OF ALL MECHANICAL EQUIPMENT. THE CONTRACTOR SHALL COORDINATE THE MECHANICAL LOADS WITH THE METAL BUILDING MANUFACTURER AND THE MECHANICAL DRAWINGS.
DESIGN LOADS SHALL CONFORM WITH THESE GENERAL NOTES. LOAD COMBINATIONS SHALL COMPLY WITH MBMA SPECIFICATIONS.
ANCHOR BOLTS SHOWN ON THESE DRAWINGS ARE BASED ON PRELIMINARY DESIGN CALCULATIONS. THESE SIZES MAY REQUIRE MODIFICATIONS PER THE METAL BUILDING MANUFACTURER'S FINAL GRAVITY AND LATERAL DESIGN CALCULATIONS.
THE METAL BUILDING MANUFACTURER SHALL DESIGN THE SUPPORTS FOR ALL CONNECTIONS OF MASONRY AND/OR METAL STUD WALLS TO THE METAL BUILDING COMPONENTS AND PROVIDE CALCULATIONS FOR THE DESIGN OF THE SUPPORTS.
PREPARE THE SHOP DRAWINGS AND CALCULATIONS UNDER THE SEAL OF A PROFESSIONAL STRUCTURAL ENGINEER REGISTERED IN THE STATE THE PROJECT IS LOCATED.
COORDINATE WITH ARCHITECTURAL DRAWINGS.
GLASS CURTAIN WALL SYSTEM:
ALL LATERAL AND GRAVITY SUPPORT FOR THE GLASS CURTAIN WALL SYSTEM SHALL BE PER THE MANUFACTURER'S RECOMMENDATIONS. SHOP DRAWINGS AND STAMPED CALCULATIONS SHALL BE SUBMITTED FOR APPROVAL BY THE ENGINEER OF RECORD AND THE ARCHITECT PRIOR TO INSTALLATION.
THE ENGINEER STAMPING THE SHOP DRAWINGS SHALL BE REGISTERED IN THE STATE THAT THE PROJECT IS LOCATED.
THE BEAMS AT ALL FLOORS HAVE BEEN DESIGNED TO SUPPORT THE GRAVITY LOAD OF THE GLASS CURTAIN WALL SYSTEM. THE GLASS CURTAIN WALL SYSTEM SHALL BE LATERALLY SUPPORTED AT ALL FLOORS AND ROOF LEVEL.



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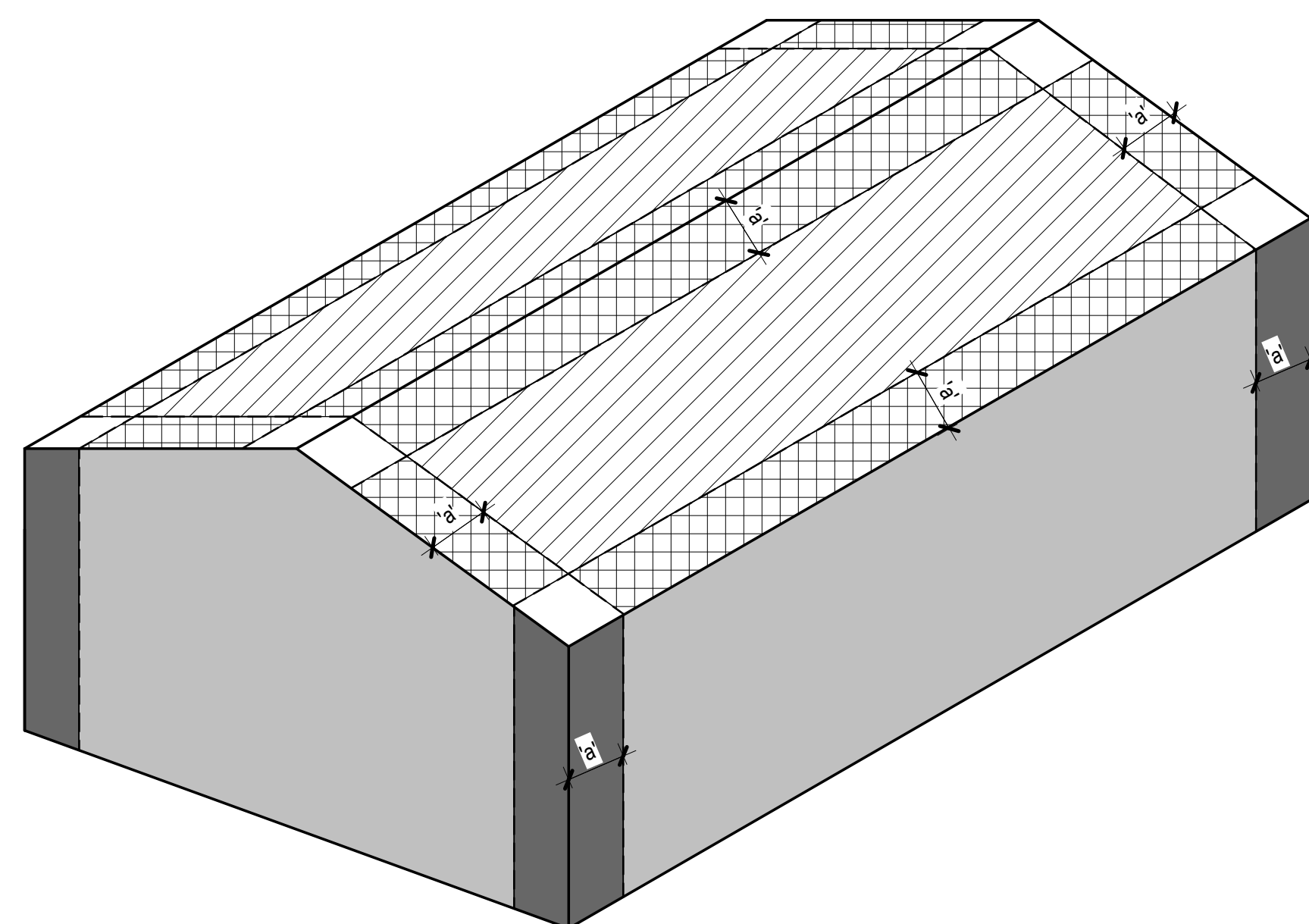
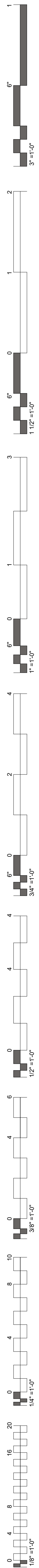
PROJECT PHASE:
BID PACKAGE 01

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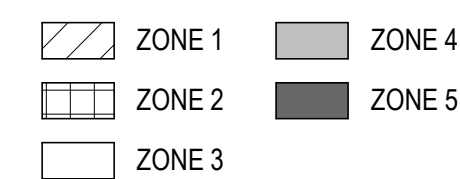
DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER: S0.02

GENERAL STRUCTURAL NOTES



COMPONENTS AND CLADDING WIND PRESSURES (PSF) CALCULATED AT MEAN ROOF HEIGHT = {} FEET			
a = {} FT	EFFECTIVE WIND AREA (FT ²)		
ZONE	10	100	500
1	34.0	32.0	30.6
2	59.1	52.0	47.3
1 AND 2 OVERHANGS	59.1	59.1	59.1
3	87.5	77.0	70.0
3 OVERHANGS	100.7	94.6	90.6
4	40.3	33.5	28.2
4 PARAPETS	96.3	79.9	67.4
5	62.3	51.7	43.6
5 PARAPETS	121.5	100.9	85.1



A1 COMPONENT AND CLADDING WIND LOADING DIAGRAM
SCALE: 3/8" = 1'-0"

GENERAL STRUCTURAL NOTES

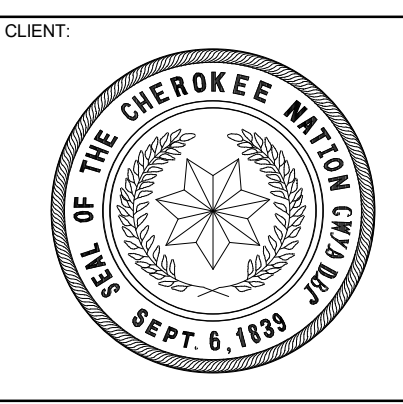
ELEVATORS:
THE STRUCTURE HAS BEEN DESIGNED FOR A KONE ELEVATOR.
ALL STRUCTURAL SUPPORTS, FLOOR PENETRATION SIZES AND PIT DIMENSIONS HAVE BEEN DESIGNED BASED ON THE ABOVE INFORMATION. SHOULD THE ACTUAL ELEVATOR(S) SELECTED DIFFER FROM THE INFORMATION PROVIDED ABOVE, THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY ADDITIONAL CONSTRUCTION AND REDESIGN COSTS ASSOCIATED WITH THE ALTERNATE ELEVATOR(S).
THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING ALL ELEVATOR PIT AND FLOOR PENETRATION LOCATIONS AND DIMENSIONS.
THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING ALL ELEVATOR OVERRUN REQUIREMENTS.
ELEVATOR CAR RAIL AND COUNTERWEIGHT RAIL SUPPORTS SHALL BE PROVIDED BY THE CONTRACTOR. CAR RAIL AND COUNTERWEIGHT RAIL SUPPORTS SHALL BE PROVIDED AT AND BETWEEN ALL FLOOR LEVELS SERVICED BY THE ELEVATOR, ABOVE THE LAST STOP OF THE ELEVATOR, AND BETWEEN THE BASEMENT AND THE GROUND FLOOR AS REQUIRED BY THE ELEVATOR MANUFACTURER. IF THE ELEVATOR MANUFACTURER REQUIRES RAIL SUPPORTS THAT DIFFER FROM THOSE PROVIDED, THE ELEVATOR MANUFACTURER SHALL BE RESPONSIBLE FOR ADDITIONAL CONSTRUCTION COST AND DESIGN COST.
THE CONTRACTOR SHALL VERIFY THE DESIGN OF THE HOIST/SAFETY BEAM AND CONNECTIONS AS REQUIRED PER THE ELEVATOR MANUFACTURER.
STRUCTURAL ELEMENTS AFFECTED BY THE ELEVATOR LAYOUT SHALL NOT BE FABRICATED PRIOR TO APPROVAL OF ELEVATOR SHOP DRAWINGS.
SPECIAL INSPECTION:
THE OWNER SHALL PROVIDE FOR SERVICES OF A CERTIFIED INSPECTOR (APPROVED BY THE BUILDING OFFICIAL OR THE ENGINEER OF RECORD) IN ACCORDANCE WITH CHAPTER 17 OF THE INTERNATIONAL BUILDING CODE FOR THE SPECIAL INSPECTION ITEMS NOTED ON SHEET S0.03.
DEFERRED SUBMITTALS:
THE DEFERRED SUBMITTALS LISTED BELOW ARE THOSE PORTIONS OF THE DESIGN THAT ARE NOT COMPLETED AT THE TIME OF APPLICATION AND ARE TO BE SUBMITTED TO THE BUILDING OFFICIAL AND APPROVED PRIOR TO THE INSTALLATION OF THOSE ITEMS. THE MANUFACTURER, CONSULTANT, OR CONTRACTOR, AS APPROPRIATE, SHALL PROVIDE SUBMITTALS TO THE ENGINEER OF RECORD FOR REVIEW FOR THE FOLLOWING ITEMS:
SPECIAL STEEL JOISTS
METAL STAIRS
EXTERIOR COLD-FORMED METAL FRAMING
INTERIOR COLD-FORMED METAL FRAMING
TEMPORARY SHORING
HANDRAILS
CURTAIN WALL AND STOREFRONT
AGGREGATE PIERS / STONE COLUMN GROUND IMPROVEMENT
PRECAST CONCRETE COLUMNS AND ASSOCIATED ATTACHMENTS AND ANCHORAGE

SCHEDULE OF STRUCTURAL SPECIAL INSPECTIONS

- SPECIAL INSPECTIONS / TESTING - "SPECIAL STRUCTURAL INSPECTION" SHALL NOT RELIEVE THE OWNER OR THEIR AGENT FROM HAVING THE INSPECTIONS OF THE JURISDICTION BUILDING DEPARTMENT PER SECTION 110 OF THE IBC PERFORMED, BOTH THE JURISDICTION BUILDING DEPARTMENT INSPECTIONS AND "SPECIAL STRUCTURAL INSPECTION" SHALL BE PERFORMED.
- THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR NOTIFYING THE JURISDICTION BUILDING OFFICIAL AND SPECIAL INSPECTOR WHEN WORK IS READY FOR INSPECTION.
- REPORTING FOR SPECIAL INSPECTION - SPECIAL INSPECTION AND TESTING REPORTS SHALL BE COMPLETED AND DISTRIBUTED AT THE COMPLETION OF EACH TASK. IF A TASK IS TO TAKE LONGER THAN THREE (3) DAYS, PROVIDE REPORTS FOR EACH DAY. PROVIDE COPIES OF REPORTS TO CONTRACTOR, OWNER, ARCHITECT AND STRUCTURAL ENGINEER OF RECORD. SPECIAL INSPECTOR TO KEEP A NON-COMPLIANCE LIST DOCUMENTING ITEMS INSPECTED NOT MEETING APPROVED CONSTRUCTION DOCUMENTS AND WHEN / HOW RESOLVED.
- SEE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING CONSTRUCTION DOCUMENTS FOR ADDITIONAL NON-STRUCTURAL SPECIAL INSPECTION ITEMS.
- SPECIAL INSPECTION OF SHOP FABRICATED MEMBERS AND ASSEMBLIES SHALL BE IN ACCORDANCE WITH SECTION 1704.2, UNLESS FABRICATOR IS APPROVED TO PERFORM WORK WITHOUT SPECIAL INSPECTION.
- IN ACCORDANCE WITH IBC CHAPTER 17, THE OWNER OR THE OWNER'S AGENT, OTHER THAN THE CONTRACTOR, SHALL EMPLOY ONE OR MORE APPROVED AGENCIES TO PROVIDE SPECIAL INSPECTIONS AND TESTS, DURING CONSTRUCTION FOR THE TYPES OF WORK LISTED BELOW THESE SPECIAL INSPECTIONS AND TESTS ARE IN ADDITION TO THE INSPECTIONS BY THE BUILDING OFFICIAL IDENTIFIED IN IBC SECTION 110.
- DEFINITIONS:
* **SPECIAL INSPECTION:** INSPECTION AS HEREIN REQUIRED BY A QUALIFIED SPECIAL INSPECTOR COMPETENT WITH THE MATERIALS, INSTALLATION, FABRICATION, ERECTION OR PLACEMENT OF COMPONENTS AND CONNECTIONS REQUIRING SPECIAL EXPERTISE TO ENSURE COMPLIANCE WITH APPROVED CONSTRUCTION DOCUMENTS AND REFERENCED STANDARDS (SEE SECTION 1704).
* **CONTINUOUS SPECIAL INSPECTION:** FULL-TIME OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED.
* **PERIODIC SPECIAL INSPECTION:** THE PART-TIME OR INTERMITTENT OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK HAS BEEN OR IS BEING PERFORMED AND AT THE COMPLETION OF THE WORK.

ITEM	DESCRIPTION OF REQUIREMENTS	REQUIRED (YES/NO)
SPECIAL INSPECTION OF STRUCTURAL STEEL	TO BE PERFORMED IN ACCORDANCE WITH CHAPTER N OF AISC 360-10	YES
SPECIAL INSPECTION AND VERIFICATION OF STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.2	YES
SPECIAL INSPECTIONS AND VERIFICATIONS FOR CONCRETE CONSTRUCTION	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.3	YES
SPECIAL INSPECTIONS AND VERIFICATIONS FOR MASONRY CONSTRUCTION	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.4 AND REFERENCED STANDARDS	YES
SPECIAL INSPECTIONS AND VERIFICATIONS FOR WOOD CONSTRUCTION	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.5	NO
SPECIAL INSPECTIONS AND VERIFICATIONS OF SOILS	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.6, THE GEOTECHNICAL REPORT LISTED IN THE GENERAL FOUNDATION NOTES, AND ANY OTHER REQUIREMENTS LISTED IN THE GENERAL FOUNDATION NOTES	YES
SPECIAL INSPECTIONS AND VERIFICATIONS FOR DEEP FOUNDATIONS (DRIVEN PILES, CAST-IN-PLACE, OR HELICAL PILES AS APPLICABLE)	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTIONS 1705.7-1705.9 AS APPLICABLE, THE GEOTECHNICAL REPORT LISTED IN THE GENERAL FOUNDATION NOTES, AND ANY OTHER REQUIREMENTS LISTED IN THE CONSTRUCTION DOCUMENTS	NO
SPECIAL INSPECTIONS FOR WIND RESISTANCE (REQUIRED ONLY FOR V _W = 155MPH OR GREATER IN EXPOSURE CATEGORY B, OR V _W = 142MPH OR GREATER IN EXPOSURE CATEGORY C OR D)	TO BE PERFORMED IN ACCORDANCE WITH IBC SECTION 1705.11	NO
SPECIAL INSPECTIONS AND VERIFICATIONS FOR SEISMIC RESISTANCE (REQUIRED FOR STRUCTURES ASSIGNED TO CATEGORIES C, D, E, OR F)	TO BE PERFORMED IN ACCORDANCE WITH ALL APPLICABLE PORTIONS OF IBC SECTIONS 1705.12 AND 1705.13	NO

ADDITIONAL INSPECTIONS REQUIRED PER SIDEPLATE SYSTEMS ON SHEET S8.01



**WILMA P. MANKILLER HEALTH CENTER
EXPANSION**
 STILWELL, OKLAHOMA

KEY PLAN:

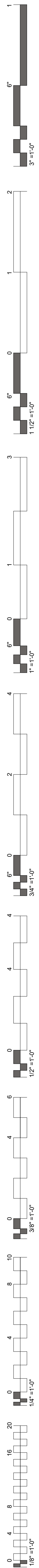
PROJECT PHASE:
BID PACKAGE 01

#	DATE	REVISIONS DESCRIPTION

DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER: S0.03

GENERAL STRUCTURAL NOTES AND SPECIAL INSPECTIONS



GENERAL STRUCTURAL NOTES

GENERAL:

STRUCTURAL DRAWINGS ARE NOT STAND-ALONE DOCUMENTS AND ARE INTENDED TO BE USED IN CONJUNCTION WITH CIVIL, ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND DRAWINGS FROM OTHER DISCIPLINES. THE CONTRACTOR SHALL COORDINATE ALL REQUIREMENTS OF THE CONTRACT DOCUMENTS INTO THE SHOP DRAWINGS AND FIELD WORK.

COORDINATE DIMENSIONS OF ALL OPENINGS, DEPRESSIONS, BLOCKOUTS, ETC. WITH ARCHITECTURAL DRAWINGS, DRAWINGS FROM OTHER DISCIPLINES, PROJECT SHOP DRAWINGS, AND FIELD CONDITIONS PRIOR TO SHOP DRAWING SUBMITTAL. THE STRUCTURAL DRAWINGS ONLY REPRESENT A PORTION OF THE REQUIREMENTS FOR THE PROJECT.

SEE ARCHITECTURAL PLANS FOR INTERIOR NON-BEARING PARTITION WALLS. PARTITION FRAMING SHALL BE CONNECTED TO THE PRIMARY STRUCTURE TO ALLOW FOR VERTICAL LIVE LOAD DEFLECTIONS OF SPAN/60 FOR FLOOR FRAMING AND SPAN/240 FOR ROOF FRAMING.

CONTRACTOR'S CONSTRUCTION AND/OR ERECTION SEQUENCES SHALL RECOGNIZE AND CONSIDER THE EFFECTS OF THERMAL MOVEMENTS OF STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PERIOD.

THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS IN THE FIELD.

SHOP DRAWINGS SHALL BE FURNISHED AND REVIEWED BEFORE ANY FABRICATION OR ERECTION IS STARTED. THE CONTRACTOR SHALL REVIEW AND APPROVE SHOP DRAWINGS PRIOR TO SUBMITTAL TO THE ARCHITECT FOR REVIEW. POORLY EXECUTED SHOP DRAWINGS WILL BE REJECTED AND SHALL BE RESUBMITTED.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING SAFE AND ADEQUATE SHORING FOR ALL PARTS OF THE STRUCTURE DURING CONSTRUCTION.

TEMPORARY PROVISIONS SHALL BE MADE FOR STRUCTURAL STABILITY DURING CONSTRUCTION. THE STRUCTURE SHOWN ON THE DRAWINGS HAS BEEN DESIGNED FOR STABILITY UNDER FINAL CONFIGURATION.

NOTCHING OR CUTTING ANY STRUCTURAL MEMBER IN THE FIELD IS PROHIBITED.

THE CONTRACTOR SHALL VERIFY THE SIZE AND LOCATION OF FOUNDATIONS UNDER MECHANICAL AND ELECTRICAL EQUIPMENT AS REQUIRED. NO CONCRETE PADS SHALL BE LOCATED ON ROOF UNLESS SHOWN ON THE STRUCTURAL DRAWINGS.

BACKFILL SHALL NOT BE PLACED BEHIND RETAINING WALLS UNTIL CONCRETE HAS ATTAINED 100 PERCENT OF DESIGN STRENGTH.

BACKFILL SHALL NOT BE PLACED BEHIND BASEMENT WALLS UNTIL THE CONCRETE HAS ATTAINED 100 PERCENT OF DESIGN STRENGTH AND THE ELEVATED FLOOR PROVIDING LATERAL SUPPORT AT THE TOP OF THE WALL IS COMPLETELY CONSTRUCTED, OR TEMPORARY BRACING/SHORING OF THE WALL IS PROVIDED. DESIGN OF ANY TEMPORARY WALL BRACING/SHORING IS THE RESPONSIBILITY OF THE CONTRACTOR.

REMOVAL OF FORMS AND SHORING SHALL BE IN ACCORDANCE WITH ACI 347. WHERE CONCRETE MUST SUPPORT SUPERIMPOSED LOADS PRIOR TO ATTAINING THE SPECIFIED DESIGN STRENGTH, RESHORE CONCRETE IN ACCORDANCE WITH ACI 347. RESHORING SHALL NOT BE REMOVED SOONER THAN 28 DAYS FROM THE DATE OF POUR OR UNTIL CONCRETE HAS ATTAINED THE SPECIFIED DESIGN STRENGTH.

THE CONTRACTOR SHALL SUBMIT FOR PRIOR APPROVAL THE END OF FOUR LOCATIONS FOR CONCRETE GRADE BEAMS, CONCRETE COLUMNS, AND CONCRETE BEAMS.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR ADHERING TO ALL APPLICABLE STANDARDS SET FORTH BY OSHA, INCLUDING THE FOLLOWING REQUIREMENTS FROM STANDARDS - 29 CFR, SECTION 1926, SUBPART R:

A. THE STEEL ERECTION CONTRACTOR SHALL NOT ERECT STEEL UNLESS THEY HAVE RECEIVED WRITTEN NOTIFICATION FROM THE CONTRACTOR THAT THE CONCRETE IN THE FOOTINGS, PIERS AND WALLS OR THE MORTAR IN THE MASONRY PIERS AND WALLS HAS ATTAINED, ON THE BASIS OF AN APPROPRIATE ASTM STANDARD TEST METHOD OF FIELD-CURED SAMPLES, EITHER 75 PERCENT OF THE INTENDED MINIMUM COMPRESSIVE DESIGN STRENGTH OR SUFFICIENT STRENGTH TO SUPPORT THE LOADS IMPOSED DURING STEEL ERECTION. PROVIDE STRUCTURAL ENGINEER A COPY OF WRITTEN NOTIFICATION WHEN IT IS PROVIDED TO THE STEEL ERECTOR.

B. ANCHOR RODS (ANCHOR BOLTS) SHALL NOT BE REPAIRED, REPLACED OR FIELD-MODIFIED WITHOUT THE APPROVAL OF THE PROJECT STRUCTURAL ENGINEER OF RECORD.

PRIOR TO ERECTION OF COLUMNS, THE CONTRACTOR SHALL PROVIDE WRITTEN NOTIFICATION TO THE STEEL ERECTOR IF THERE HAS BEEN ANY REPAIR, REPLACEMENT OR MODIFICATION OF THE ANCHOR RODS (ANCHOR BOLTS).

PROVIDE STRUCTURAL ENGINEER A COPY OF WRITTEN NOTIFICATION WHEN IT IS PROVIDED TO THE STEEL ERECTOR.

C. NO MODIFICATION THAT AFFECTS THE STRENGTH OF A STEEL JOIST OR STEEL JOIST GIRDER SHALL BE MADE WITHOUT THE APPROVAL OF THE PROJECT STRUCTURAL ENGINEER OF RECORD.

D. METAL DECKING HOLES AND OPENINGS SHALL NOT BE CUT UNTIL IMMEDIATELY PRIOR TO BEING PERMANENTLY FILLED WITH THE EQUIPMENT OR STRUCTURE, OR SHALL BE IMMEDIATELY COVERED.

PROTECTION: PROPER PRECAUTIONS SHALL BE TAKEN AT ALL TIMES TO PROTECT VEHICULAR AND PEDESTRIAN TRAFFIC FROM ANY DAMAGE OR INJURY WHICH MAY BE CAUSED, EITHER DIRECTLY OR INDIRECTLY, BY THE WORK INCLUDED ON THESE DRAWINGS. SUCH PRECAUTIONS SHALL INCLUDE THE ERECTION AND MAINTENANCE OF FENCES, BARRICADES, RAILINGS, GUARDS, SIGNS, COVERINGS, LIGHTS, AND OTHER PRECAUTIONS AS MAY BE REQUIRED. IF AT ANY TIME, IN THE OPINION OF THE OWNER OR THE OWNER'S REPRESENTATIVE, PROPER PRECAUTIONS ARE NOT BEING TAKEN TO SECURE THIS PROTECTION, THE CONTRACTOR SHALL AT NO ADDITIONAL COST TO THE OWNER, INSTALL AND MAINTAIN SUCH ADDITIONAL PROTECTION AS MAY BE DIRECTED BY THE OWNER.

POLLUTION CONTROLS: USE WATER SPRINKLING, TEMPORARY ENCLOSURES, AND OTHER SUITABLE METHODS TO LIMIT DUST AND DIRT RISING AND SCATTERING IN THE AIR TO LOWEST PRACTICAL LEVEL. COMPLY WITH GOVERNING REGULATIONS PERTAINING TO ENVIRONMENTAL PROTECTION.

DRAWINGS:

DO NOT SCALE DRAWINGS.

WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL NOTES, AND SPECIFICATIONS, THE MORE STRINGENT REQUIREMENTS SHALL GOVERN. DETAILS ON DRAWINGS TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS. DETAILS NOTED "TYPICAL" APPLY TO ALL SIMILAR CONDITIONS, WHERE NO SPECIFIC DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ELSEWHERE ON THE PROJECT.

DEMOLITION:

NOTCHING OR CUTTING ANY STRUCTURAL MEMBER IN THE FIELD IS PROHIBITED, UNLESS DETAILED OTHERWISE ON THE STRUCTURAL PLANS.

CONTRACTOR SHALL BE RESPONSIBLE FOR ADHERING TO ALL APPLICABLE STANDARDS SET FORTH BY OSHA.

PRIOR TO STARTING DEMOLITION WORK, THE CONTRACTOR SHALL MAKE AN INSPECTION OF ALL SURROUNDING IMPROVEMENTS TO REMAIN, TO DETERMINE AND RECORD THEIR EXISTING PHYSICAL CONDITION.

SHORING AND BRACING: THE CONTRACTOR SHALL FURNISH ALL SHORING, BRACING, AND INCIDENTALS NECESSARY AND REQUIRED FOR THE PROPER SUPPORT AND SAFETY OF ALL MEMBERS AFFECTED BY DEMOLITION WORK.

WHERE DEMOLITION WOULD AFFECT THE STRUCTURAL INTEGRITY OF THE REMAINING STRUCTURE, THE CONTRACTOR SHALL PROVIDE TEMPORARY SUPPORTS. THE CONTRACTOR SHALL NOTIFY THE ENGINEER OF ANY FIELD CONDITION WHICH WOULD PRESENT A HAZARDOUS CONDITION TO THE STRUCTURE BEFORE PROCEEDING.

PROTECTION: PROPER PRECAUTIONS SHALL BE TAKEN AT ALL TIMES TO PROTECT VEHICULAR AND PEDESTRIAN TRAFFIC FROM ANY DAMAGE OR INJURY WHICH MAY BE CAUSED, EITHER DIRECTLY OR INDIRECTLY, BY THE WORK INCLUDED ON THESE DRAWINGS. SUCH PRECAUTIONS SHALL INCLUDE THE ERECTION AND MAINTENANCE OF FENCES, BARRICADES, RAILINGS, GUARDS, SIGNS, COVERINGS, LIGHTS, AND OTHER PRECAUTIONS AS MAY BE REQUIRED. IF AT ANY TIME, IN THE OPINION OF THE OWNER OR THE OWNER'S REPRESENTATIVE, PROPER PRECAUTIONS ARE NOT BEING TAKEN TO SECURE THIS PROTECTION, THE CONTRACTOR SHALL AT NO ADDITIONAL COST TO THE OWNER, INSTALL AND MAINTAIN SUCH ADDITIONAL PROTECTION AS MAY BE DIRECTED BY THE OWNER.

POLLUTION CONTROLS: USE WATER SPRINKLING, TEMPORARY ENCLOSURES, AND OTHER SUITABLE METHODS TO LIMIT DUST AND DIRT RISING AND SCATTERING IN THE AIR TO LOWEST PRACTICAL LEVEL. COMPLY WITH GOVERNING REGULATIONS PERTAINING TO ENVIRONMENTAL PROTECTION.

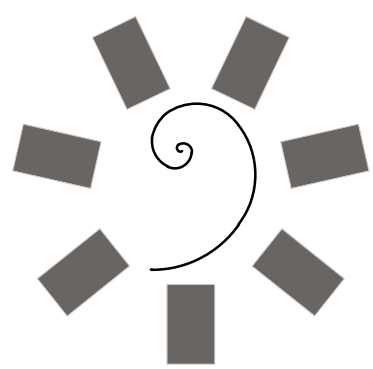
REMOVE DEBRIS FROM THE SITE AS IT ACCUMULATES. UNLESS OTHERWISE NOTED, DO NOT STORE, SELL, BURN, OR OTHERWISE DISPOSE OF DEBRIS ON THE SITE. REMOVAL OF DEBRIS INCLUDES CLEARING OF ALL LOWER LEVELS AND SIMILAR BELOW GRADE STRUCTURES. REMOVE ALL DEBRIS IN SUCH A MANNER AS TO PREVENT SPILLAGE. KEEP ALL PAVEMENTS AND AREAS ADJACENT TO THE SITE CLEAN AND FREE FROM MUD, DIRT, AND DEBRIS AT ALL TIMES.

USE OF EXPLOSIVES: THE CONTRACTOR IS ABSOLUTELY PROHIBITED FROM USING DYNAMITE OR ANY OTHER EXPLOSIVES IN ANY OF THE WORK OR OPERATIONS SHOWN ON THESE PLANS AT THE PROJECT SITE.

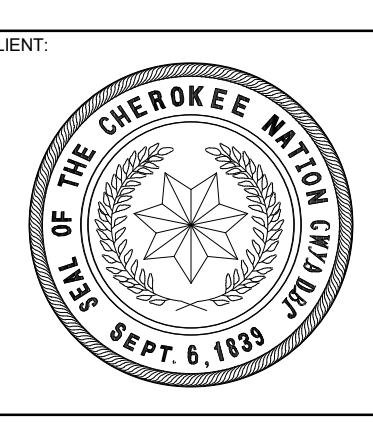
DEMOLITION SHALL BE PERFORMED IN A MANNER THAT WILL NOT DAMAGE ADJOINING SURFACES INDICATED TO REMAIN. SURFACES SHALL BE PATCHED, IF REQUIRED, TO PROVIDE A SUITABLE SUBSTRATE FOR NEW CONSTRUCTION.

SPECIFIC DEMOLITION NOTES ARE NOT TO BE CONSIDERED ALL INCLUSIVE OR COMPLETE IN THEMSELVES. CONTRACTOR SHALL PROVIDE ALL DEMOLITION INCIDENTALS TO OR REQUIRED FOR CONSTRUCTION WHETHER SPECIFICALLY NOTED OR NOT.

STRUCTURAL DEMOLITION DRAWINGS SHOW STRUCTURAL DEMOLITION ONLY. SEE ARCHITECTURAL DRAWINGS FOR DEMOLITION OF EXISTING ARCHITECTURAL ELEMENTS.



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**WILMA P. MANKILLER HEALTH CENTER
EXPANSION**
STILWELL, OKLAHOMA

KEY PLAN:

PROJECT PHASE:

BID PACKAGE 01

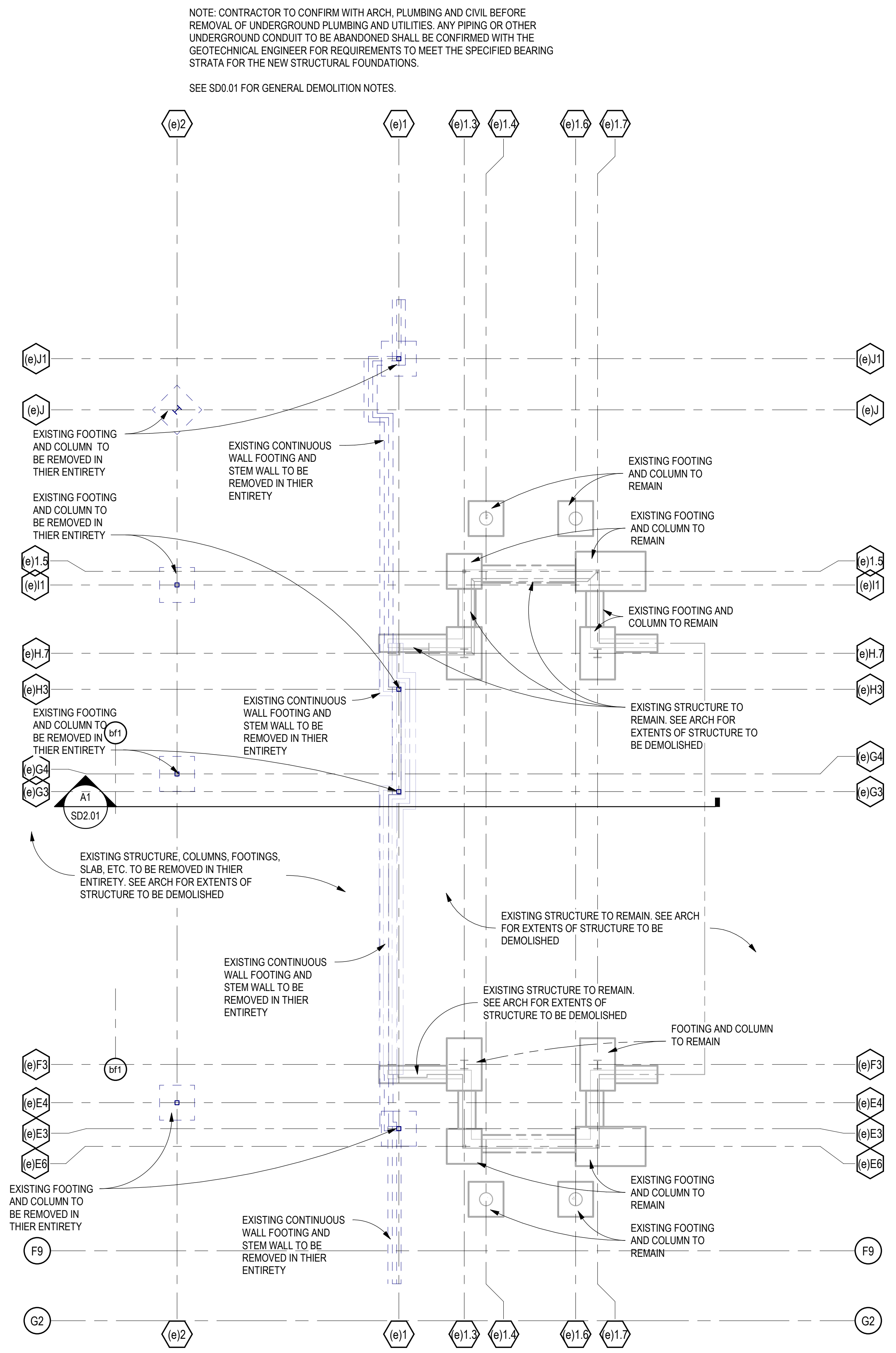
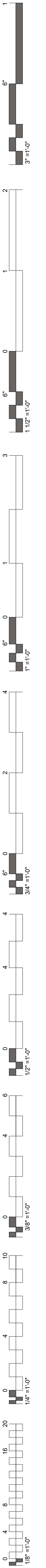
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DATE:	JOB NUMBER:
11-01-19	18-01.01

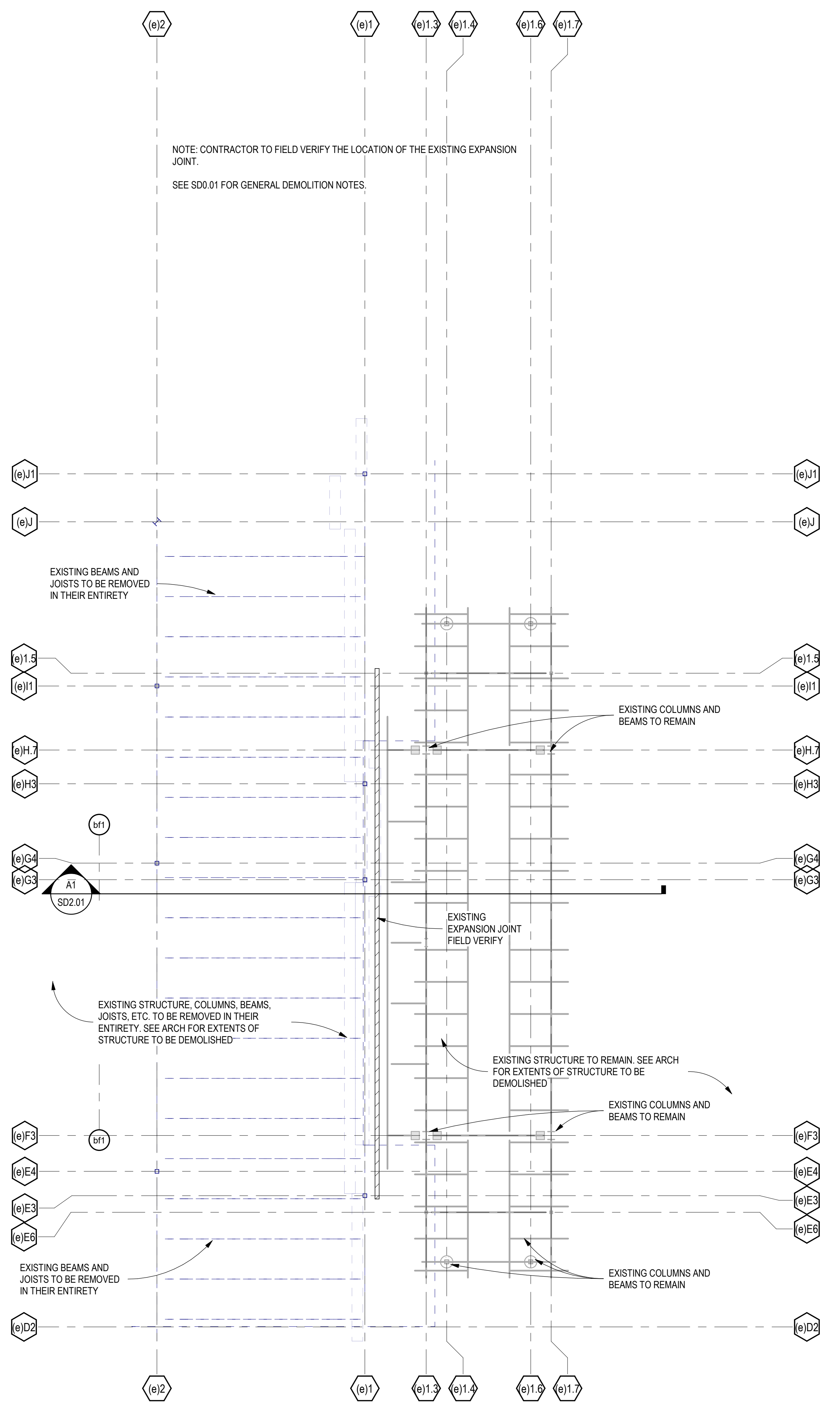
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DEMOLITION GENERAL
STRUCTURAL NOTES



A1 EXISTING FOUNDATION DEMOLITION PLAN
SCALE: 1/8" = 1'-0"



A3 EXISTING ROOF DEMOLITION PLAN
SCALE: 1/8" = 1'-0"

GENERAL SHEET NOTES

- EXISTING CONSTRUCTION IS PER AVAILABLE EXISTING DRAWINGS. ALL EXISTING CONSTRUCTION AND DIMENSIONS SHALL BE VERIFIED PRIOR TO CONSTRUCTION. SHOULD CONDITIONS VARY FROM THOSE SHOWN, CONTRACT ENGINEER BEFORE PROCEEDING.
- NO STRUCTURAL MEMBERS SHALL BE CUT OR REMOVED UNLESS SPECIFICALLY INDICATED ON THESE DRAWINGS. NOTIFY ARCHITECT AND ENGINEER IF CONDITIONS DIFFER FROM THOSE SHOWN HERE.

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**WILMA P. MANKILLER HEALTH CENTER
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STILWELL, OKLAHOMA

KEY PLAN:

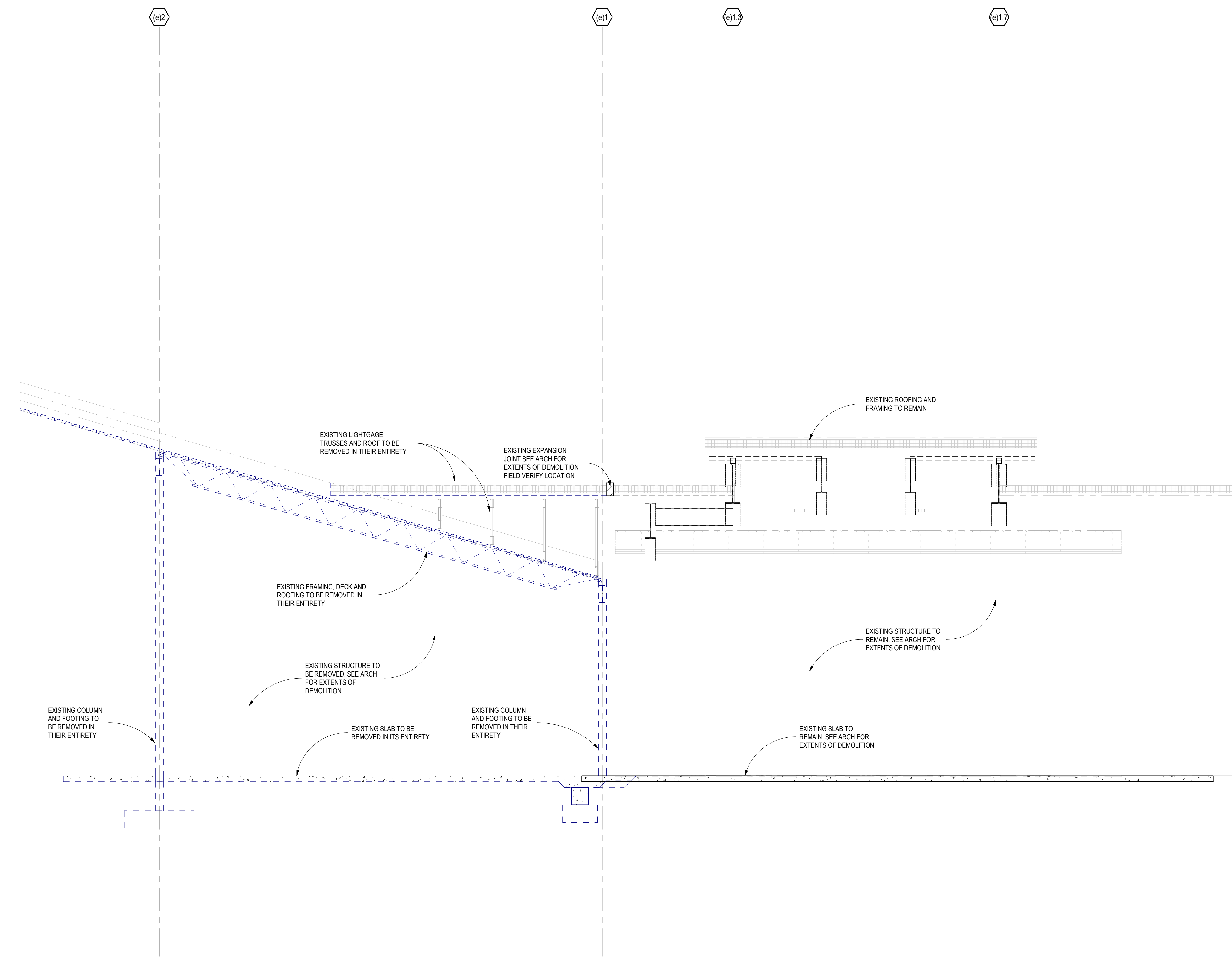
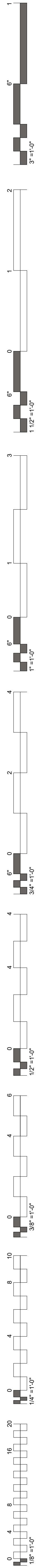
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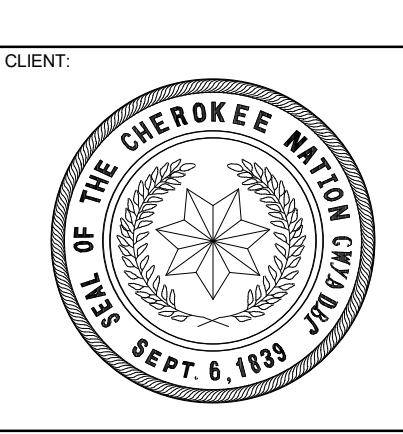
DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER:
SD1.01

DEMOLITION PLANS -
SECTOR 1



A1 EXISTING DEMOLITION SECTION
SCALE: 3/8" = 1'-0"



**WILMA P. MANKILLER HEALTH CENTER
EXPANSION**
STILWELL, OKLAHOMA

KEY PLAN:

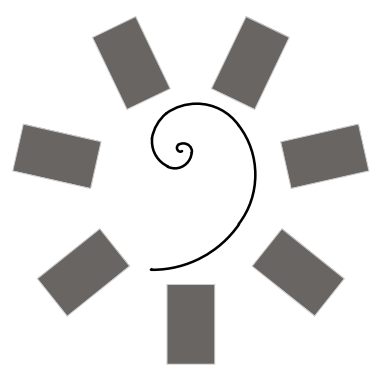
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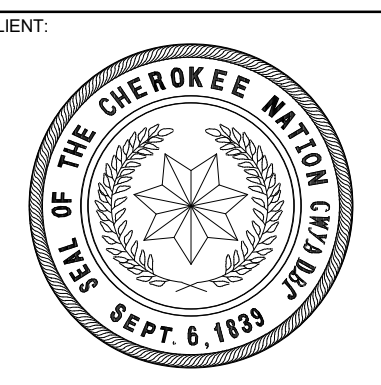
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SHEET NUMBER:
SD2.01

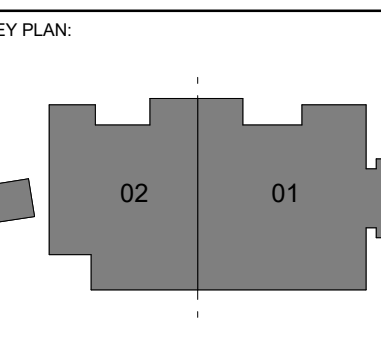
DEMOLITION SECTIONS



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**WILMA P. MANKILLER HEALTH CENTER
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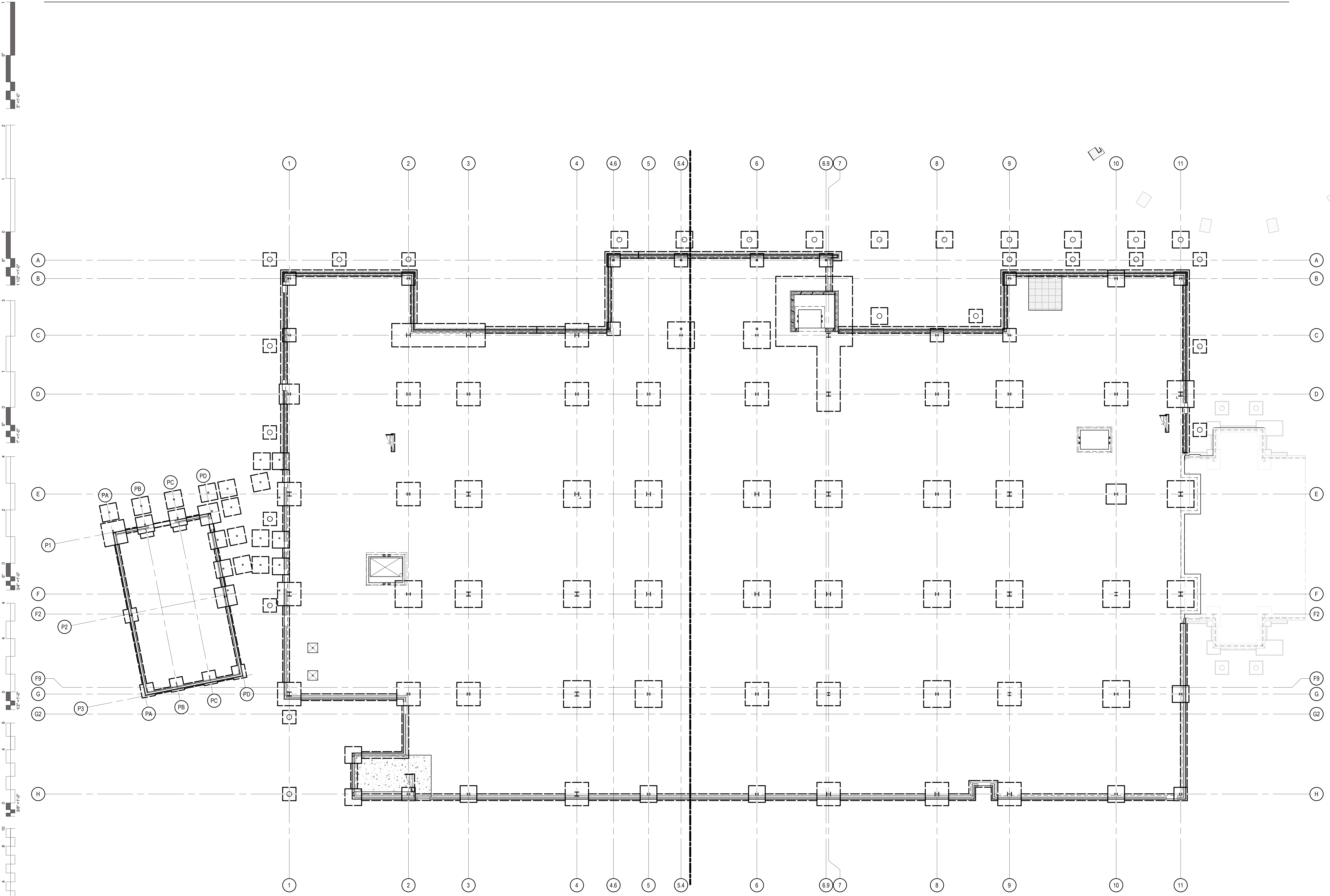
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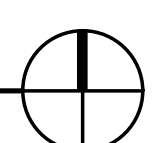
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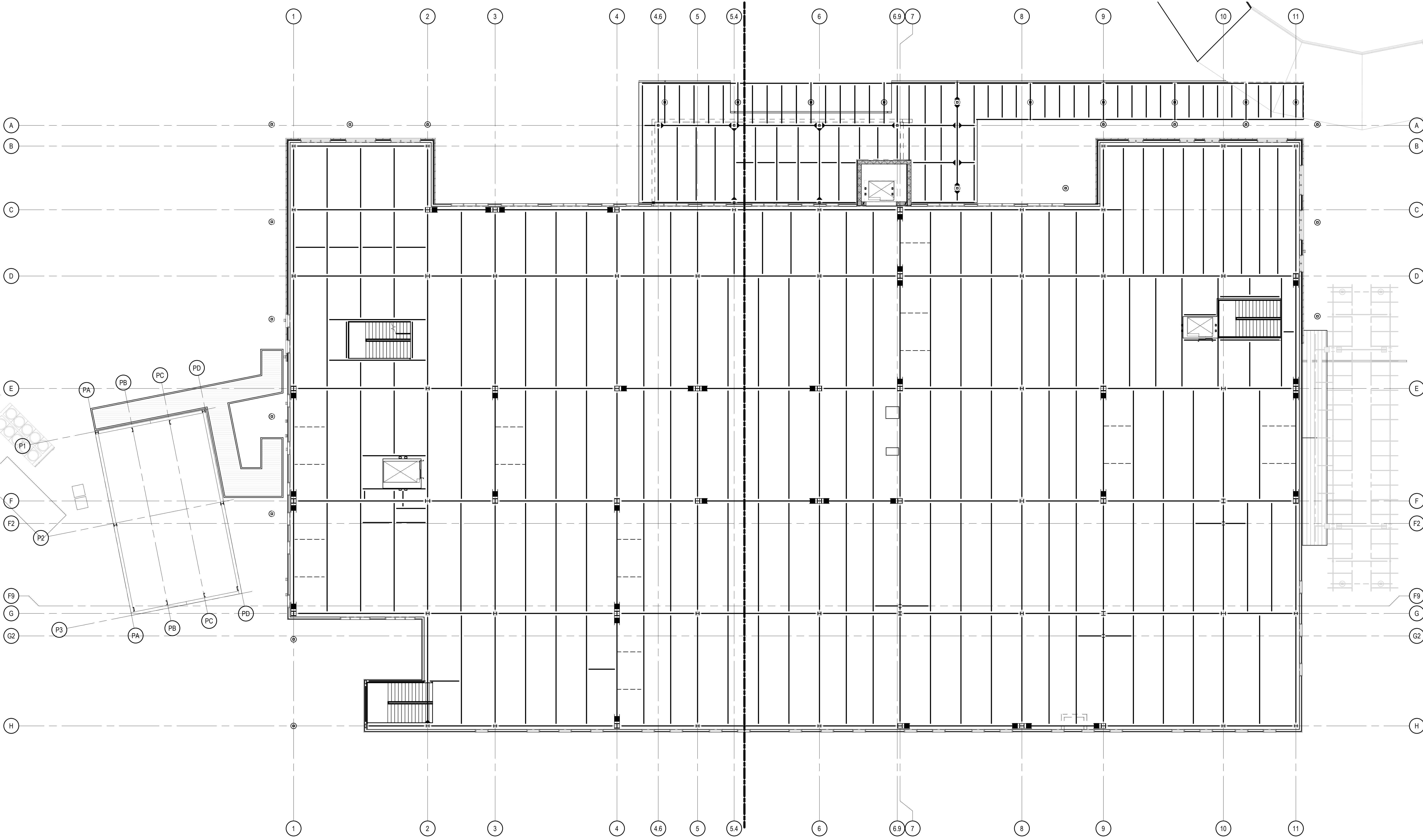
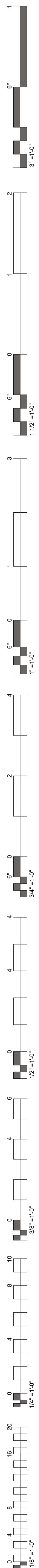
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S1.00

OVERALL PLAN -
FOUNDATION



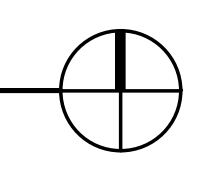
A1 FOUNDATION PLAN - OVERALL
SCALE: 3/32" = 1'-0"

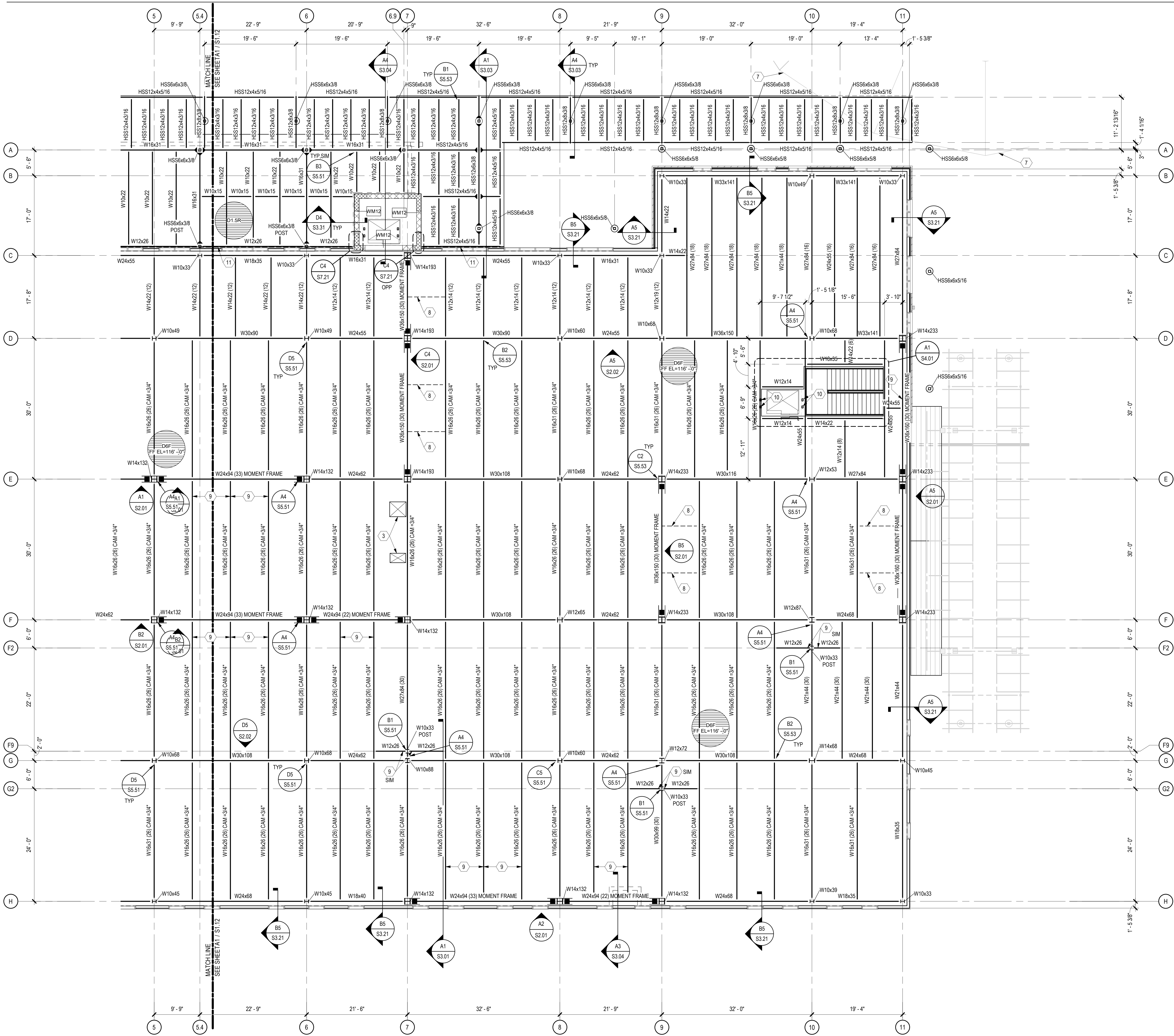


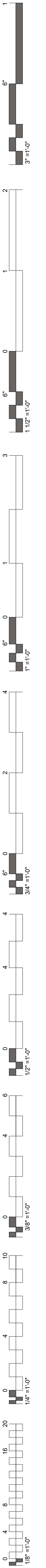


A1 FLOOR FRAMING PLAN - OVERALL
SCALE: 3/32" = 1'-0"

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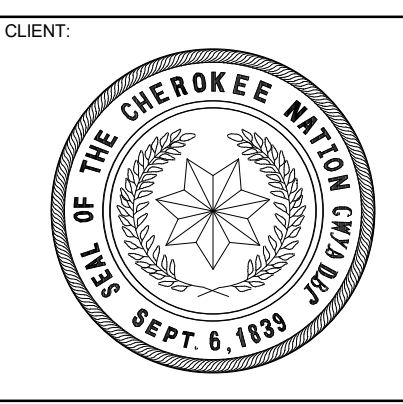
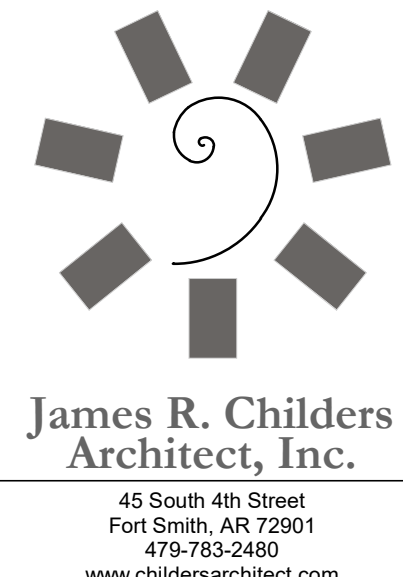






GENERAL SHEET NOTES

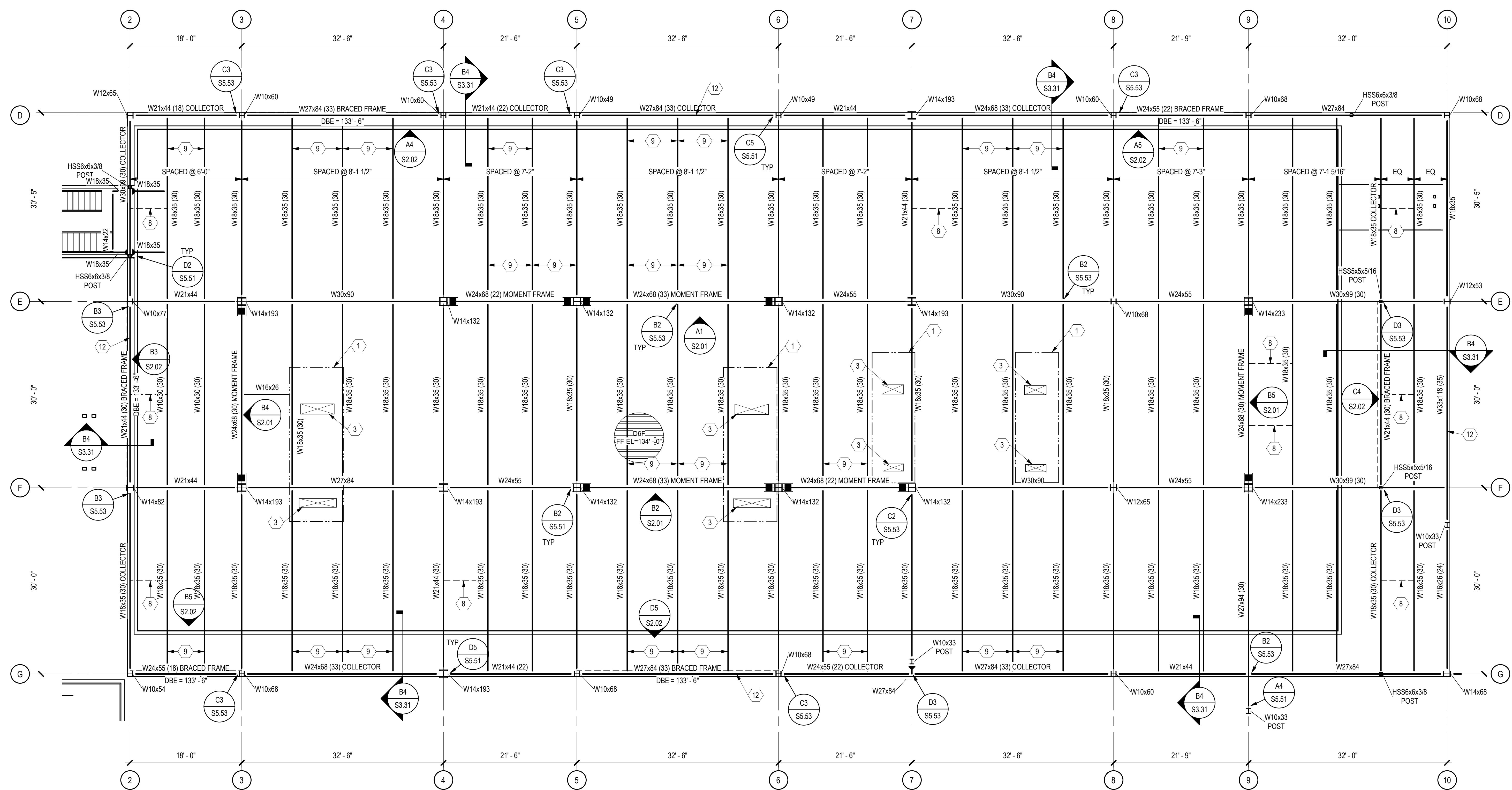
- SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
- DIMENSIONS ARE TO THE FACE OF STUD UNLESS NOTED OTHERWISE.
- SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
- EXISTING CONSTRUCTION IS PER AVAILABLE EXISTING DRAWINGS. ALL EXISTING CONSTRUCTION AND DIMENSIONS SHALL BE VERIFIED PRIOR TO CONSTRUCTION. SHOULD CONDITIONS VARY FROM THOSE SHOWN, CONTACT ENGINEER BEFORE PROCEEDING.
- STRUCTURAL COLD FORMED METAL STUDS SHALL BE 60S162-43 AT 16" ON CENTER UNLESS NOTED OTHERWISE.
- BEAMS AND JOISTS ARE SPACED EQUALLY BETWEEN GRIDS AND COLUMNS UNLESS NOTED OTHERWISE.
- SEE SHEET S7.00 SERIES SHEETS FOR TYPICAL FLOOR FRAMING SECTIONS.
- SEE SHEET S6.01 FOR SCHEDULES.
- ▬ DENOTES MOMENT CONNECTION PER TYPICAL DETAILS.
- ▬ DENOTES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE DRAWINGS.



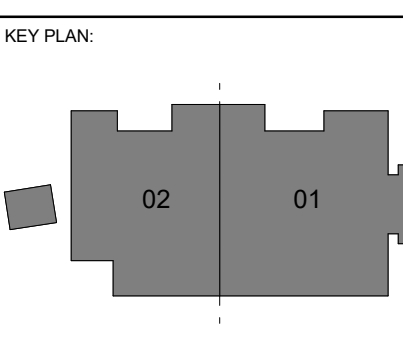
SHEET KEYNOTE

- MECHANICAL UNIT. COORDINATE EXACT SIZE AND LOCATION WITH MECHANICAL DRAWINGS.
- OPERABLE PARTITION BELOW. COORDINATE EXACT LOCATION WITH ARCHITECTURAL DRAWINGS. SEE A5 / S5.52 AND B5 / S5.52 FOR SUPPORT.
- MECHANICAL OPENING. COORDINATE EXACT SIZE AND LOCATION WITH MECHANICAL DRAWINGS. SEE C5 / S7.42
- HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST. COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE A2 / S5.41, B2 / S5.41, C2 / S5.41, AND D3 / S5.41
- HSS6x4x1/4 COLLECTOR BRACING BETWEEN BEAMS. SEE D4 / S5.52. ATTACH BRACING TO DECK VALLEYS PER DECK SCHEDULE. PROVIDE 20 GAGE PLATE AS REQUIRED TO MAKE ATTACHMENT.
- 4" HOUSEKEEPING PAD REINFORCED WITH #4 @ 18" ON CENTER EACH WAY AND #4 VERT DOUBLES DRILLED AND EPOXYED 2" INTO CONCRETE SLAB BELOW @ 48" ON CENTER EACH WAY (12" FROM EDGES AND CORNERS). PAD SHALL EXTEND 6" BEYOND FACE OF MECHANICAL UNIT ALL AROUND. COORDINATE EXACT SIZE AND LOCATION OF PAD WITH MECHANICAL DRAWINGS.
- EXISTING CANOPY. SEE ARCHITECTURAL DEMOLITION PLANS FOR EXTENT OF DEMOLITION.
- BOTTOM FLANGE BRACING AT EQUAL SPACING. UNLESS NOTED OTHERWISE, BRACE TO BE ATTACHED TO BOTTOM FLANGE OF BEAM NOTED AS MOMENT FRAME OR BRACED FRAME TO TOP FLANGE OF ADJACENT BEAM. SEE B3 / S5.52
- BOTTOM FLANGE BRACING. SEE A3 / S5.52
- HSS6x4x1/2 ELEVATOR RAIL SUPPORT BEAM. COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE A1 / S5.41 AND B1 / S5.41 FOR TYPICAL DETAILS.
- 2" BUILDING EXPANSION JOINT. SEE ARCHITECTURAL DRAWINGS.
- SLAB EDGE TO BE LOCATED AT OUTSIDE FLANGE OF BEAM. SEE S7.41 FOR SLAB EDGE DETAILS.

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A1 LOW ROOF FRAMING PLAN
SCALE: 1/8" = 1'-0"



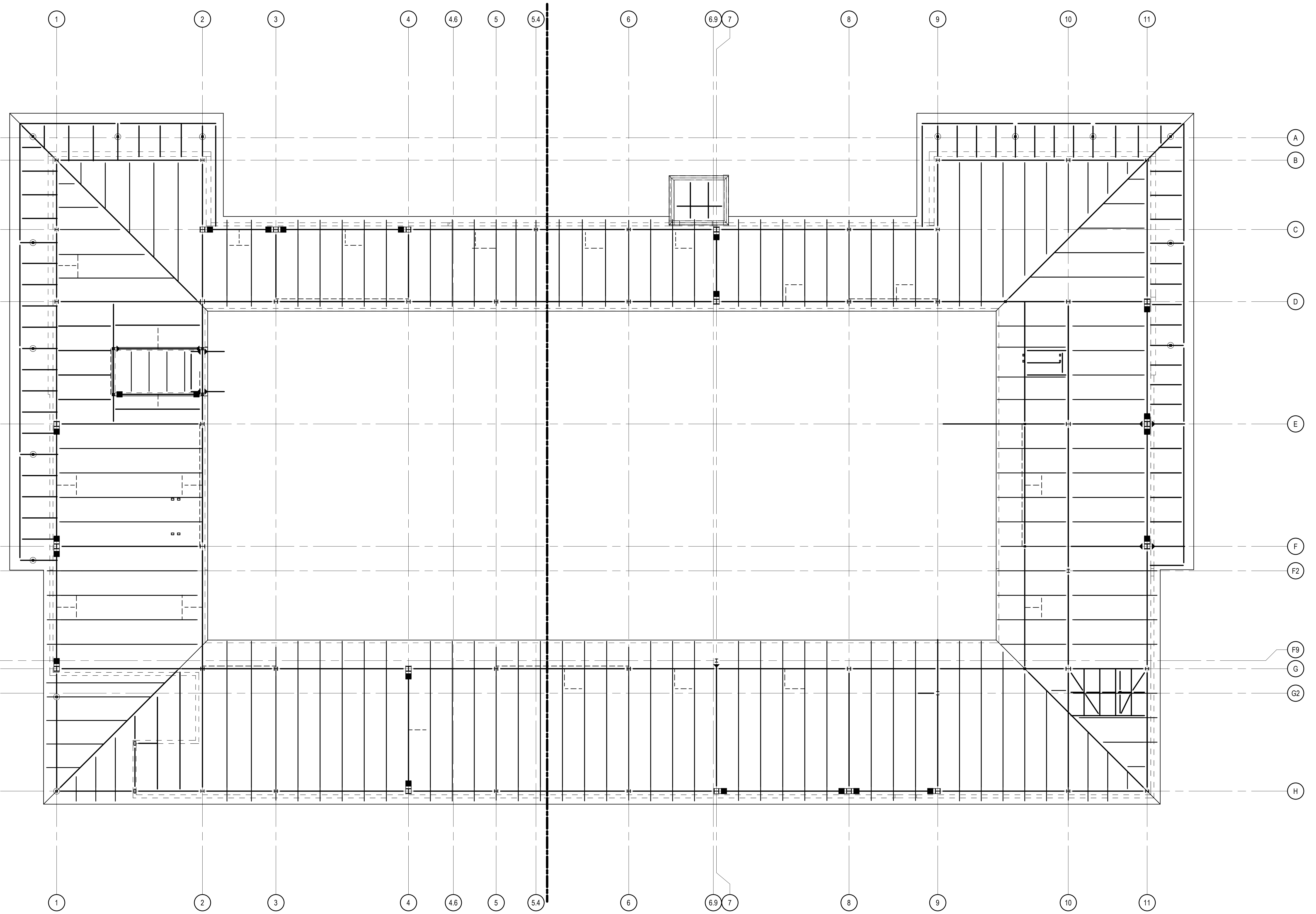
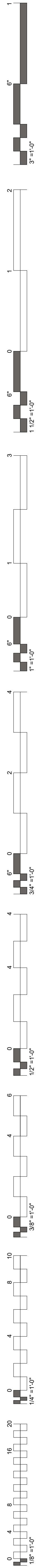
PROJECT PHASE:
BID PACKAGE 01

#	DATE	REVISIONS	DESCRIPTION

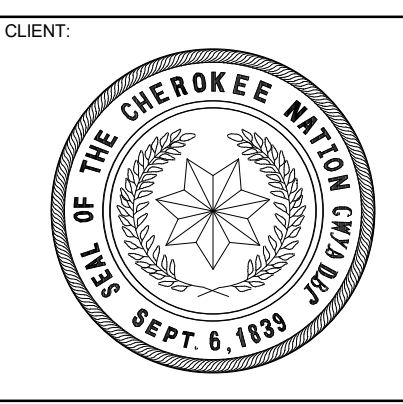
DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER: S1.13

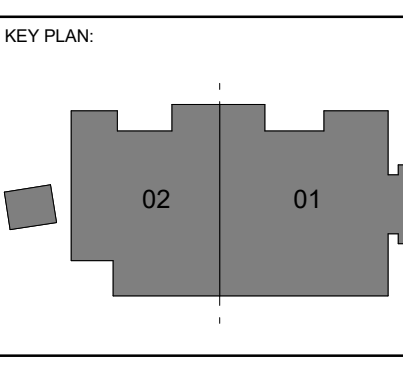
LOW ROOF FRAMING PLAN



A1 ROOF FRAMING PLAN - OVERALL
SCALE: 3/32" = 1'-0"



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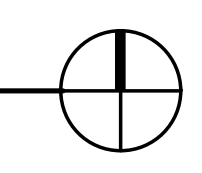
PROJECT PHASE:
BID PACKAGE 01

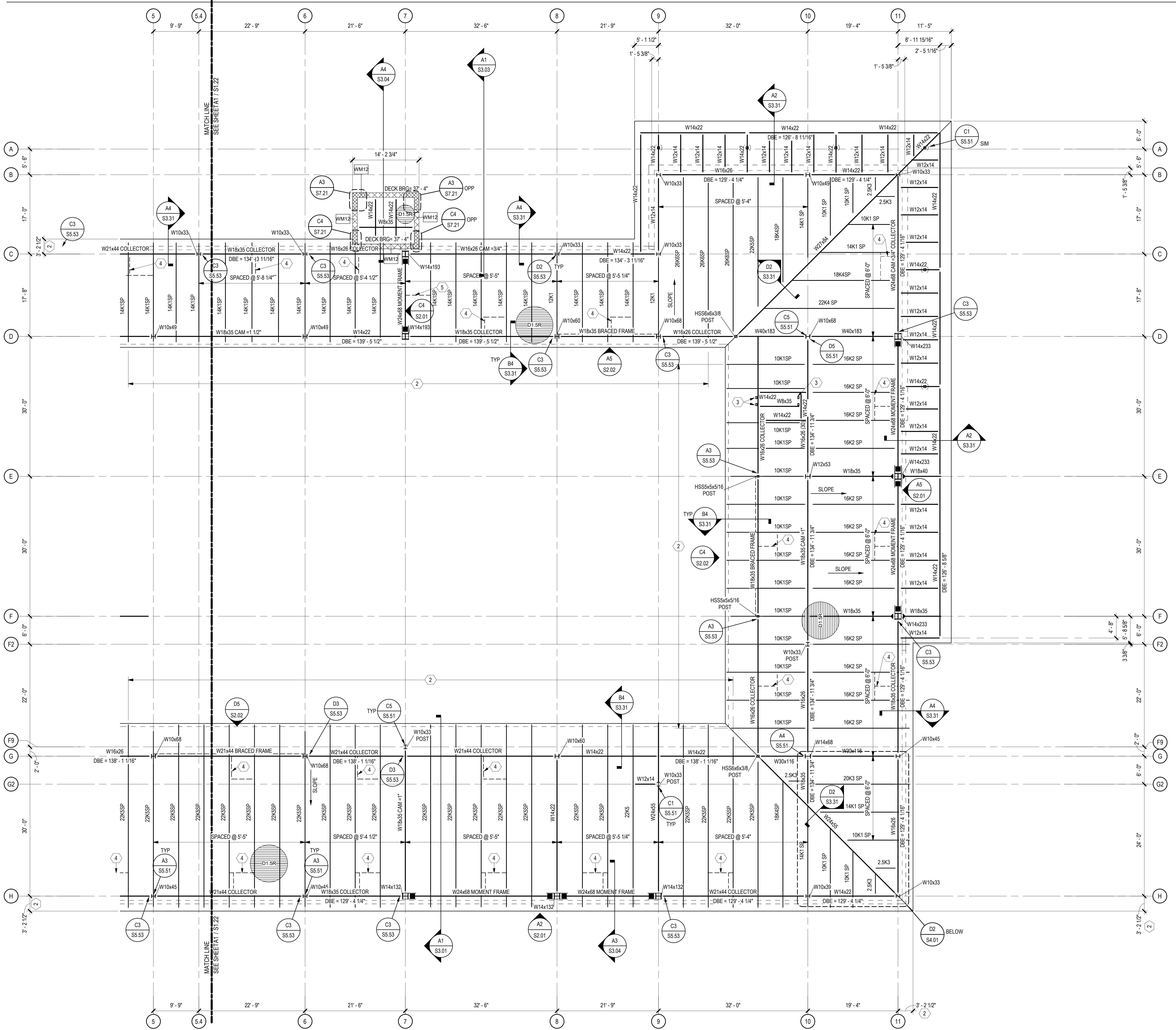
#	DATE	REVISIONS	DESCRIPTION

DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER:
S1.20

OVERALL PLAN - ROOF FRAMING





GENERAL SHEET NOTES

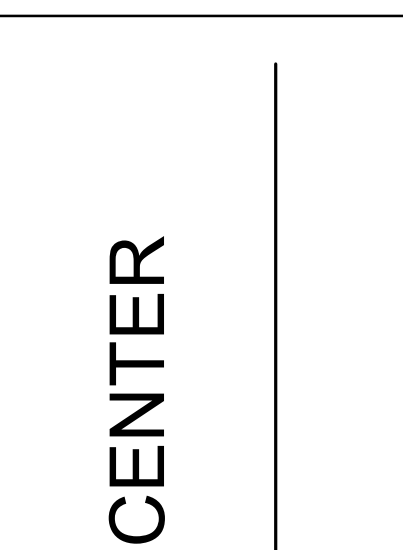
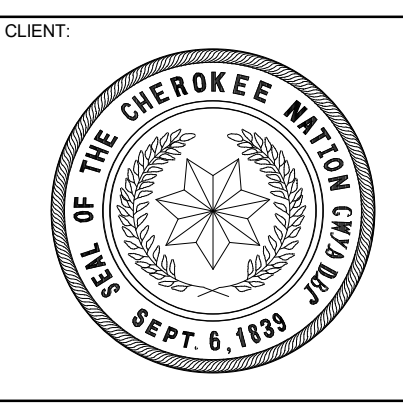
- SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACING OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
- DIMENSIONS ARE TO THE FACE OF STUD UNLESS NOTED OTHERWISE.
- SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
- EXISTING CONSTRUCTION IS PER AVAILABLE EXISTING DRAWINGS. ALL EXISTING CONSTRUCTION AND DIMENSIONS SHALL BE VERIFIED PRIOR TO CONSTRUCTION. SHOULD CONDITIONS VARY FROM THOSE SHOWN, CONTACT ENGINEER BEFORE PROCEEDING.
- BEAMS AND JOISTS ARE SPACED EQUALLY BETWEEN GRIDS AND COLUMNS UNLESS NOTED OTHERWISE.
- PROVIDE JOIST BRIDGING PER THE 42ND EDITION OF THE SJI SPECIFICATIONS AND OSHA REQUIREMENTS.
- STEEL JOIST MANUFACTURER SHALL DESIGN ROOF JOISTS AND ROOF JOIST GIRDERS SUPPORTING MECHANICAL UNITS FOR 1.2x MECHANICAL UNIT WEIGHTS SHOWN. USE 28 PSF DEAD LOAD AND 20 PSF LIVE LOAD UNLESS NOTED OTHERWISE. CONTRACTOR SHALL VERIFY ACTUAL MECHANICAL LOADS. NOTIFY STEEL JOIST MANUFACTURER OF ANY DISCREPANCIES.
- STRUCTURAL COLD FORMED METAL STUDS SHALL BE 6" IN WIDTH, UNLESS NOTED OTHERWISE.
- SEE SHEET S7.00 SERIES SHEETS FOR TYPICAL ROOF FRAMING SECTIONS.
- SEE SHEET S6.01 FOR SCHEDULES.
- NOTES MOMENT CONNECTION PER TYPICAL DETAILS.
- IDENTIFIES SIDEPLATE MOMENT CONNECTION. SEE SIDEPLATE DRAWINGS.
- PROVIDE HSSxJOIST SEAT DEPTHx1/4 BETWEEN JOISTS AT ALL BEAMS LABELED AS: MOMENT FRAME, BRACED FRAME, AND COLLECTOR. SEE C2 / S7.41

SHEET KEYNOTE

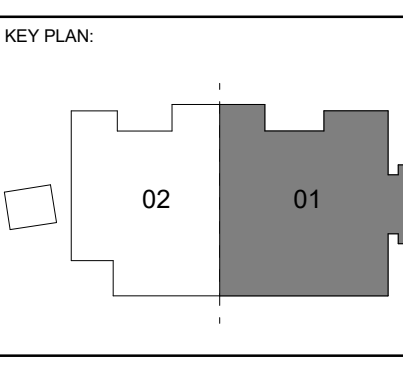
- MECHANICAL UNIT. COORDINATE EXACT SIZE AND LOCATION WITH MECHANICAL DRAWINGS.
- JOIST EXTENDED END. DESIGN EXTENDED END FOR 20 PSF DEAD LOAD, 20 PSF LIVE LOAD, AND ANY POSITIVE OR NEGATIVE WIND PRESSURES PER ROOF WIND LOADING DIAGRAM ON S0.03. DEPTH OF EXTENDED END PER JOIST MANUFACTURER.
- HSS5x5x1/8 ELEVATOR RAIL SUPPORT POST. COORDINATE EXACT LOCATION WITH ELEVATOR MANUFACTURER. SEE A2 / S5.41, B2 / S5.41, C2 / S5.41, AND D3 / S5.41
- BOTTOM FLANGE BRACE AT EQUAL SPACING, UNLESS NOTED OTHERWISE. BRACE TO BE ATTACHED TO BOTTOM FLANGE OF BEAM NOTED AS MOMENT FRAME OR BRACED FRAME AND TO TOP FLANGE OF ADJACENT BEAM OR JOIST. SEE B3 / S5.52. JOISTS TO BE DESIGNED FOR 1,500# VERTICAL (REVERSIBLE) WIND AND SEISMIC LOAD FROM BRACE.
- BOTTOM FLANGE BRACING AT EQUAL SPACING, UNLESS NOTED OTHERWISE. SEE D1 / S5.51. JOISTS TO BE DESIGNED FOR 1,500# VERTICAL (REVERSIBLE) WIND AND SEISMIC LOAD FROM BRACE.
- BOTTOM FLANGE BRACING ANGLE. SEE A3 / S5.52



**James R. Childers
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PROJECT PHASE:
BID PACKAGE 01

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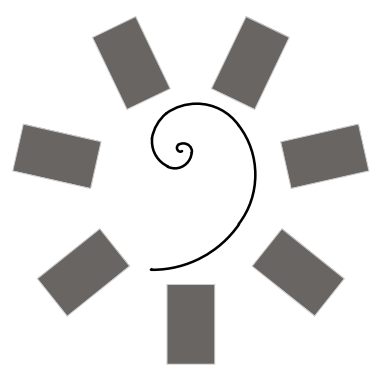
DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER:
S1.21

ROOF FRAMING PLAN - SECTOR 1

A1 ROOF FRAMING PLAN - SECTOR 1
SCALE: 1/8" = 1'-0"

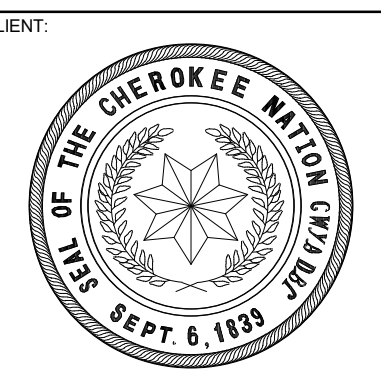




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CONSULTANT LOGO



WILMA P. MANKILLER HEALTH CENTER
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KEY PLAN

PROJECT PHASE:
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DATE: 11-01-19 JOB NUMBER: 18-01.01

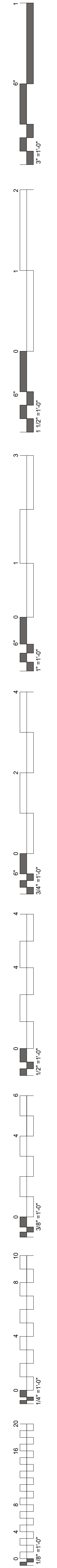
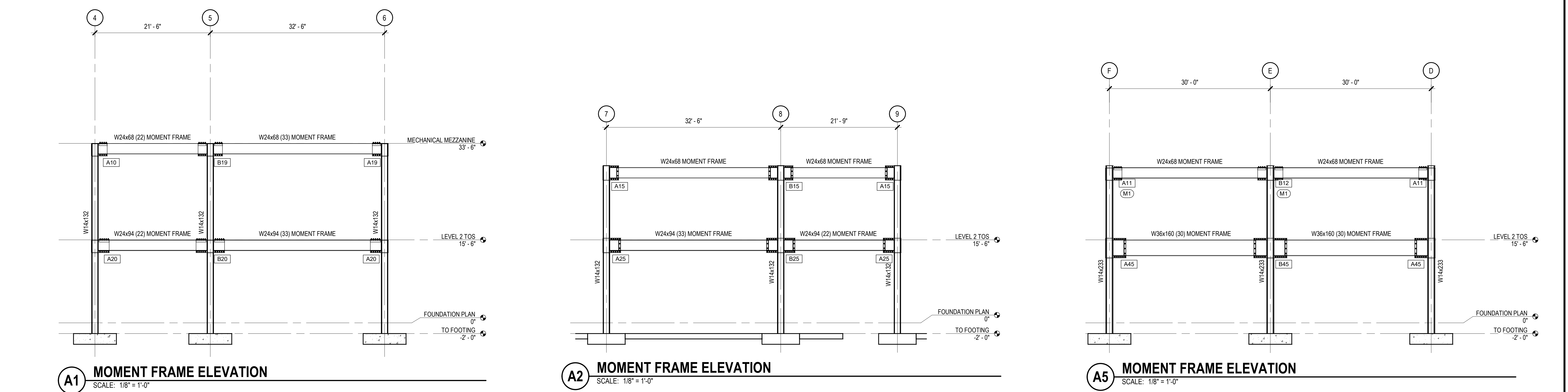
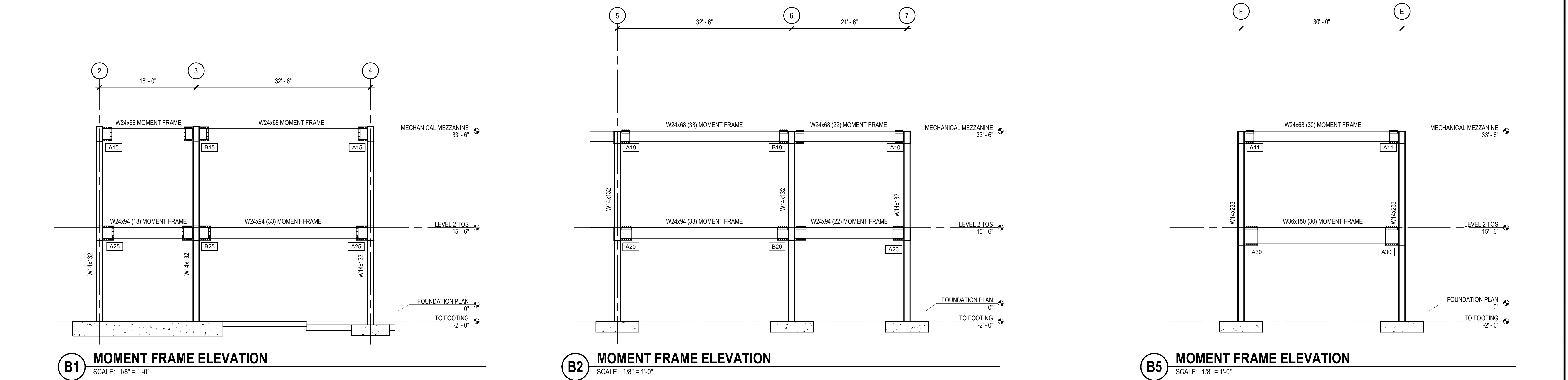
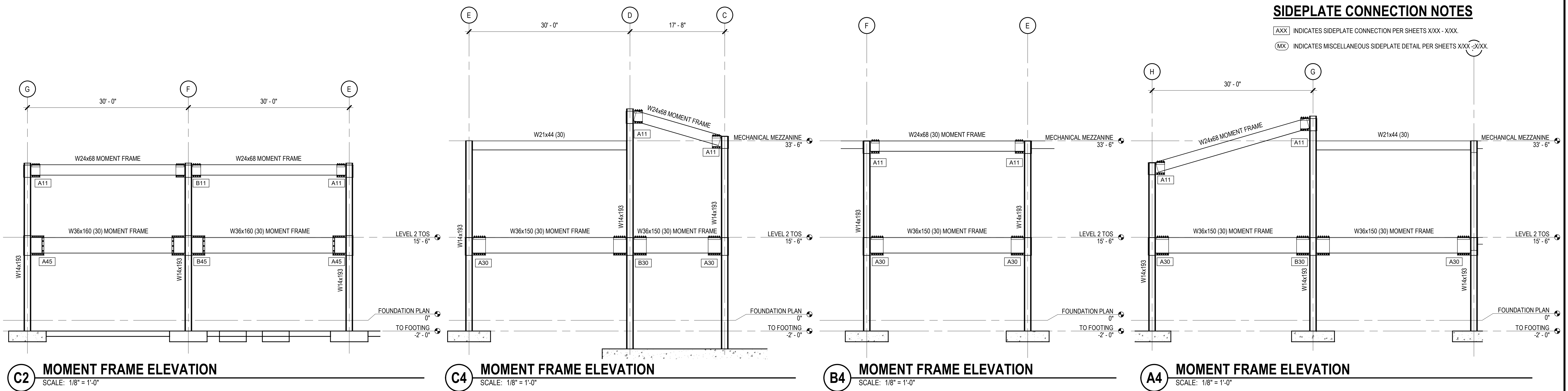
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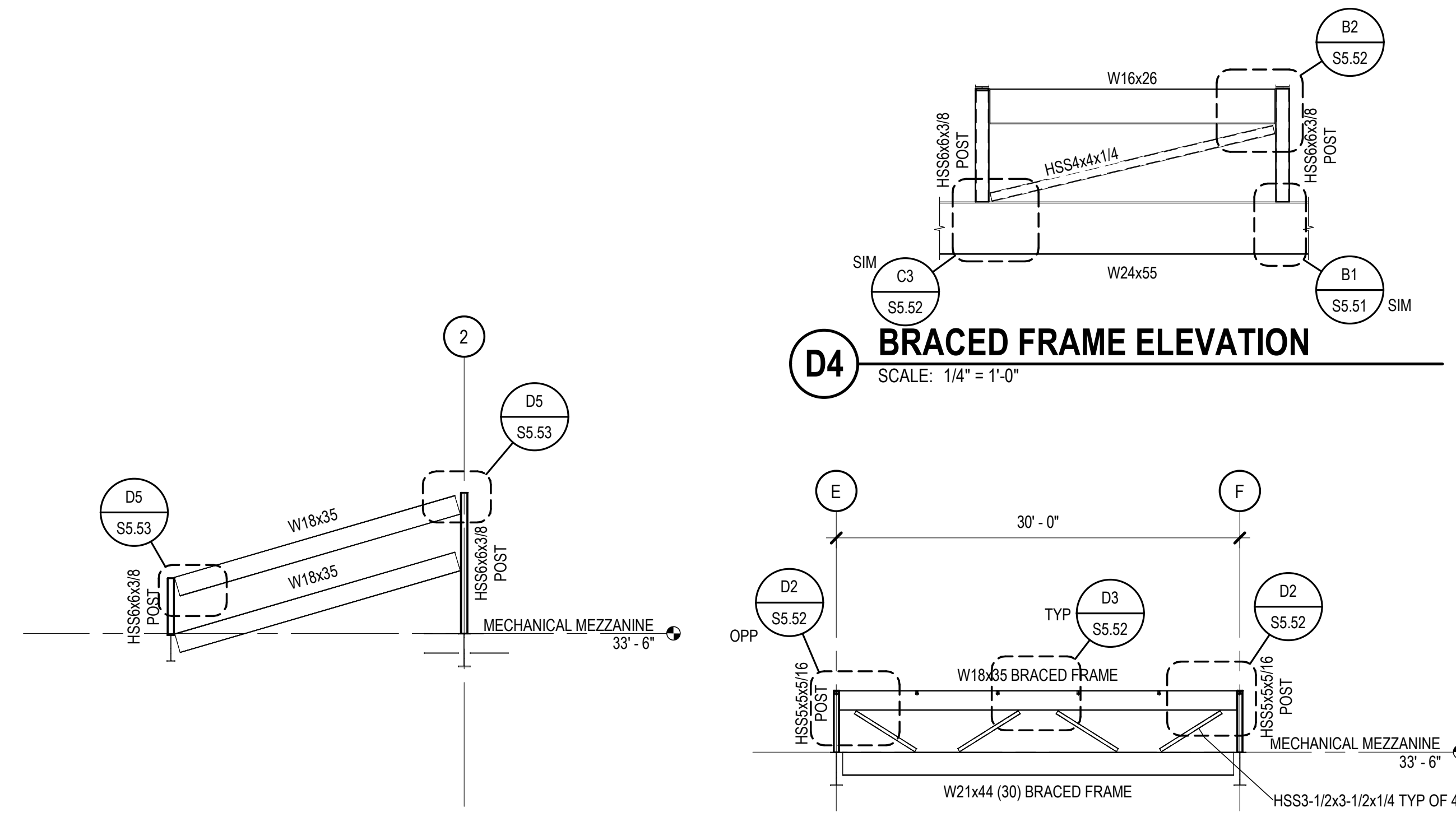
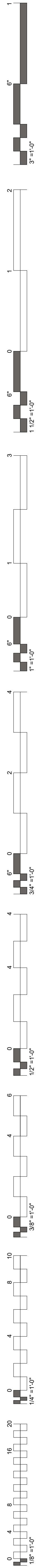
MOMENT FRAME ELEVATIONS

SIDEPLATE CONNECTION NOTES

(AXX) INDICATES SIDEPLATE CONNECTION PER SHEETS XXXX - XXXX.

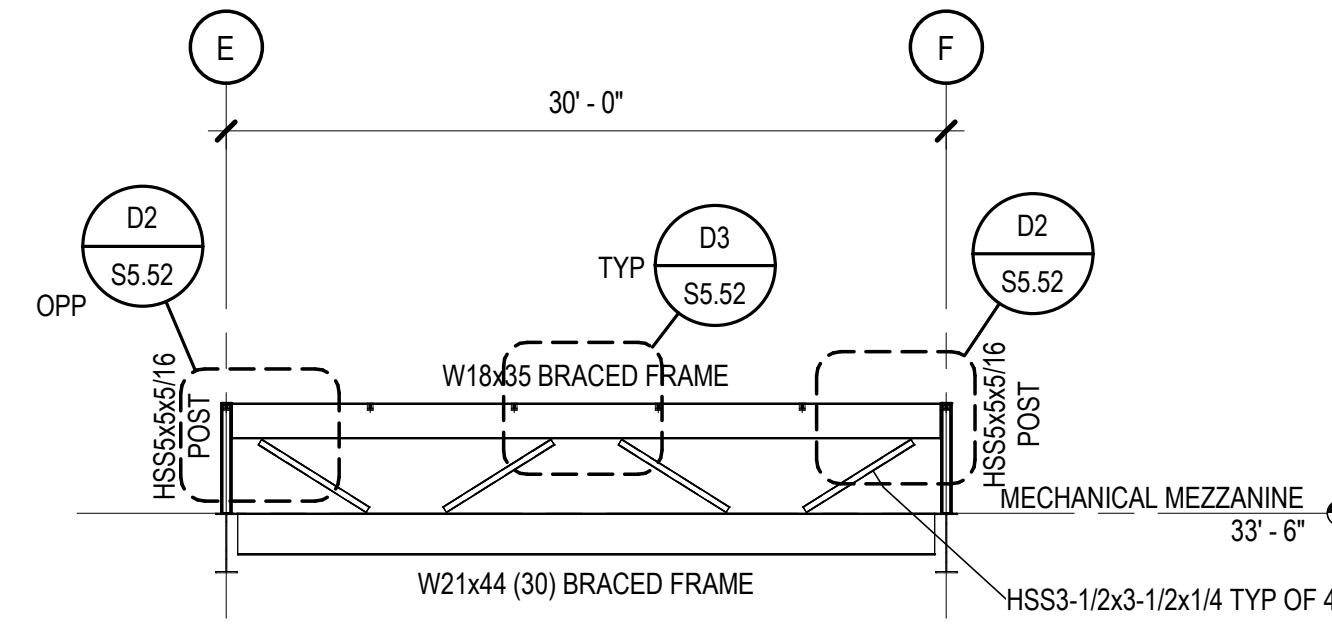
(MX) INDICATES MISCELLANEOUS SIDEPLATE DETAIL PER SHEETS XXXX - XXXX.



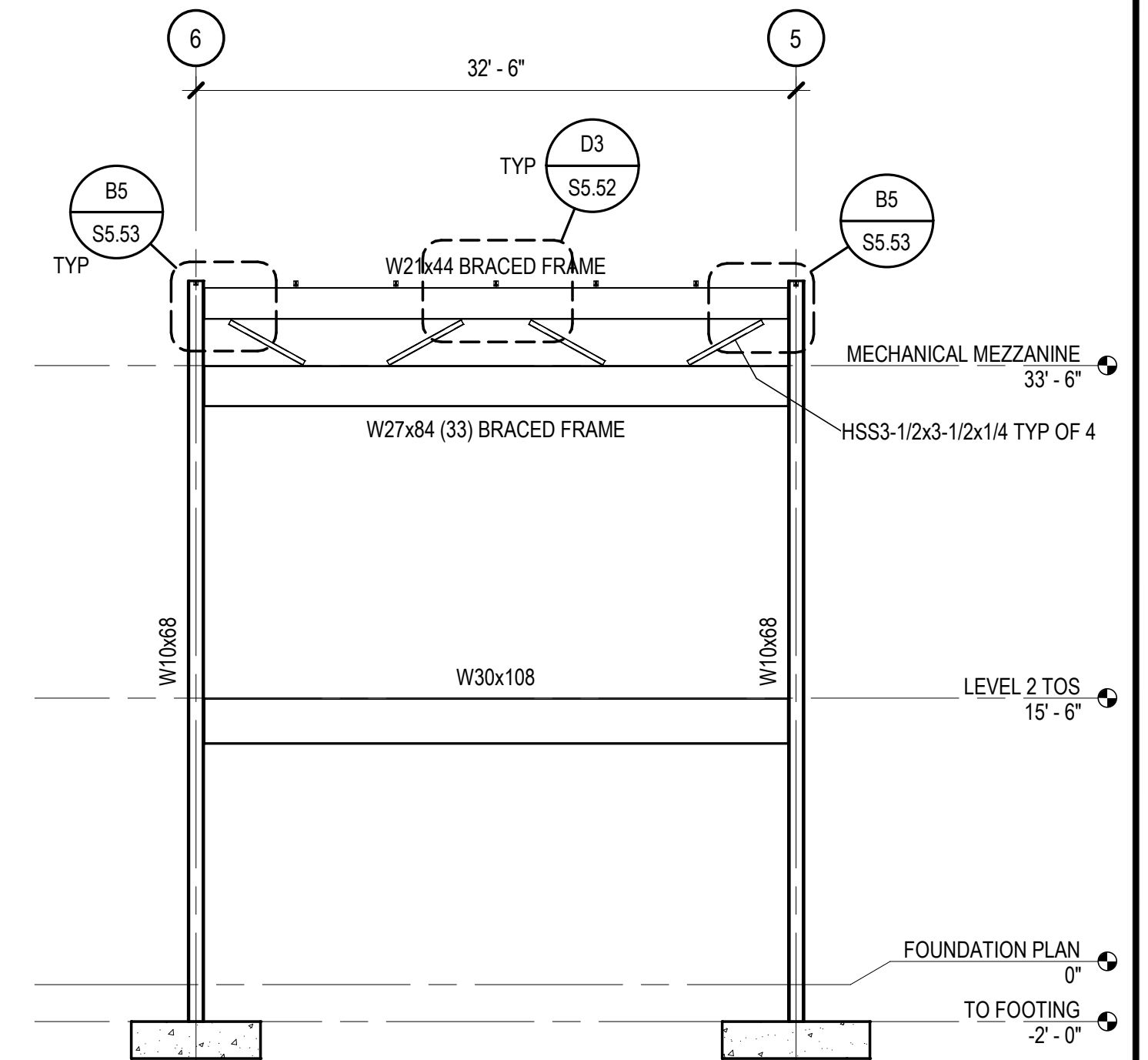


C3 BRACED FRAME ELEVATION
SCALE: 1/8" = 1'-0"

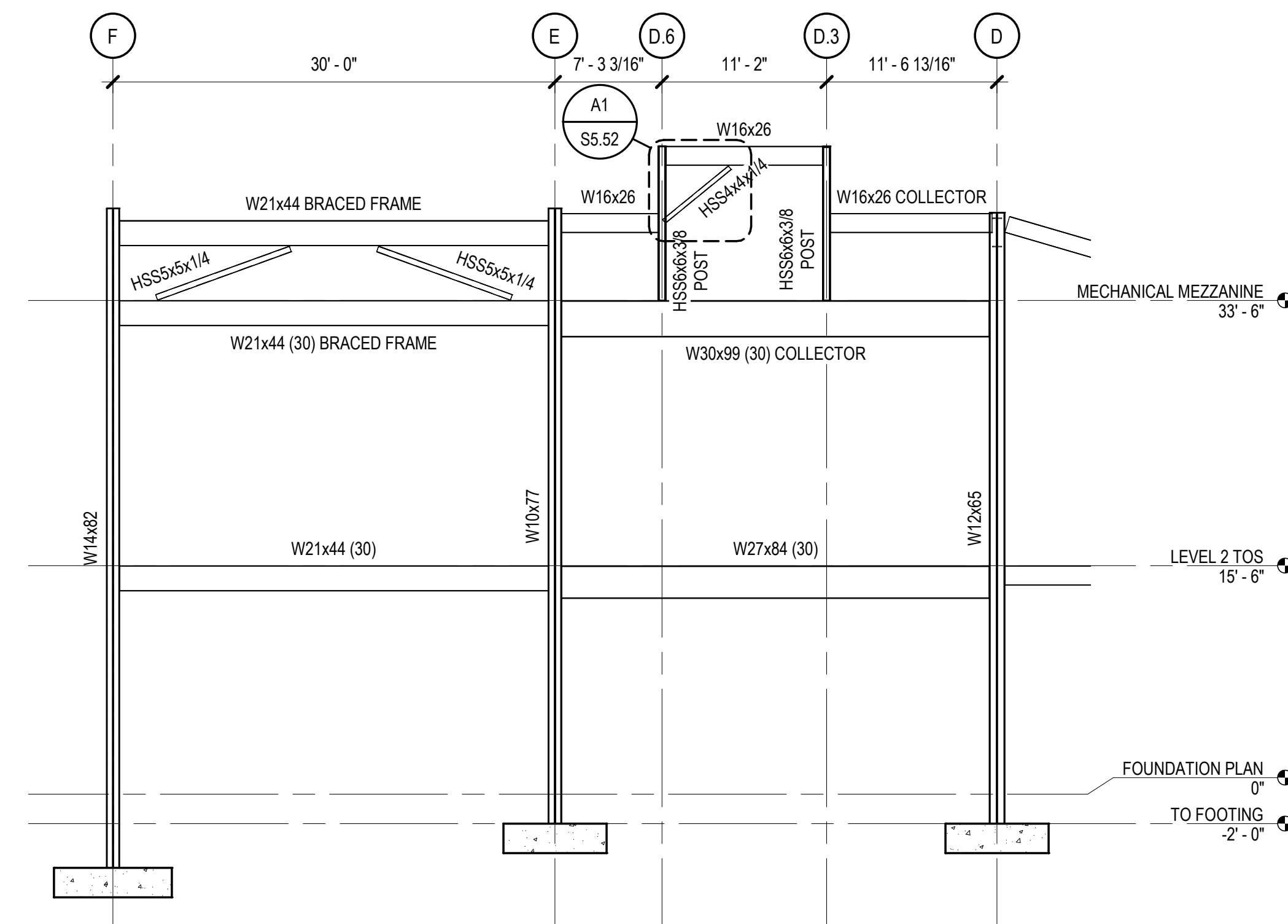
D4 BRACED FRAME ELEVATION
SCALE: 1/4" = 1'-0"



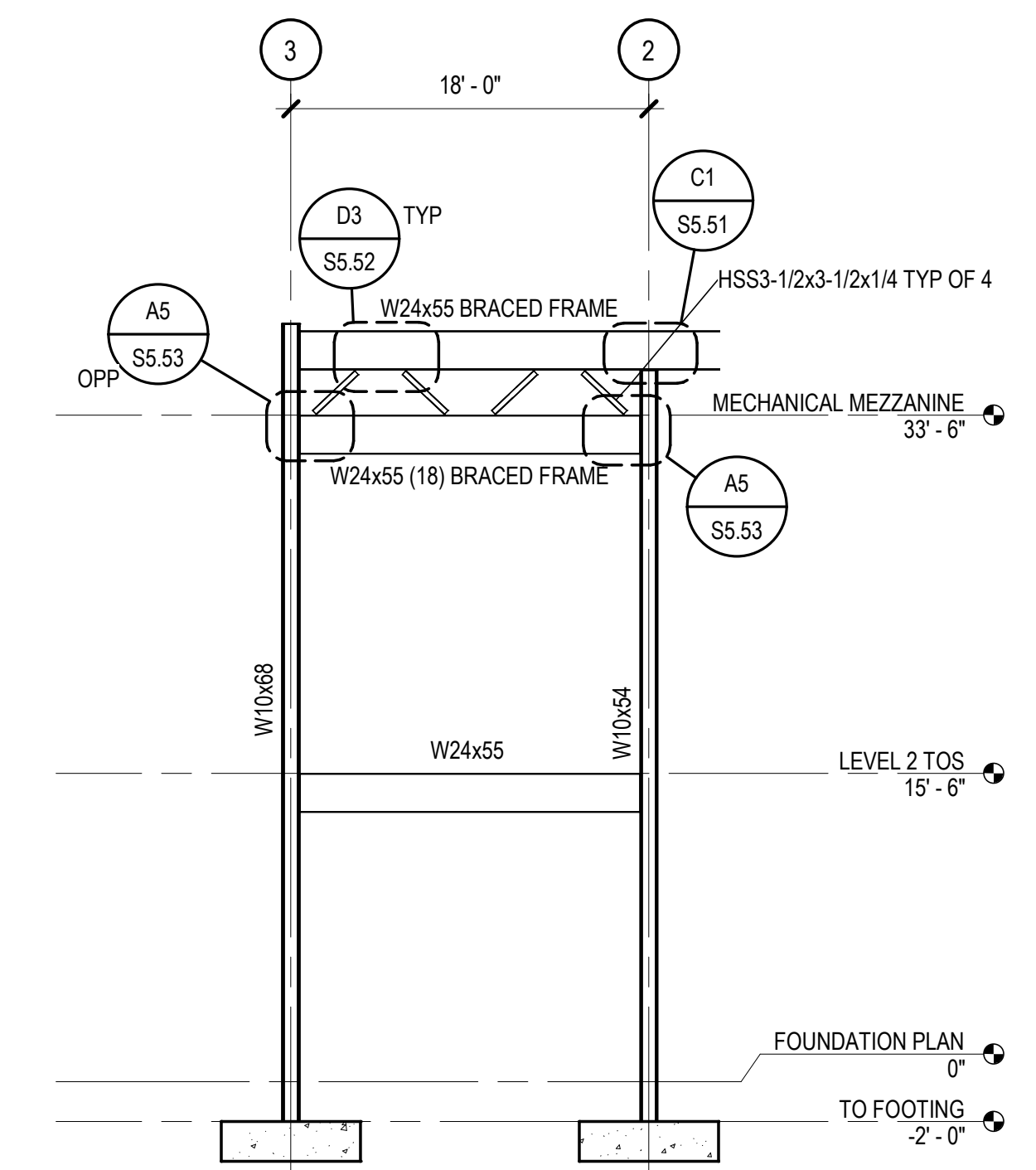
C4 BRACED FRAME ELEVATION
SCALE: 1/8" = 1'-0"



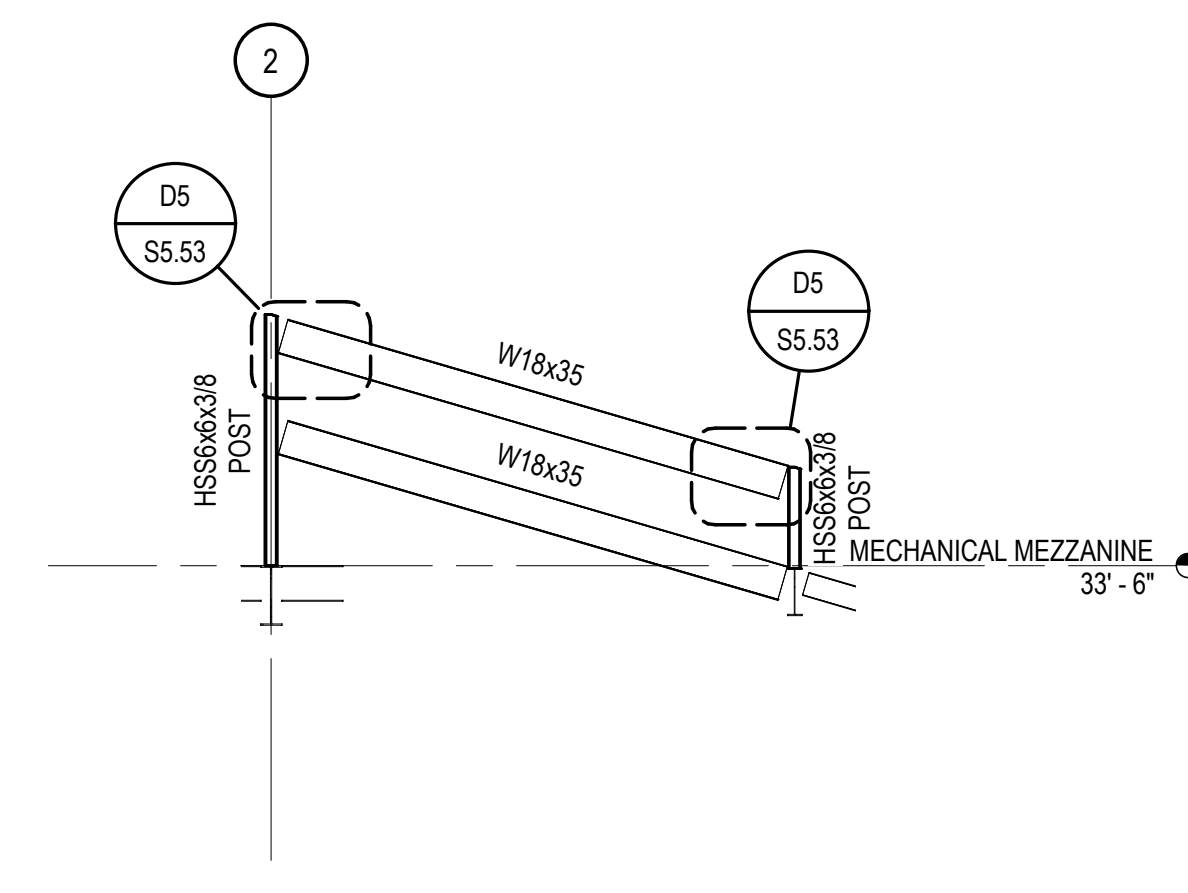
D5 BRACED FRAME ELEVATION
SCALE: 1/8" = 1'-0"



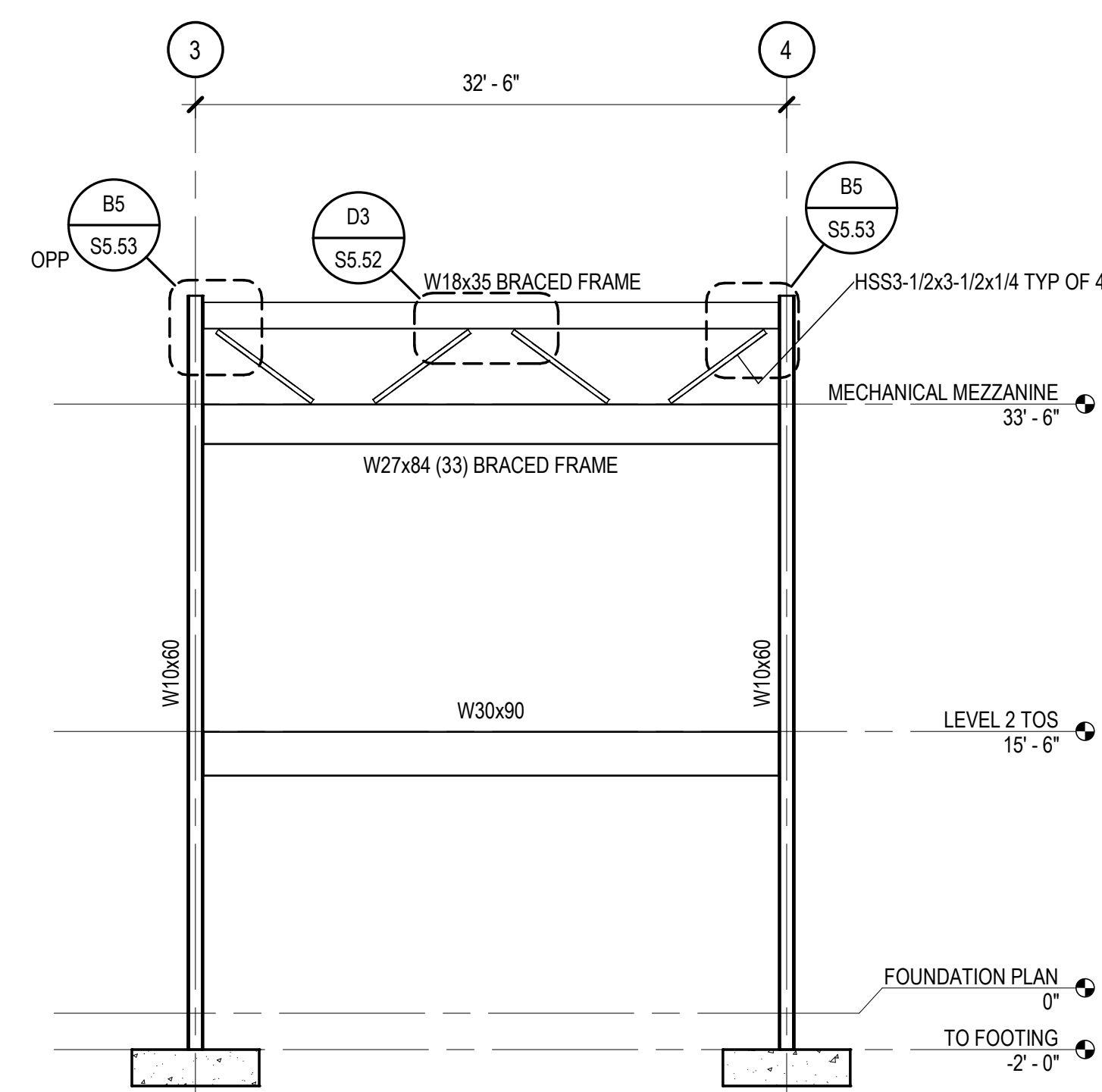
B3 BRACED FRAME ELEVATION
SCALE: 1/8" = 1'-0"



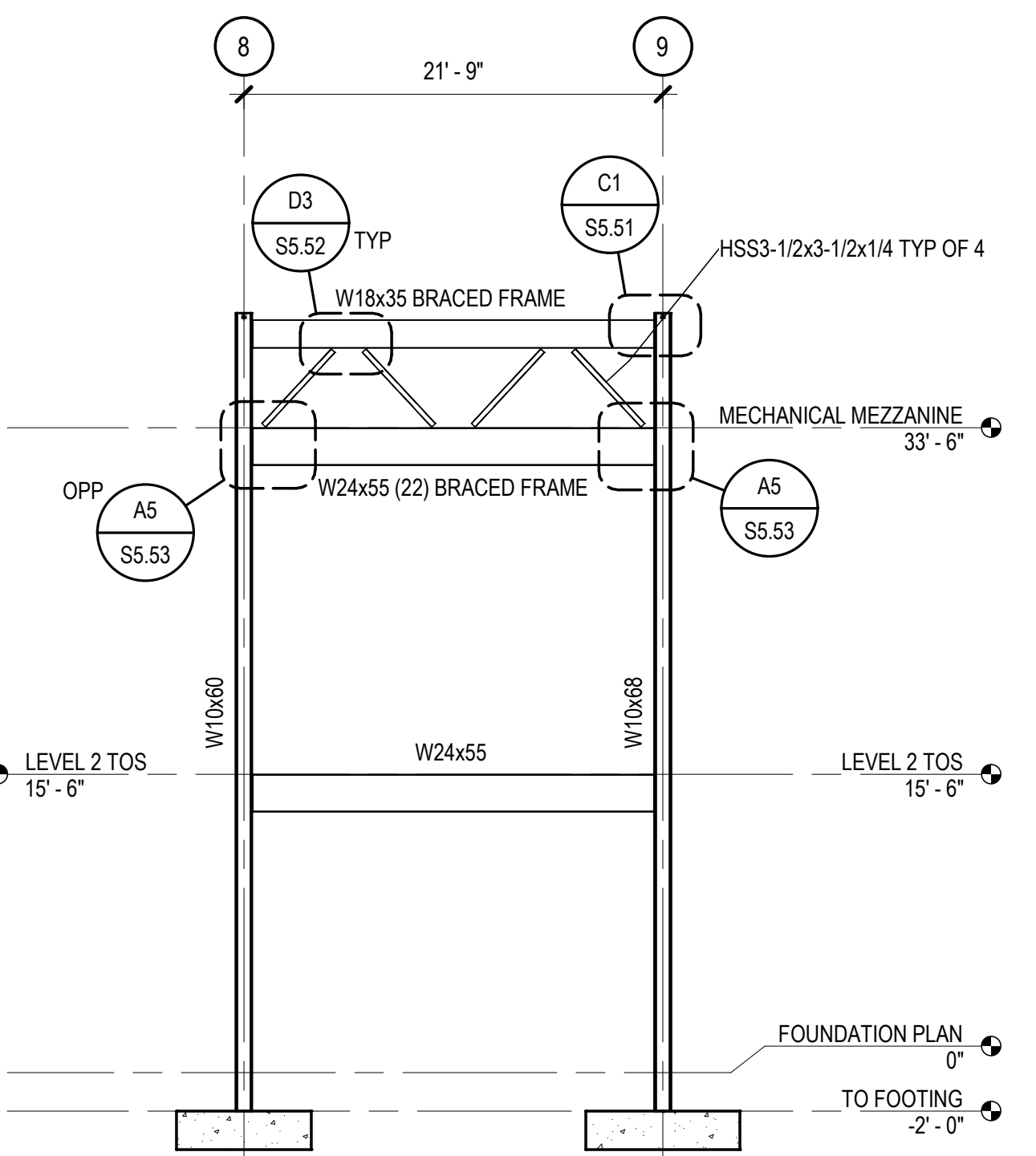
B5 BRACED FRAME ELEVATION
SCALE: 1/8" = 1'-0"



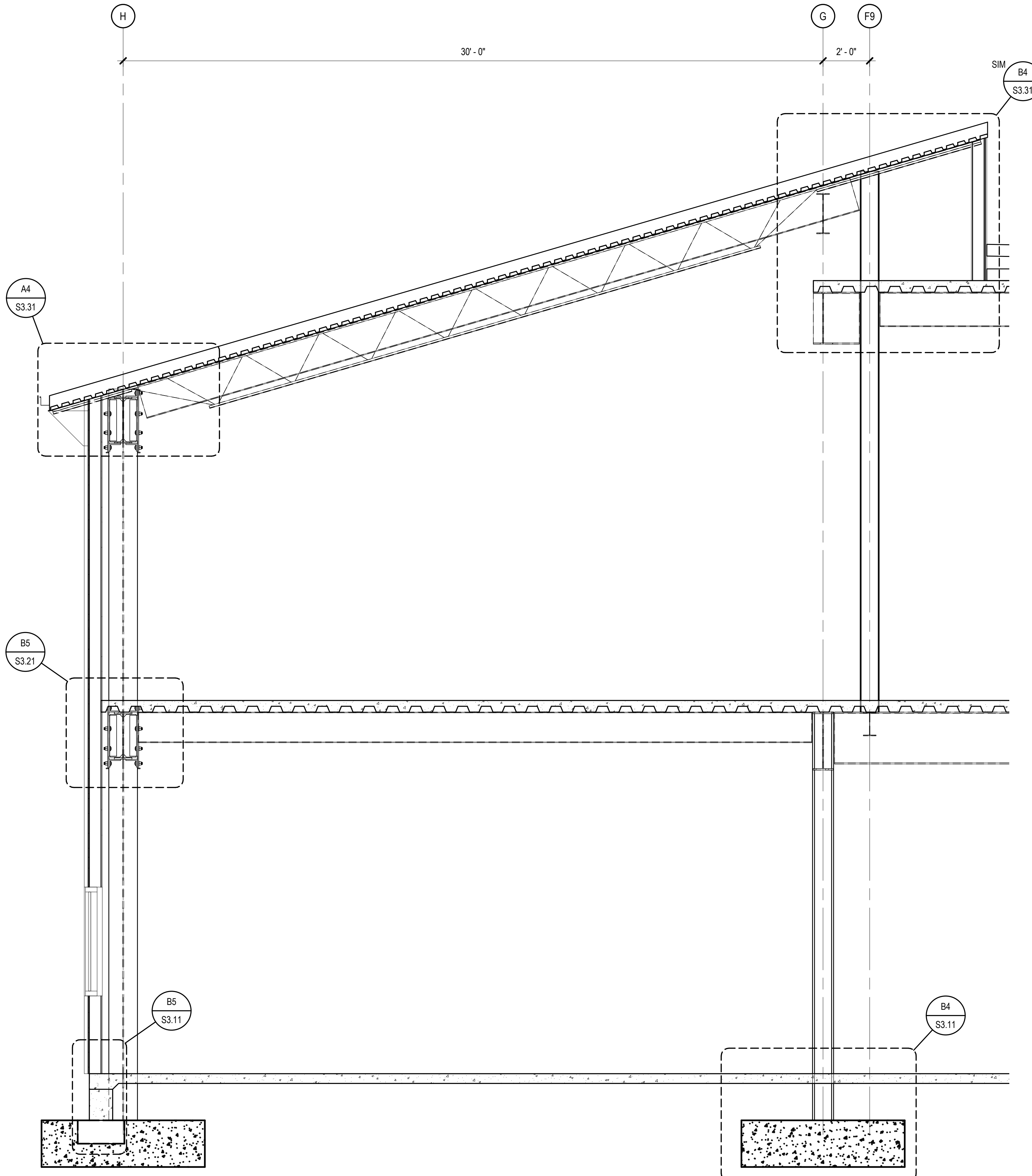
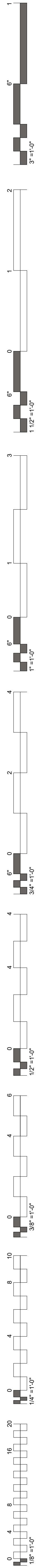
A3 MOMENT FRAME ELEVATION
SCALE: 1/8" = 1'-0"



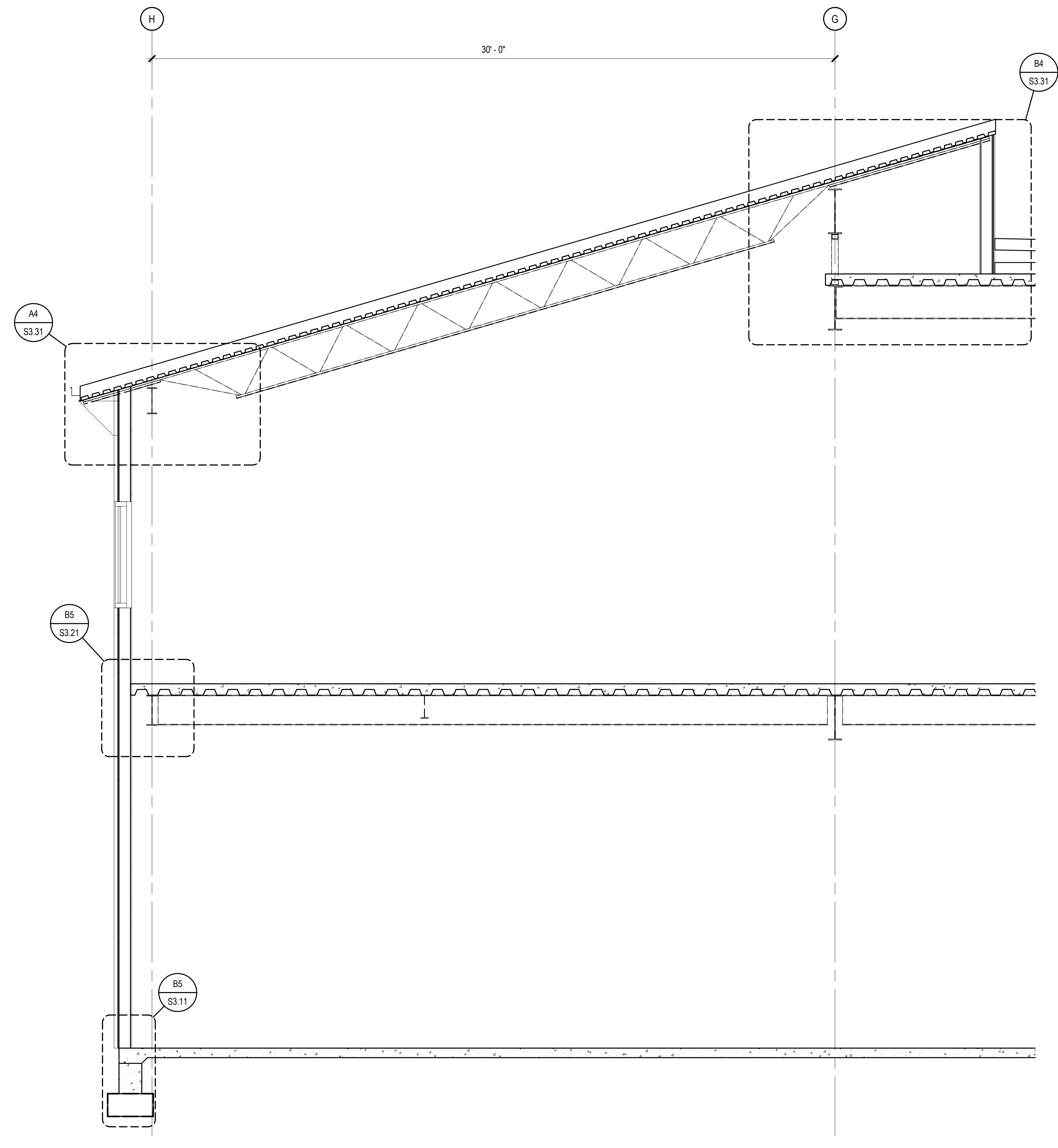
A4 BRACED FRAME ELEVATION
SCALE: 1/8" = 1'-0"



A5 BRACED FRAME ELEVATION
SCALE: 1/8" = 1'-0"



A1 WALL SECTION
SCALE: 3/8" = 1'-0"



A3 WALL SECTION
SCALE: 3/8" = 1'-0"

**WILMA P. MANKILLER HEALTH CENTER
EXPANSION**
STILWELL, OKLAHOMA

KEY PLAN:

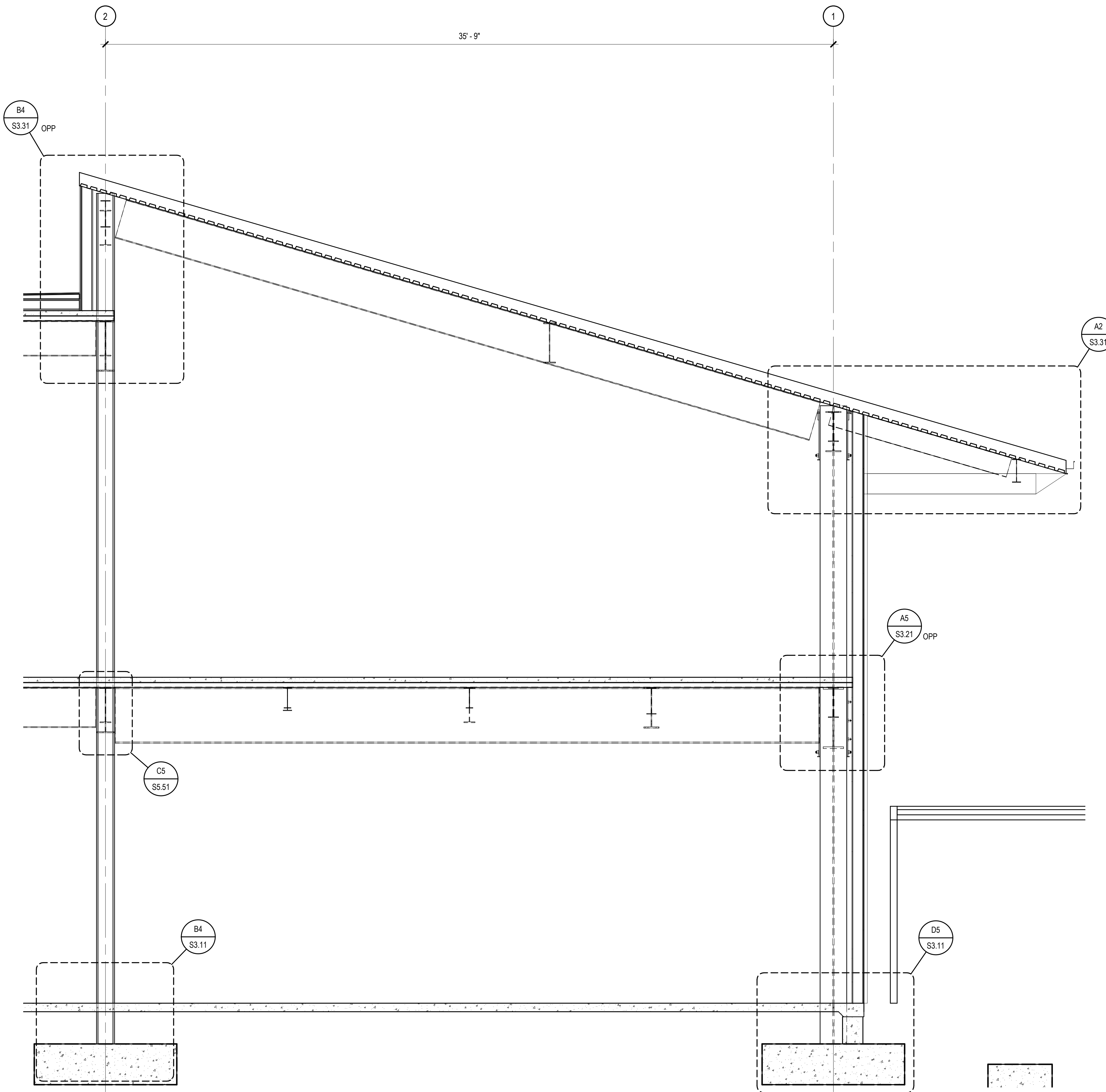
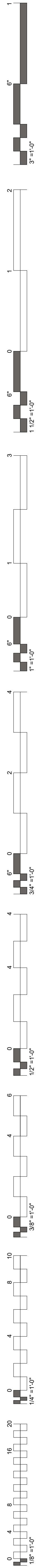
PROJECT PHASE:
 BID PACKAGE 01

#	DATE	REVISIONS	DESCRIPTION

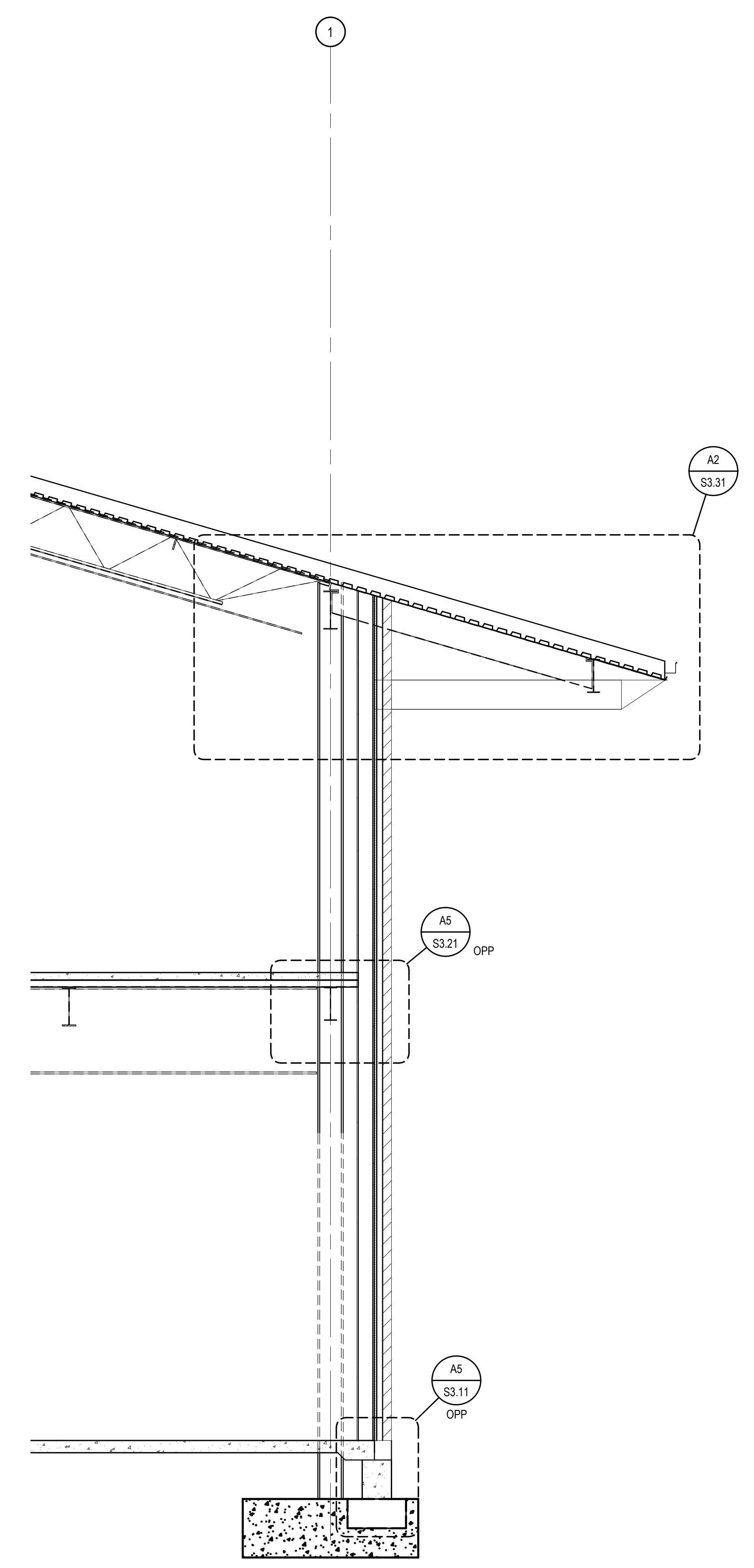
DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER:
S3.01

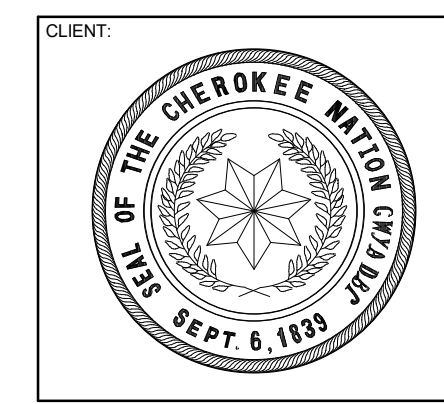
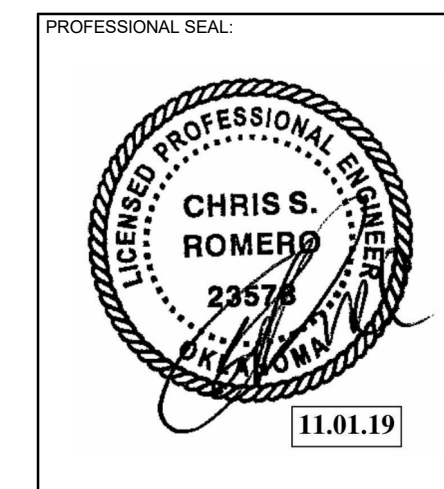
WALL SECTIONS



A1 WALL SECTION
SCALE: 3/8" = 1'-0"



A4 WALL SECTION
SCALE: 3/8" = 1'-0"



**WILMA P. MANKILLER HEALTH CENTER
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STILWELL, OKLAHOMA

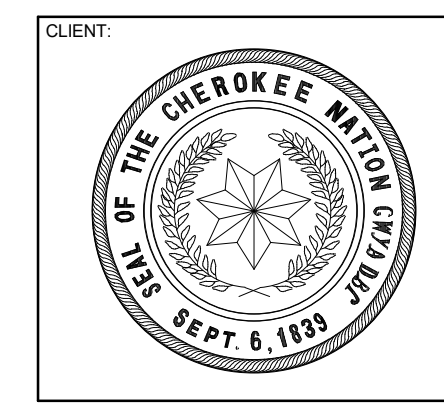
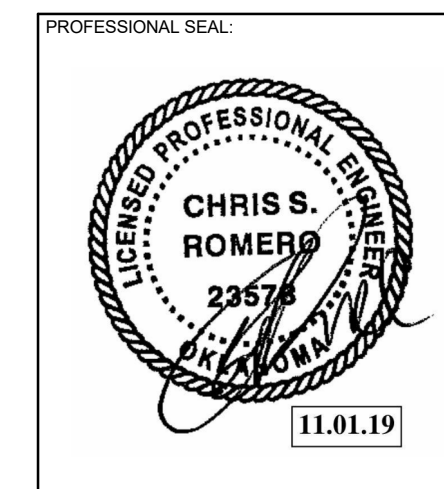
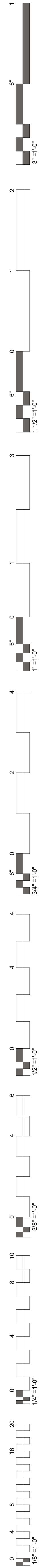
KEY PLAN:
PROJECT PHASE:
BID PACKAGE 01

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DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER:
S3.02

WALL SECTIONS



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KEY PLAN:

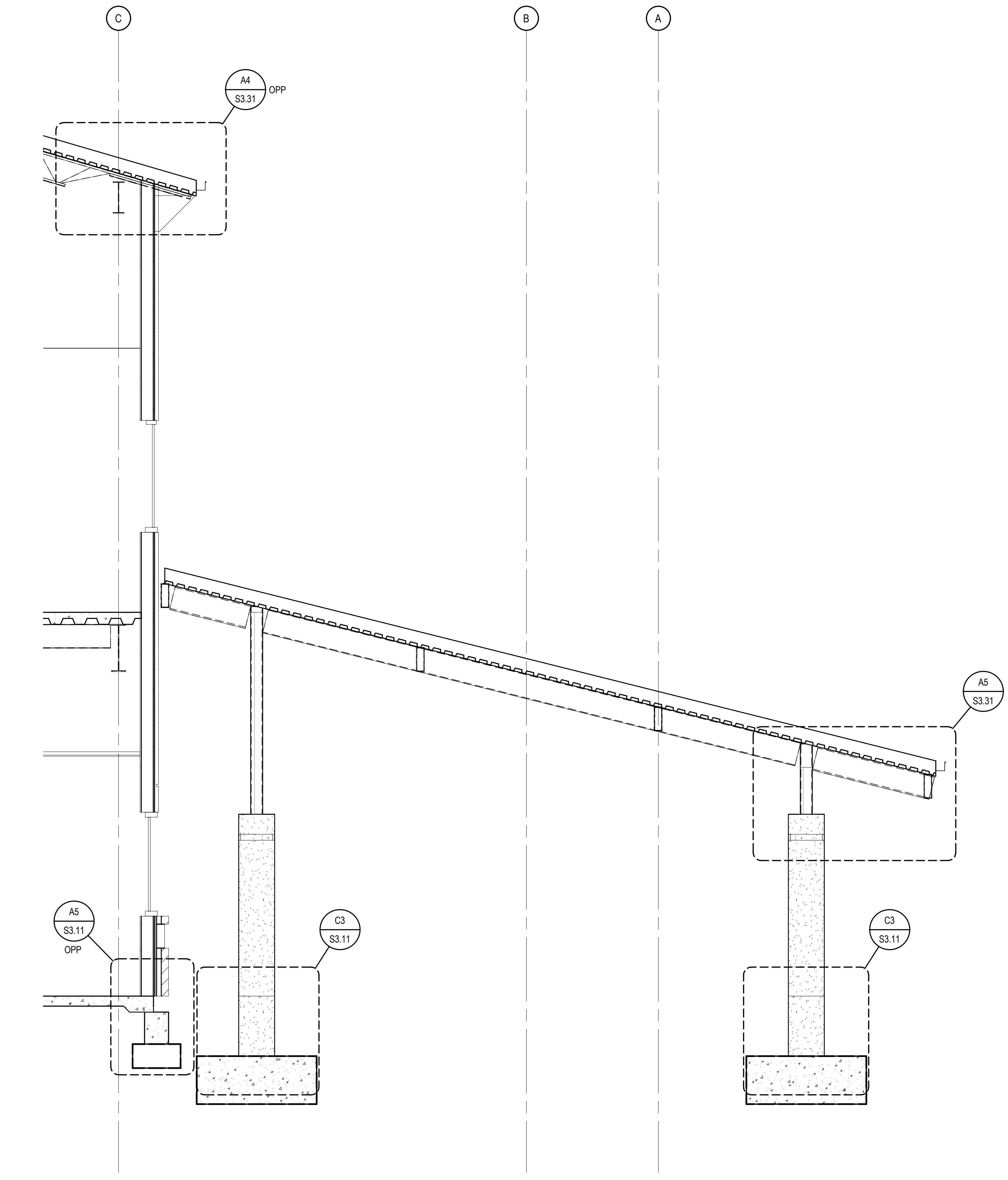
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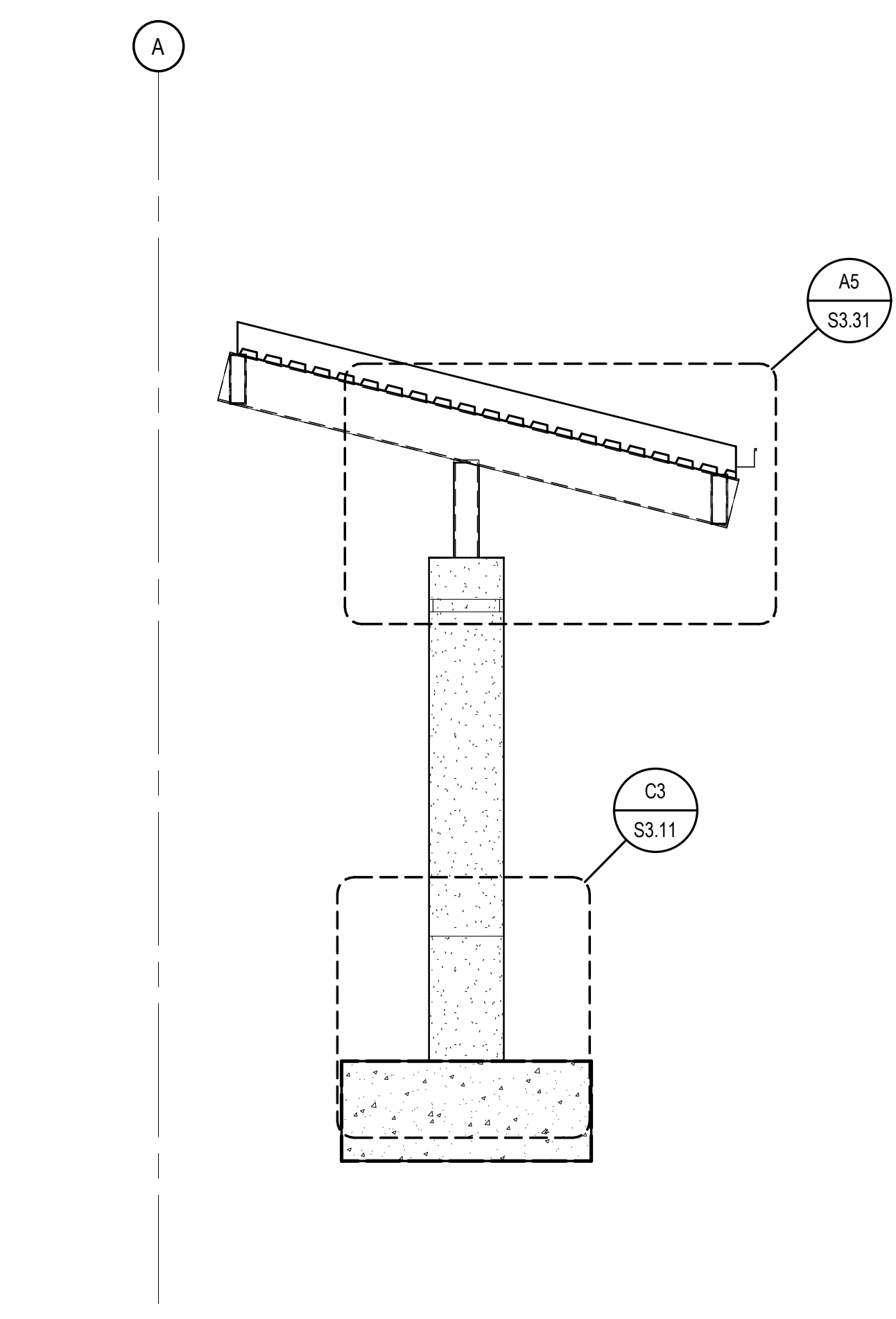
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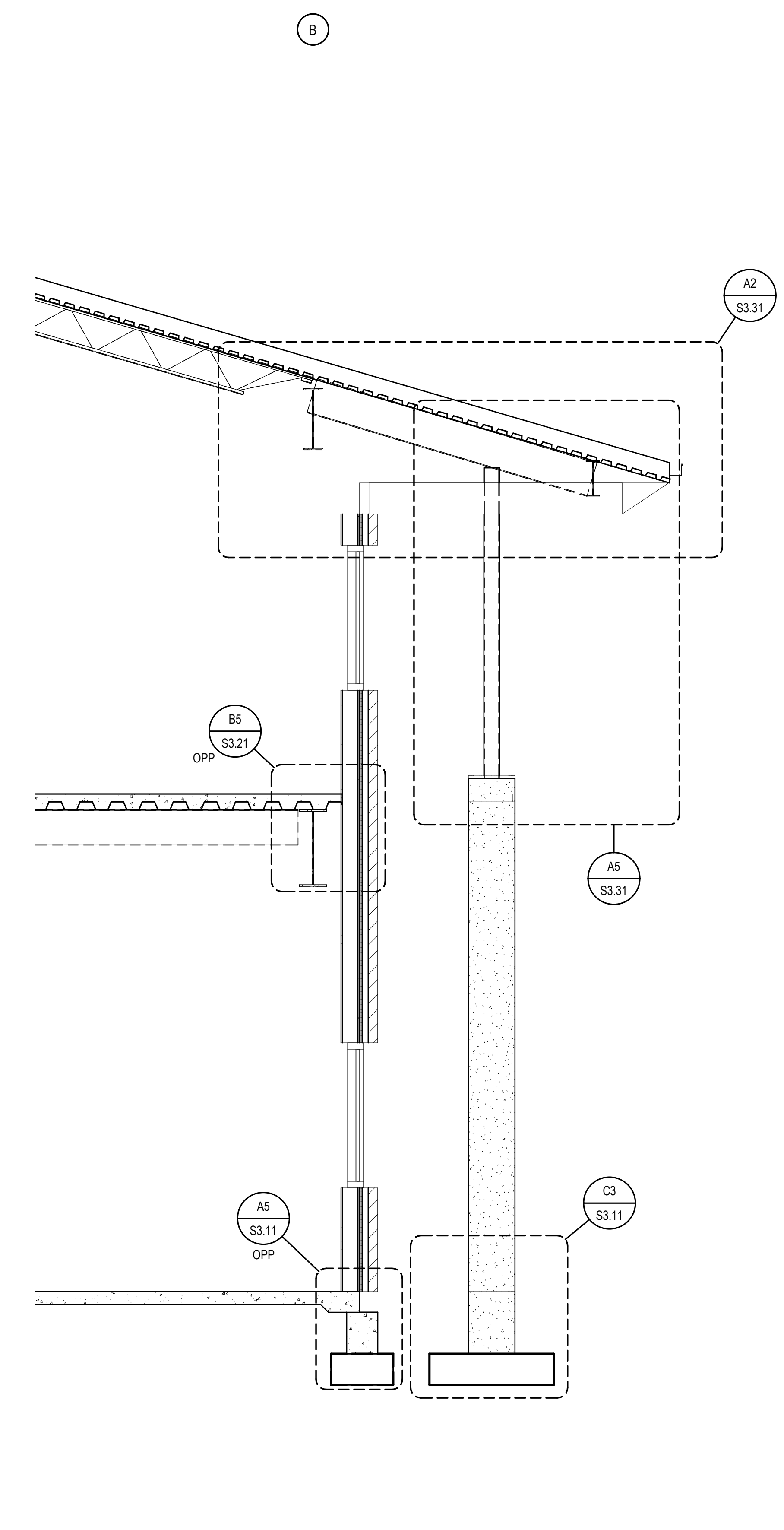
WALL SECTIONS



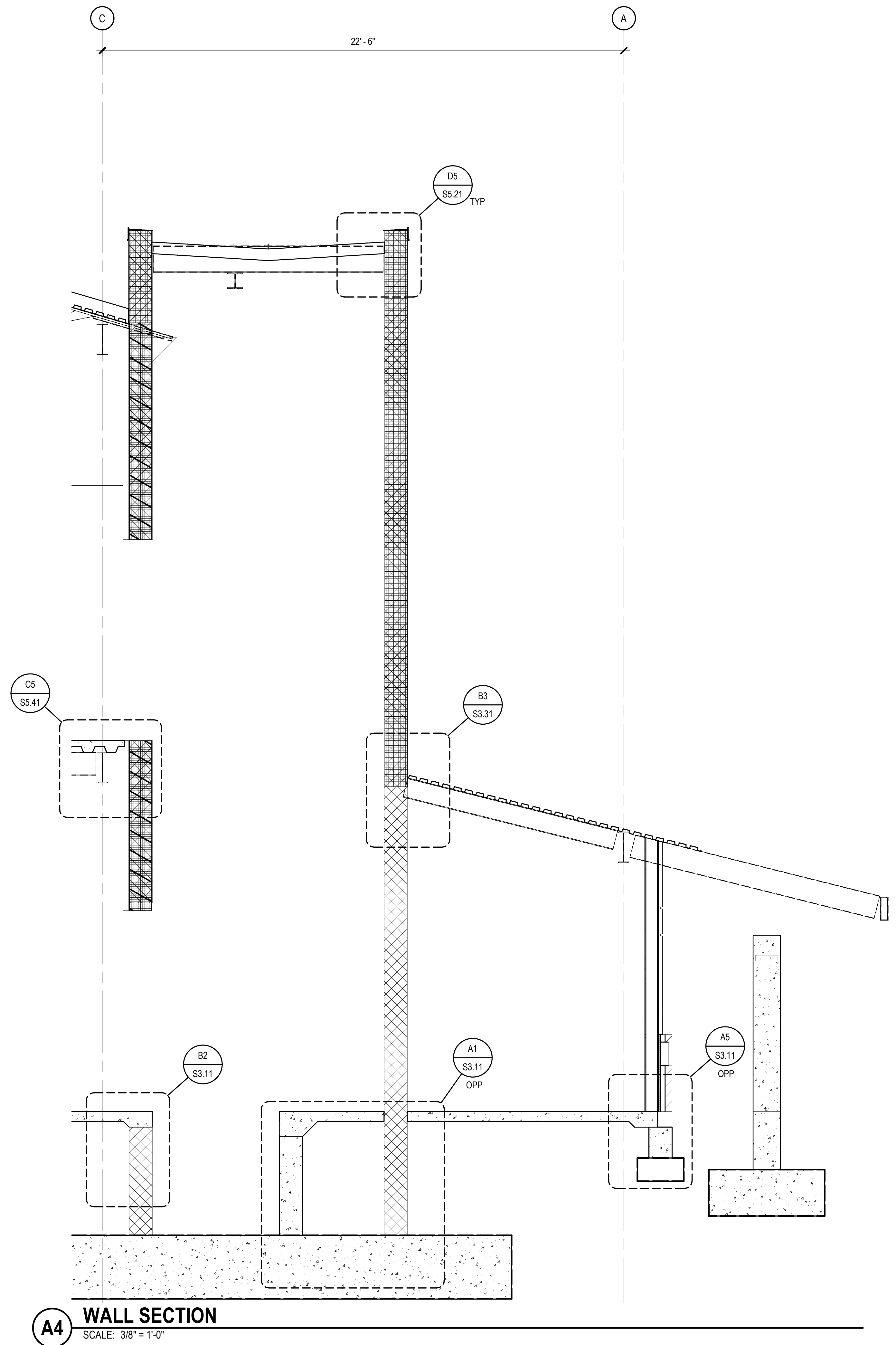
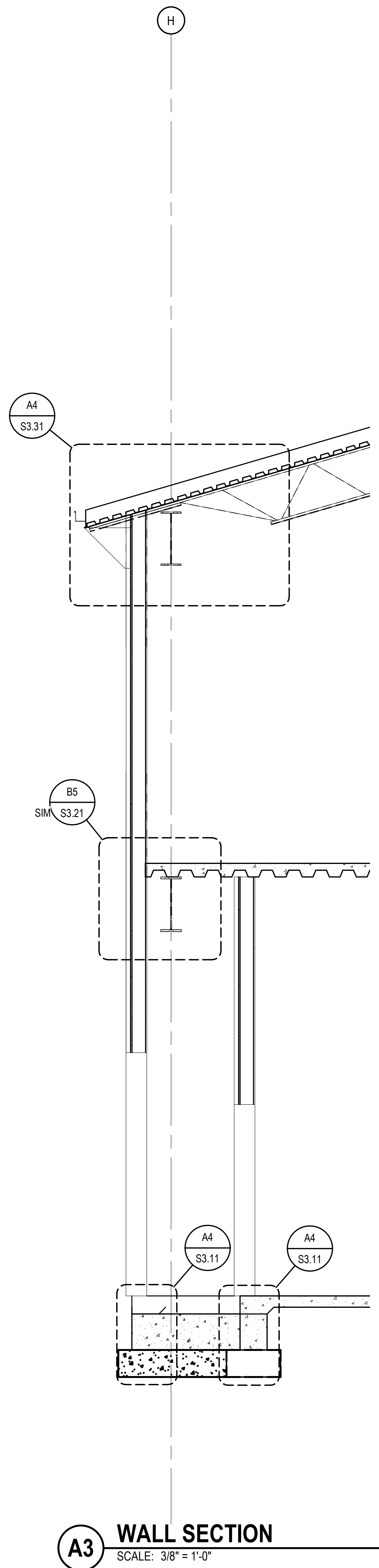
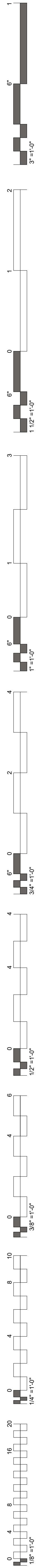
A1 WALL SECTION
SCALE: 3/8" = 1'-0"



A4 WALL SECTION
SCALE: 3/8" = 1'-0"

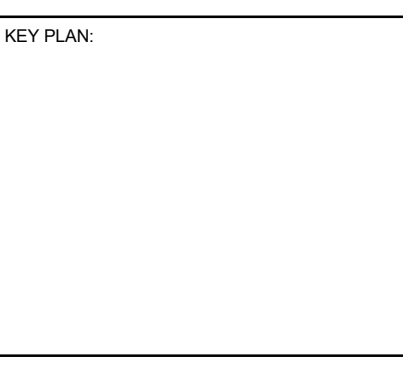


A5 WALL SECTION
SCALE: 3/8" = 1'-0"



WILMA P. MANKILLER HEALTH CENTER
EXPANSION
 STILLWELL, OKLAHOMA

KEY PLAN:



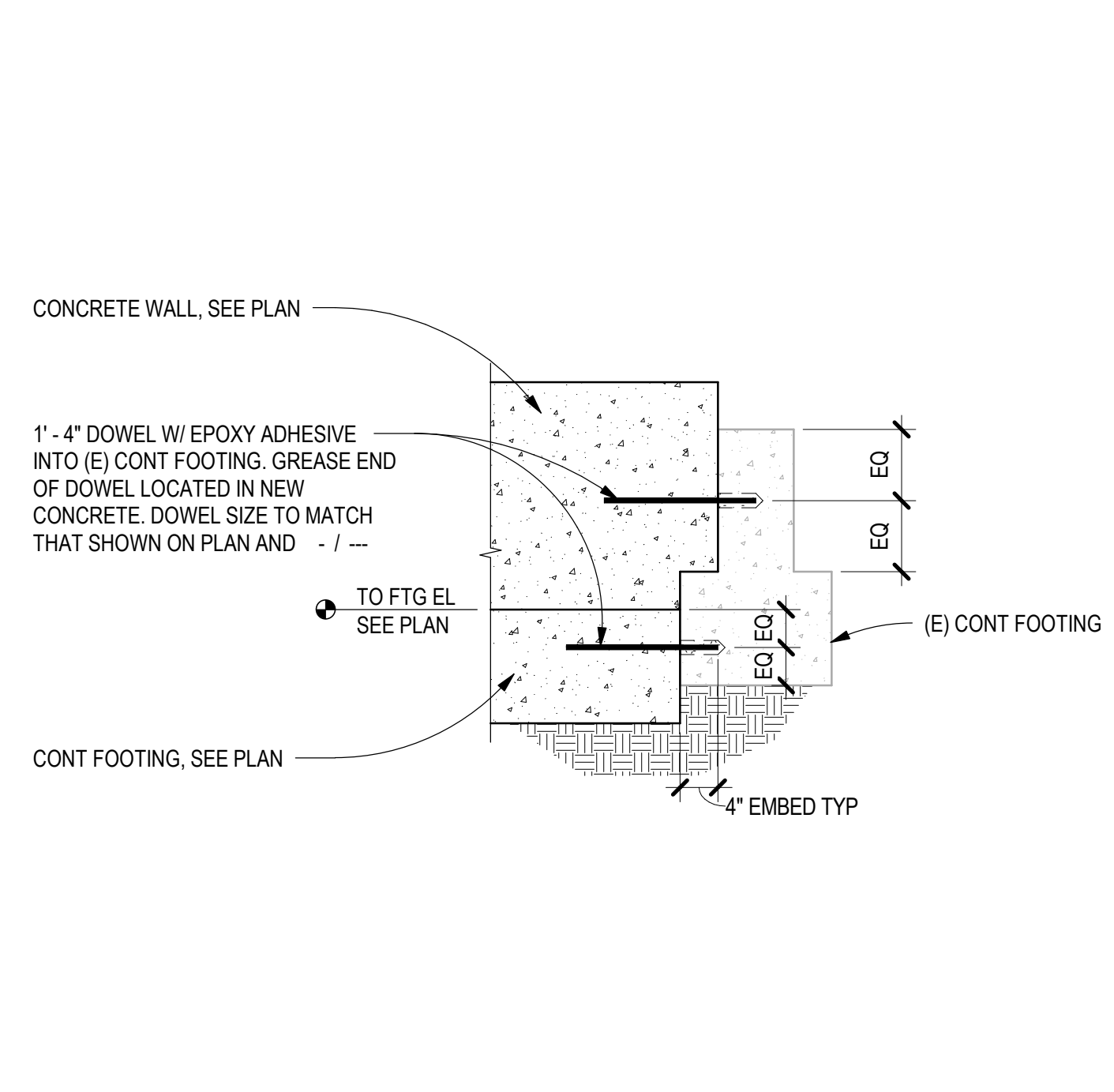
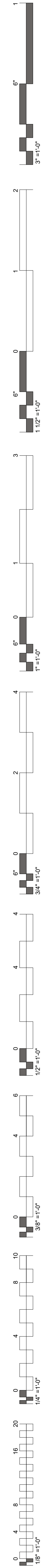
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#	DATE	REVISIONS	DESCRIPTION

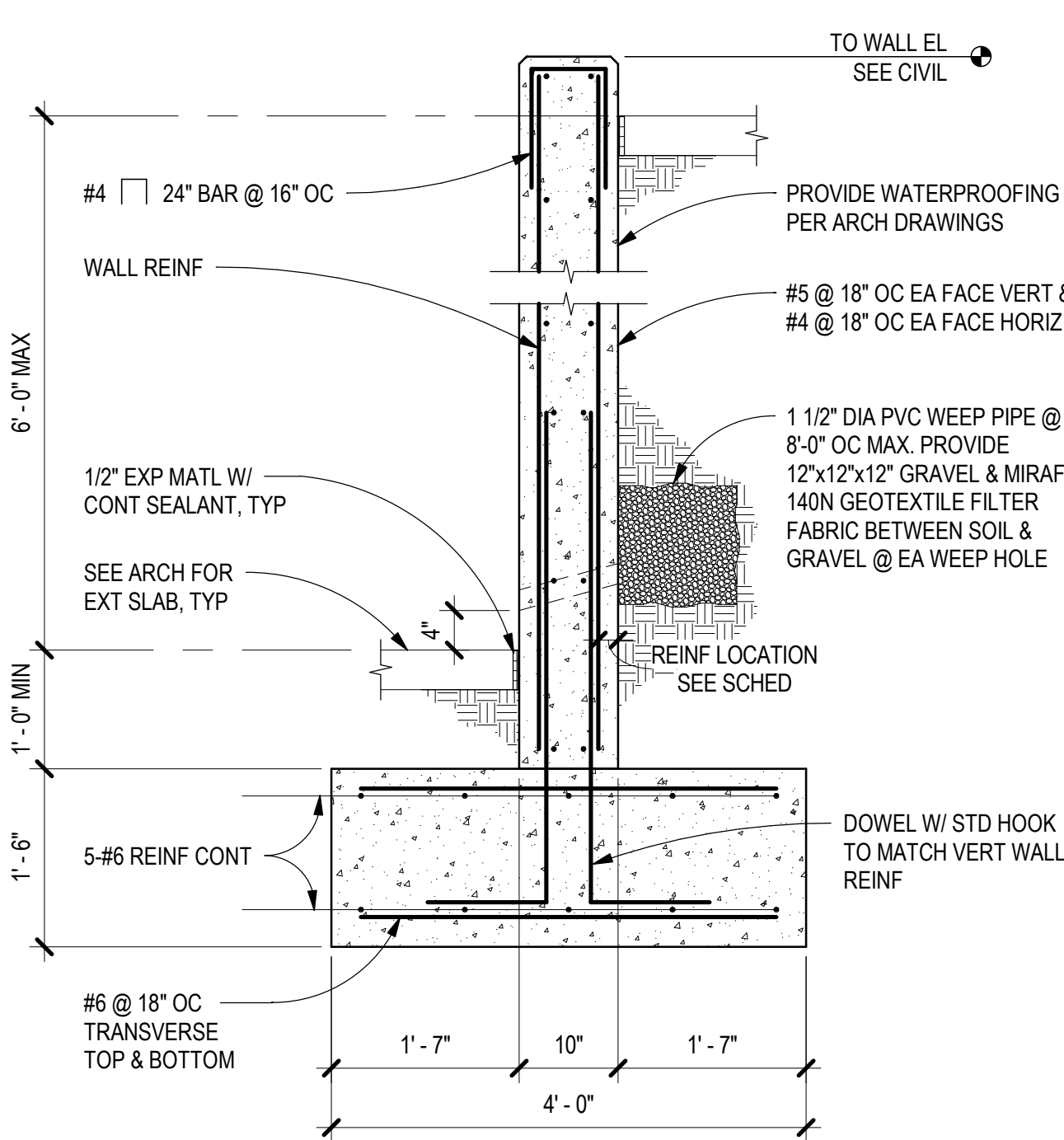
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S3.04

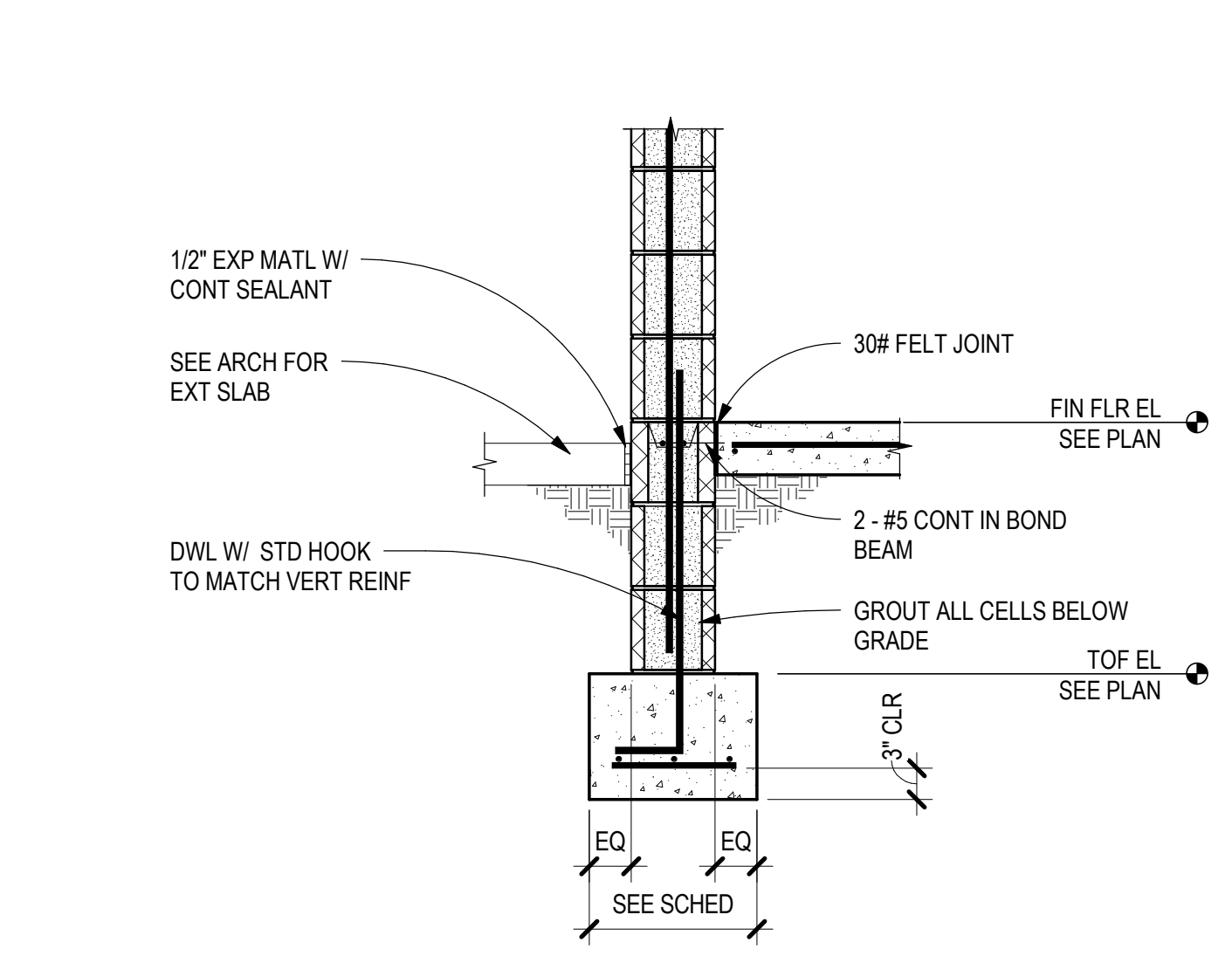
WALL SECTIONS



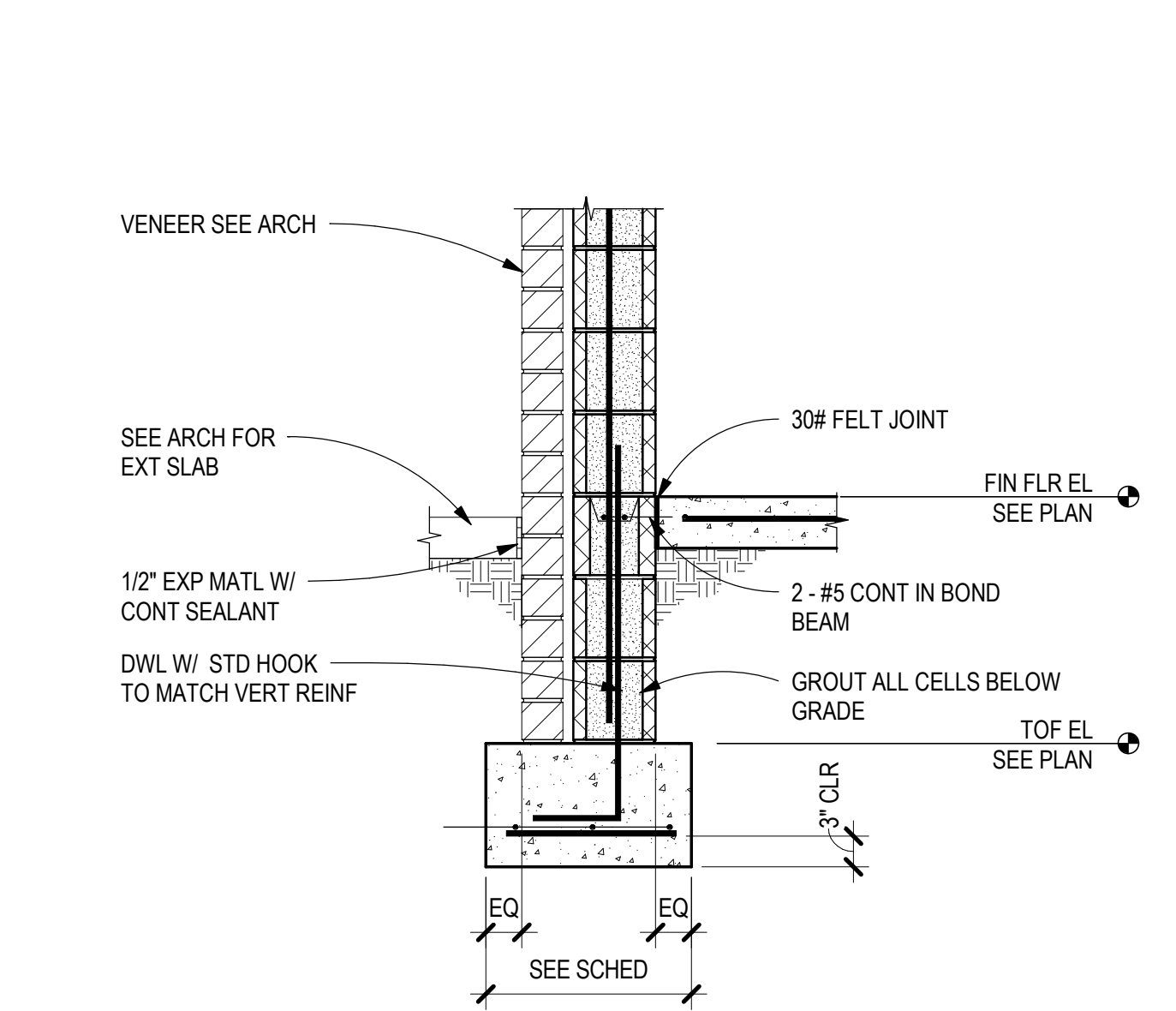
D1 DOWEL INTO (E) CONTINUOUS FOOTING
SCALE: 3/4" = 1'-0"



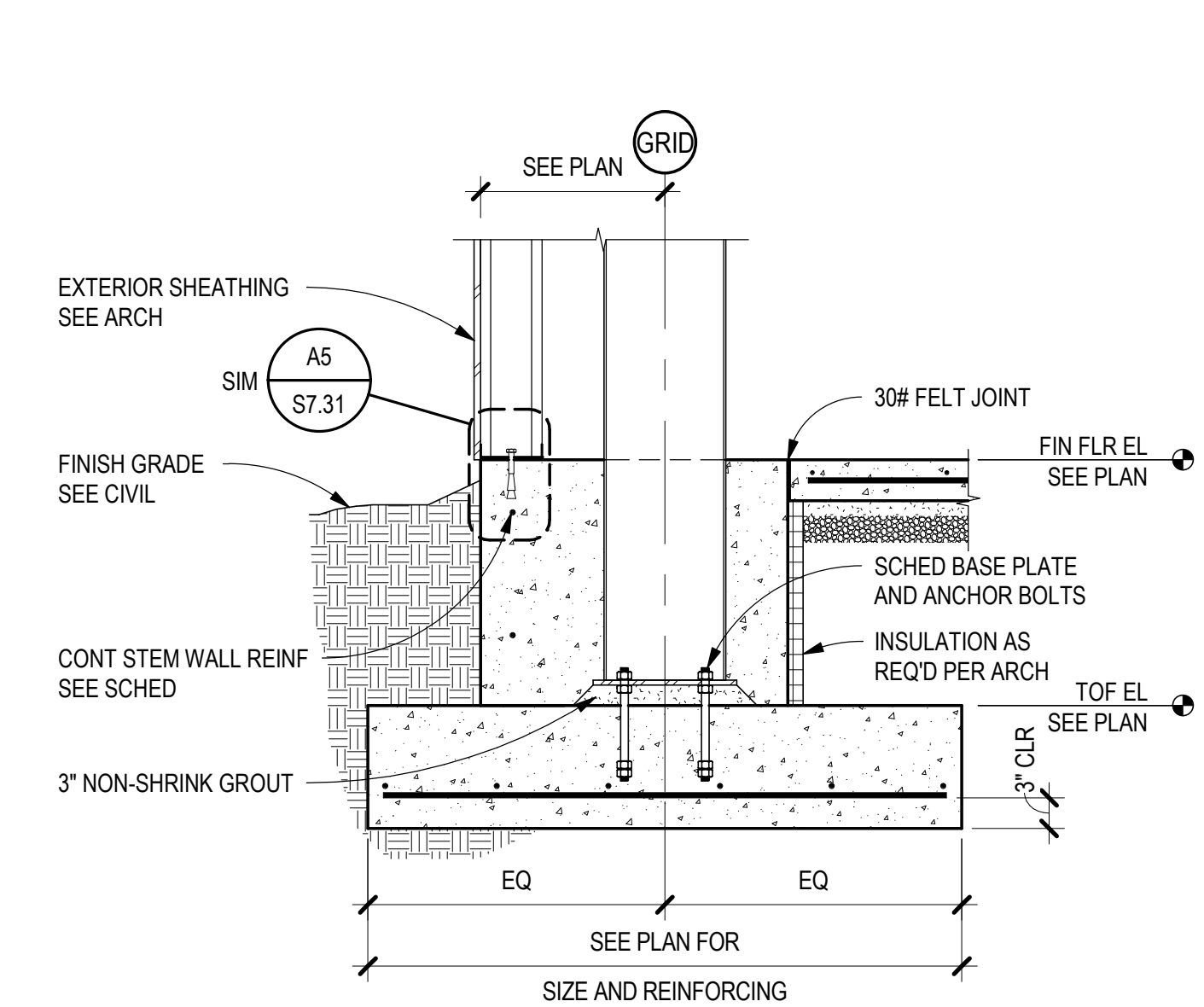
D2 RETAINING WALL SECTION
SCALE: 3/4" = 1'-0"



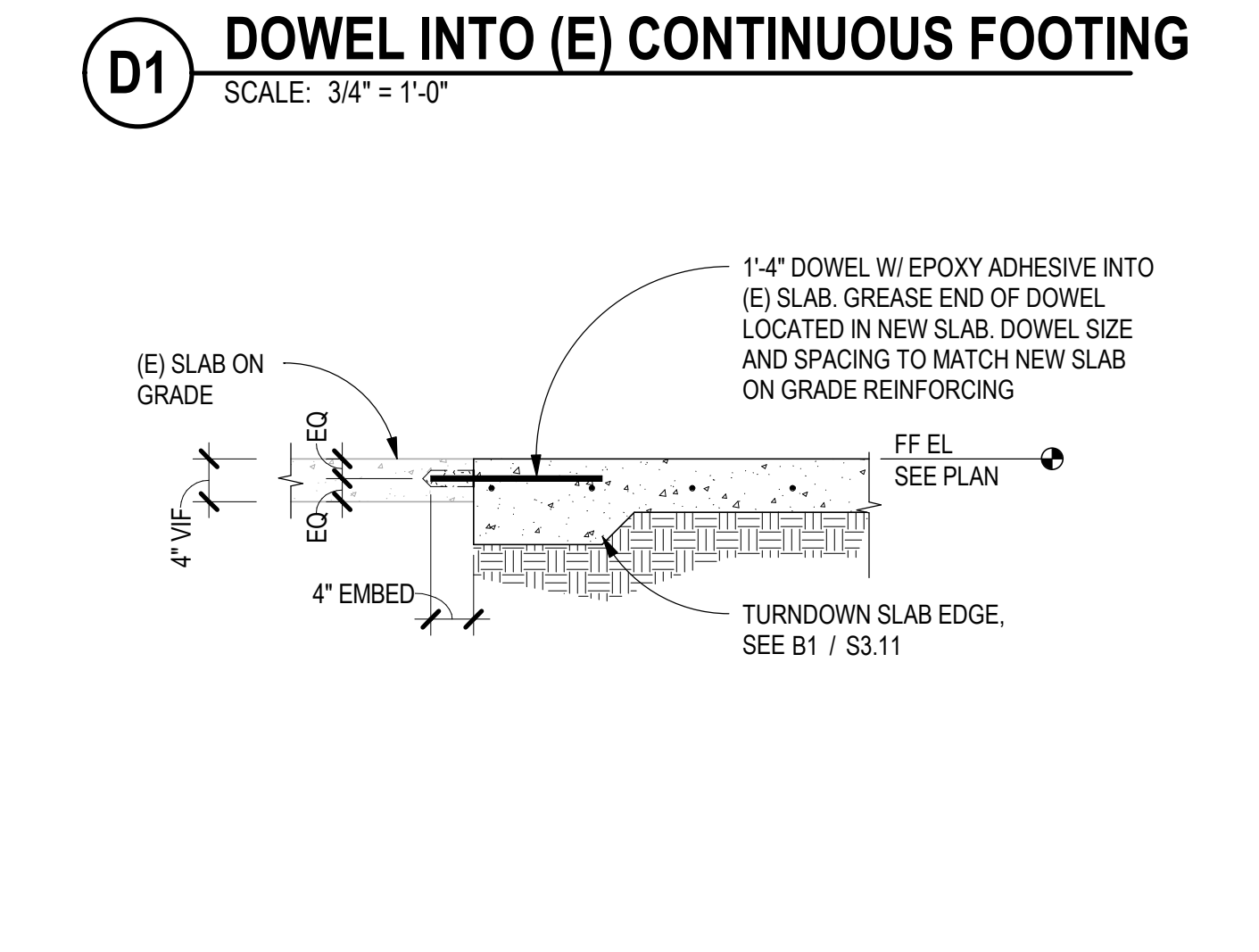
D3 PERIMETER FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



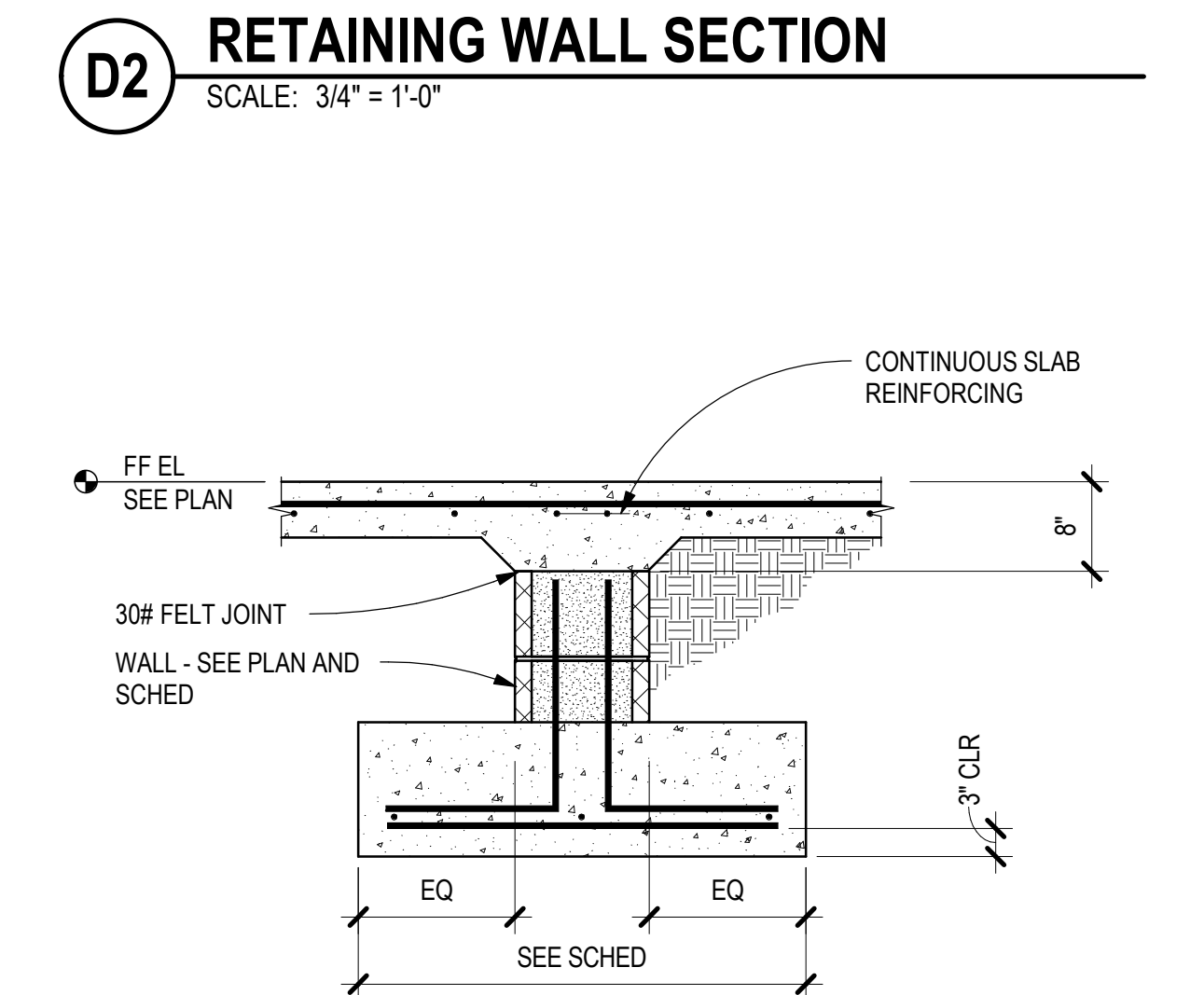
D4 PERIMETER FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



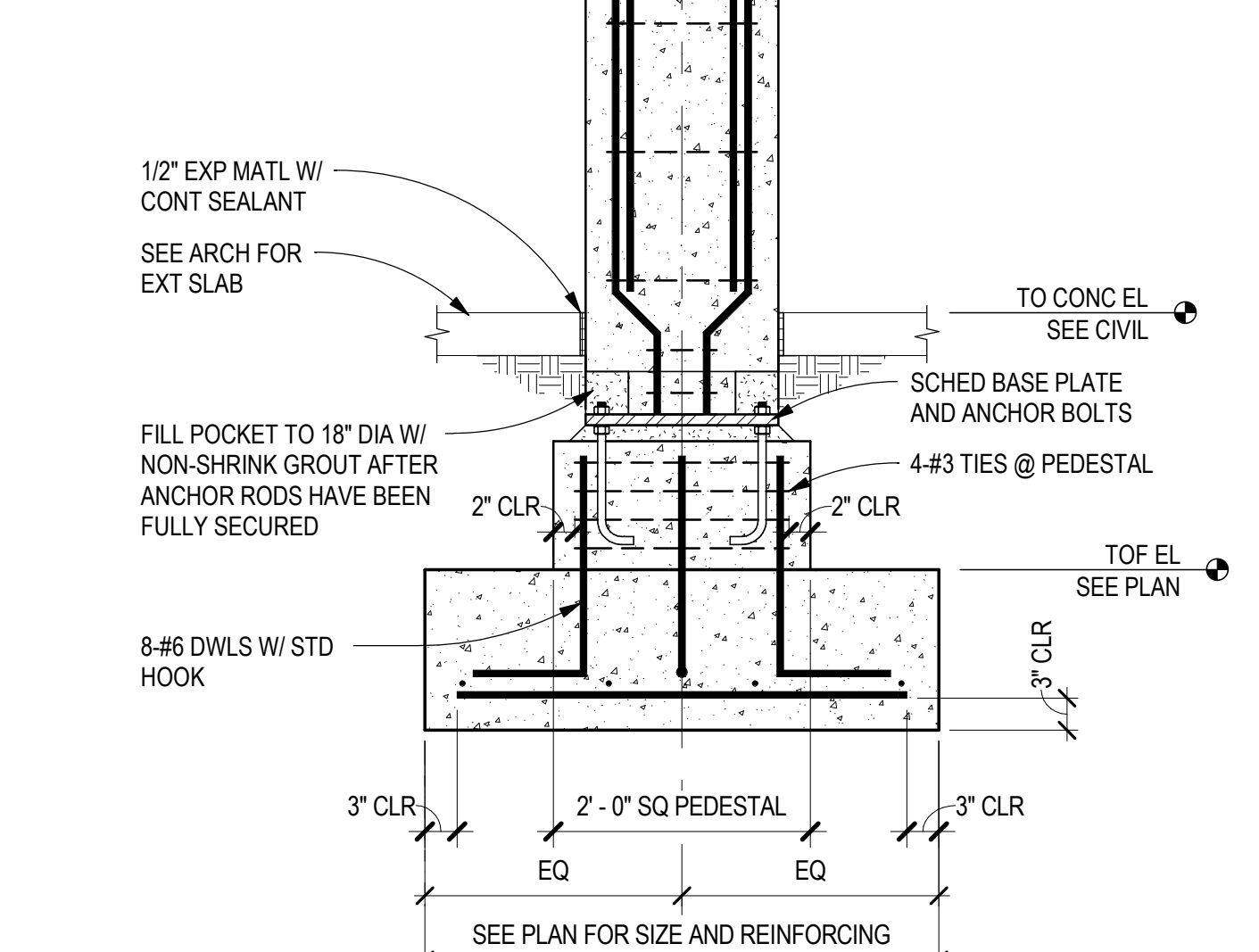
D5 FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



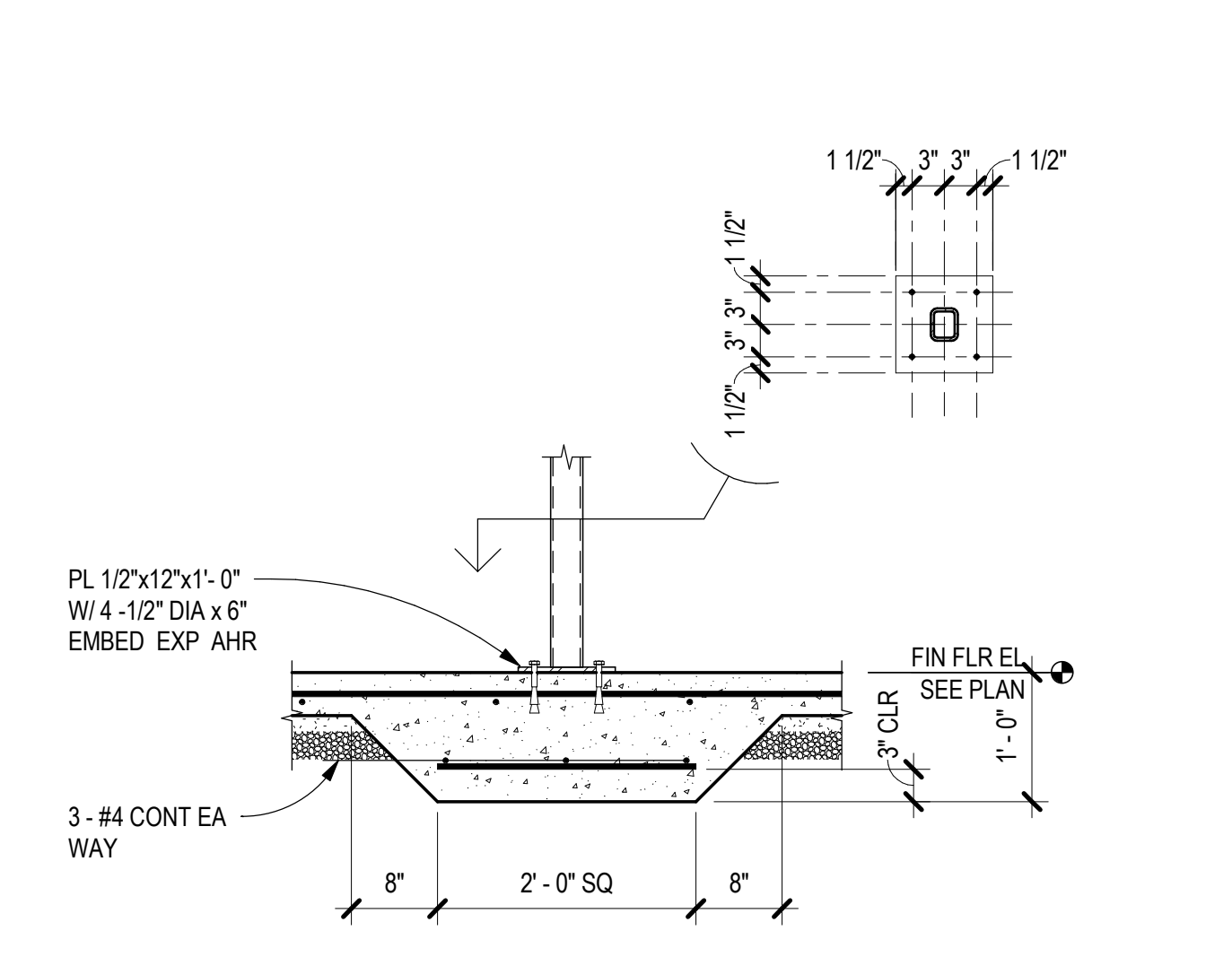
C1 DOWEL INTO (E) SLAB ON GRADE
SCALE: 3/4" = 1'-0"



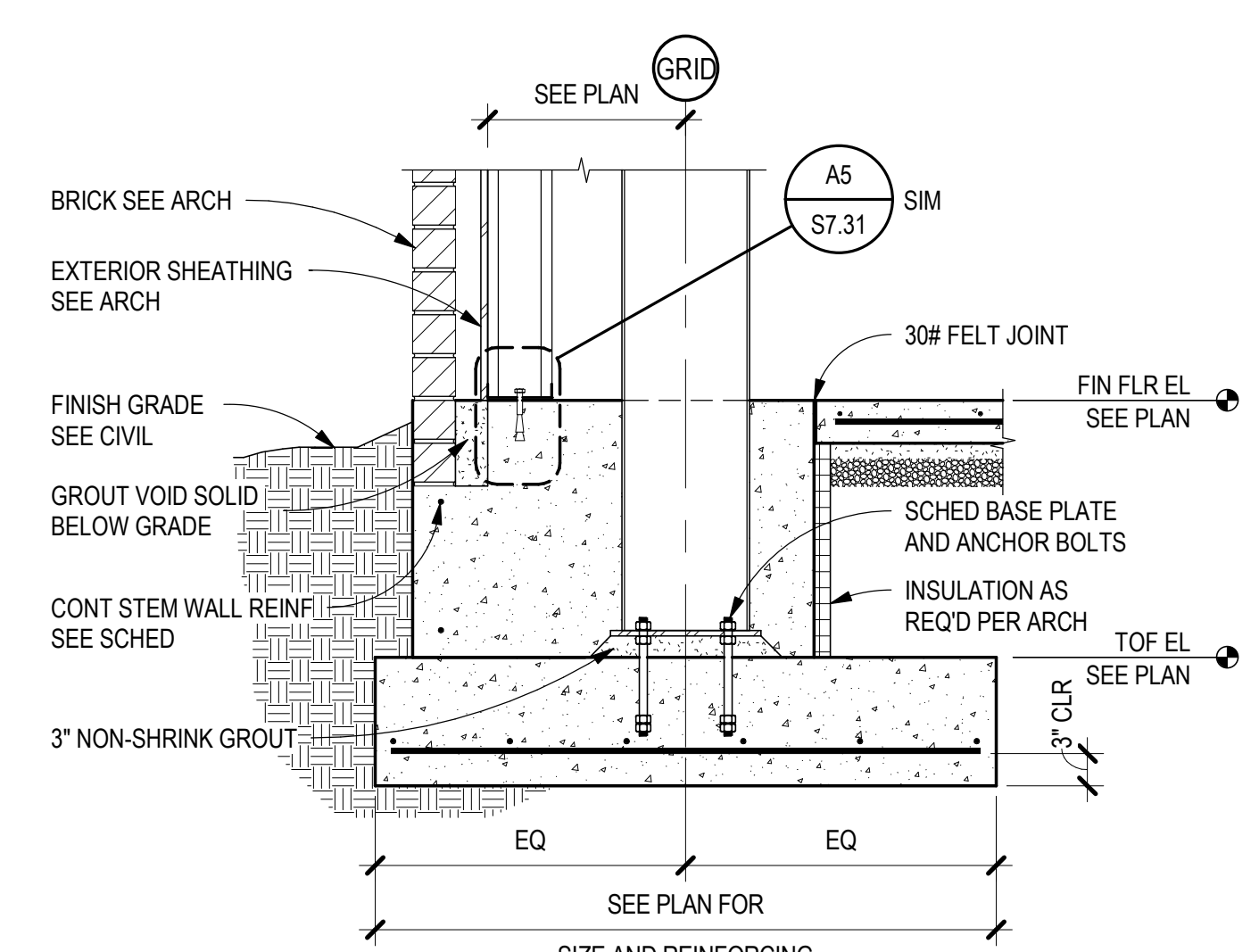
C2 INT SECTION @ DOORWAYS CMU
SCALE: 3/4" = 1'-0"



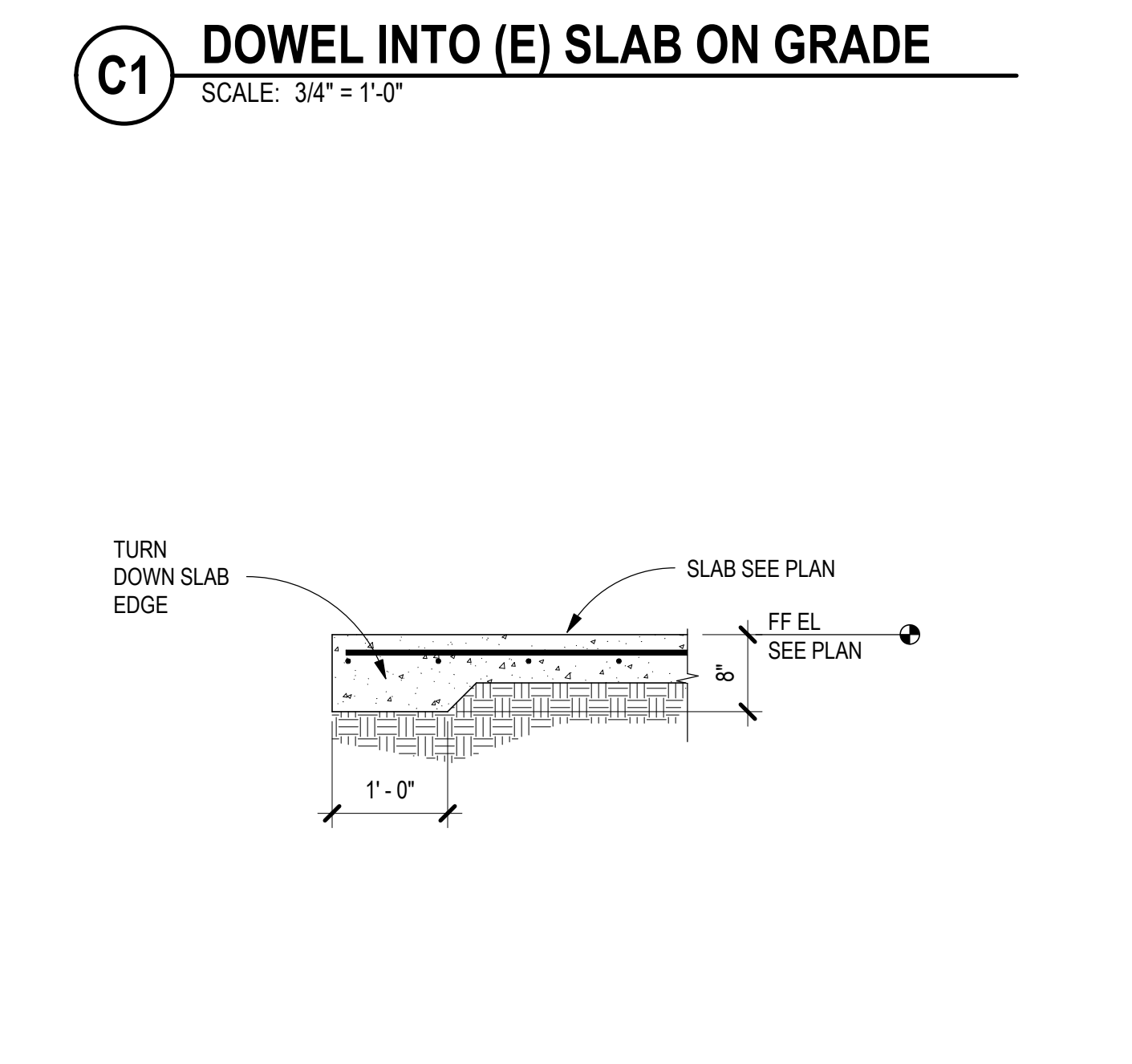
C3 EXT CONC COLUMN SECTION
SCALE: 3/4" = 1'-0"



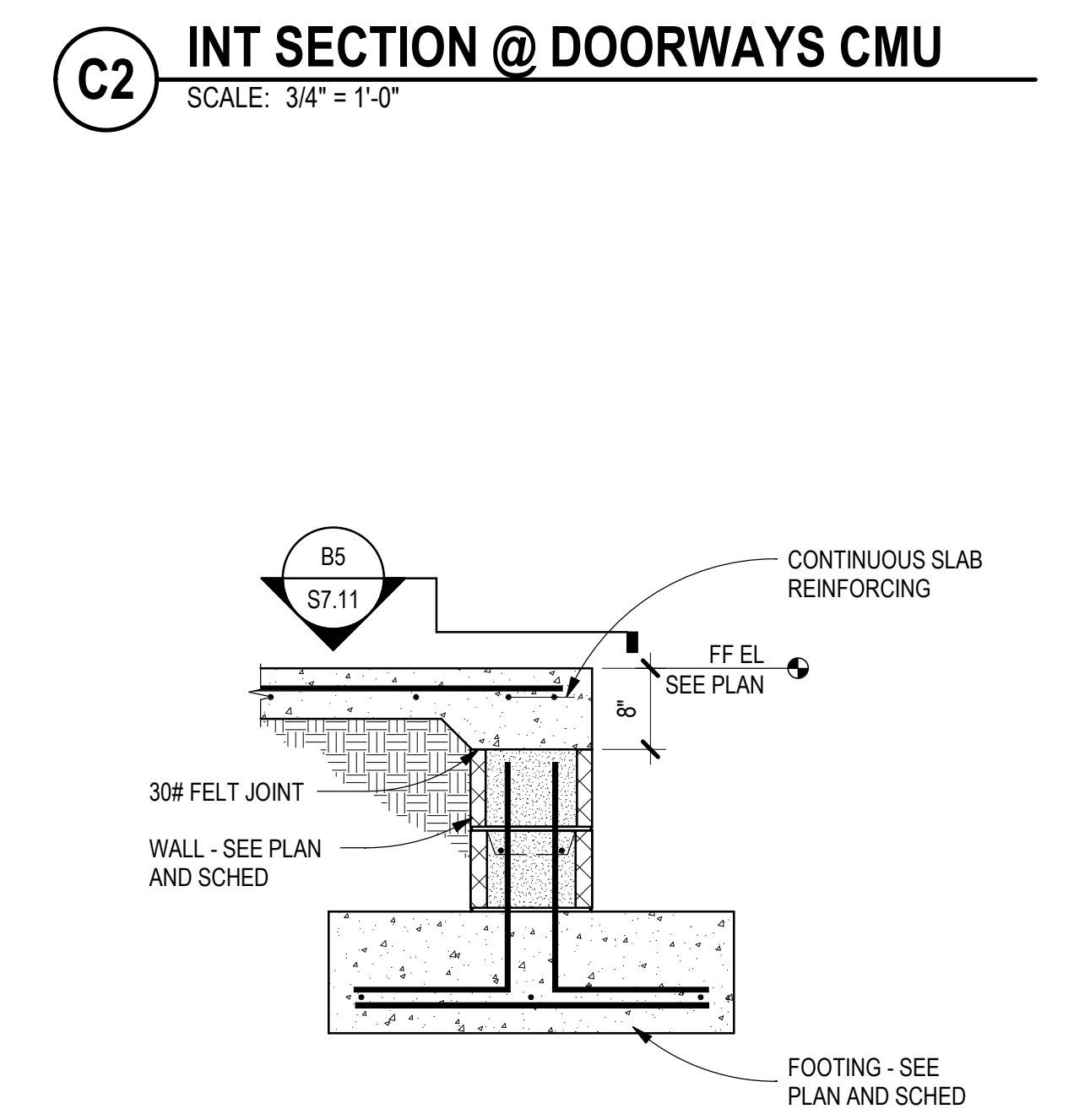
C4 INT COLUMN AT THICKENED SLAB
SCALE: 3/4" = 1'-0"



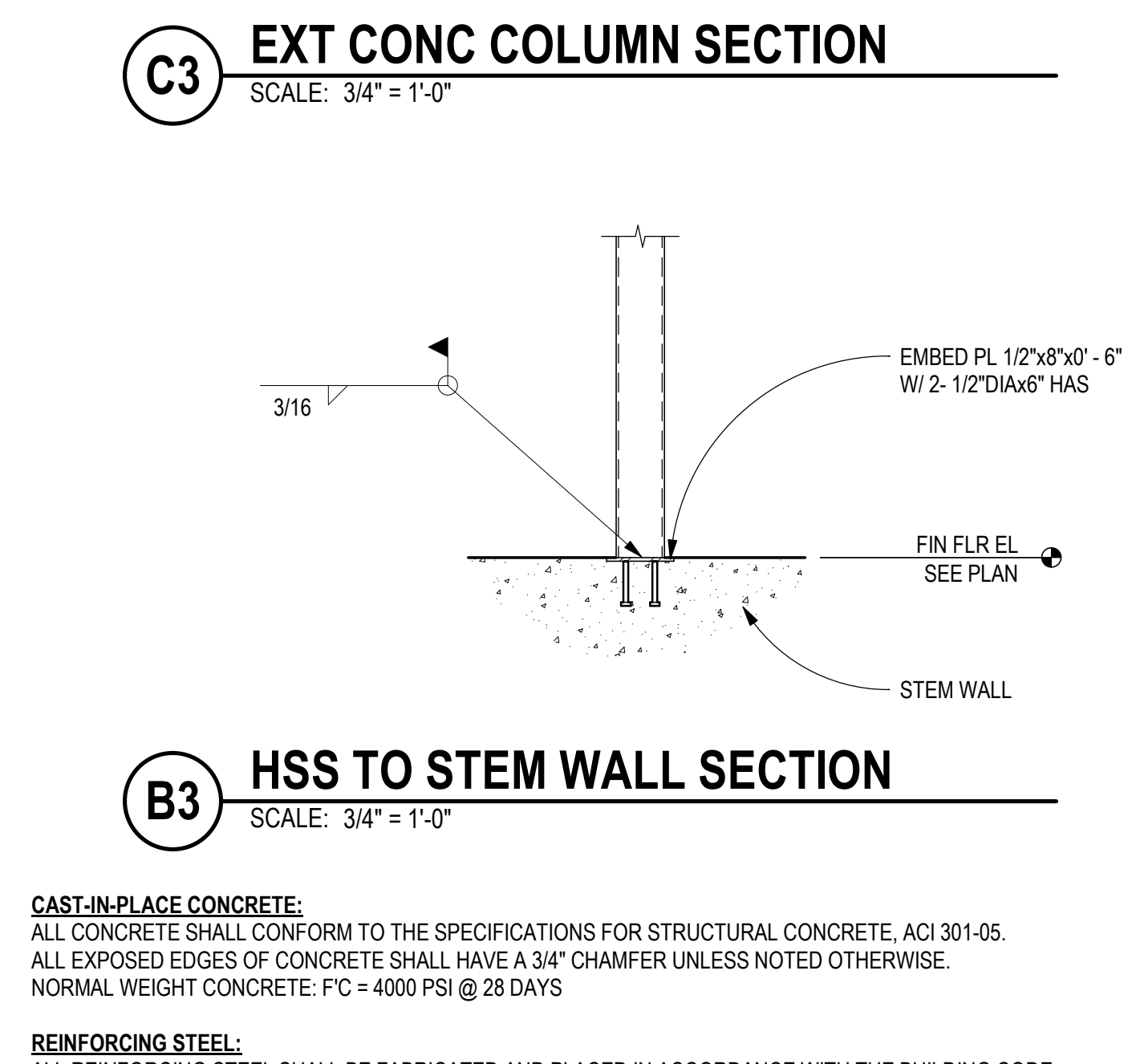
C5 FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



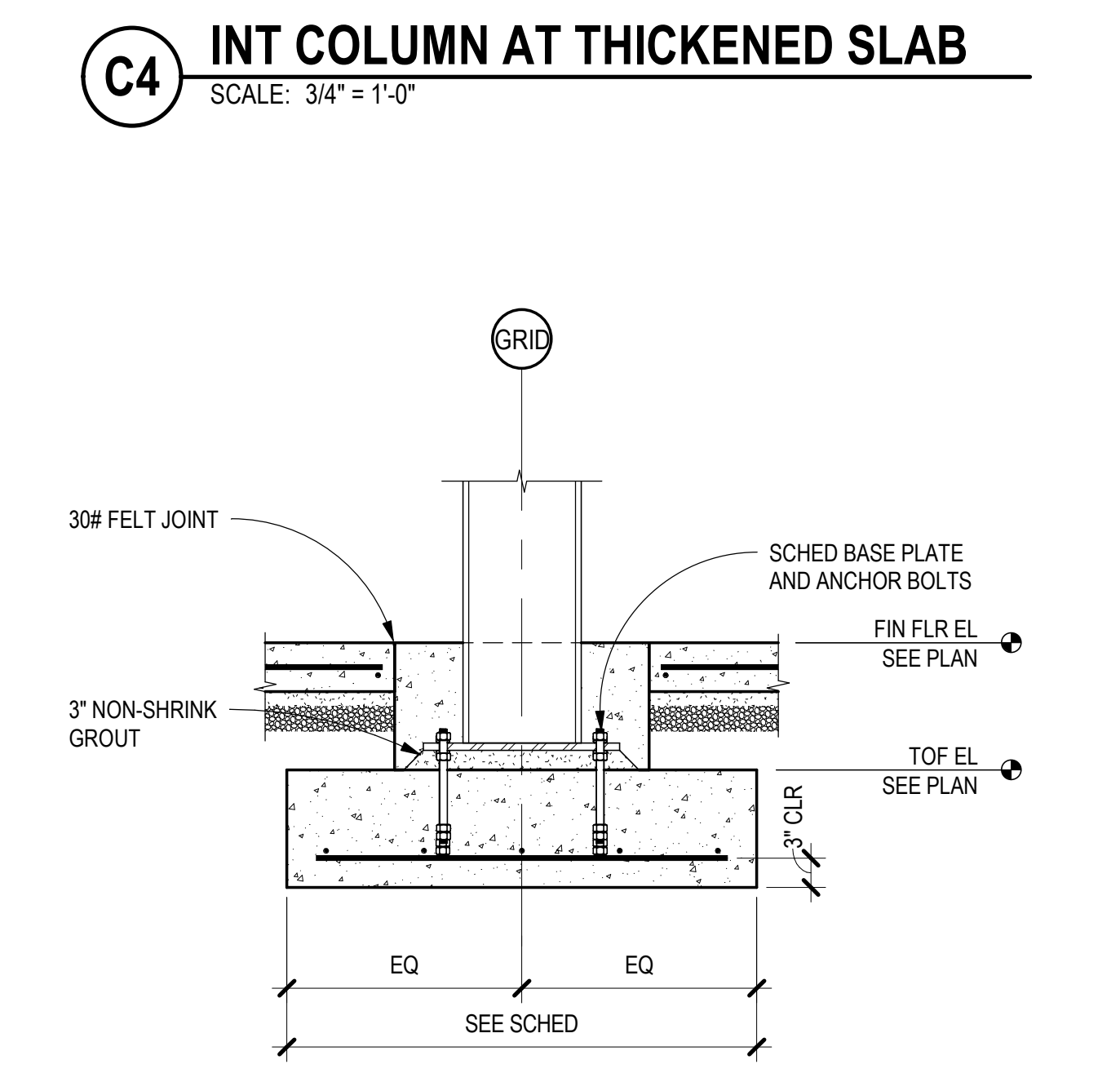
B1 TYP TURN DOWN SLAB EDGE
SCALE: 3/4" = 1'-0"



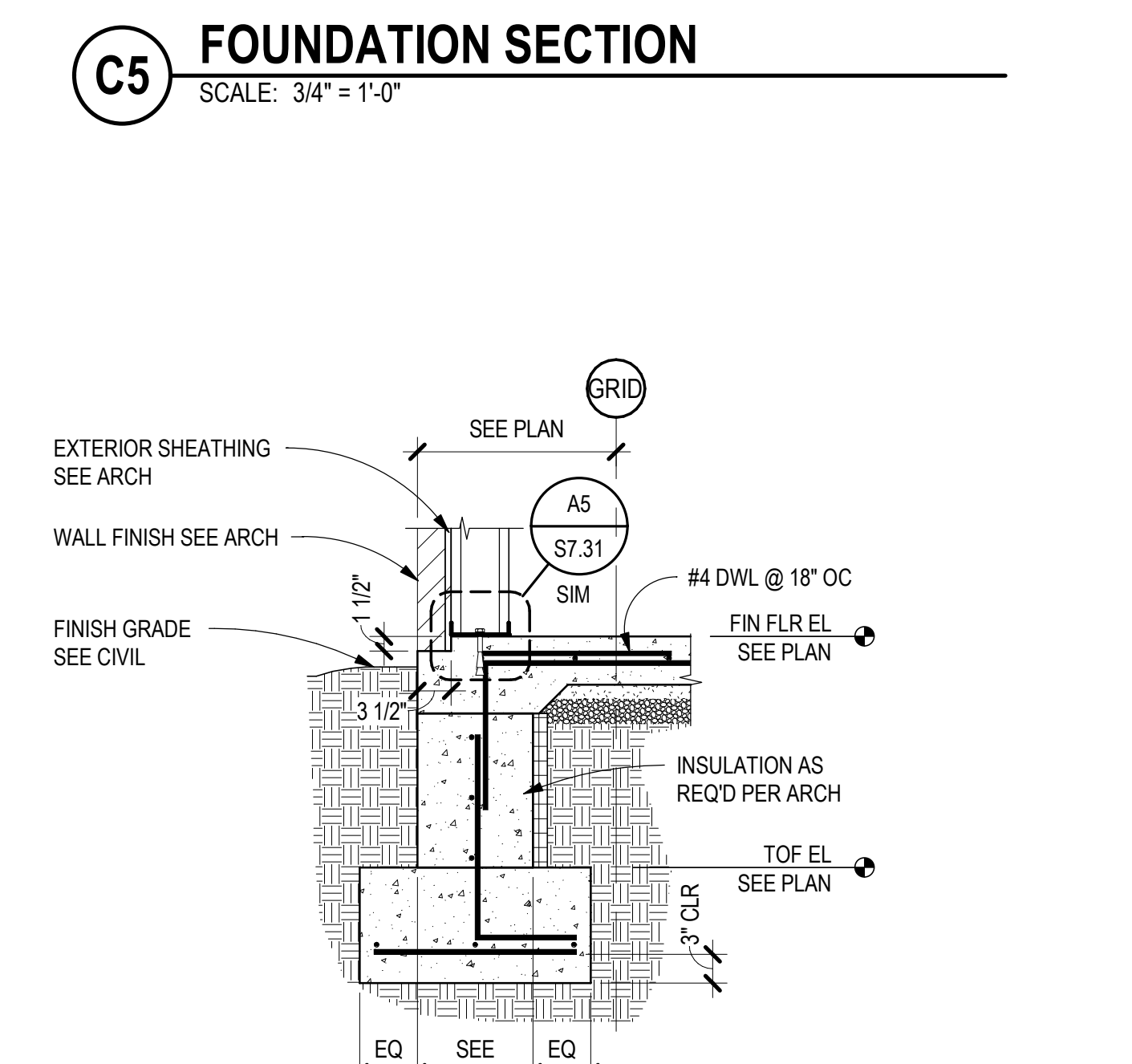
B2 PERIM SECTION AT DOORWAYS CMU
SCALE: 3/4" = 1'-0"



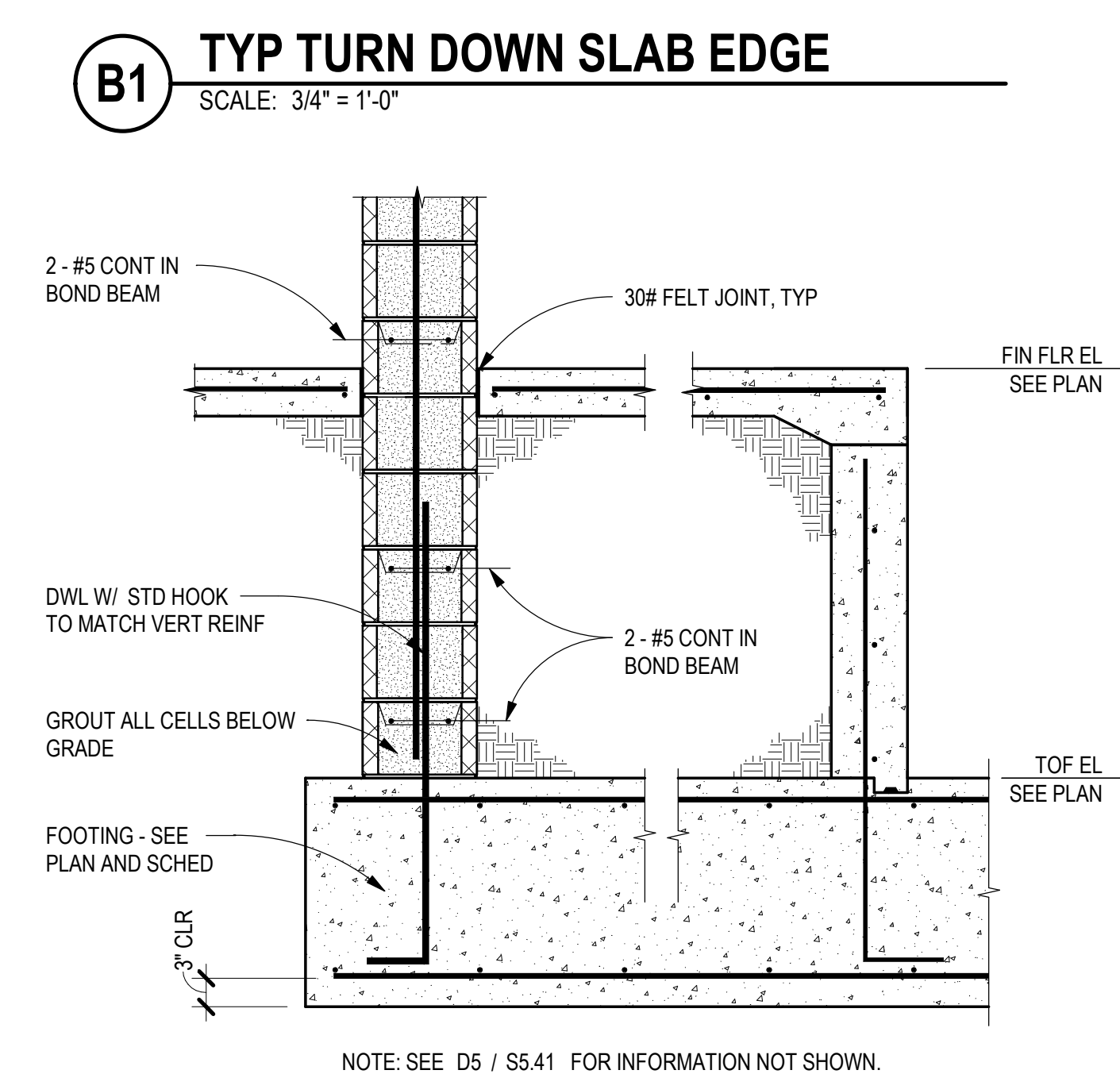
B3 HSS TO STEM WALL SECTION
SCALE: 3/4" = 1'-0"



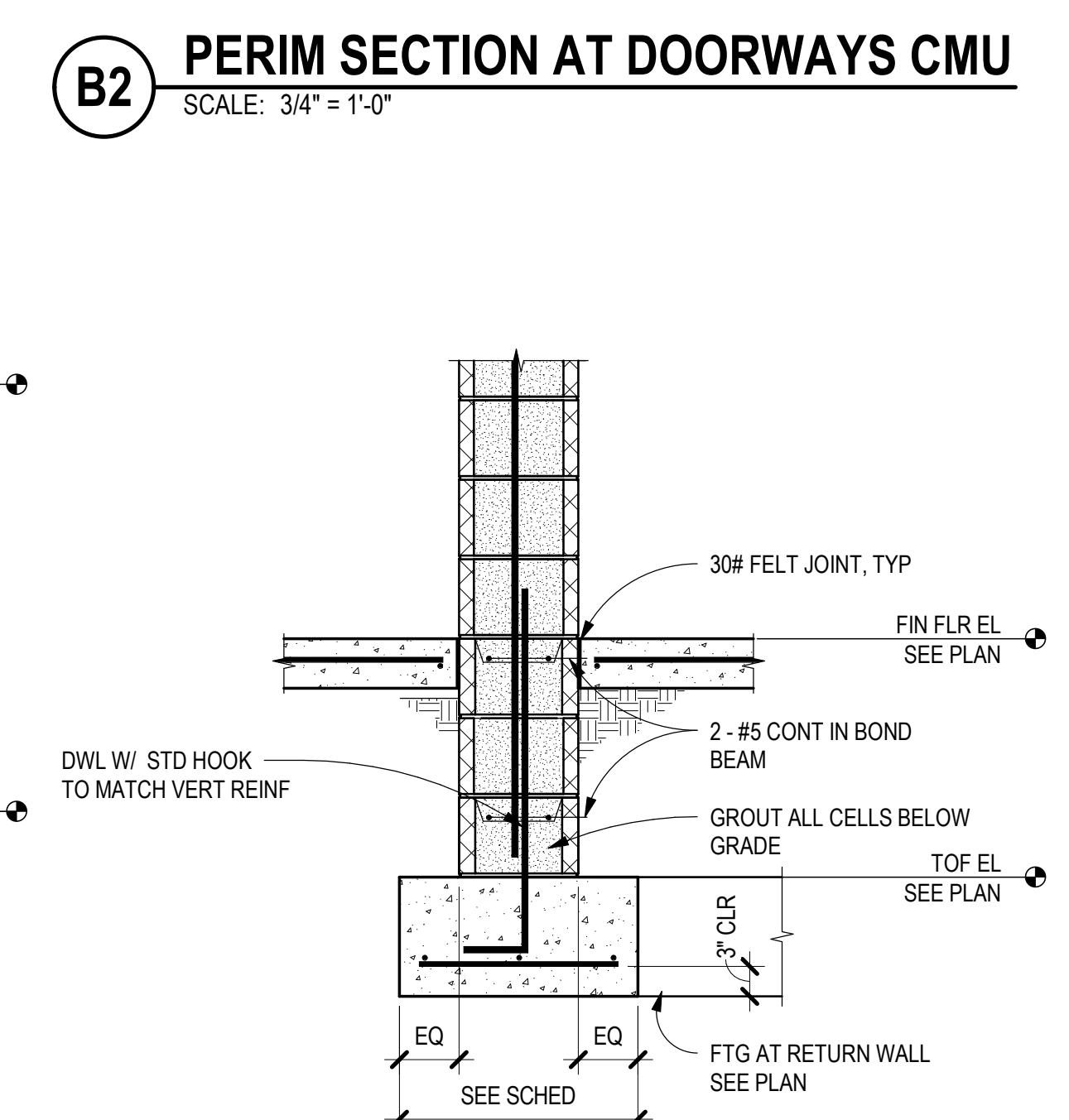
B4 INTERIOR COLUMN SECTION
SCALE: 3/4" = 1'-0"



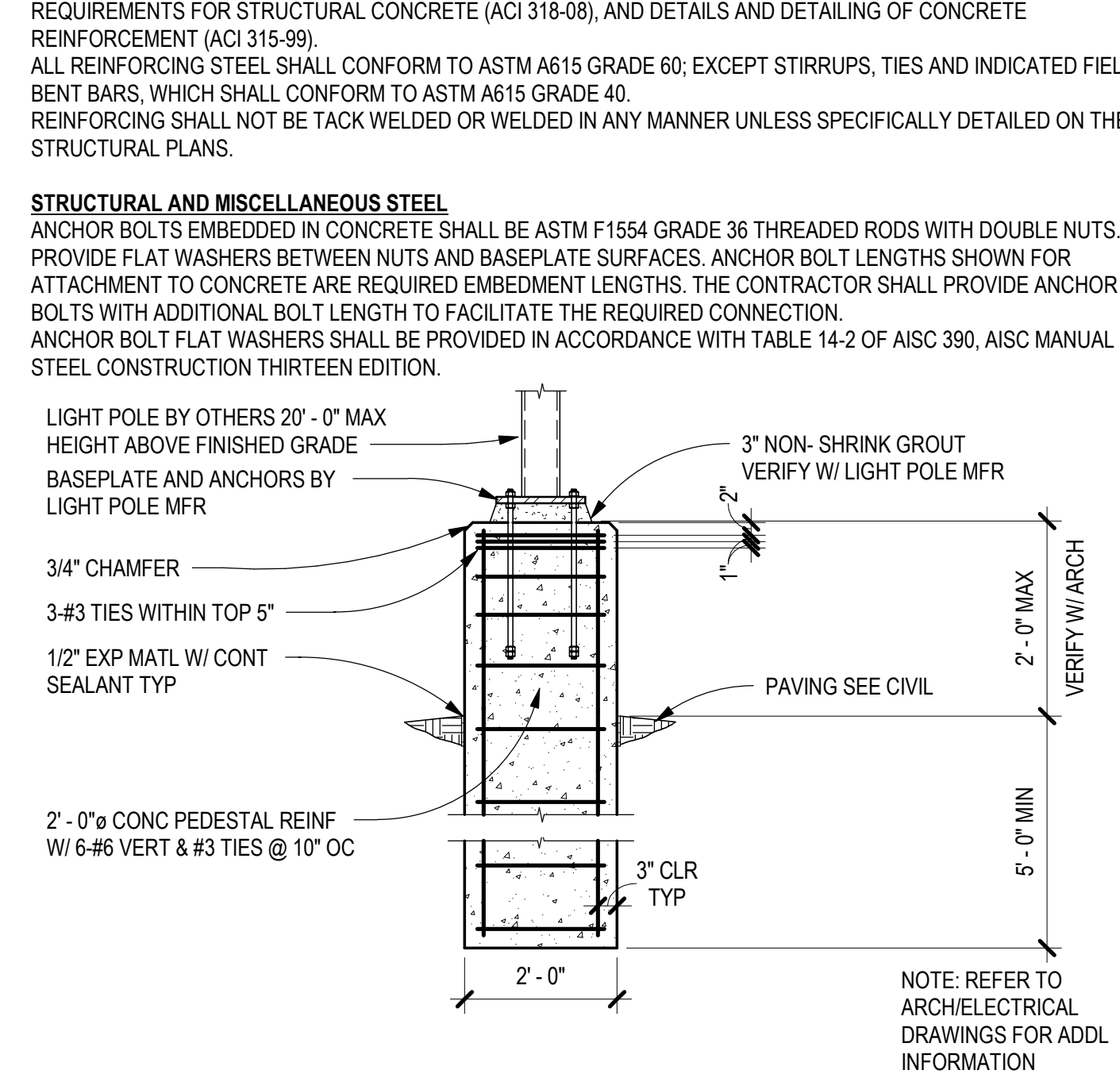
B5 FOUNDATION SECTION NO VENEER
SCALE: 3/4" = 1'-0"



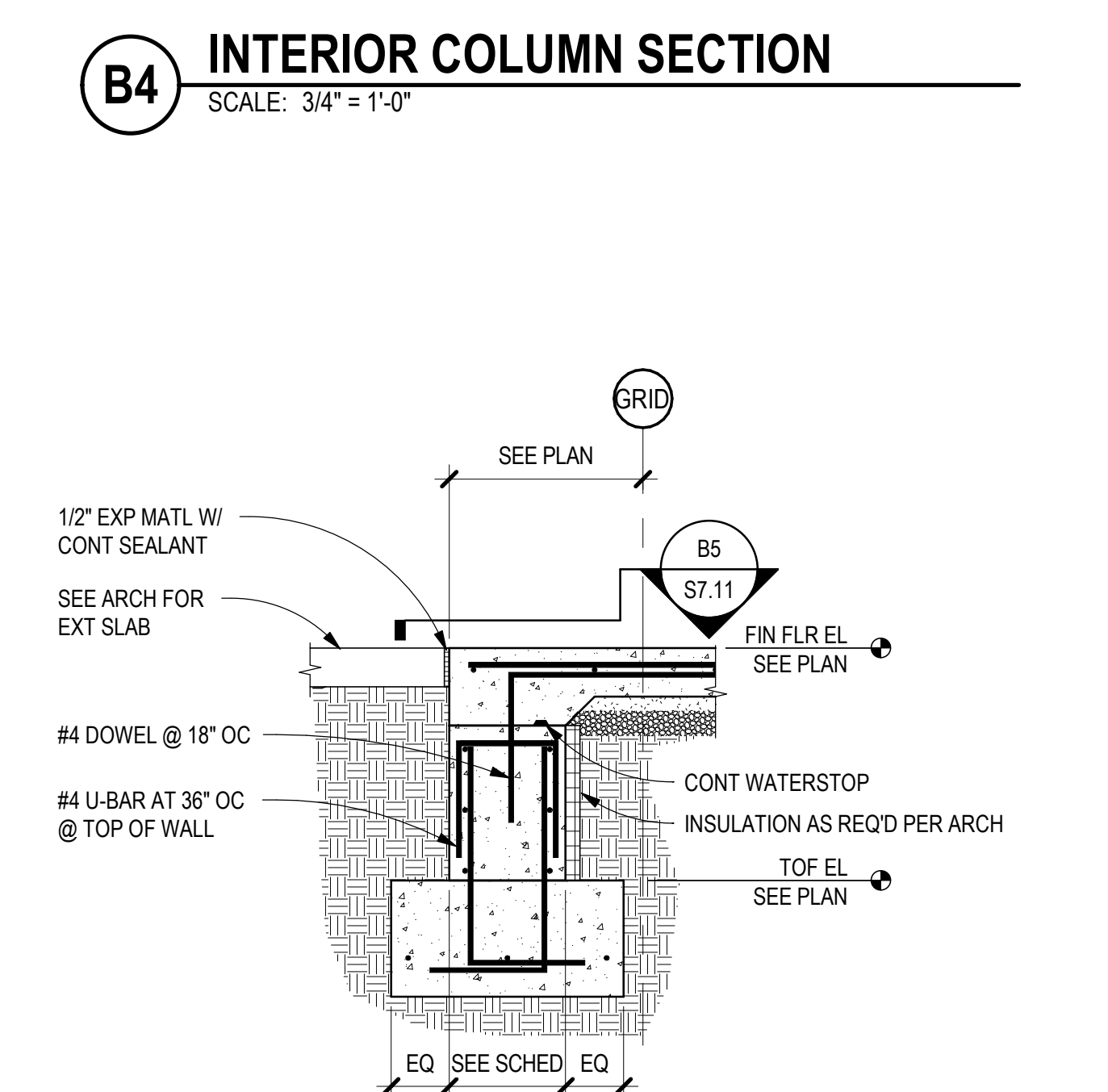
A1 FOUNDATION SECTION @ ELEVATOR
SCALE: 3/4" = 1'-0"



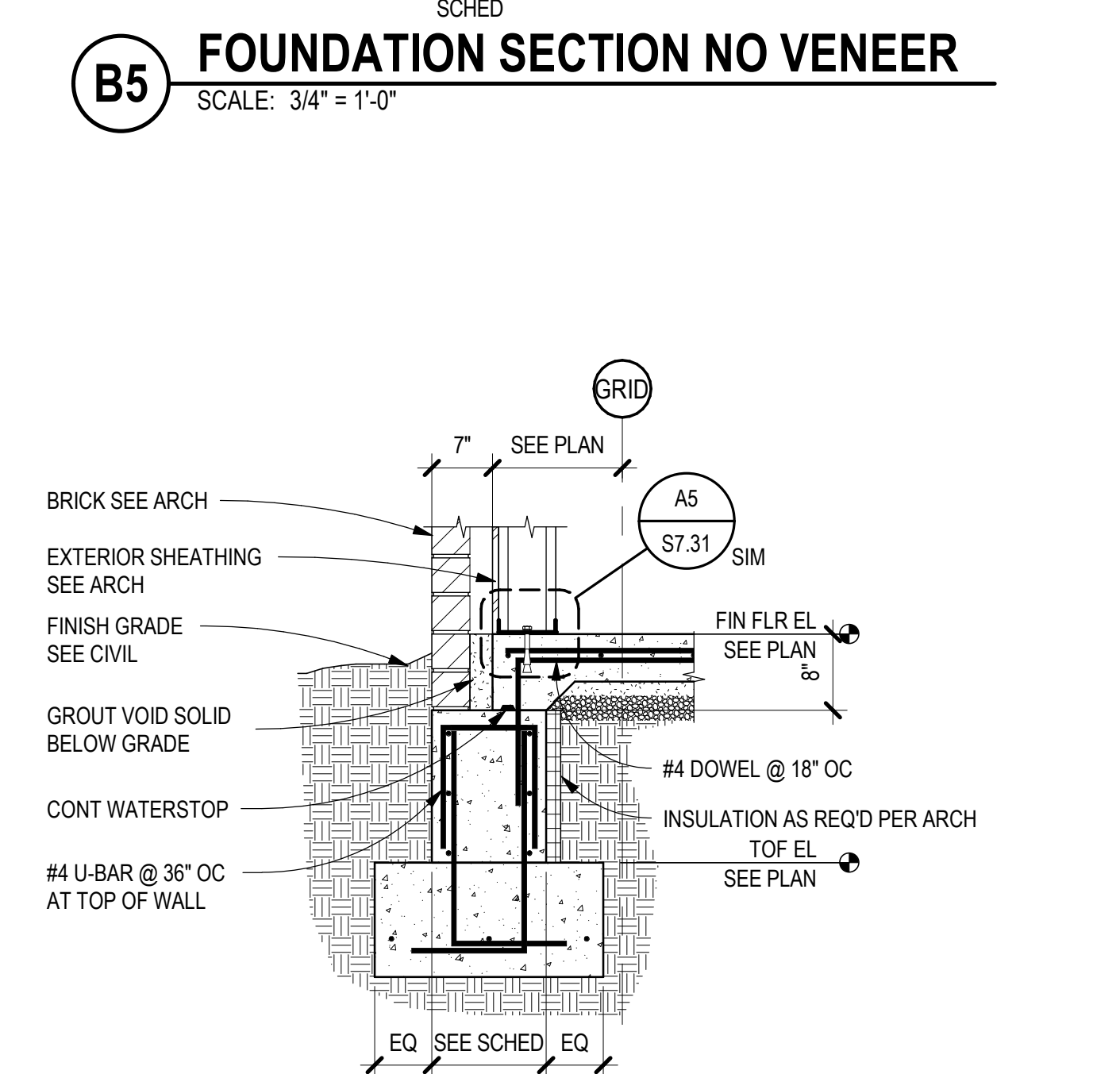
A2 PERIMETER FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



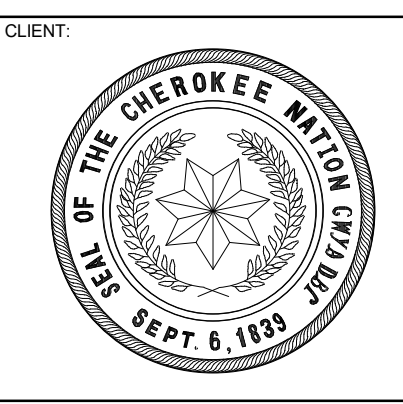
A3 LIGHT POLE BASE
SCALE: 1/2" = 1'-0"



A4 FOUNDATION SECTION AT DOORWAY
SCALE: 3/4" = 1'-0"



A5 TYPICAL FOUNDATION SECTION
SCALE: 3/4" = 1'-0"



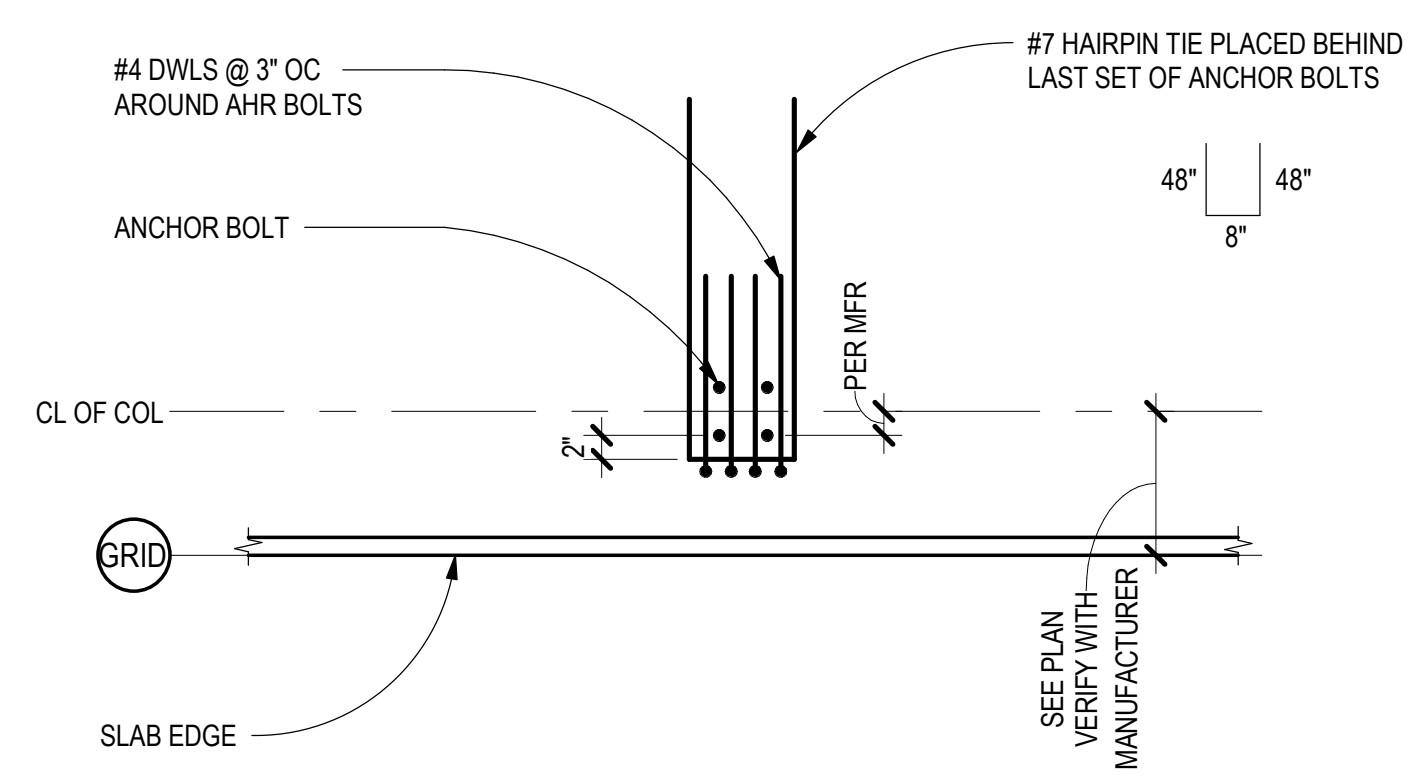
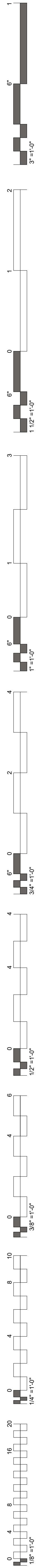
KEY PLAN:

PROJECT PHASE:
BID PACKAGE 01

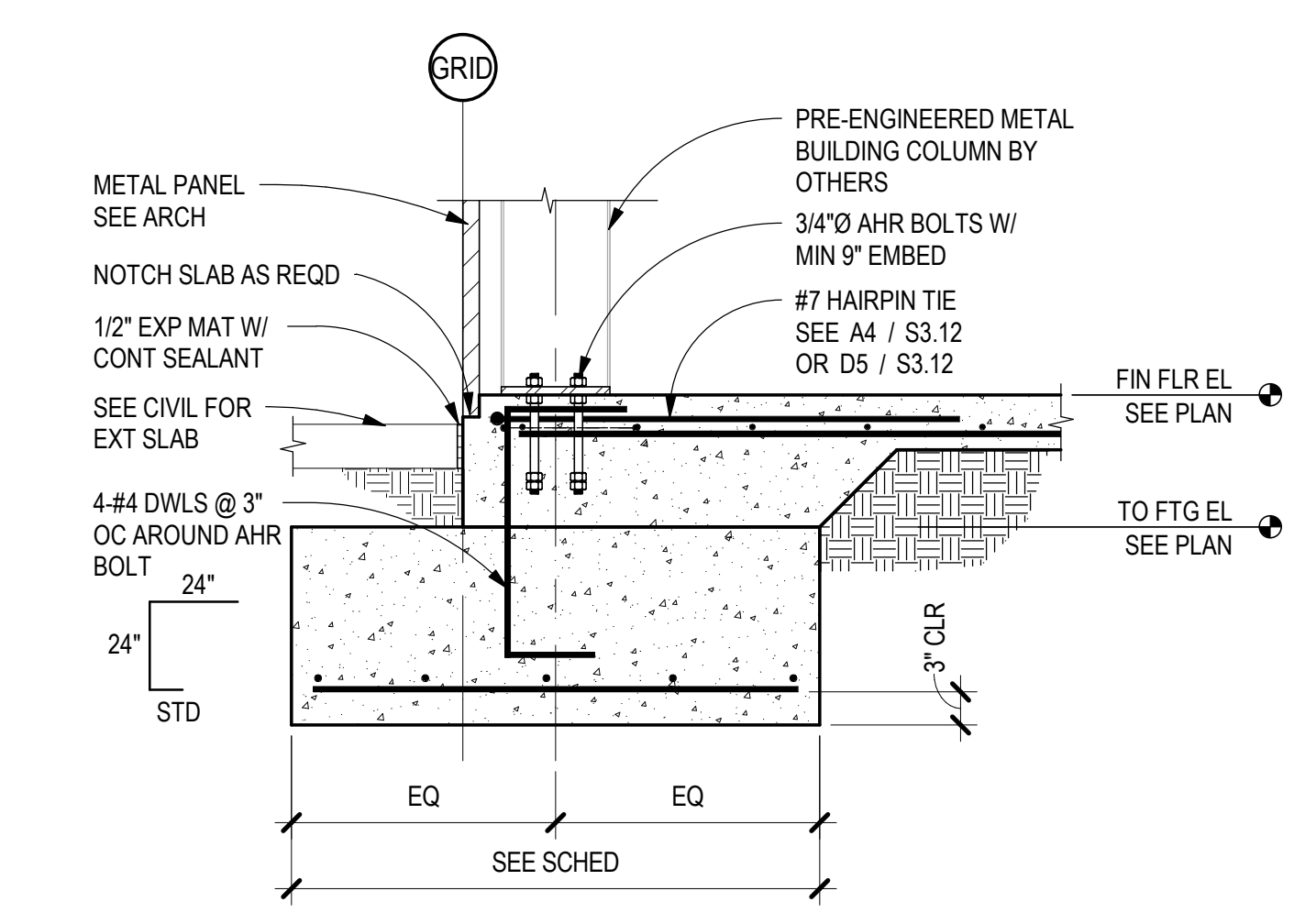
#	DATE	REVISIONS	DESCRIPTION

DATE: 11-01-19 JOB NUMBER: 18-01.01

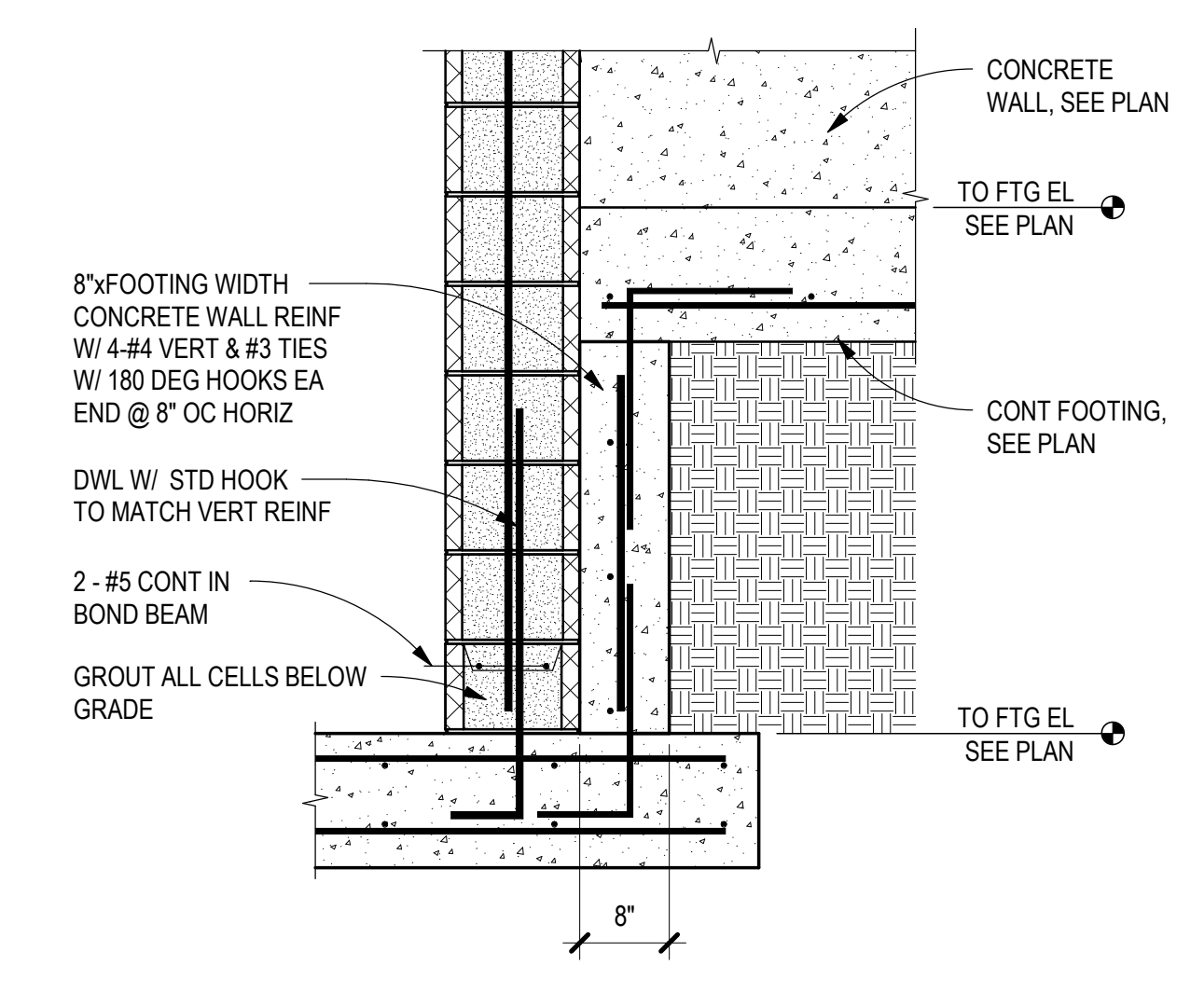
SHEET NUMBER: S3.11



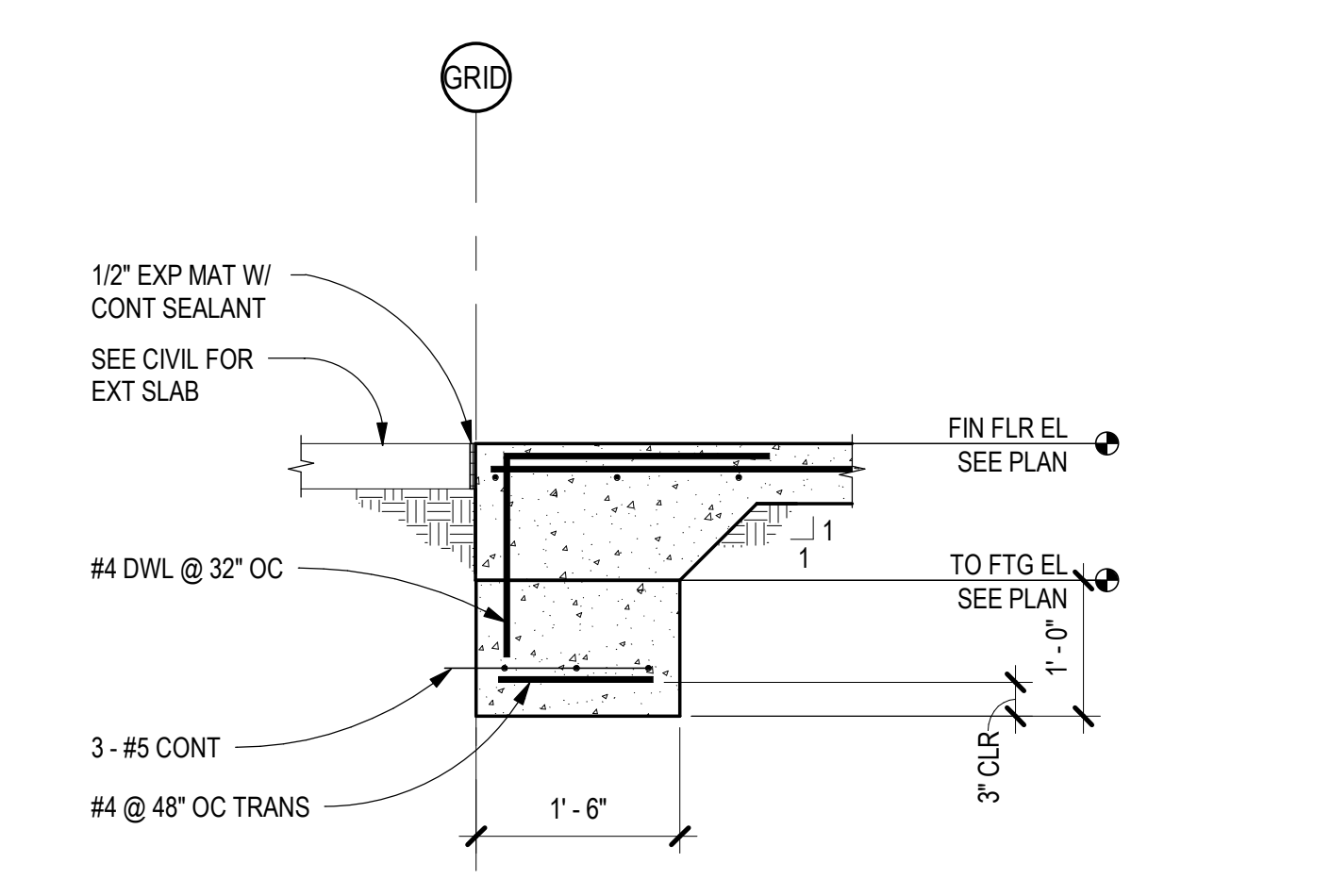
D5 HAIRPIN REINF AT COLUMN
SCALE: 3/4" = 1'-0"



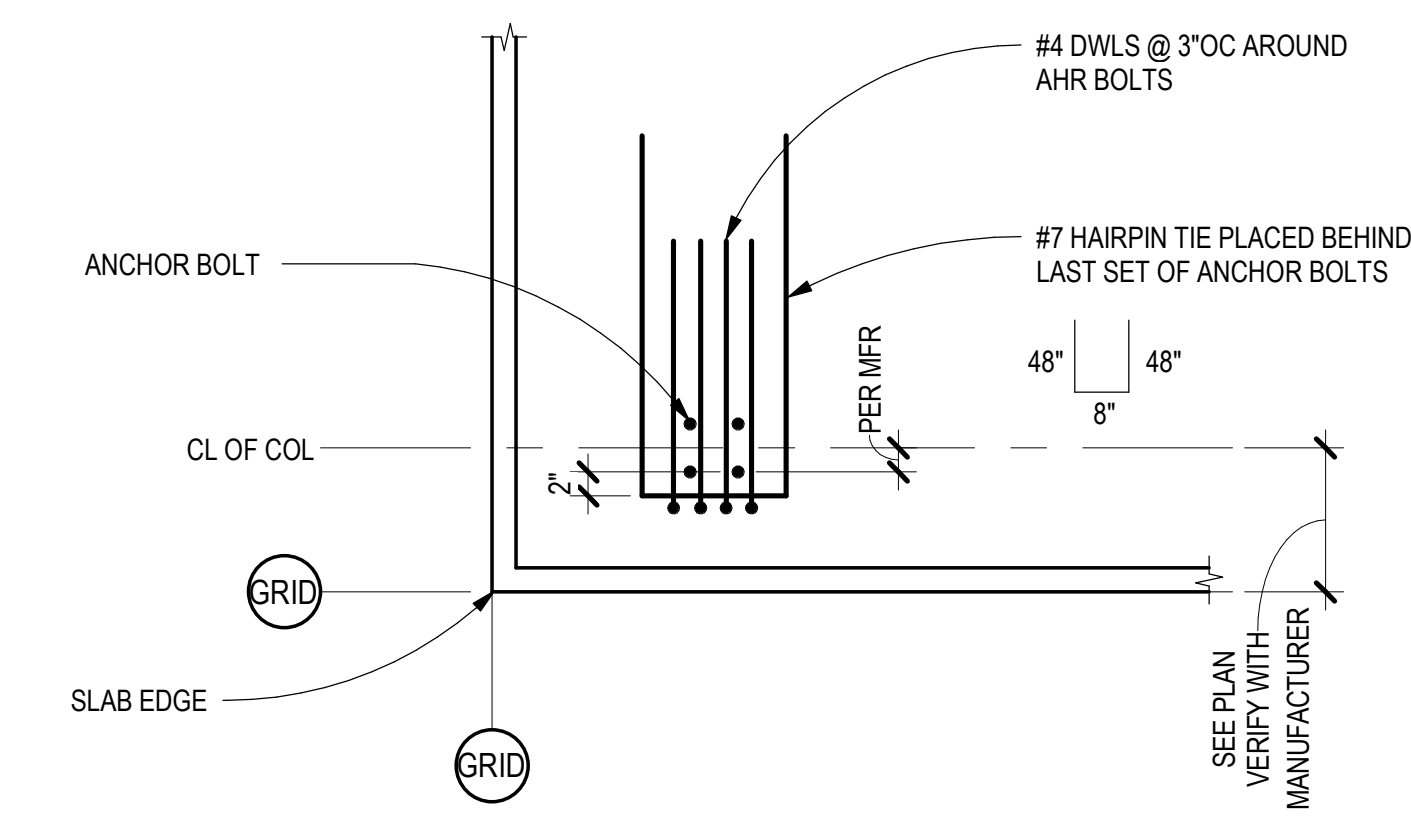
C5 PERIMETER SECTION @ COLUMN
SCALE: 3/4" = 1'-0"



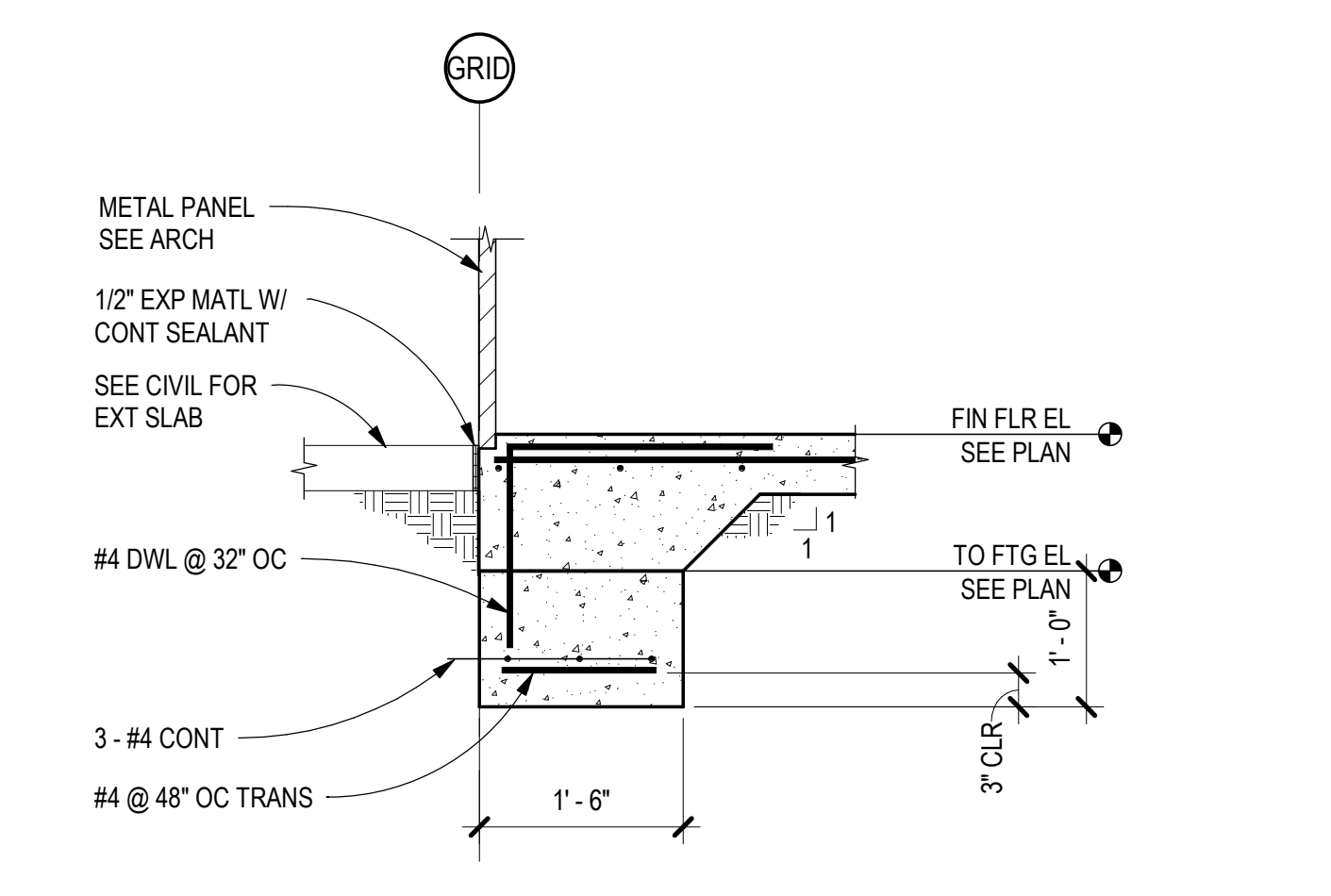
B4 FOUNDATION SECTION @ ELEVATOR
SCALE: 3/4" = 1'-0"



B5 FOUNDATION SECT @ OPENING
SCALE: 3/4" = 1'-0"

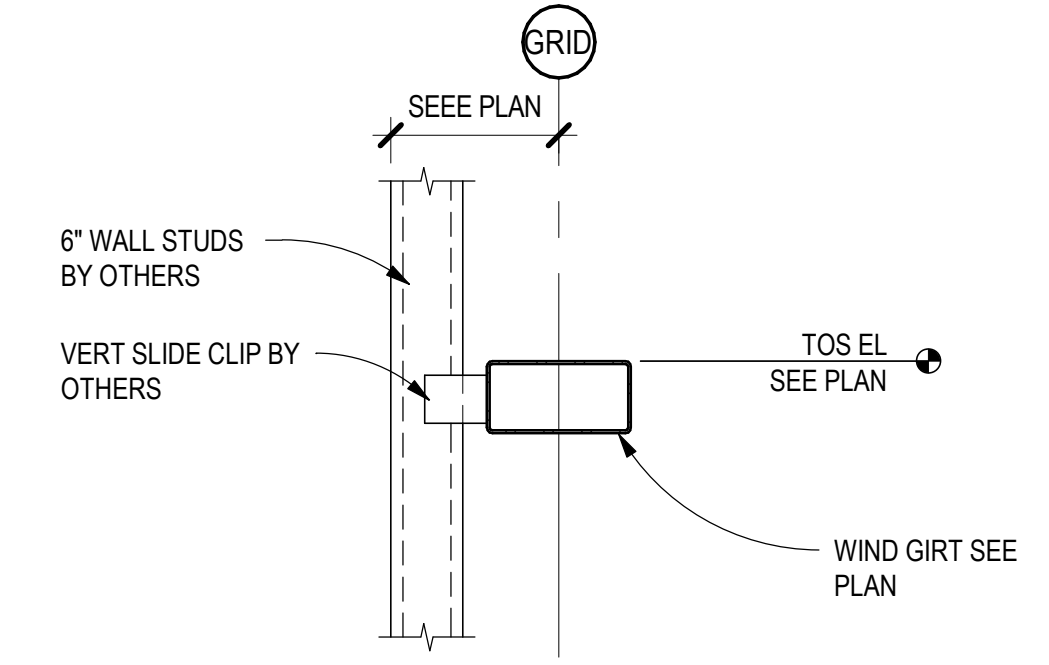
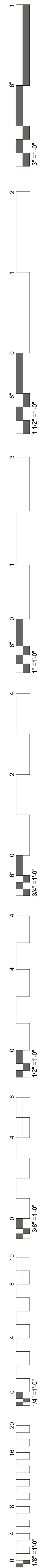


A4 HAIRPIN REINF AT CORNER COLUMN
SCALE: 3/4" = 1'-0"

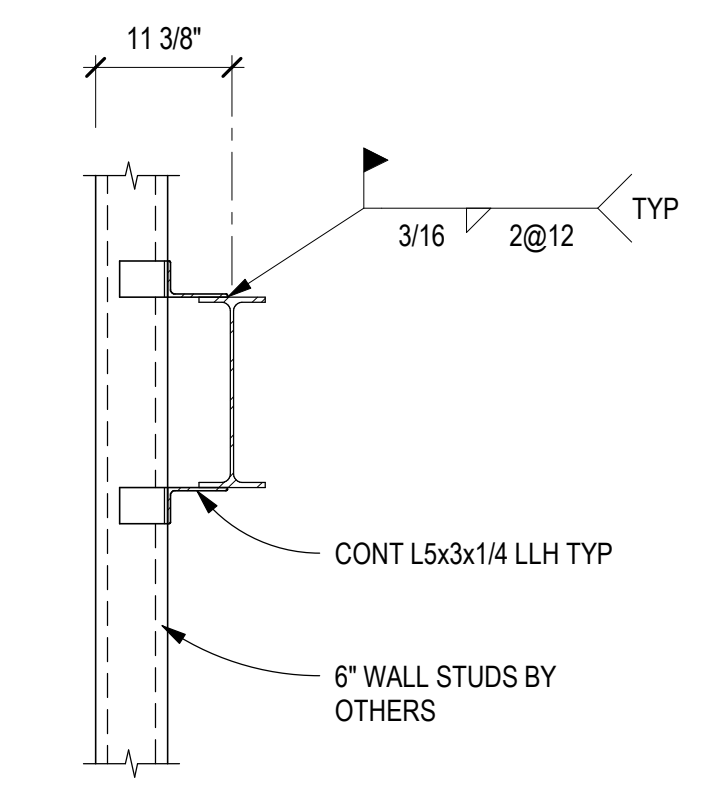


A5 PERIMETER FOUNDATION SECTION
SCALE: 3/4" = 1'-0"

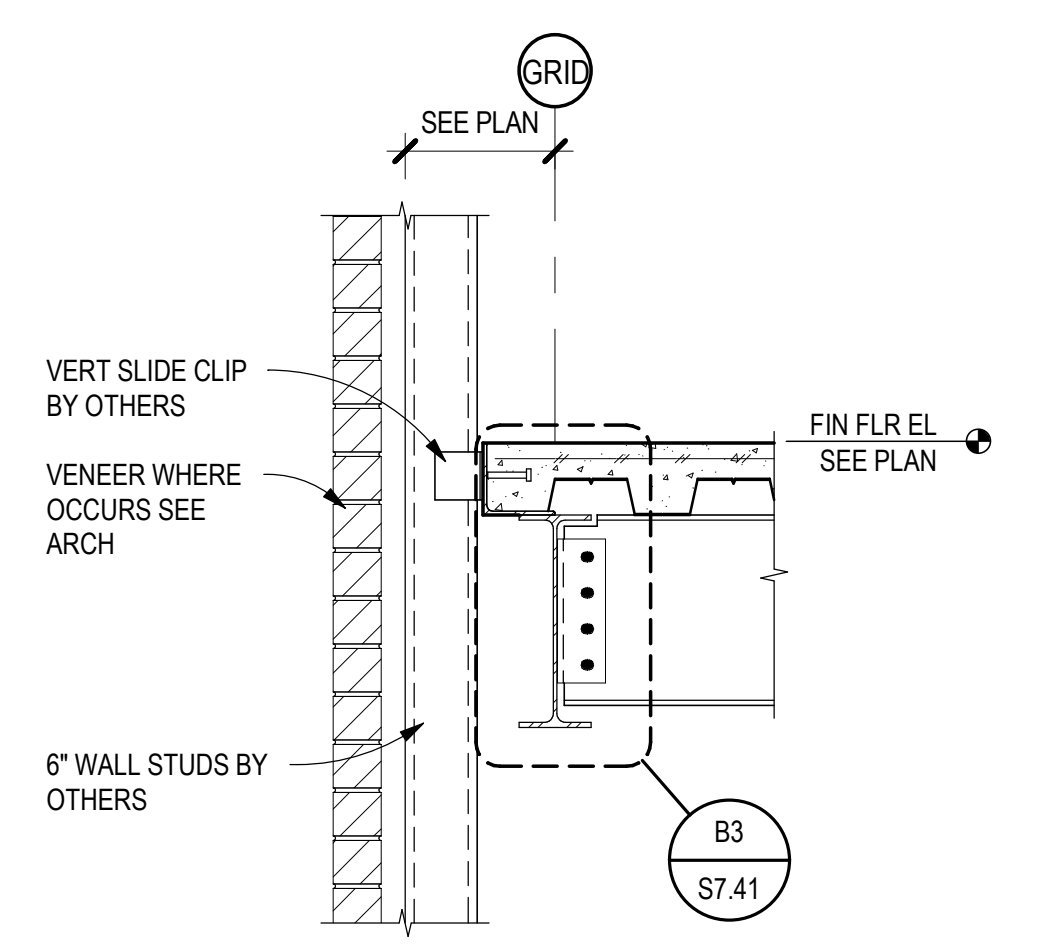
#	DATE	REVISIONS	DESCRIPTION



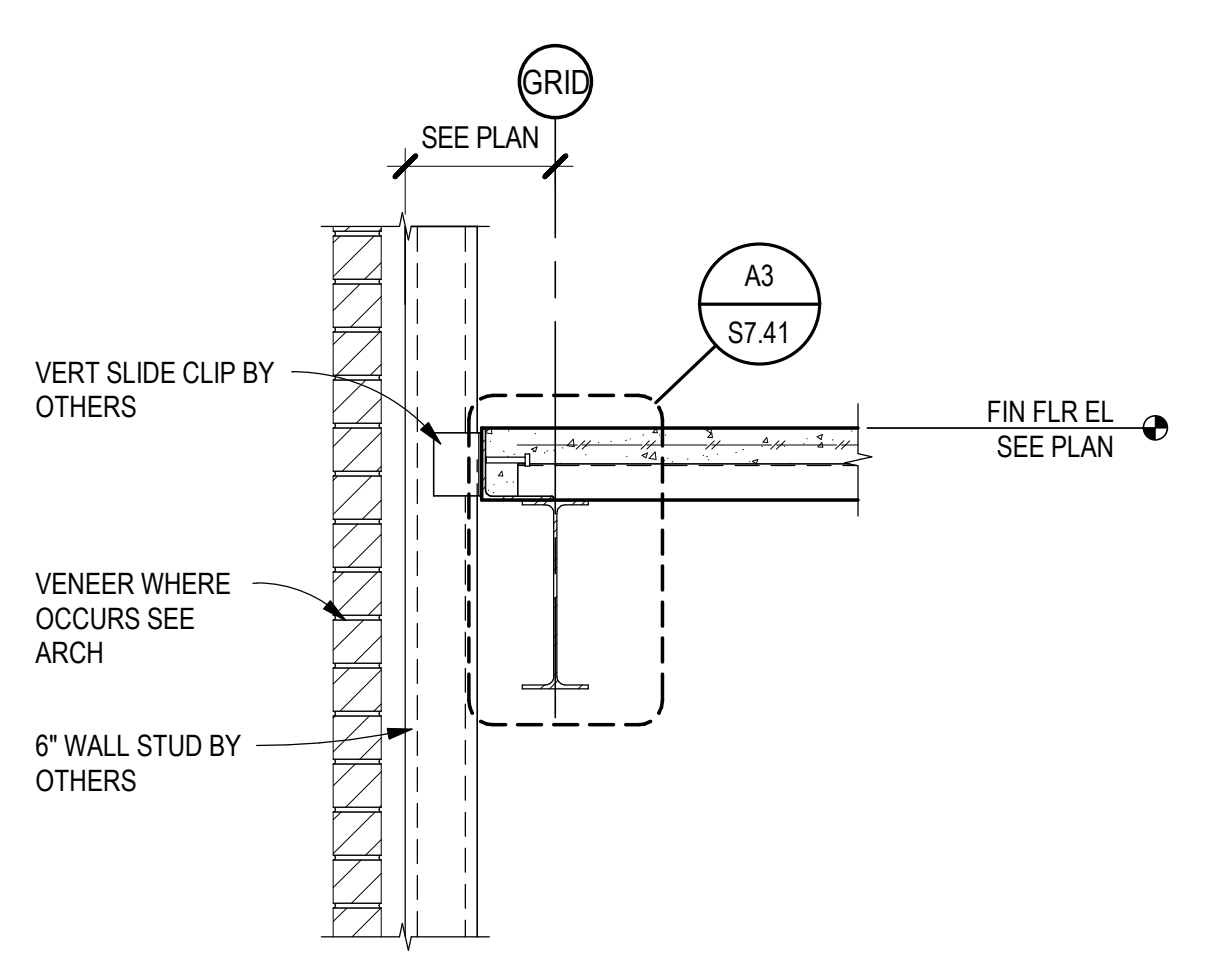
D5 FRAMING SECTION
SCALE: 3/4" = 1'-0"



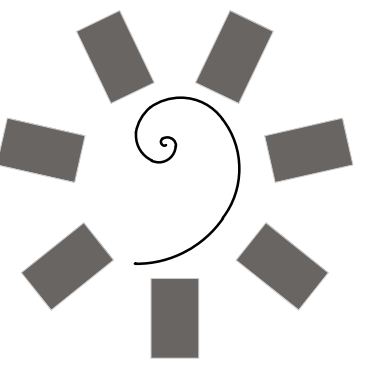
C5 FRAMING SECTION
SCALE: 3/4" = 1'-0"



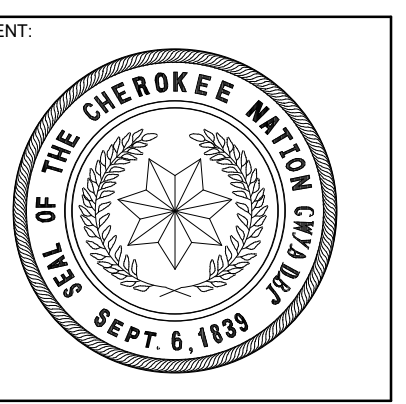
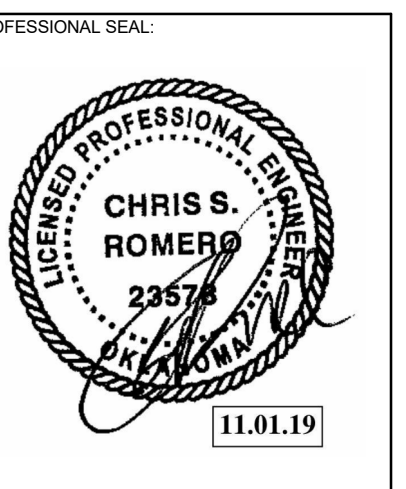
B5 FLOOR FRAMING SECTION
SCALE: 3/4" = 1'-0"



A5 FLOOR FRAMING SECTION
SCALE: 3/4" = 1'-0"



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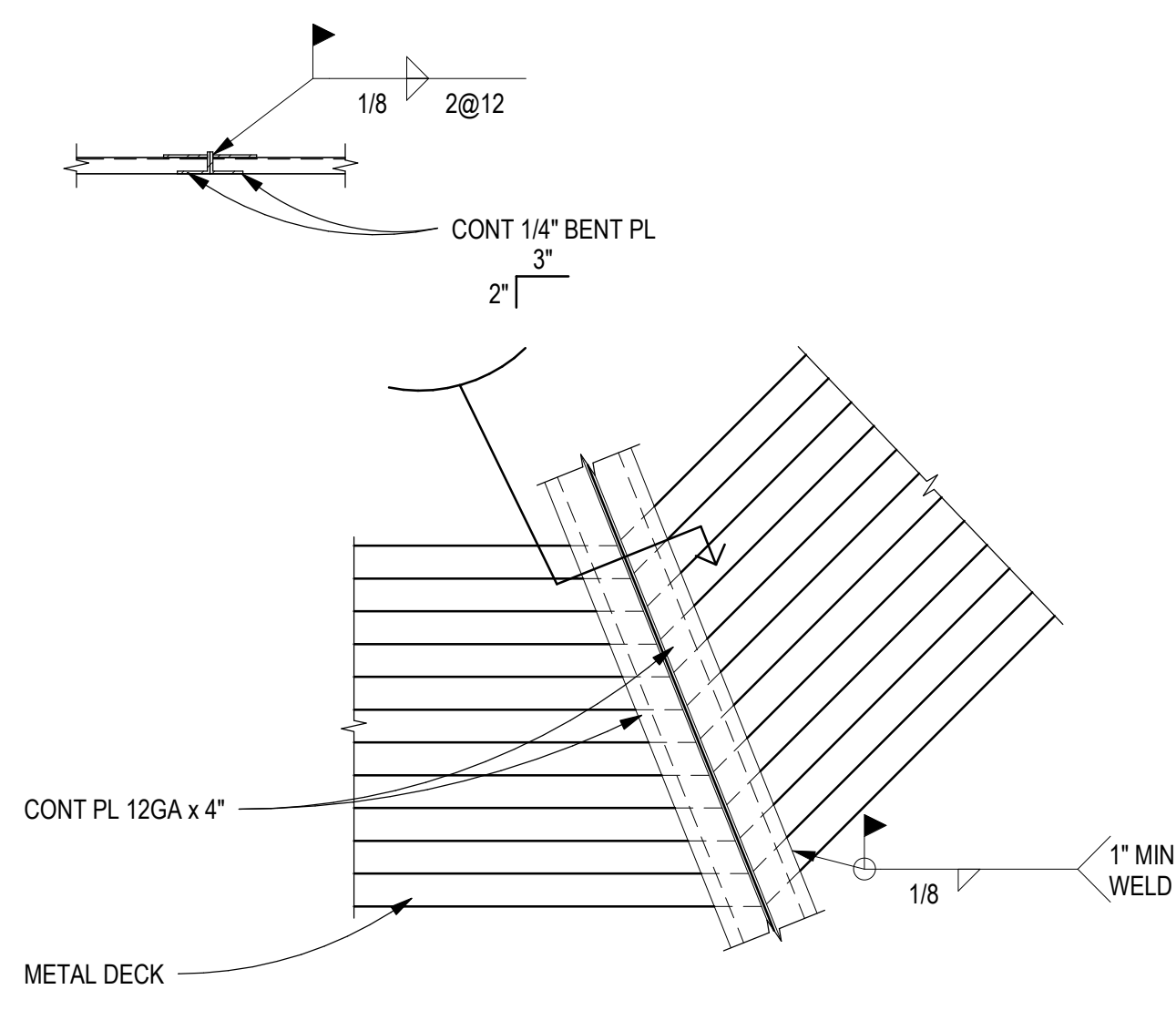
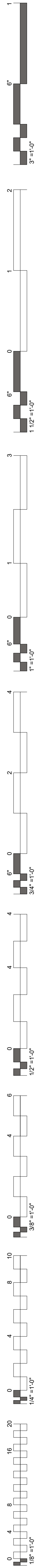
PROJECT PHASE:
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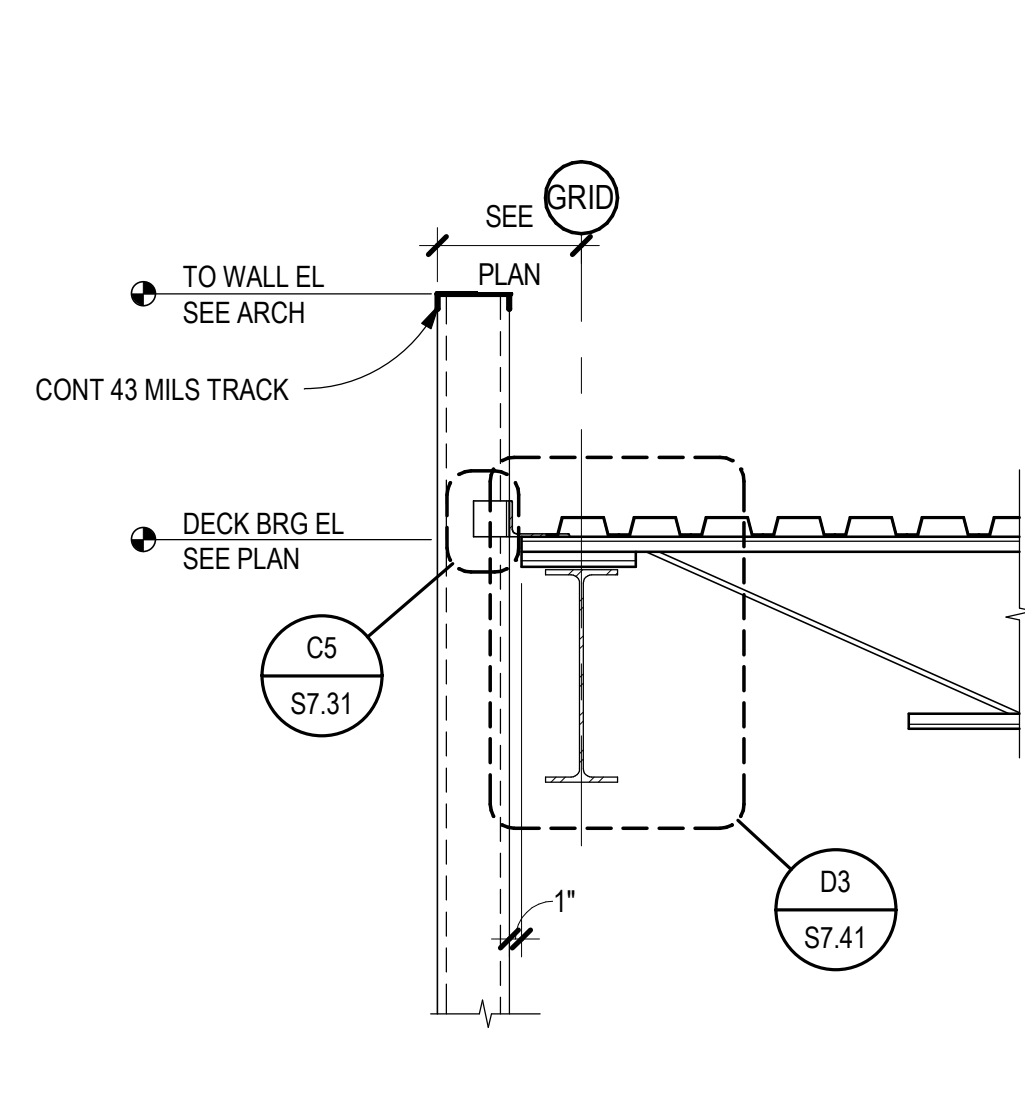
DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER:
S3.21

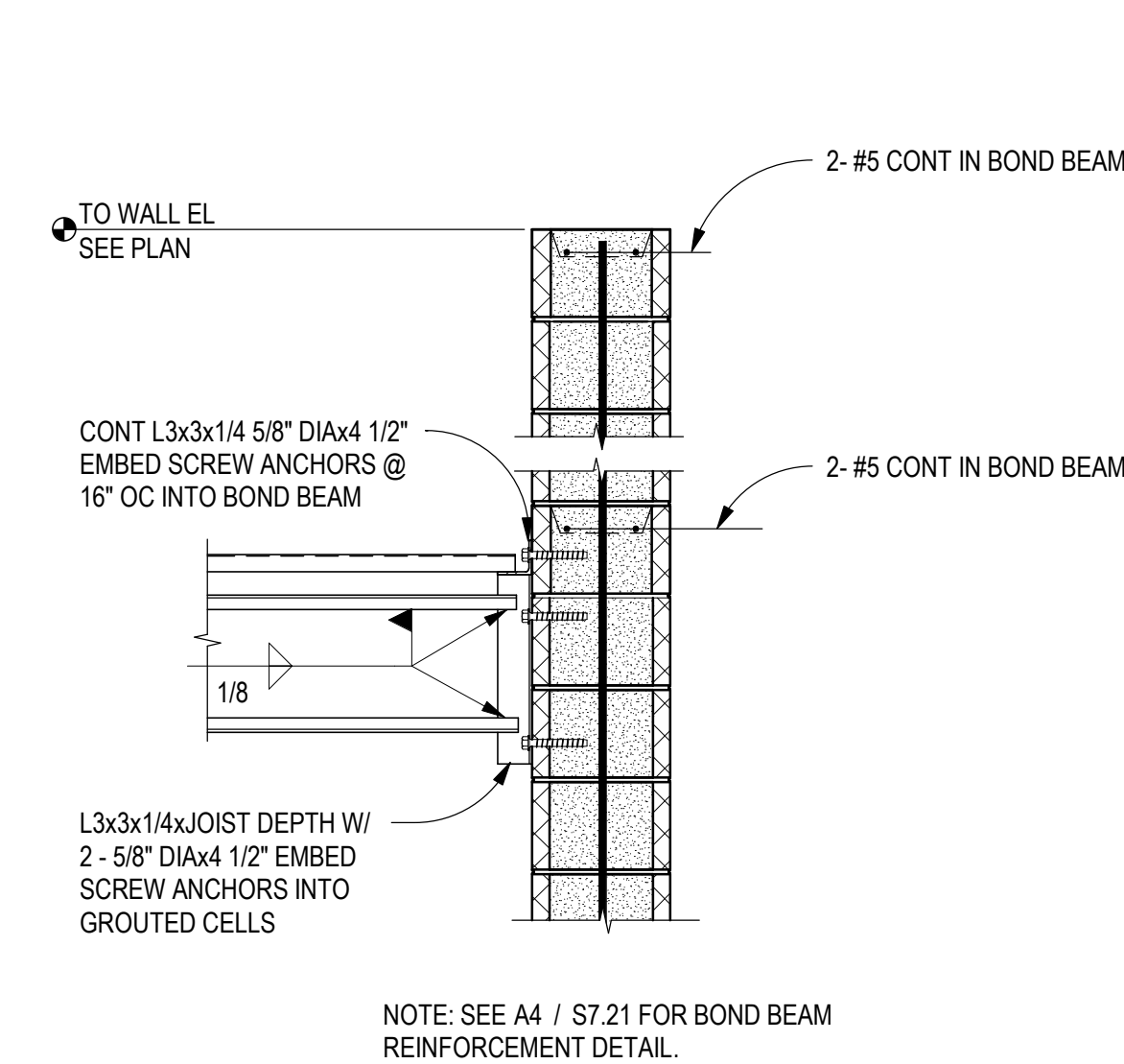
FLOOR FRAMING
SECTIONS



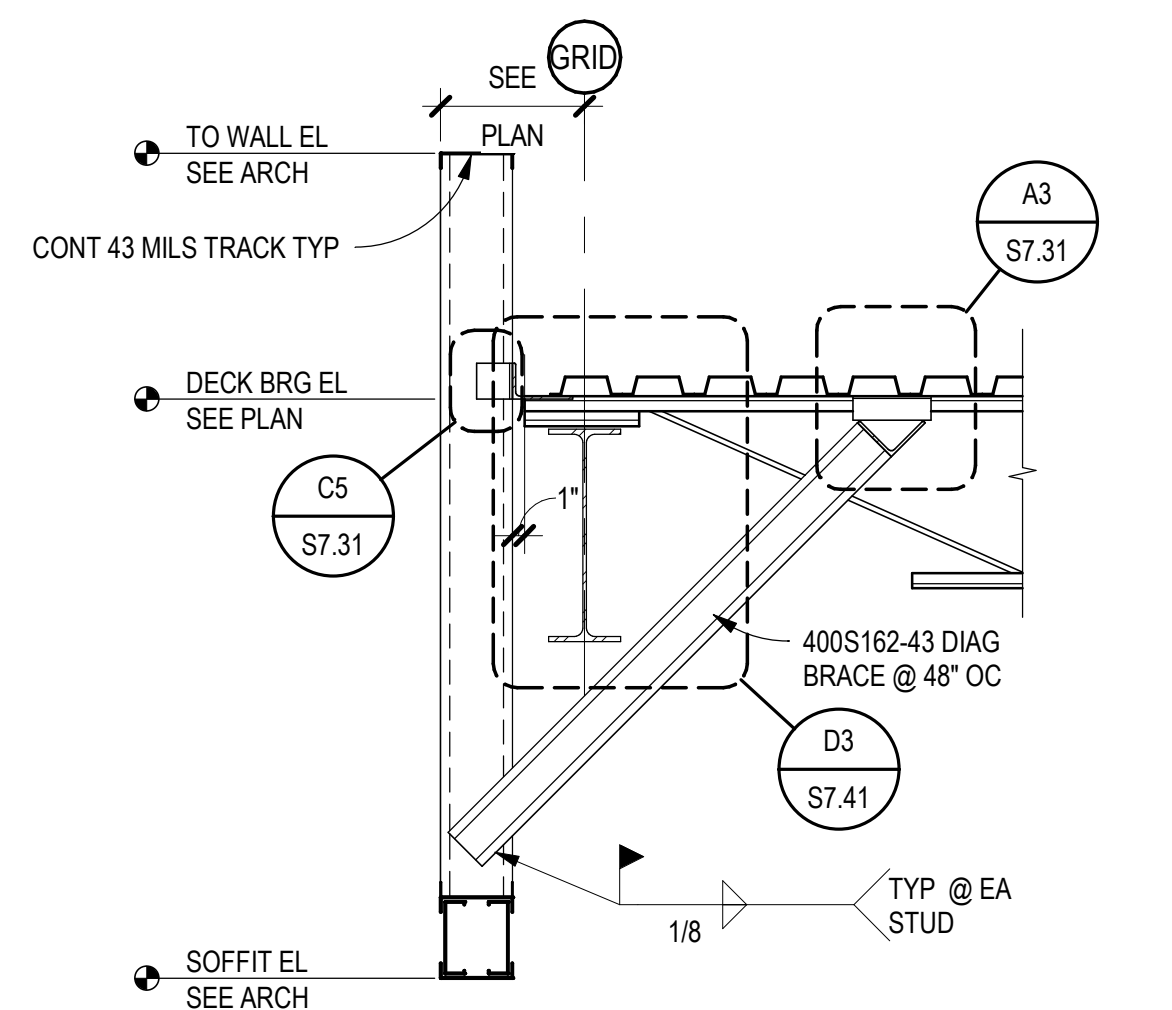
D2 ROOF DECK TRANSITION
SCALE: 3/4" = 1'-0"



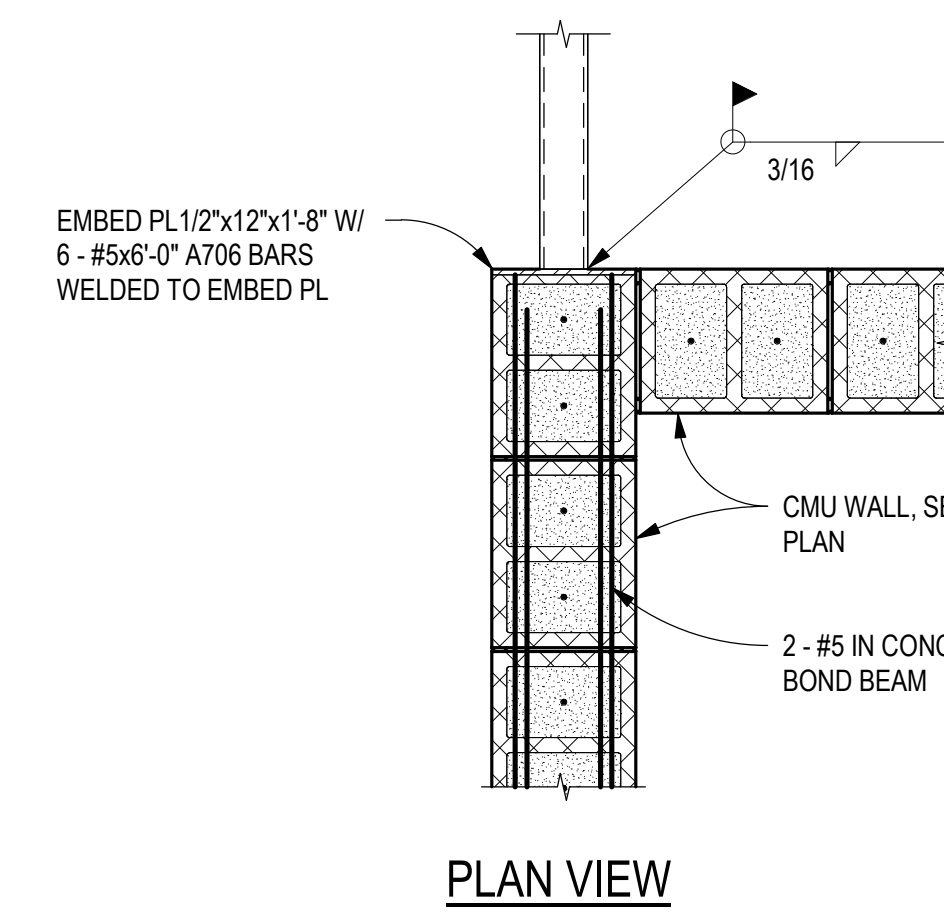
D3 ROOF FRAMING SECTION
SCALE: 3/4" = 1'-0"



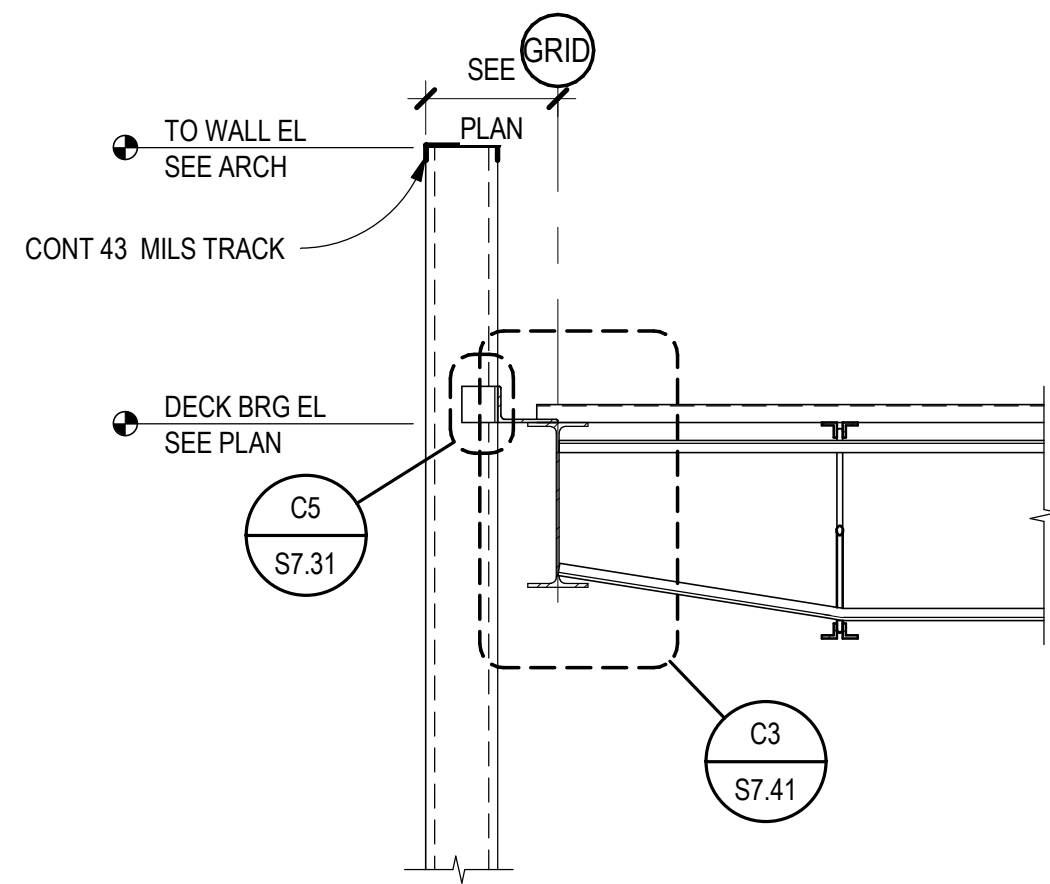
D4 ROOF NON-BEARING AT CMU WALL
SCALE: 3/4" = 1'-0"



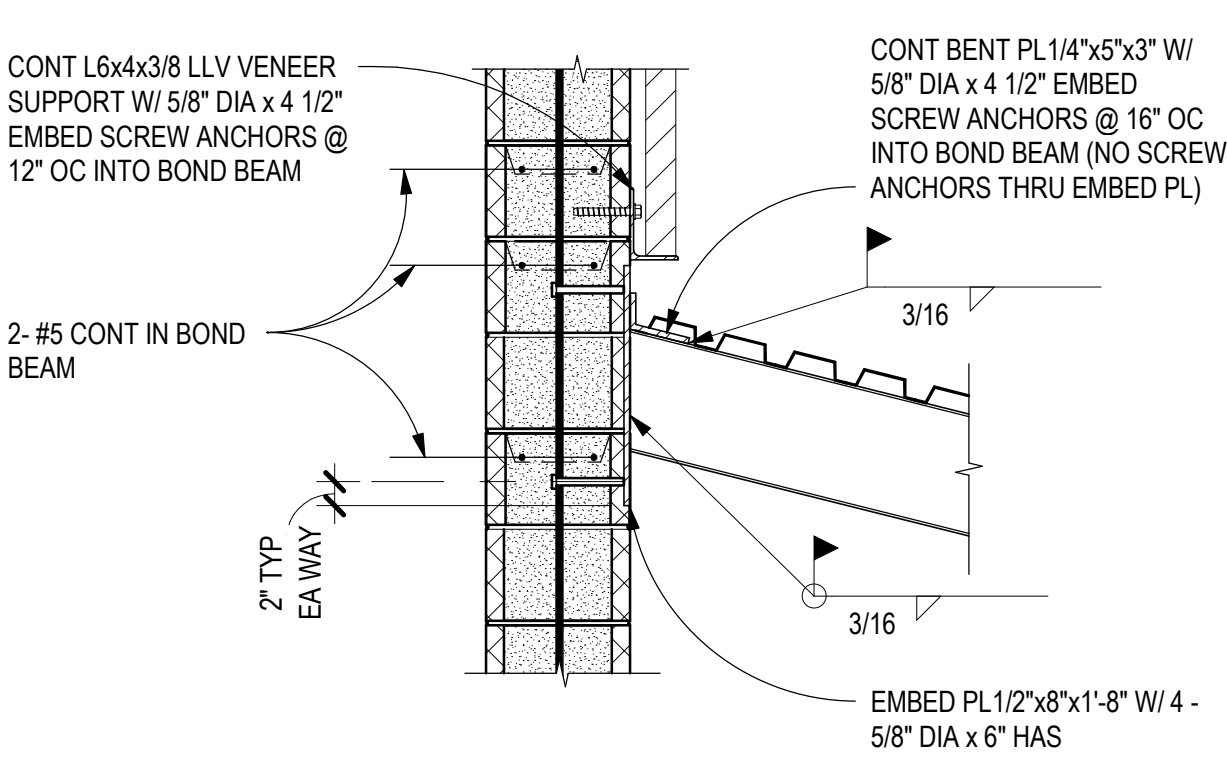
C2 ROOF FRAMING SECTION
SCALE: 3/4" = 1'-0"



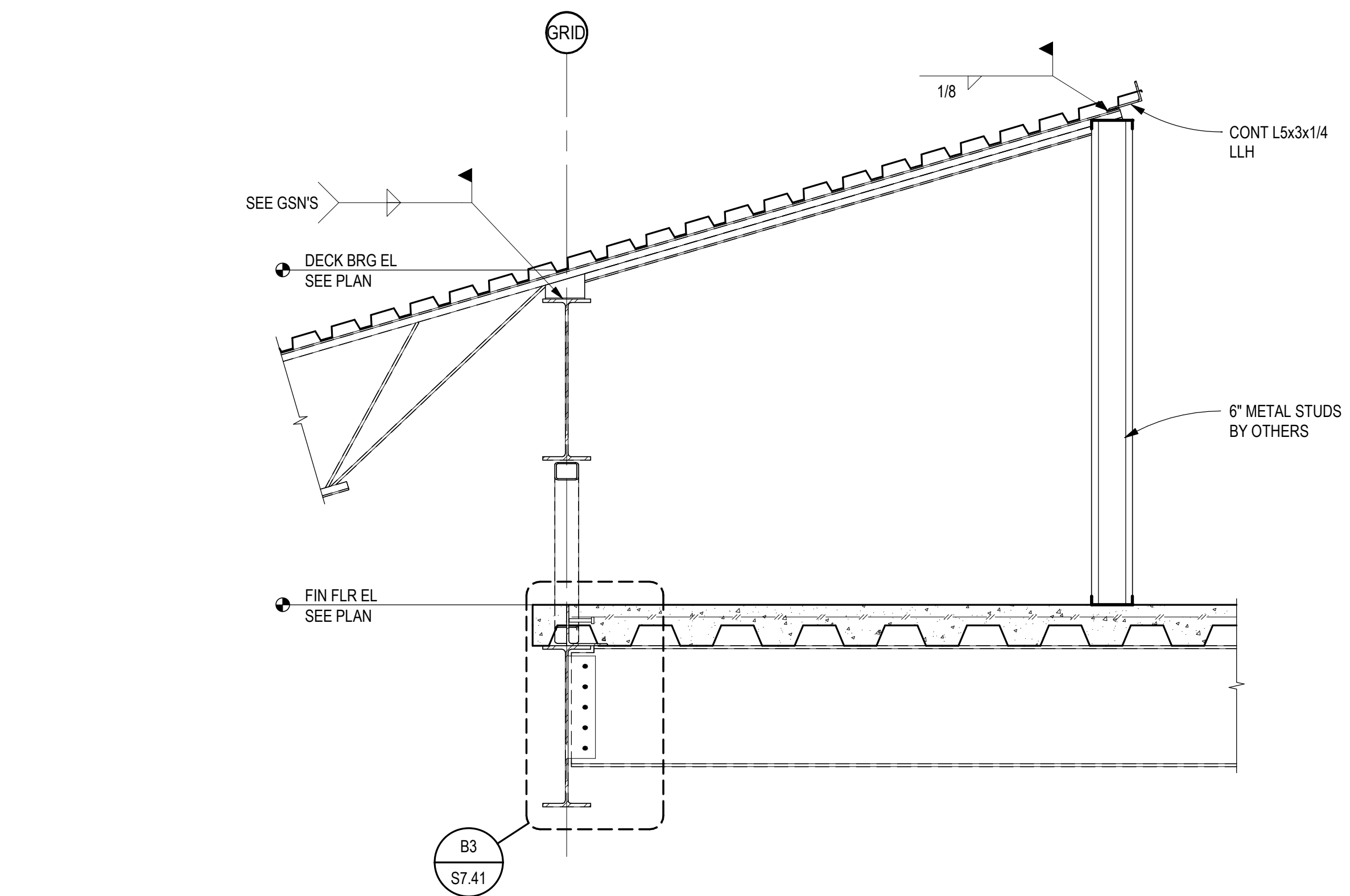
C3 COLLECTOR BLOCKING
SCALE: 3/4" = 1'-0"



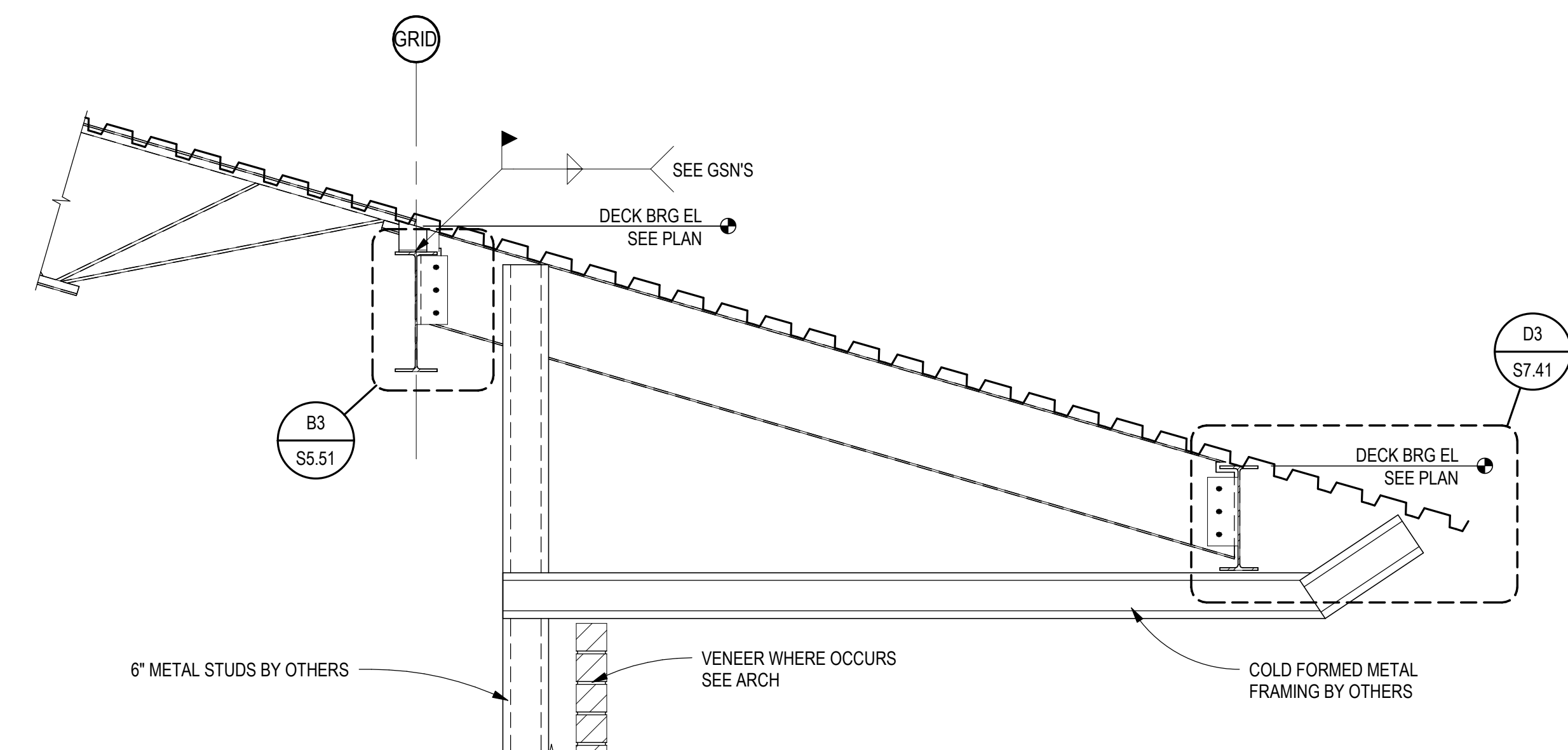
B2 ROOF FRAMING SECTION
SCALE: 3/4" = 1'-0"



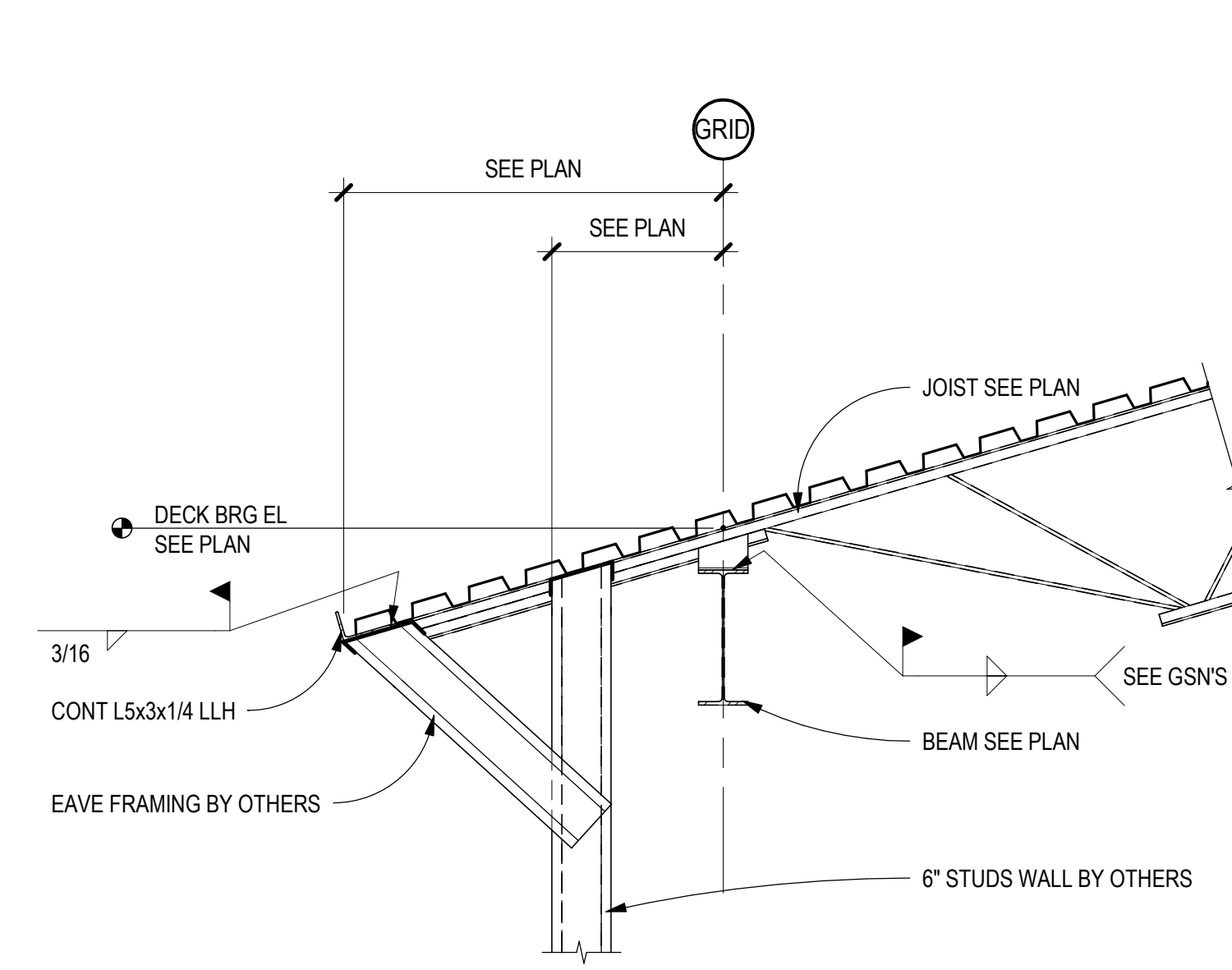
B3 FRAMING SECTION
SCALE: 3/4" = 1'-0"



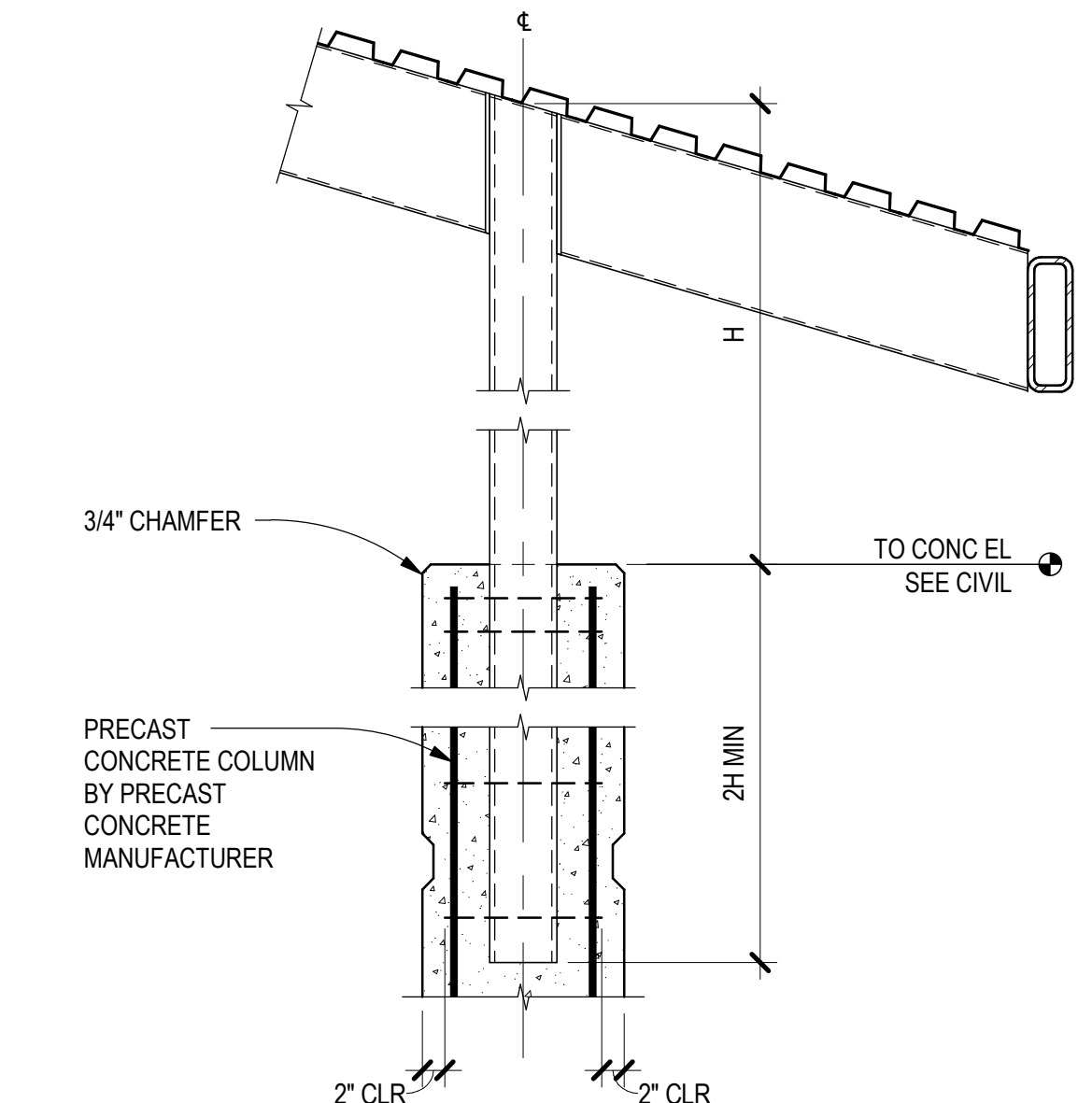
B4 ROOF FRAMING SECTION
SCALE: 3/4" = 1'-0"



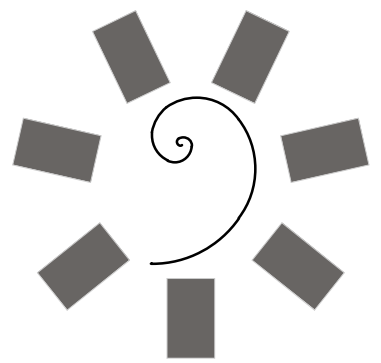
A2 ROOF FRAMING SECTION
SCALE: 3/4" = 1'-0"



A4 TYPICAL ROOF FRAMING SECTION
SCALE: 3/4" = 1'-0"



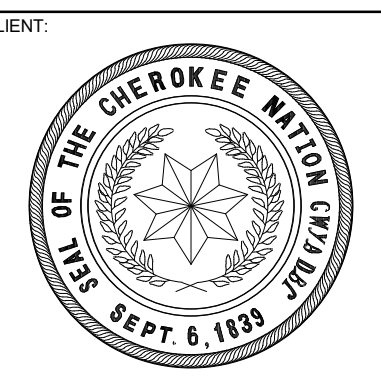
A5 EXT CONC COLUMN SECTION
SCALE: 3/4" = 1'-0"



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KEY PLAN

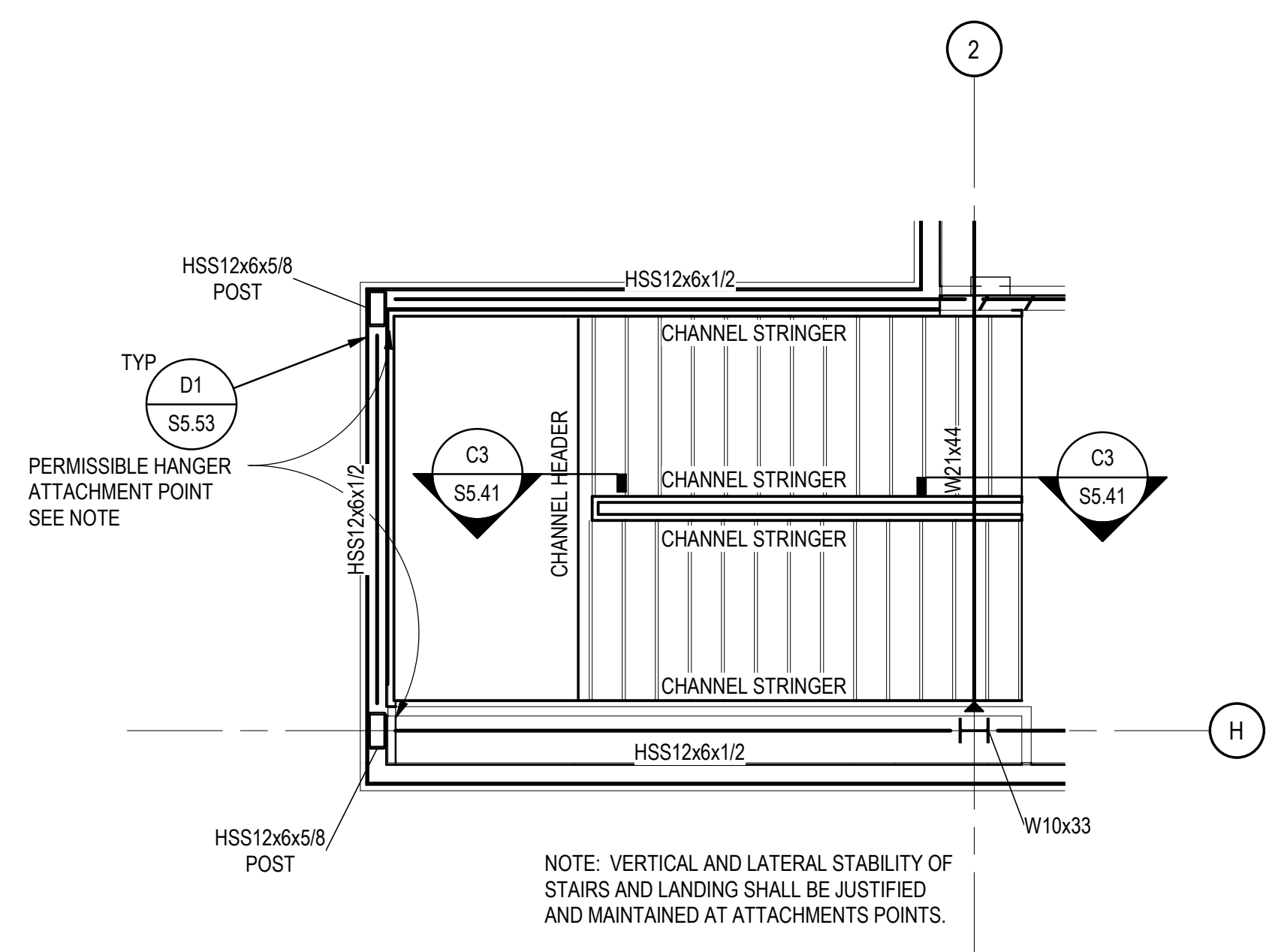
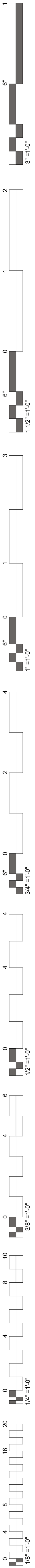
PROJECT PHASE:
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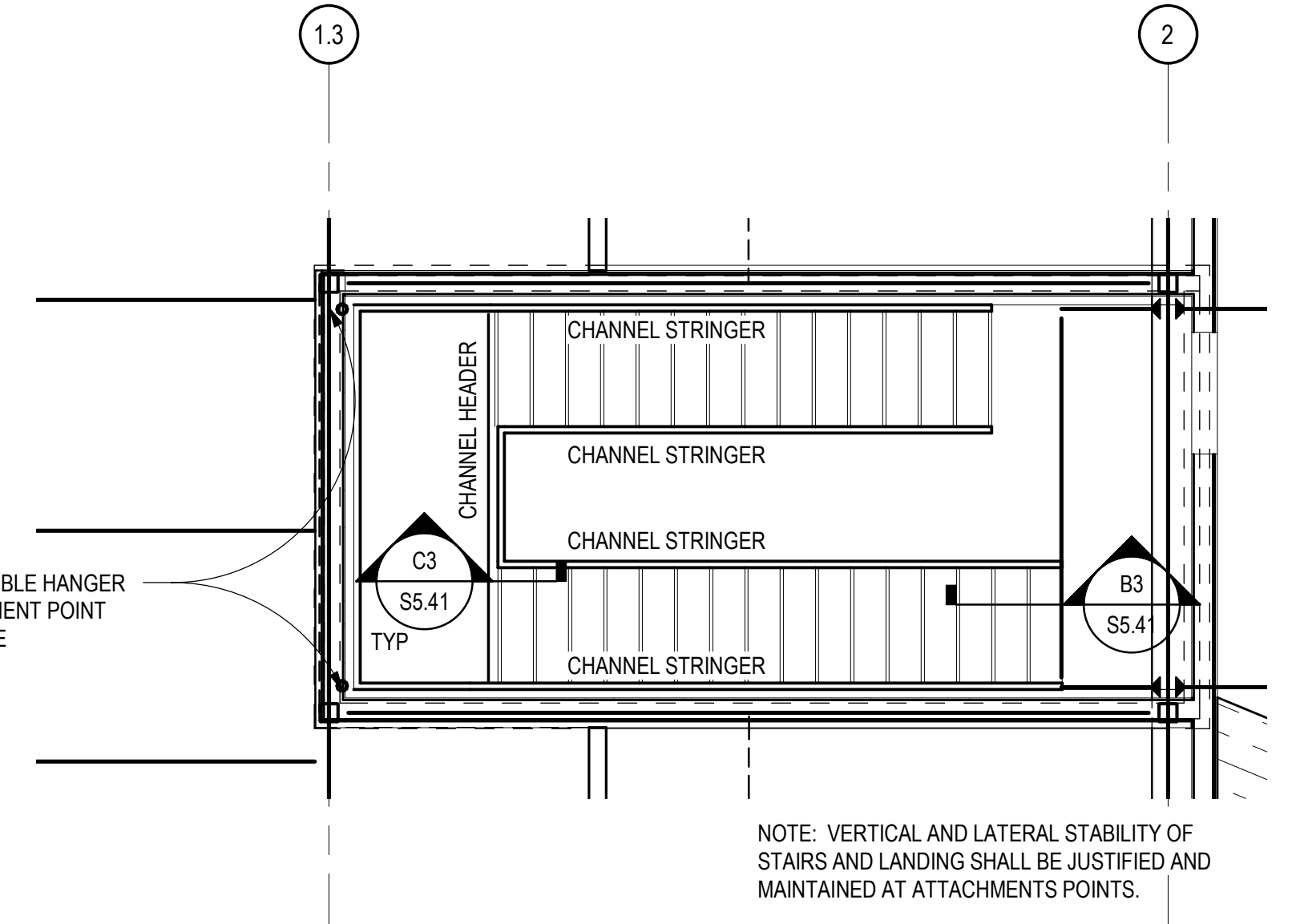
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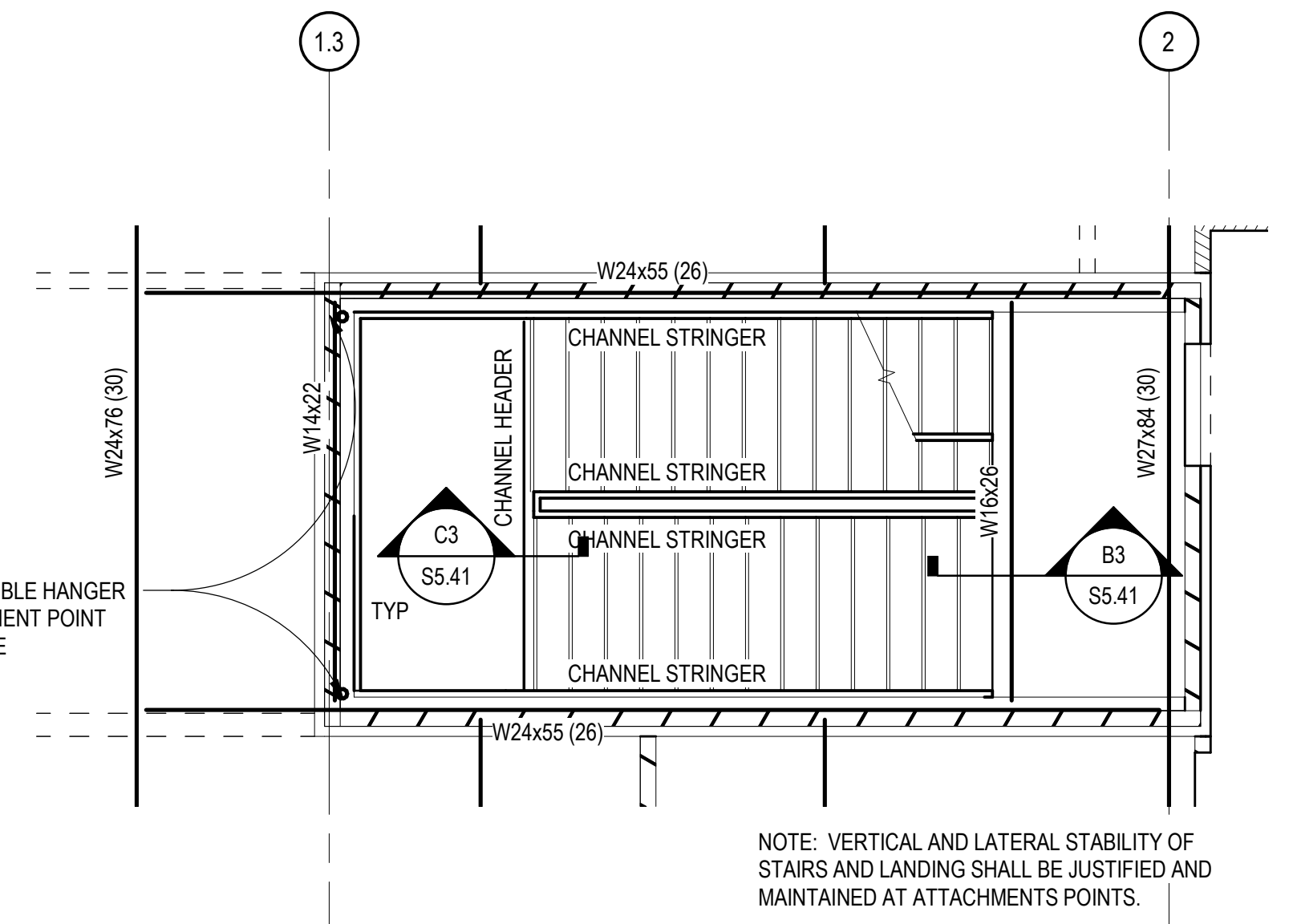
S3.31
ROOF FRAMING SECTIONS



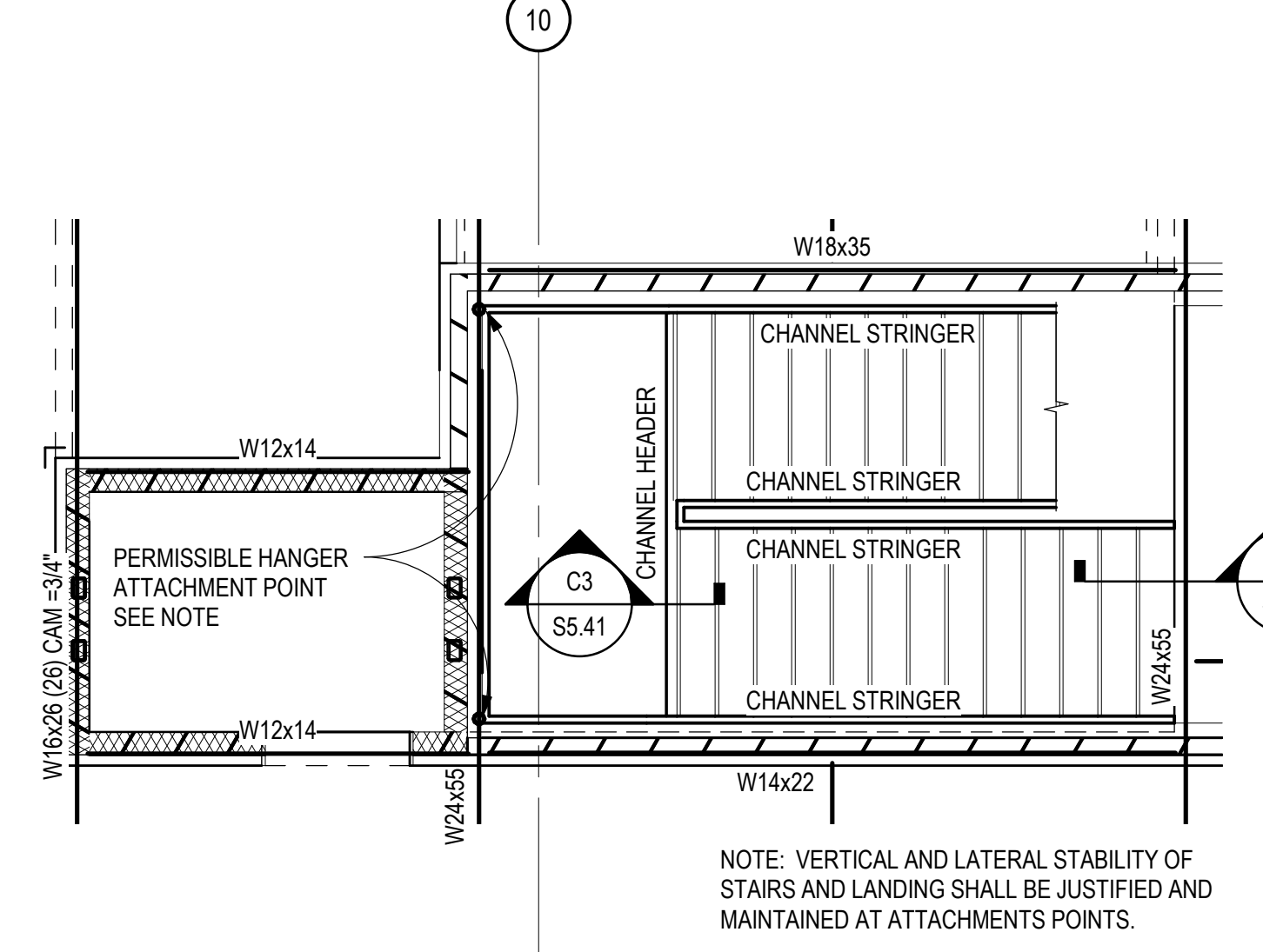
D1 ENLARGED SOUTHWEST STAIR FRAMING PLAN
SCALE: 1/4" = 1'-0"



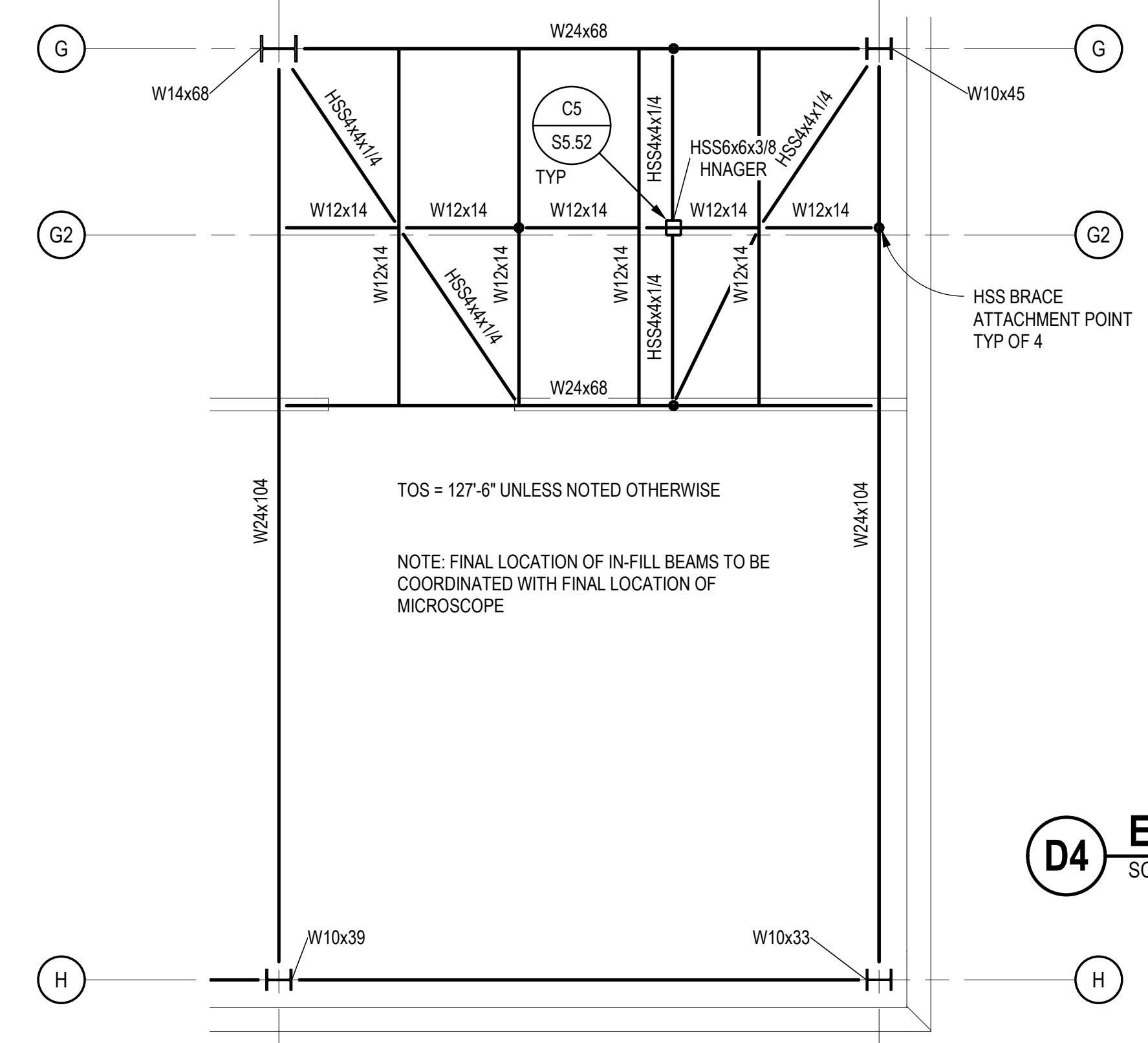
C1 ENLARGED WEST STAIR FRAMING PLAN TO MEZZANINE
SCALE: 1/4" = 1'-0"



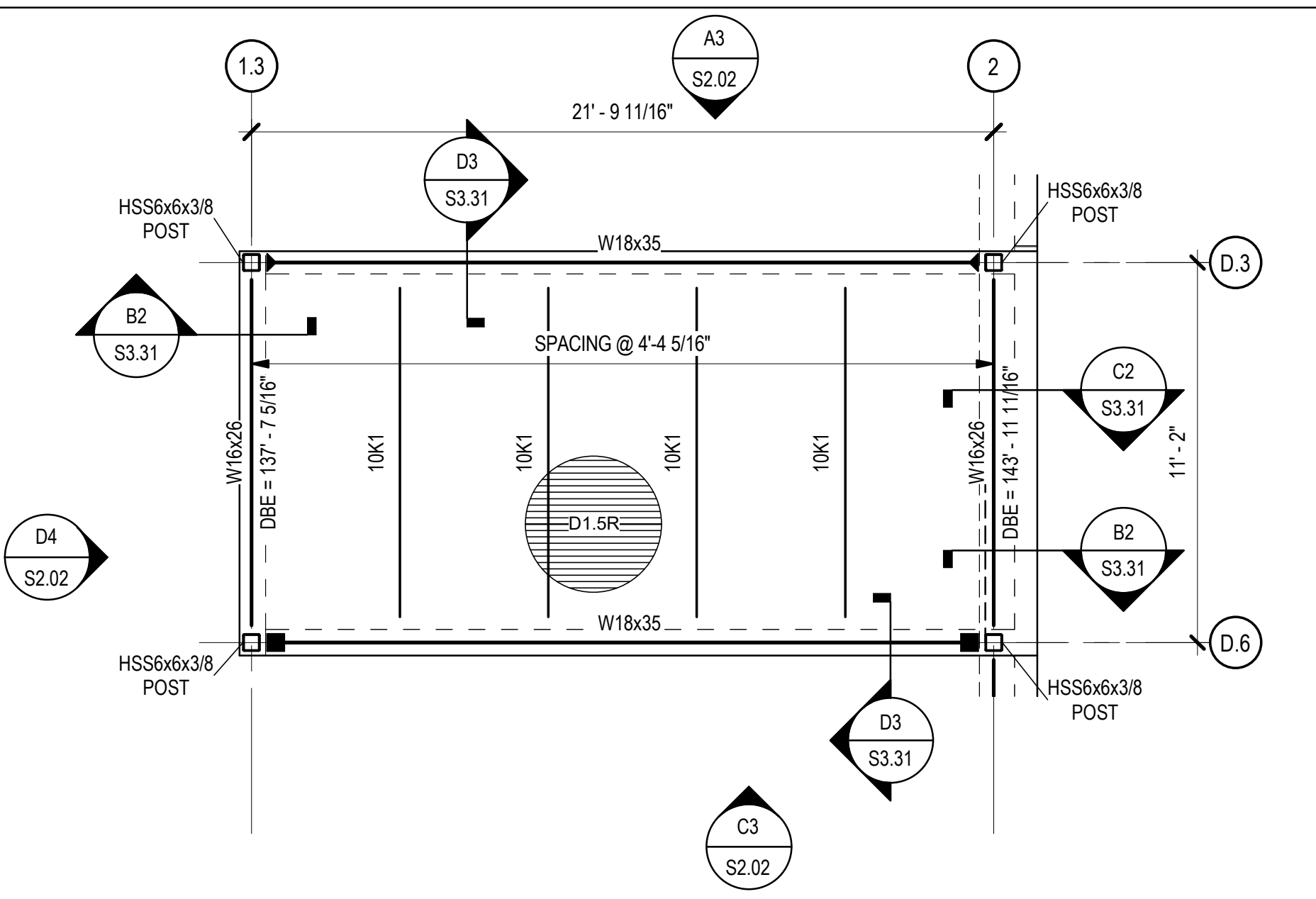
B1 ENLARGED WEST STAIR FRAMING PLAN
SCALE: 1/4" = 1'-0"



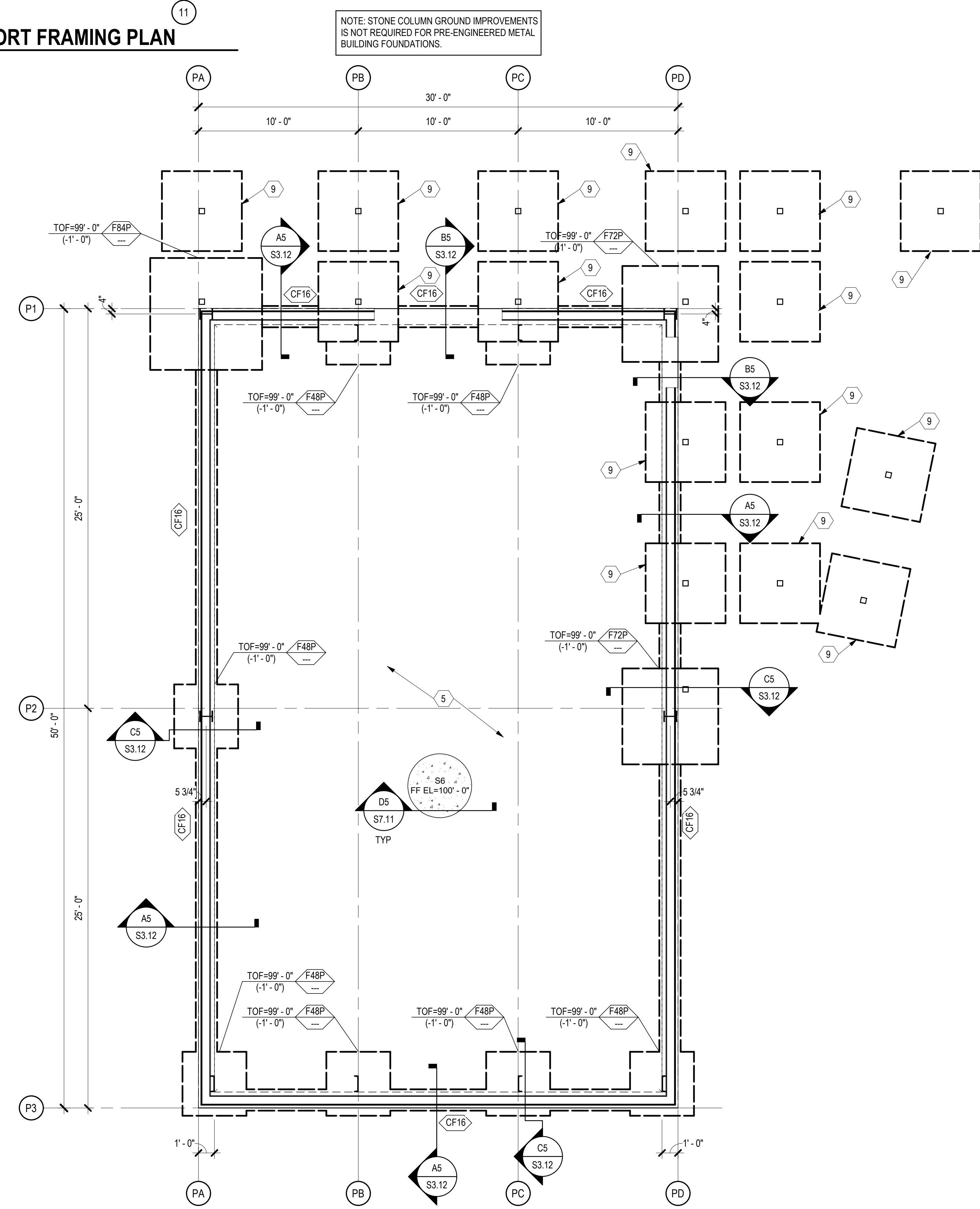
A1 ENLARGED EAST STAIR FRAMING PLAN
SCALE: 1/4" = 1'-0"



D2 MICROSCOPE SUPPORT FRAMING PLAN
SCALE: 1/4" = 1'-0"



D4 ENLARGED PLAN - WEST ROOF POP-UP
SCALE: 1/4" = 1'-0"



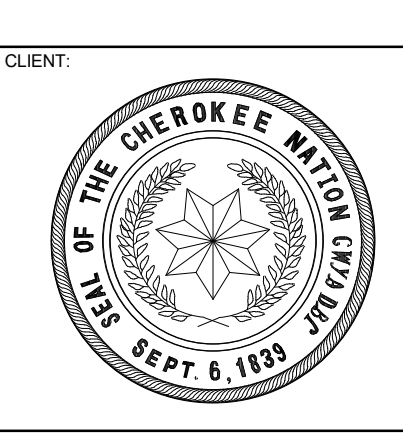
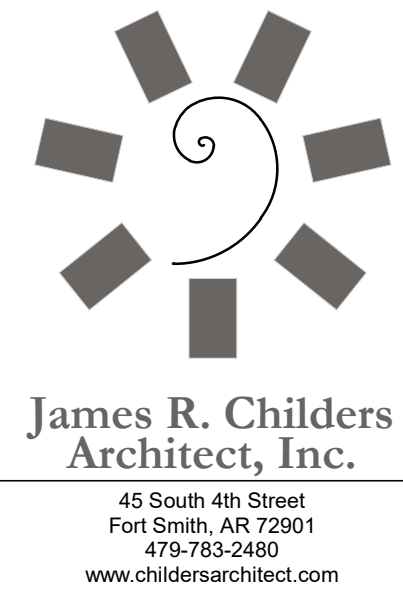
A3 ENLARGED PLAN - PEMB FOUNDATION
SCALE: 1/4" = 1'-0"

GENERAL SHEET NOTES

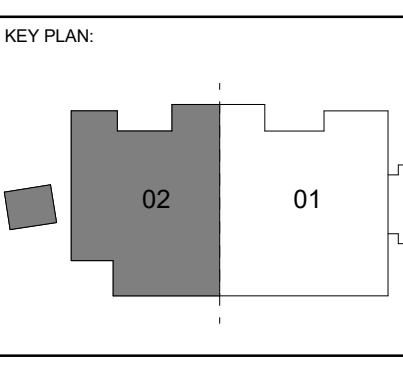
- SOME SHEET KEYNOTES MAY NOT APPLY TO THIS SHEET.
- REFERENCE FINISH FLOOR ELEVATION 100'-0" = MEAN SEA FINISH FLOOR ELEVATION. SEE CIVIL DRAWINGS.
- TOP OF FOOTING ELEVATION = 98'-0" (-2'-0"), UNLESS NOTED OTHERWISE ON PLAN.
- NOTE TO CONTRACTOR: ENLARGED SLAB BLOCKOUTS MAY BE REQUIRED AT FRAME COLUMNS FOR MOMENT FRAME BASE PLATE CLEARANCE.
- NOTE TO ERECTOR: LATERAL STABILITY OF THE STEEL FRAME IS DEPENDENT UPON THE MOMENT FRAMES. THE ERECTOR SHALL PROVIDE TEMPORARY BRACINGS OF THE STEEL FRAME IN ACCORDANCE WITH SECTION 7.10 OF THE AISC CODE OF STANDARD PRACTICES.
- DIMENSIONS ARE TO THE FACE OF STUD UNLESS NOTED OTHERWISE.
- SEE ARCHITECTURAL DRAWINGS FOR MASONRY DIMENSIONS NOT SHOWN.
- EXISTING CONSTRUCTION IS PER AVAILABLE EXISTING DRAWINGS. ALL EXISTING CONSTRUCTION AND DIMENSIONS SHALL BE VERIFIED PRIOR TO CONSTRUCTION. SHOULD CONDITIONS VARY FROM THOSE SHOWN, CONTACT ENGINEER BEFORE PROCEEDING.
- PROVIDE SLAB JOINTS AT 10'-0" ON CENTER MAXIMUM. THE AREA OF THE CONTROL JOINT SHALL NOT EXCEED A 2:1 RATIO. CONTROL JOINTS SHALL BE LOCATED AT COLUMN LINES WHERE THE LAYOUT PERMITS. AT RE-ENTRANT CORNERS THAT DO NOT HAVE CONTROL JOINTS, PROVIDE 2#4 x 3'-0" DIAGONAL TO THE RE-ENTRANT CORNER.
- STRUCTURAL COLD FORMED METAL STUDS SHALL BE 6" WIDE UNLESS NOTED OTHERWISE. STUD THICKNESS AND SPACING BY OTHERS.
- SEE SHEET S7.00 SERIES SHEETS FOR TYPICAL FOUNDATION SECTIONS AND DETAILS.
- SEE SHEET S6.01 FOR SCHEDULES.

SHEET KEYNOTE

- FLOOR DRAIN: SLOPE SLAB TO DRAIN 1/8" PER FOOT. COORDINATE EXACT SIZE AND LOCATION WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
- POST AND FOUNDATION AS REQUIRED FOR STAIR SUPPORT. STAIR ENGINEER TO PROVIDE REQUIRED LOADS AND LOCATIONS.
- ELEVATOR SUMP PIT. COORDINATE EXACT SIZE AND LOCATION WITH ELEVATOR MANUFACTURER. SEE A4 / S5.41
- HSS6x4x1/2 ELEVATOR RAIL SUPPORT POST. COORDINATE LOCATION AND SPACING WITH ELEVATOR MANUFACTURER. SEE B4 / S5.41
- PRE-ENGINEERED METAL BUILDING STEEL AND ANCHORAGE BY OTHERS. CONTRACTOR TO CONFIRM LOCATIONS OF FOUNDATIONS WITH FINAL PRE-ENGINEERED METAL BUILDING SHOP DRAWINGS.
- NOTCH MASONRY AS REQUIRED TO FACILITATE BASEPLATE INSTALLATION. STEP BOND BEAM AT THIS LOCATION.
- CUT AND REMOVE EXISTING SLAB AS REQUIRED TO PLACE NEW FOOTING. NEW SLAB TO POUR UP TO REMAINING SLAB.
- CENTER FOOTING ON GRID C.
- F60A PRE-MANUFACTURED SUNSHADE CONCRETE FOOTING. TOP OF FOOTING = 99'-0" (-1'-0"). SEE SHEET S6.01 FOR FOOTING SCHEDULE. COORDINATE FINAL LOCATION WITH SUNSHADE MANUFACTURER.
- EXISTING CANOPY. SEE ARCHITECTURAL DEMOLITION PLANS FOR EXTENT OF DEMOLITION.
- HSS8x6x1/2 ELEVATOR SUPPORT POST. COORDINATE EXACT LOCATION AND SPACING WITH ELEVATOR MANUFACTURER. SEE B4 / S5.41, D3 / S5.41, A2 / S5.41, B2 / S5.41, AND C2 / S5.41
- 1 1/2" RECESSED SLAB AT ADA SHOWER. COORDINATE EXACT SIZE, LOCATION, AND SLOPE REQUIREMENTS WITH ARCHITECTURAL DRAWINGS. SEE C4 / S7.11
- 18" DIAMETER PRECAST CONCRETE COLUMN BY OTHERS.
- 18" DIAMETER PRECAST CONCRETE CANOPY COLUMN BY OTHERS.



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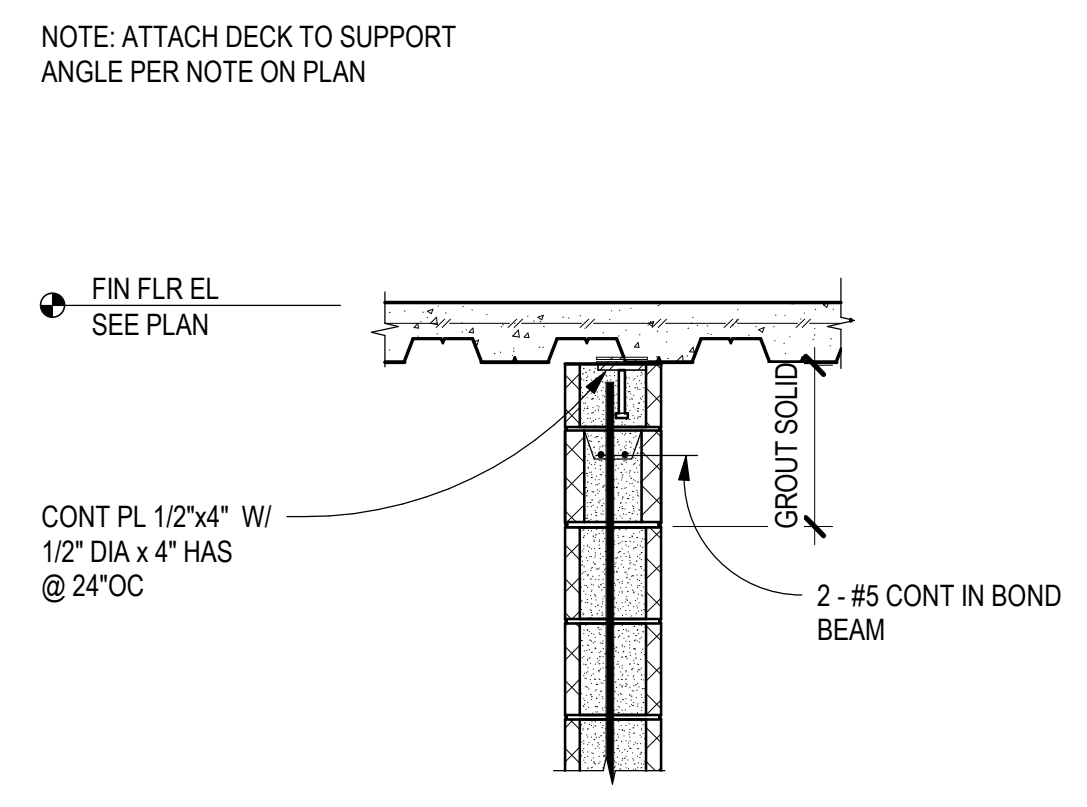
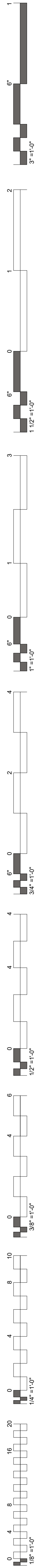


PROJECT PHASE:
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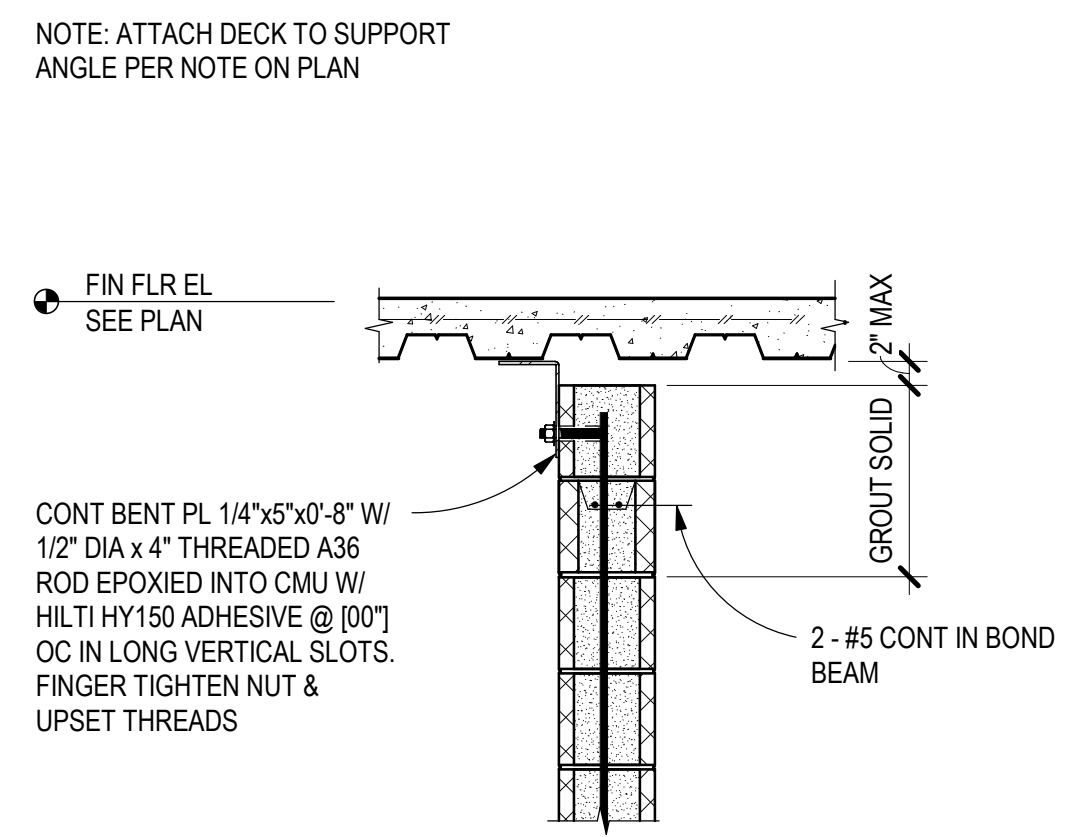
#	DATE	REVISIONS	DESCRIPTION

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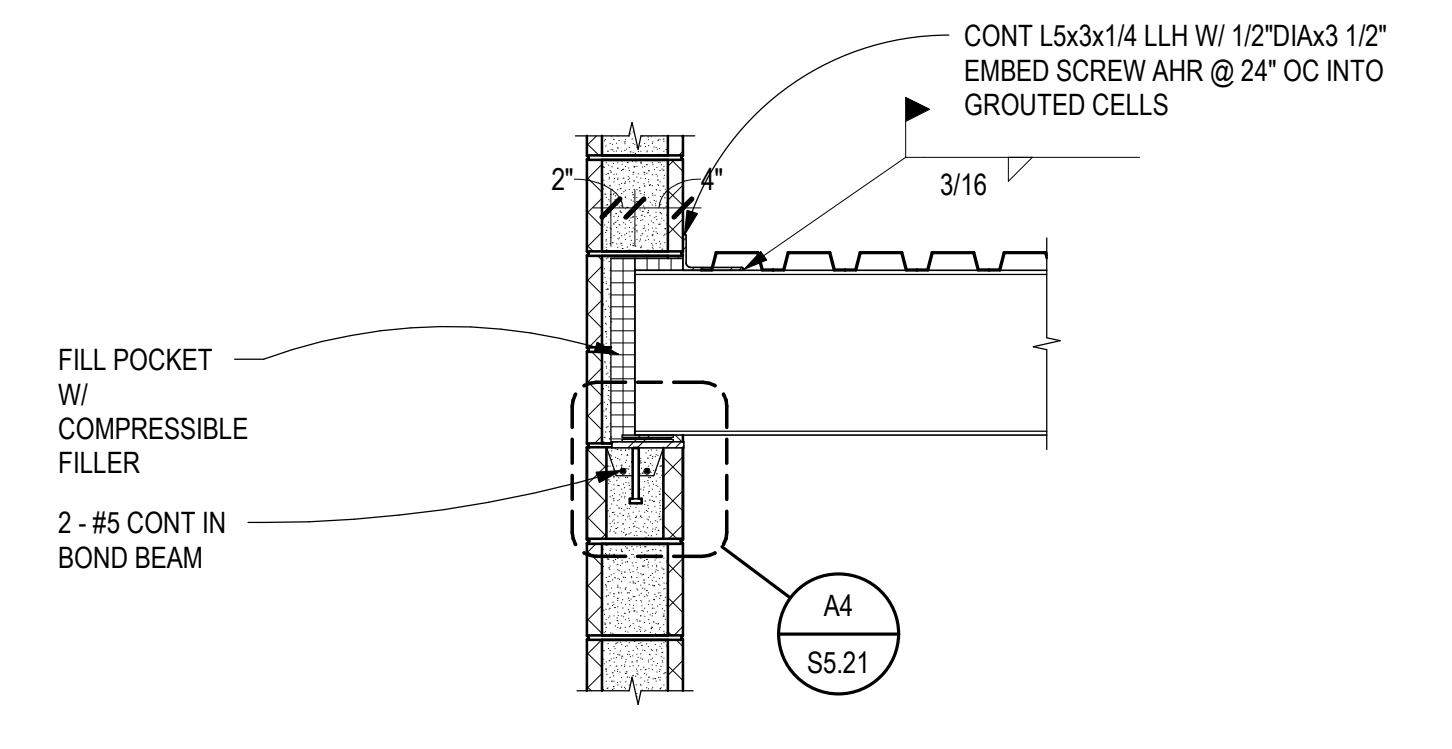
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S4.01
ENLARGED PLANS



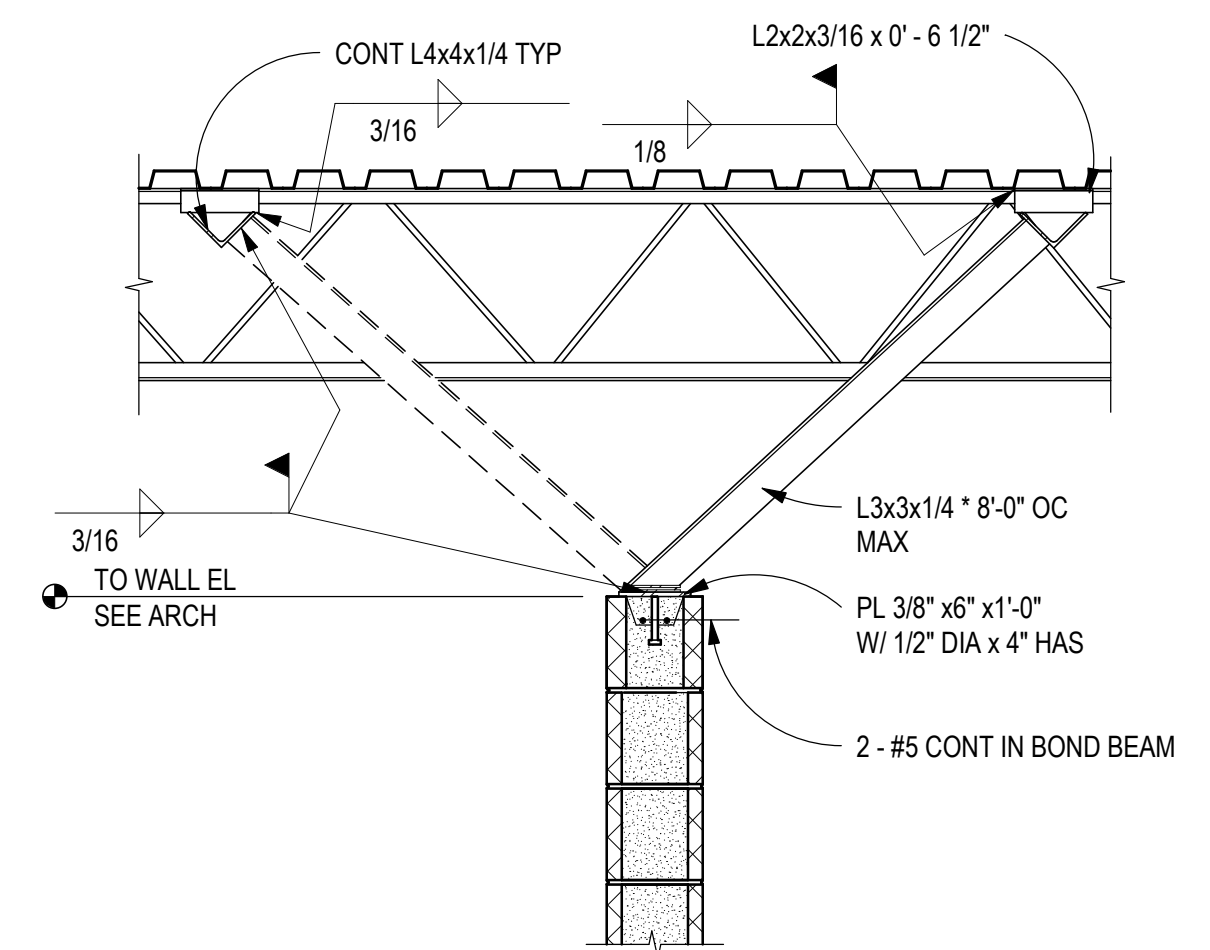
D3 CMU BEARING WALL TO DECK
SCALE: 3/4" = 1'-0"



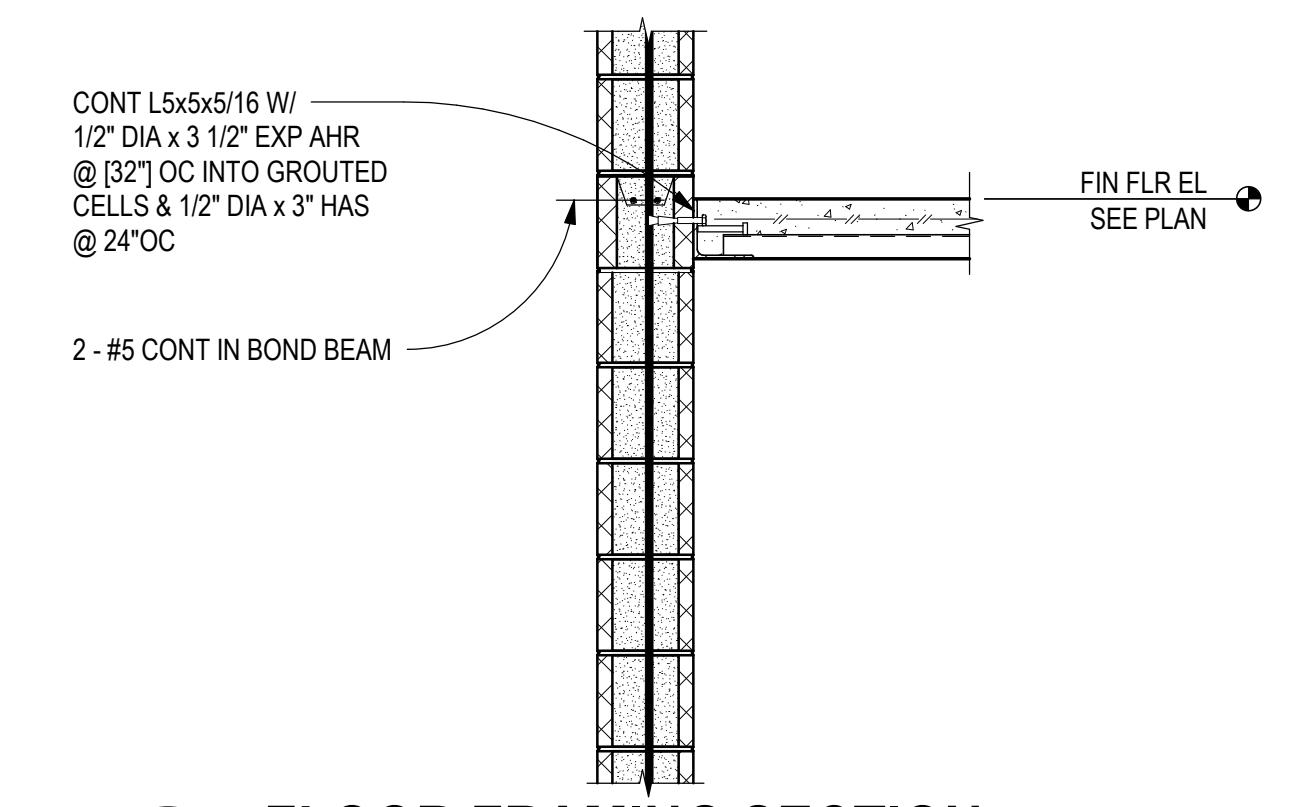
D4 CMU NON-BRG WALL TO DECK
SCALE: 3/4" = 1'-0"



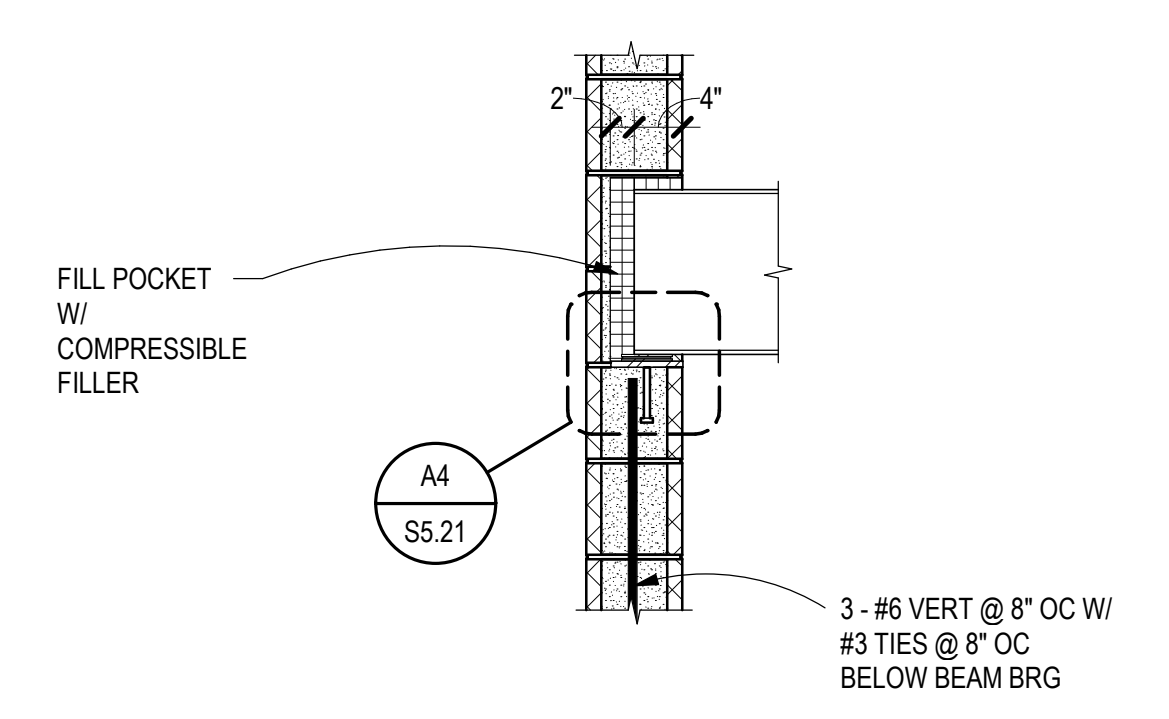
D5 TYPICAL BEAM TO WALL SLIDE BRG
SCALE: 3/4" = 1'-0"



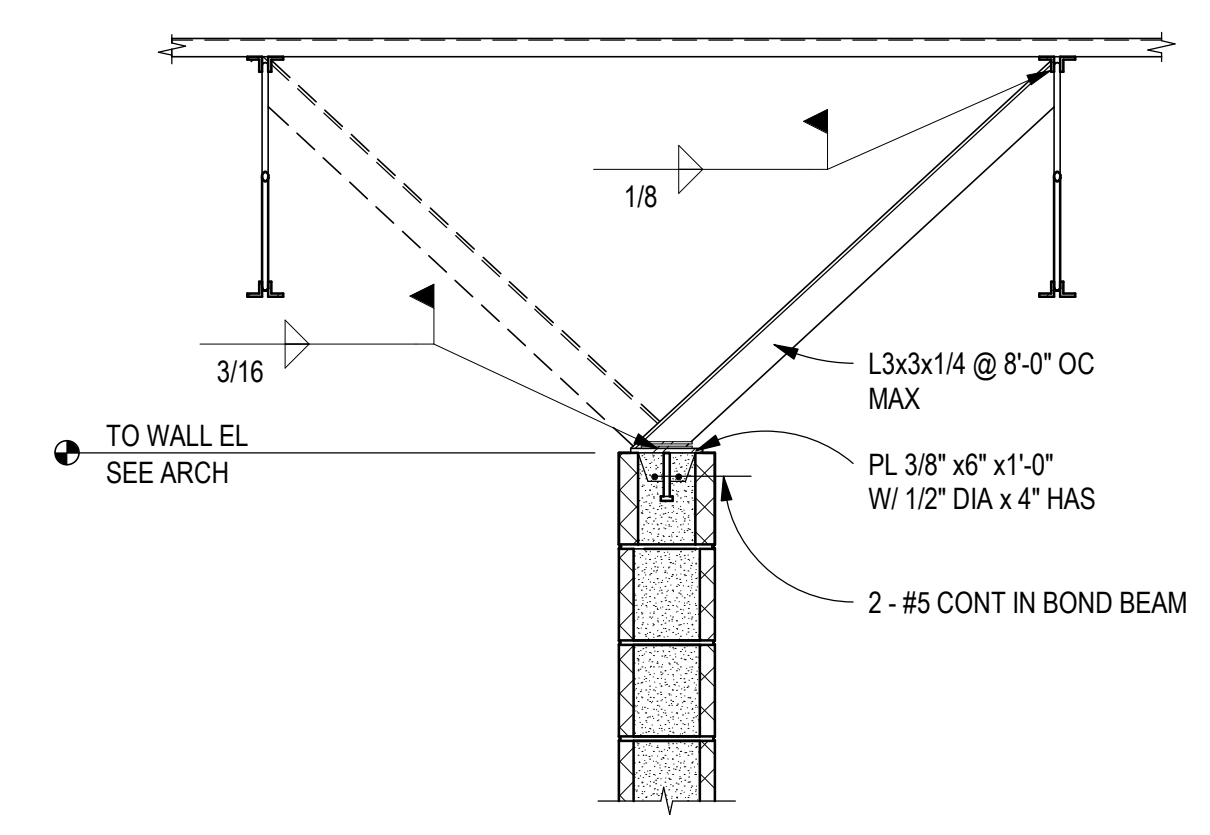
C3 CMU WALL BRACING SECTION
SCALE: 3/4" = 1'-0"



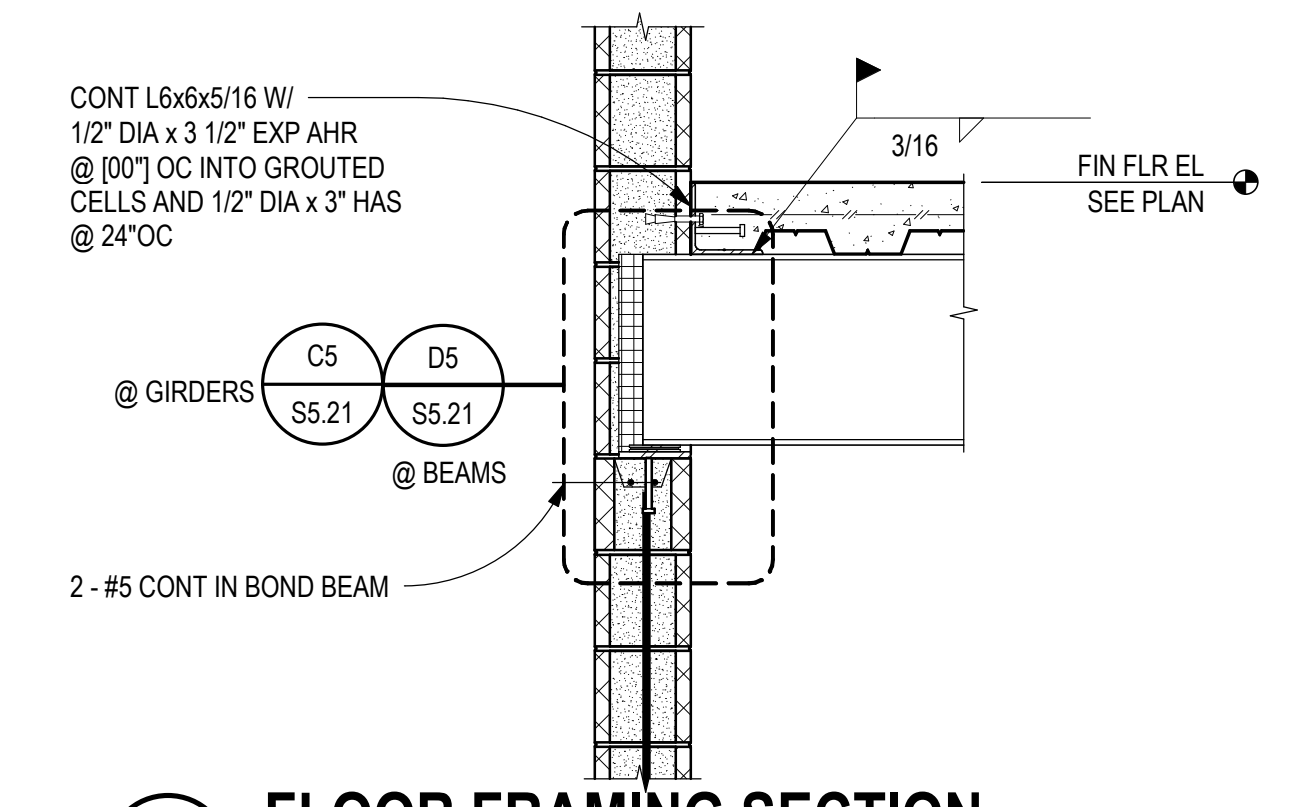
C4 FLOOR FRAMING SECTION
SCALE: 3/4" = 1'-0"



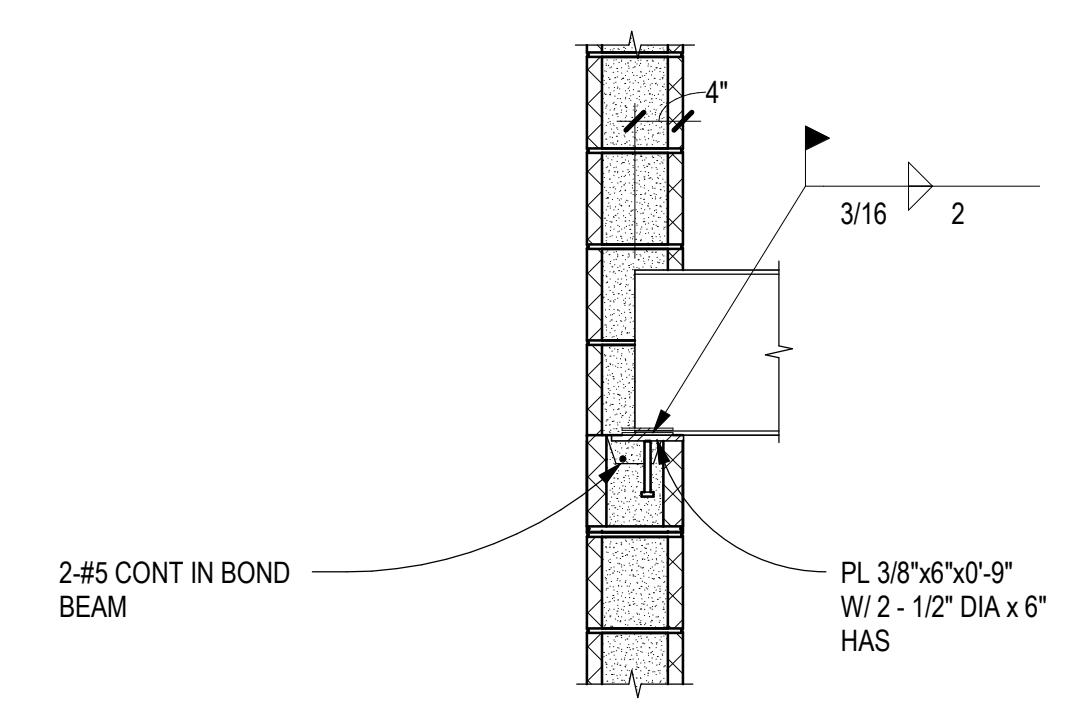
C5 TYPICAL GIRDER TO WALL SLIDE BRG
SCALE: 3/4" = 1'-0"



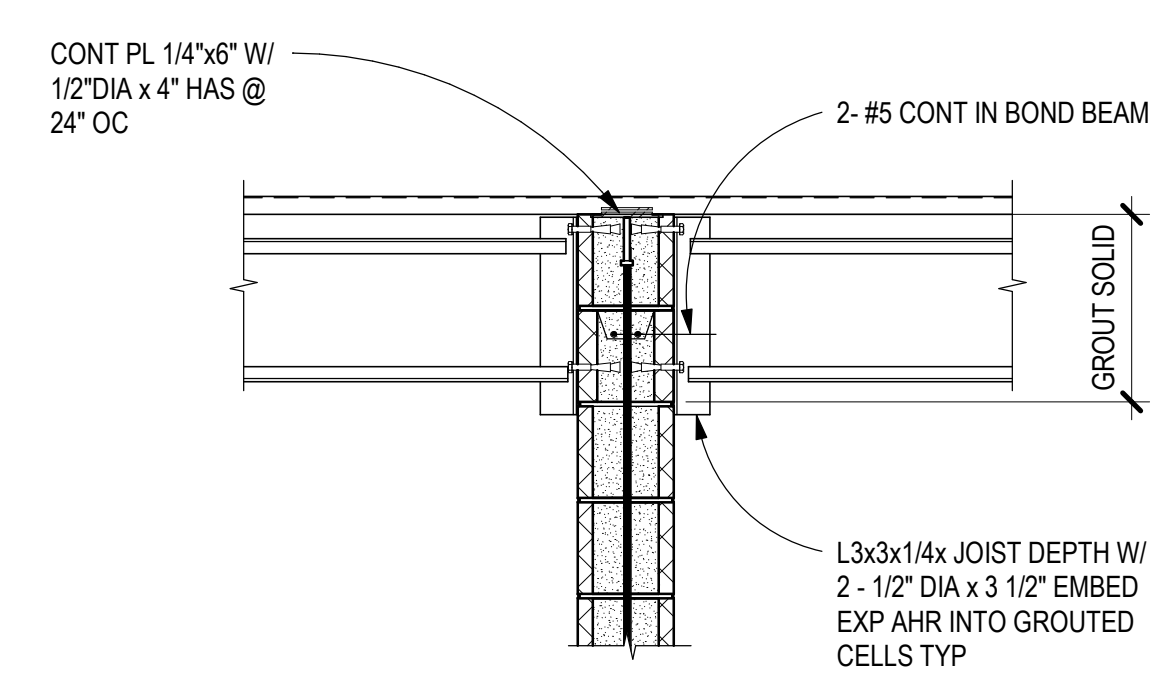
B3 CMU WALL BRACING SECTION
SCALE: 3/4" = 1'-0"



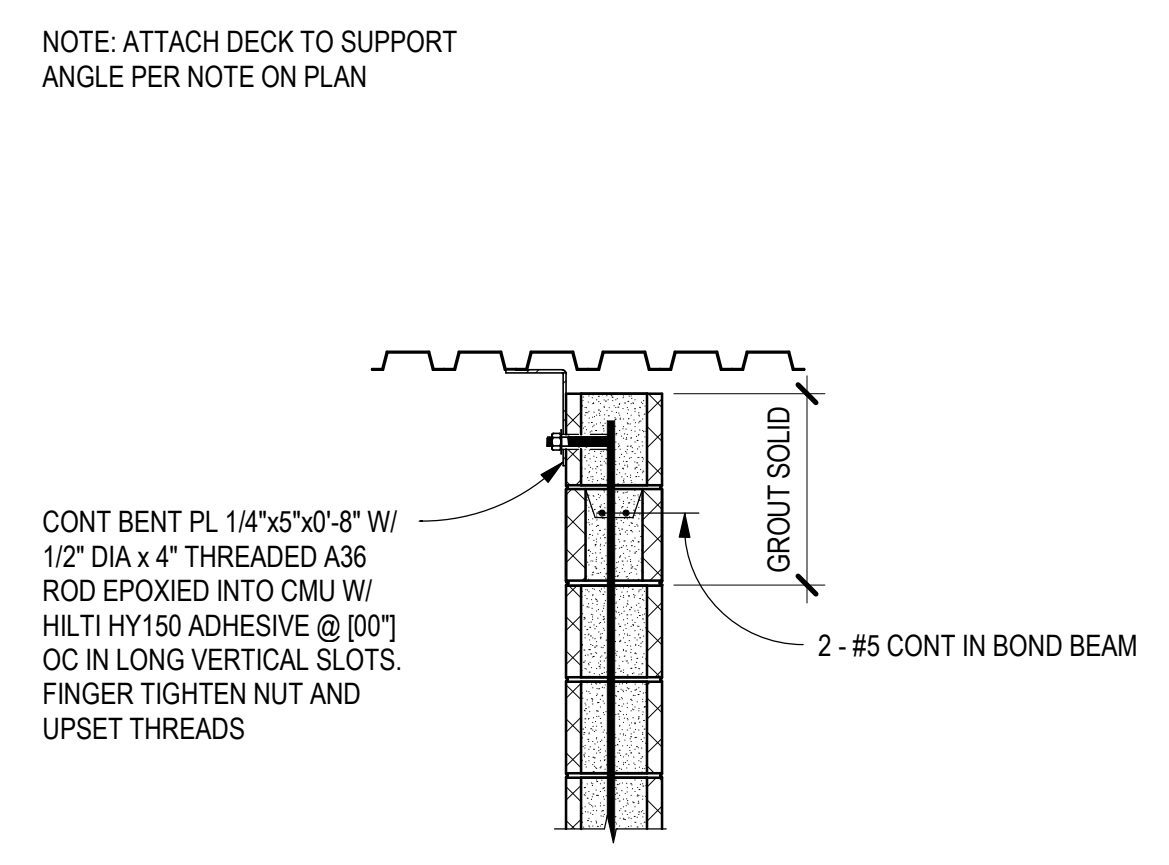
B4 FLOOR FRAMING SECTION
SCALE: 3/4" = 1'-0"



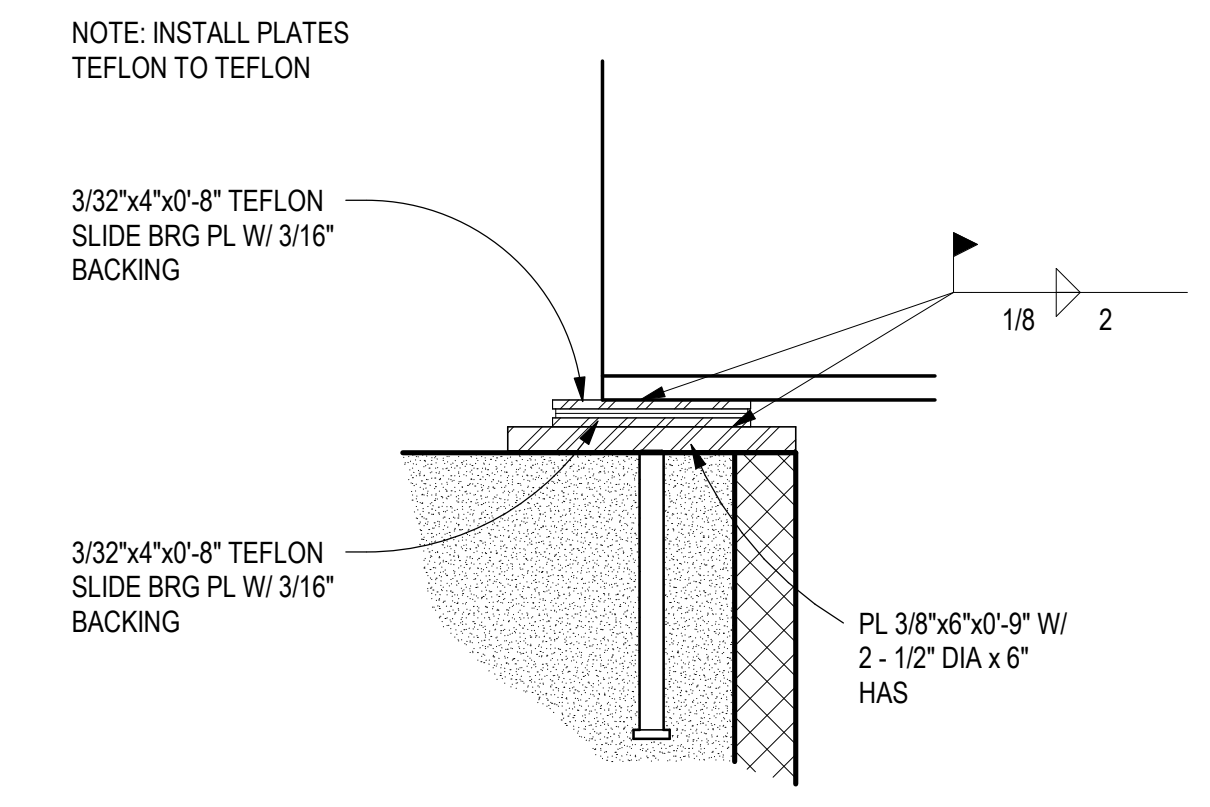
B5 TYPICAL BEAM TO CMU WALL
SCALE: 3/4" = 1'-0"



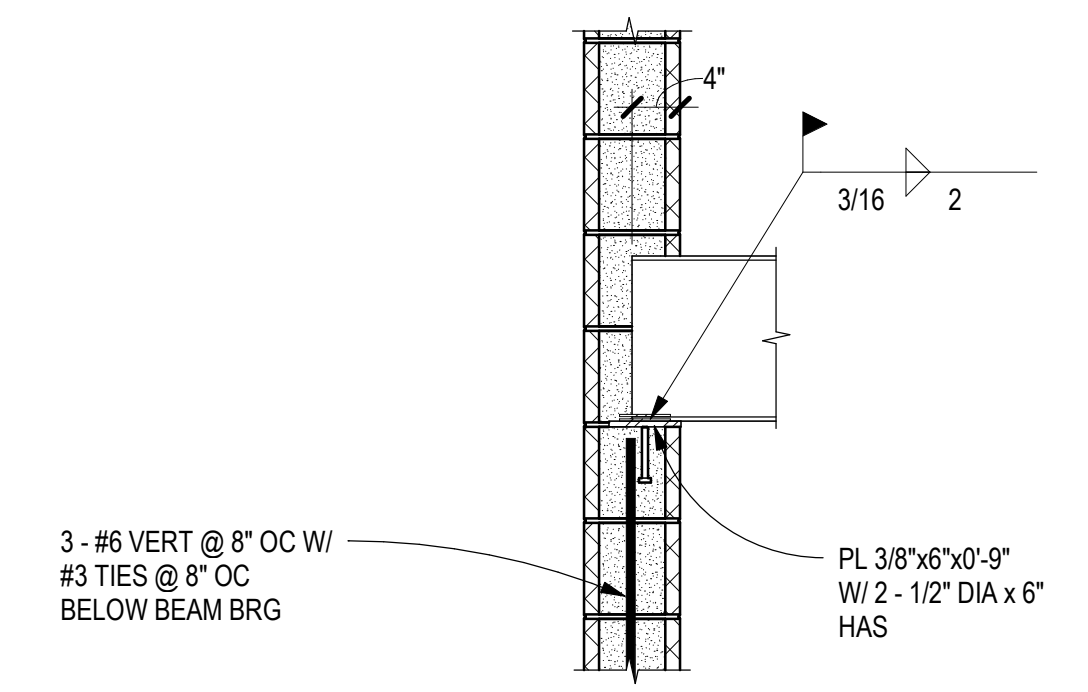
A2 INT CMU BEARING WALL TO DECK
SCALE: 3/4" = 1'-0"



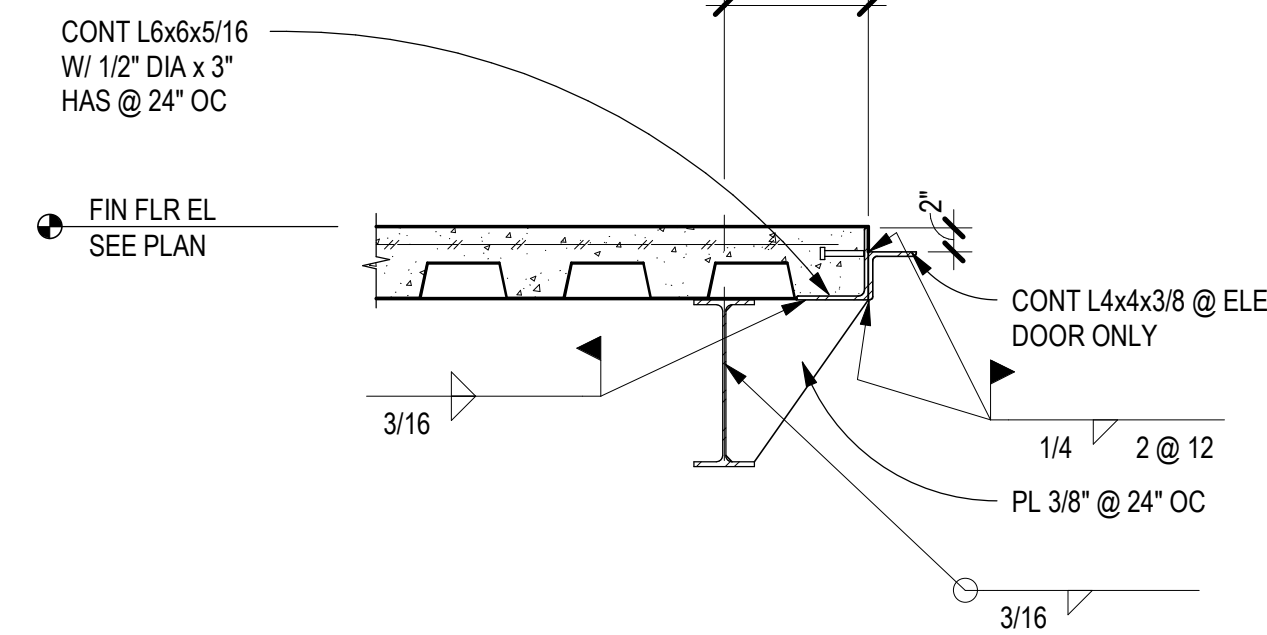
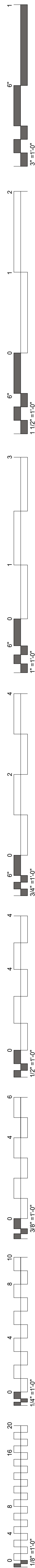
A3 CMU NON-BRG WALL TO DECK
SCALE: 3/4" = 1'-0"



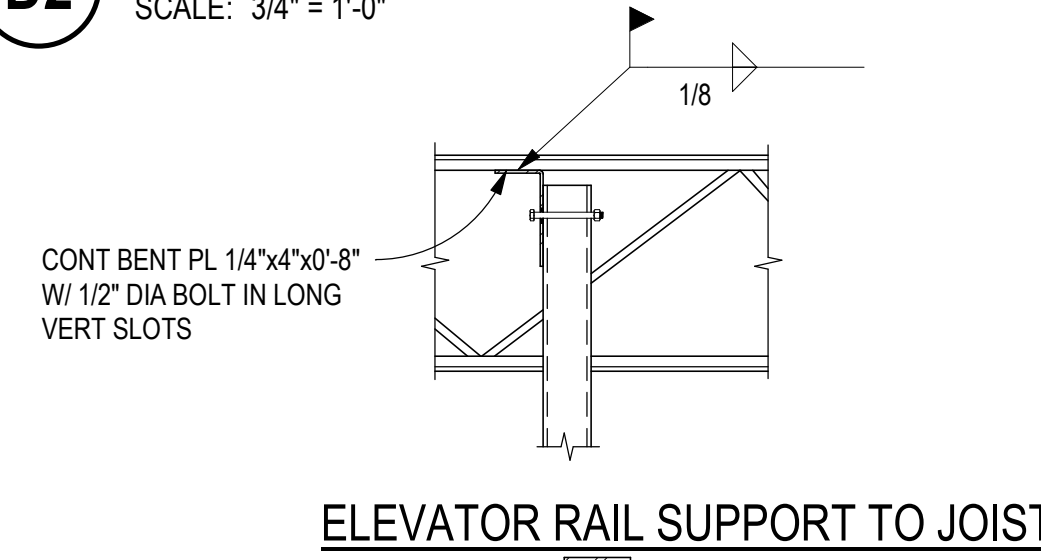
A4 TYPICAL SLIDE BRG CONN
SCALE: 1 1/2" = 1'-0"



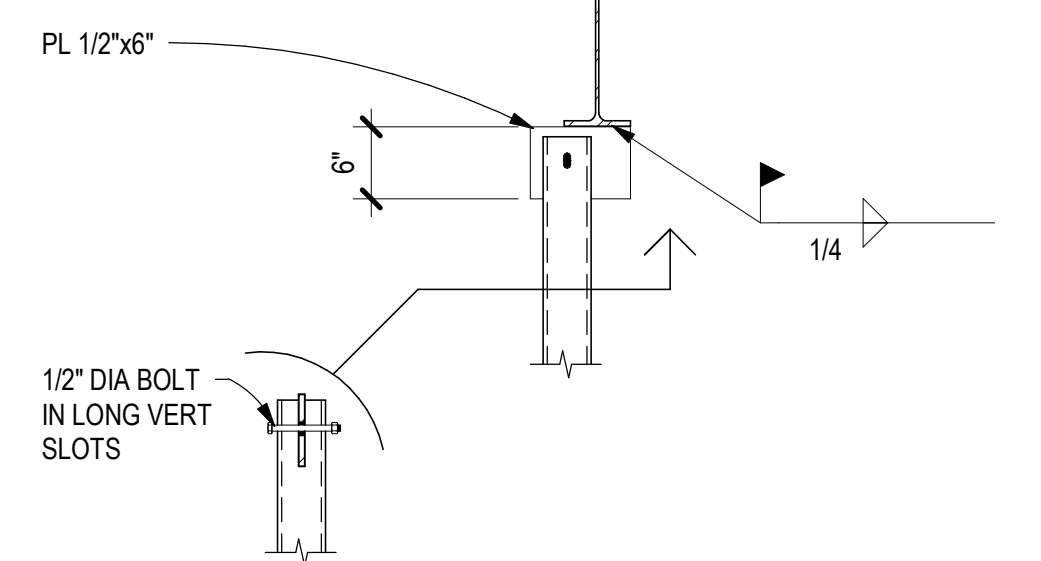
A5 TYPICAL GIRDER TO CMU WALL
SCALE: 3/4" = 1'-0"



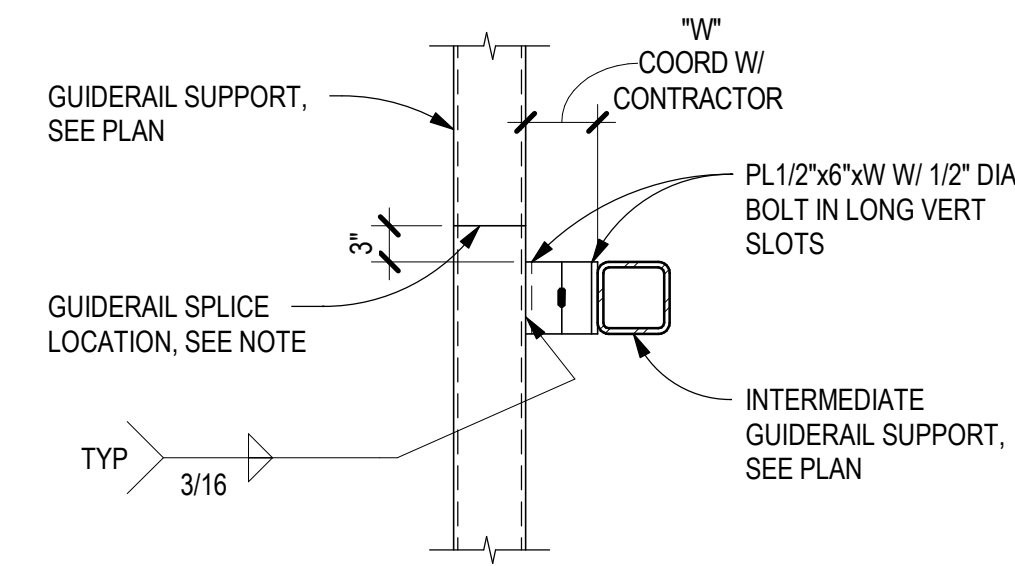
D2 ELEVATOR OPENING SECTION
SCALE: 3/4" = 1'-0"



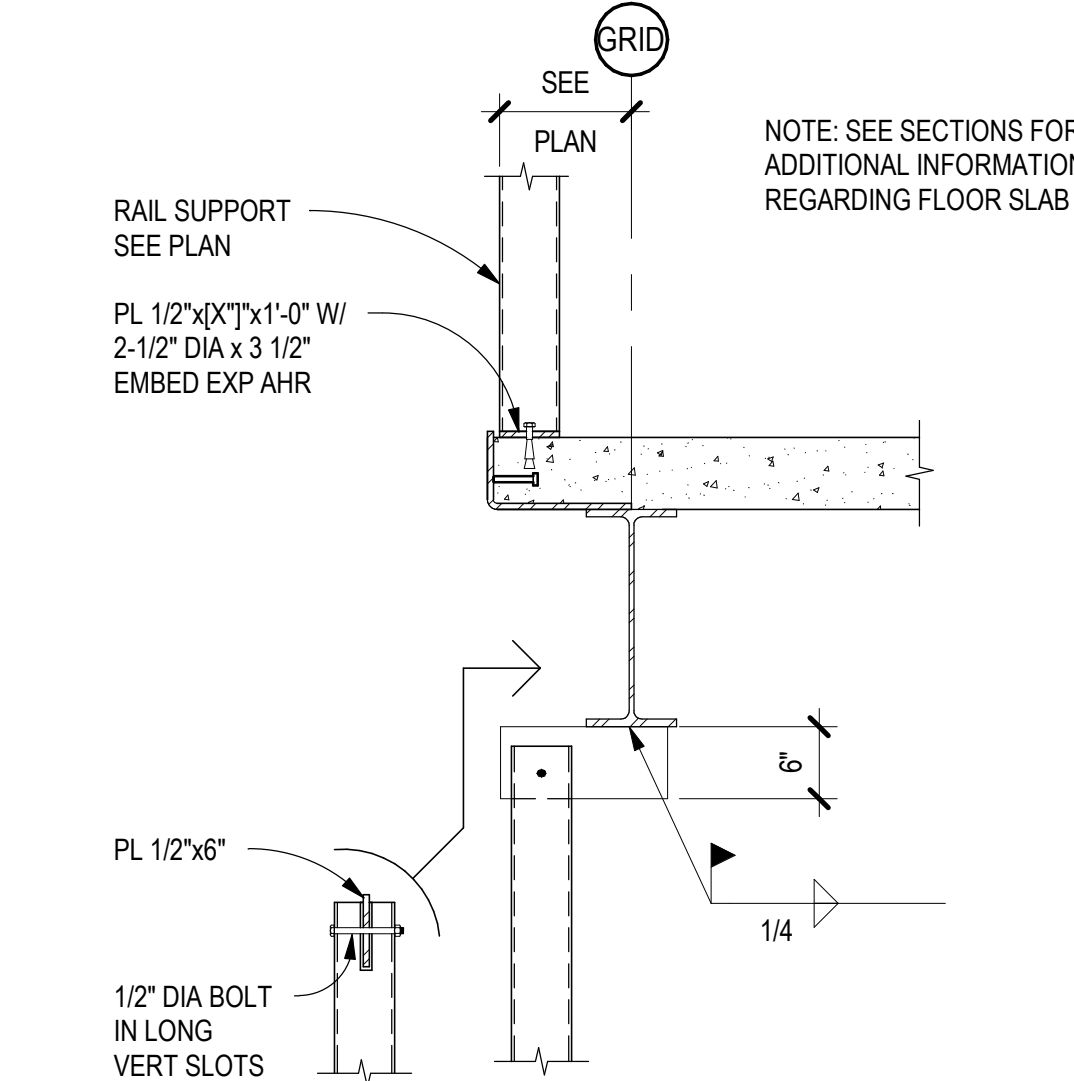
ELEVATOR RAIL SUPPORT TO JOIST



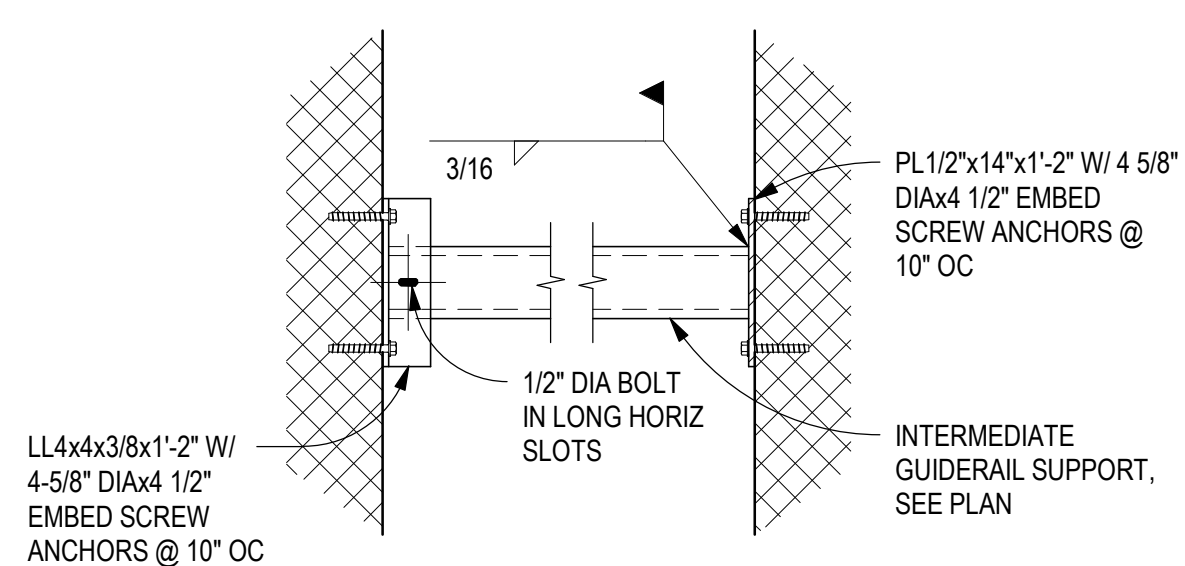
C2 ELEV RAIL SUPPORT TO ROOF
SCALE: 3/4" = 1'-0"



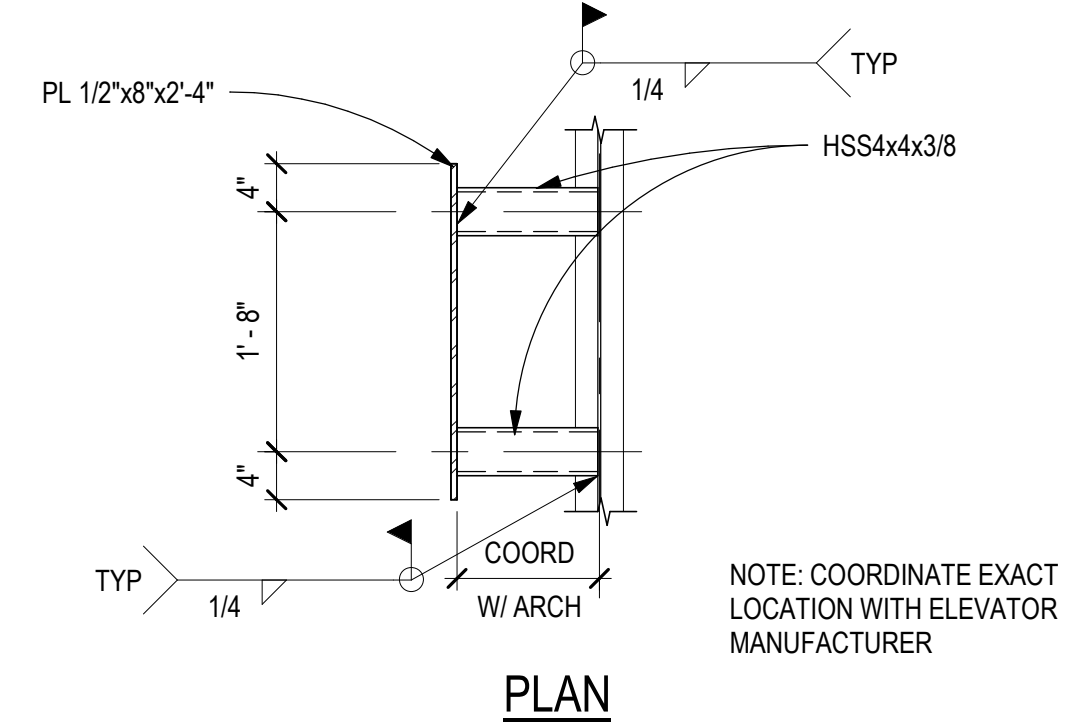
B1 GUIDERAIL @ INTERMEDIATE SUPPORT
SCALE: 3/4" = 1'-0"



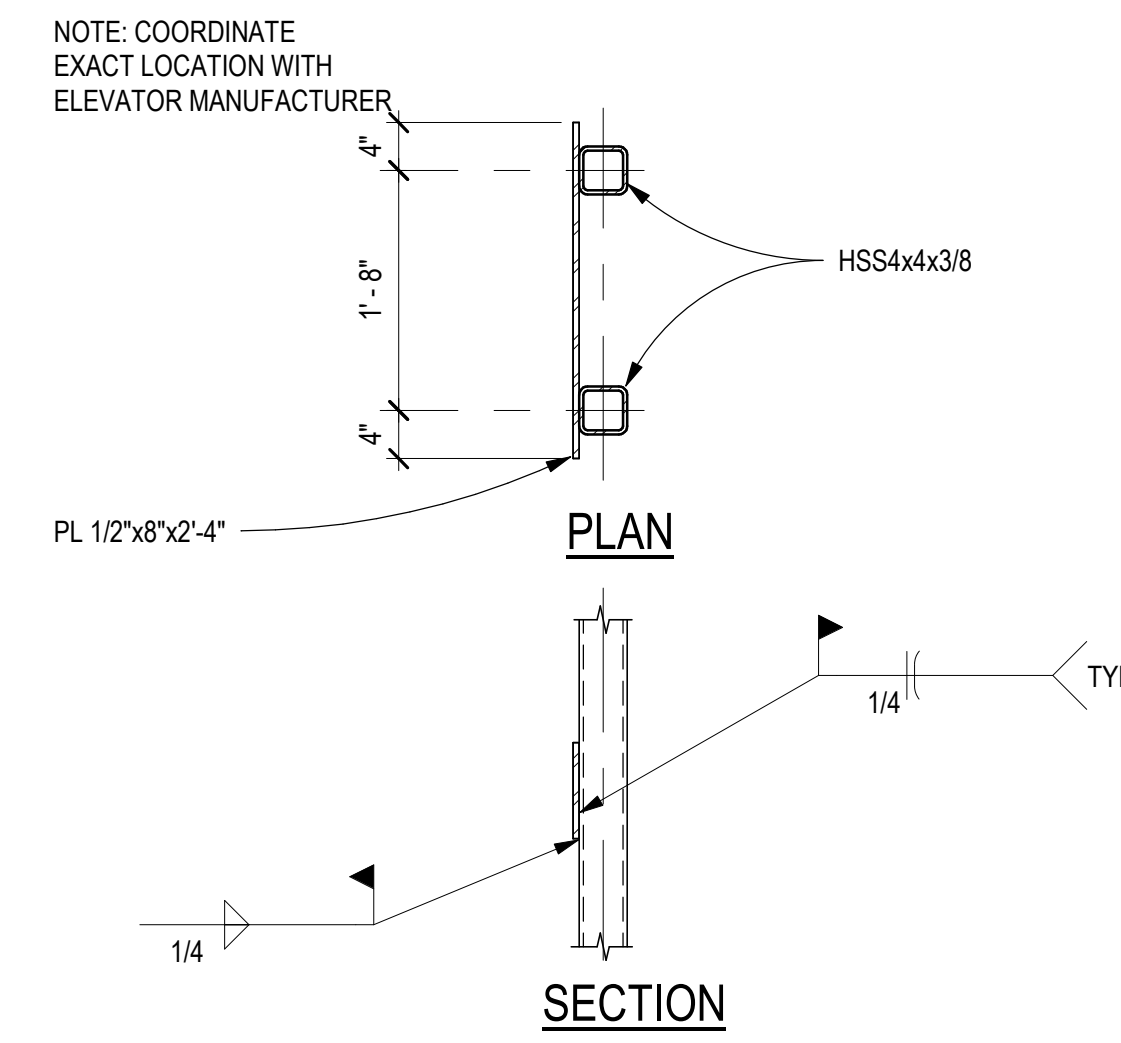
B2 ELEVATOR DETAIL
SCALE: 3/4" = 1'-0"



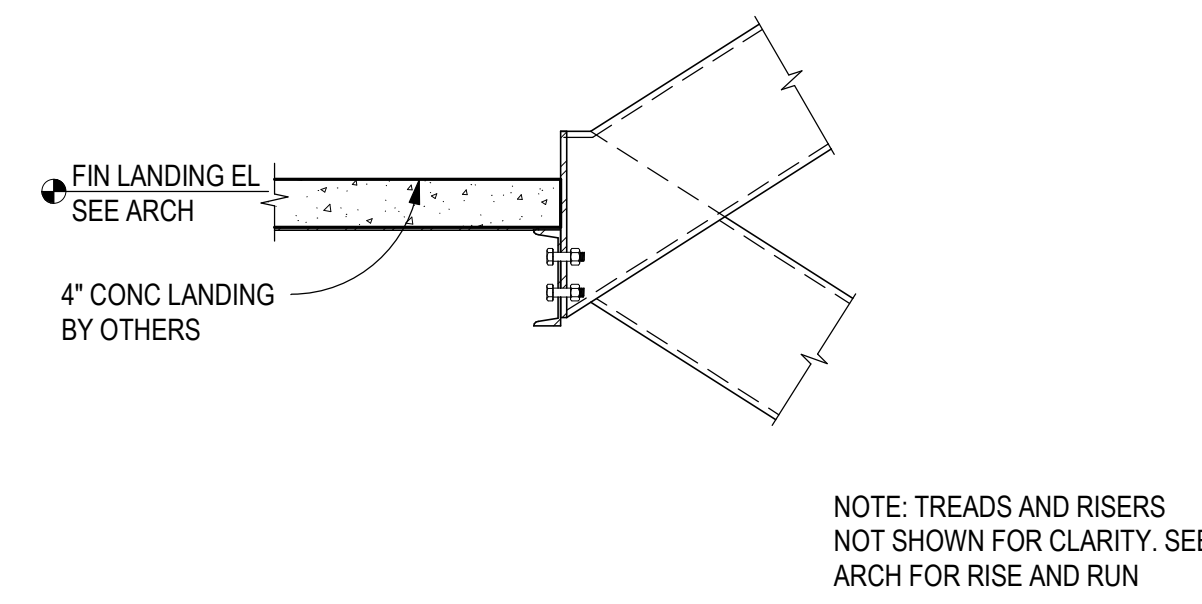
A1 INTERMEDIATE SUPPORT @ CMU
SCALE: 3/4" = 1'-0"



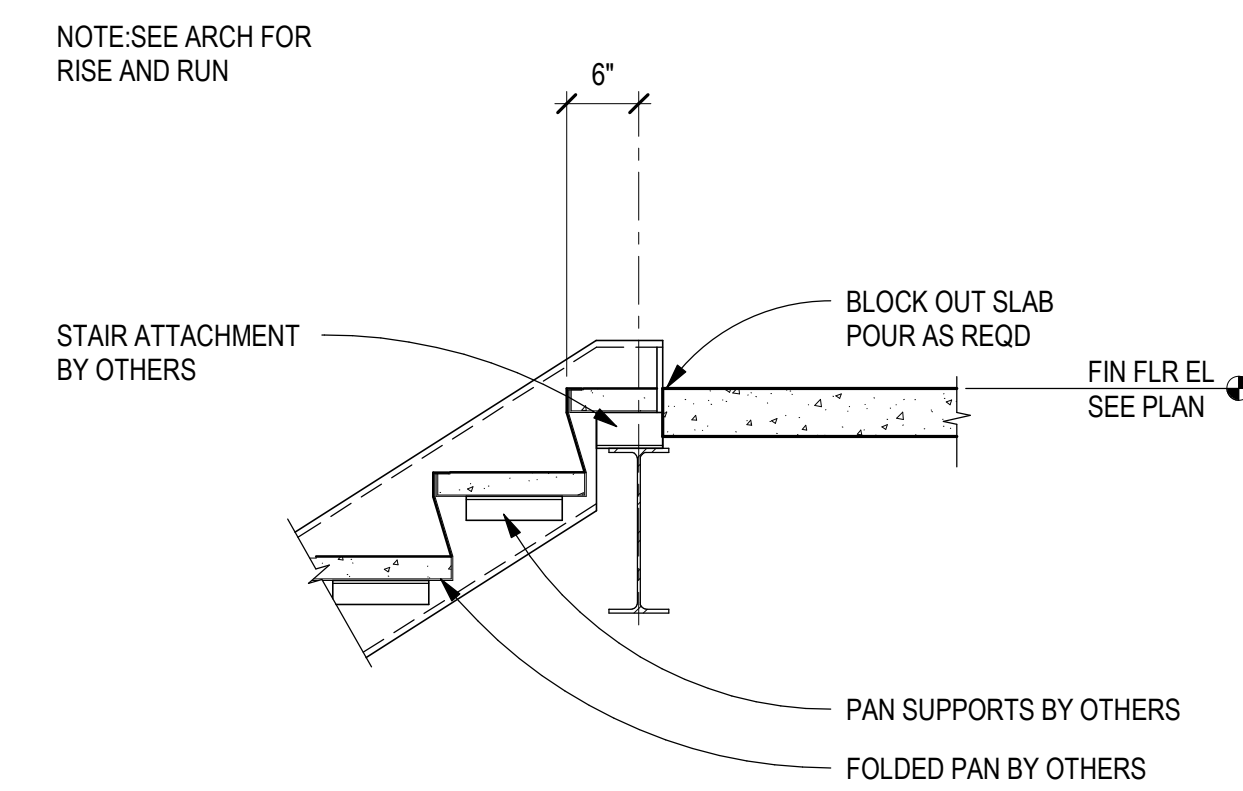
A2 ELEV RAIL BRACKET SUPPORT
SCALE: 3/4" = 1'-0"



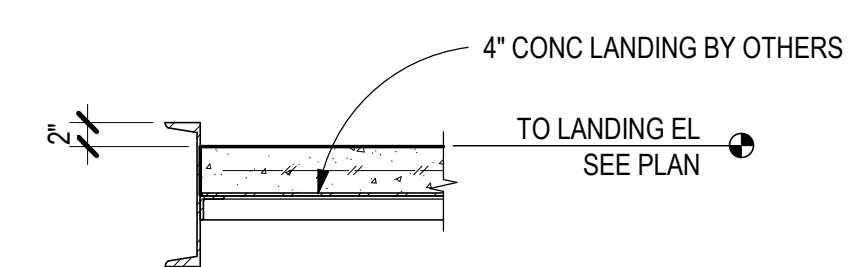
D3 ELEV RAIL BRACKET SUPPORT
SCALE: 3/4" = 1'-0"



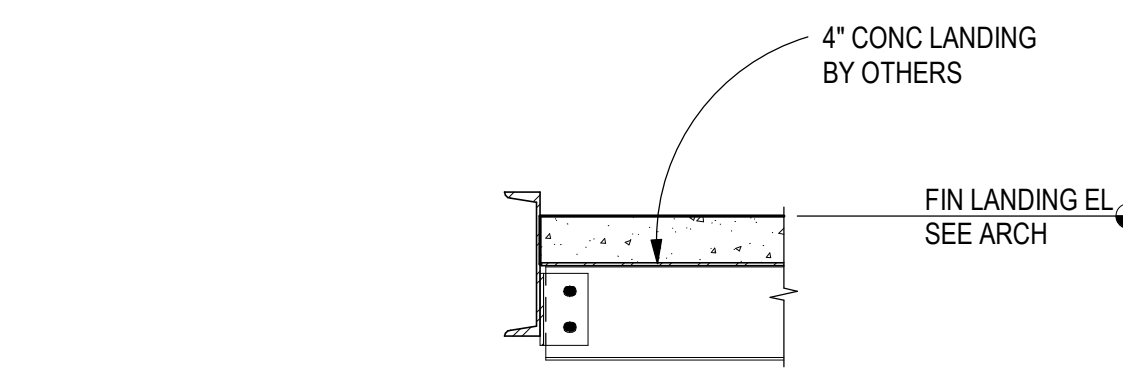
C3 INTERMEDIATE LANDING SECTION
SCALE: 3/4" = 1'-0"



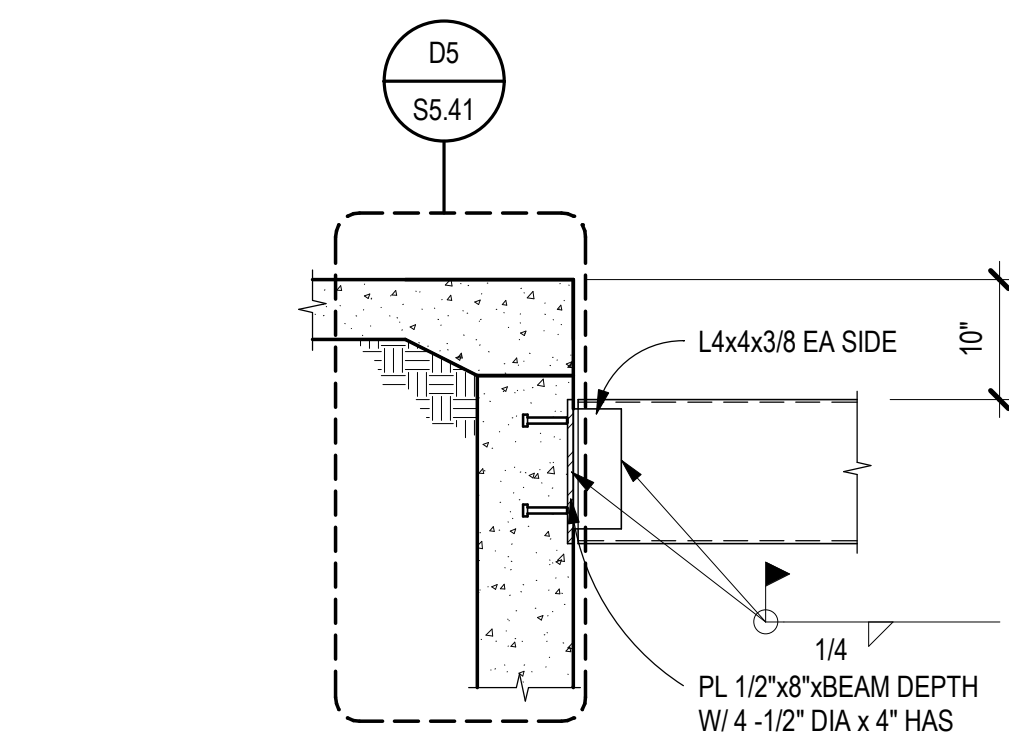
B3 STAIR STRINGER AT LANDING
SCALE: 3/4" = 1'-0"



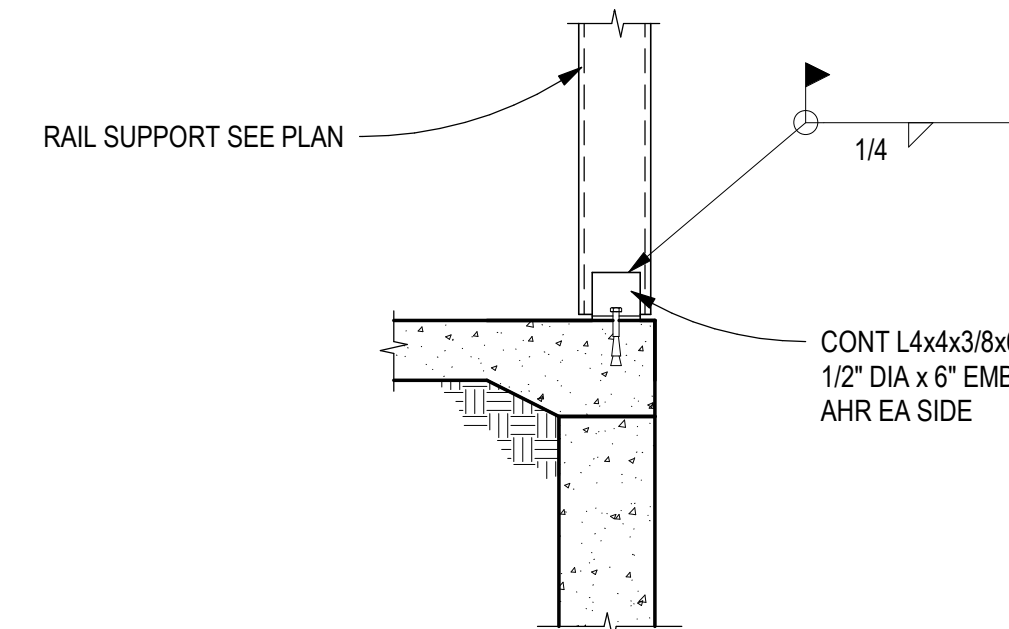
A3 STAIR LANDING SECTION
SCALE: 3/4" = 1'-0"



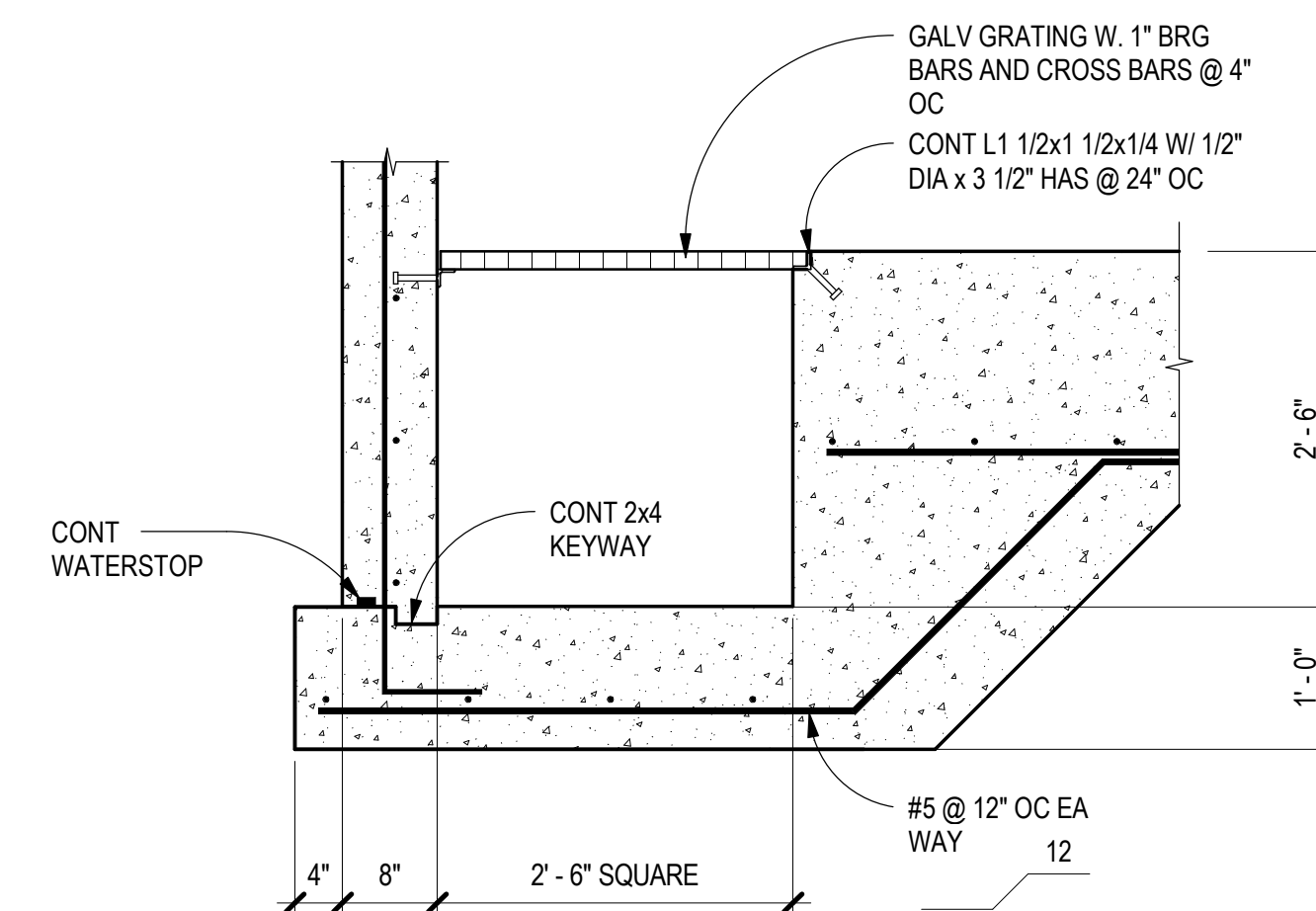
D4 STAIR LANDING SECTION
SCALE: 3/4" = 1'-0"



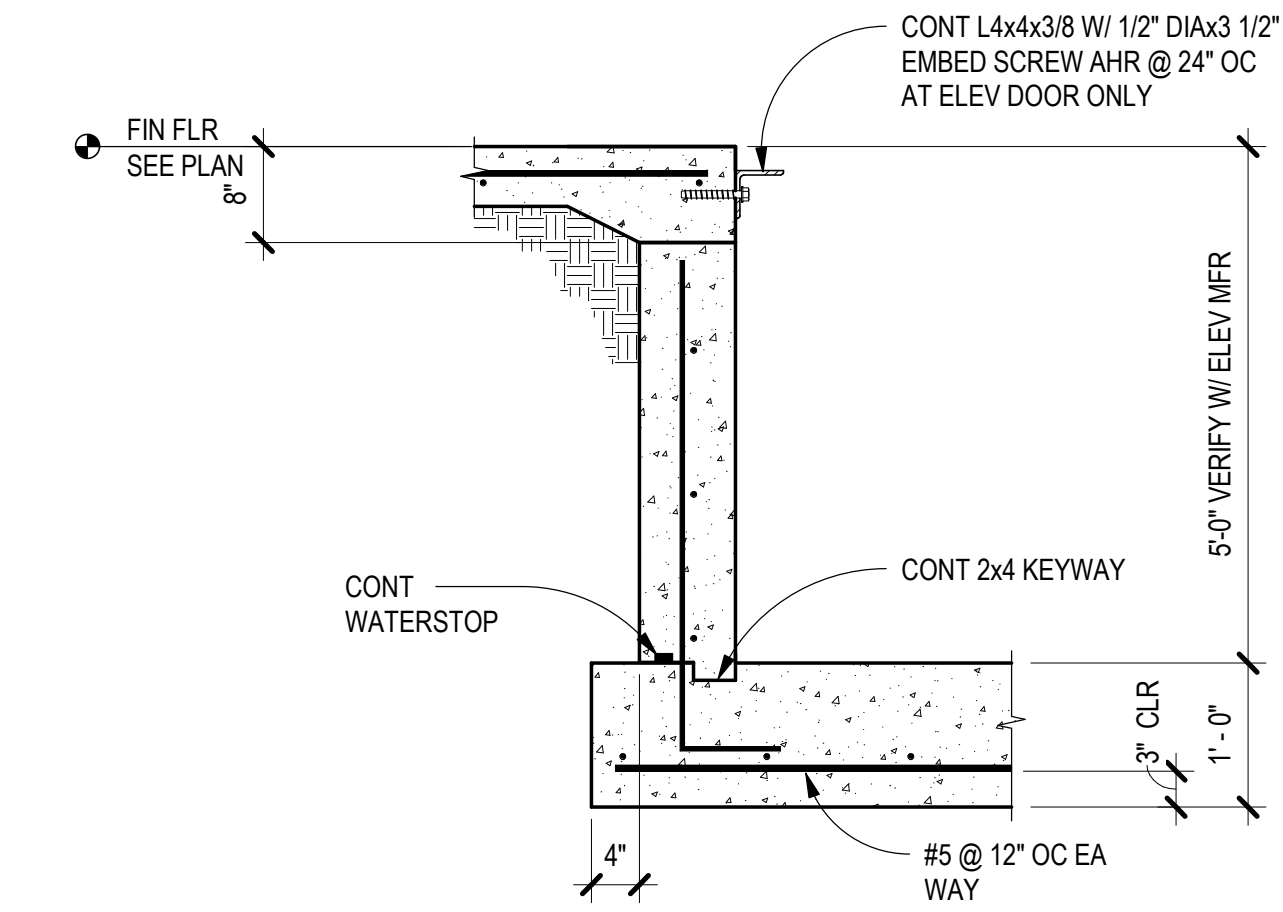
C4 ELEVATOR SEPARATOR BEAM
SCALE: 3/4" = 1'-0"



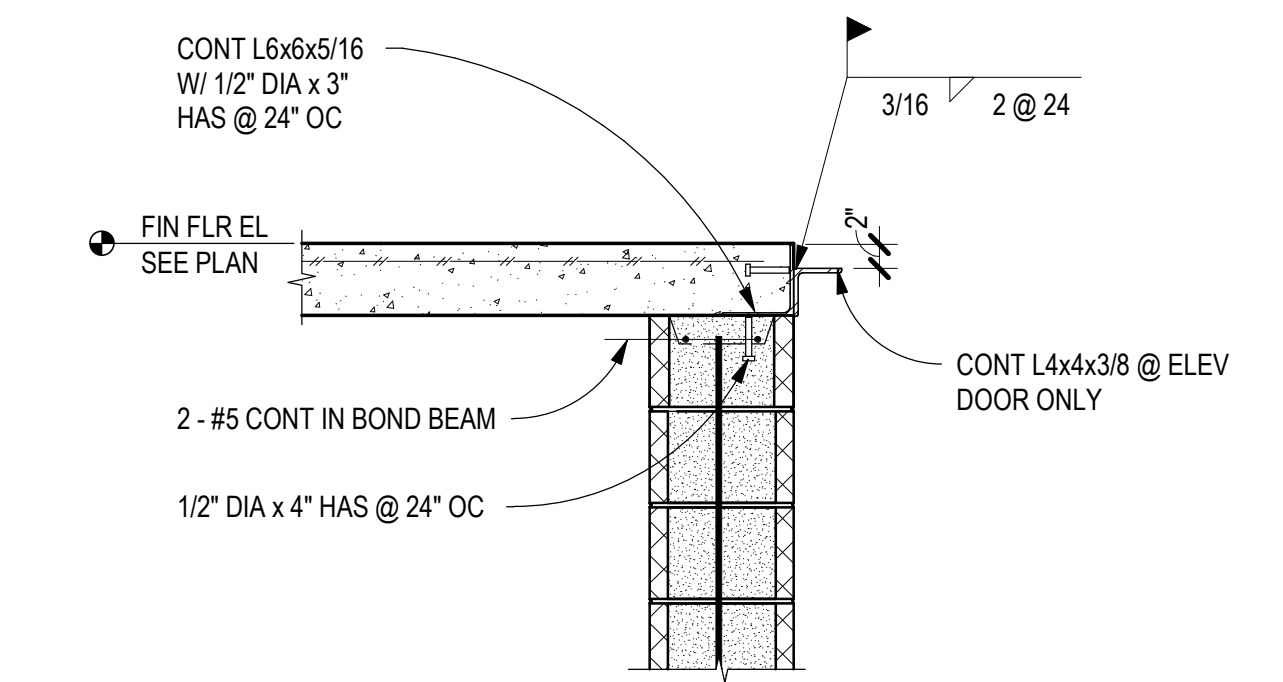
B4 ELEVATOR PIT SECTION
SCALE: 3/4" = 1'-0"



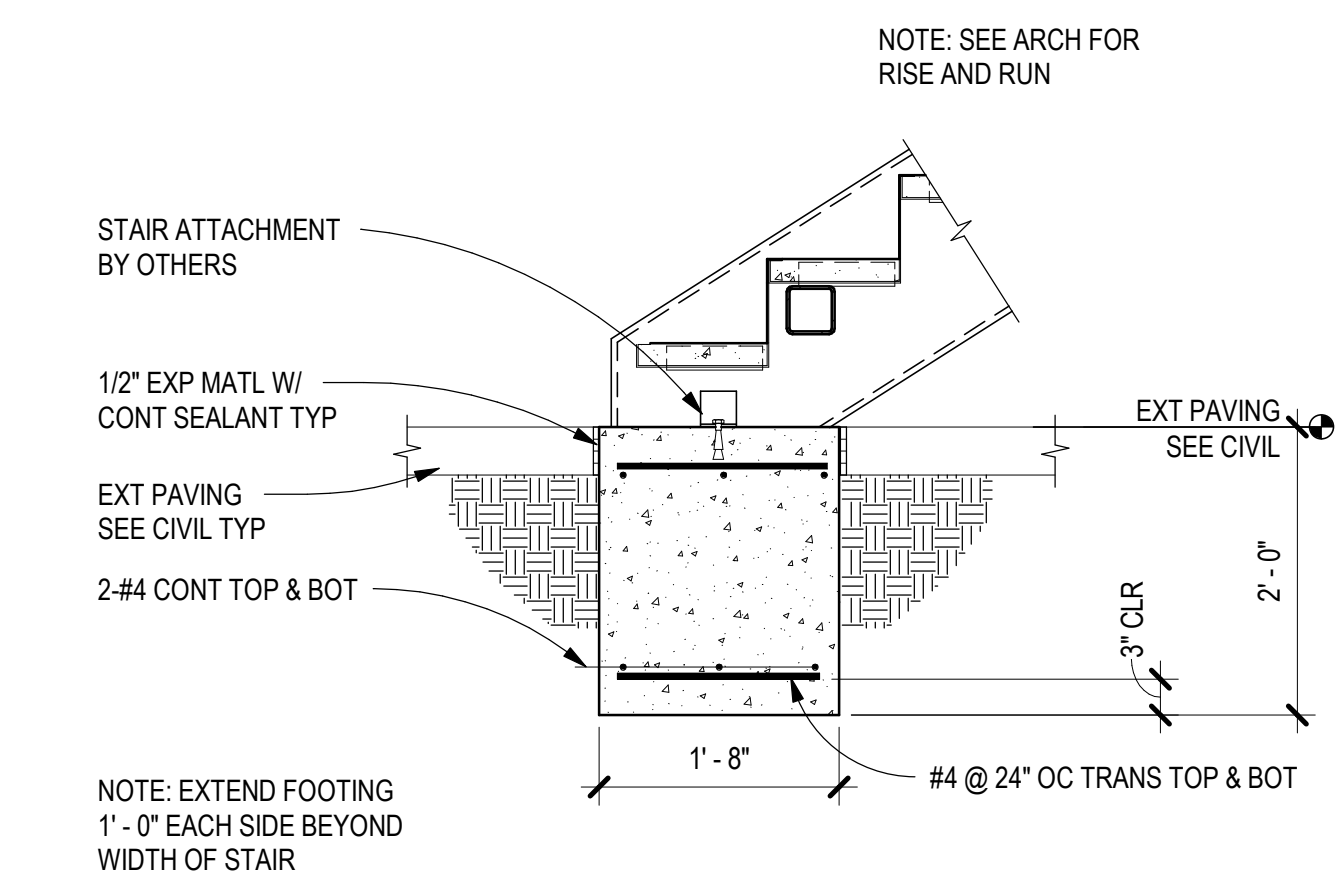
A4 ELEVATOR SUMP PIT SECTION
SCALE: 3/4" = 1'-0"



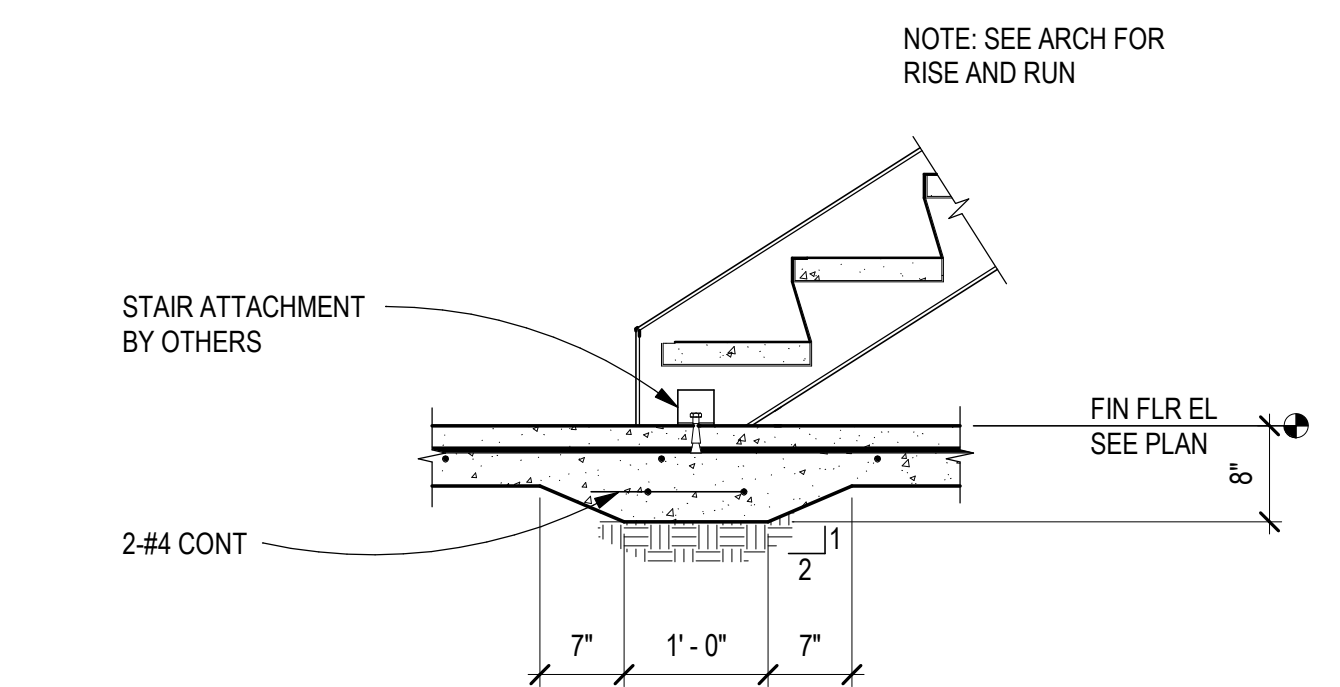
D5 ELEVATOR PIT SECTION
SCALE: 3/4" = 1'-0"



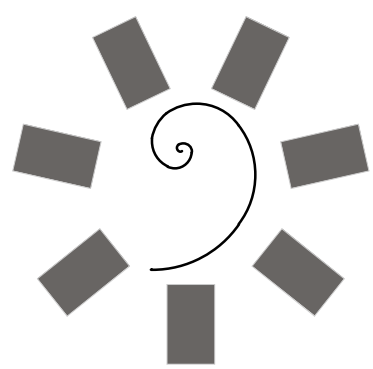
C5 ELEVATOR OPENING SECTION
SCALE: 3/4" = 1'-0"



B5 STAIR BASE DETAIL
SCALE: 3/4" = 1'-0"



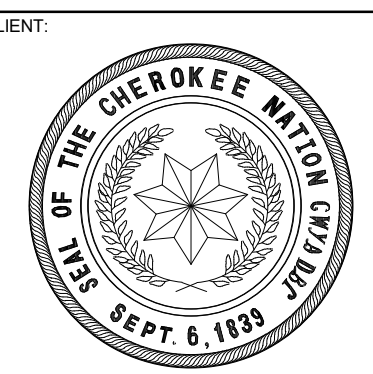
A5 TYPICAL STAIR BASE DETAIL
SCALE: 3/4" = 1'-0"



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EXPANSION
STILWELL, OKLAHOMA

KEY PLAN

PROJECT PHASE:
BID PACKAGE 01

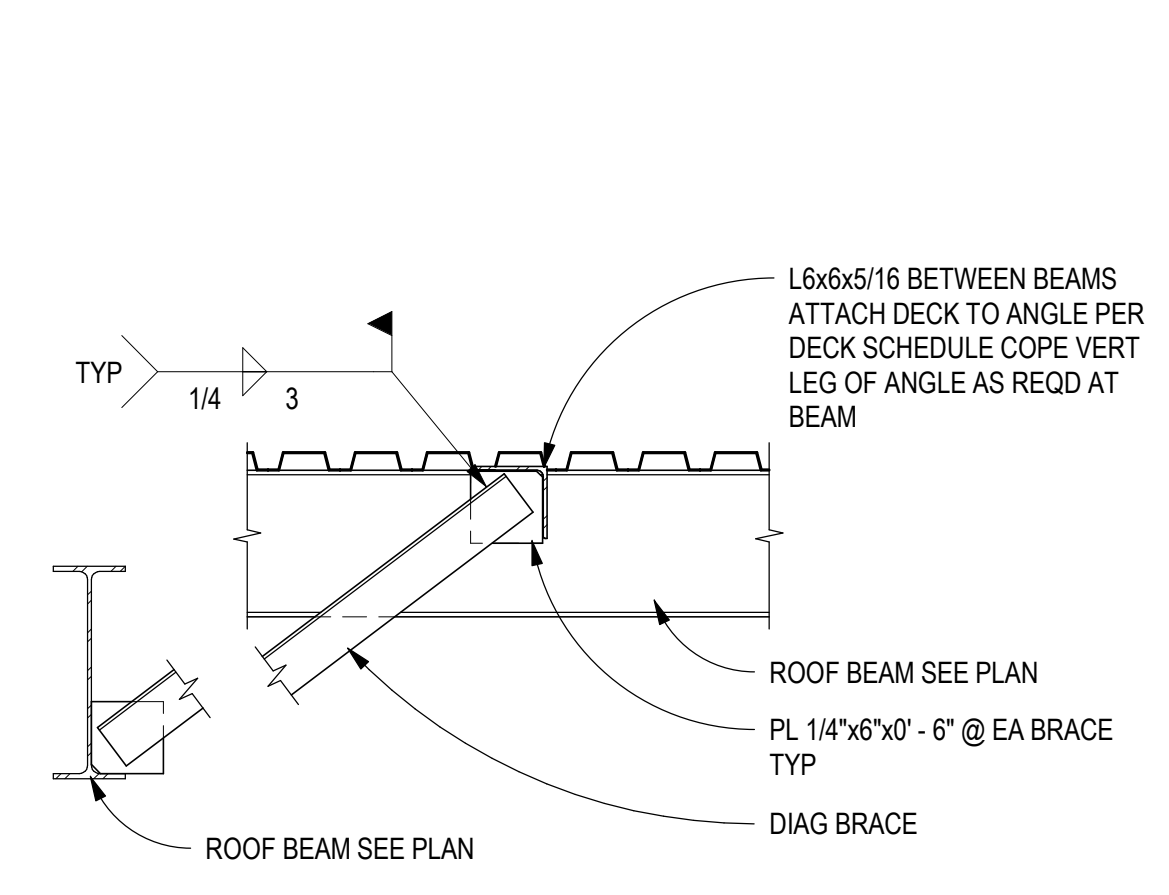
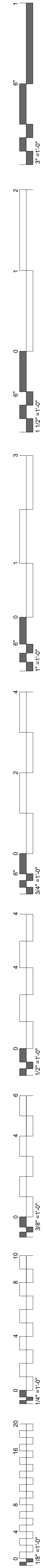
#	DATE	REVISIONS	DESCRIPTION

DATE: 11-01-19 JOB NUMBER: 18-01.01

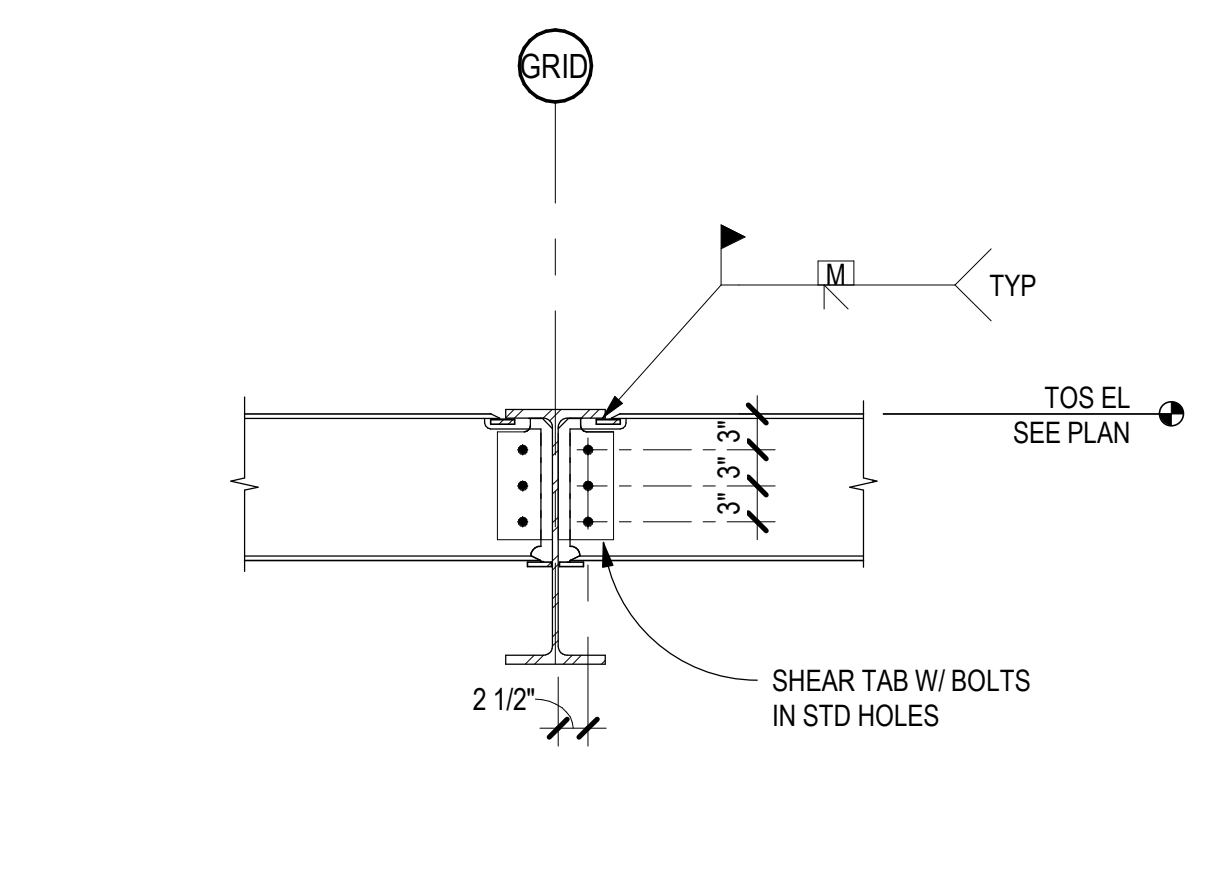
SHEET NUMBER:

S5.41

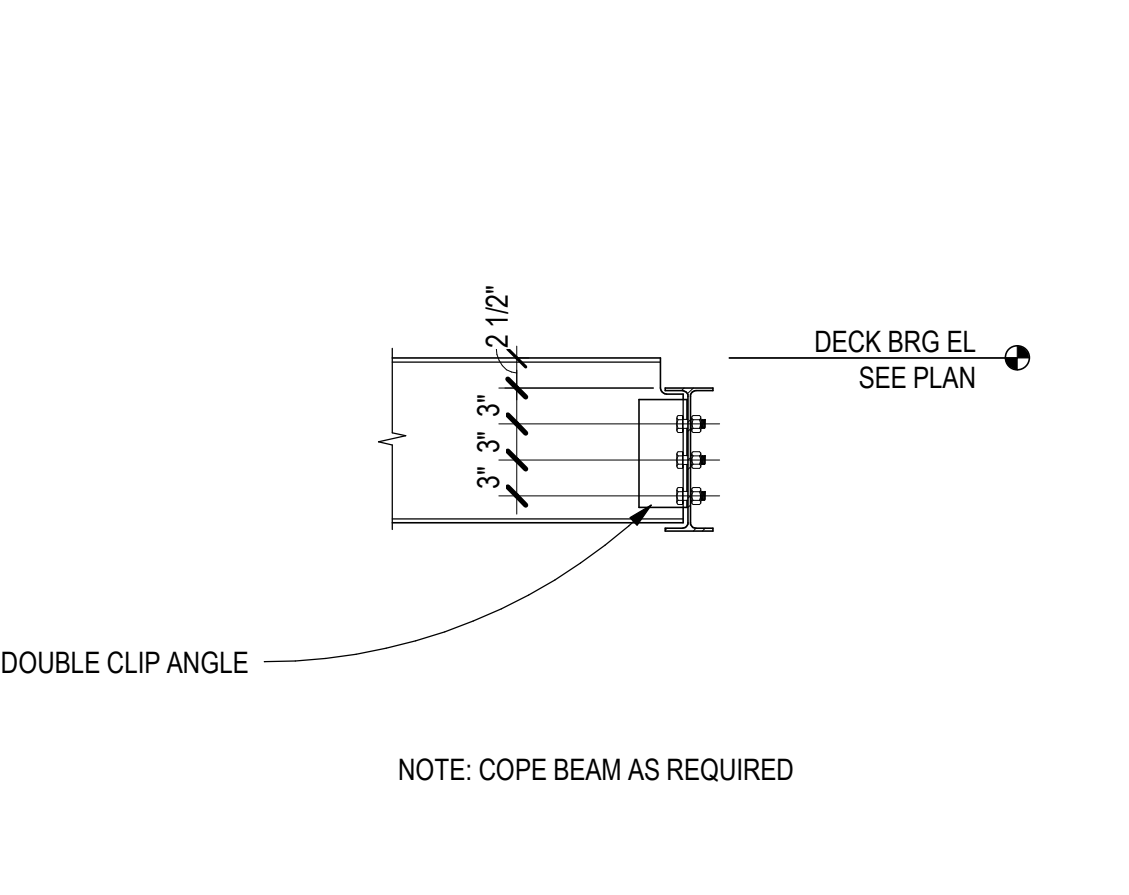
VERTICAL CIRCULATION
DETAILS



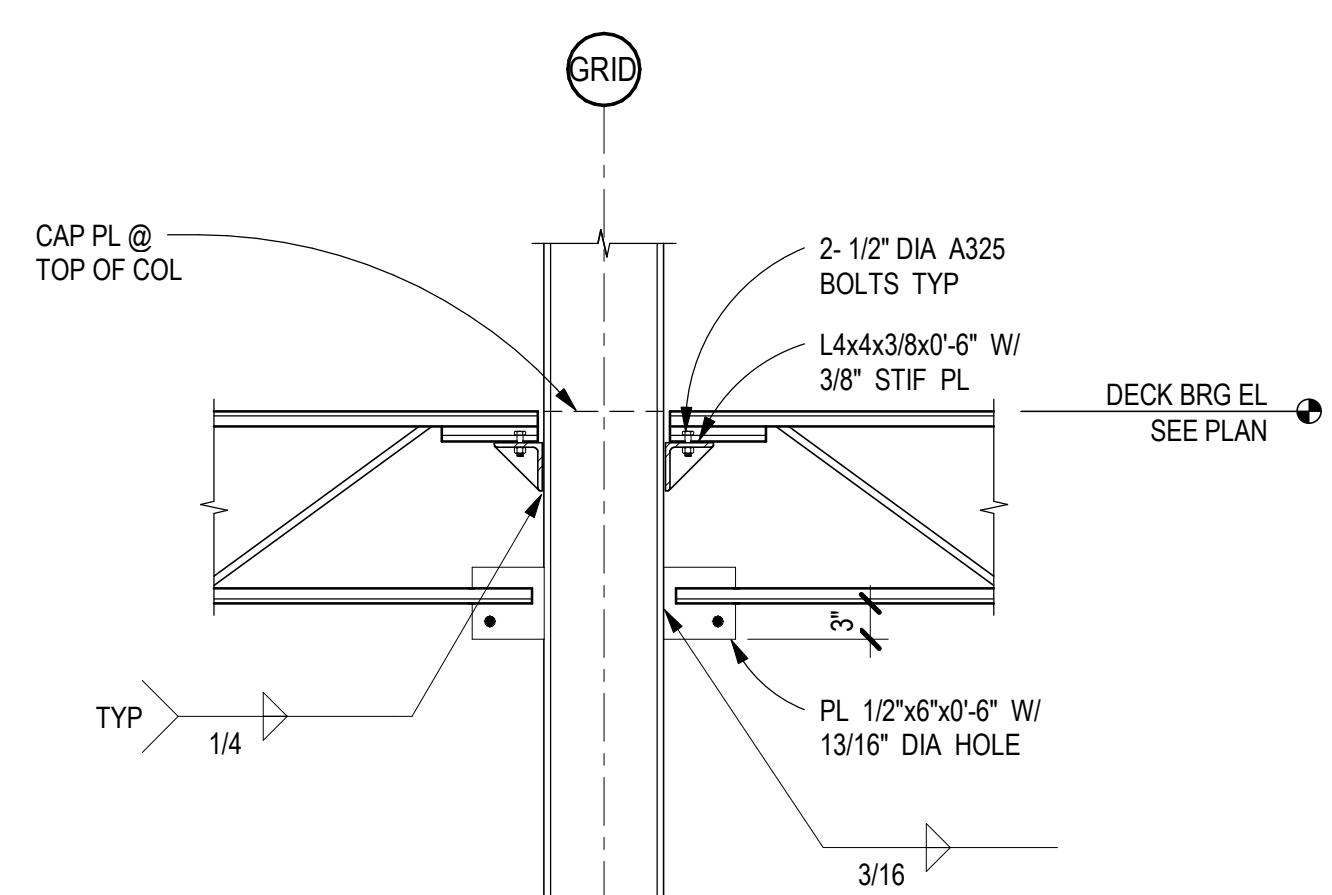
D1 **DIAG BRACE TO ROOF STRUCTURE**
SCALE: 3/4" = 1'-0"



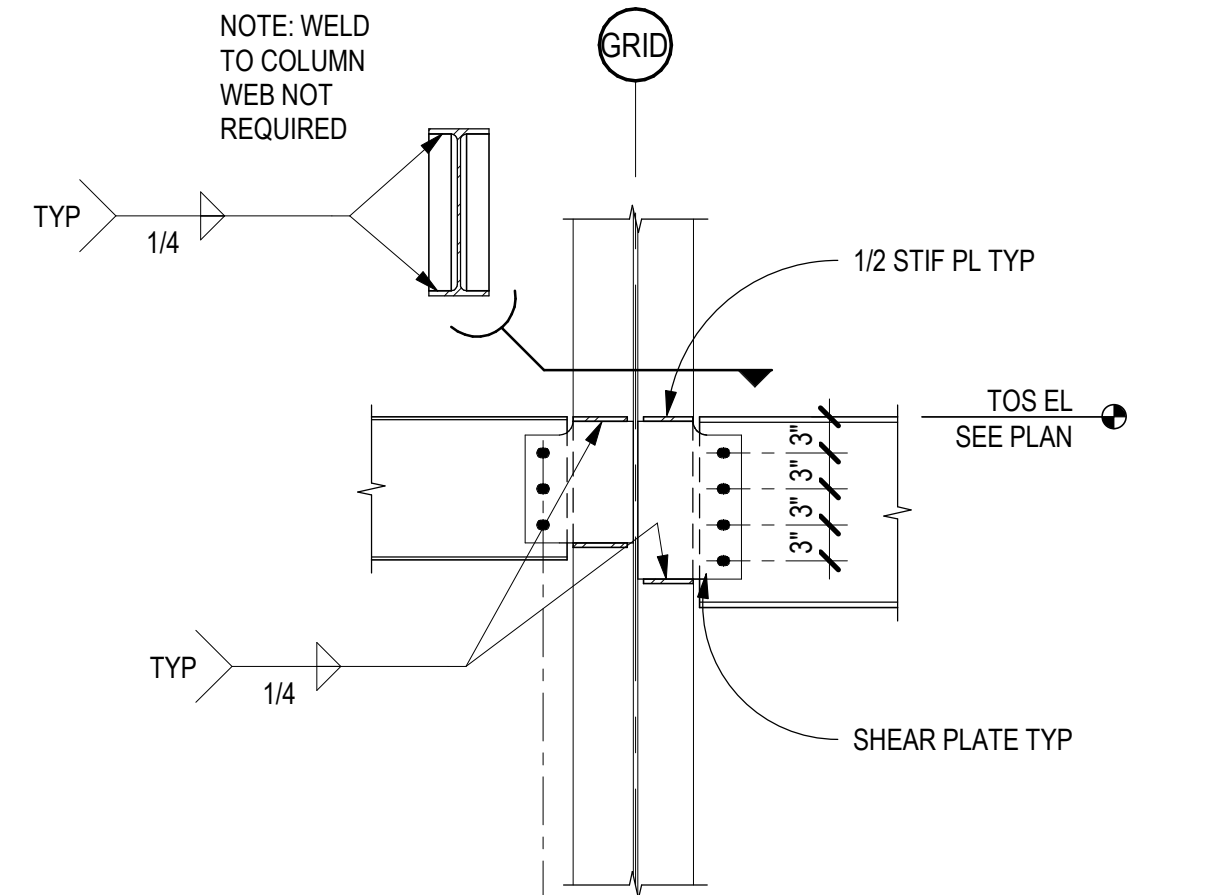
D2 **BEAM TO BEAM MOMENT CONN**
SCALE: 3/4" = 1'-0"



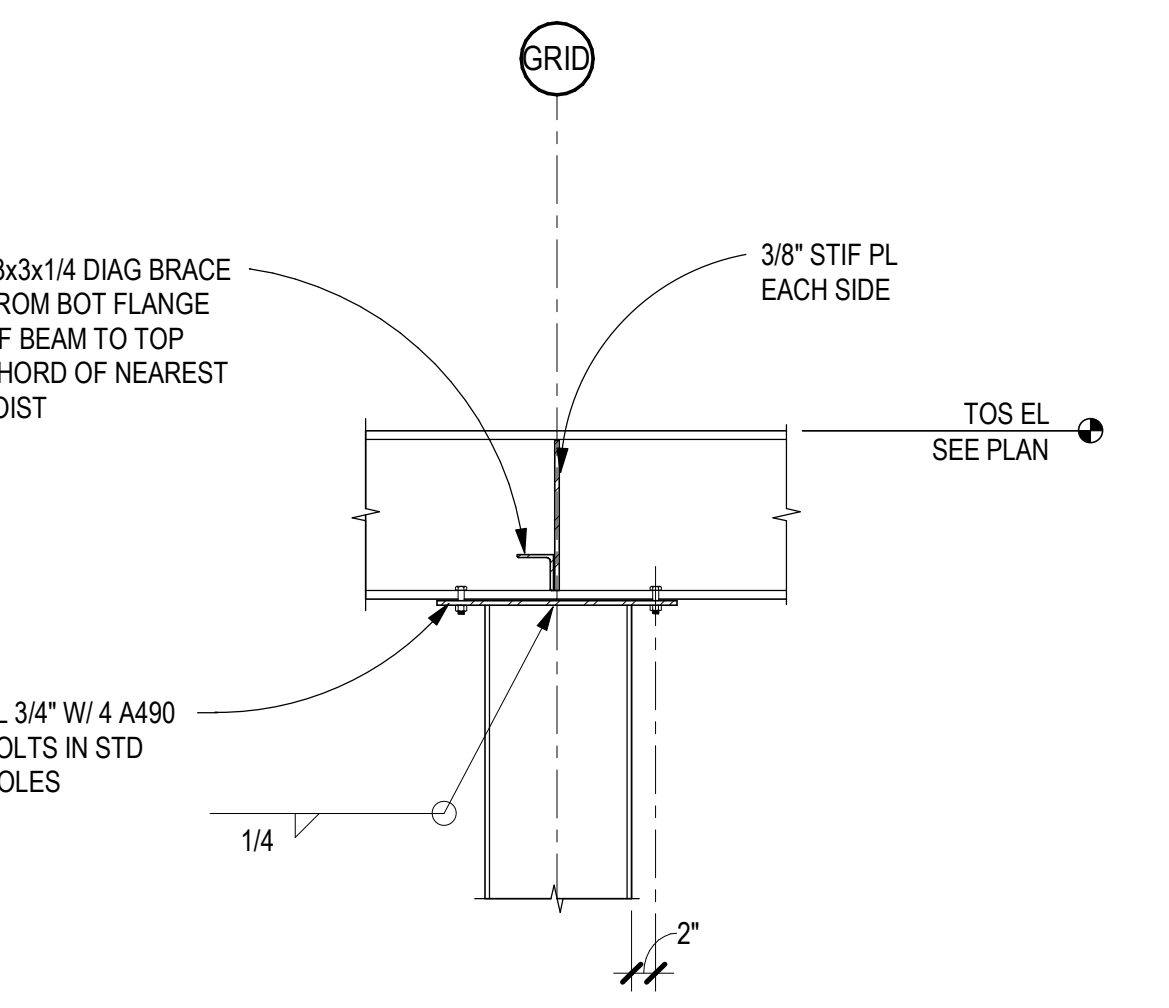
D3 **BEAM TO BEAM CONN**
SCALE: 3/4" = 1'-0"



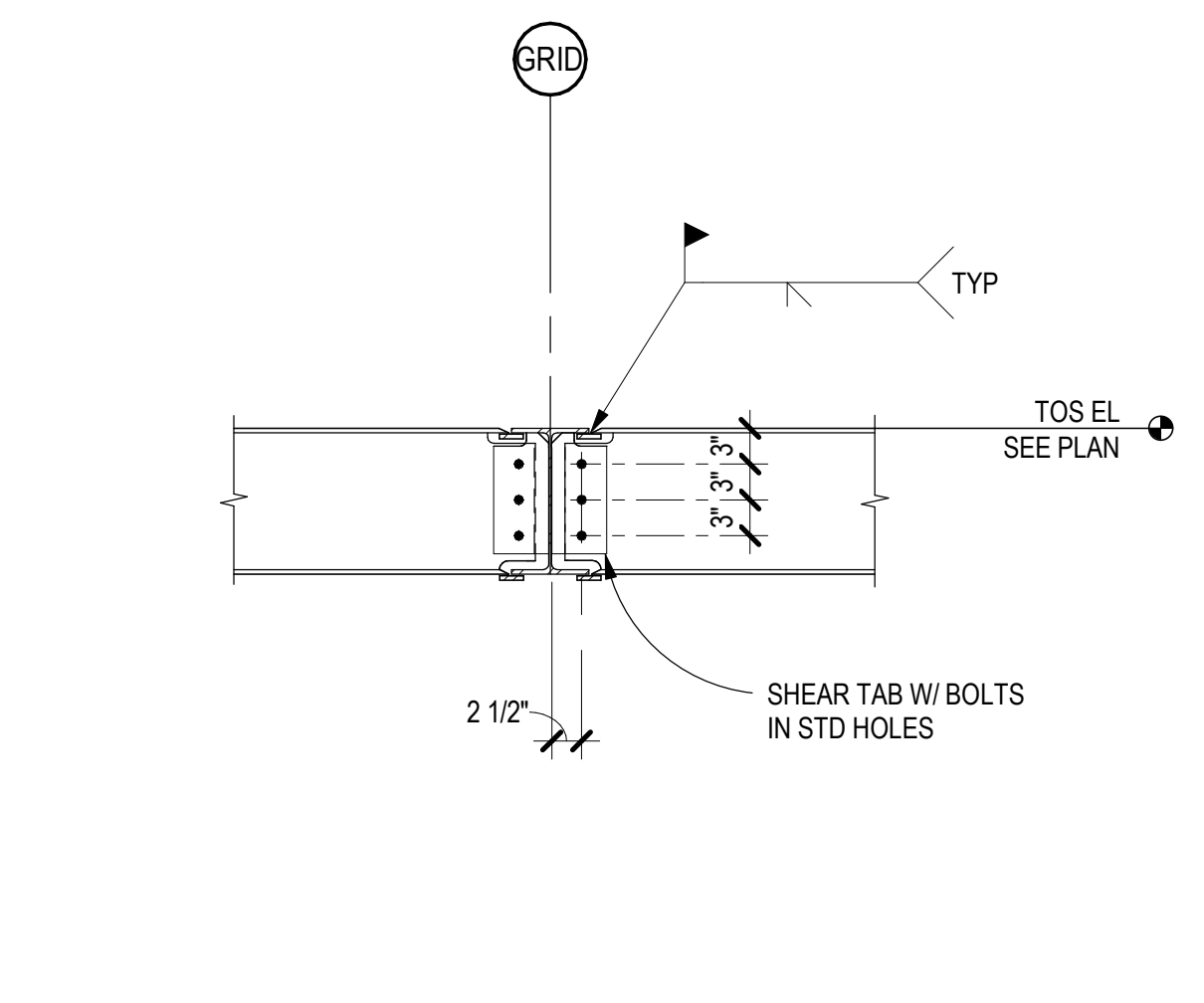
D4 **JOIST TO COLUMN FLANGE CONN**
SCALE: 3/4" = 1'-0"



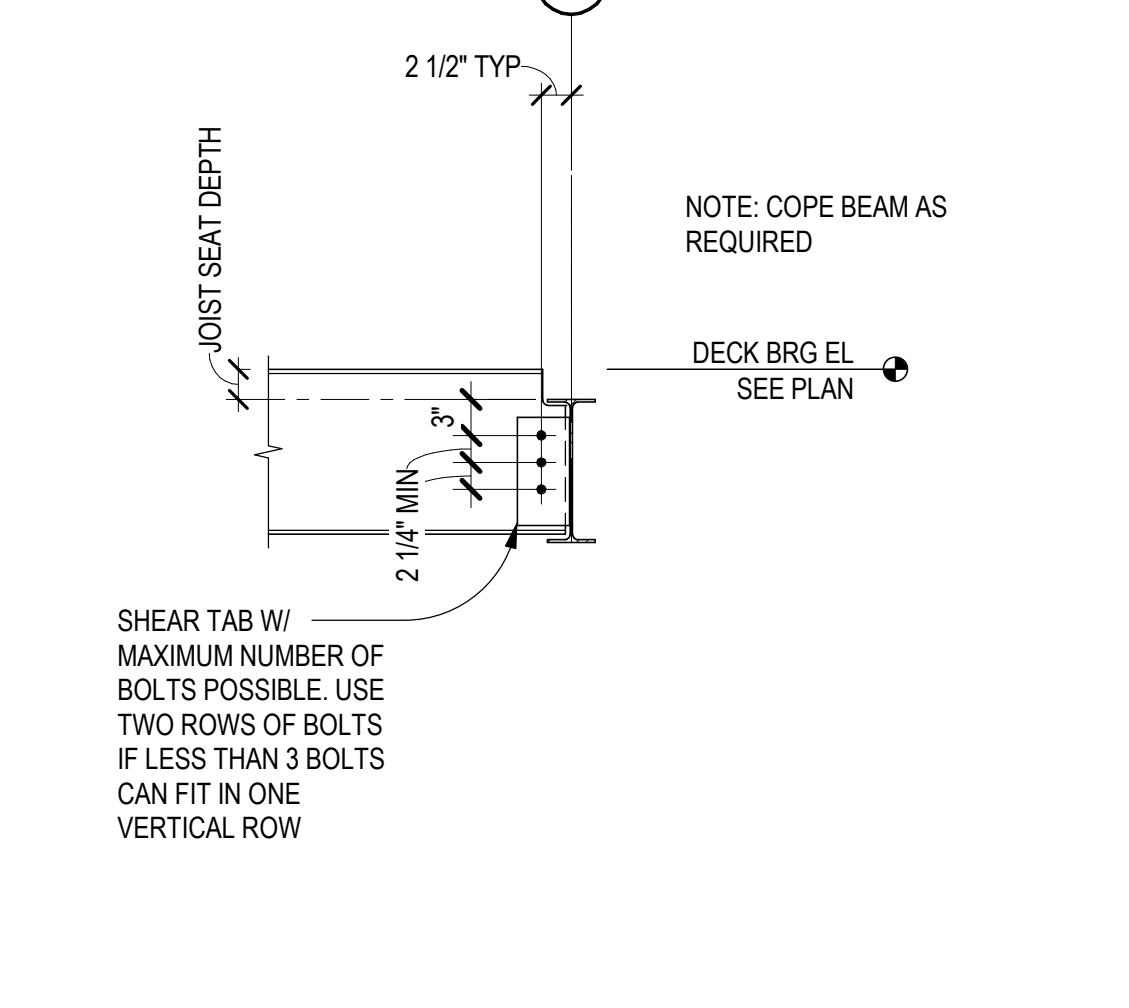
D5 **BEAM TO COLUMN WEB CONN**
SCALE: 3/4" = 1'-0"



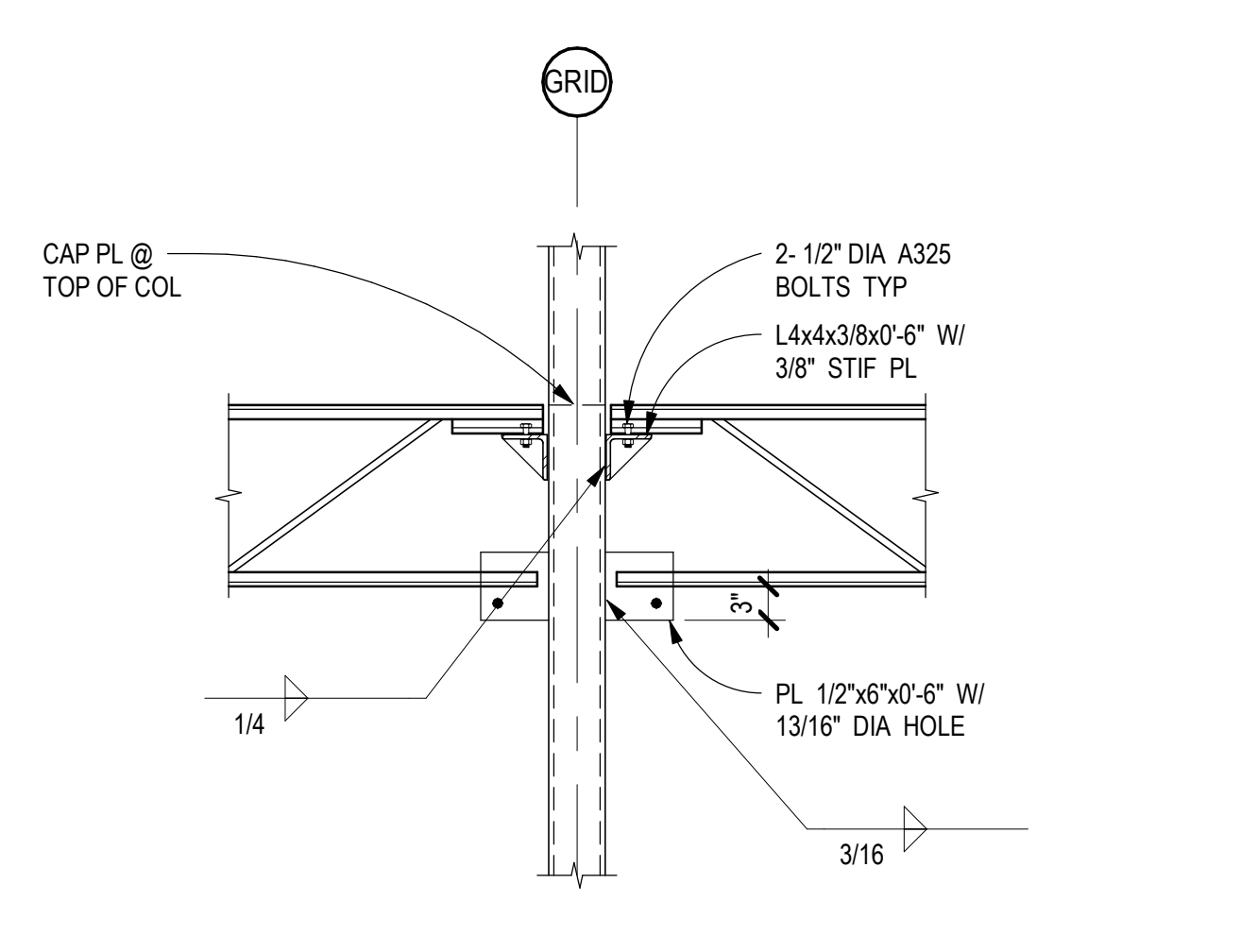
C1 **BEAM OVER COLUMN CONN**
SCALE: 3/4" = 1'-0"



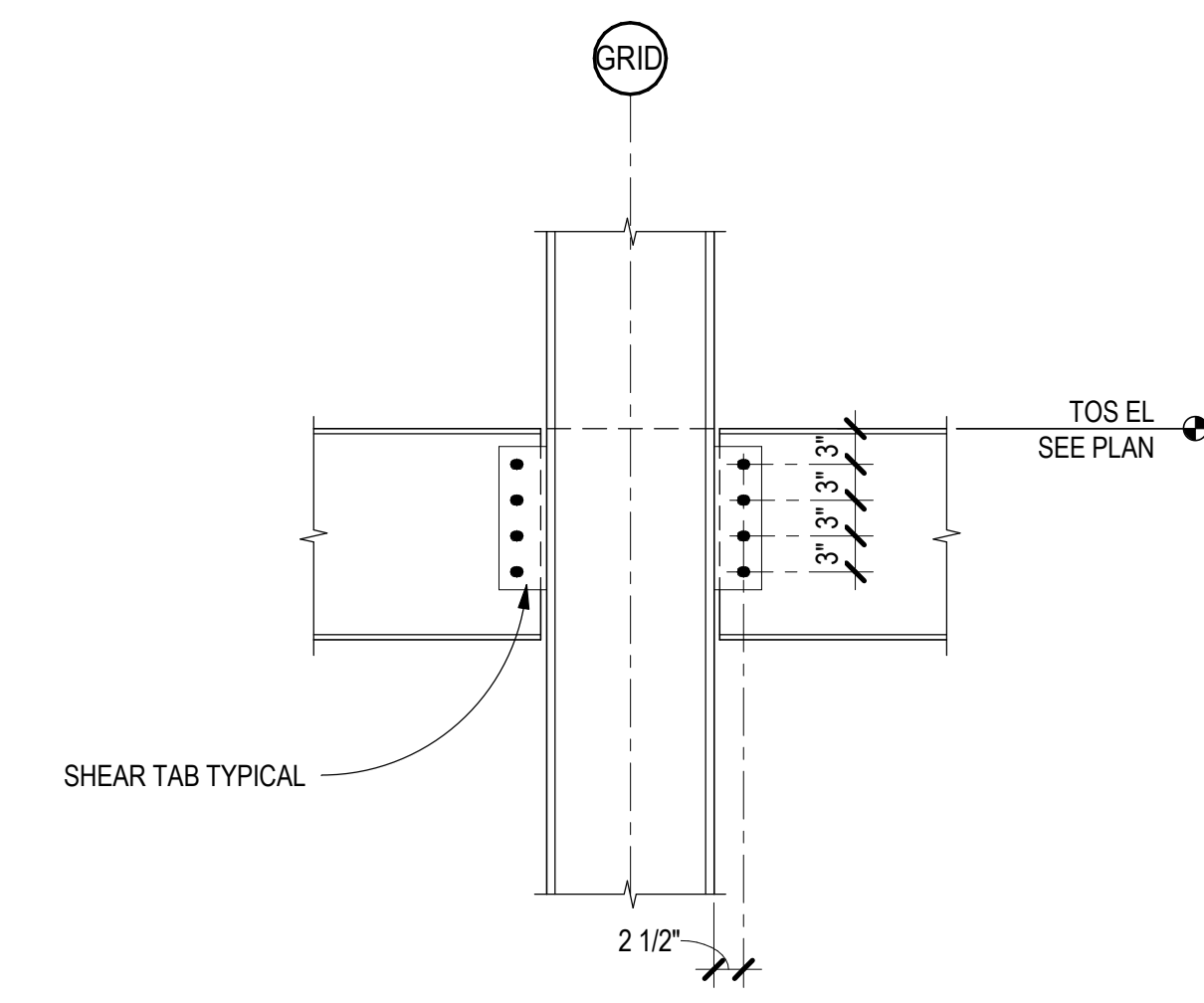
C2 **BEAM TO BEAM MOMENT CONN**
SCALE: 3/4" = 1'-0"



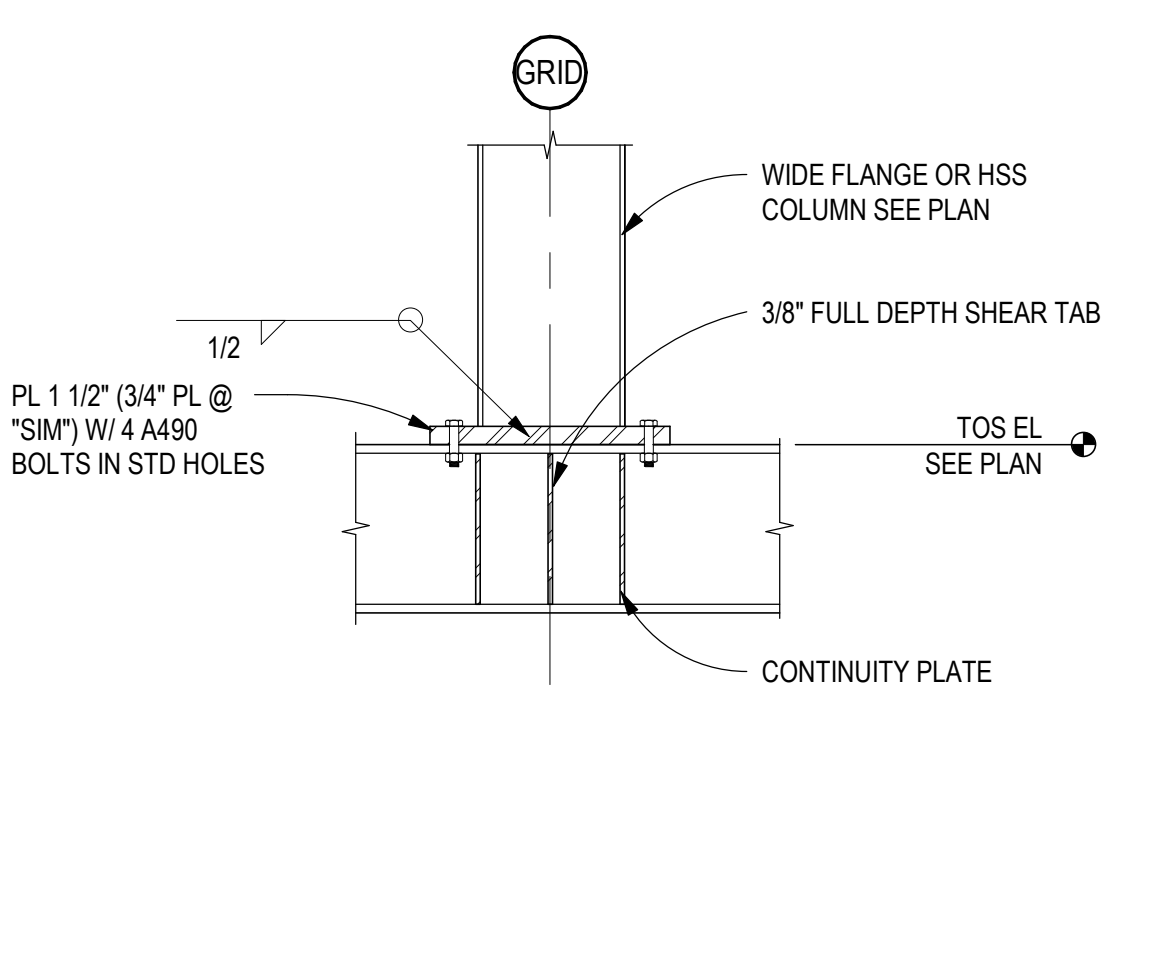
C3 **BEAM TO BEAM CONNECTION**
SCALE: 3/4" = 1'-0"



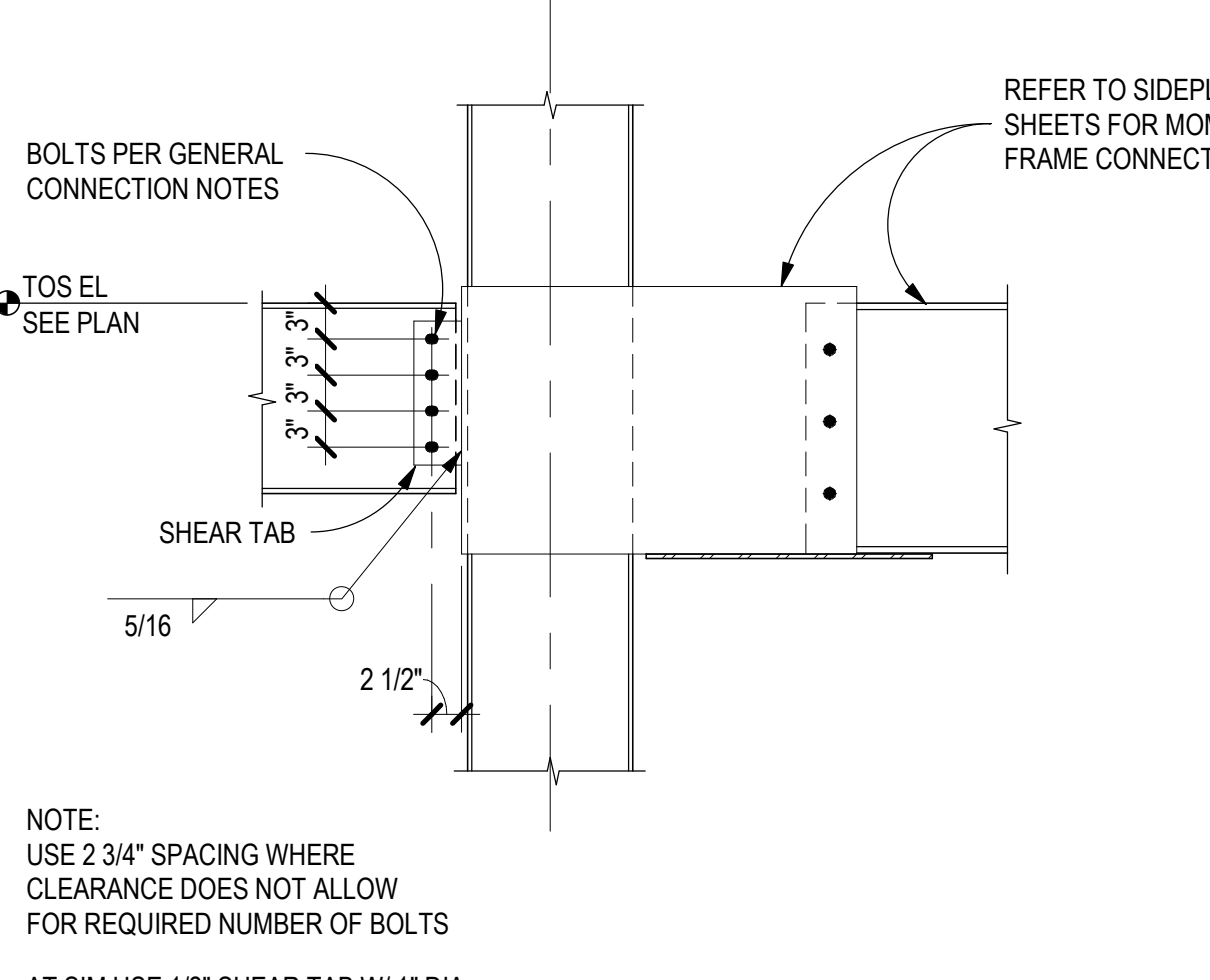
C4 **JOISTS TO COLUMN CONN**
SCALE: 3/4" = 1'-0"



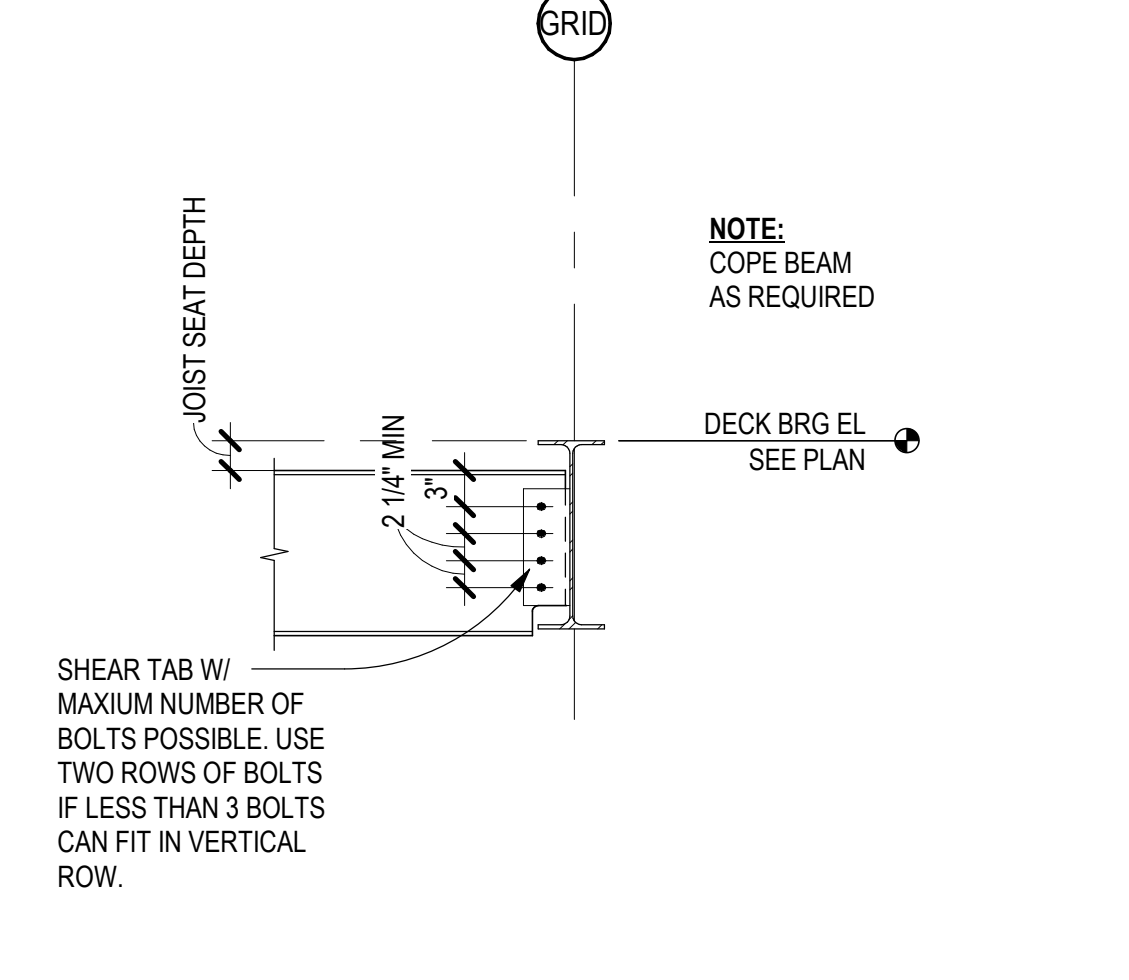
C5 **BEAM TO COLUMN FLANGE CONN**
SCALE: 3/4" = 1'-0"



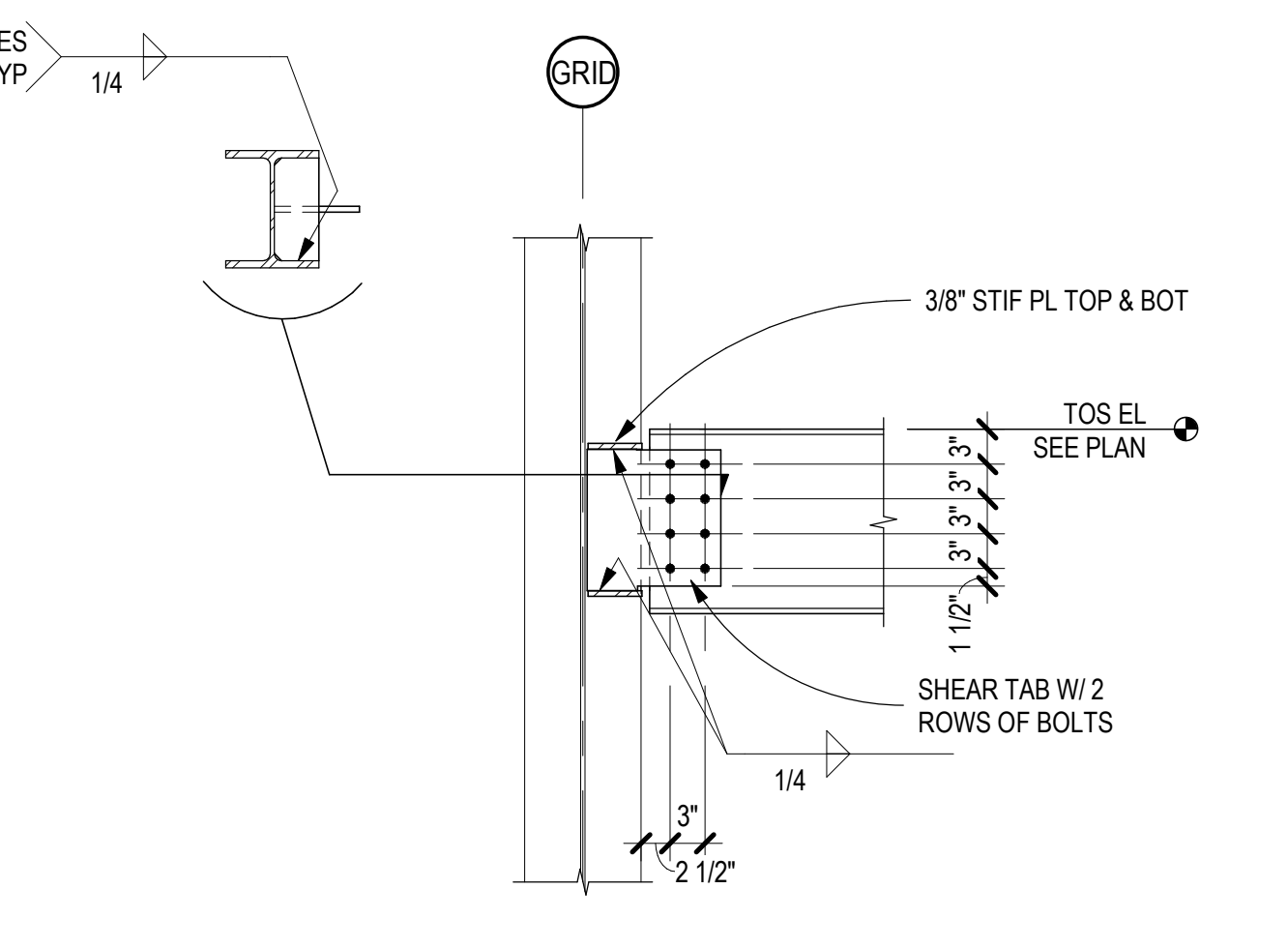
B1 **COL TO TRANSFER GIRDER**
SCALE: 3/4" = 1'-0"



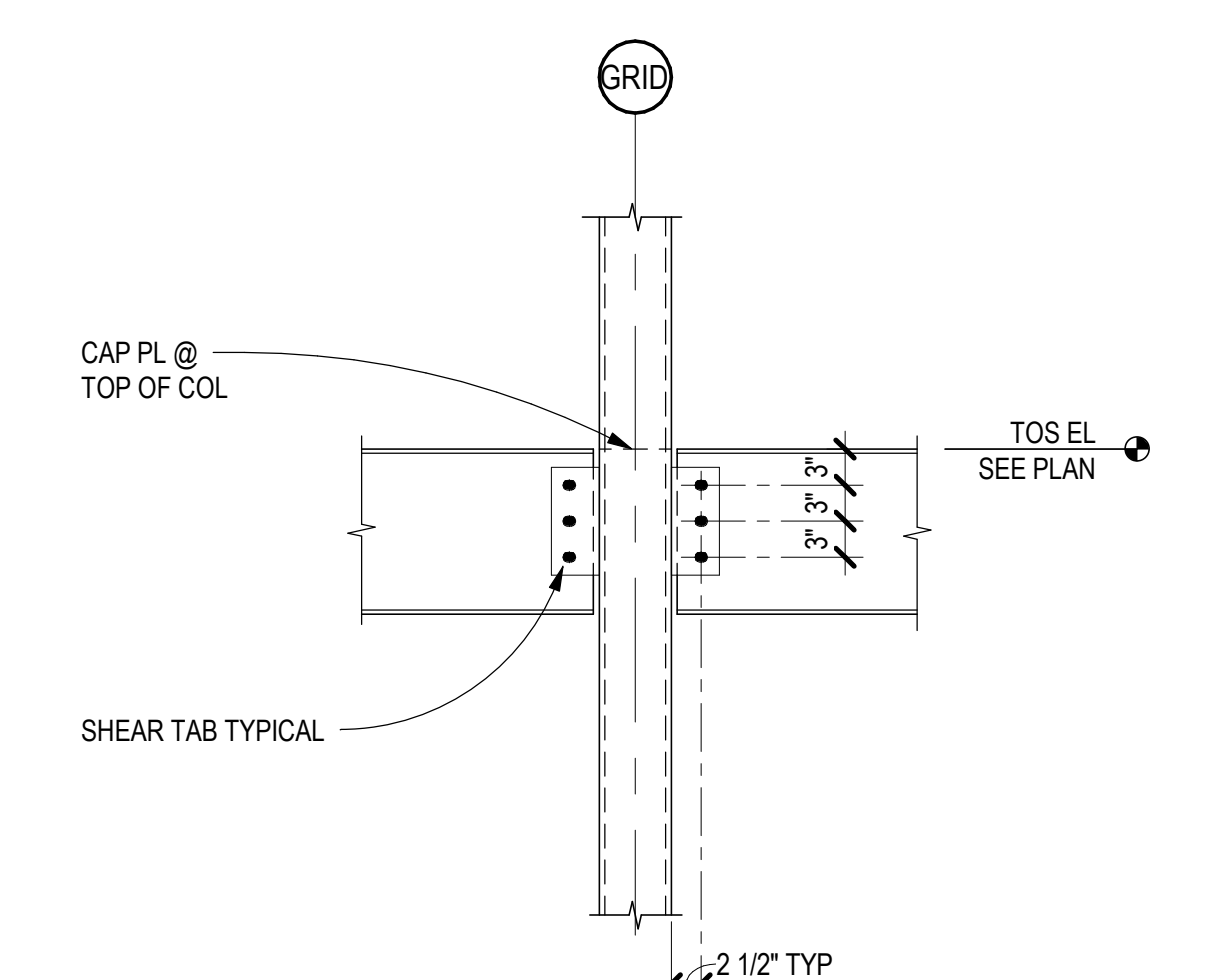
B2 **BM TO COL FLANGE AT SIDEPLATE**
SCALE: 3/4" = 1'-0"



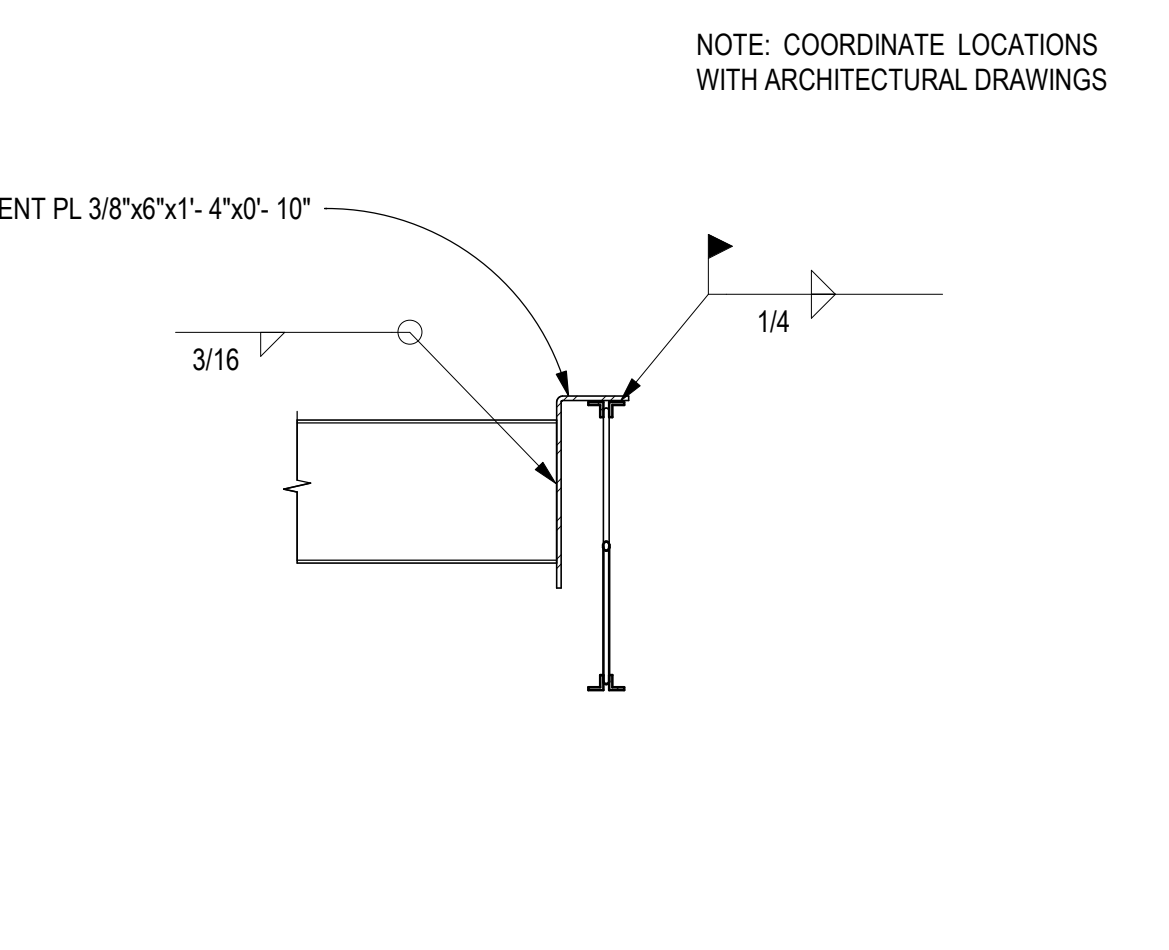
B3 **TYPICAL BEAM TO BEAM CONN**
SCALE: 3/4" = 1'-0"



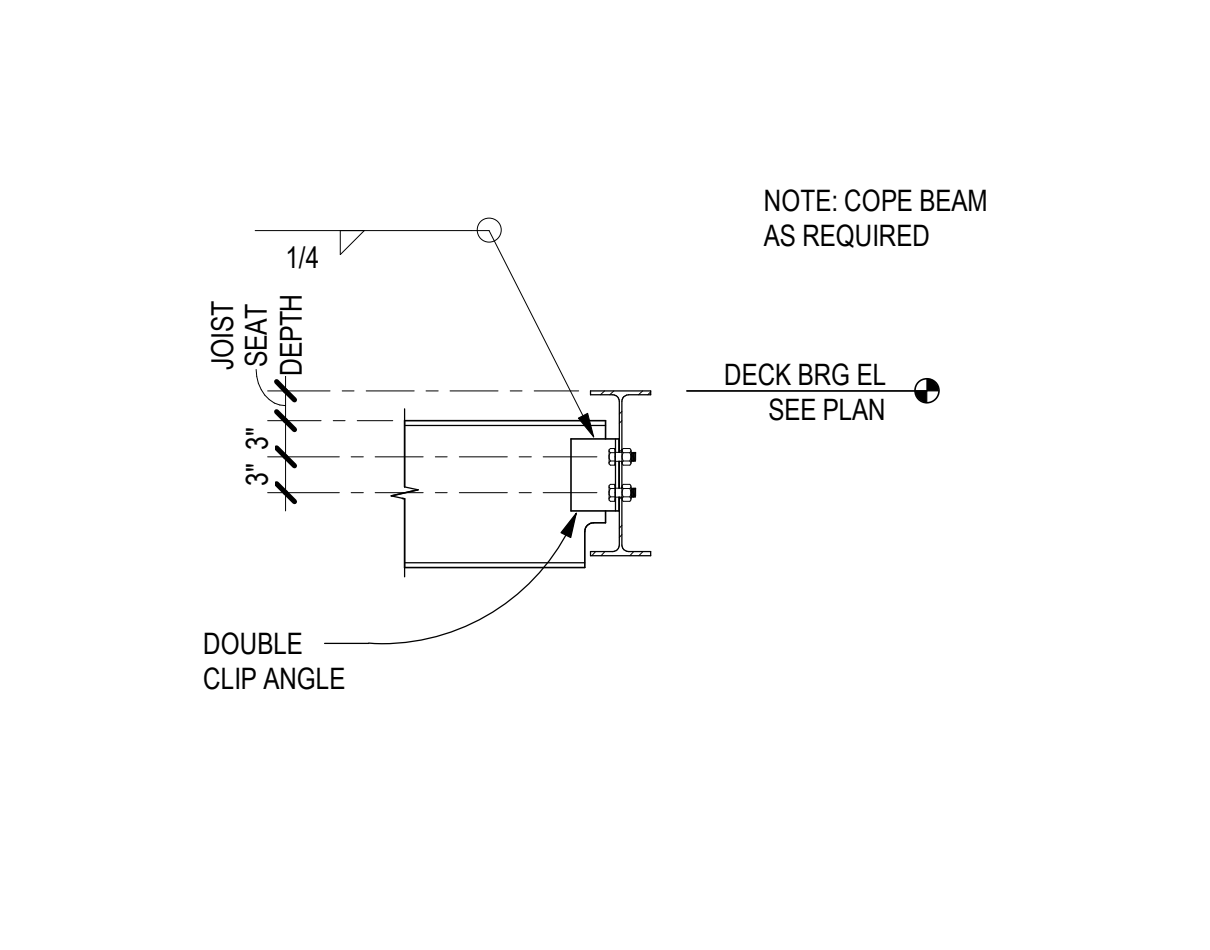
B4 **BEAM TO COL - 2 ROWS OF BOLTS**
SCALE: 3/4" = 1'-0"



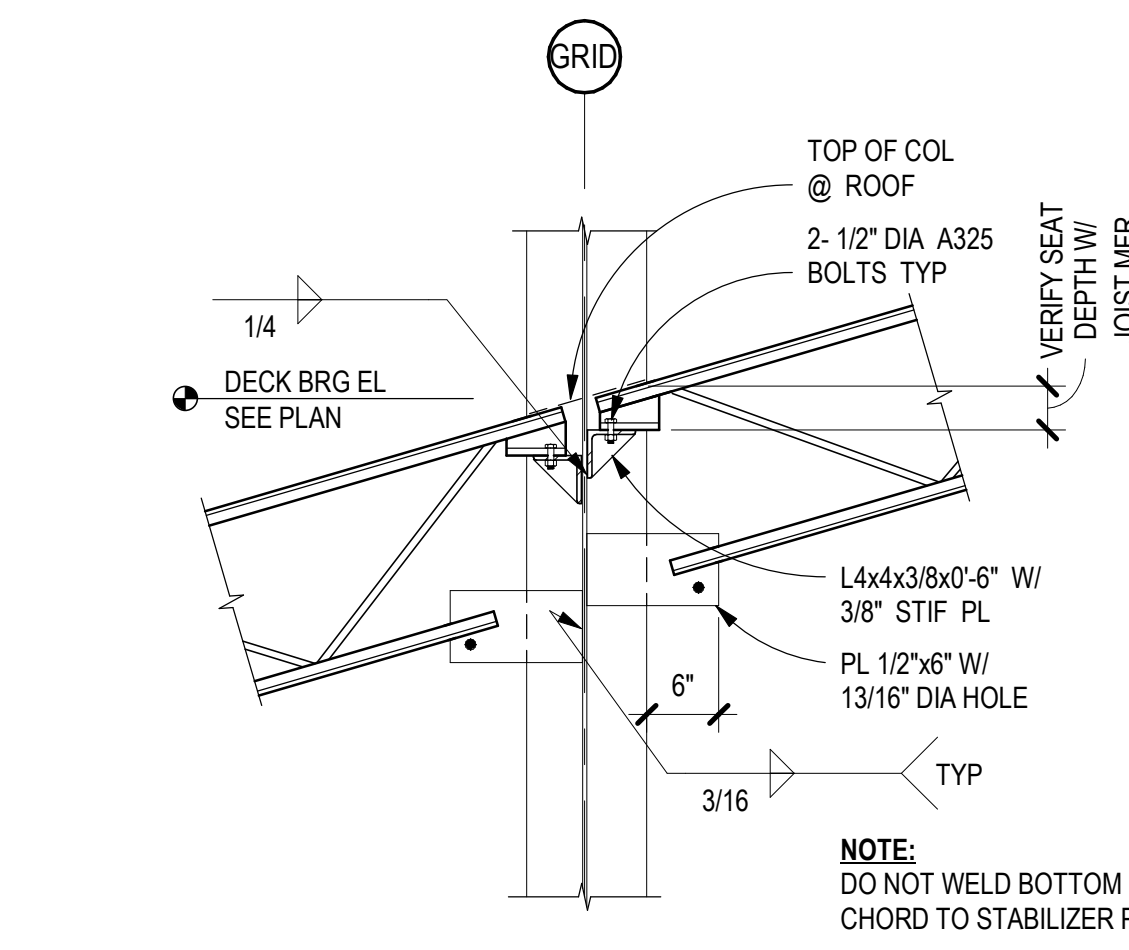
B5 **BEAM TO COLUMN CONN**
SCALE: 3/4" = 1'-0"



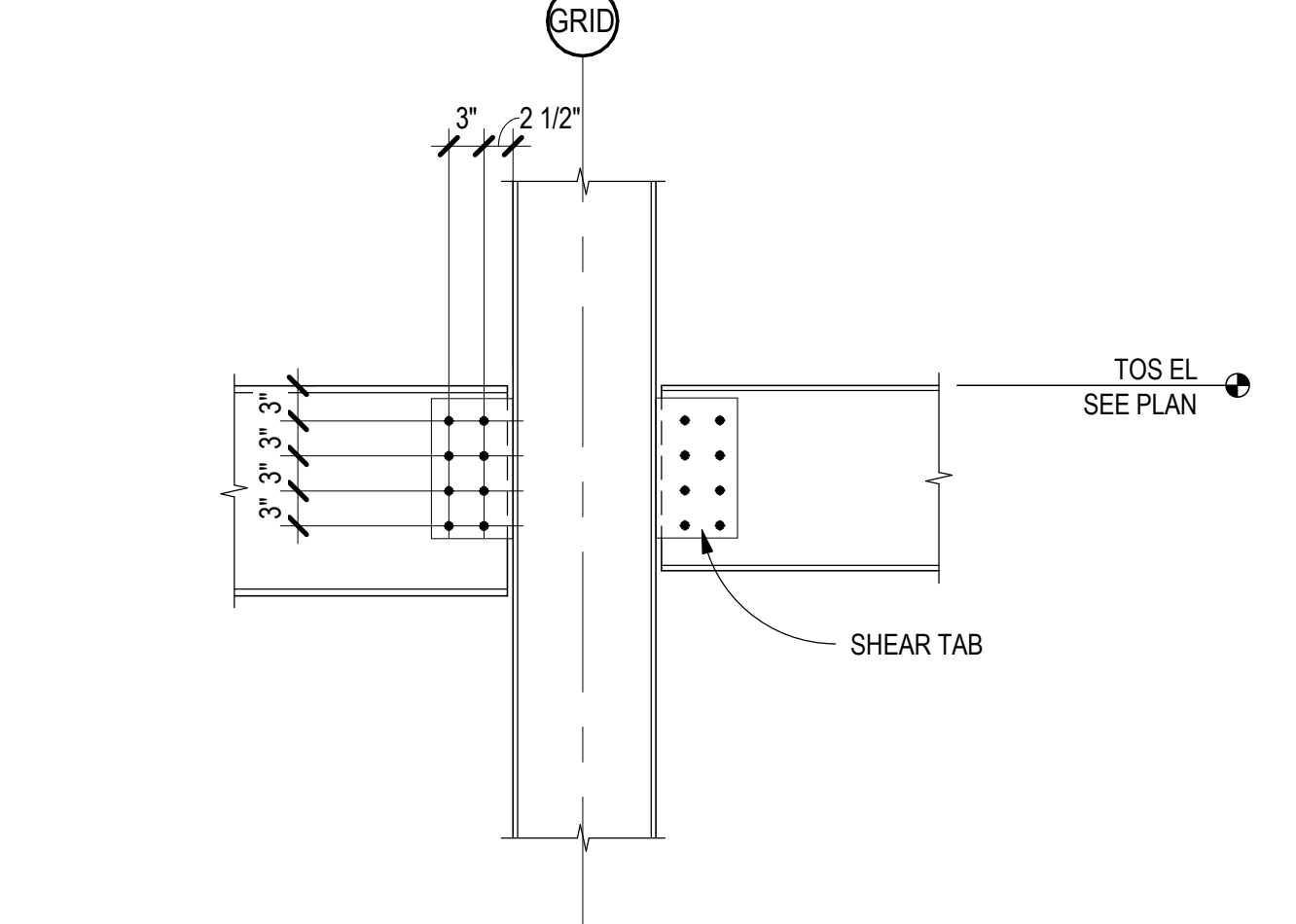
A1 **BEAM TO JOIST CONNECTION**
SCALE: 3/4" = 1'-0"



A2 **BEAM TO BEAM CONN**
SCALE: 3/4" = 1'-0"



A3 **JOIST TO COLUMN CONN**
SCALE: 3/4" = 1'-0"

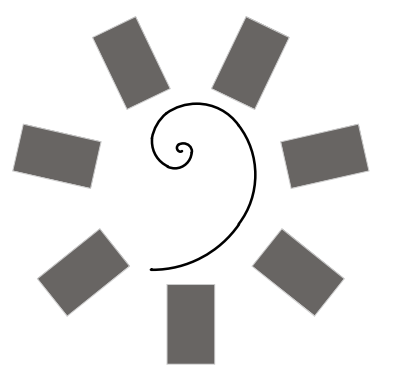


A4 **BEAM TO COL - 2 ROWS OF BOLTS**
SCALE: 3/4" = 1'-0"

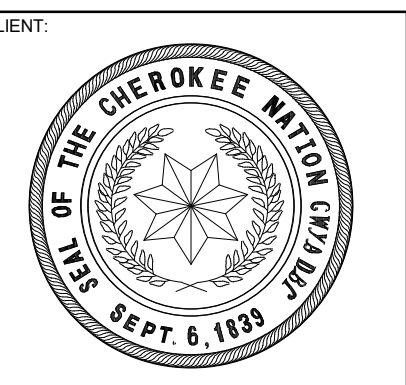
1. PROVIDE 1/4" CAP PLATE AT THE TOP OF ALL HOLLOW STRUCTURAL STEEL (HSS) COLUMNS UNLESS NOTED OTHERWISE.
2. PROVIDE 1/4" CAP PLATE AT THE TOP OF ALL PIPE COLUMNS UNLESS NOTED OTHERWISE.
3. PROVIDE 1/2" CAP PLATE AT THE TOP OF ALL WIDE FLANGE COLUMNS UNLESS NOTED OTHERWISE.
4. PROVIDE 1/4" END PLATE AT ALL EXPOSED HSS MEMBERS UNLESS NOTED OTHERWISE.
5. ALL CONNECTION CLIP ANGLES SHALL BE L4x4x3/8 UNLESS NOTED OTHERWISE. BOLT ANGLE TO SUPPORTING MEMBER UNLESS NOTED OTHERWISE.
6. ALL CONNECTION SHEAR TABS SHALL BE PL 3/8" UNLESS NOTED OTHERWISE. PROVIDE 5/16" FILLET WELD EACH SIDE OF SHEAR TAB TO SUPPORTING MEMBER.
7. ALL BOLTS SHALL BE 3/4" DIAMETER A325N IN SHORT SLOTS AS FOLLOWS UNLESS NOTED OTHERWISE:
2 @ W8, W10
3 @ W12
4 @ W14, W16
5 @ W18, S21
6 @ W24
7 @ W27
8 @ W30
9 @ W33
10 @ W36
11 @ W40
12 @ W44

A5 **TYPICAL STEEL CONN NOTES**
SCALE: 3/4" = 1'-0"

#	DATE	REVISIONS	DESCRIPTION



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STILWELL, OKLAHOMA

KEY PLAN:

PROJECT PHASE:
BID PACKAGE 01

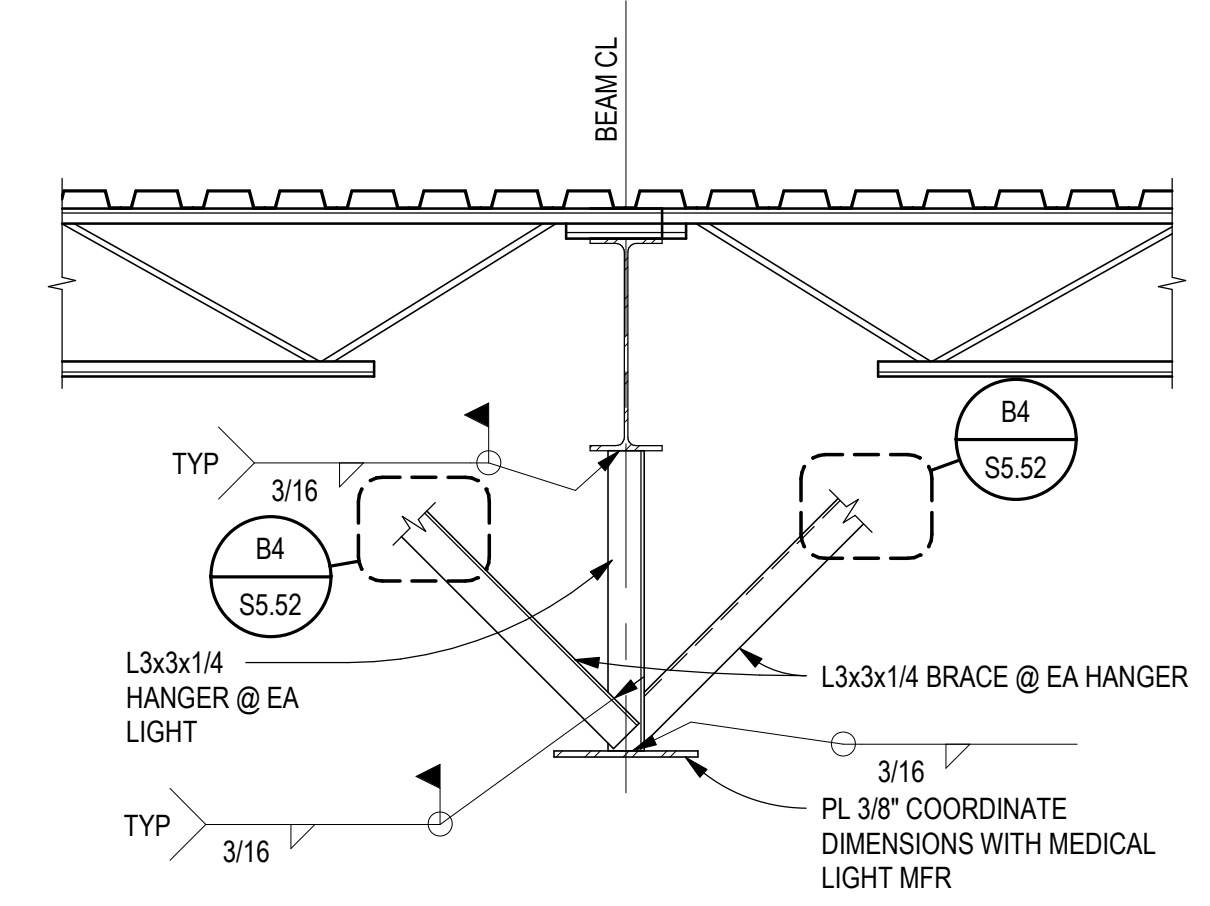
#	DATE	REVISIONS	DESCRIPTION

DATE: 11-01-19 JOB NUMBER: 18-01.01

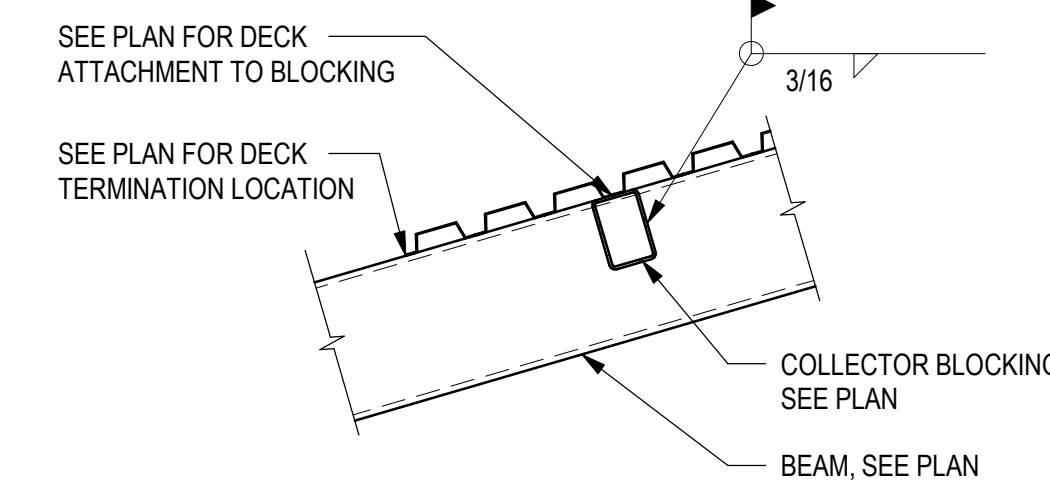
SHEET NUMBER:

S5.52

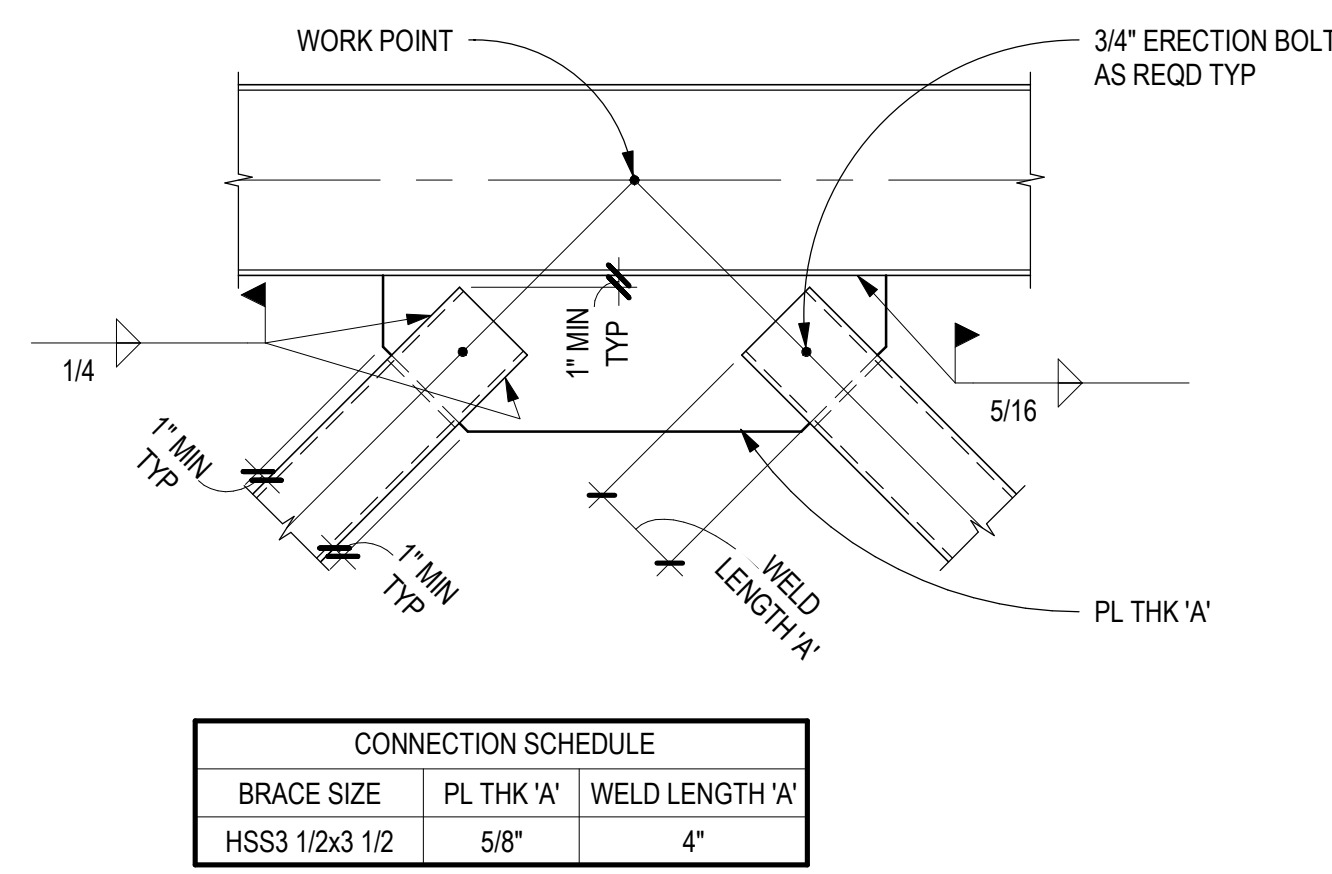
STEEL DETAILS



D5 MEDICAL LIGHT SUPPORT
SCALE: 3/4" = 1'-0"



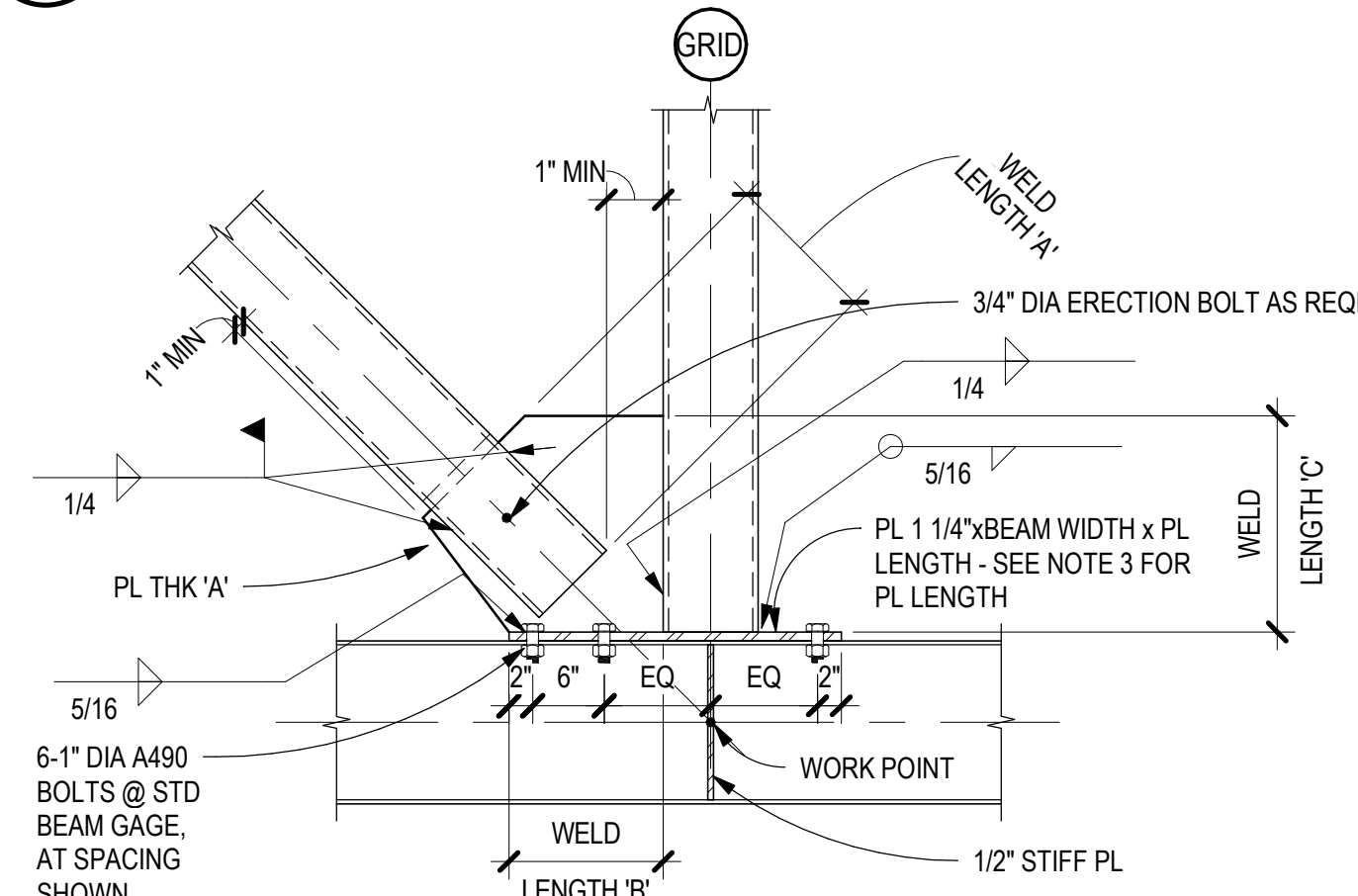
D4 CANOPY COLLECTOR BLOCKING
SCALE: 3/4" = 1'-0"



BRACE SIZE	PL THK 'A'	WELD LENGTH 'A'
HSS3 1/2x3 1/2	5/8"	4"

NOTES
1. LENGTHS GIVEN ARE SINGLE-SIDE LENGTHS AND MINIMUM REQUIRED LENGTHS
2. LONGER GUSSET/WELD LENGTHS MAY BE REQ'D WHERE BRACE SLOPE VARIES FROM 1:1
3. BRACE AND GUSSET CONNECTION CAN OCCUR ON TOP AND BOTTOM FLANGES

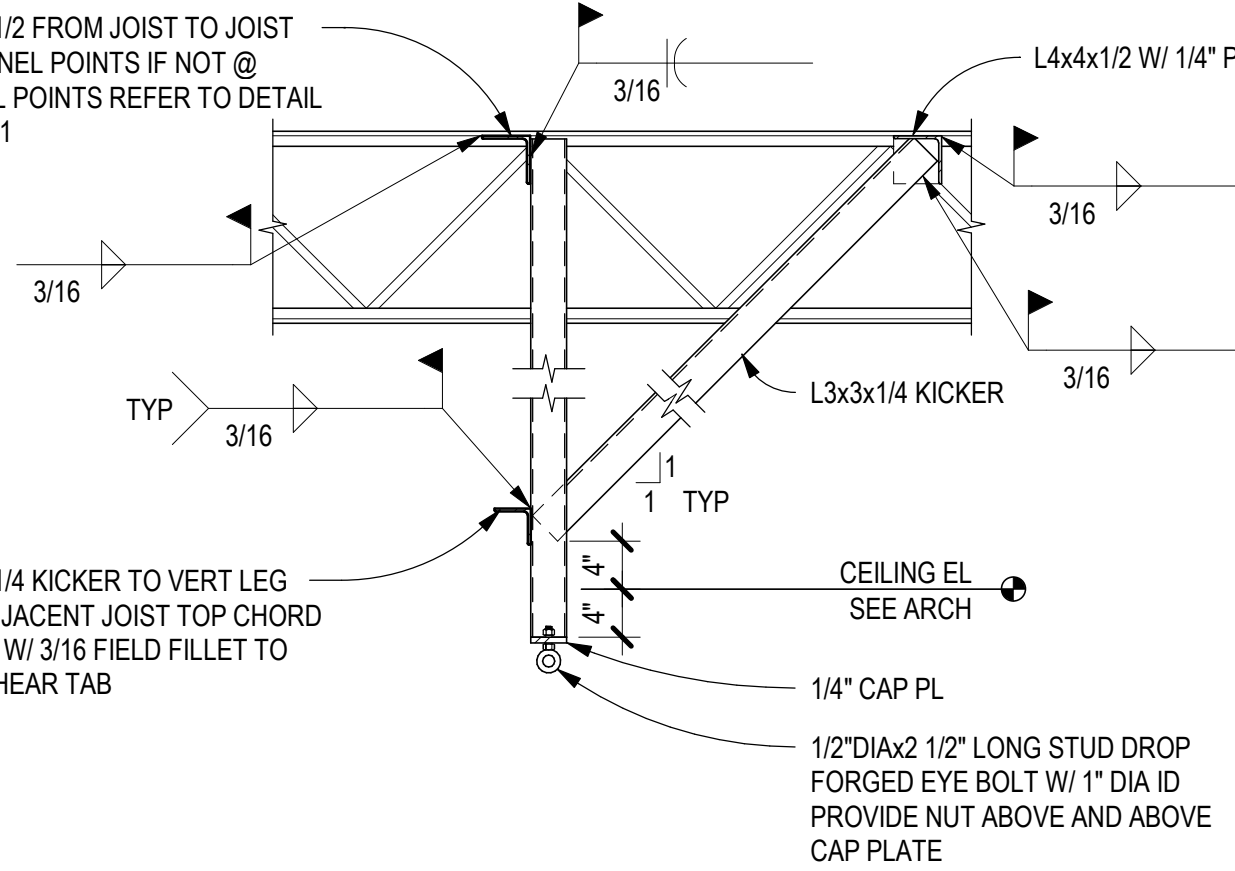
D3 BRACED FRAME CONNECTION
SCALE: 3/4" = 1'-0"



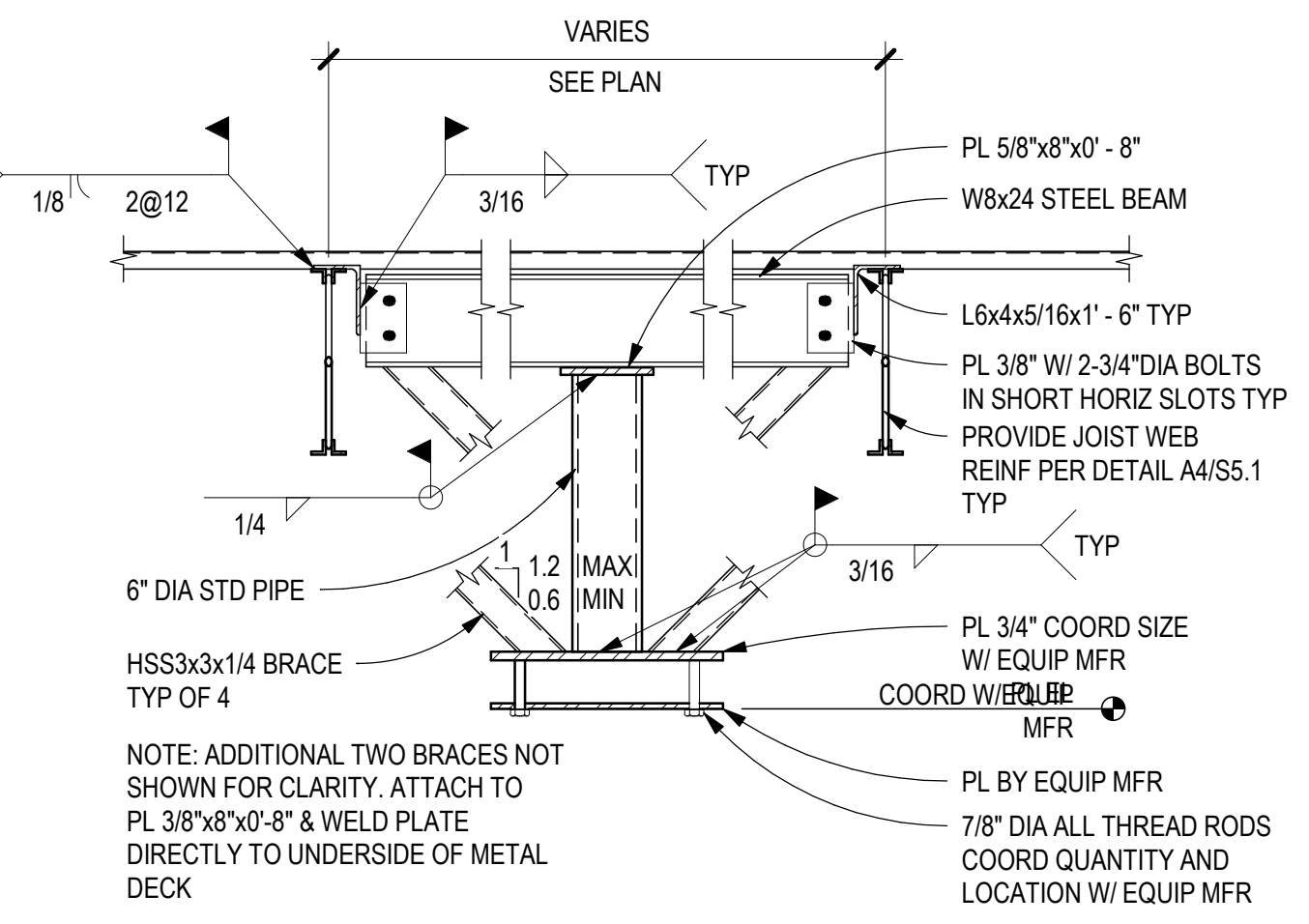
BRACE SIZE	PL THK 'A'	WELD LENGTH 'A'	WELD LENGTH 'B'	WELD LENGTH 'C'
HSS3 1/2x3 1/2	5/8"	4"	12"	12"
HSS5x5	1/2"	5"	12"	12"

NOTES
1. LENGTHS GIVEN ARE SINGLE-SIDE LENGTHS AND MINIMUM LENGTHS
2. LONGER GUSSET/WELD LENGTHS MAY BE REQ'D WHERE BRACE SLOPE VARIES FROM 1:1
3. PLATE LENGTHS: BASED ON POST SIZE
A. HSS5x5 & HSS5x6: 2' - 0"

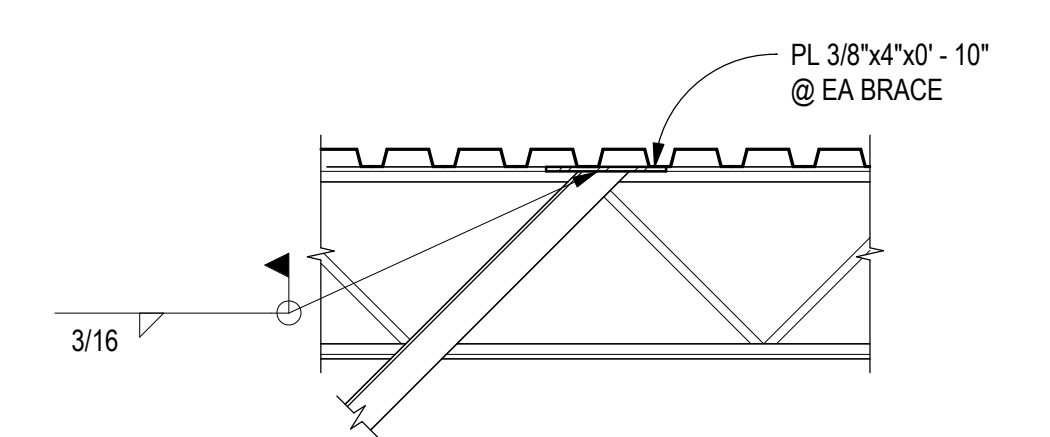
C3 BRACED FRAME CONNECTION
SCALE: 3/4" = 1'-0"



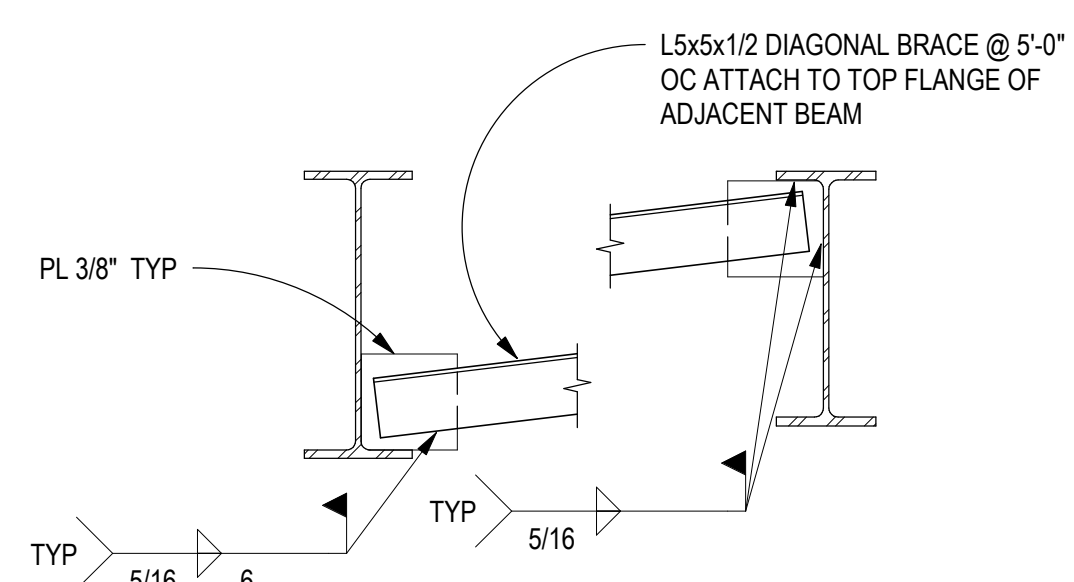
C4 SWING SUPPORT BETWEEN JOISTS
SCALE: 3/4" = 1'-0"



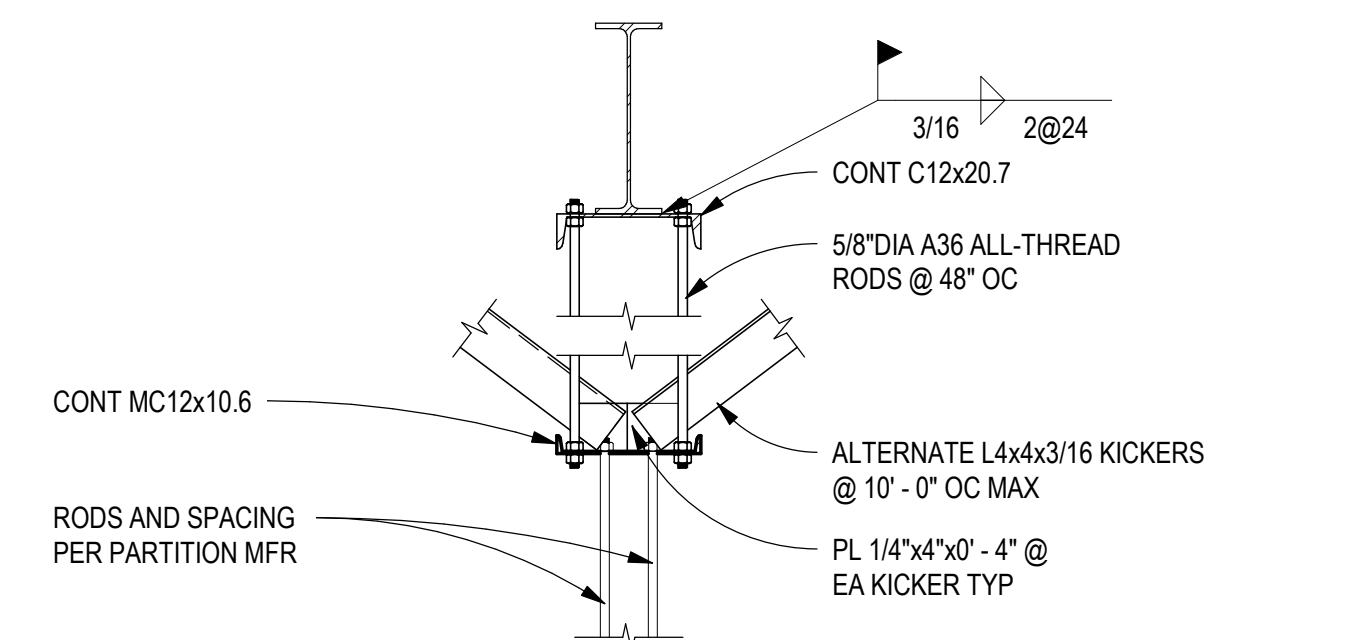
C5 SURGICAL LIGHT SUPPORT
SCALE: 3/4" = 1'-0"



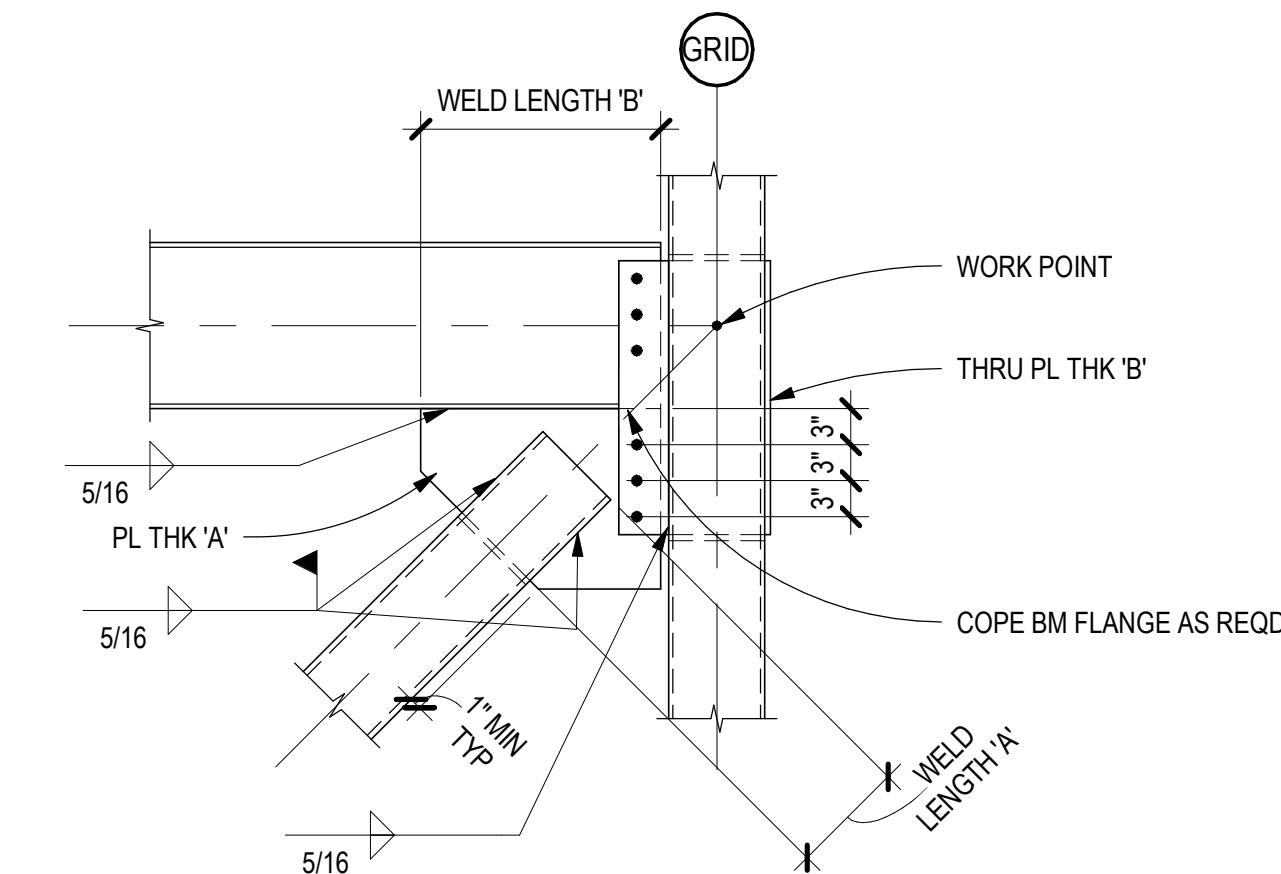
B4 LIGHT SUPPORT BETWEEN JOISTS
SCALE: 3/4" = 1'-0"



B3 DIAG ANGLE AT MOMENT CONN
SCALE: 3/4" = 1'-0"



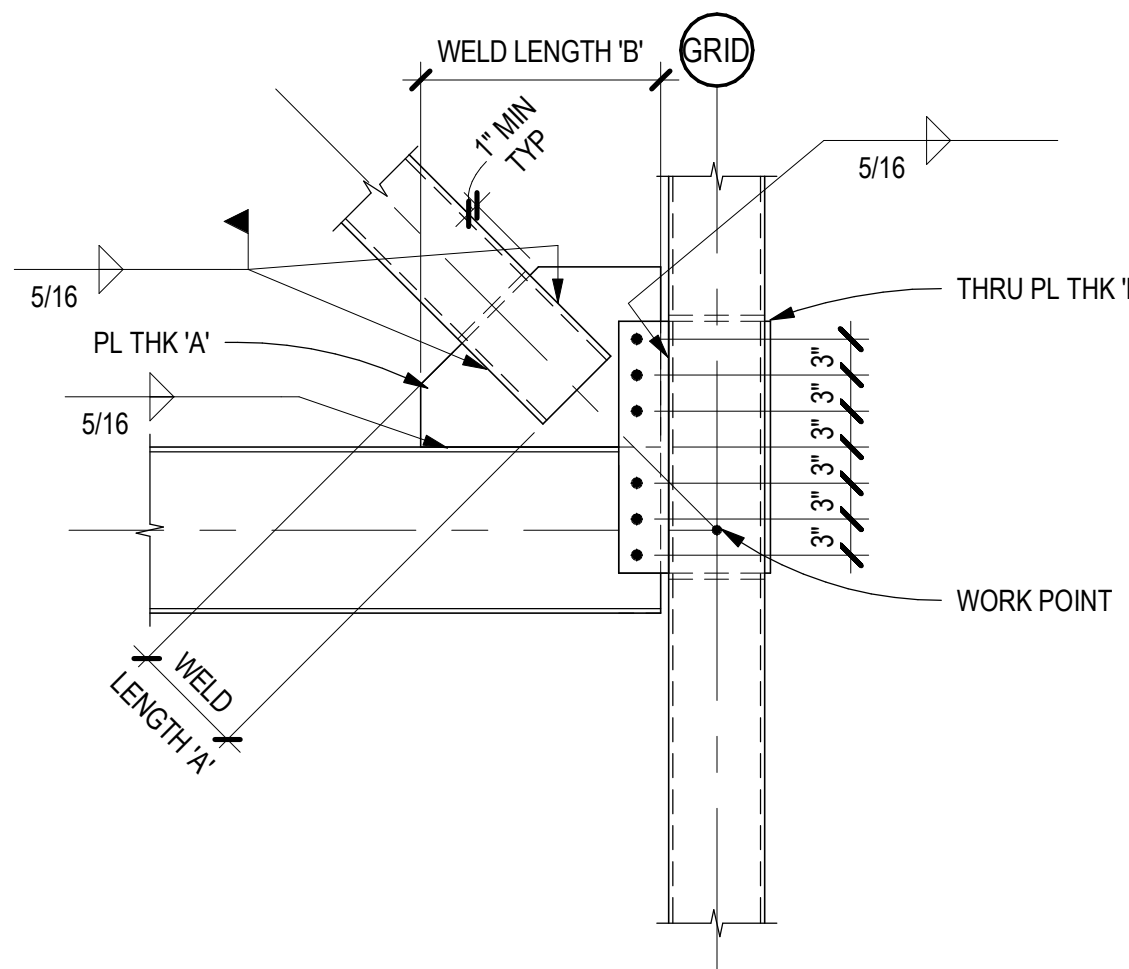
B5 PARTITION SUPPORT SECTION
SCALE: 3/4" = 1'-0"



BRACE SIZE	PL THK 'A'	THRU PL THK 'B'	WELD LENGTH 'A'	WELD LENGTH 'B'	GUSSET PL BOLTS
HSS4x4	5/8"	5/8"	4"	20"	3
HSS5x5	5/8"	5/8"	5"	20"	3
HSS6x6	5/8"	5/8"	6"	22"	3
HSS8x8	3/4"	3/4"	8"	24"	4
HSS12x8	1"	1"	12"	28"	5

NOTES
1. LENGTHS GIVEN ARE SINGLE-SIDE LENGTHS AND MINIMUM LENGTHS
2. LONGER GUSSET/WELD LENGTHS MAY BE REQ'D WHERE BRACE SLOPE VARIES FROM 1:1
3. ALL BOLTS IN STANDARD HOLES

D2 BRACED FRAME CONNECTION
SCALE: 3/4" = 1'-0"

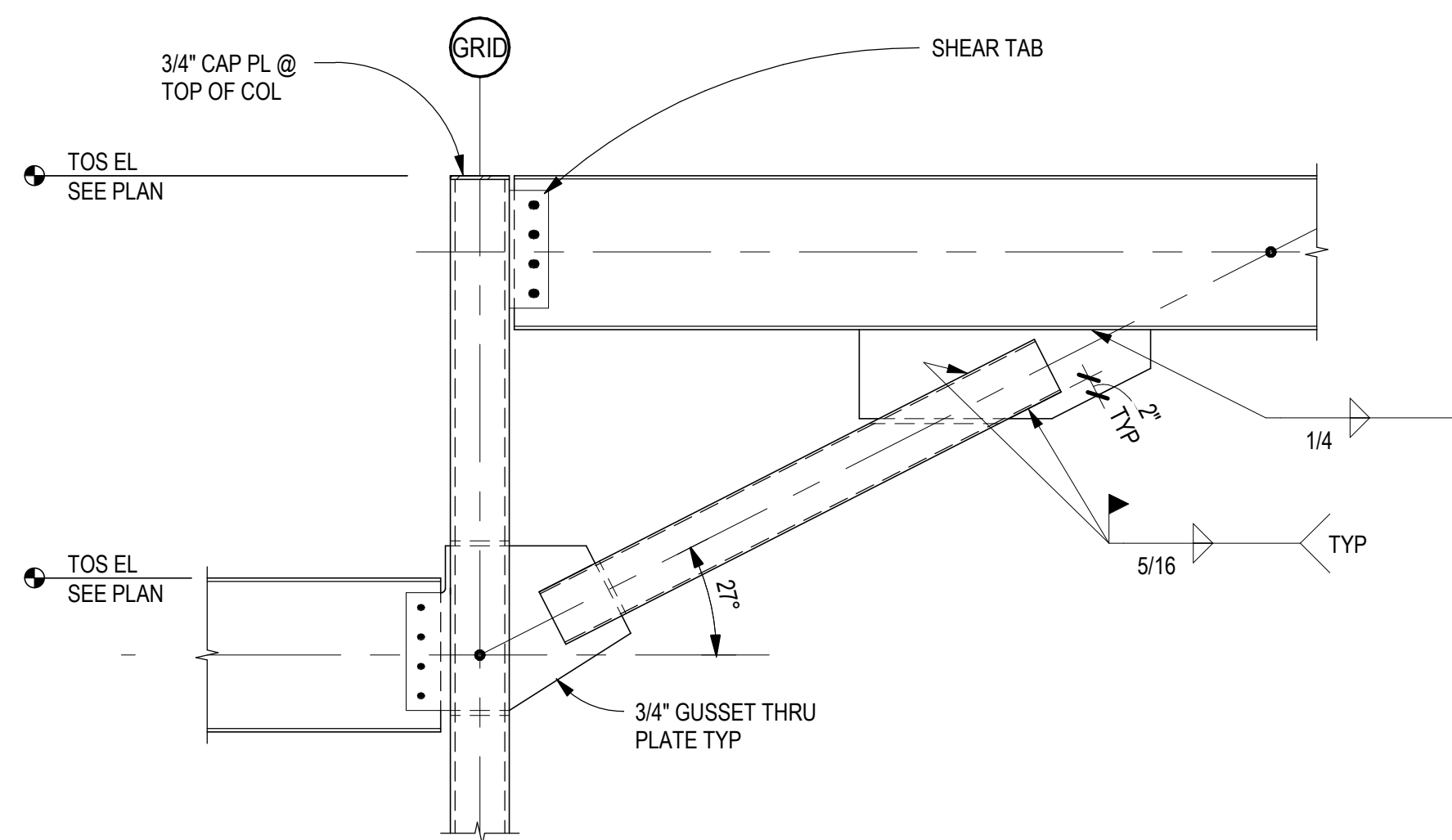


B2 BRACED FRAME CONNECTION
SCALE: 3/4" = 1'-0"

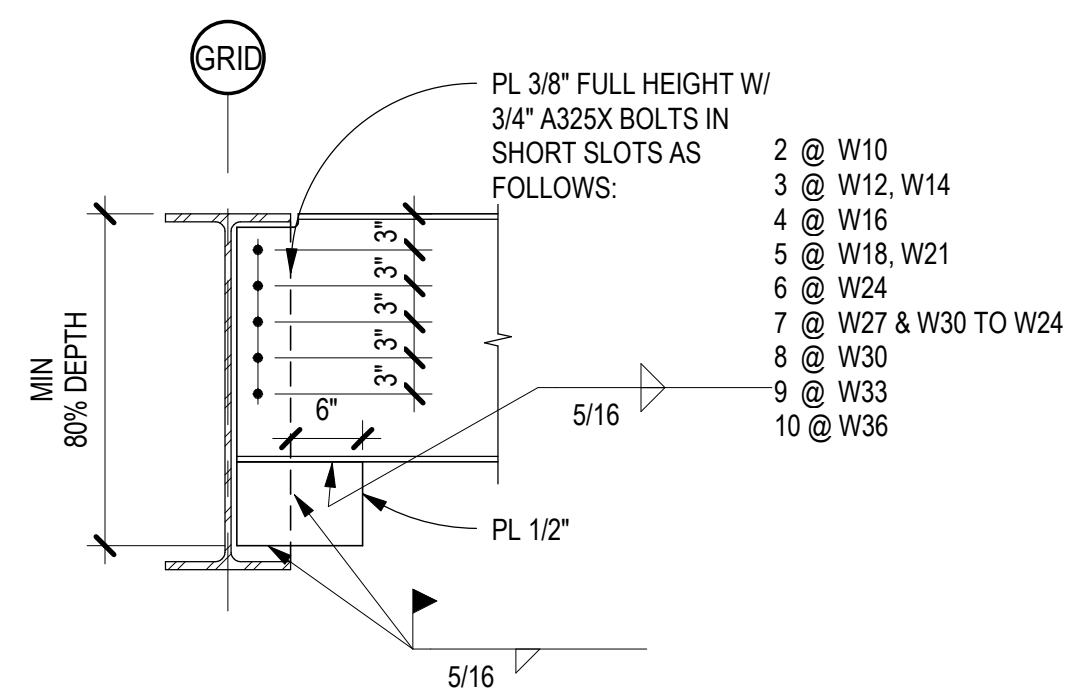
BRACE SIZE	PL THK 'A'	THRU PL THK 'B'	WELD LENGTH 'A'	WELD LENGTH 'B'	GUSSET PL BOLTS
HSS4x4	5/8"	5/8"	4"	20"	3
HSS5x5	5/8"	5/8"	5"	20"	3
HSS6x6	5/8"	5/8"	6"	22"	3
HSS8x8	3/4"	3/4"	8"	24"	4
HSS12x8	1"	1"	12"	28"	5

NOTES
1. LENGTHS GIVEN ARE SINGLE-SIDE LENGTHS AND MINIMUM LENGTHS
2. LONGER GUSSET/WELD LENGTHS MAY BE REQ'D WHERE BRACE SLOPE VARIES FROM 1:1
3. ALL BOLTS IN STANDARD HOLES

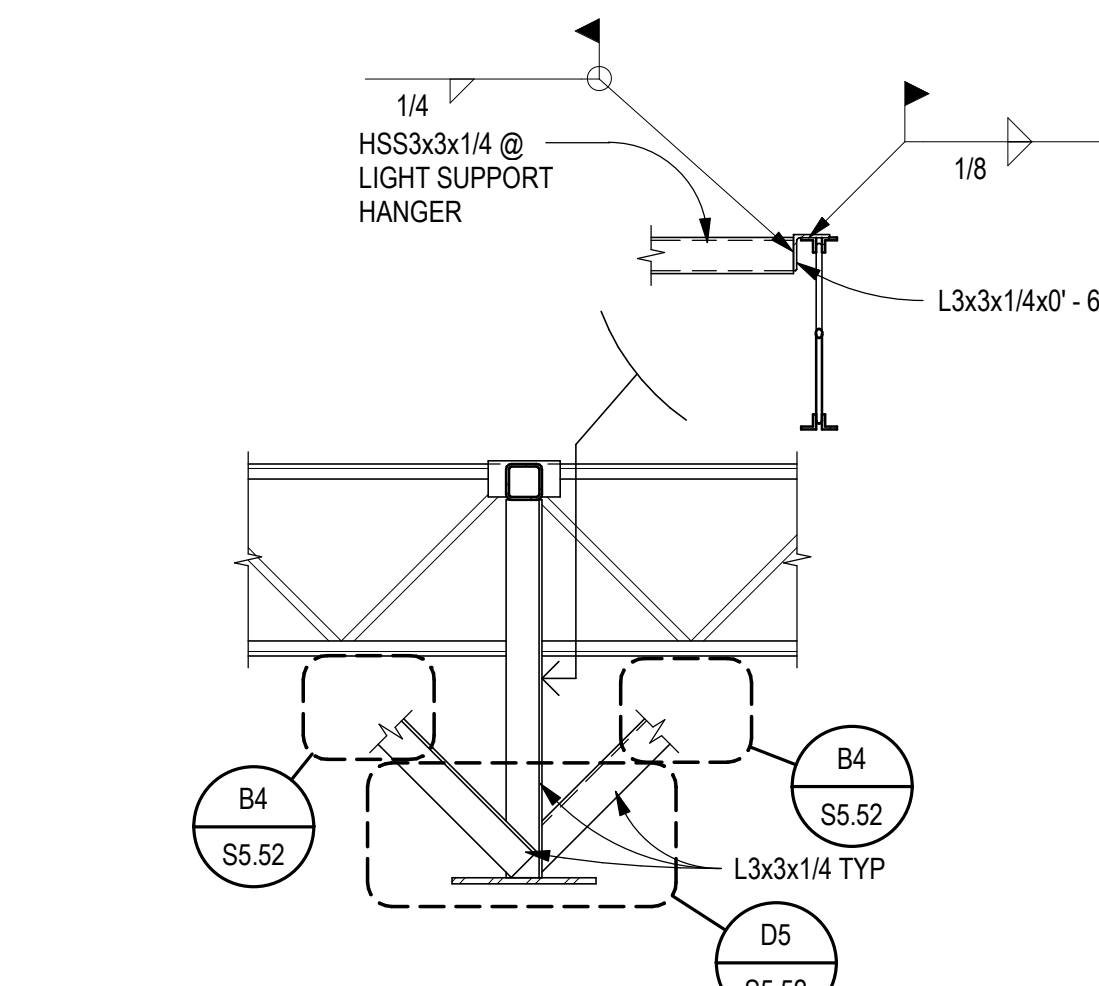
B2 BRACED FRAME CONNECTION
SCALE: 3/4" = 1'-0"



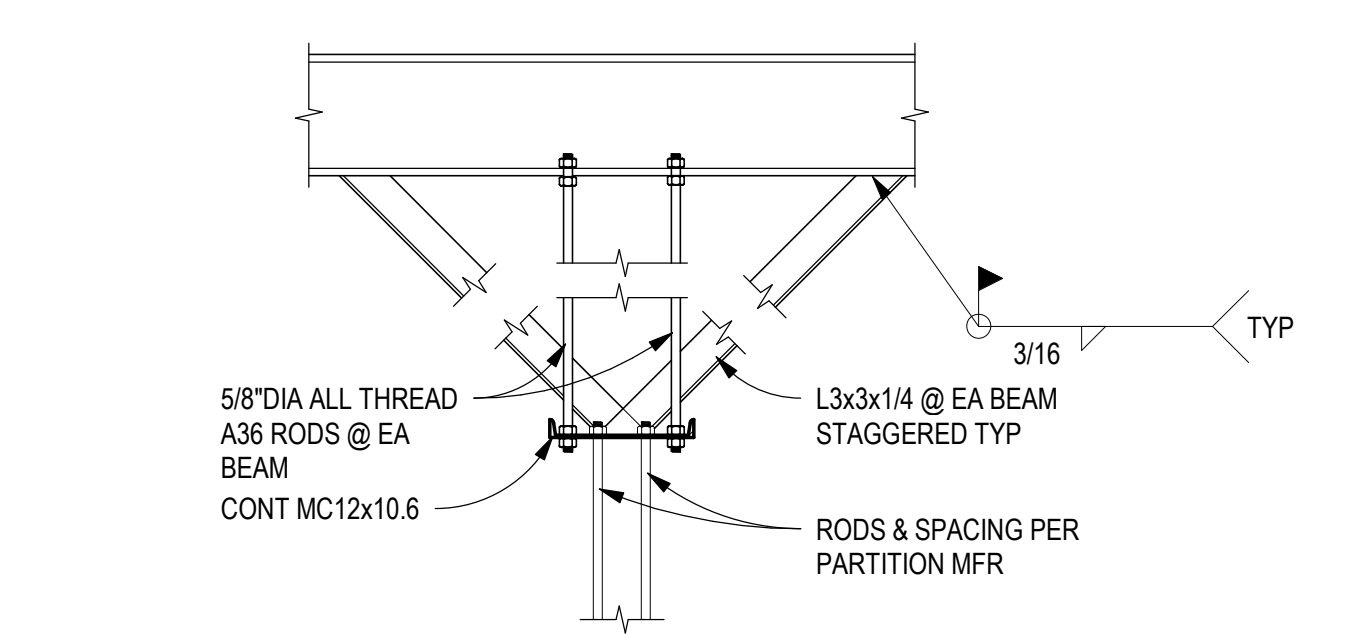
A1 COLLECTOR FRAMING DETAIL
SCALE: 3/4" = 1'-0"



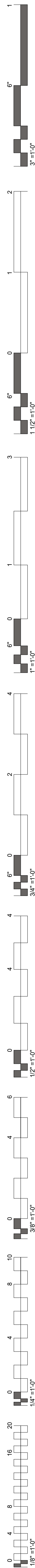
A3 BEAM CONN AT MOMENT (WUF) CONN
SCALE: 3/4" = 1'-0"

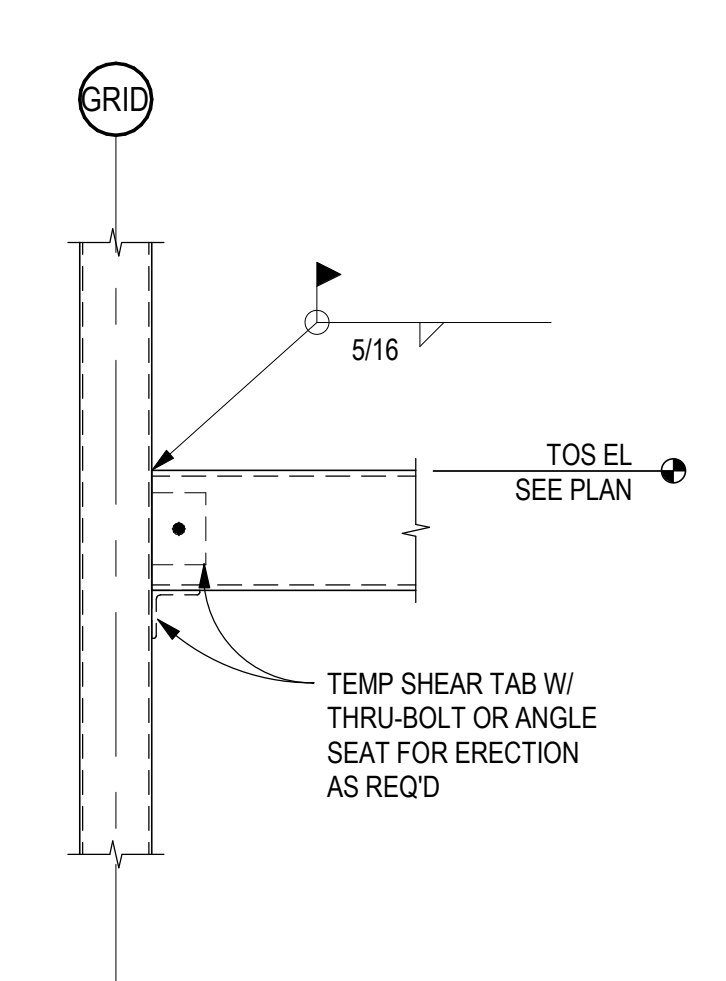
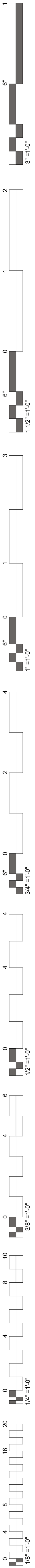


A4 LIGHT SUPPORT BETWEEN JOISTS
SCALE: 3/4" = 1'-0"

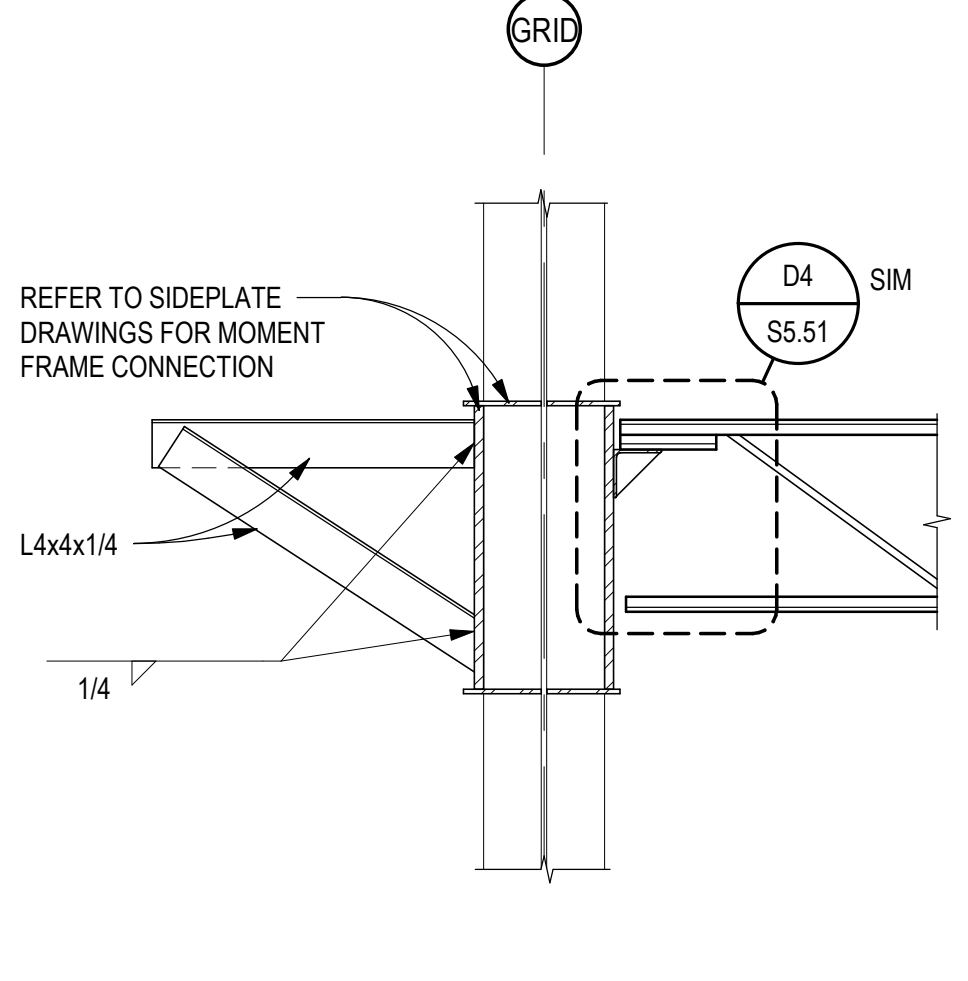


A5 PARTITION FRAMING SECTION
SCALE: 3/4" = 1'-0"

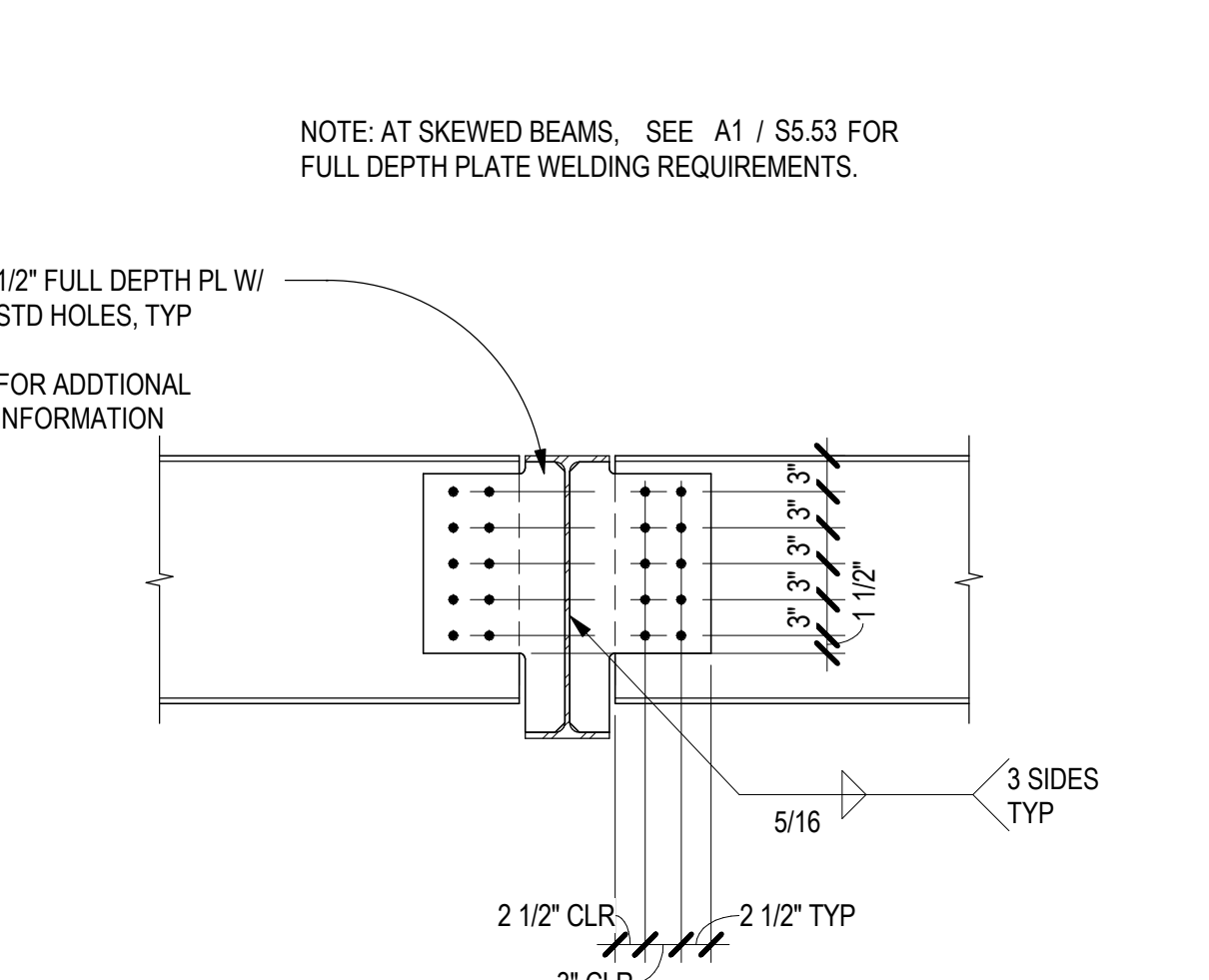




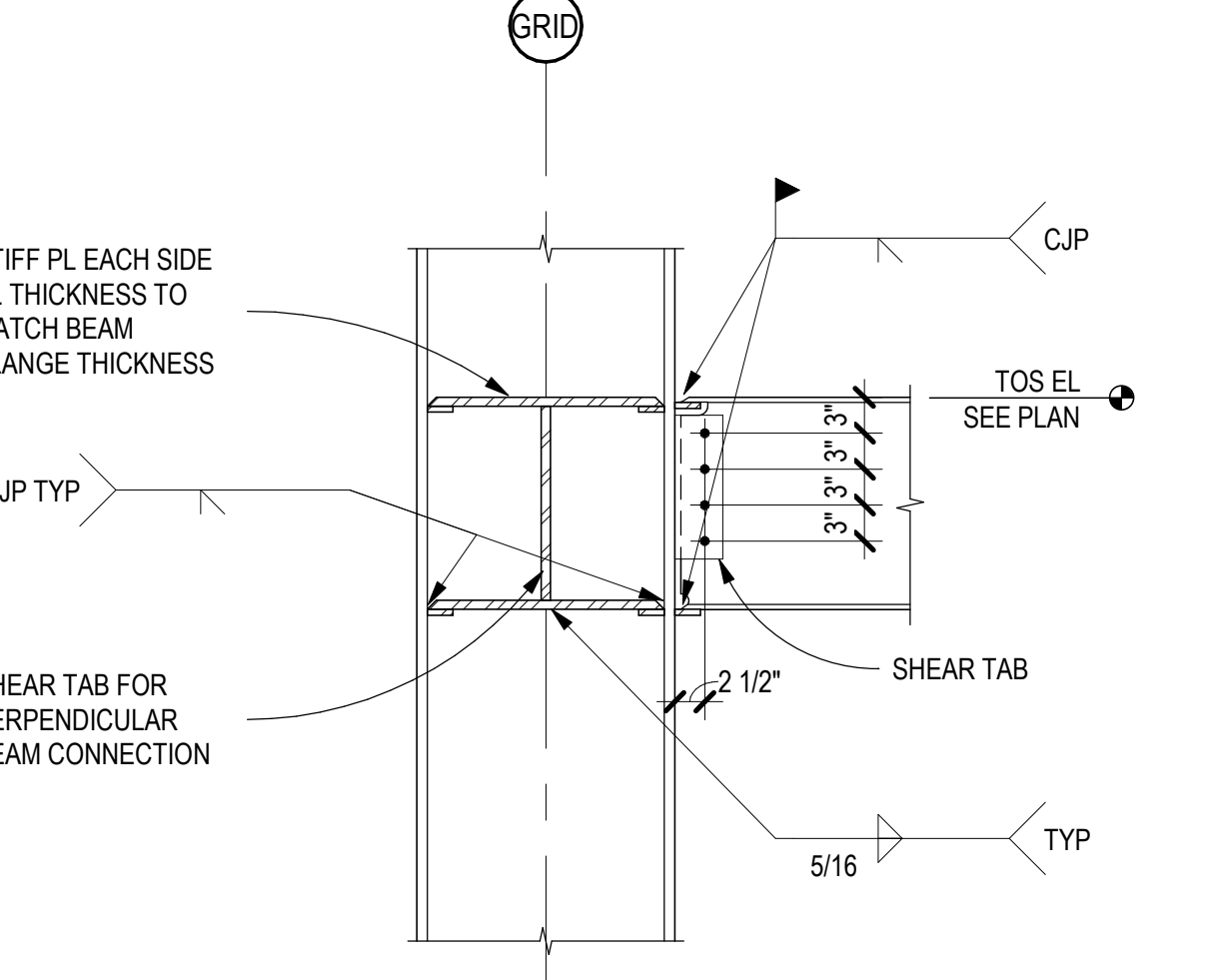
D1 HSS TO COLUMN WITH PARTIAL FIXITY
SCALE: 3/4" = 1'-0"



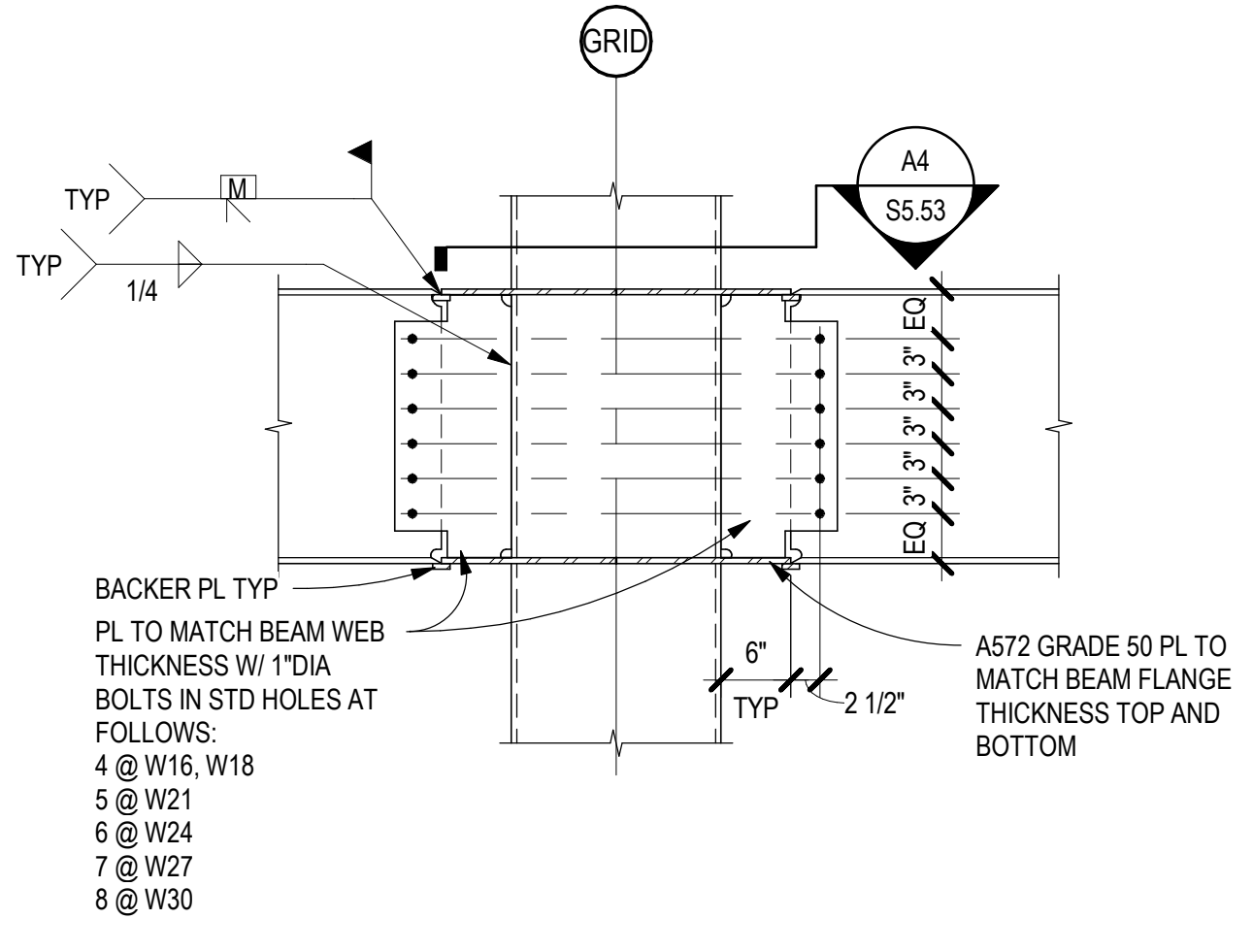
D2 BM TO COL WEB CONN AT SIDEPLATE
SCALE: 3/4" = 1'-0"



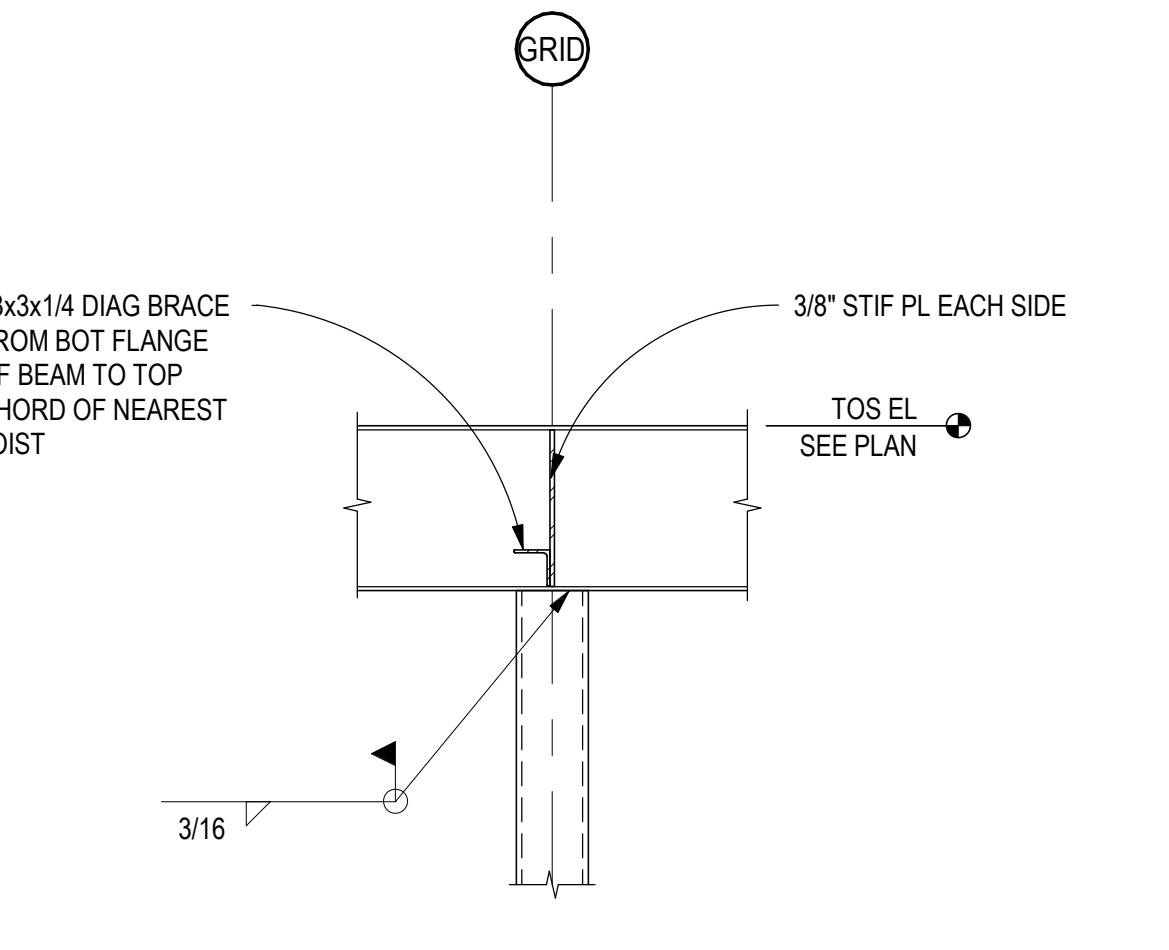
D3 BEAM TO BEAM COLLECTOR DETAIL
SCALE: 3/4" = 1'-0"



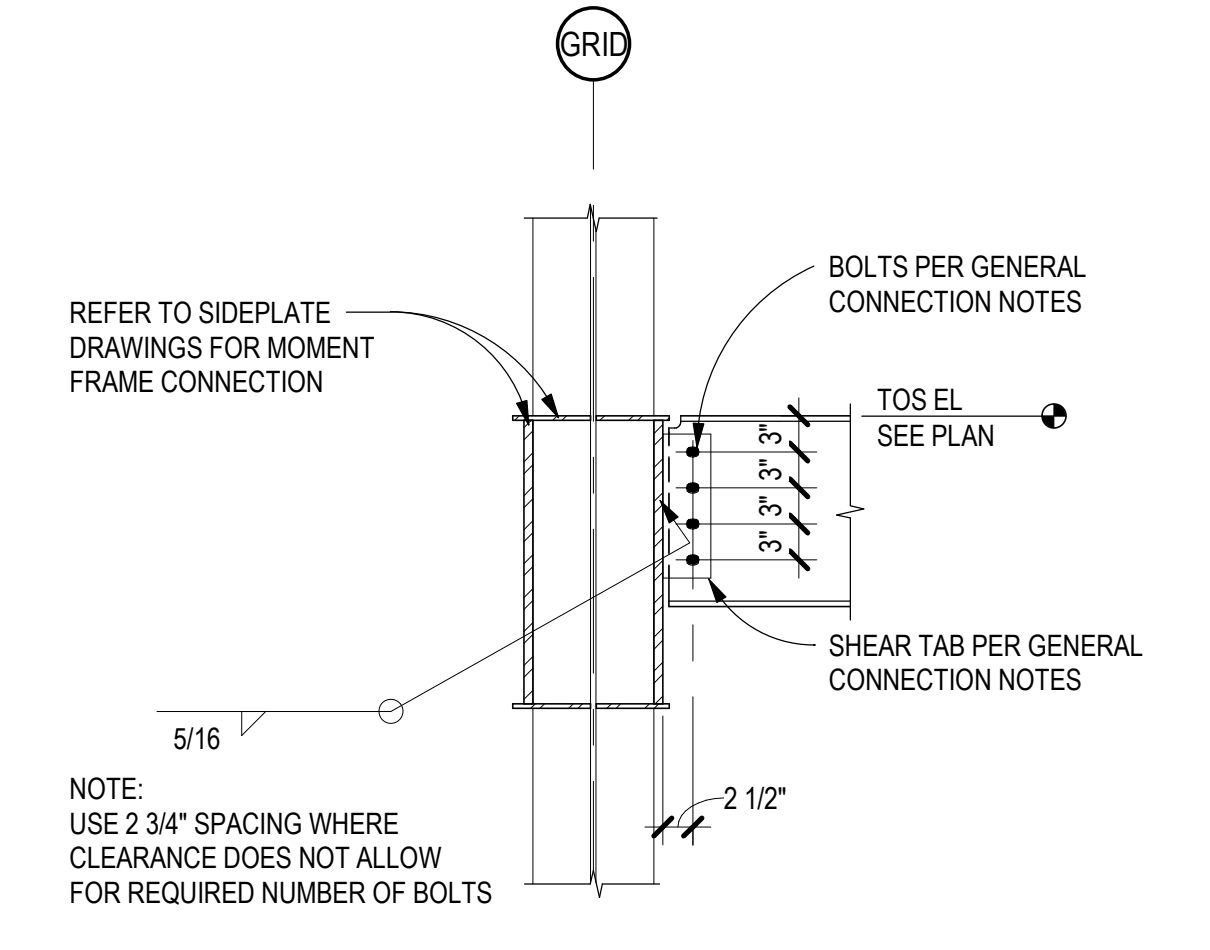
D4 BEAM TO COL FLANGE MOMENT CONN
SCALE: 3/4" = 1'-0"



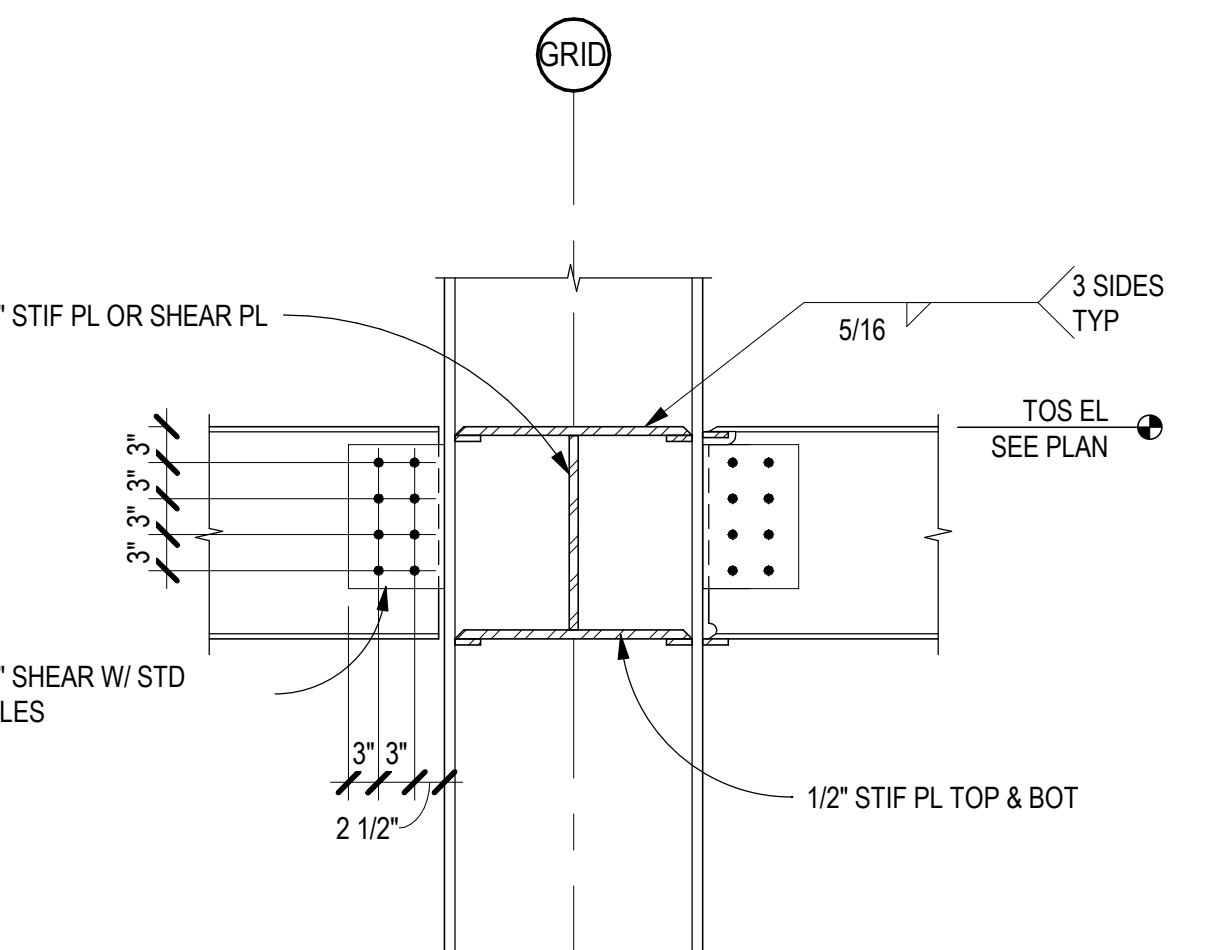
D5 TYPICAL FLANGE PLATE TO COLUMN
SCALE: 3/4" = 1'-0"



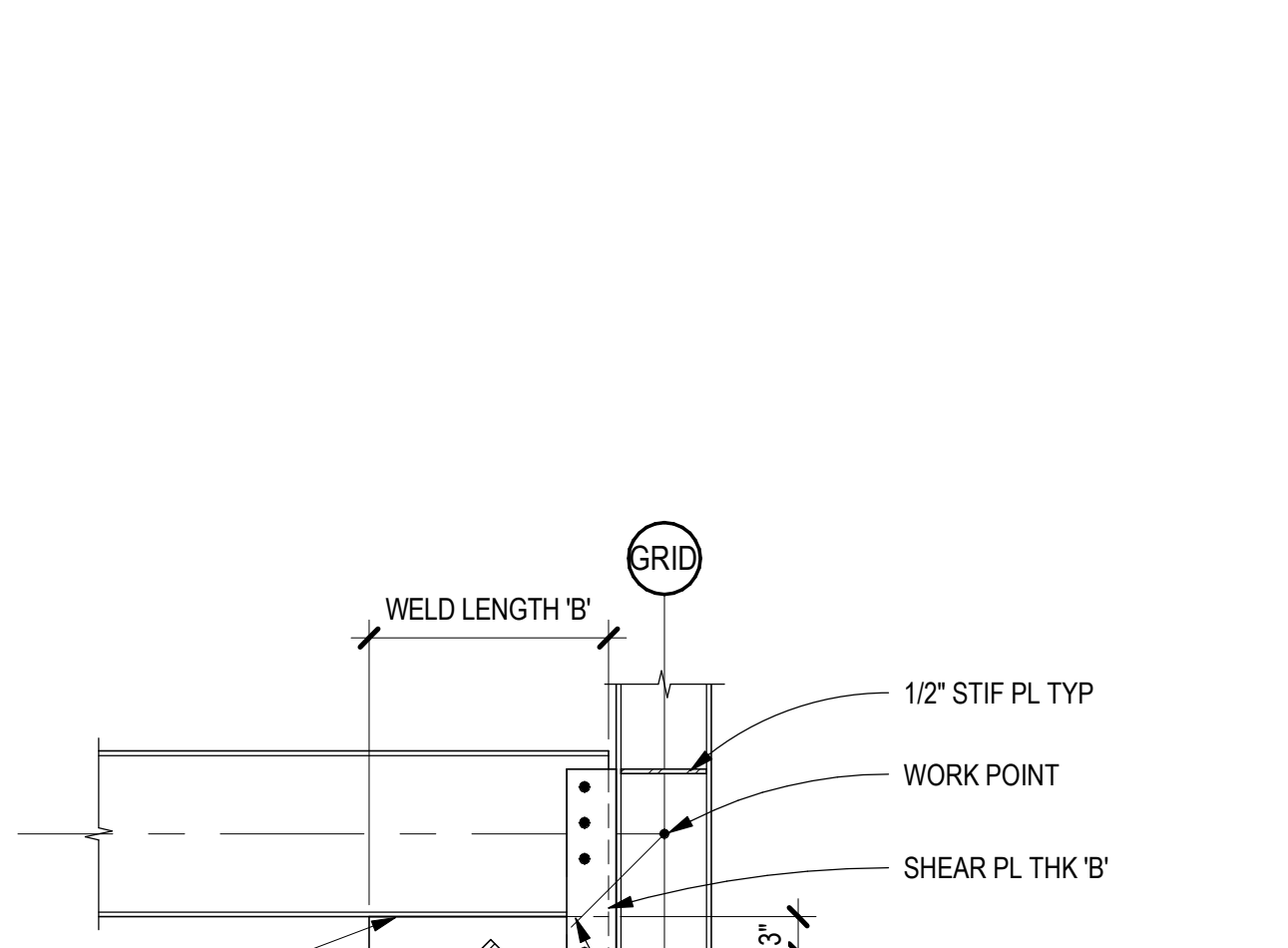
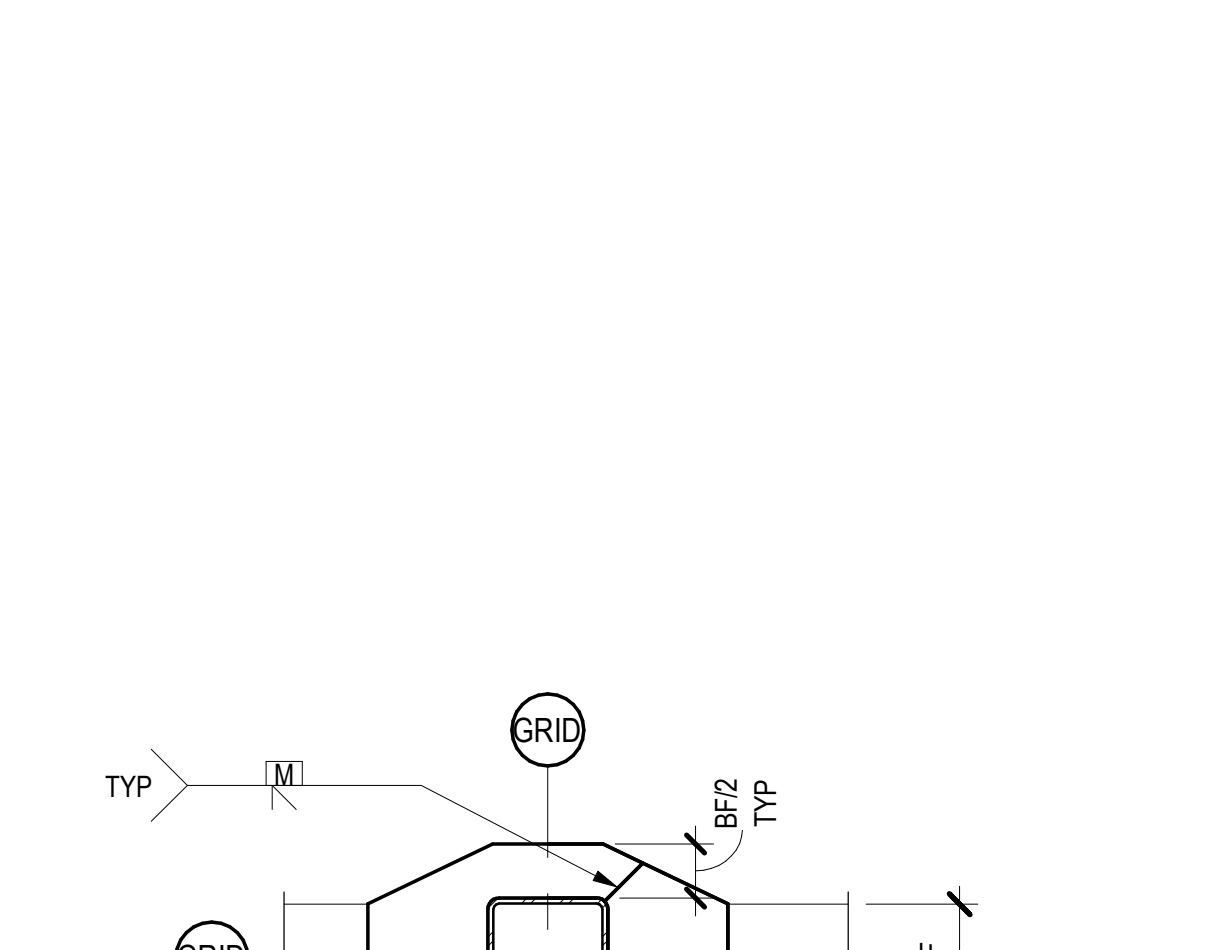
C1 BEAM OVER COLUMN CONN
SCALE: 3/4" = 1'-0"



C2 BM TO COL WEB CONN AT SIDEPLATE
SCALE: 3/4" = 1'-0"



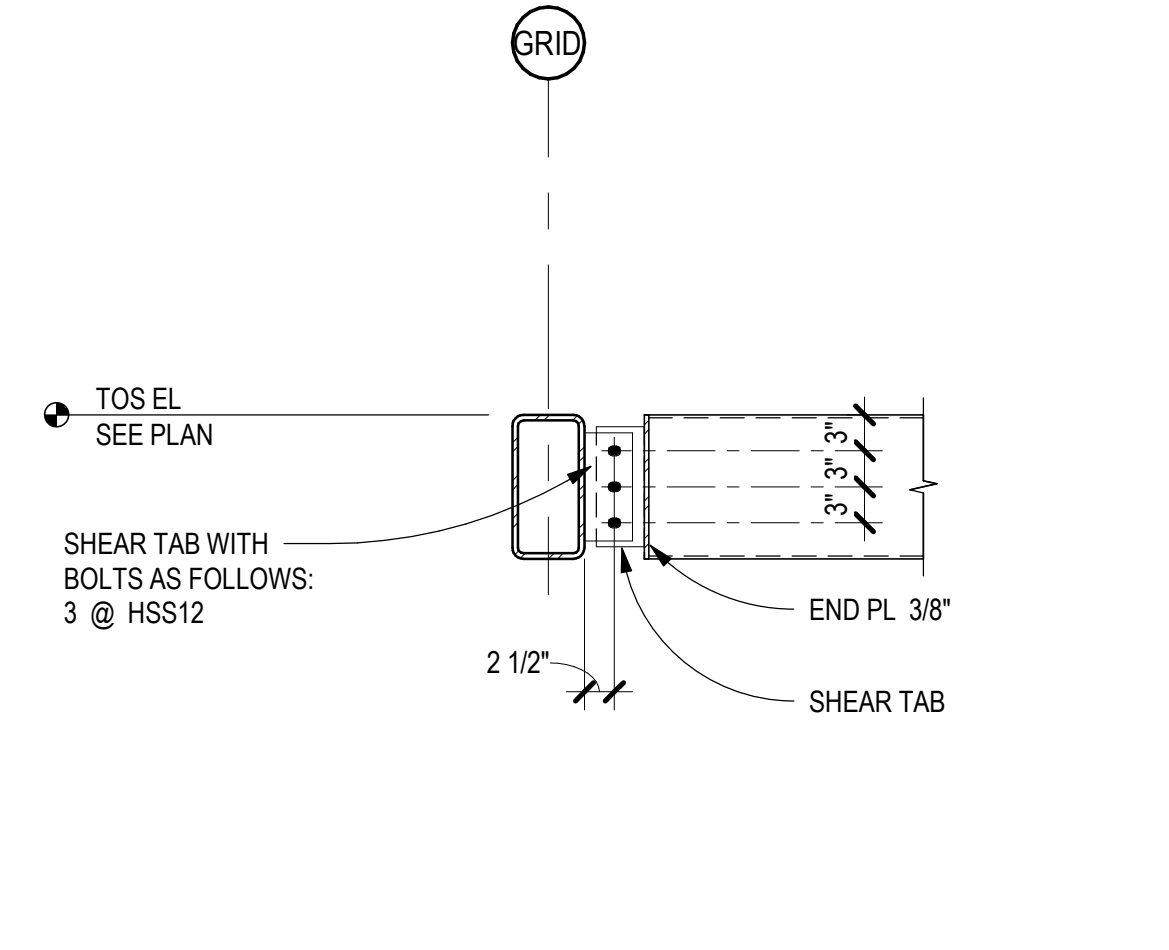
C3 COLLECTOR BM TO COL MOMENT CONN
SCALE: 3/4" = 1'-0"



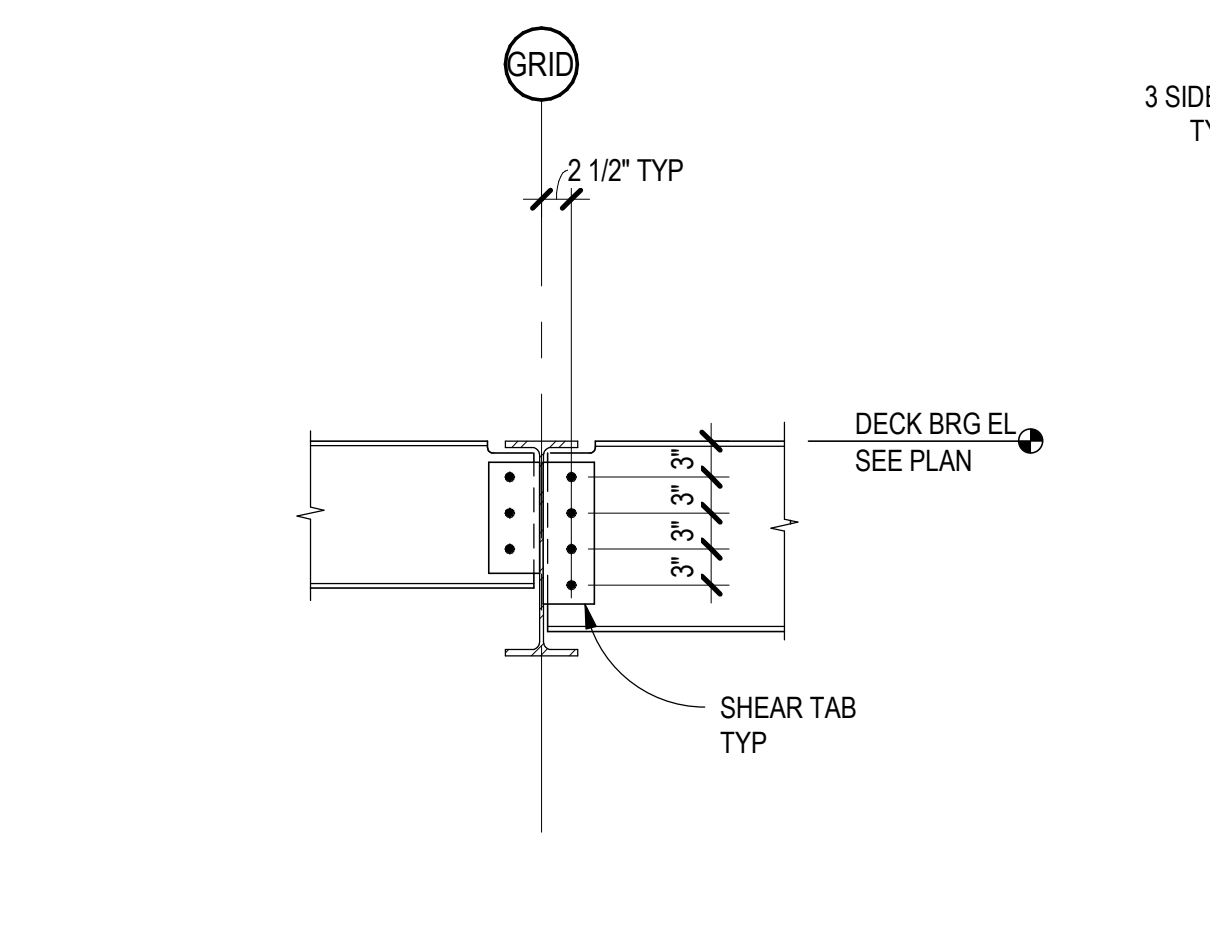
CONNECTION SCHEDULE					
BRACE SIZE	PL THK 'A'	THRU PL THK 'B'	WELD LENGTH 'A'	WELD LENGTH 'B'	GUSSET PL BOLTS
HSS4x4	5/8"	5/8"	4"	20"	3
HSS5x5	5/8"	5/8"	5"	20"	3
HSS6x6	5/8"	5/8"	6"	22"	3
HSS8x8	3/4"	3/4"	8"	24"	4
HSS12x8	1"	1"	12"	28"	5

NOTES:
 1. LENGTHS GIVEN ARE SINGLE-SIDE LENGTHS AND MINIMUM LENGTHS
 2. LONGER GUSSET/WELD LENGTHS MAY BE REQ'D WHERE BRACE SLOPE VARIES FROM 1:1
 3. ALL BOLTS IN STANDARD HOLES

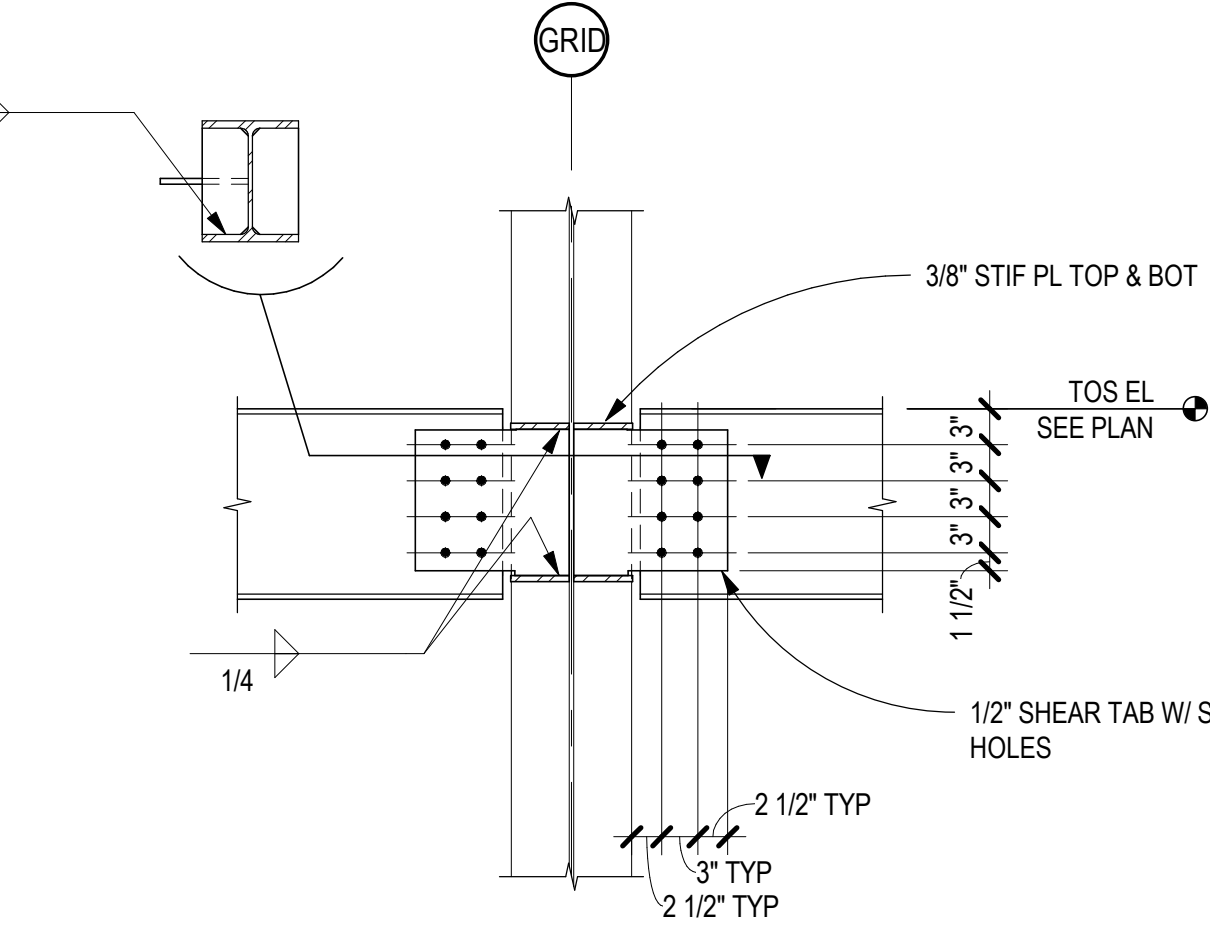
B5 BRACED FRAME CONNECTION
SCALE: 3/4" = 1'-0"



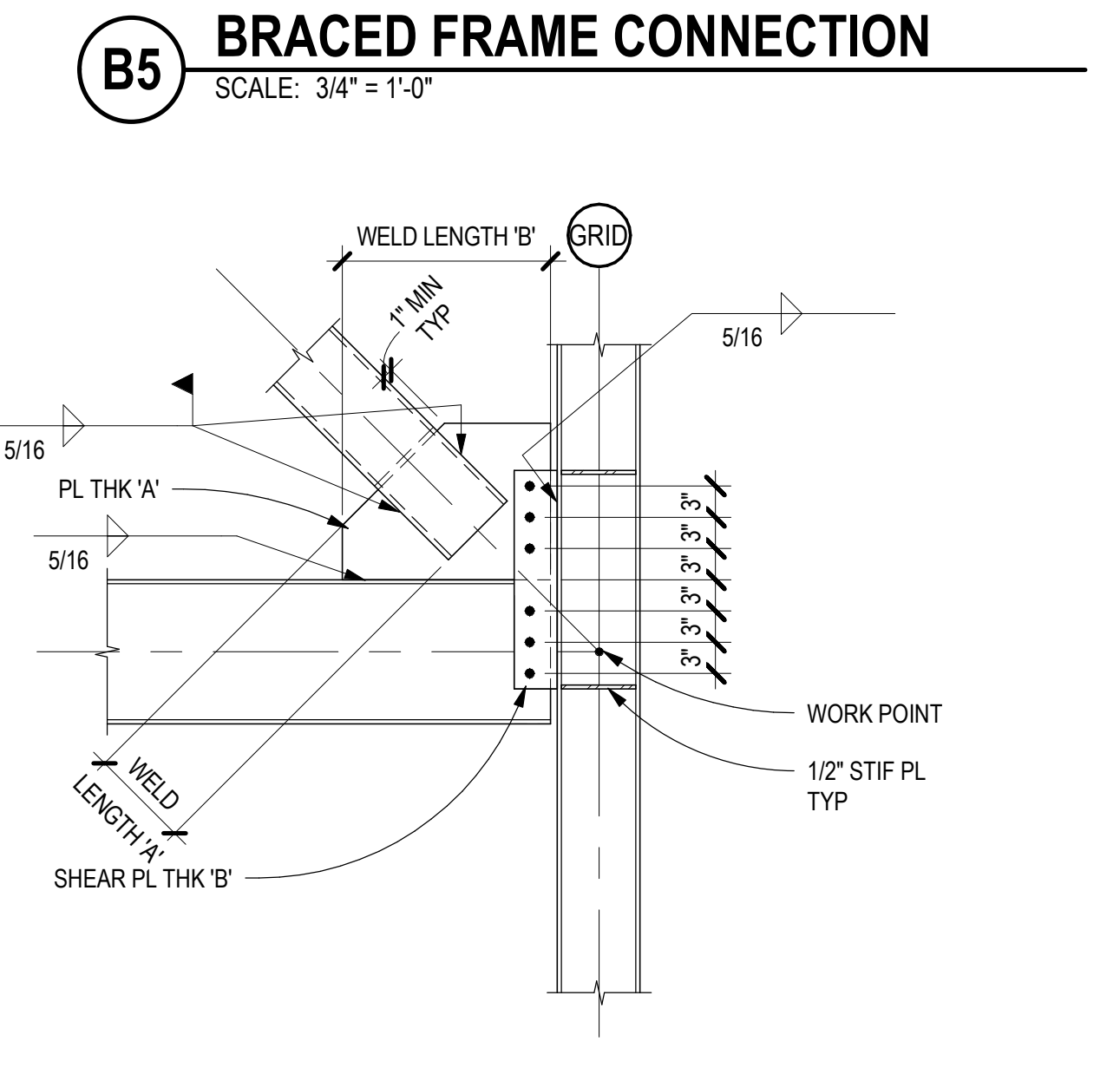
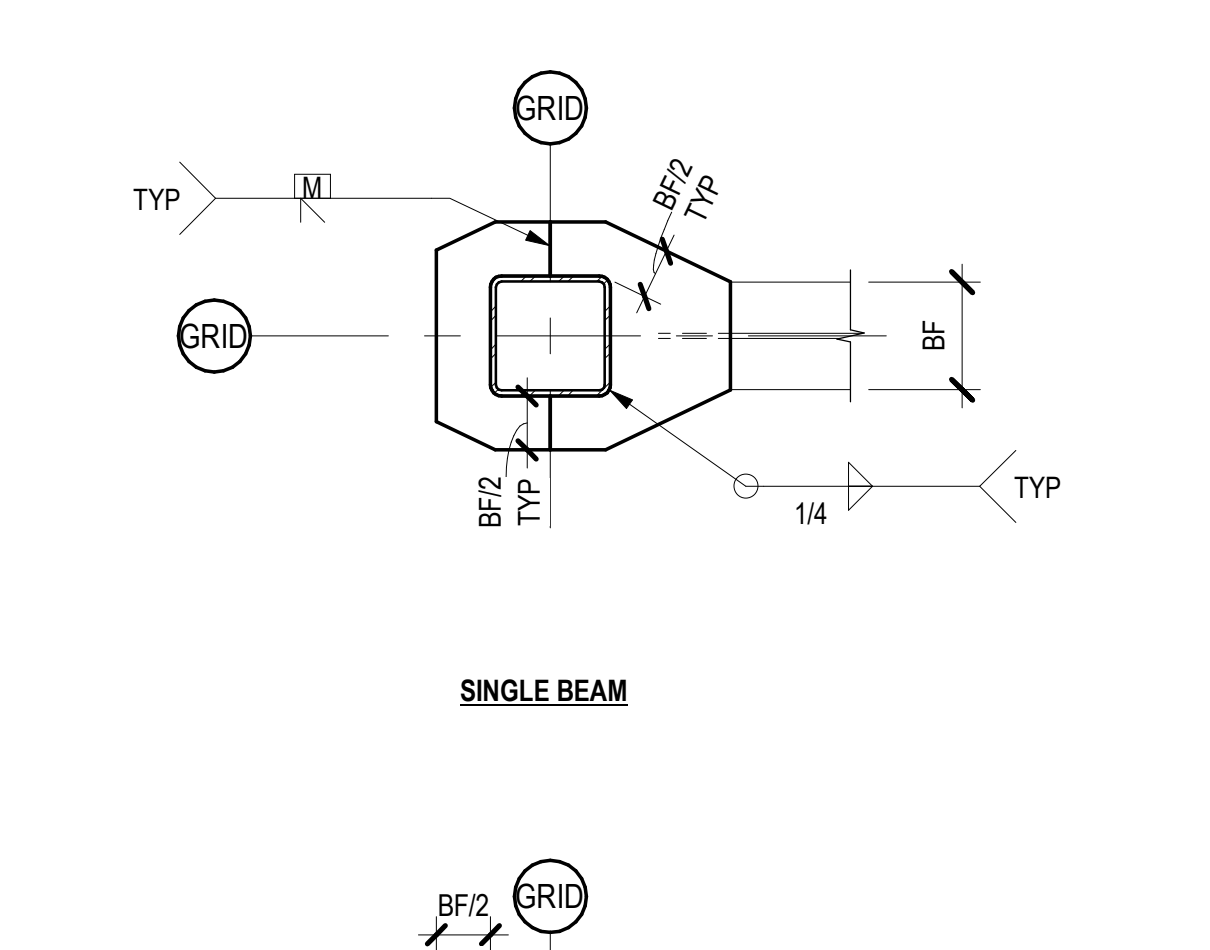
B1 BEAM TO BEAM CONN
SCALE: 3/4" = 1'-0"



B2 TYPICAL BEAM TO BEAM CONN
SCALE: 3/4" = 1'-0"



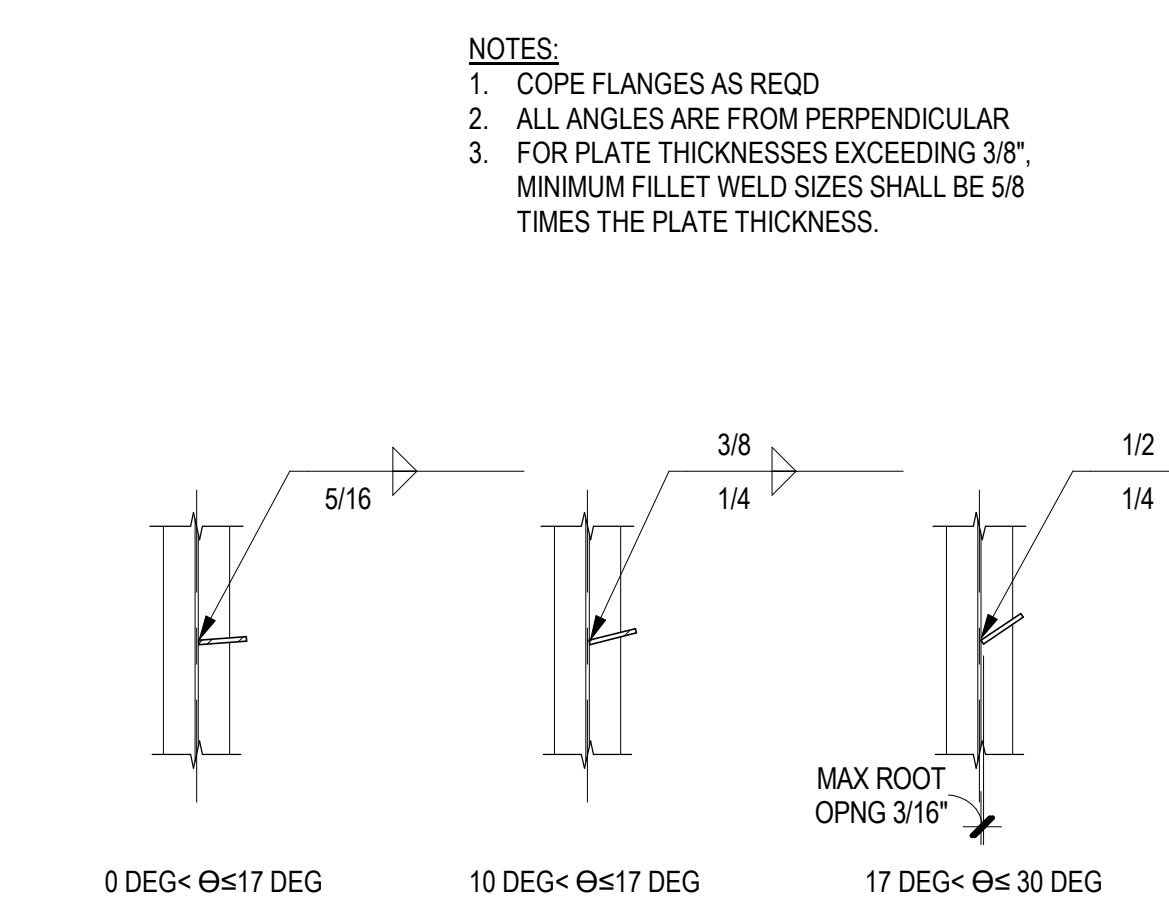
B3 COLLECTOR BM TO COL WEB CONN
SCALE: 3/4" = 1'-0"



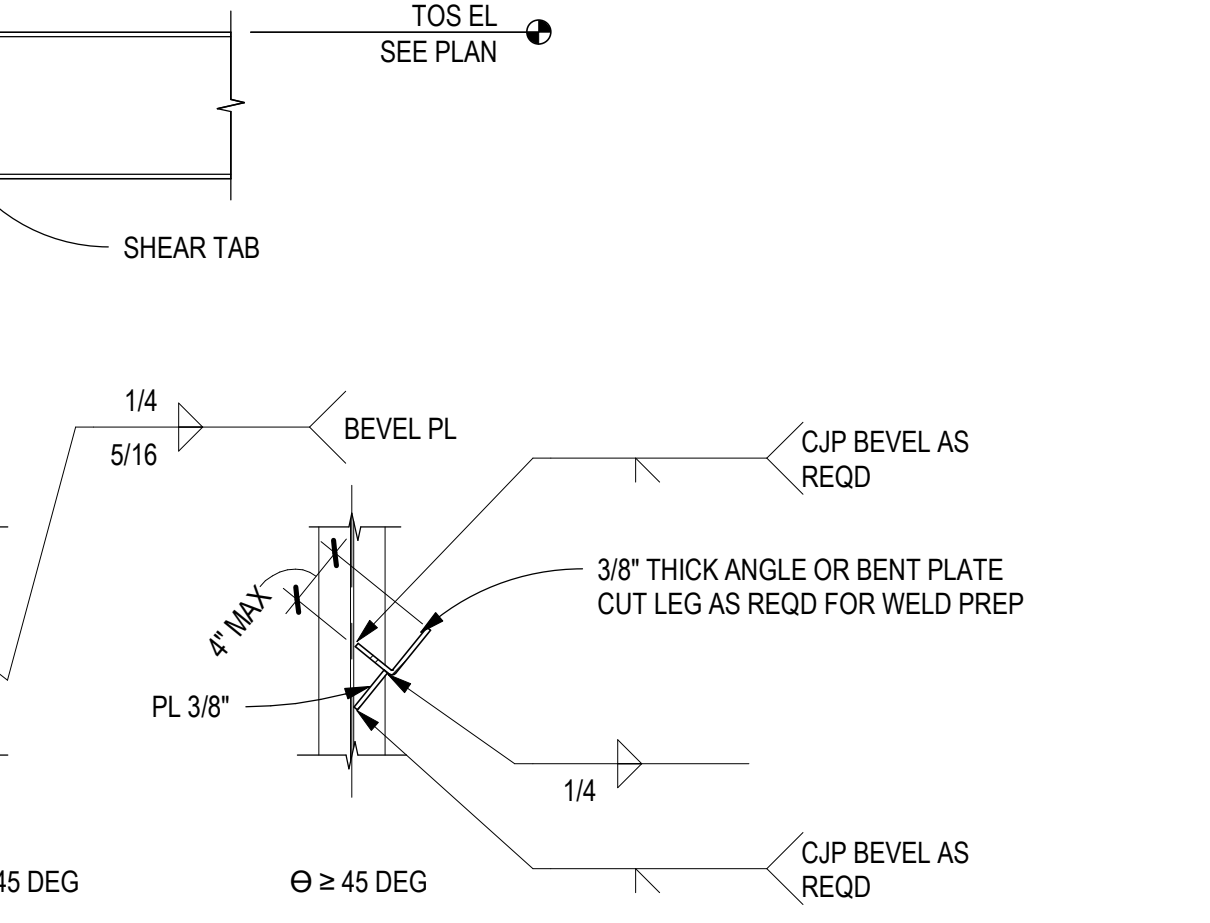
CONNECTION SCHEDULE					
BRACE SIZE	PL THK 'A'	THRU PL THK 'B'	WELD LENGTH 'A'	WELD LENGTH 'B'	GUSSET PL BOLTS
HSS4x4	5/8"	5/8"	4"	20"	3
HSS5x5	5/8"	5/8"	5"	20"	3
HSS6x6	5/8"	5/8"	6"	22"	3
HSS8x8	3/4"	3/4"	8"	24"	4
HSS12x8	1"	1"	12"	28"	5

NOTES:
 1. LENGTHS GIVEN ARE SINGLE-SIDE LENGTHS AND MINIMUM LENGTHS
 2. LONGER GUSSET/WELD LENGTHS MAY BE REQ'D WHERE BRACE SLOPE VARIES FROM 1:1
 3. ALL BOLTS IN STANDARD HOLES

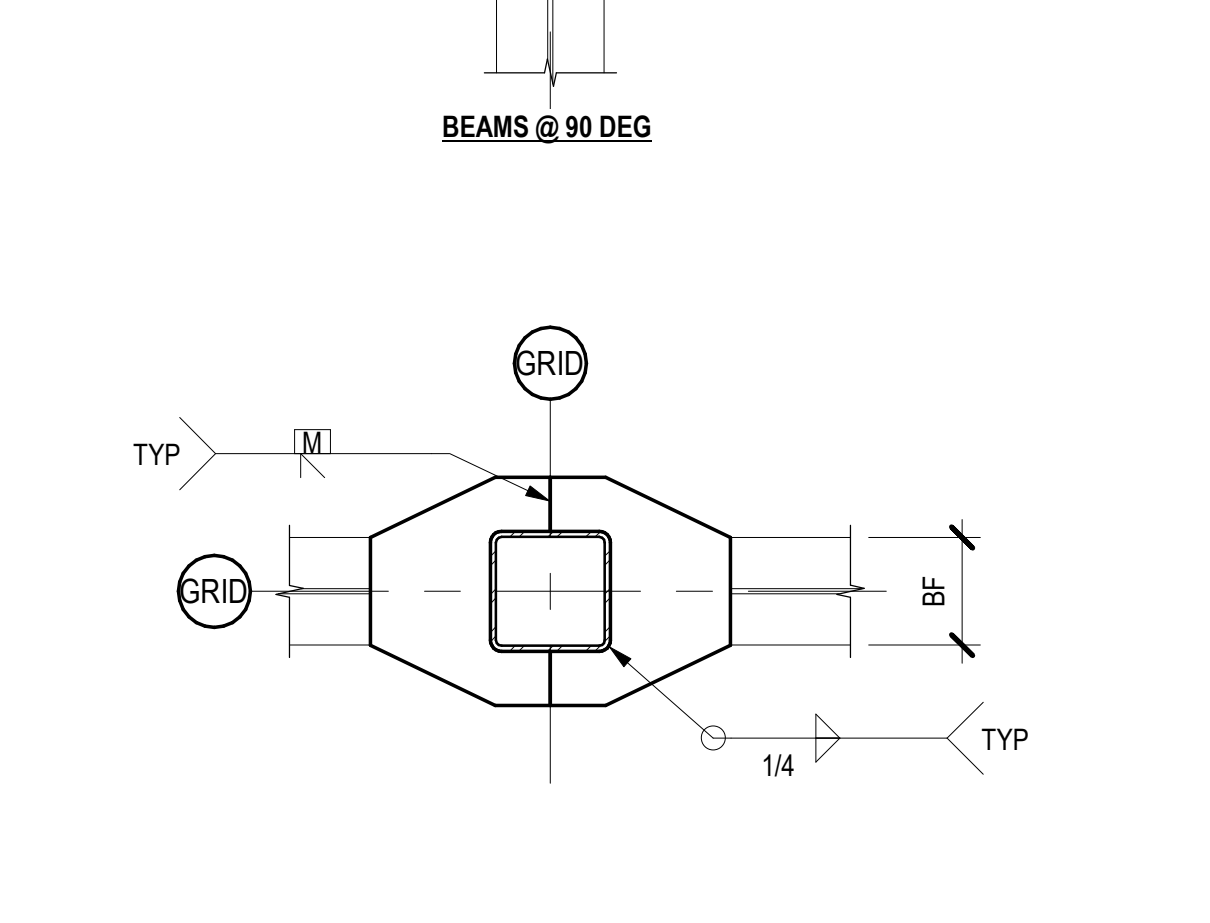
A5 BRACED FRAME CONNECTION
SCALE: 3/4" = 1'-0"



A1 SKEWED BEAM TO BEAM CONN
SCALE: 3/4" = 1'-0"



A3 COLLECTOR BEAM TO COLUMN CONN
SCALE: 3/4" = 1'-0"

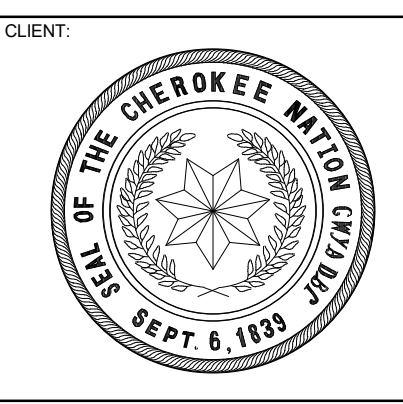


A4 FLANGE PLATE CONN AT HSS COL
SCALE: 3/4" = 1'-0"

CONNECTION SCHEDULE					
BRACE SIZE	PL THK 'A'	THRU PL THK 'B'	WELD LENGTH 'A'	WELD LENGTH 'B'	GUSSET PL BOLTS
HSS4x4	5/8"	5/8"	4"	20"	3
HSS5x5	5/8"	5/8"	5"	20"	3
HSS6x6	5/8"	5/8"	6"	22"	3
HSS8x8	3/4"	3/4"	8"	24"	4
HSS12x8	1"	1"	12"	28"	5

NOTES:
 1. LENGTHS GIVEN ARE SINGLE-SIDE LENGTHS AND MINIMUM LENGTHS
 2. LONGER GUSSET/WELD LENGTHS MAY BE REQ'D WHERE BRACE SLOPE VARIES FROM 1:1
 3. ALL BOLTS IN STANDARD HOLES

A5 BRACED FRAME CONNECTION
SCALE: 3/4" = 1'-0"



**WILMA P. MANKILLER HEALTH CENTER
 EXPANSION
 STILWELL, OKLAHOMA**

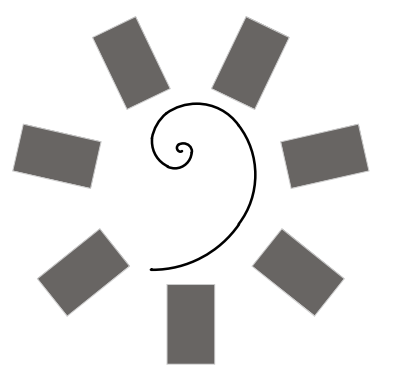
KEY PLAN:	

PROJECT PHASE:
BID PACKAGE 01

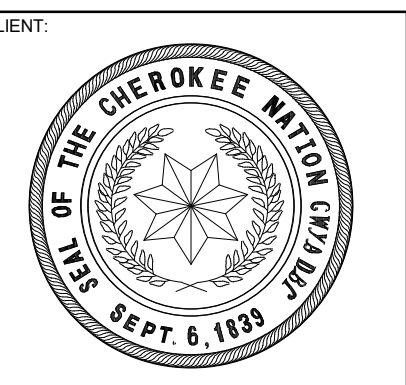
#	DATE	REVISIONS	DESCRIPTION

DATE: 11-01-19
 JOB NUMBER: 18-01.01

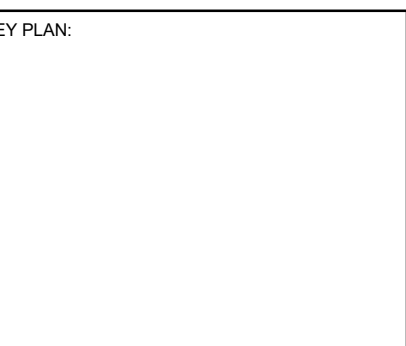
SHEET NUMBER:
S5.53
 STEEL DETAILS



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WILMA P. MANKILLER HEALTH CENTER
EXPANSION
STILWELL, OKLAHOMA



PROJECT PHASE:
BID PACKAGE 01

#	DATE	REVISIONS	DESCRIPTION

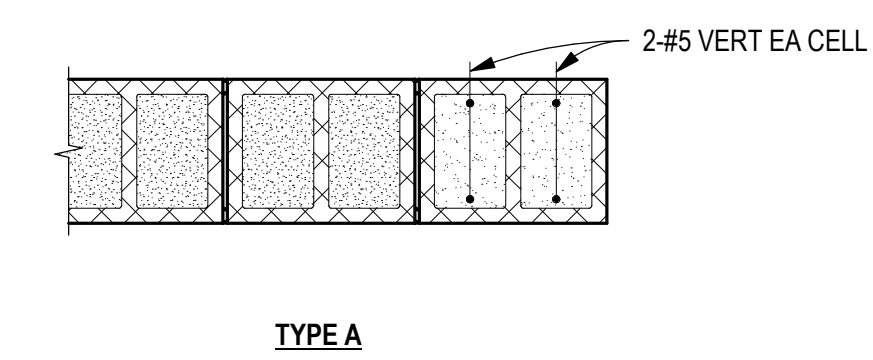
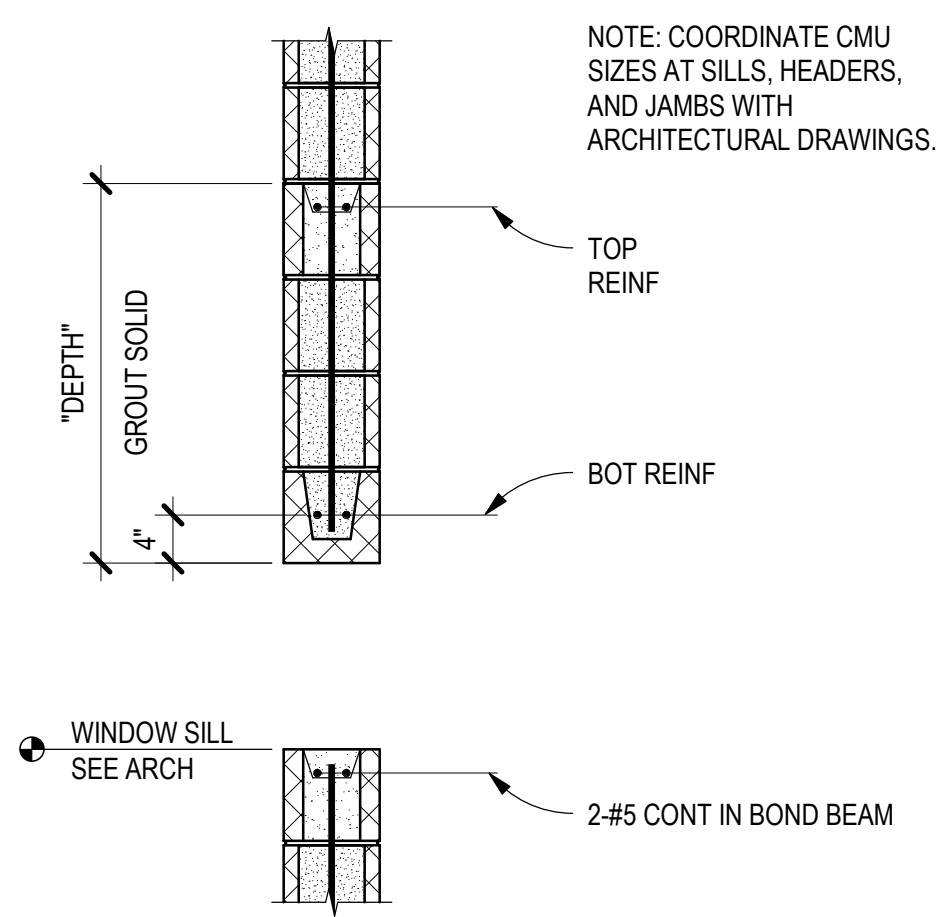
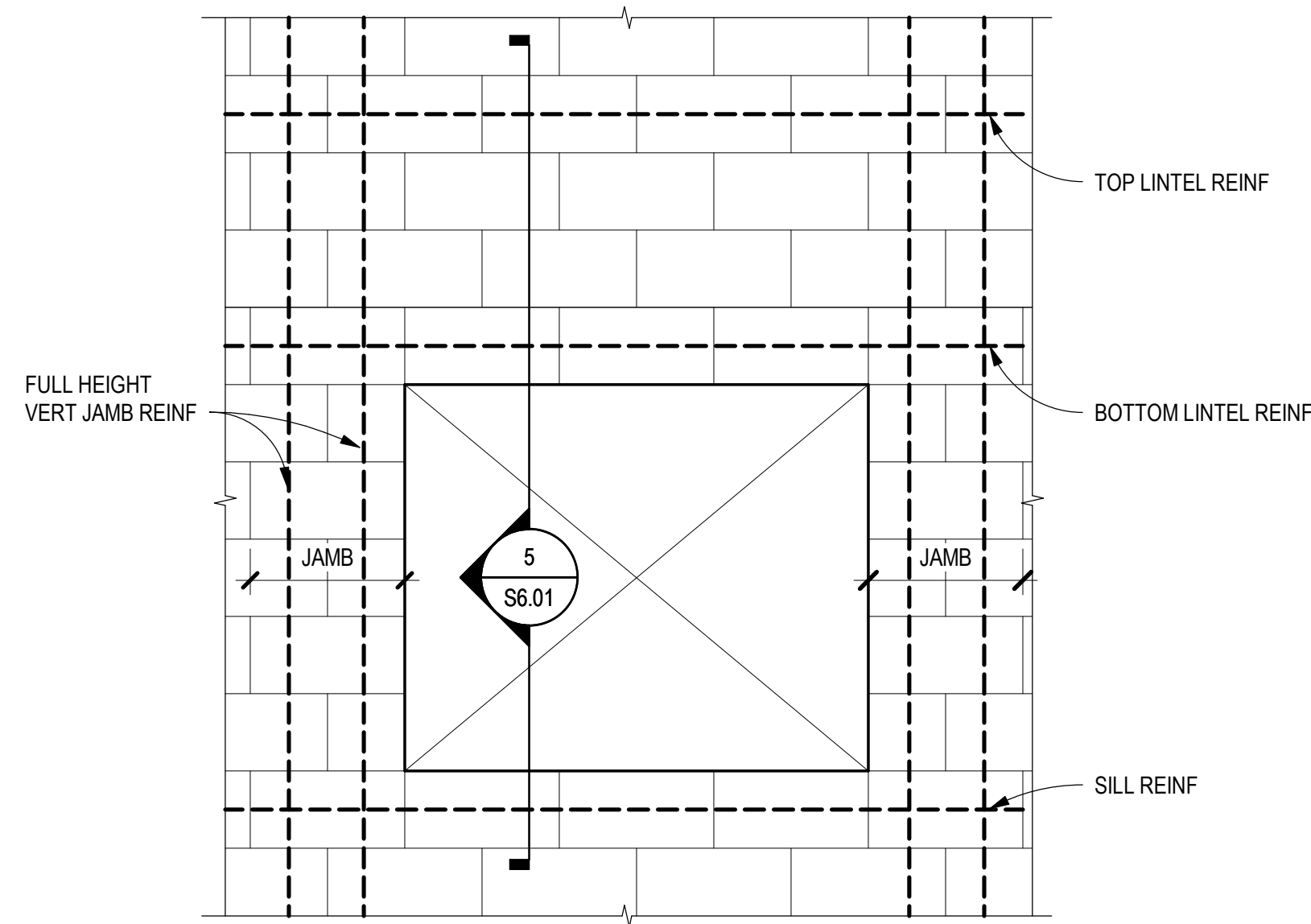
DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER:
S6.01

SCHEDULES

CMU LINTEL SCHEDULE						
OPENING WIDTH	WIDTH	DEPTH	LINTEL REINFORCING		SILL	
			TOP	BOTTOM	DEPTH	REINFORCING
0'-0" - 8'-0"	12"	32"	2-#5	2-#5	8"	2-#5
					LINTEL JAMB TYPE	
					TYPE A	

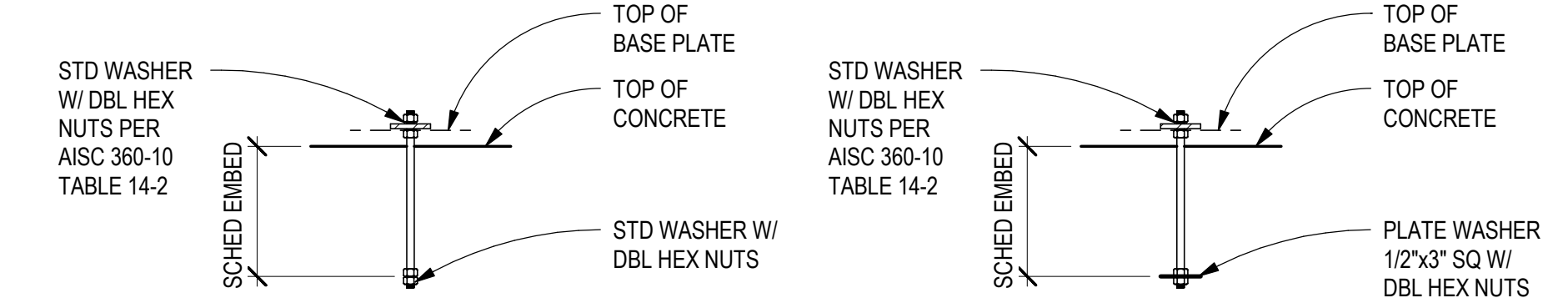
DECK SCHEDULE													
MARK	COMPOSITE SLAB	SLAB			METAL DECK			DECK ATTACHMENTS			TOTAL SLAB / DECK THICKNESS	COMMENTS	
		THICK	MATL	REINF	THICK	TYPE	GAGE	FINISH	ATTACH PERP TO RIBS	ATTACH PARALLEL TO RIBS			ATTACH SIDELAPS
D1.5R	X	3"	NW CONC	6x6 - W2.1xW2.1 WELD WIRE FABRIC IN FLAT SHEETS	1 1/2"	W3	20	GALVANIZED	4-5/8" DIA PUDDLE WELDS PER 36" WIDE SHEET	5/8" DIA PUDDLE WELDS @ 12" OC	#10 SCREWS @ 12" OC	6"	



NOTE: SEE TYPICAL CMU PLAN DETAILS SHEET S7.21 FOR TYPICAL HORIZONTAL REINFORCING REQUIREMENTS.

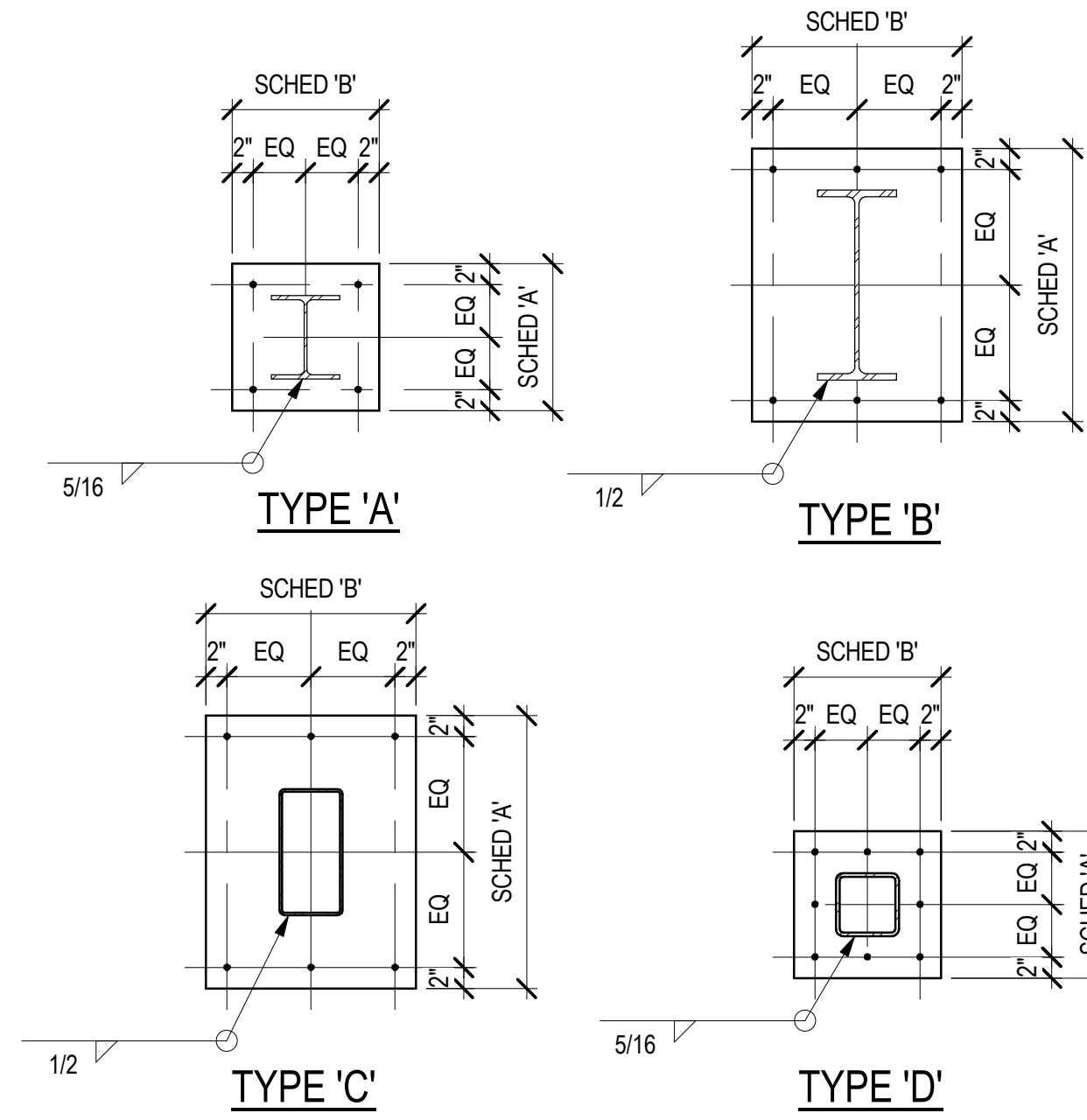
SLAB-ON-GRADE SCHEDULE					
MARK	SLAB		REINFORCING	BEARING STRATA	COMMENTS
	THICKNESS	MATL			
S5	5"	CONC	#4 @ 18" OC EA WAY	15 MIL VAPOR RETARDER OVER 1/2" SAND BLOTTER LAYER OVER 4" COMPACTED GRANULAR FILL OVER 14" OF COMPACTED STRUCTURAL FILL OVER COMPACTED SUBGRADE. SUBGRADE WILL BE PLACED WITH LASER LEVEL.	PREPARE SUBGRADE AND STRUCTURAL FILL PER GEOTECHNICAL REPORT
S6	6"	CONC	#4 @ 12" OC EA WAY	15 MIL VAPOR RETARDER OVER 1/2" SAND BLOTTER LAYER OVER 4" COMPACTED GRANULAR FILL OVER 14" OF COMPACTED STRUCTURAL FILL OVER COMPACTED SUBGRADE. SUBGRADE WILL BE PLACED WITH LASER LEVEL.	PREPARE SUBGRADE AND STRUCTURAL FILL PER GEOTECHNICAL REPORT

BASE PLATE SCHEDULE				
MARK	TYPE	SIZE "X"X"X"X"	ANCHOR BOLTS	
			F1554	TYPE
BP1	A	PL 1 1/4"x18"x1"-6"	4- 3/4" DIA x 9"	GRAVITY
BP2	A	PL 1 3/4"x18"x1"-6"	4- 3/4" DIA x 9"	GRAVITY
BP3	A	PL 1 3/4"x20"x1"-8"	4- 3/4" DIA x 9"	GRAVITY
BP4	A	PL 1 3/4"x22"x1"-10"	6- 1" DIA x 18"	GRAVITY
BP5	A	PL 3/4"x14"x1"-2"	4- 3/4" DIA x 9"	GRAVITY
BP6	B	PL 1 3/4"x20"x1"-8"	6- 1" DIA x 18"	LATERAL
BP7	B	PL 1 3/4"x22"x1"-8"	6- 1" DIA x 18"	LATERAL
BP8	C	PL 1 1/4"x20"x1"-4"	6- 1" DIA x 18"	LATERAL
BP9	D	PL 3/4"x14"x1"-2"	4- 3/4" DIA x 9"	GRAVITY
BP10	D	PL 3/4"x14"x1"-2"	8- 3/4" DIA x 9"	GRAVITY



GRAVITY ANCHOR BOLT

LATERAL ANCHOR BOLT



WALL SCHEDULE						
MARK	VENEER	WALL	REINFORCING			COMMENTS
			VERTICAL	HORIZONTAL	GRADE	
WC8	--	8" CONC	#4 @ 12" OC	#4 @ 12" OC	A615	
WC12	SEE ARCH	12" CONC	#5 @ 12" OC EA FACE	#5 @ 12" OC EA FACE	A615	
WM12	SEE ARCH	12" CMU	#7 @ 16" OC EA FACE	#5 @ 24" OC EA FACE & STD LADDER TYPE JOINT REINF @ 16" OC	A615	GROUT ALL CELLS SOLID. SEE 7.21 FOR MASONRY DETAILS. D4/S7.21 FOR HORIZONTAL REINFORCEMENT LOCATION

REINFORCEMENT TYPE	REQUIRED LAP SPLICES ACI 318-14/IBC 2015						MINIMUM LENGTH (IN)	COMMENTS
	#6 AND SMALLER (#6)	#7 THROUGH #11 (#6b)	3000PSI	4000PSI	5000PSI	3000PSI		
CONTINUOUS WALL FOOTINGS AND HORIZONTAL REINFORCEMENT IN SITE WALLS	30	30	30	30	30	30	18	
CONCRETE WALLS: ALL VERTICAL REINFORCEMENT	44	38	34	55	48	43	12	
CONCRETE WALLS: ALL HORIZONTAL REINFORCEMENT, EXCLUDING SITE WALLS AND STEM WALLS	57	50	45	72	62	56	12	
CONCRETE COLUMNS	44	38	34	55	48	43	12	
TOP FLEXURAL REINFORCEMENT, INCLUDING BEAMS, GRADE BEAMS, AND COMBINED COLUMN FOOTINGS	57	50	45	72	62	56	12	
BOTTOM FLEXURAL REINFORCEMENT, INCLUDING BEAMS, GRADE BEAMS, AND COMBINED COLUMN FOOTINGS	44	38	34	55	48	43	12	
MINIMUM EMBEDMENT OF STANDARD HOOKS INTO CONCRETE	22	19	17	22	19	17	6	ALLOWED FOR BARS LARGER THAN #11
SLABS ON GRADE	30	30	30	30	30	30	12	WWF MINIMUM LAP LENGTH = 6 IN
SLABS ON METAL DECK	30	30	30	30	30	30	12	

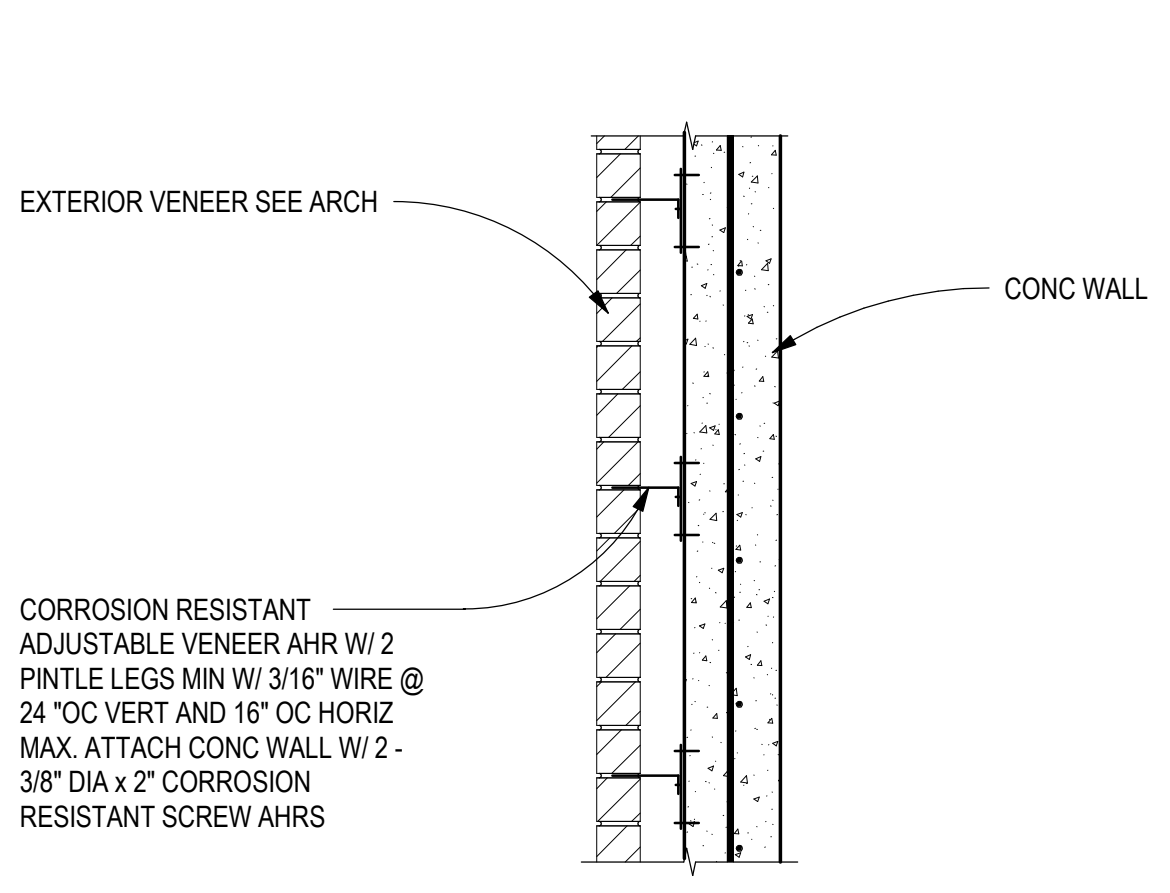
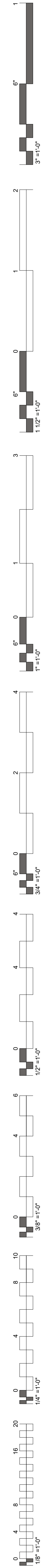
- NOTES:
- LAP SPLICES SHALL NOT BE PERMITTED FOR BARS LARGER THAN #11.
 - LAP SPLICES FOR BUNDLED BARS SHALL BE IN ACCORDANCE WITH ACI 318-14 SECTION 25.5.1.4
 - LAP LENGTHS FOR LIGHTWEIGHT CONCRETE SHALL BE INCREASED BY 33%
 - LAP LENGTHS FOR EPOXY COATED BARS SHALL BE INCREASED BY 50%
 - FOR INTERMEDIATE OR LARGER VALUES OF F_c, USE THE CLOSEST LOWER VALUE IN THE TABLE. DO NOT INTERPOLATE

MASONRY LAP SPLICES (#6) ACI 530-13/IBC 2015								
	#3	#4	#5	#6	#7	#8	#9	
8" BLOCK WITH 1-LAYER OF REINFORCEMENT	32	40	51	72	N/A	N/A	N/A	
8" BLOCK WITH 2-LAYERS OF REINFORCEMENT	51	68	72	72	N/A	N/A	N/A	
12" BLOCK WITH 1-LAYER OF REINFORCEMENT	32	24	23	37	43	57	65	
12" BLOCK WITH 2-LAYERS OF REINFORCEMENT	51	68	72	72	72	N/A	N/A	
16" BLOCK WITH 1-LAYER OF REINFORCEMENT	32	24	23	30	32	42	48	
16" BLOCK WITH 2-LAYERS OF REINFORCEMENT	51	68	72	72	72	72	72	

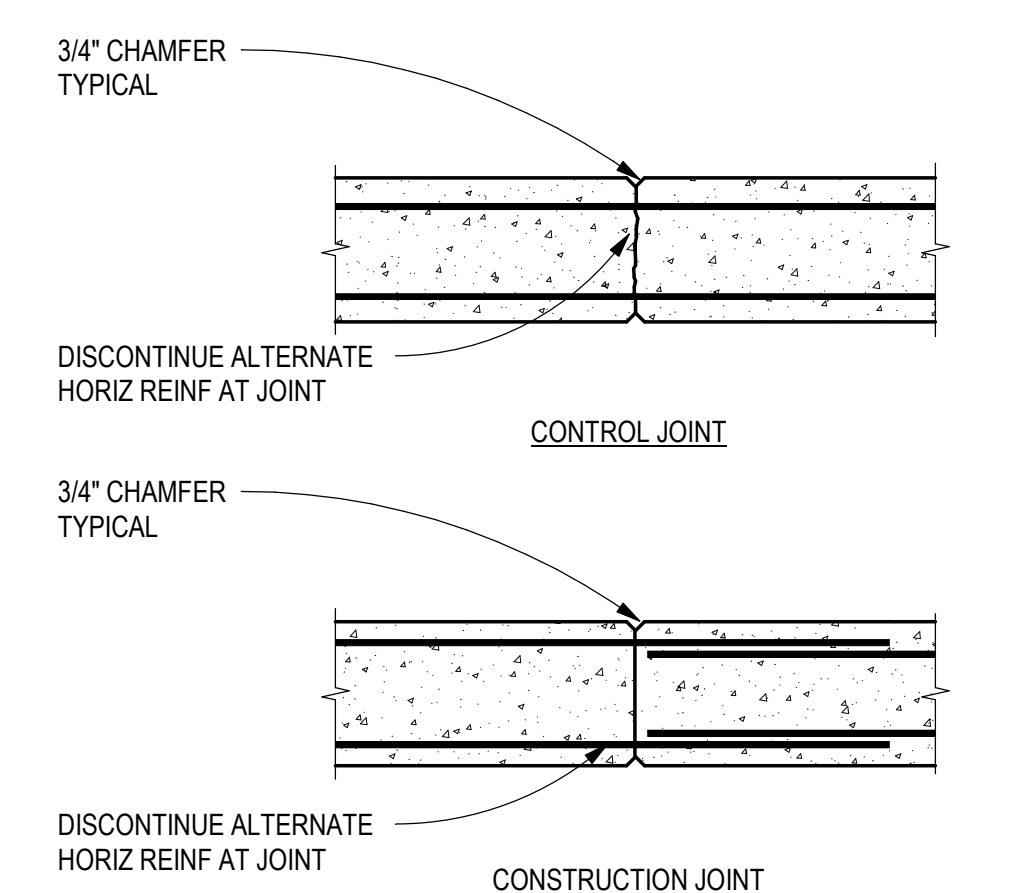
BARS LARGER THAN #9 SHALL BE SPLICED USING MECHANICAL CONNECTIONS

SPOT FOOTING SCHEDULE						
MARK	WIDTH	SIZE		REINFORCING		COMMENTS
		LENGTH	DEPTH	REINFORCING	GRADE	
F48	4'-0"	4'-0"	1'-0"	4- #5 EA WAY BOT	A615	
F48P	4'-0"	4'-0"	1'-0"	4- #5 EA WAY BOT	A615	
F60	5'-0"	5'-0"	1'-6"	5- #6 EA WAY BOT	A615	
F60A	5'-0"	5'-0"	2'-0"	6- #6 EA WAY TOP & BOT	A615	
F72	6'-0"	6'-0"	1'-6"	6- #6 EA WAY BOT	A615	
F72A	6'-0"	6'-0"	2'-0"	8- #6 EA WAY TOP & BOT	A615	TOP BARS TO HAVE STD HOOKS AT ENDS
F72P	6'-0"	6'-0"	1'-6"	6- #6 EA WAY TOP & BOT	A615	TOP BARS TO HAVE STD HOOKS AT ENDS
F84	7'-0"	7'-0"	2'-0"	9- #6 EA WAY BOT	A615	
F84A	7'-0"	7'-0"	2'-0"	9- #6 EA WAY TOP & BOT	A615	TOP BARS TO HAVE STD HOOKS AT ENDS
F84P	7'-0"	7'-0"	2'-0"	9- #6 EA WAY TOP & BOT	A615	TOP BARS TO HAVE STD HOOKS AT ENDS
F96	8'-0"	8'-0"	2'-0"	7- #7 EA WAY BOT	A615	
F96A	8'-0"	8'-0"	2'-0"	7- #7 EA WAY TOP & BOT	A615	TOP BARS TO HAVE STD HOOKS AT ENDS
F276	23'-0"	21'-0"	2'-9"	#8 @ 9" OC EA WAY TOP & BOT	A615	TOP BARS TO HAVE STD HOOKS AT ENDS

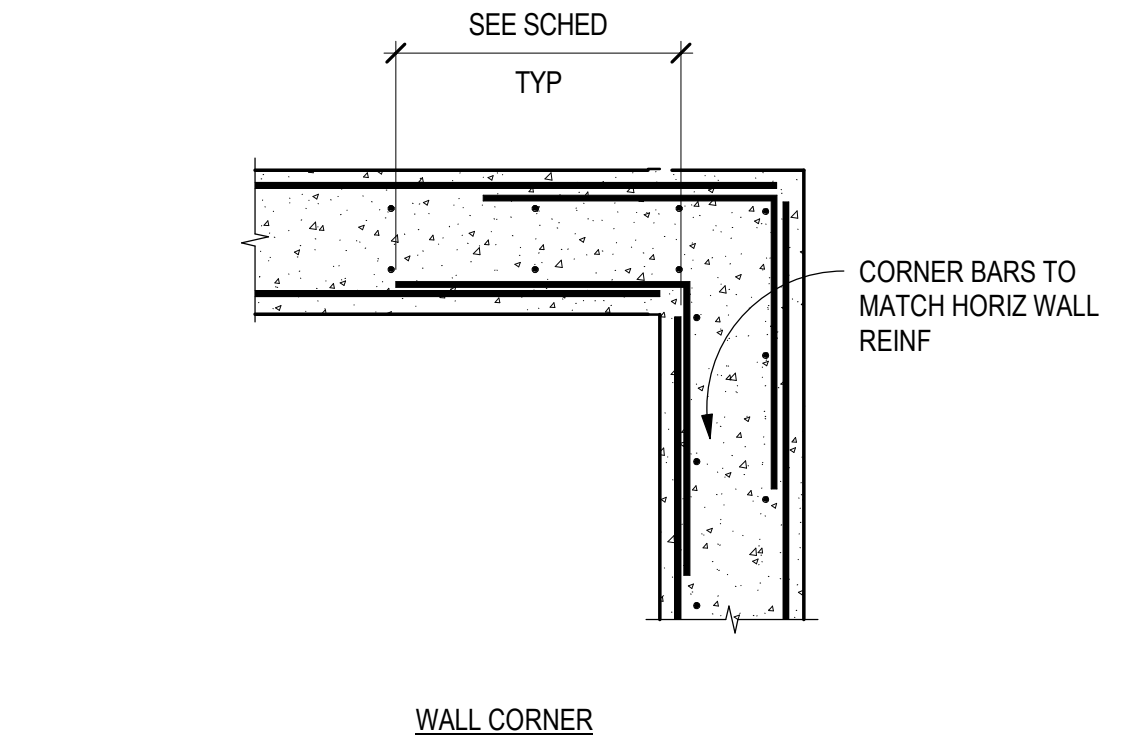
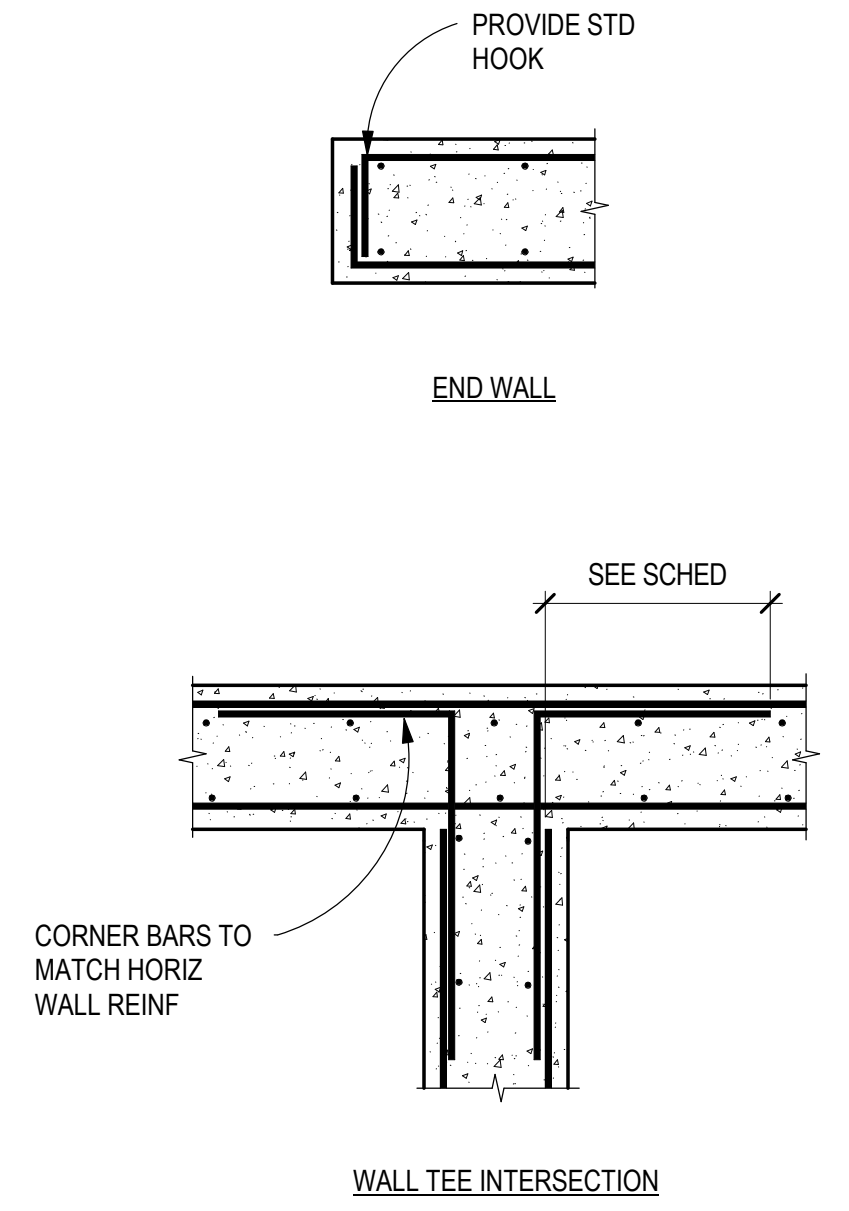
CONTINUOUS FOOTING SCHEDULE					
MARK	WIDTH	DEPTH	REINFORCING		COMMENTS
			CONTINUOUS	TRANSVERSE	
CF16	1'-4"	1'-0"	3-#4	#4 @ 48" OC	
CF24	2'-0"	1'-0"	3-#4	#4 @ 48" OC	
CF84	7'-0"	2'-9"	8-#8	#8 @ 9" OC	TOP BARS TO HAVE STD HOOKS AT ENDS



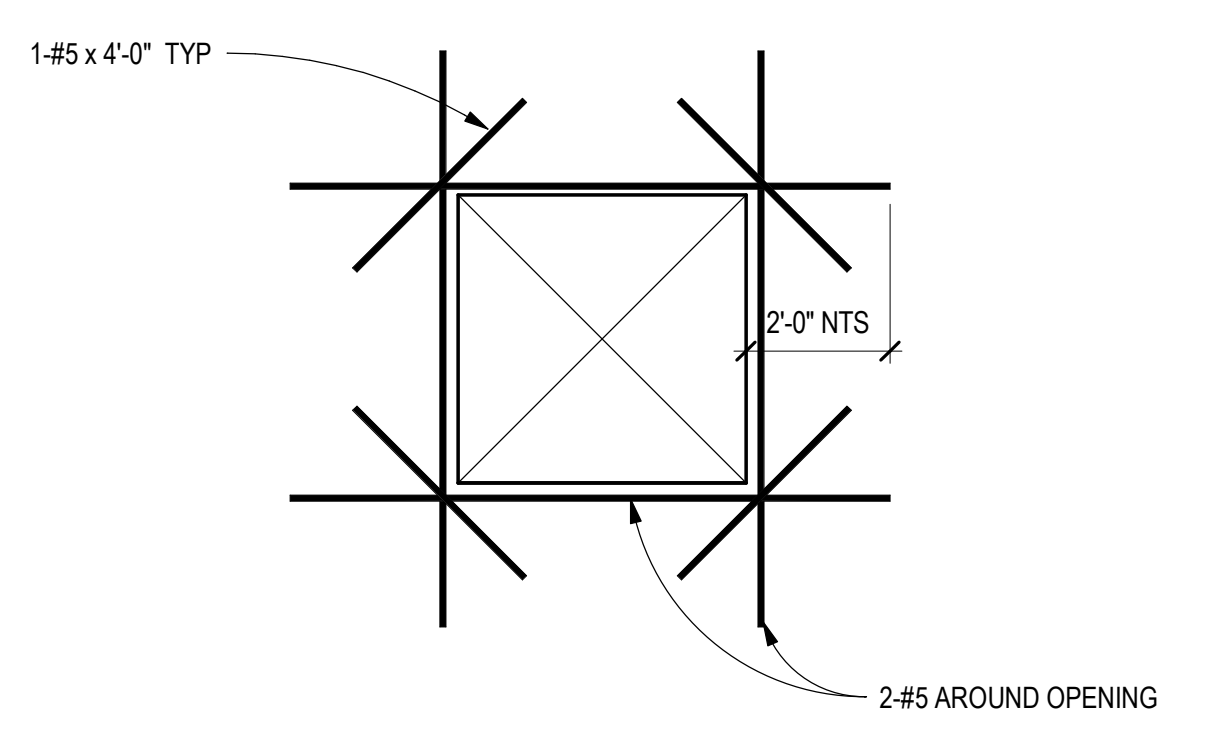
D1 TYPICAL VENEER TO CONC WALL
SCALE: NTS



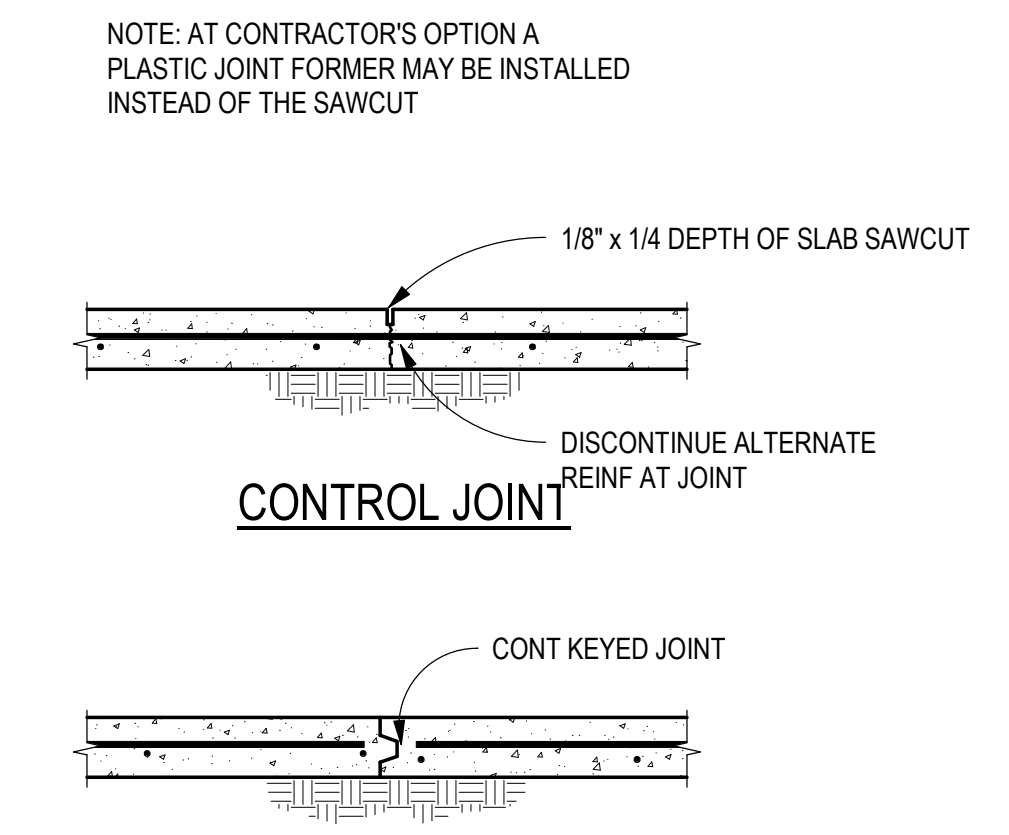
D2 TYPICAL WALL JOINT DETAIL
SCALE: NTS



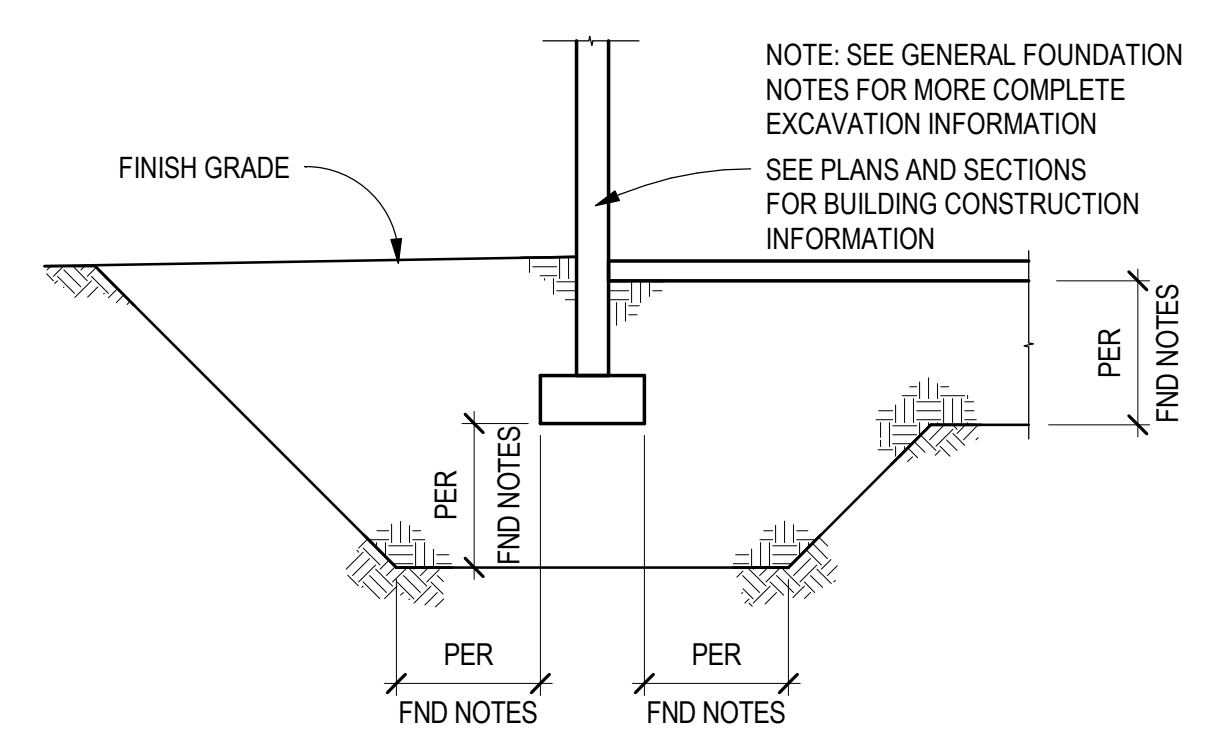
C3 TYPICAL DOUBLE MAT WALL REINF
SCALE: NTS



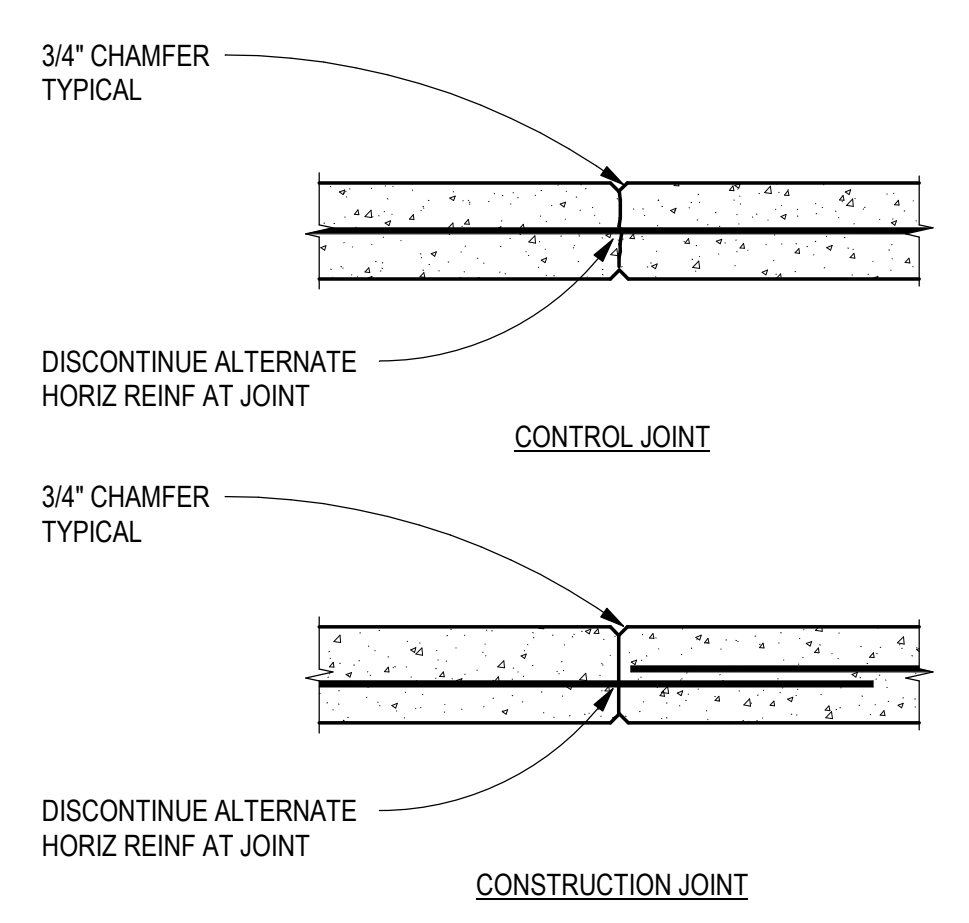
D4 TYPICAL OPNG IN CONC WALL DETAIL
SCALE: NTS



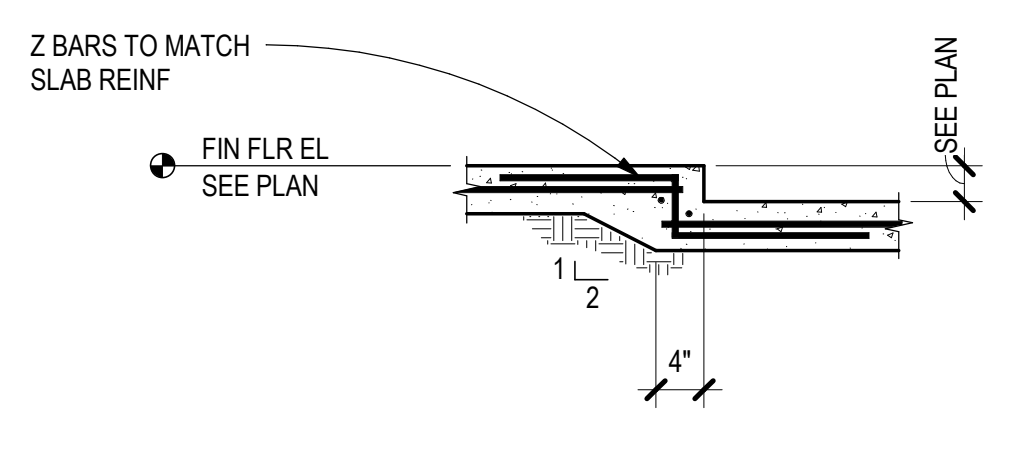
D5 TYPICAL SLAB JOINT
SCALE: NTS



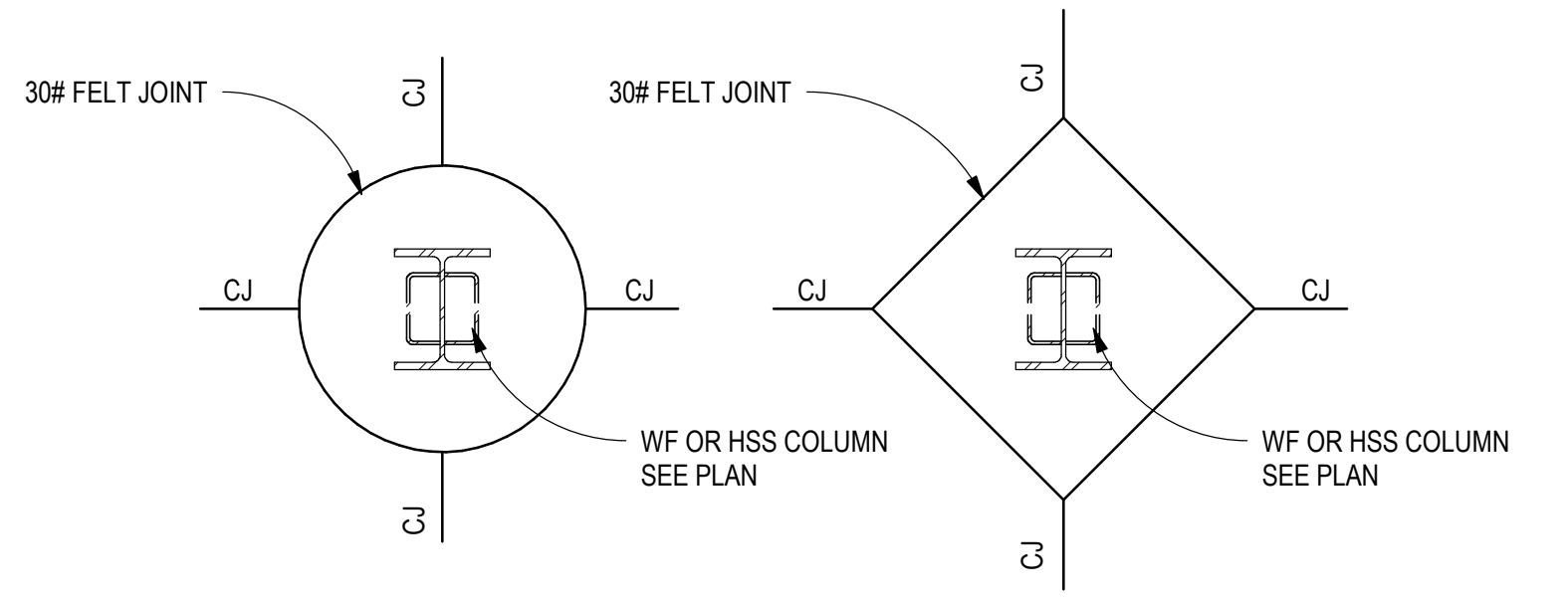
C1 TYPICAL FND EXCAVATION DETAIL
SCALE: NTS



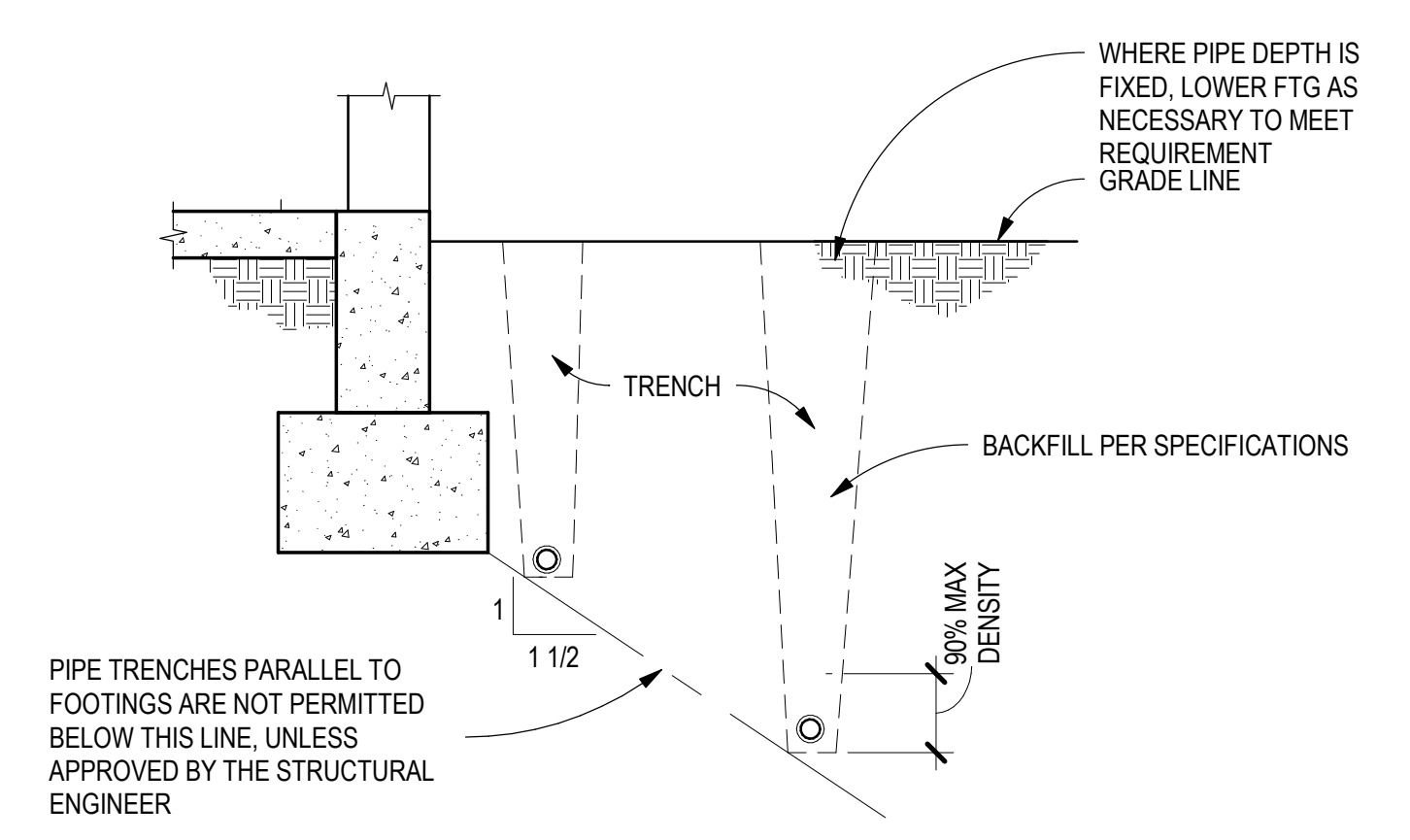
C2 TYPICAL WALL JOINT DETAIL
SCALE: NTS



C4 TYPICAL DEPRESSED SLAB
SCALE: 3/4\"/>



C5 TYPICAL COLUMN BLOCKOUT
SCALE: NTS



A1 TYPICAL PIPE PENETRATION AND TRENCH DETAILS
SCALE: NTS

PIPE AND TRENCHES PARALLEL TO FOOTINGS

NOTE: ALL PIPE SLEEVES SHALL BE 2\"/>

NOTE: PIPE DEPTH IS FIXED, LOWER FTG AS NECESSARY TO MEET REQUIREMENT GRADE LINE

WHERE PIPE DEPTH IS FIXED, LOWER FTG AS NECESSARY TO MEET REQUIREMENT GRADE LINE

BACKFILL PER SPECIFICATIONS

PIPE TRENCHES PARALLEL TO FOOTINGS ARE NOT PERMITTED BELOW THIS LINE, UNLESS APPROVED BY THE STRUCTURAL ENGINEER

PIPE SLEEVE TYP

NO PIPE SHALL PASS THRU CONTINUOUS FOOTING

PIPE AND TRENCHES TRANSVERSE TO FOOTINGS

NOTES:

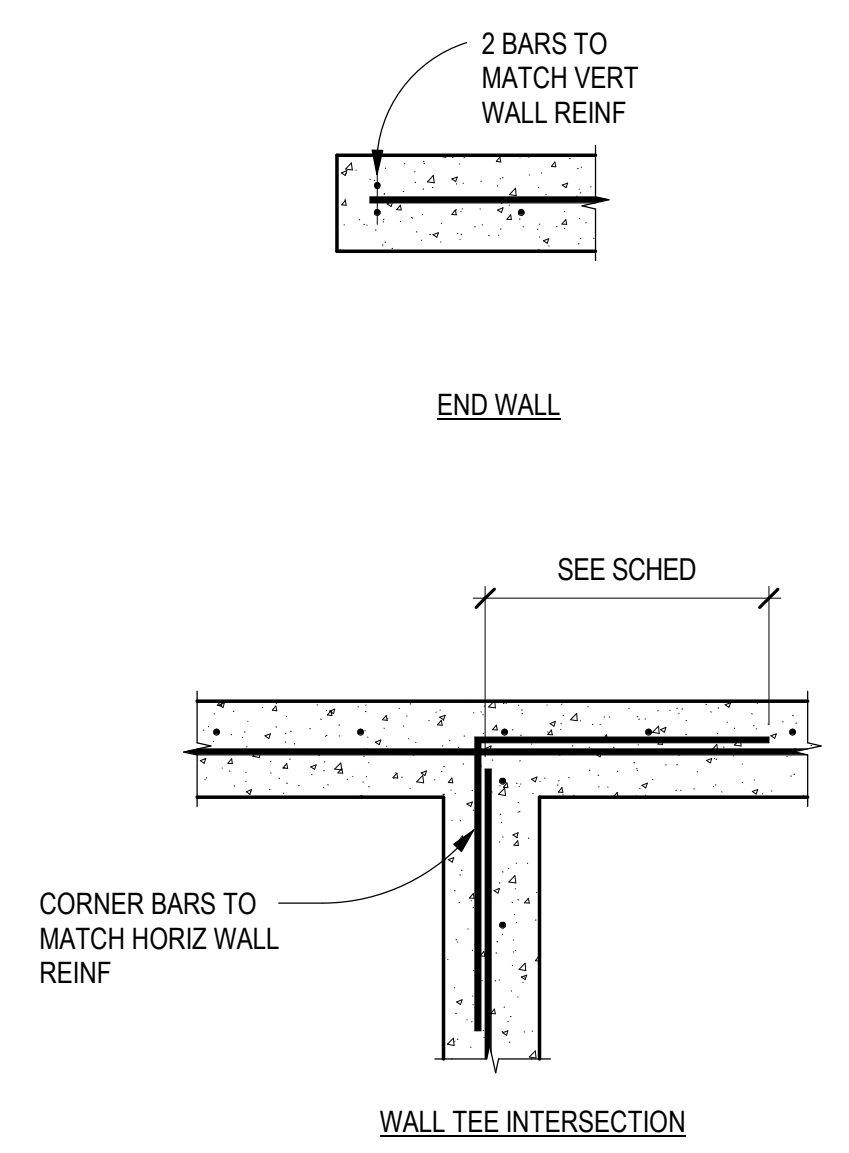
- FOR PIPES MORE THAN 3\"/>
- TRENCHES AND PIPES ARE NOT PERMITTED BELOW COLUMN FTGS

FOR PIPE WITHIN 3\"/>

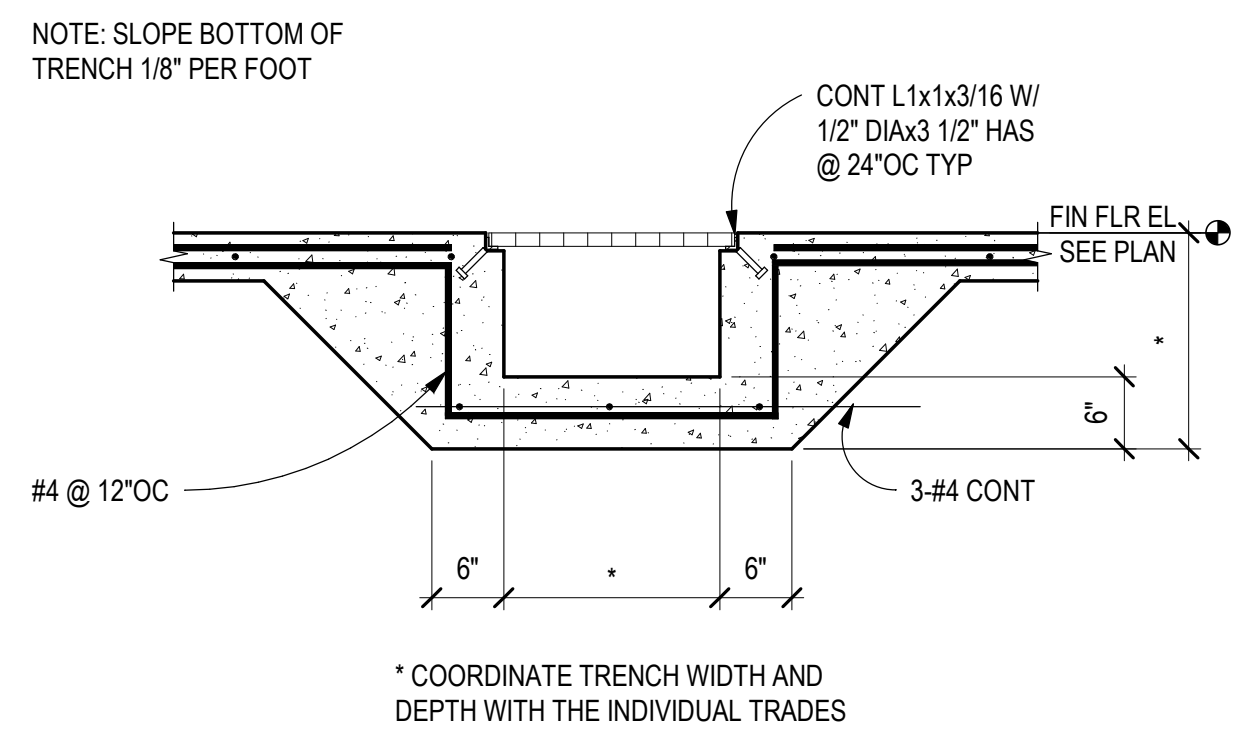
MIN ELEVATIO N

OUTLINE OF CONT FOOTING

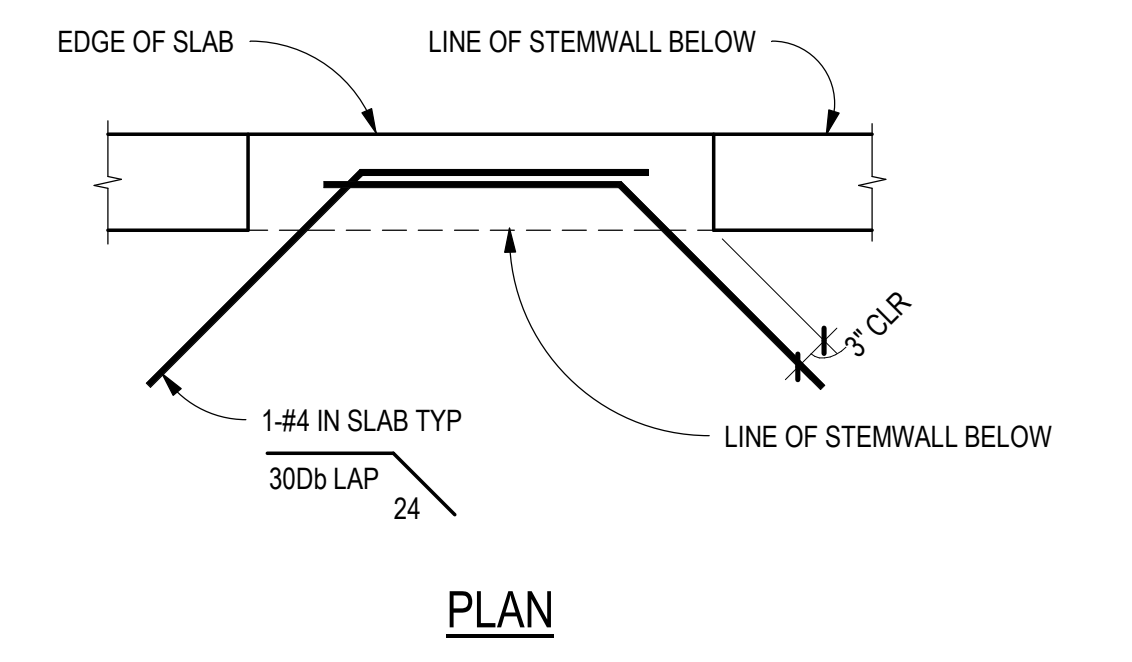
PLACE CONC FILL AROUND SLEEVES BEFORE CASTING FOOTING



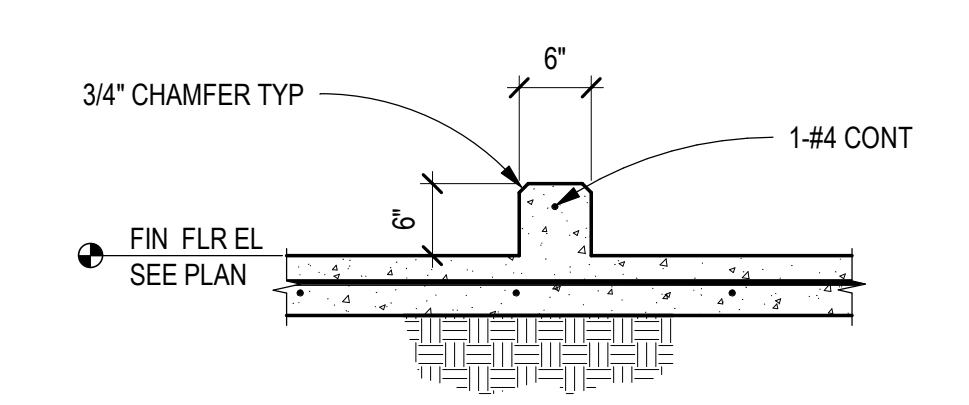
A3 TYPICAL SINGLE MAT WALL REINF
SCALE: NTS



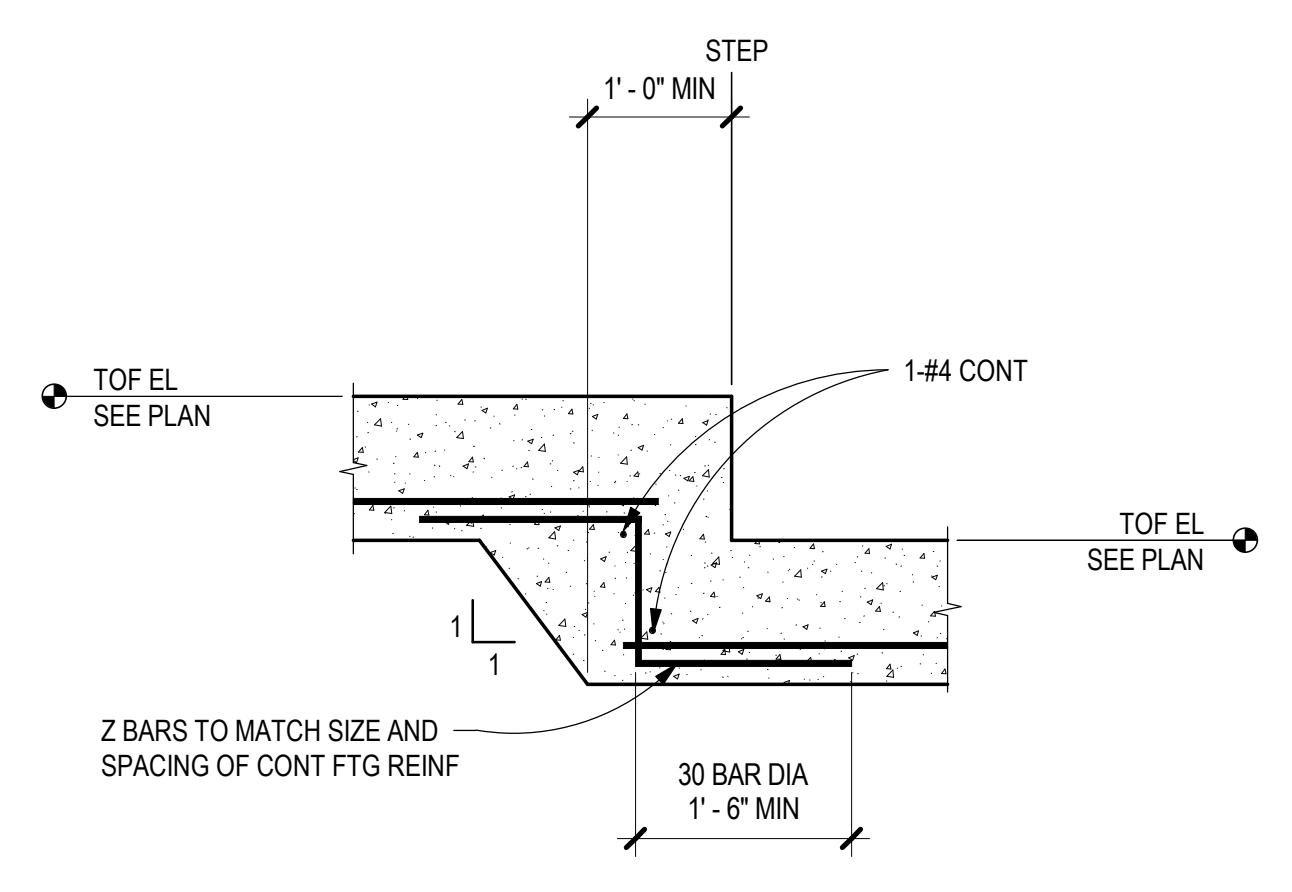
B4 TYPICAL TRENCH SECTION
SCALE: NTS



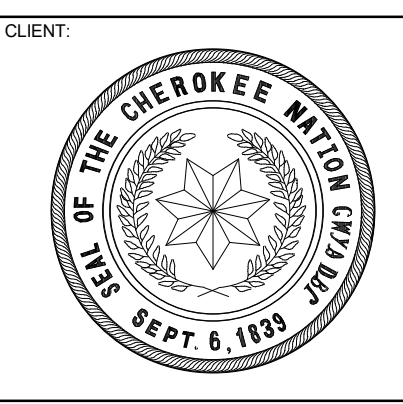
B5 TYPICAL SLAB REINF AT OPNG
SCALE: NTS



A4 TYPICAL CURB SECTION
SCALE: NTS



A5 TYPICAL STEPPED FOOTING DETAIL
SCALE: NTS



**WILMA P. MANKILLER HEALTH CENTER
EXPANSION**
STILWELL, OKLAHOMA

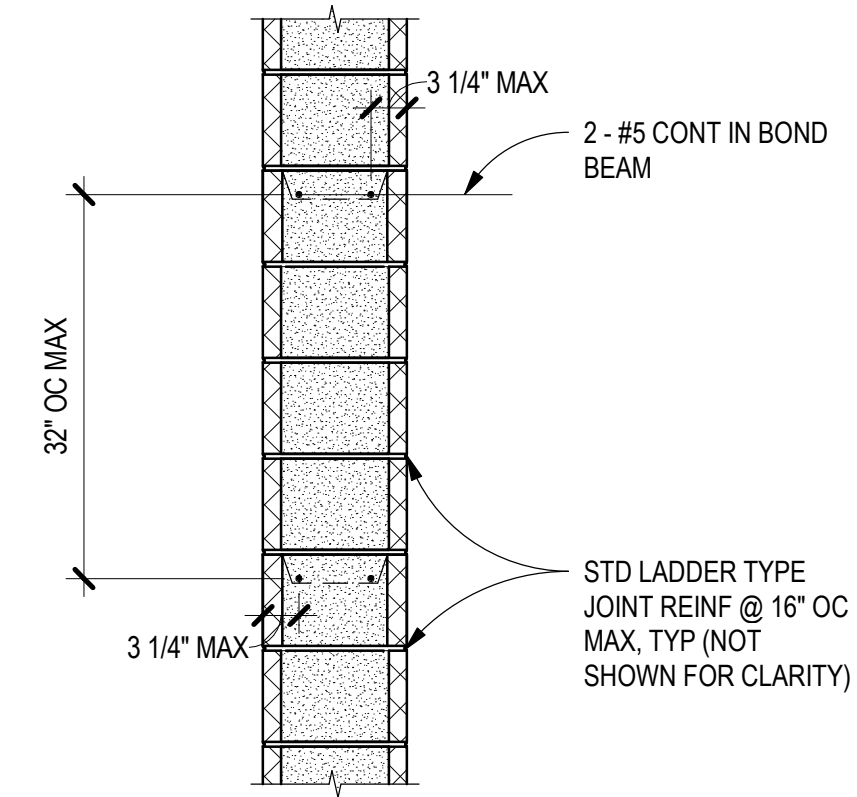
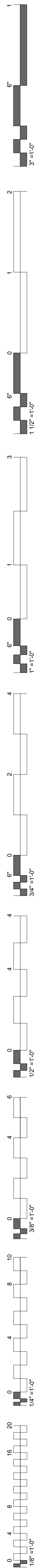
KEY PLAN:

PROJECT PHASE:
BID PACKAGE 01

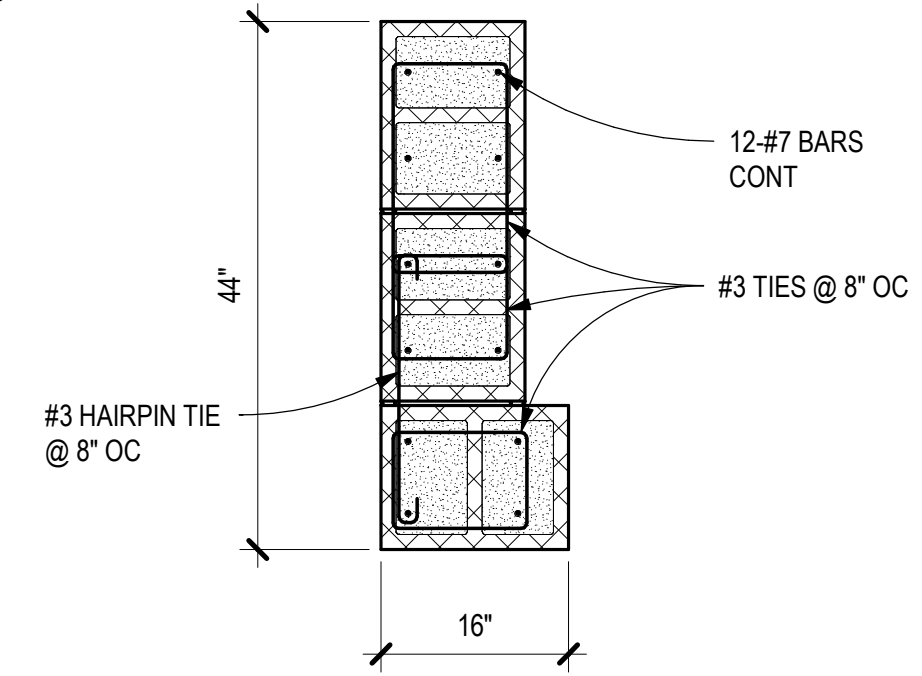
#	DATE	REVISIONS	DESCRIPTION

DATE: 11-01-19
JOB NUMBER: 18-01.01

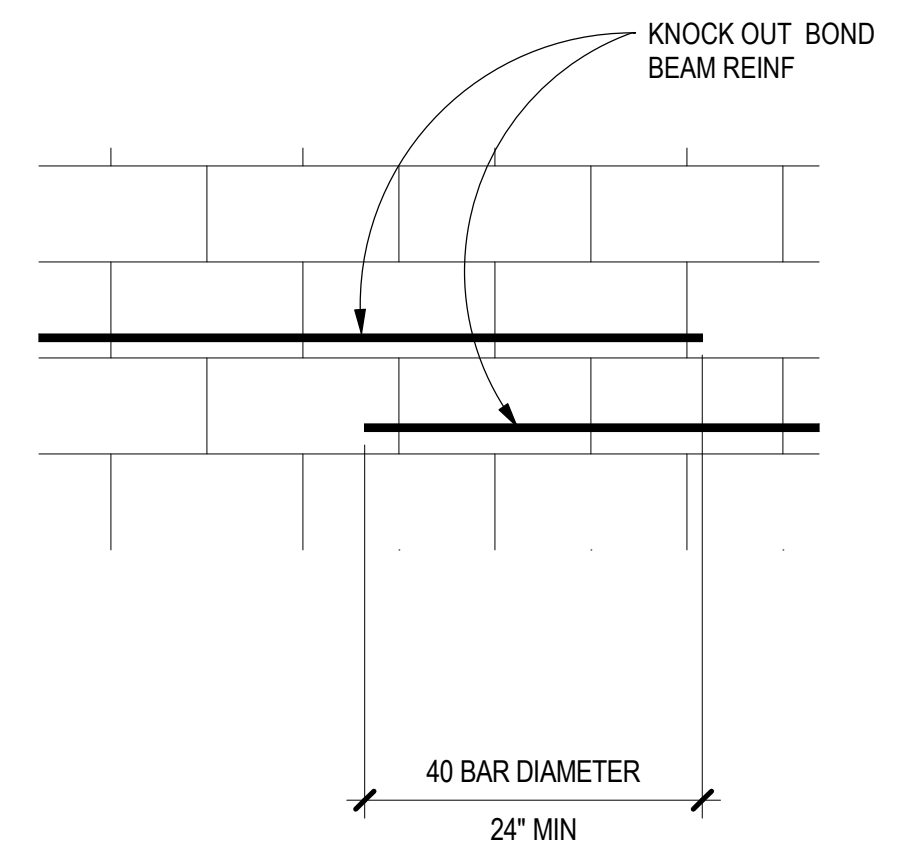
SHEET NUMBER:
S7.11
TYPICAL CONCRETE
DETAILS



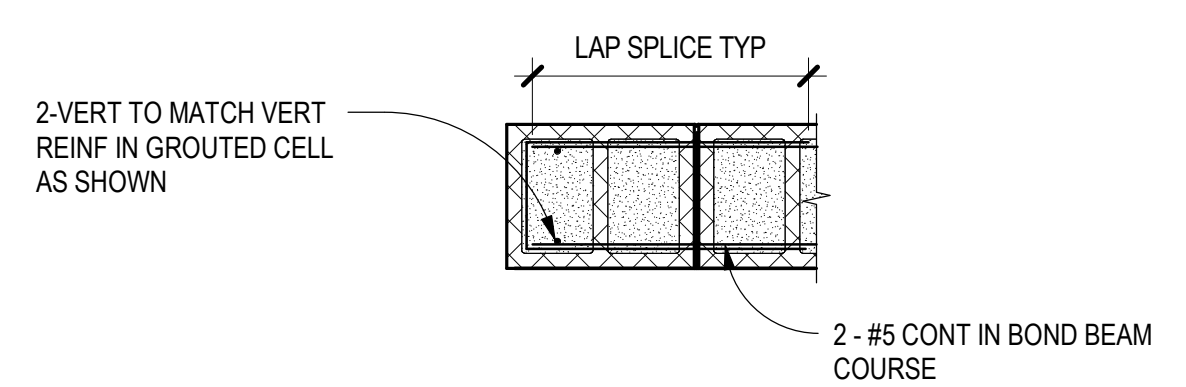
D4 TYPICAL BOND BEAM DETAIL
SCALE: 3/4" = 1'-0"



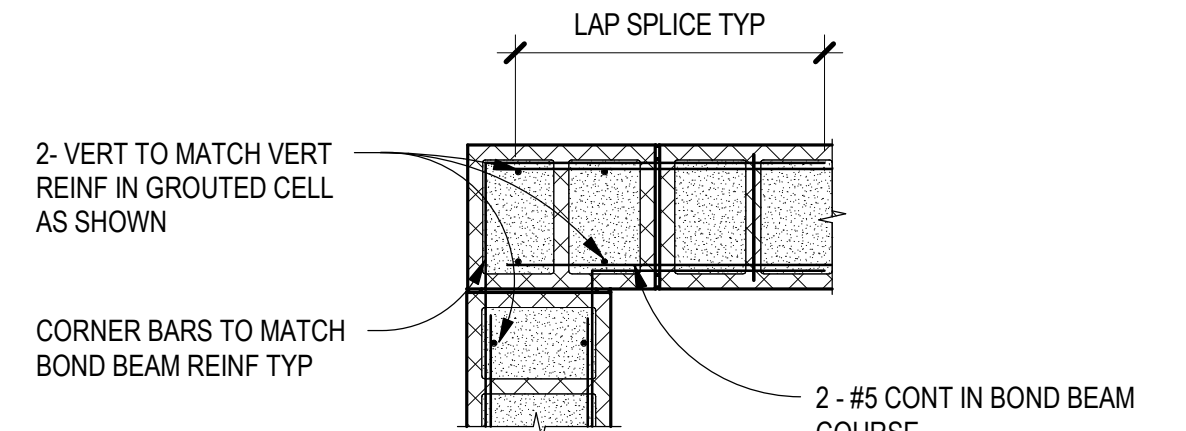
C4 PILASTER DETAIL
SCALE: 3/4" = 1'-0"



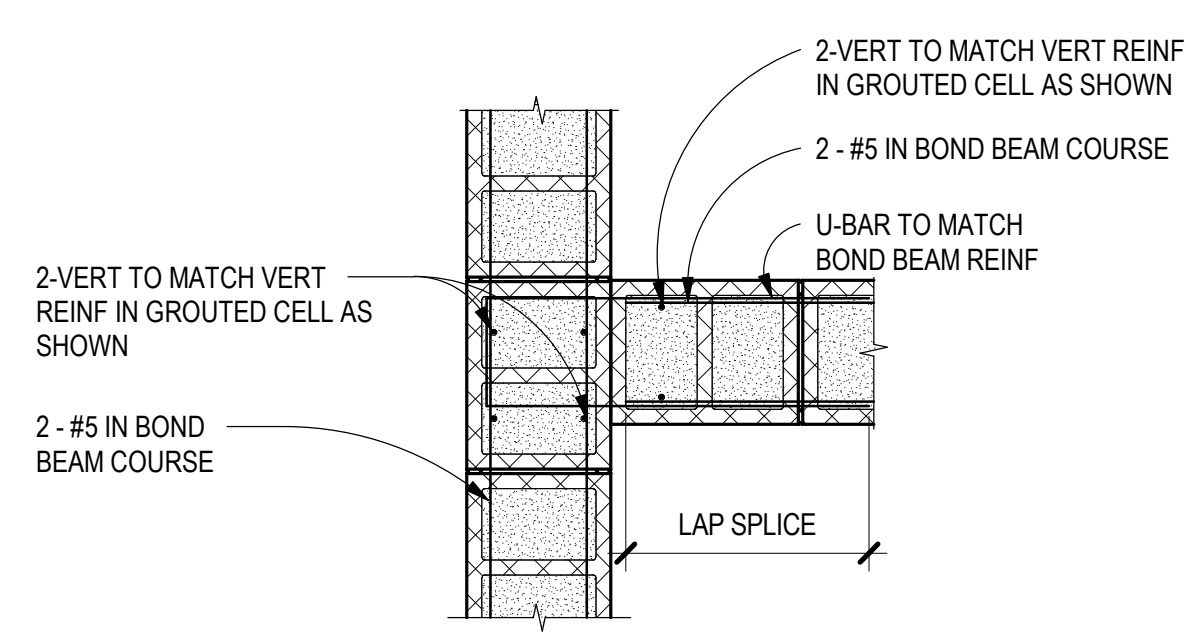
B4 TYPICAL STEP IN BOND BEAM
SCALE: 3/4" = 1'-0"



BOND BEAM AT WALL END

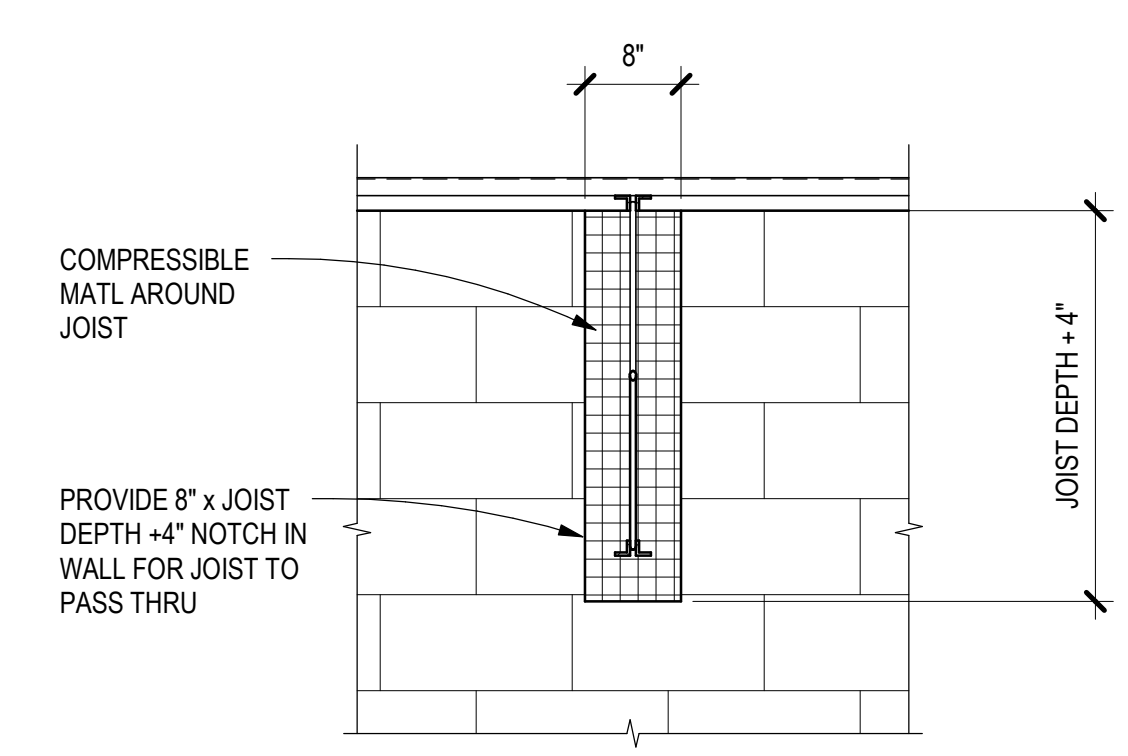


BOND BEAM AT WALL CORNER

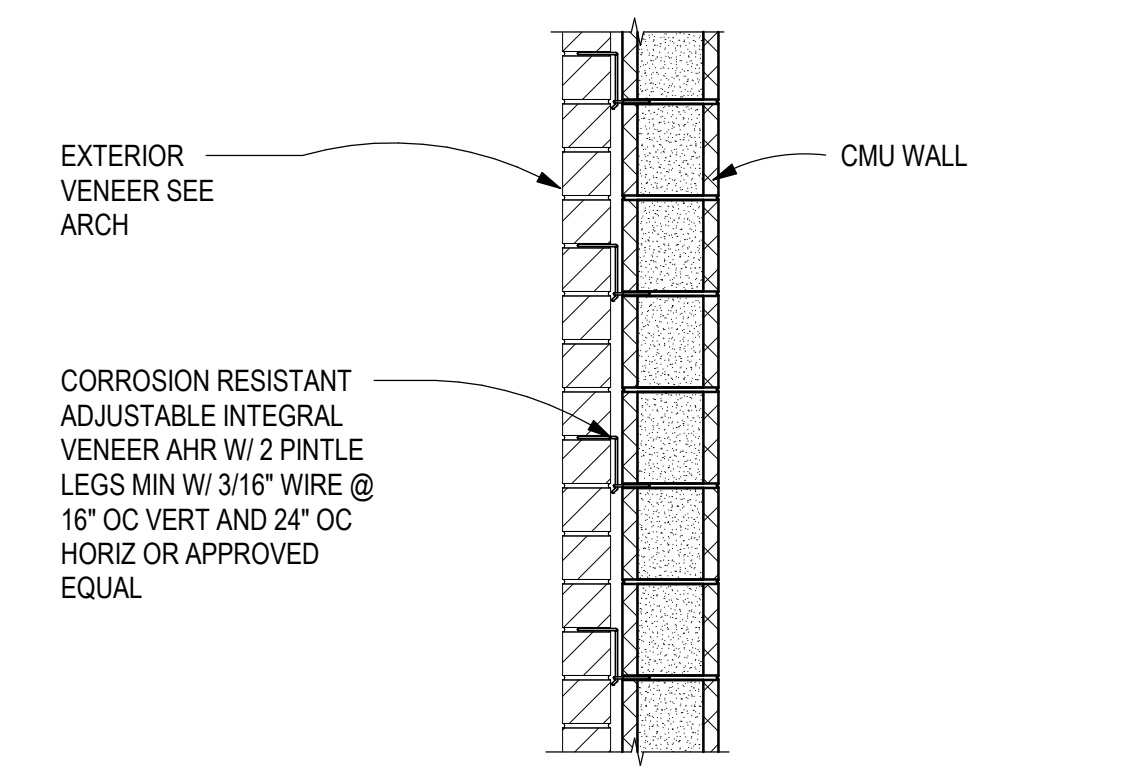


BOND BEAM AT WALL TEE

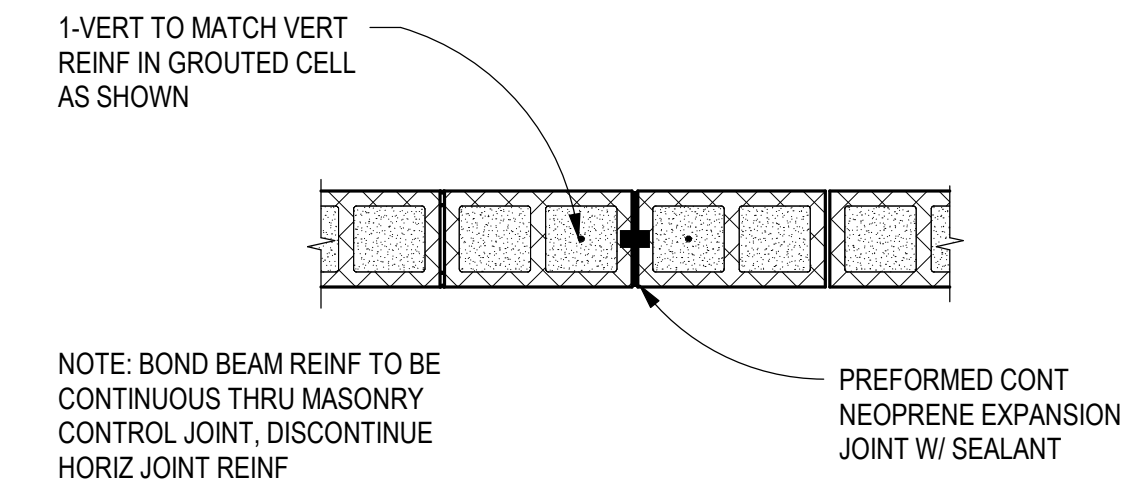
A4 TYPICAL 12" CMU PLAN DETAILS
SCALE: 3/4" = 1'-0"



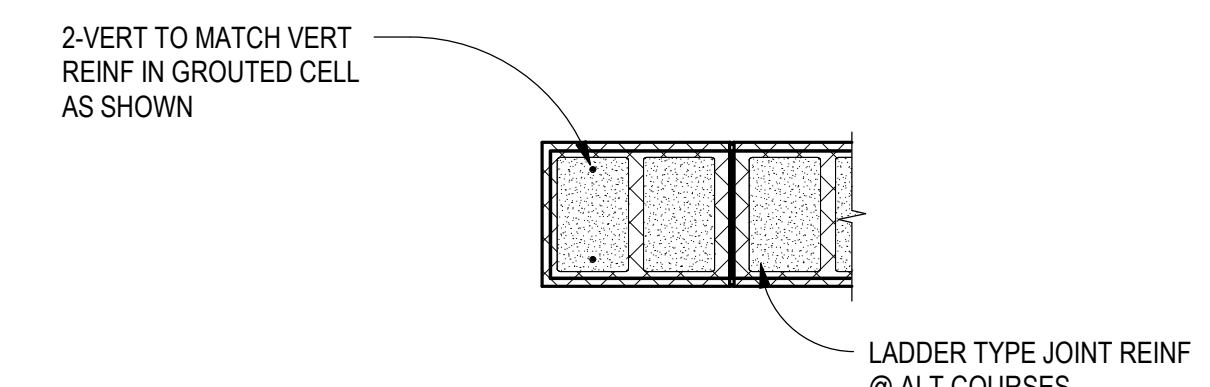
D5 TYPICAL JOIST THRU CMU WALL
SCALE: 3/4" = 1'-0"



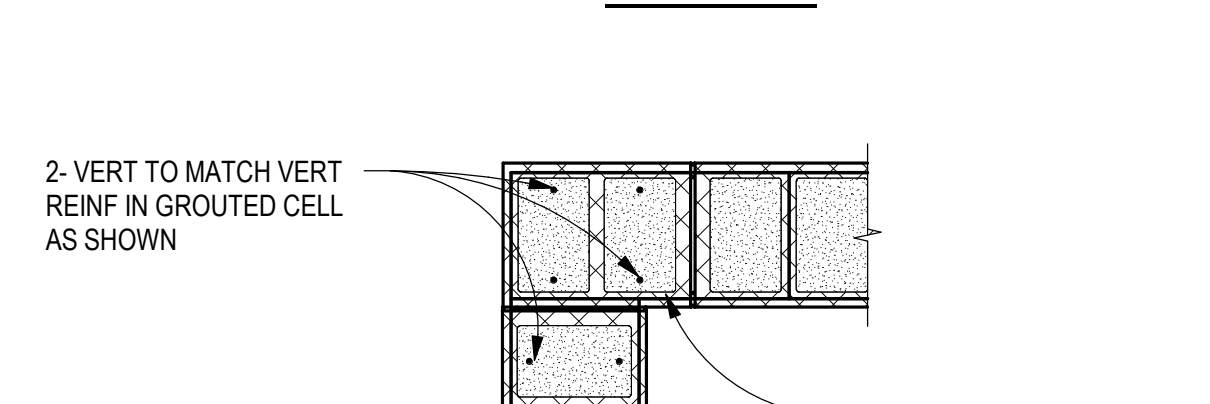
C5 TYPICAL VENEER TO CMU WALL DETAIL
SCALE: 3/4" = 1'-0"



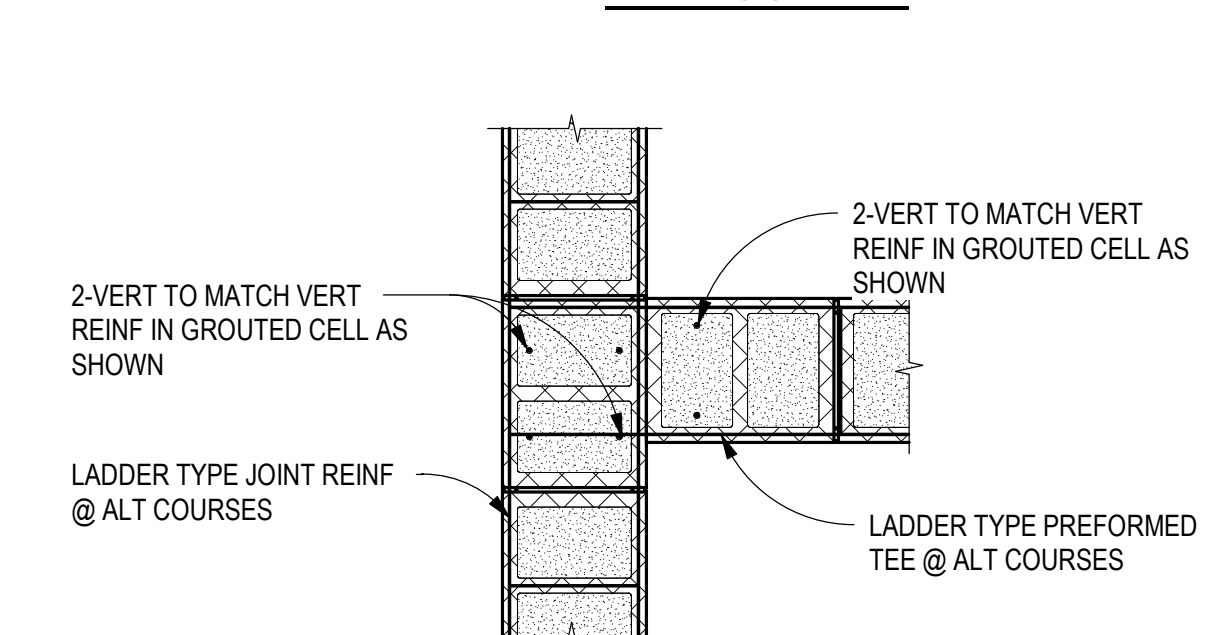
B5 TYPICAL CMU CNRTL JOINT (MCJ)
SCALE: 3/4" = 1'-0"



WALL END

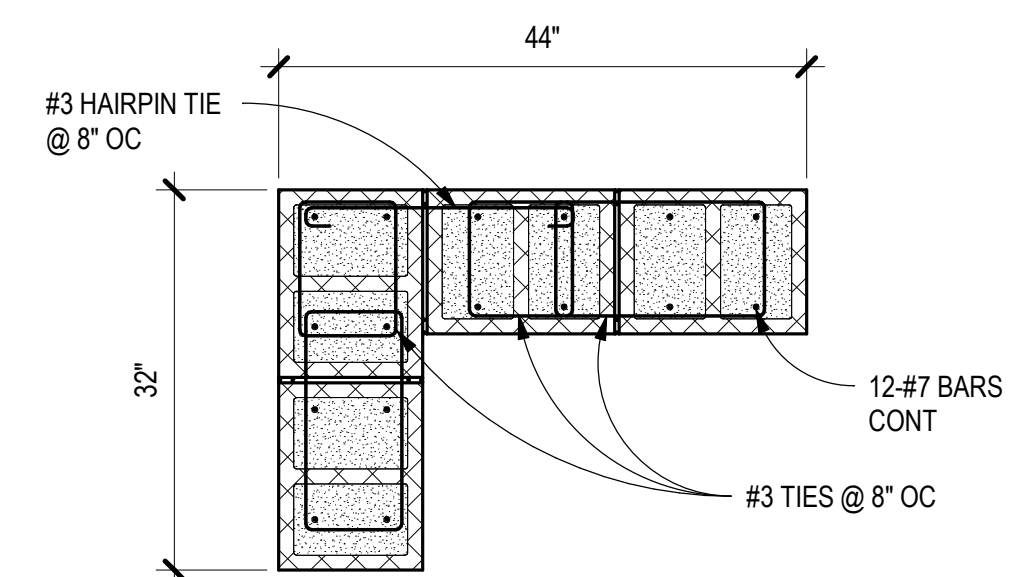


WALL CORNER



WALL TEE

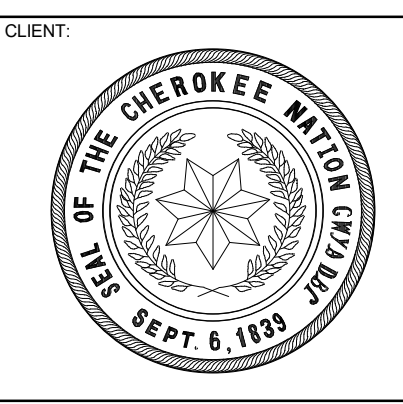
A5 TYPICAL 12" CMU PLAN DETAILS
SCALE: 3/4" = 1'-0"



A3 PILASTER DETAIL
SCALE: 3/4" = 1'-0"

NOTES:
1. ALL PILASTER REINFORCING IS IN ADDITION TO REINFORCING SPECIFIED IN WALL SCHEDULE.
2. ALL REINFORCING SHALL BE CONTINUOUS ALONG ENTIRE HEIGHT OF WALL, UNLESS NOTED OTHERWISE.

PLAN VIEW



**WILMA P. MANKILLER HEALTH CENTER
EXPANSION**
STILWELL, OKLAHOMA

KEY PLAN:

PROJECT PHASE:
BID PACKAGE 01

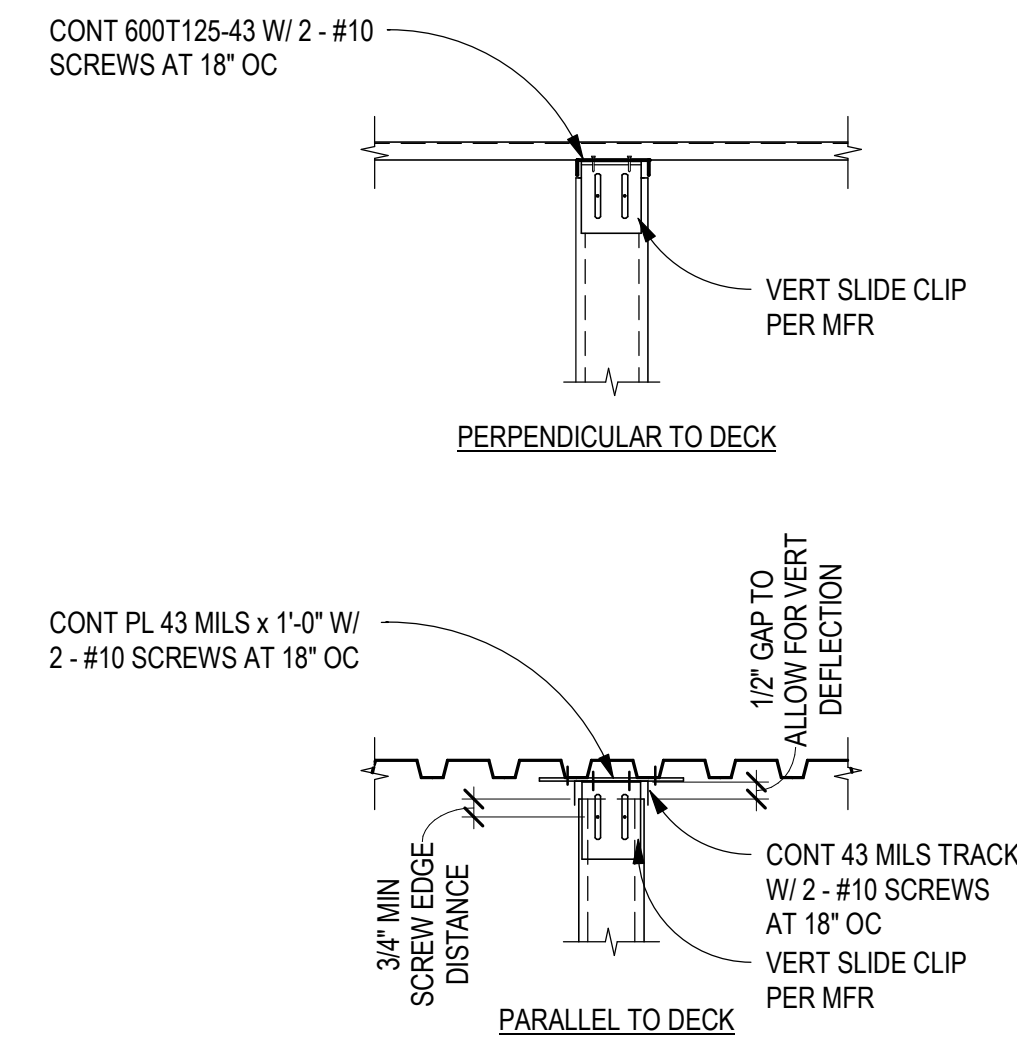
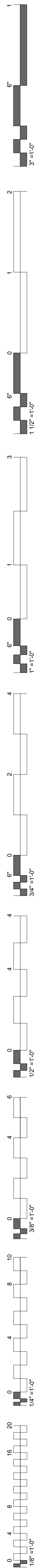
#	DATE	REVISIONS	DESCRIPTION

DATE: 11-01-19 JOB NUMBER: 18-01.01

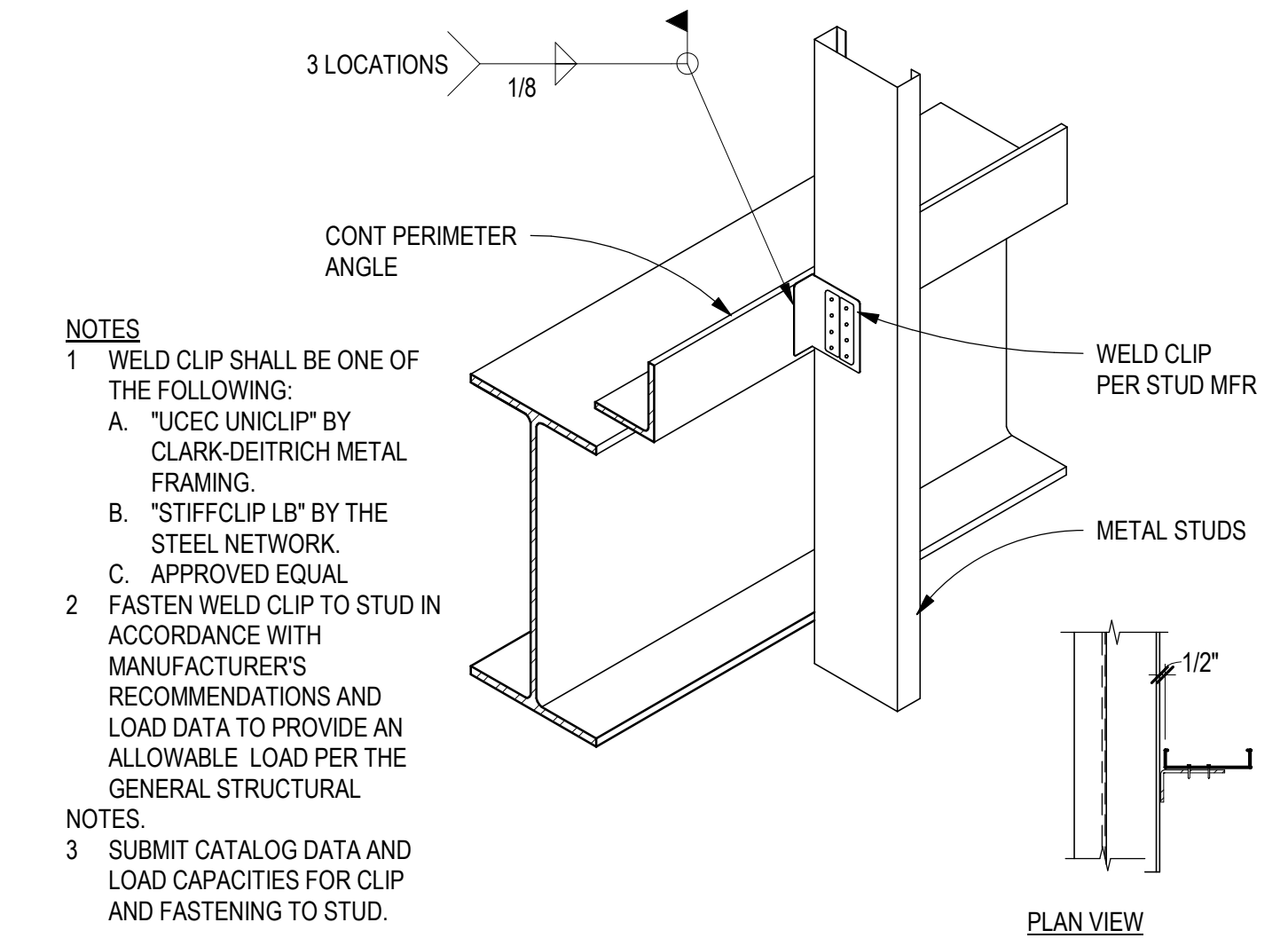
SHEET NUMBER:

S7.21

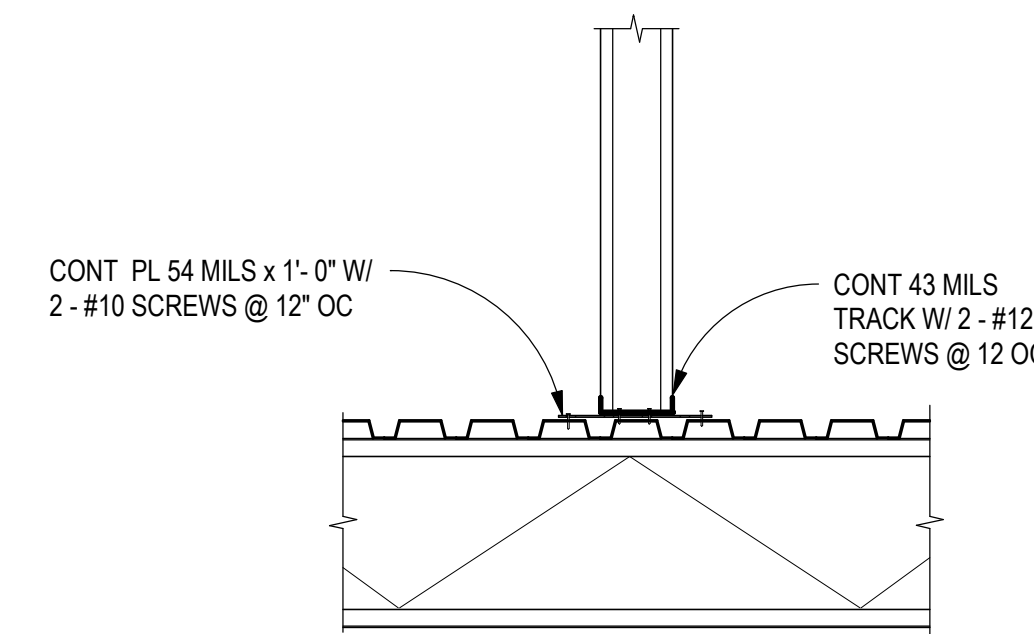
TYPICAL MASONRY DETAILS



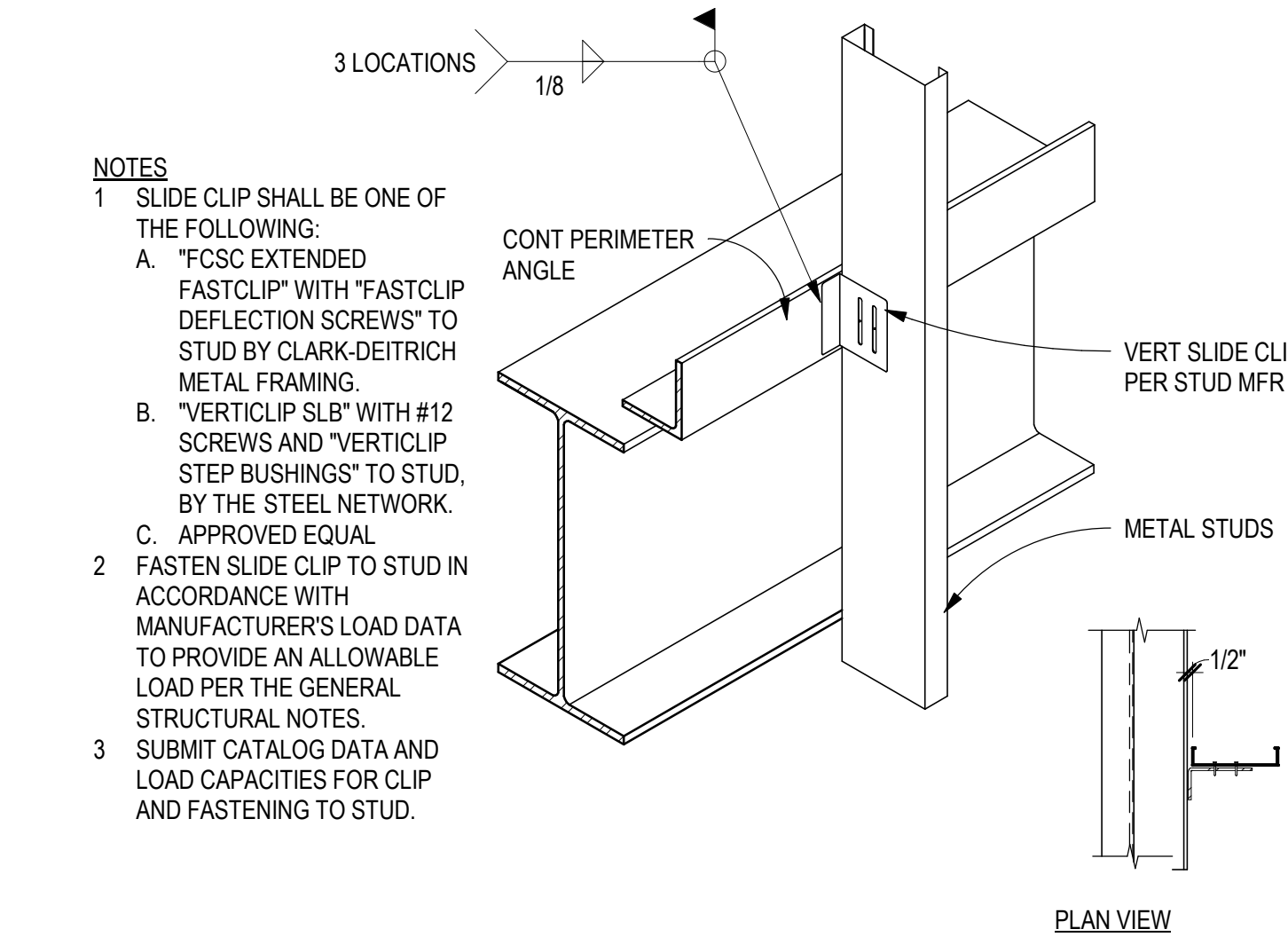
D4 TYPICAL SLIP TRACK ASSEMBLY
SCALE: NTS



D5 TYPICAL WELD CLIP
SCALE: NTS

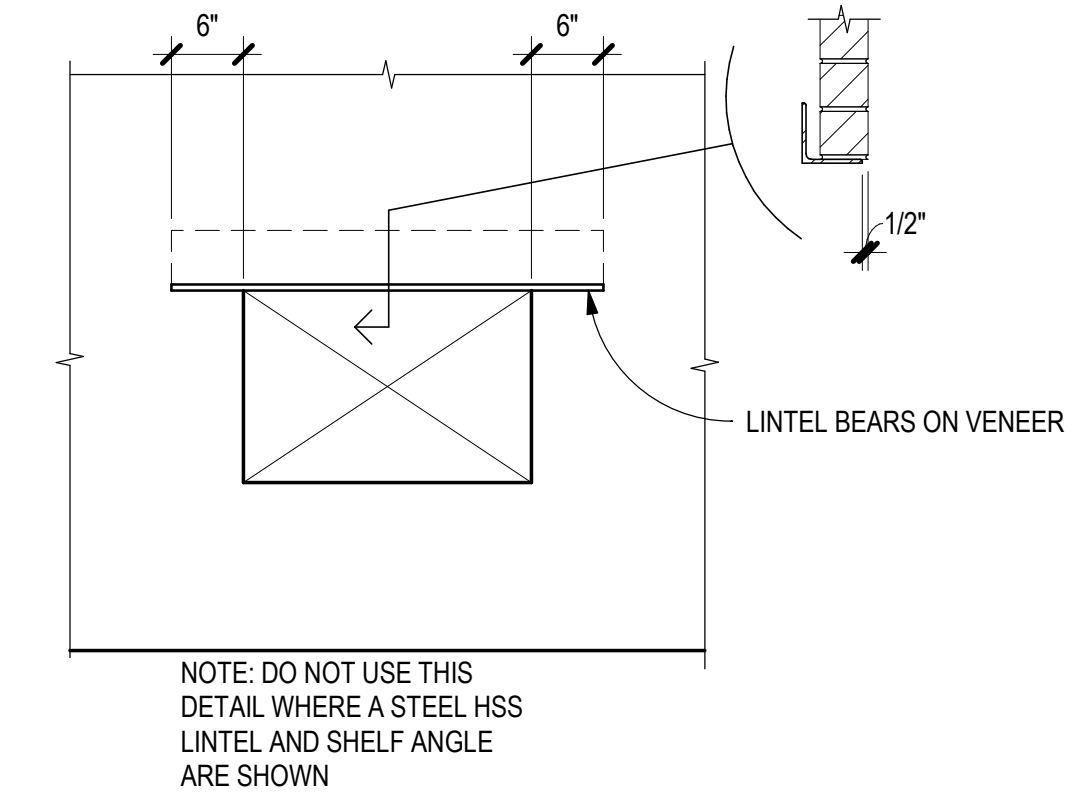


C4 TYPICAL STUD AT METAL DECK
SCALE: 3/4" = 1'-0"

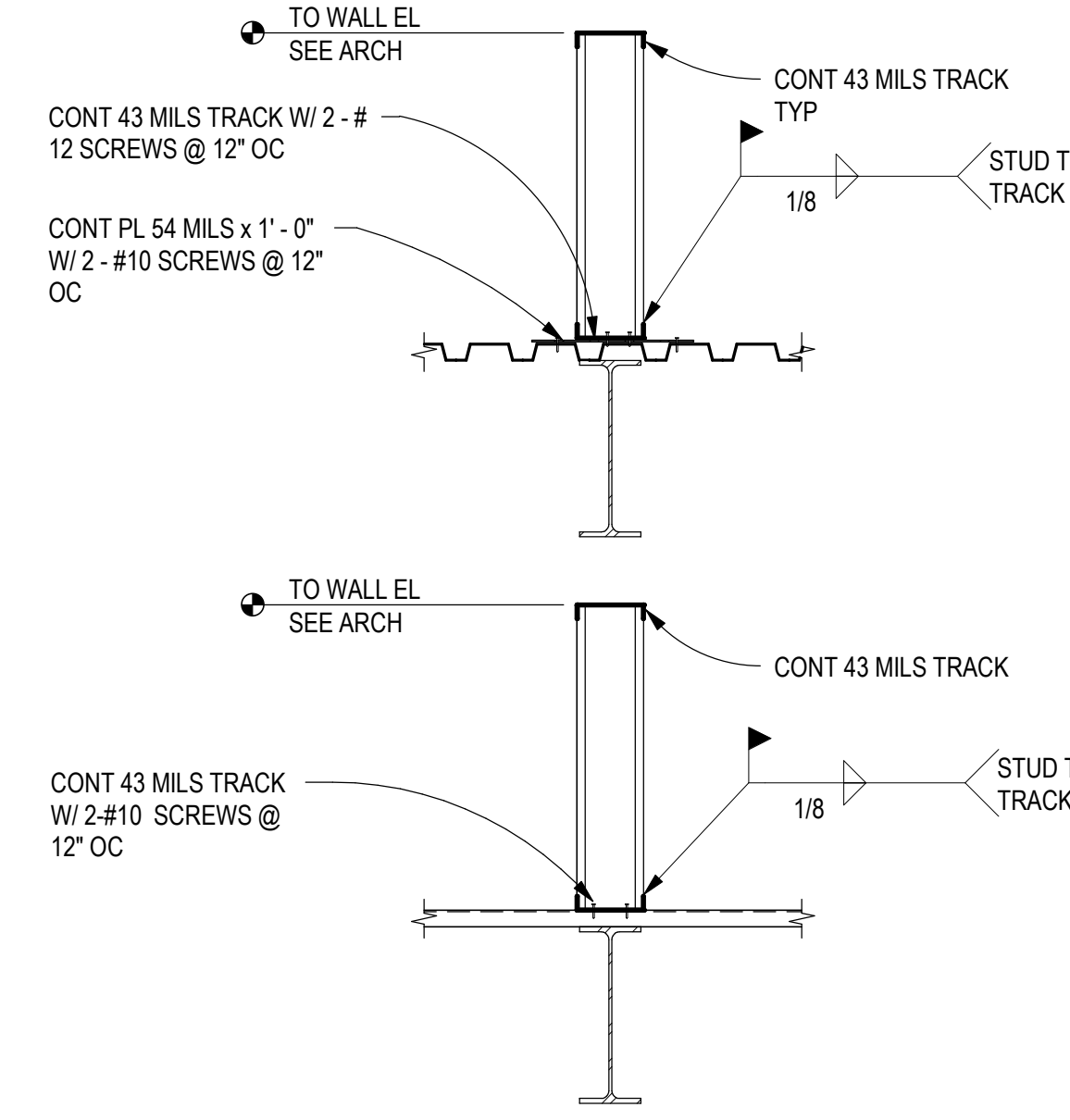


C5 TYPICAL VERTICAL SLIDE CLIP
SCALE: NTS

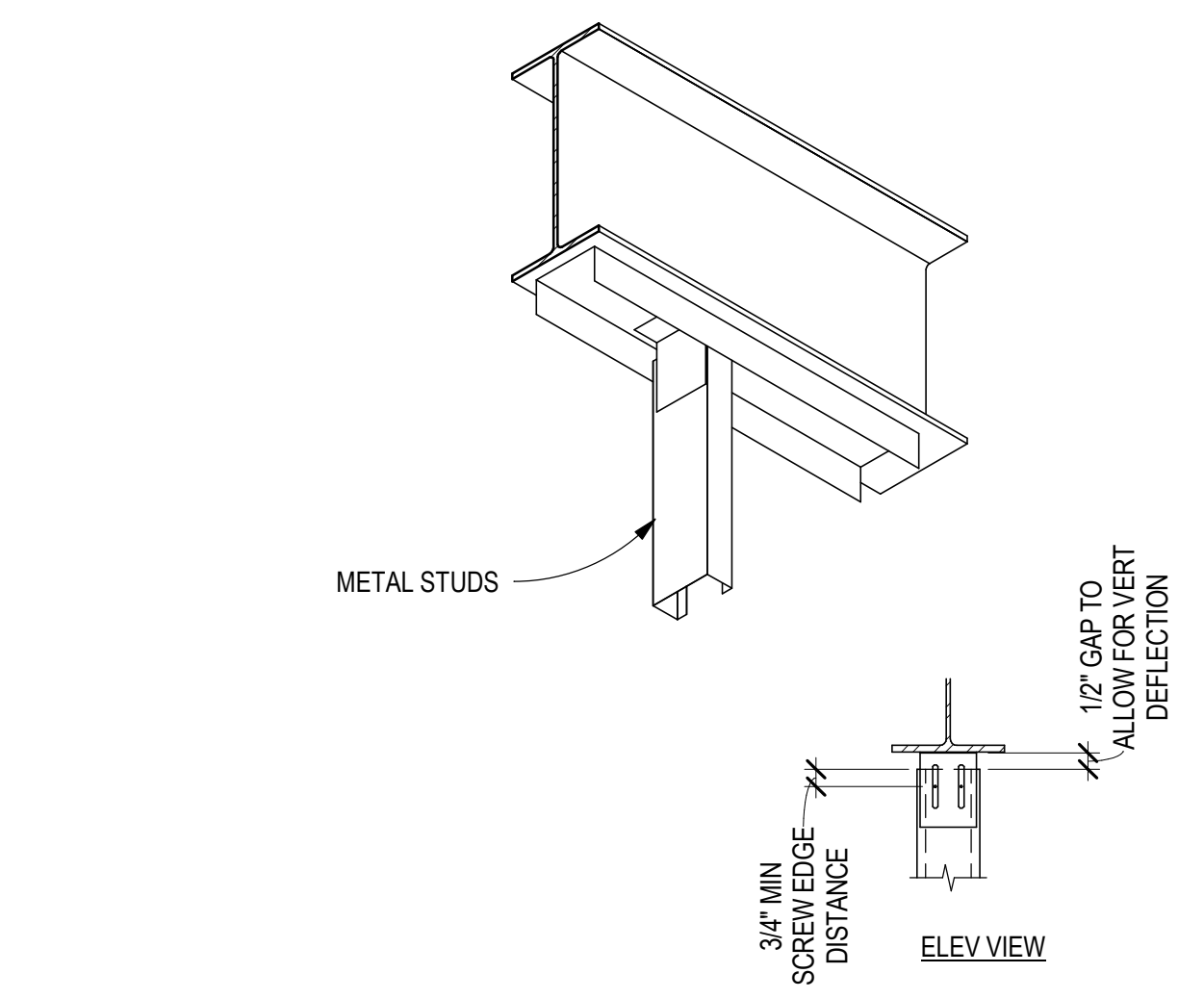
VENEER LINTEL	
OPENING WIDTH	ANGLE
0' - 0" TO 2' - 0"	L5x3x1/4 LLH
2' - 1" TO 3' - 4"	L5x3x3/16 LLH
3' - 5" TO 4' - 0"	L5x3x3/8 LLH
4' - 1" TO 6' - 4"	L6x6x1/2
6' - 5" TO 8' - 0"	L6x6x1/2



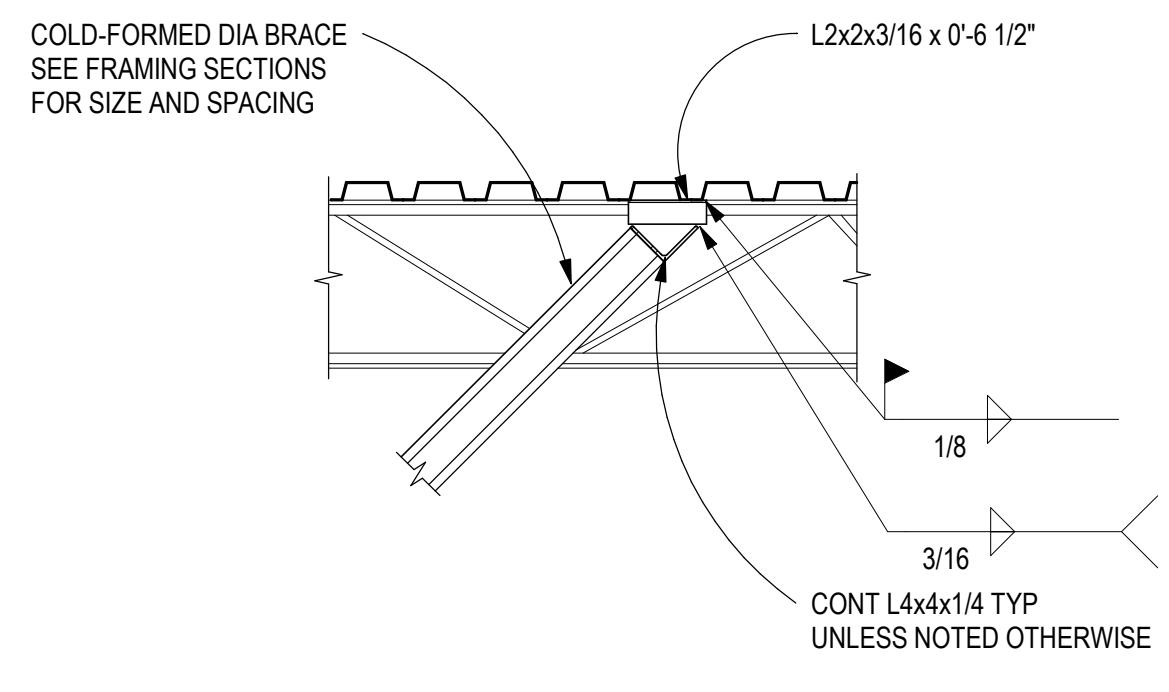
B3 COLD-FORMED LINTEL SECTION @ VENEER
SCALE: 3/4" = 1'-0"



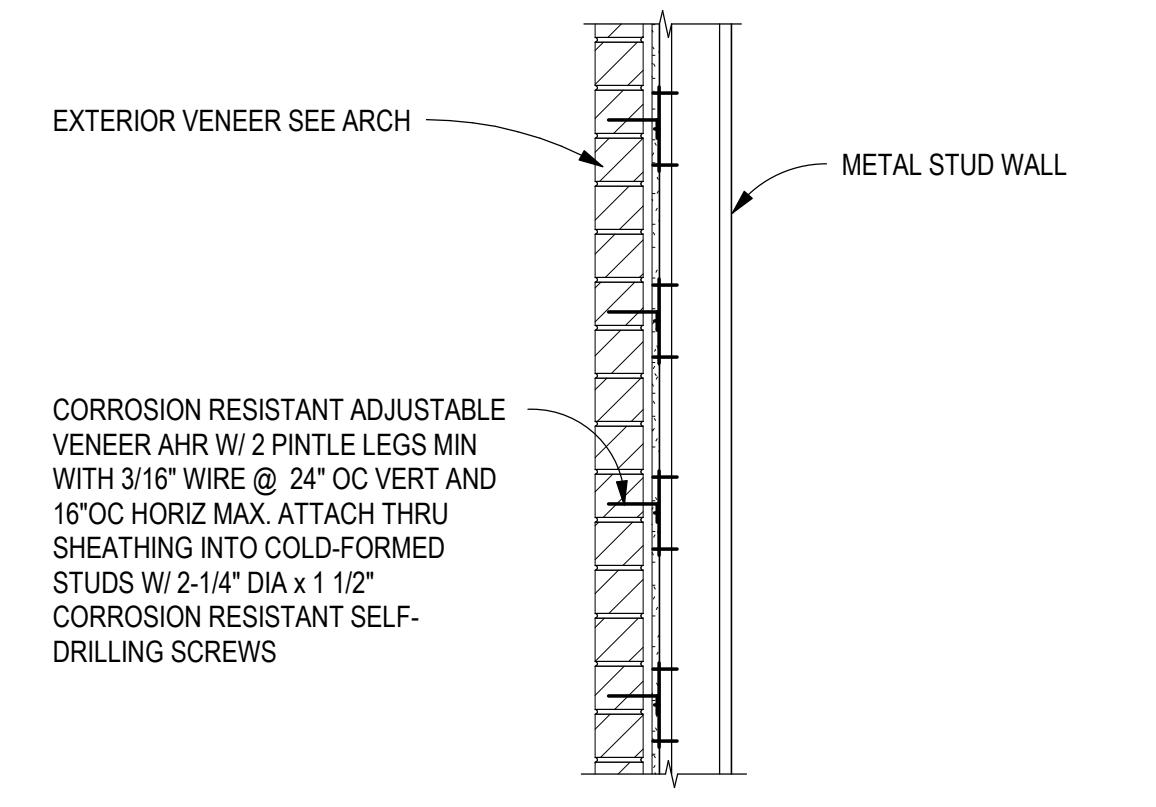
B4 TYPICAL PARAPET TO DECK
SCALE: 3/4" = 1'-0"



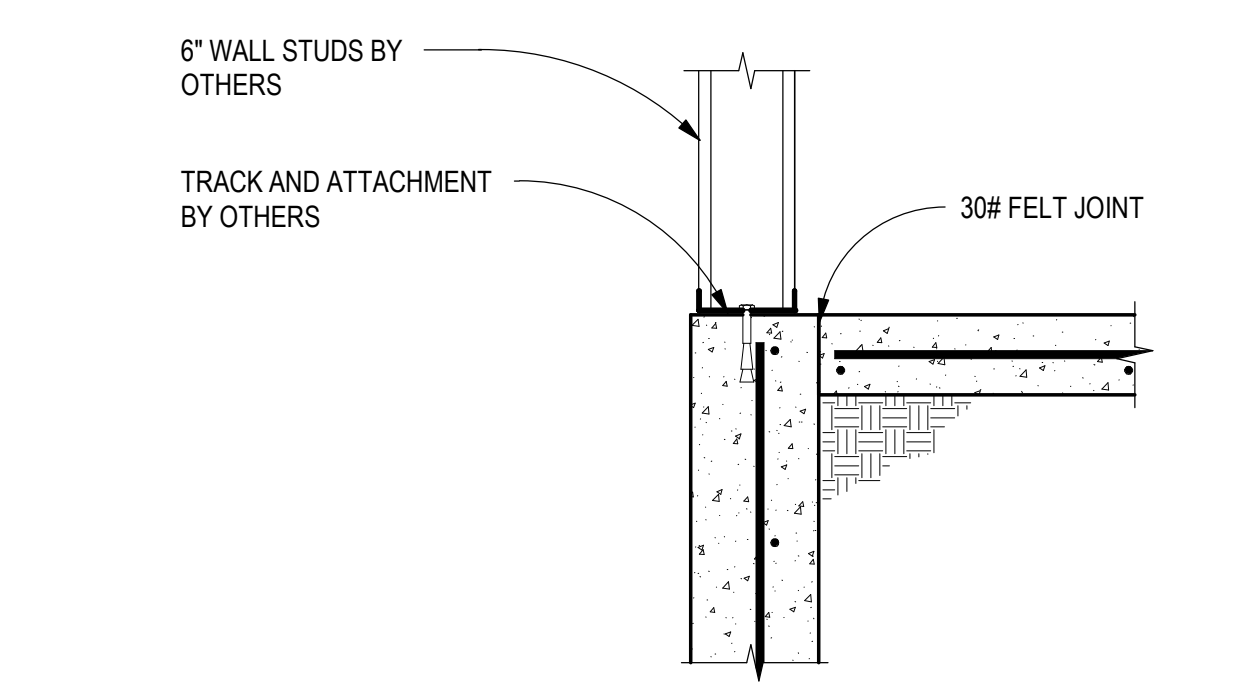
B5 TYPICAL SLIP TRACK ASSEMBLY
SCALE: NTS



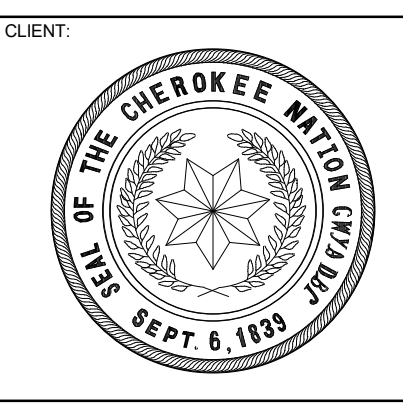
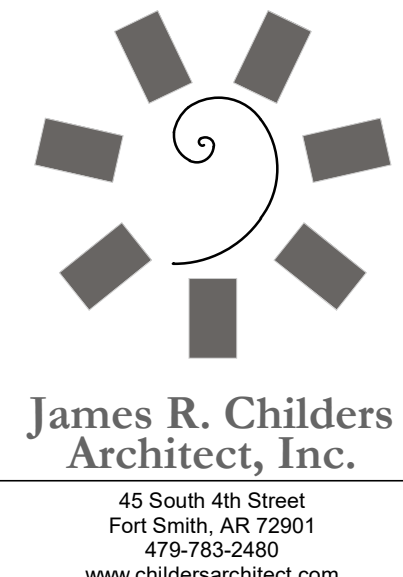
A3 TYPICAL DIAG BRACE TO DECK
SCALE: 3/4" = 1'-0"



A4 TYPICAL VENEER ON MTL STUDS
SCALE: 3/4" = 1'-0"



A5 TYPICAL STUDS AT STEMWALL
SCALE: 1" = 1'-0"



**WILMA P. MANKILLER HEALTH CENTER
EXPANSION**
 STILWELL, OKLAHOMA

KEY PLAN:

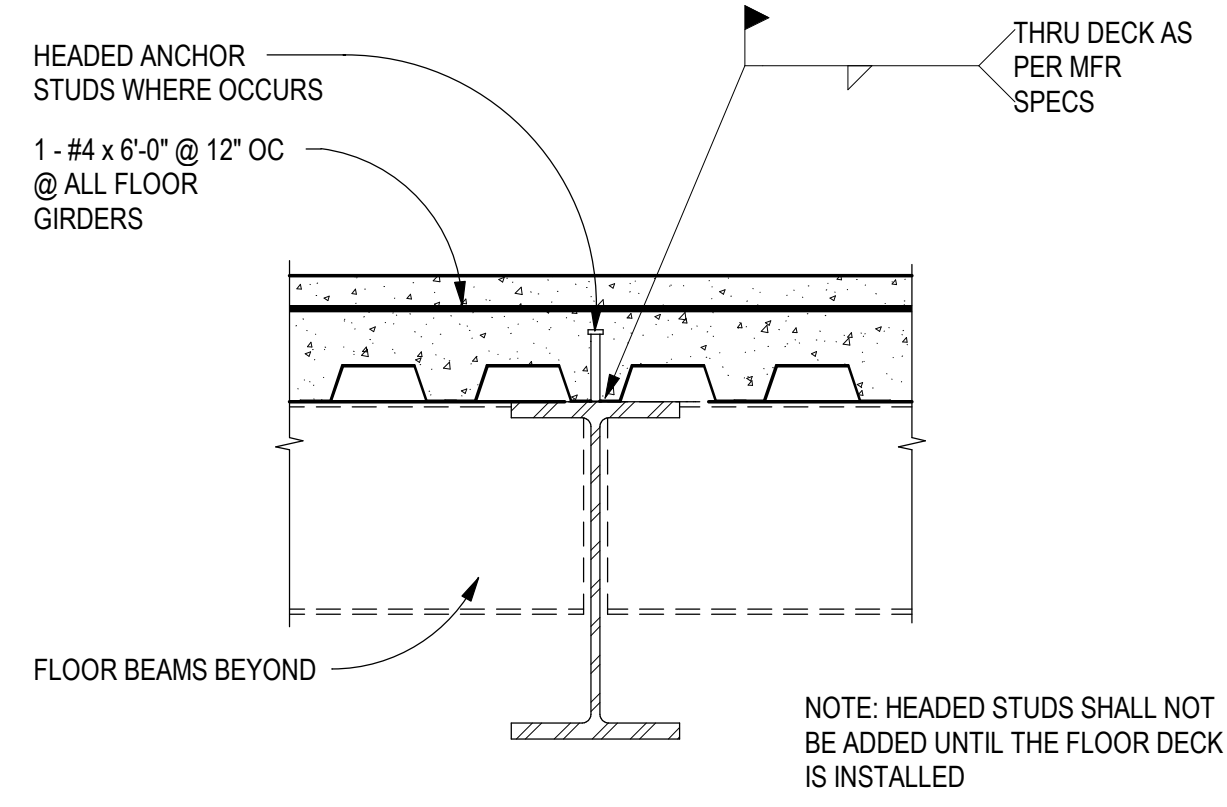
PROJECT PHASE:
BID PACKAGE 01

#	DATE	REVISIONS DESCRIPTION

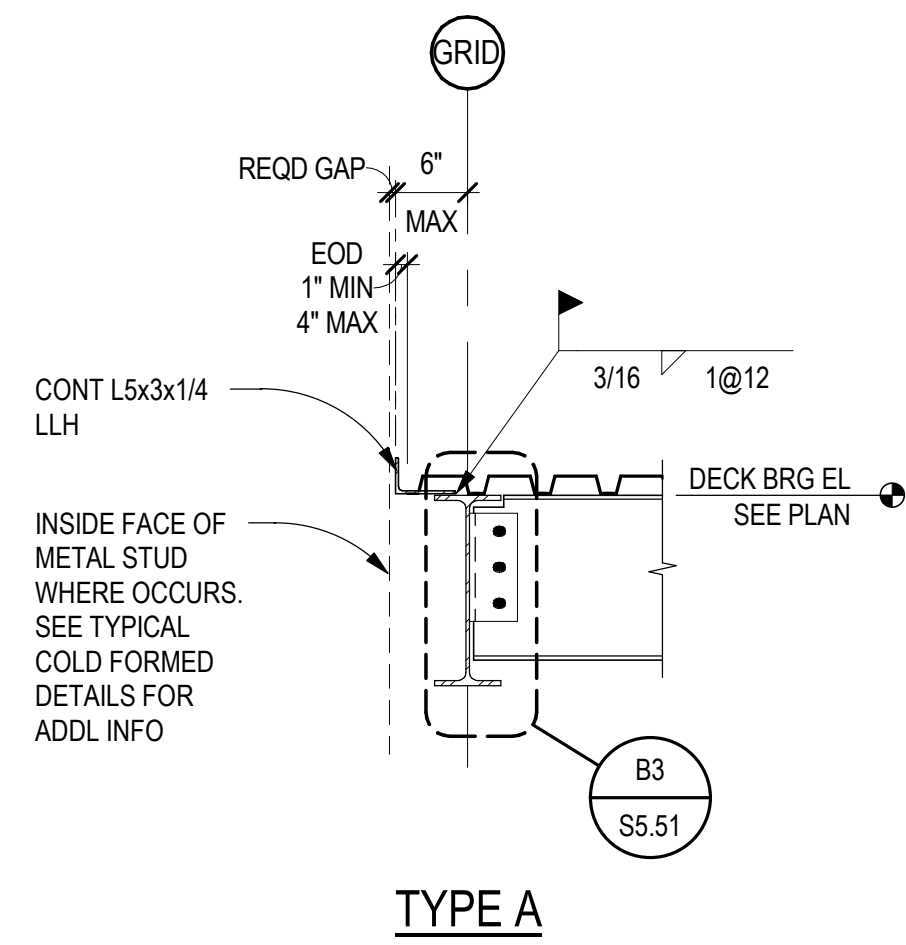
DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER:
S7.31

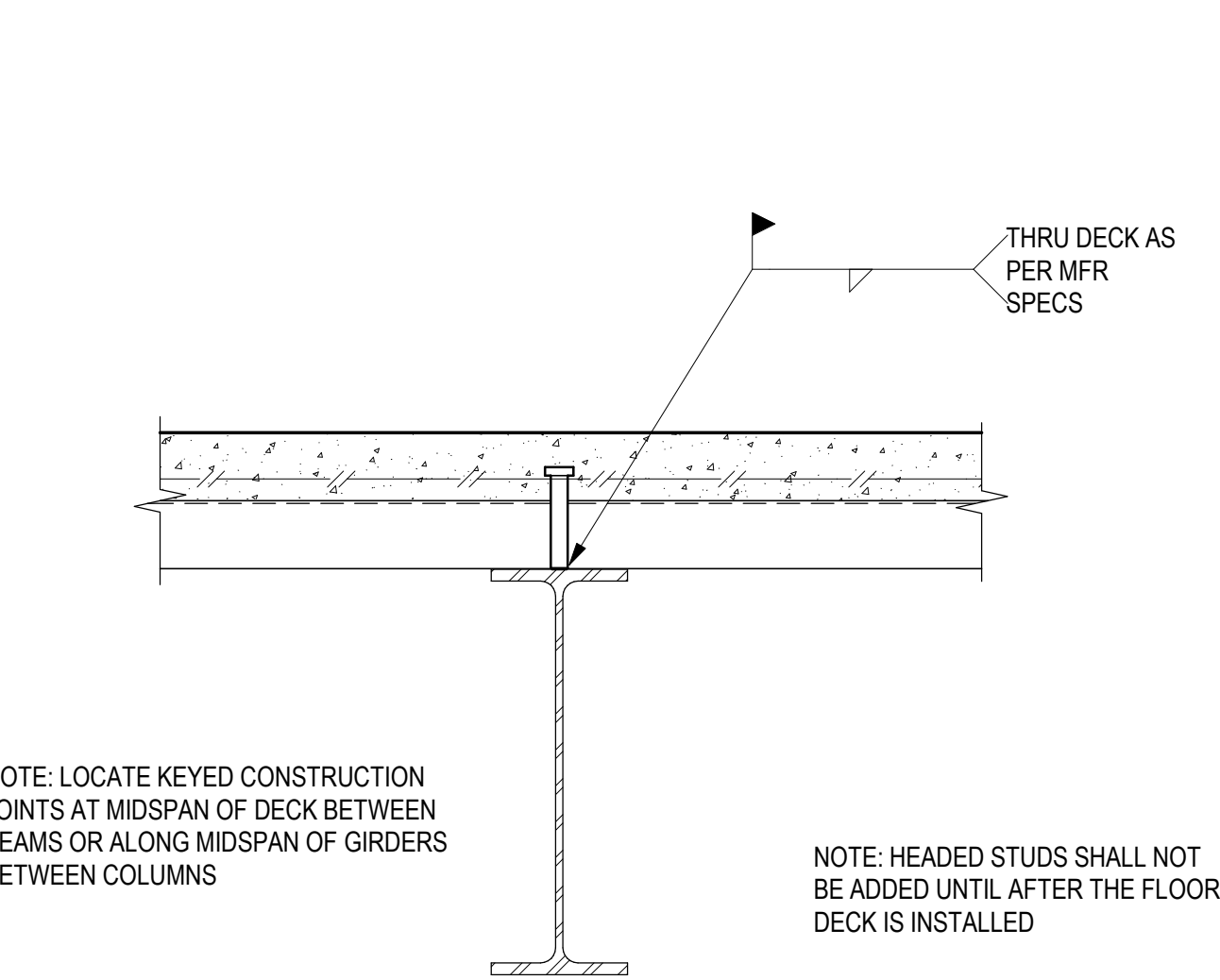
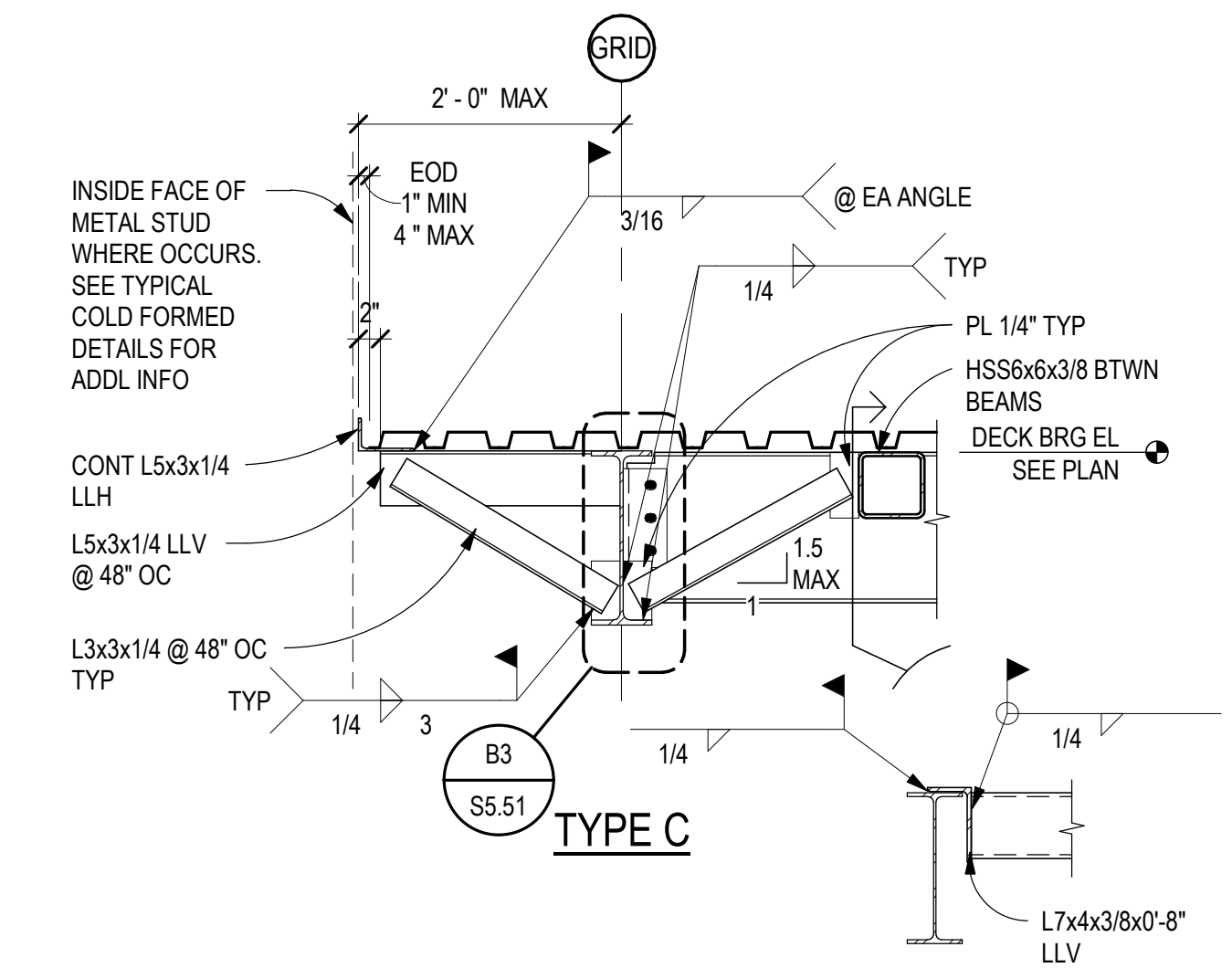
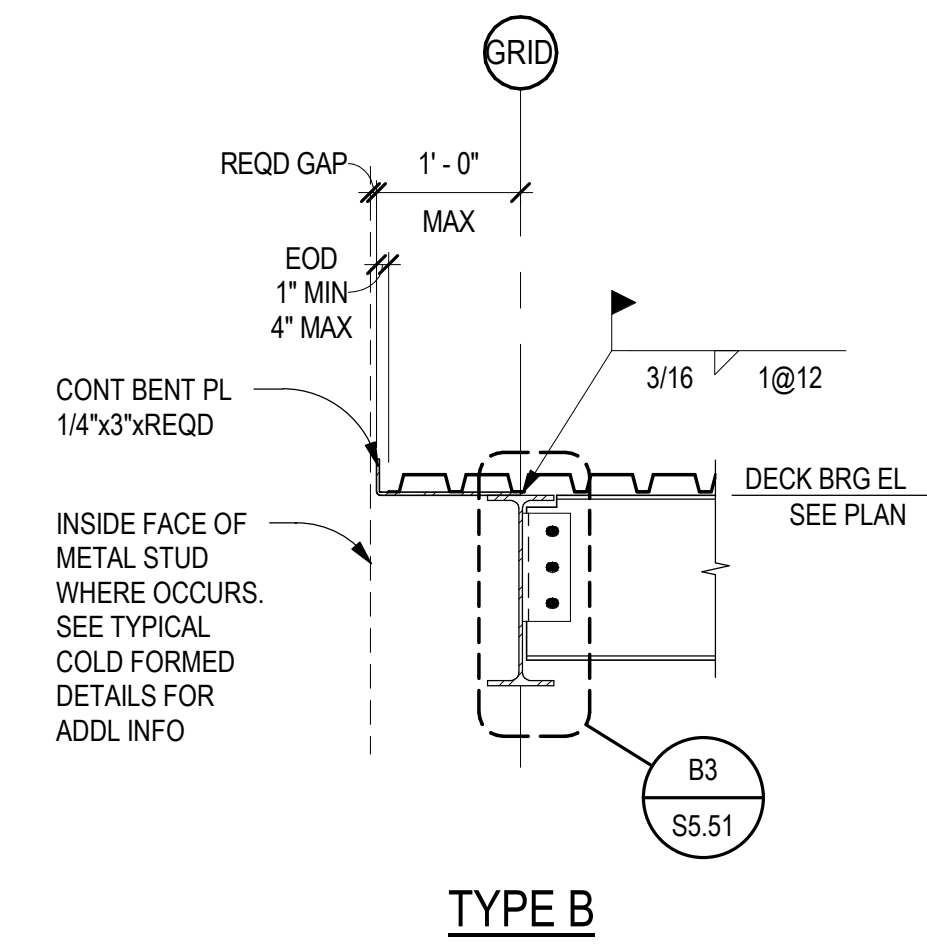
TYPICAL COLD-FORMED DETAILS



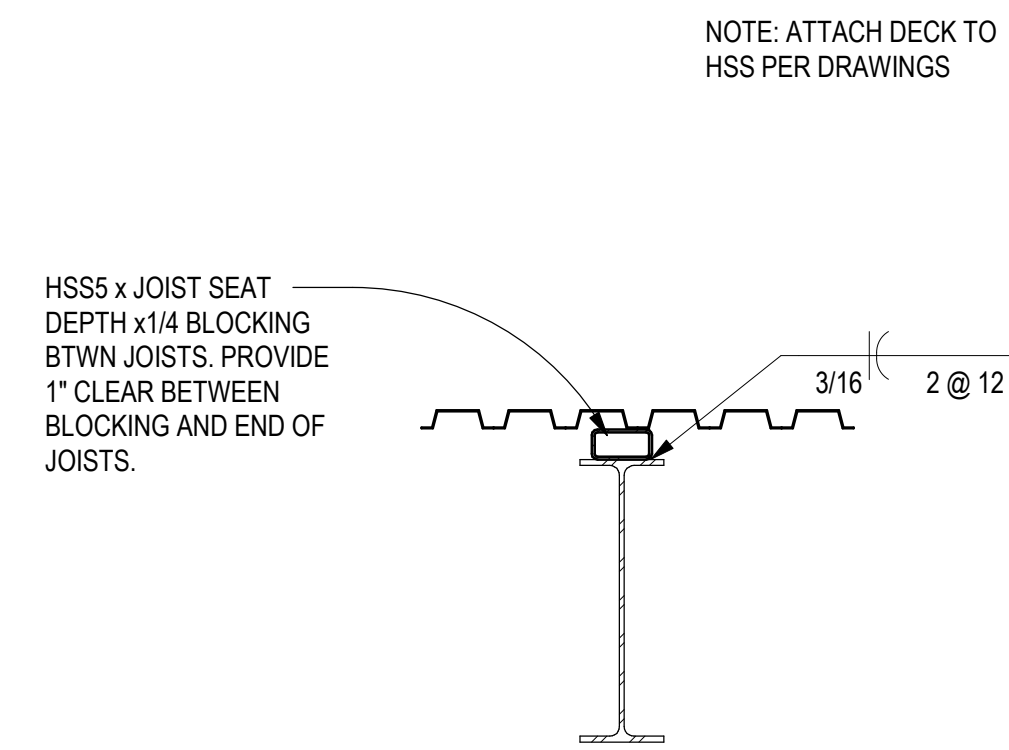
D1 TYPICAL FLOOR GIRDER
SCALE: 3/4" = 1'-0"



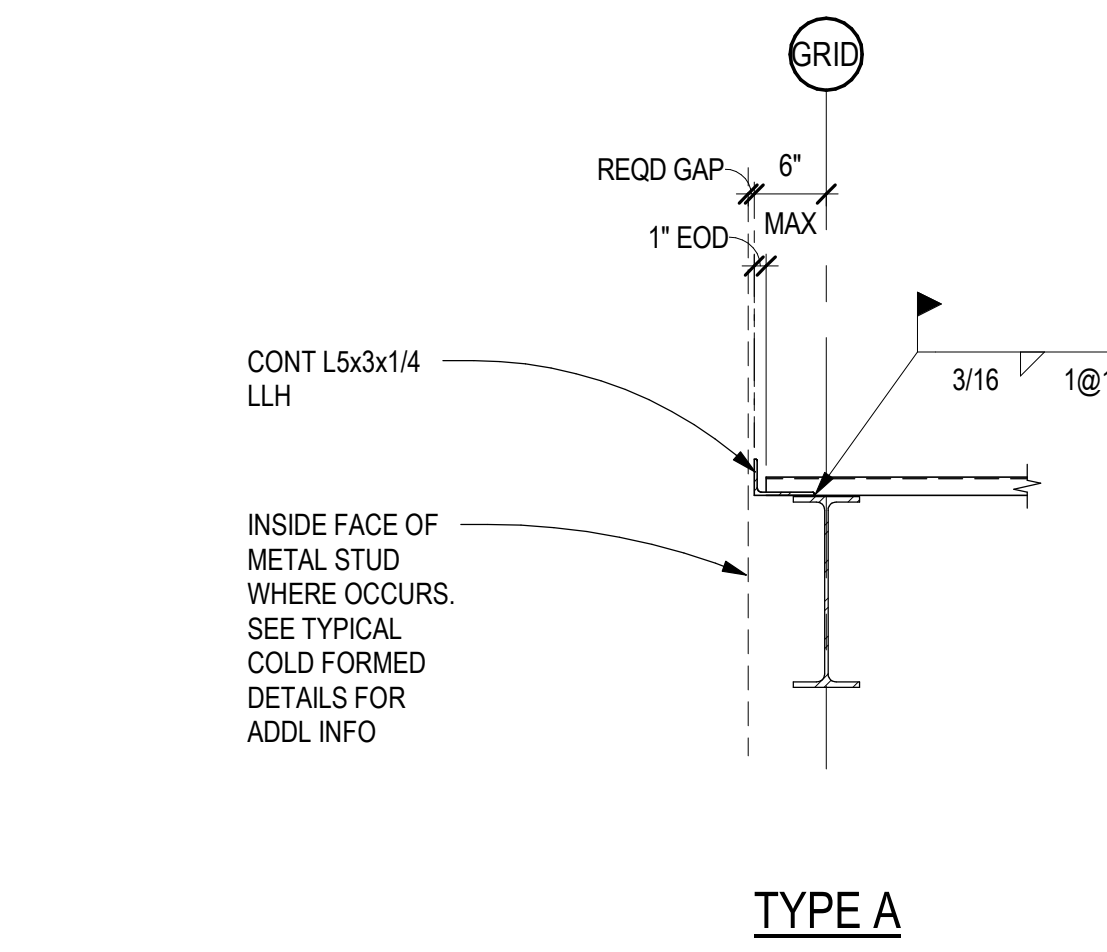
D3 TYPICAL DECK EDGE AT NON-BEARING BEAM
SCALE: 3/4" = 1'-0"



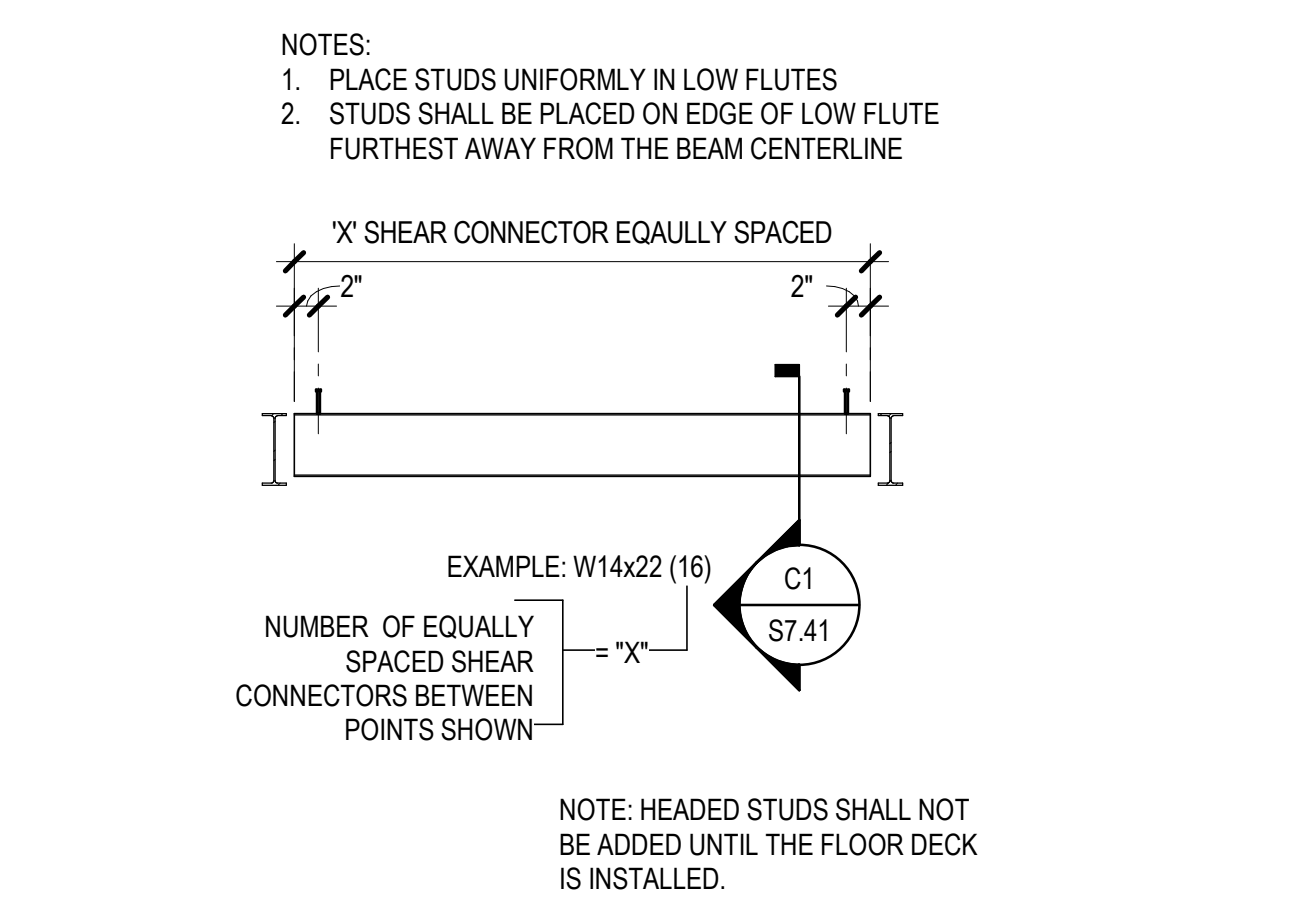
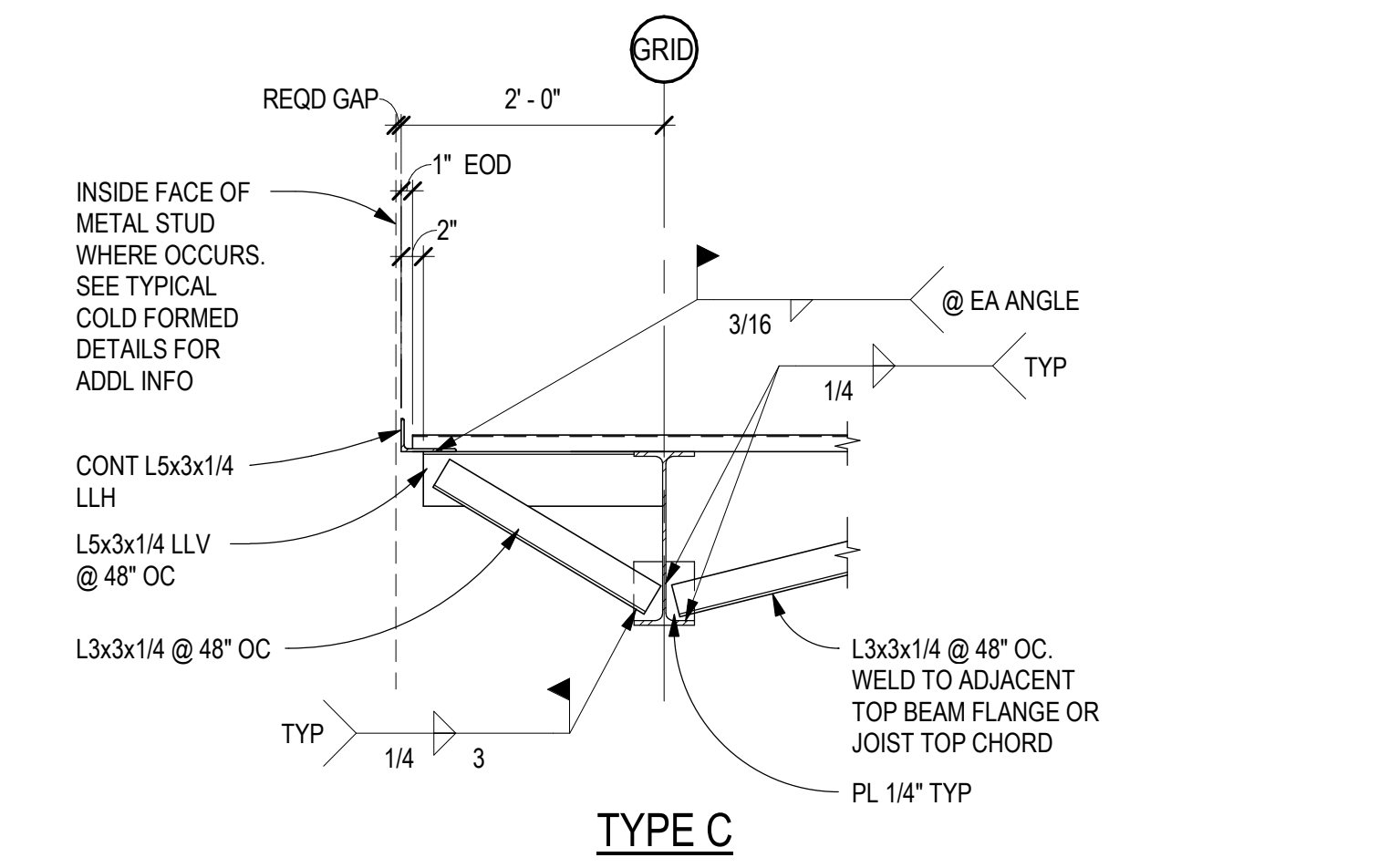
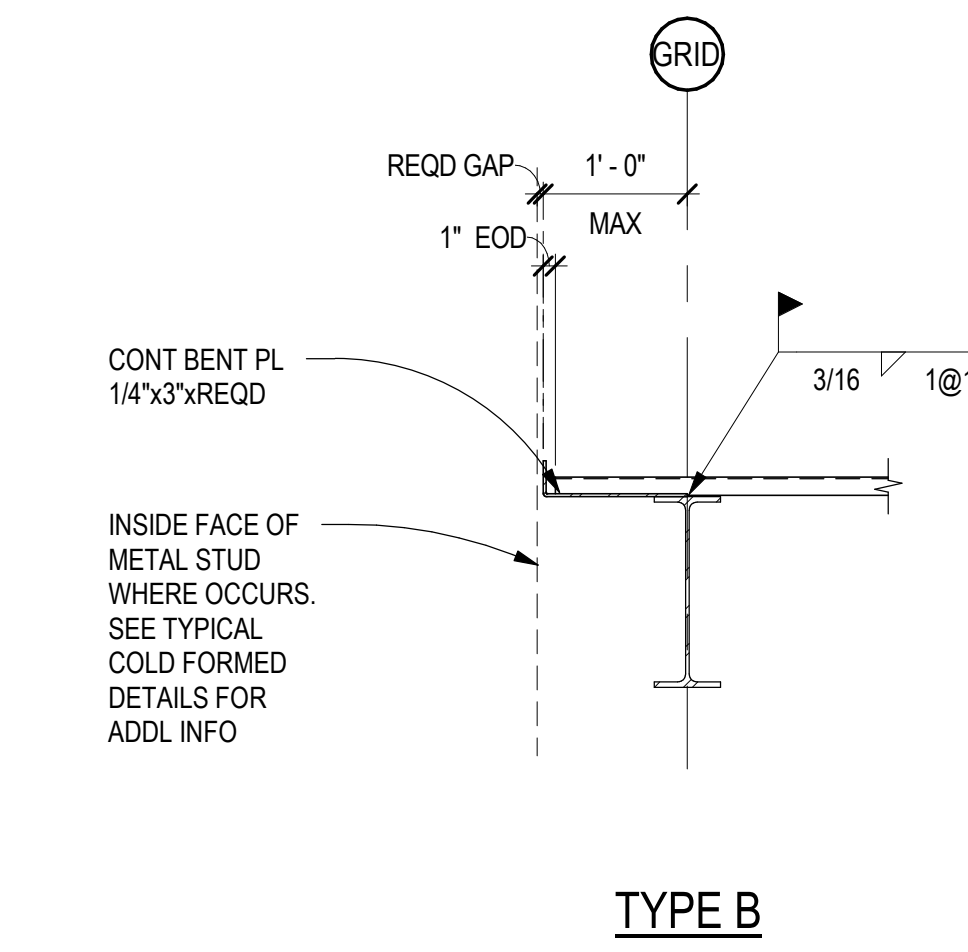
C1 TYPICAL COMPOSITE BEAM
SCALE: 1 1/2" = 1'-0"



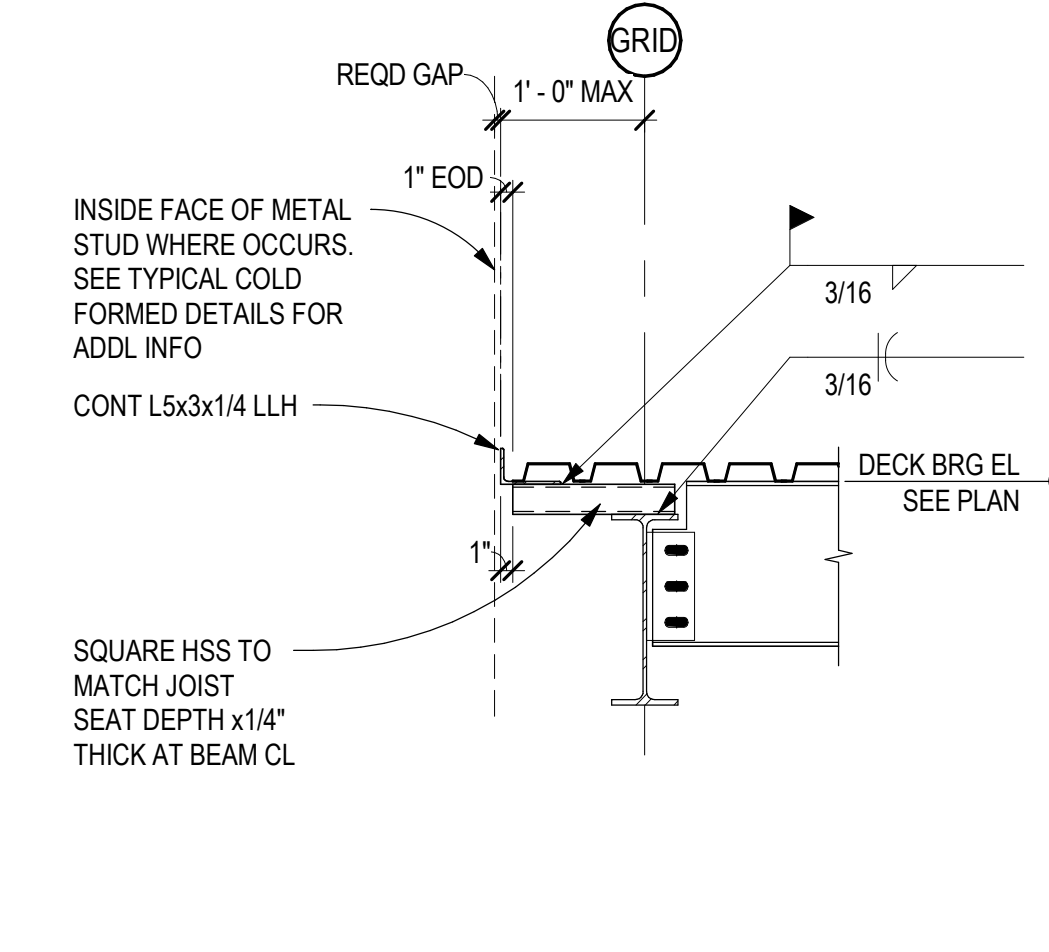
C2 TYPICAL HSS BLOCKING DETAIL
SCALE: 3/4" = 1'-0"



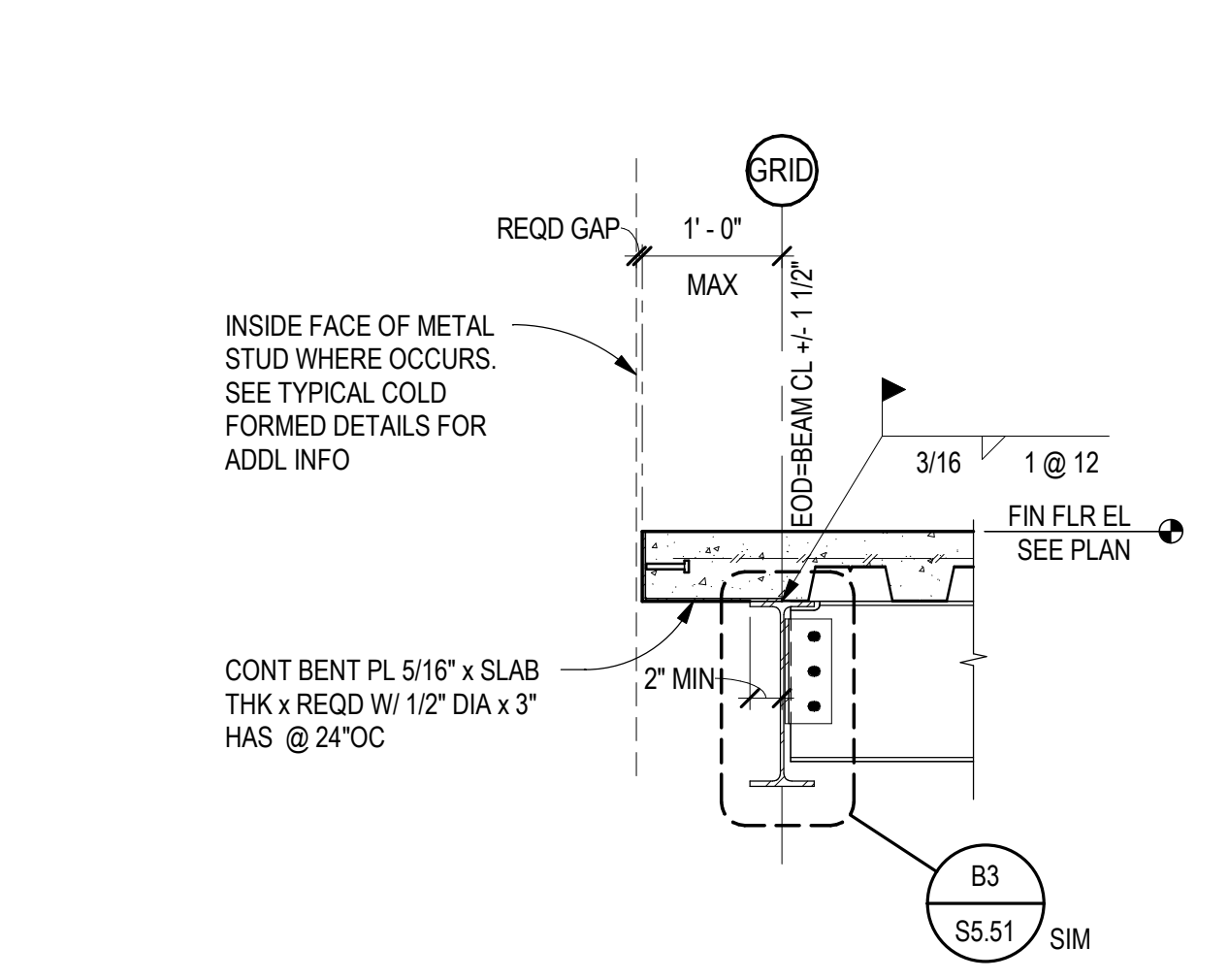
C3 TYPICAL DECK EDGE AT NON-BEARING
SCALE: 3/4" = 1'-0"



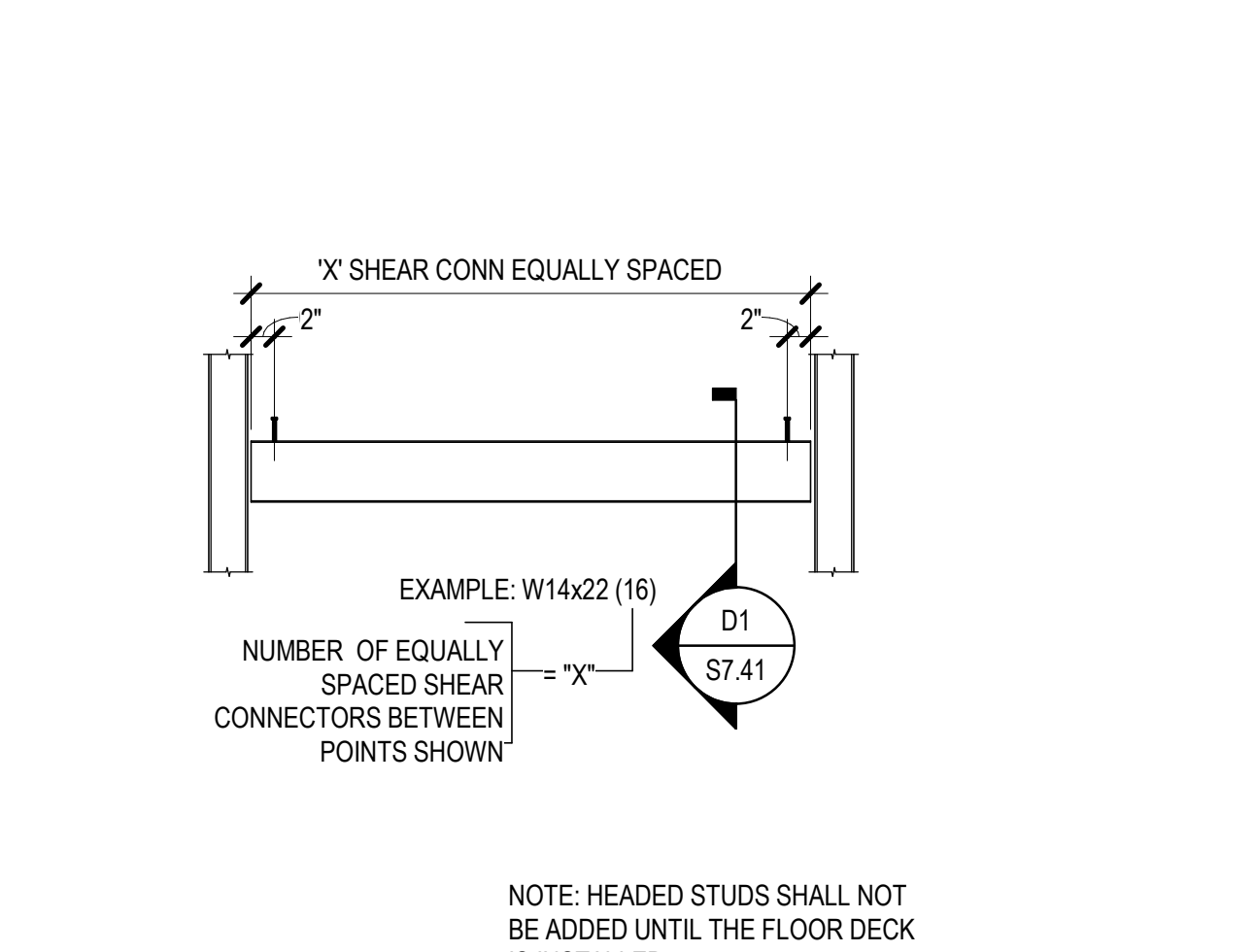
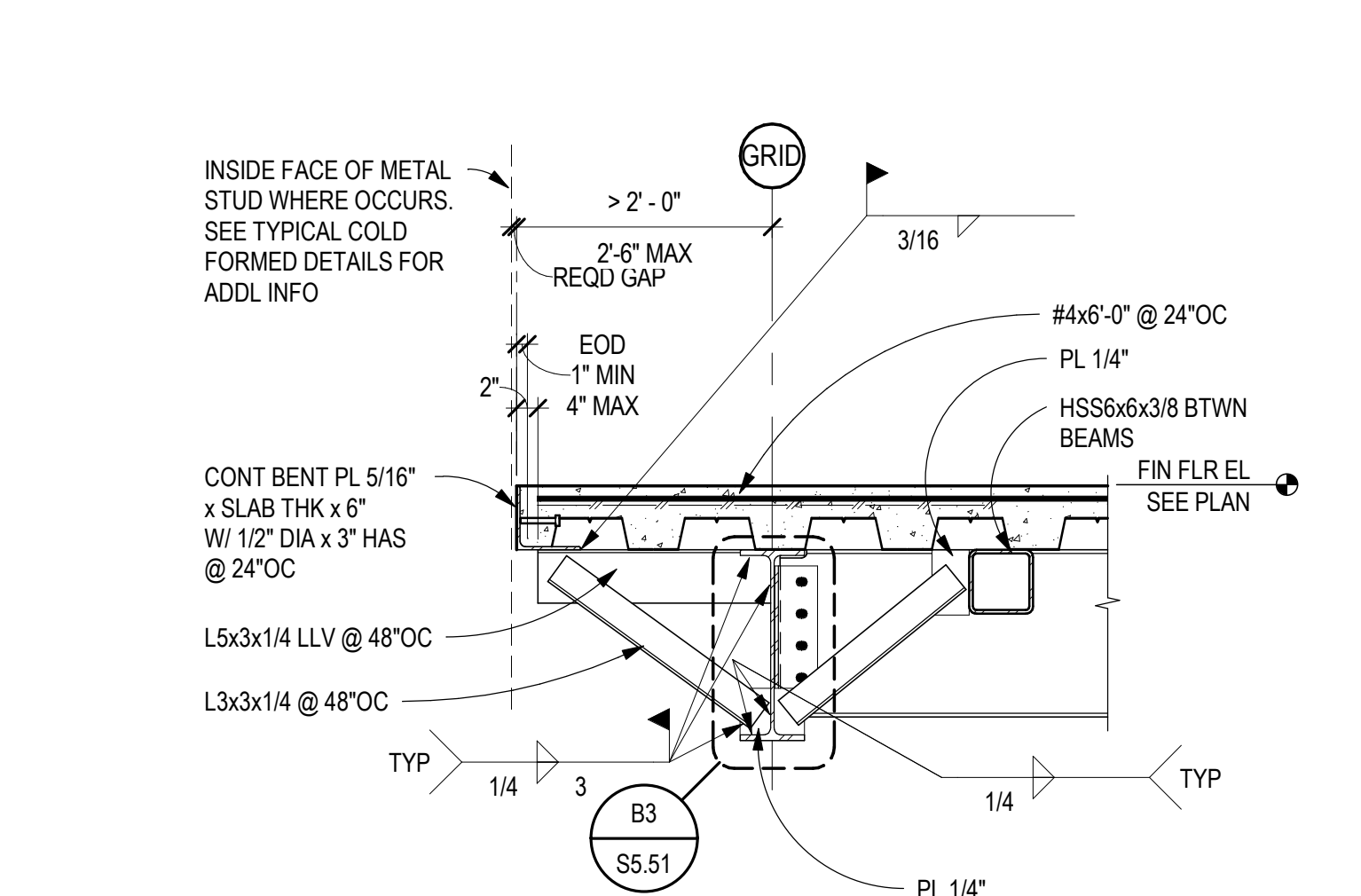
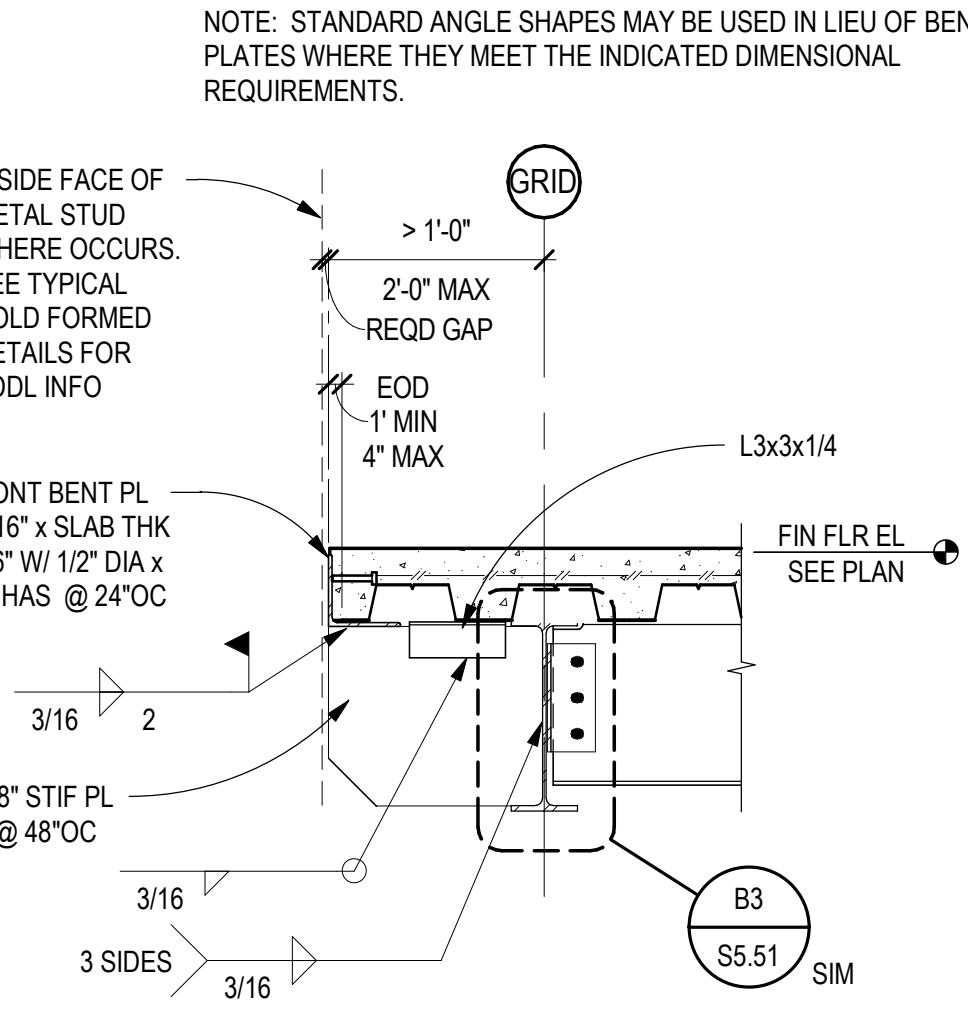
B1 TYPICAL COMPOSITE BEAM ELEV
SCALE: 1/4" = 1'-0"



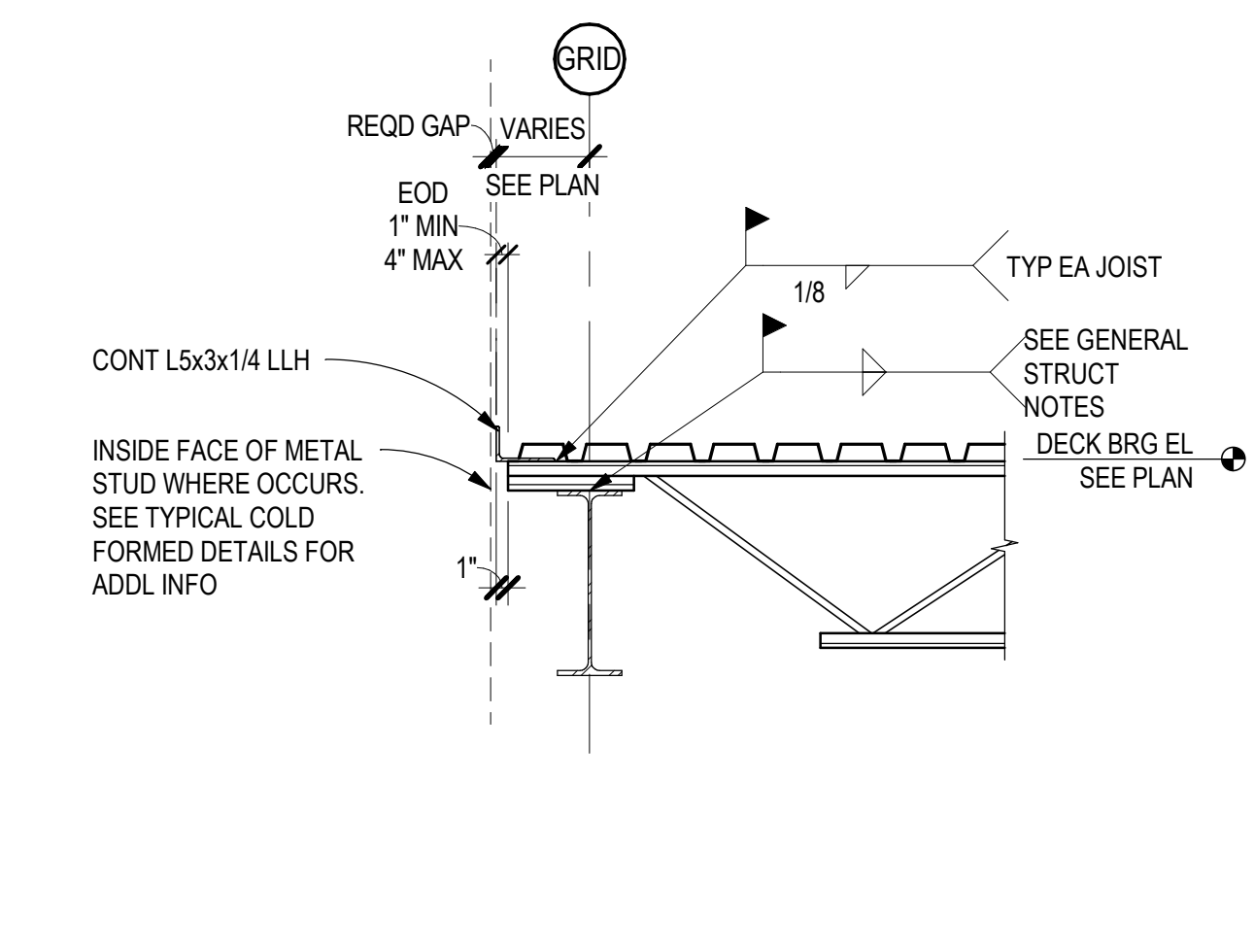
B2 TYPICAL DECK EDGE AT BEAM
SCALE: 3/4" = 1'-0"



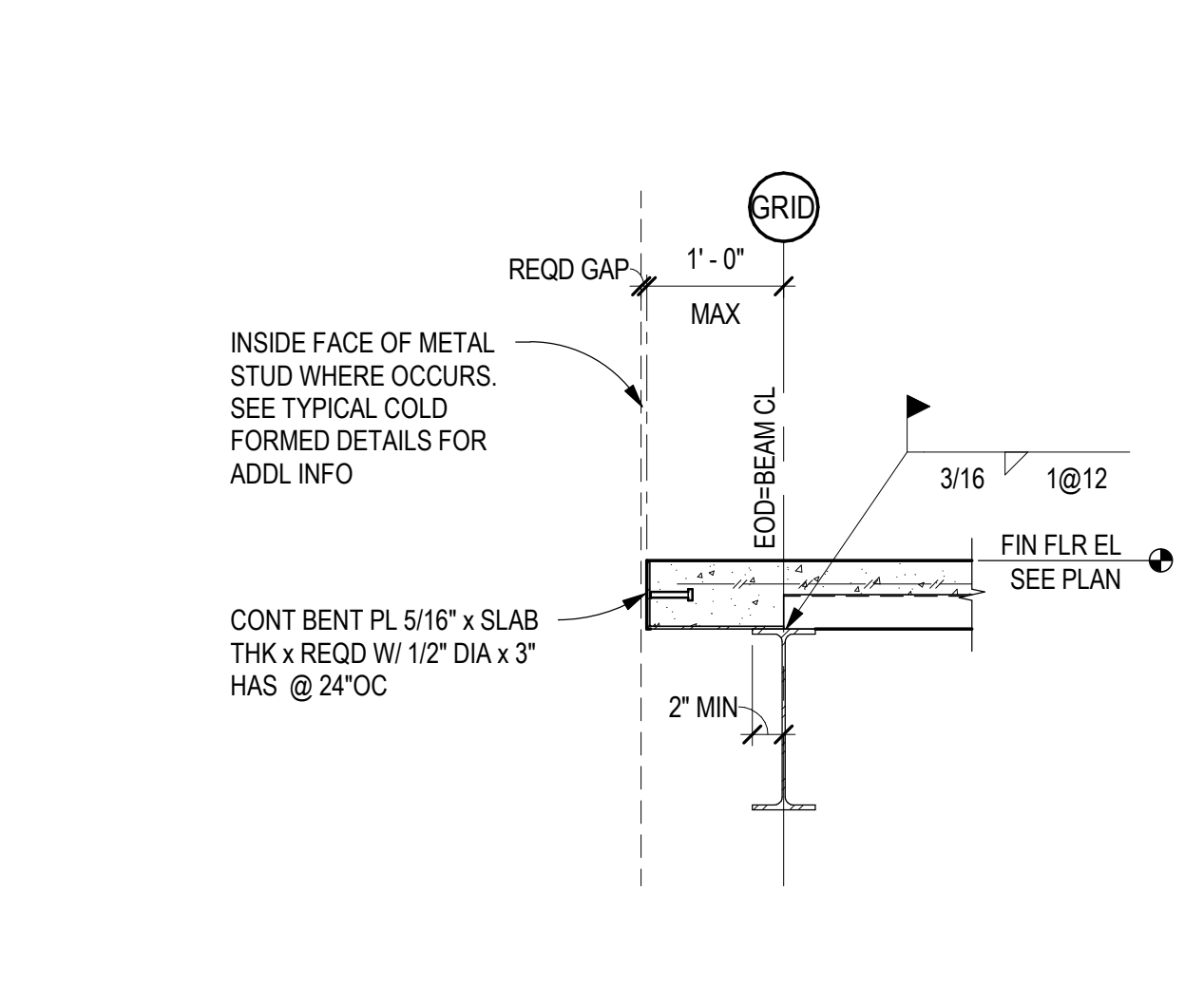
B3 TYPICAL SLAB EDGE AT BEARING CONDITION
SCALE: 3/4" = 1'-0"



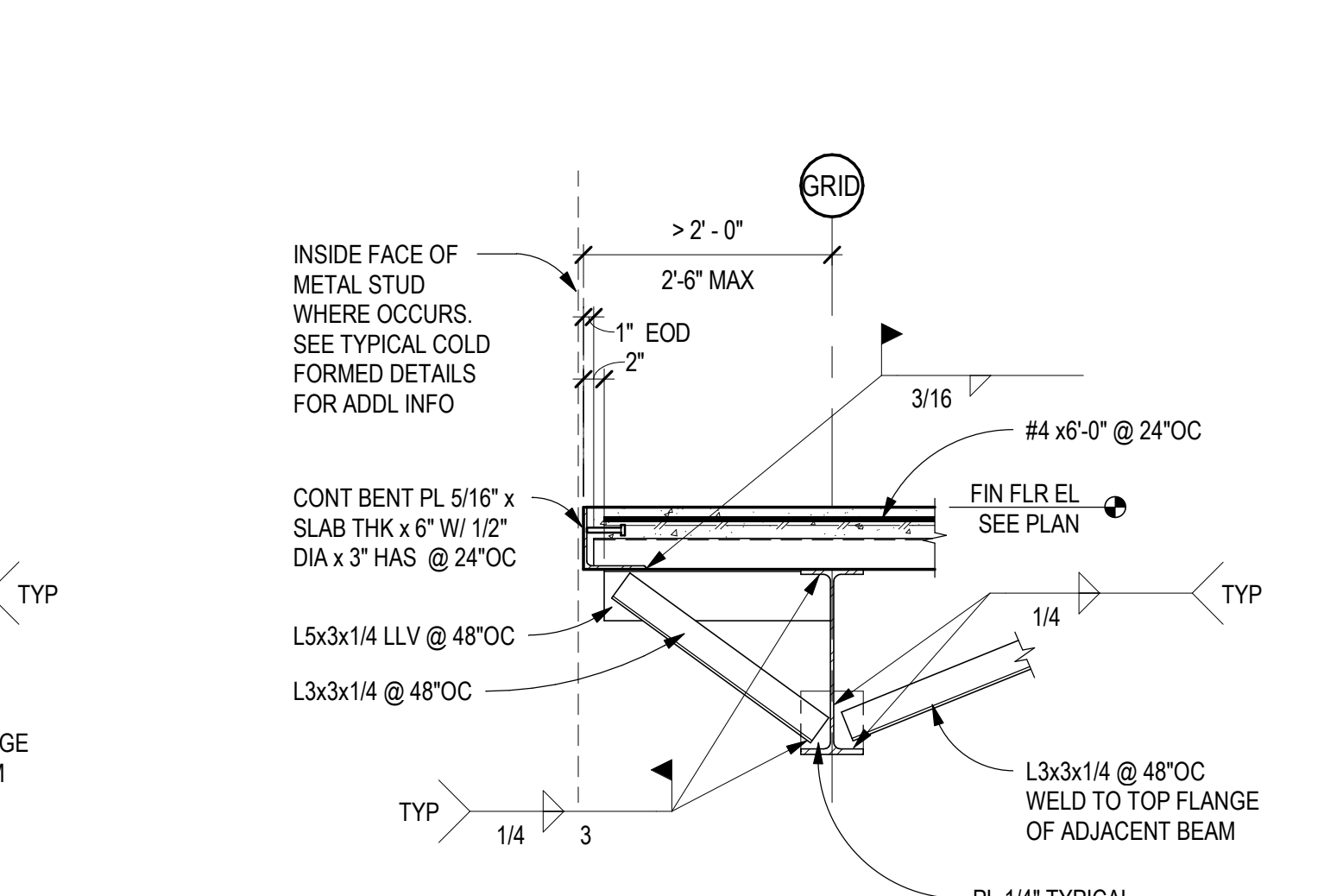
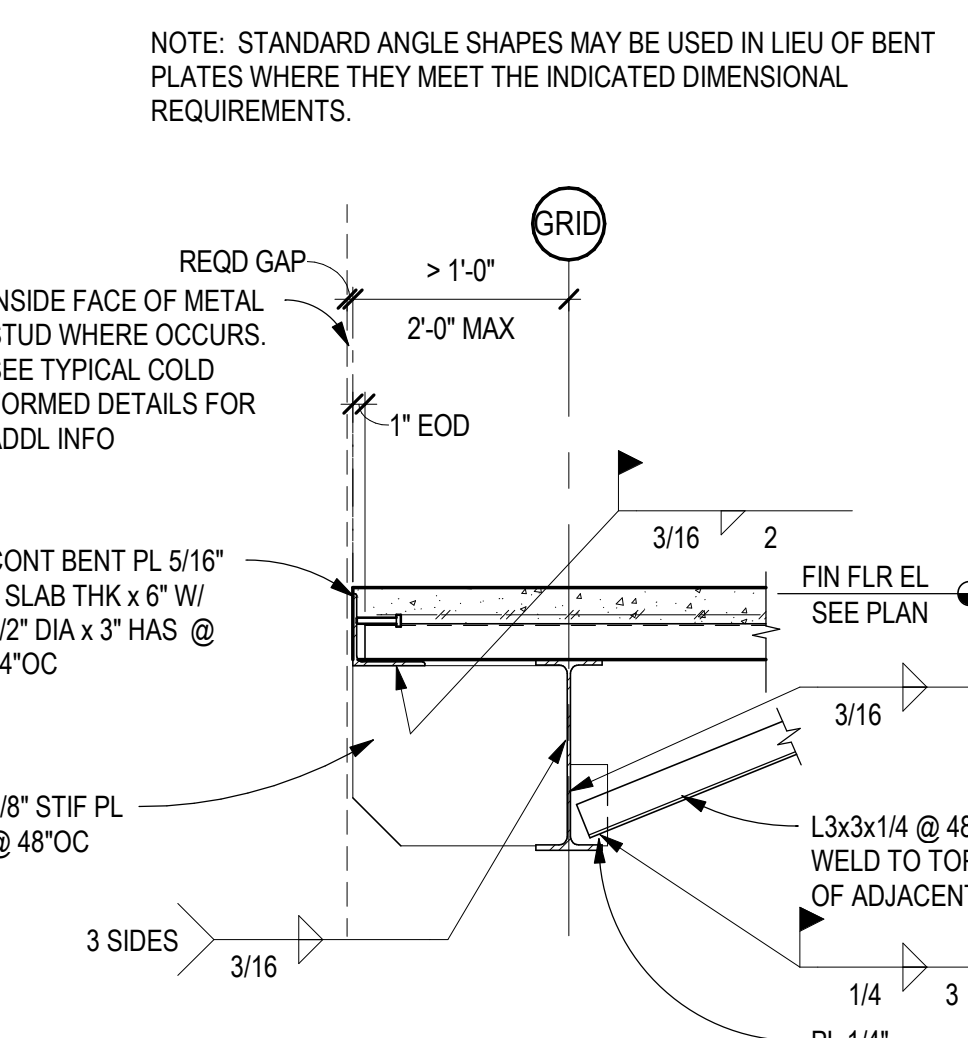
A1 TYPICAL COMPOSITE GIRDER ELEV
SCALE: 1/4" = 1'-0"

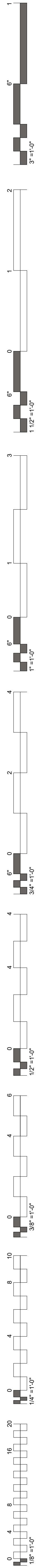


A2 TYPICAL ROOF BRG DECK EDGE
SCALE: 3/4" = 1'-0"

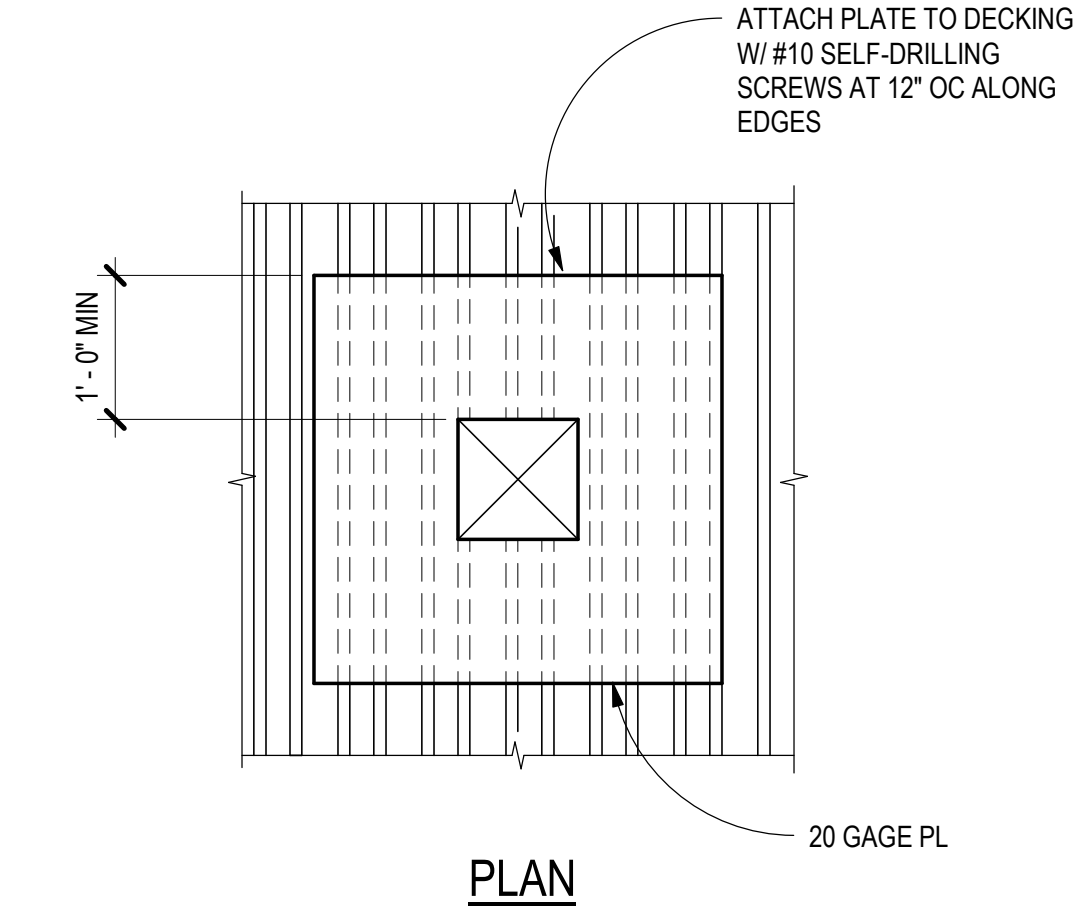


A3 TYPICAL SLAB EDGE AT NON-BEARING CONDITION
SCALE: 3/4" = 1'-0"

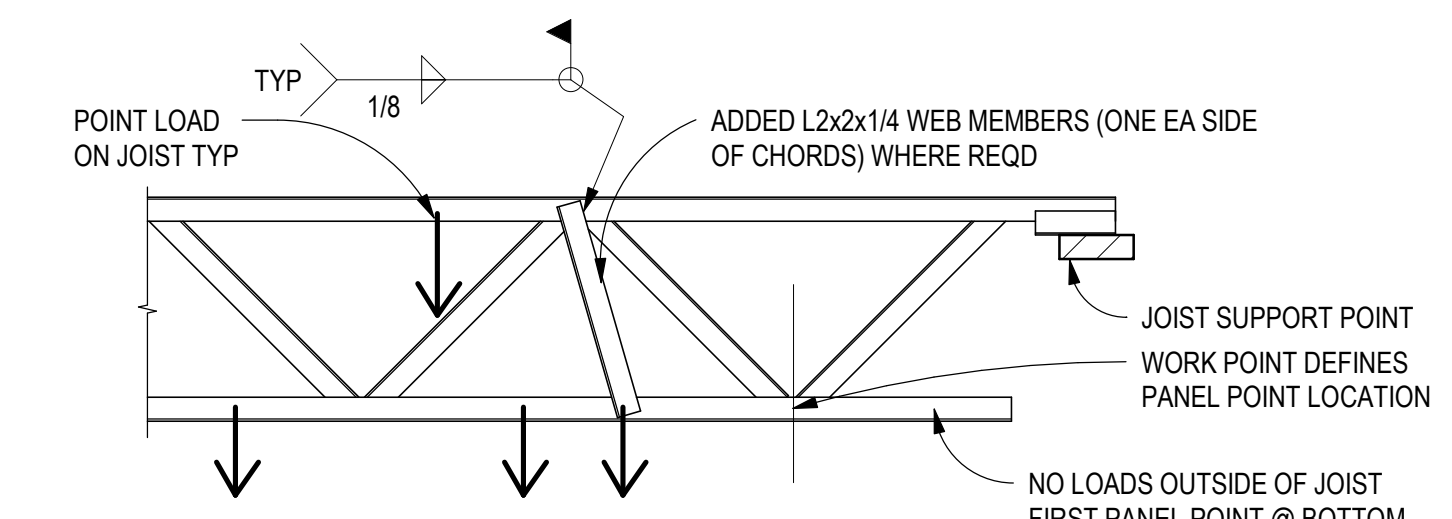




NOTE: INSTALL OPENINGS IN COMPLIANCE WITH OSHA REQUIREMENTS INCLUDING 1926.754 OF 29 CFR



D5 TYPICAL ROOF OPENING 0''-12''
SCALE: 3/4" = 1'-0"

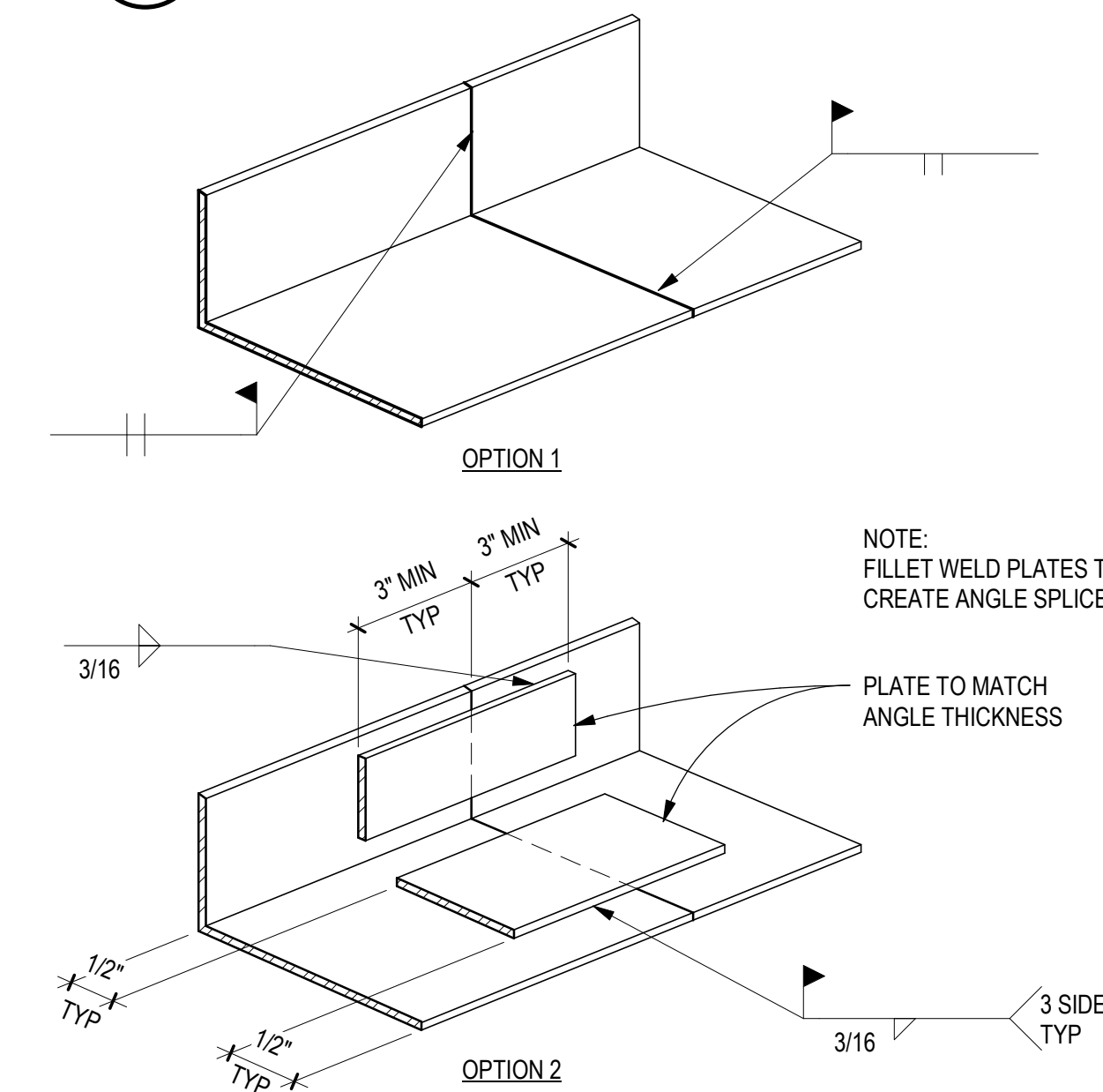


POINT LOADS ON K-SERIES OR LH-SERIES (NON SP JOISTS) SHALL MEET ALL OF THE FOLLOWING REQUIREMENTS:

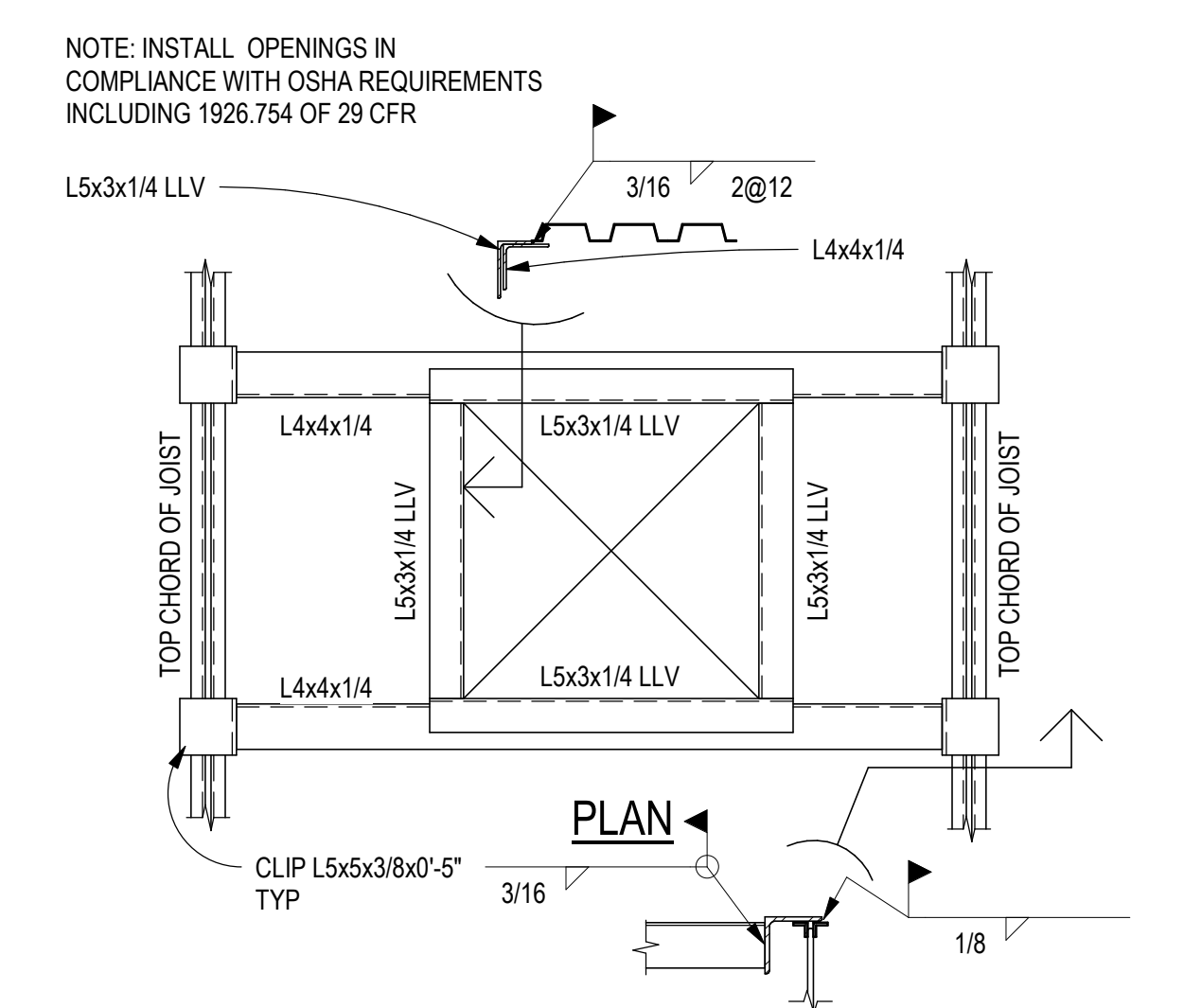
- POINT LOADS ON THE JOIST TOP CHORD OR BOTTOM CHORD THAT EXCEED 100LBS SHALL BE PLACED WITHIN 4" OF A JOIST PANEL POINT, OR ADDITIONAL WEB MEMBERS SHALL BE ADDED AT THE POINT OF LOAD APPLICATION.
- WHERE MULTIPLE POINT LOADS ARE PLACED BETWEEN THE SAME TWO PANEL POINTS, THE SUM OF THOSE LOADS THAT ARE NOT REINFORCED WITH ADDITIONAL WEB MEMBERS SHALL NOT EXCEED 100LBS.
- POINT LOADS SHALL BE CONCENTRIC WITH THE CHORD FROM WHICH IT IS HUNG. BEAM CLAMPS OR OTHER CONNECTIONS THAT INDUCE NON-CONCENTRIC LOADS ARE NOT PERMITTED.
- POINT LOADS SHALL BE SPACED SO THAT THE COMBINED TOP CHORD PLUS BOTTOM CHORD POINT LOADS DO NOT EXCEED AN EQUIVALENT LINE LOAD OF 65PLF AT ANY POINT ALONG THE JOIST.
- LOADS SHALL NOT BE PLACED ON THE BOTTOM CHORD OUTSIDE OF THE FIRST PANEL POINT.
- POINT LOADS IN EXCESS OF 350LBS THAT ARE NOT SPECIFICALLY NOTED ON THE STRUCTURAL DRAWINGS SHALL NOT BE PLACED WITHOUT APPROVAL FROM THE ENGINEER OF RECORD AND THE JOIST MANUFACTURER.

NOTE: JOISTS LABELED AS "SP" HAVE BEEN DESIGNED FOR THE LOADS NOTED ON THE STRUCTURAL PLANS. THOSE LOADS SHALL BE PLACED WITHIN 2" OF A PANEL POINT, OR ADDITIONAL WEB MEMBERS SHALL BE ADDED.

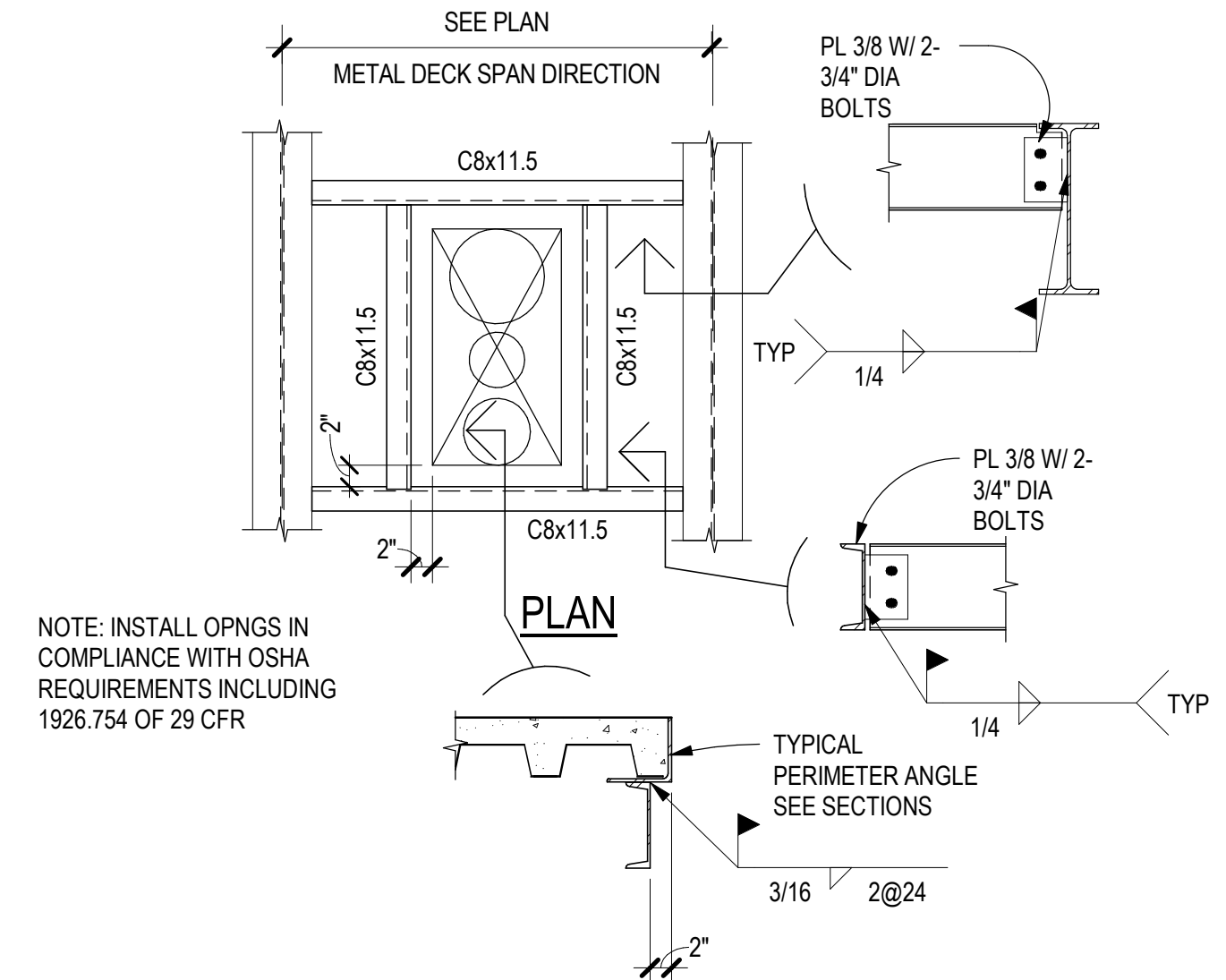
C4 TYPICAL JOIST REINFORCING
SCALE: 1/2" = 1'-0"



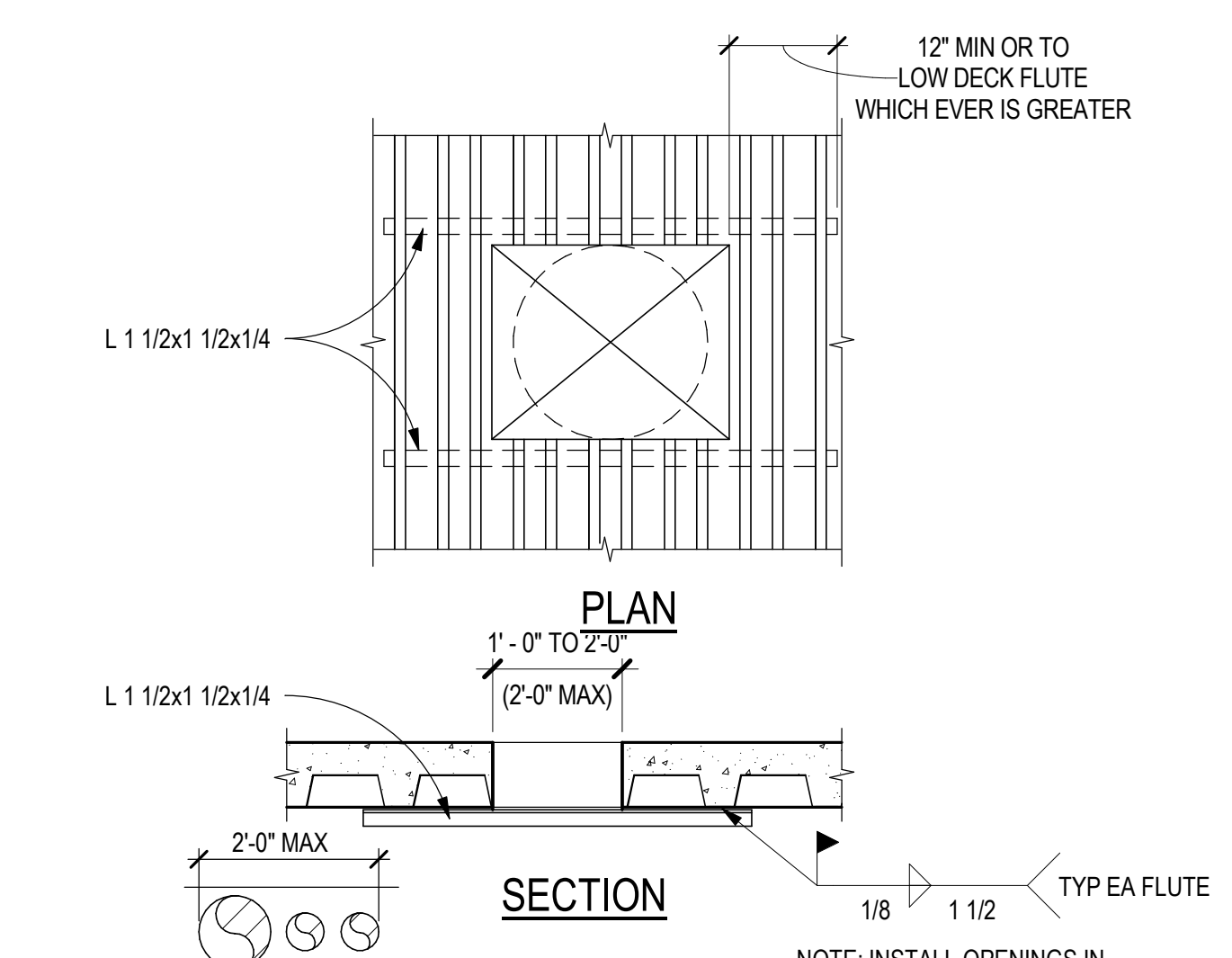
B4 TYPICAL PERIMETER ANGLE SPLICE
SCALE: 3/4" = 1'-0"



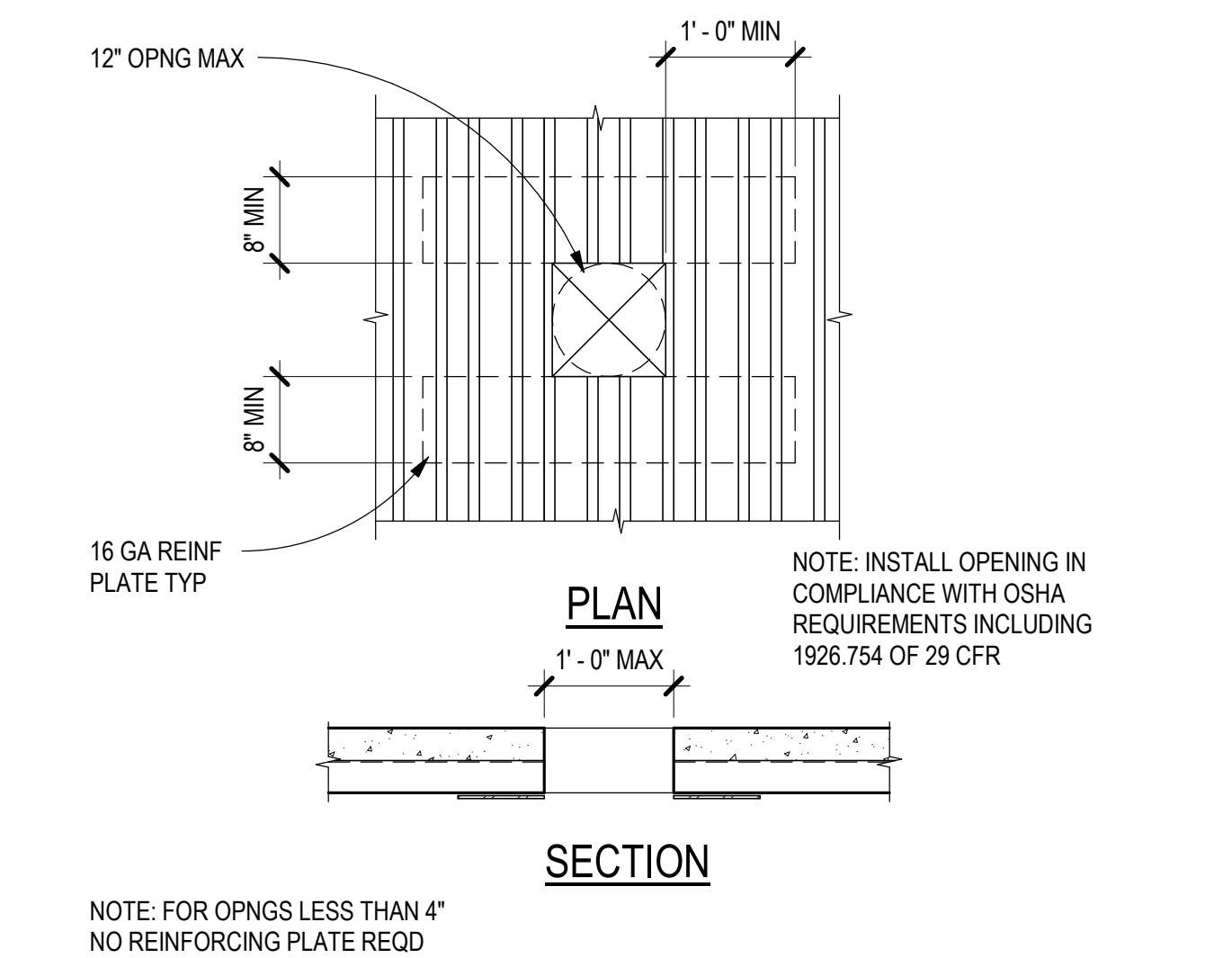
A4 TYPICAL ROOF OPENING > 12''
SCALE: 3/4" = 1'-0"



C5 TYPICAL SLAB OPNG > 24''
SCALE: 3/4" = 1'-0"



B5 TYPICAL SLAB OPENING 12''-24''
SCALE: 3/4" = 1'-0"



A5 TYPICAL SLAB OPENING 4''-12''
SCALE: 3/4" = 1'-0"

#	DATE	REVISIONS DESCRIPTION

PROJECT SPECIFIC INFORMATION

Table with 2 columns: Field and Value. Includes PROJECT NUMBER (18250), SUBMITTAL NUMBER (S-02), SUBMITTAL DATE (10/28/2019), PROJECT NAME (STILWELL HEALTH CLINIC), LOCATION (STILWELL, OK), CONNECTION TYPE (SIDE PLATE), NUMBER OF BUILDINGS (1), APPROX. TOTAL GROSS SQUARE FOOTAGE (104714), NUMBER OF STORIES (2).

DATA:

- 1. THERE MAY BE eDATA AVAILABLE FOR YOUR PROJECT WHICH IS AVAILABLE FOR DOWNLOAD AT WWW.SIDEPLATE.COM. eDATA MAY INCLUDE:
- eSTIMATE FILE IN EXCEL FORMAT FOR USE IN AFFIRMING SIDEPLATE CONNECTION MATERIAL QUANTITIES.
- eCOMPRIE/MIL FILE FOR USE IN ASSISTING DETAILING EFFORTS.
2. ESTIMATED NUMBER OF SIDEPLATE JOINTS FOR THIS PROJECT = 54
3. ESTIMATED NUMBER OF SIDEPLATE JOINTS FOR THIS PROJECT THAT ARE NOT SUPPORTED BY eDATA = 6
4. MISCELLANEOUS DETAILS, TYPICALLY DESIGNATED BY MR, ARE NOT SUPPORTED.

INSTRUCTIONS TO STEEL FABRICATOR

- 1. SIDEPLATE LICENSE FEE
a. THE STEEL FABRICATOR'S BID PRICE FOR PROCUREMENT, FABRICATION AND ERECTION OF STRUCTURAL AND MISCELLANEOUS STEEL SHALL INCLUDE THE SIDEPLATE LICENSE FEE FOR THE PROJECT. EACH PROSPECTIVE STEEL FABRICATOR WHO BIDS THE PROJECT SHALL FORMALLY REQUEST THE SIDEPLATE LICENSE FEE BY ACCESSING THE SIDEPLATE WEBSITE (http://www.sideplate.com).
b. UPON THE SUCCESSFUL STEEL FABRICATOR SIGNING A CONTRACT TO FABRICATE STRUCTURAL STEEL FOR THIS PROJECT, THE STEEL FABRICATOR SHALL SUBMIT A PURCHASE ORDER (PO) TO SIDEPLATE SYSTEMS, INC. FOR THE TOTAL AMOUNT OF THE SIDEPLATE LICENSE FEE AND SHALL INCLUDE SAID FEE IN ITS FIRST CONSTRUCTION DRAW.
c. THE STEEL FABRICATOR SHALL MAKE PAYMENT OF THE SIDEPLATE LICENSE FEE DIRECTLY TO:
SIDEPLATE SYSTEMS, INC.
25909 PALA, SUITE 200
MISSION VIEJO, CA 92691
TEL: 949-238-9900

SUBMITTALS

- 1. IN ADDITION TO THE REQUIRED SUBMITTALS SPECIFIED BY THE BALANCE OF THE CONTRACT DOCUMENTS, THE FOLLOWING SUBMITTALS SHALL BE SENT TO SIDEPLATE SYSTEMS, INC. ELECTRONICALLY VIA THE STRUCTURAL ENGINEER OF RECORD FOR THEIR REVIEW AND DISPOSITION:
a. QUALITY CONTROL PROGRAM (REQUIRED IF NOT AISC CERTIFIED)
b. ONE ELECTRONIC COPY OF ALL STRUCTURAL STEEL DRAWINGS THAT EITHER DIRECTLY PERTAINS TO AND/OR AFFECTS THE SHOP FABRICATION OR FIELD ERECTION OF THE SIDEPLATE STEEL FRAME CONNECTION SYSTEM, INCLUDING THE INITIAL SUBMITTAL AND ALL CORRECTED RE-SUBMITTALS OF AFFECTED DRAWINGS. SIDEPLATE SYSTEMS, INC. SHALL BE GIVEN, AS A MINIMUM, THE SAME SPECIFIED REVIEW TIME (NOT LESS THAN SEVEN BUSINESS DAYS) AS THE ENGINEER OF RECORD.

MEETINGS

- 1. PRE-DETAILING MEETING
a. PRIOR TO THE START OF DETAILING OF THE SHOP DRAWINGS, THE FABRICATION CONTRACTOR SHALL FORMALLY REQUEST A PRE-DETAILING MEETING FROM SIDEPLATE SYSTEMS, INC. THIS MEETING IS TYPICALLY A WEBINAR TO DISCUSS BEST PRACTICES FOR THE DETAILING OF THE SIDEPLATE CONNECTIONS, AND TO CREATE A PROACTIVE FORUM TO ANSWER ANY QUESTIONS.
2. PRE-FABRICATION MEETING
a. PRIOR TO THE START OF FABRICATION, THE FABRICATION CONTRACTOR SHALL FORMALLY REQUEST A PRE-FABRICATION MEETING FROM SIDEPLATE SYSTEMS, INC. THIS MEETING IS TYPICALLY A WEBINAR TO DISCUSS BEST PRACTICES FOR THE FABRICATION OF THE SIDEPLATE CONNECTIONS, AND TO CREATE A PROACTIVE FORUM TO ANSWER ANY QUESTIONS.
3. PRE-ERECTION MEETING
a. PRIOR TO THE START OF STEEL ERECTION, THE ERECTION CONTRACTOR SHALL FORMALLY REQUEST A PRE-ERECTION MEETING FROM SIDEPLATE SYSTEMS, INC. THIS MEETING IS TYPICALLY A WEBINAR TO DISCUSS BEST PRACTICES FOR FIELD ERECTION OF THE SIDEPLATE BEAMS AND COLUMNS, AND TO CREATE A PROACTIVE FORUM TO ANSWER ANY QUESTIONS.

GENERAL

- 1. THE GOVERNING CODES SHALL CONSIST OF ANSIAWS D1.1-2010 (AWS D1.1), AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES (APRIL 14, 2010), 2009 RCSC SPECIFICATIONS FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS, AND ALL APPLICABLE BUILDING AND JURISDICTIONAL CODES AND PROJECT STANDARDS SPECIFIED IN THE PROJECT SPECIFICATION STRUCTURAL STEEL SECTION WHERE THE REQUIREMENTS DIFFER BETWEEN SIDEPLATE CONNECTION NOTES, THE GENERAL STRUCTURAL NOTES, AND THE GOVERNING CODES, THE MORE STRINGENT SECTION CRITERIA SHALL CONTROL.
2. ALPHA AND NUMERIC DESIGNATORS (A) & (H) USED HEREIN TO SIMPLIFY THE IDENTIFICATION OF PLATES, ANGLES, AND WELDS ARE DEFINED BELOW.

- (A) SIDE PLATE FOR UNAXIAL CONNECTIONS
(B) BEAM FLANGE COVER PLATE, AS REQUIRED
(C) VERTICAL SHEAR PLATE OR FLAT BAR, AS REQUIRED
(D) HORIZONTAL SHEAR PLATE OR FLAT BAR, AS REQUIRED
(E) VERTICAL ANGLE WELDED TO THE VERTICAL SHEAR PLATE (C), AS REQUIRED
(F) VERTICAL SHEAR ELEMENT (VSE) WHICH CONSISTS OF PLATE (C) AND ANGLE (E) MATERIAL, AS REQUIRED
(G) LONGITUDINAL ANGLE WELDED TO THE OUTSIDE FACE OF SIDE PLATE (A), AS REQUIRED
(H) LONGITUDINAL ANGLE WELDED TO THE BOTTOM BEAM FLANGE (OR TOP BEAM FLANGE AS REQUIRED)
(I) HORIZONTAL PLATE WELDED TO THE OUTSIDE FACE OF SIDE PLATE (A), AS REQUIRED
(1) FILLET WELD CONNECTING SIDE PLATE (A) TO HORIZONTAL SHEAR PLATE (D) OR COLUMN
(2) FILLET (AND/OR FLARE BEVEL) WELD CONNECTING INSIDE FACE OF SIDE PLATE (A) TO COLUMN
(3) FILLET WELD CONNECTING HORIZONTAL SHEAR PLATE (D) TO COLUMN, AS REQUIRED
(4) FILLET WELD TO CONSTRUCT VSE (F) AND TO CONNECT IT TO THE WEB OF THE BEAM, AS REQUIRED
(5) FILLET (AND/OR PJP) WELD CONNECTING BEAM FLANGE TIPS TO COVER PLATE (B) AND/OR LONGITUDINAL ANGLE (H), AS REQUIRED
(5a) FILLET WELD CONNECTING OUTSIDE FACE OF BEAM FLANGE TO COVER PLATE (B) AND/OR LONGITUDINAL ANGLE (H), AS REQUIRED
(5b) FILLET WELD CONNECTING COVER PLATE (B) EDGE TO TOP FACE OF BEAM FLANGE, ACROSS ITS WIDTH
(5c) PJP WELD CONNECTING ANGLE (H) TO BEVELED BEAM FLANGE
(8) FILLET (AND/OR PJP) WELD CONNECTING LONGITUDINAL ANGLE (G) (AND/OR PLATE (I)) TO SIDE PLATE (A), AS REQUIRED
(8b) PJP WELD CONNECTING PLATE (I) TO SIDE PLATE (A) AND/OR CONNECTING BUILT UP ANGLE (H) PLATES TOGETHER, AS REQUIRED
(9) FILLET WELD CONNECTING SIDE PLATE (A) TO COLUMN FACE, WRAPPED AROUND THREE SIDES OF SIDE PLATE (A)
(10) FILLET WELD TO CONSTRUCT SIDE PLATE SLOTTED INTERLOCK ASSEMBLY
(10a) PJP WELD TO CONSTRUCT SIDE PLATE SLOTTED INTERLOCK ASSEMBLY
(10b) REINFORCING FILLET WELD TO CONSTRUCT SIDE PLATE SLOTTED INTERLOCK ASSEMBLY
3. ALPHA DESIGNATORS, USED HEREIN TO SIMPLIFY THE IDENTIFICATION OF DIMENSIONS OF THE SIDEPLATE CONNECTIONS, ARE DEFINED BELOW:
GAP PHYSICAL SEPARATION BETWEEN THE END OF THE MOMENT FRAME BEAM AND THE ADJOINING FACE OF THE COLUMN FLANGE
B DEPTH OF SIDE PLATE (A)
C LENGTH OF COVER PLATE (B) AND/OR LONGITUDINAL ANGLE (H)
D LENGTH OF SLOT FROM THE TOE OF THE RADIUS IN THE COVER PLATE (B), AS REQUIRED
E EDGE DISTANCE OF BOLT HOLES IN COVER PLATE (B), AS REQUIRED
G GAGE DISTANCE TO CENTERLINE OF BOLT HOLES IN ANGLES (G) AND (H), AND PLATE (I), AS REQUIRED
H ADDED DIMENSION TO COLUMN FLANGE WIDTH TO DEFINE TOTAL COVER PLATE (B) WIDTH
J DISTANCE FROM END OF THE BEAM TO CENTERLINE OF VERTICAL BOLT HOLES IN VSE (F), AS REQUIRED
R RADIUS OF SLOT DIMENSION IN COVER PLATE (B)
S HORIZONTAL SPACING BETWEEN BOLT HOLES
Y ADDED DIMENSION TO COLUMN FLANGE WIDTH FOR ALLOWABLE SPREAD OF SIDE PLATES (A)

MATERIAL

- 1. PLATE, FLAT BAR, AND ANGLE MATERIAL:
a. ALL PLATE MATERIAL SHALL HAVE A MINIMUM YIELD STRENGTH (Fy) OF 50 KSI.
b. ANGLE AND BAR MATERIAL SHALL HAVE A HIGH STRENGTH STEEL SPECIFICATION AND SHALL HAVE A MINIMUM YIELD STRENGTH (Fy) OF 50 KSI.
2. HIGH STRENGTH BOLT FASTENERS:
a. BOLTS SHALL BE TYPE 1 OR TYPE 3 AND SHALL BE A490X HEAVY HEX, F2280 TWIST-OFF-TYPE TENSION-CONTROL BOLT ASSEMBLIES, OR F3148 FIXED SPLINE BOLT ASSEMBLIES. THE BOLT HEAD SHALL BE DISTINCTIVELY MARKED WITH A MINIMUM MARKING OF A490, A490TC, OR 144 RESPECTIVELY, AN ALTERNATIVE DESIGN THAT MEETS THE REQUIREMENTS OF RCSC SECTION 2.8 MAY BE USED, WITH THE WRITTEN APPROVAL FROM SIDEPLATE SYSTEMS, INC.
b. WASHERS SHALL BE ORDINARY THICKNESS AND ASTM F436 TYPE 1 OR TYPE 3.
c. NUTS SHALL BE ASTM A563 GRADE DH OR DH3.
d. THE BOLT ASSEMBLY SHALL BE COVERED IN A LIGHT PROTECTIVE OIL, F2280 AND F3148 ASSEMBLIES SHALL ONLY BE LUBRICATED BY THE SUPPLIER.
e. THE MILL TEST REPORT (MTR) MUST HAVE DOCUMENTED LOT TRACEABILITY, STATEMENT OF DIMENSIONAL RESULTS, FULL CHEMICAL AND MECHANICAL TEST RESULTS TO THE SPECIFICATIONS ABOVE.
f. THE USE OF FINGER SHIMS ARE ACCEPTABLE PER BOLTING SECTION 8.
3. ROLLED SHAPES:
a. ALL ROLLED SHAPES USED FOR COLUMNS AND BEAMS IN CONSTRUCTING SIDEPLATE MOMENT FRAMES SHALL BE ASTM A992 GRADE 50 UNO.
b. HSS TUBE SHAPES:
a. ALL HSS SHAPES USED FOR COLUMNS AND BEAMS IN CONSTRUCTING SIDEPLATE MOMENT FRAMES SHALL, AS A MINIMUM, BE ASTM A500 GRADE B OR GRADE C OR ASTM1085.

PREPARATION

- 1. THE STEEL FABRICATION AND ERECTION SUBCONTRACTORS SHALL EMPLOY A DISTORTION CONTROL PROGRAM PRIOR TO THE START OF SIDEPLATE MOMENT FRAME FABRICATION. THE DISTORTION CONTROL PROGRAM SHALL BE IN ACCORDANCE WITH THE PROVISIONS OF AWS D1.1 SECTION 5.21 AND 5.22 TO ENSURE THAT THE FOLLOWING ARE MAINTAINED:
- DIMENSIONAL ACCURACY
- FRAMING AND ALIGNMENT TOLERANCES
- COMPLIANCE WITH AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES, SECTION 7.0, ERECTION PROVISIONS
- CONTROL OF DISTORTION AND WELD SHRINKAGE

WELDING

- 1. WELDER QUALIFICATION: THE PERFORMANCE OF ALL WELDERS, WELDING OPERATORS AND TACK WELDERS SHALL BE QUALIFIED IN CONFORMANCE WITH AWS D1.1, SECTION 4, PART C TO DEMONSTRATE ABILITY TO PRODUCE SOUND WELDS.

BOLTING

- 1. BOLTS/FASTENERS SHALL BE INSTALLED TO PRETENSIONED CONDITION USING ONE OF THE METHODS PRESCRIBED HERE: TURN-OF-NUT (A490), CALIBRATED WRENCH (A490), TWIST-OFF-TYPE TENSION-CONTROL BOLT (F2280), OR TORQUE AND ANGLE METHOD (F3148).
2. FOR ALL PRETENSIONING METHODOLOGIES, ALL FASTENER ASSEMBLIES WITHIN THE JOINT SHALL FIRST BE BROUGHT TO A SNUG TIGHT CONDITION, FOLLOWED BY A SYSTEMATIC PRETENSIONING PROCESS. PRETENSIONING SHALL BEGIN AT THE MOST RIGID PART OF THE JOINT AND CONTINUE IN A MANNER THAT WILL MINIMIZE THE RELAXATION OF PREVIOUSLY PRETENSIONED FASTENERS, UNTIL THE CONNECTED PLIES ARE IN AS FIRM CONTACT AS POSSIBLE.

- 3. REUSE OF A490, F2280, AND F3148 BOLT ASSEMBLIES SHALL NOT BE ALLOWED. TOUCHING UP OR RE-TIGHTENING BOLTS THAT MAY HAVE BEEN LOOSENEED BY THE INSTALLATION OF ADJACENT BOLTS SHALL NOT BE CONSIDERED TO BE A REUSE.
4. ALL BOLT HOLES SHALL BE ALIGNED TO PERMIT INSERTION OF THE BOLTS WITHOUT UNDUE DAMAGE TO THE THREADS.
5. THE BOLT LENGTH USED SHALL BE SUCH THAT THE BOLT THREAD EXTENDS BEYOND OR IS AT LEAST FLUSH WITH THE OUTER FACE OF THE NUT WHEN PROPERLY INSTALLED.
6. FASTENER COMPONENTS SHALL BE PROTECTED FROM DIRT AND MOISTURE IN CLOSED CONTAINERS AT THE SITE OF INSTALLATION.
7. F2280 OR F3148 ASSEMBLIES AND ALTERNATIVE DESIGN FASTENERS THAT MEET THE SPECIFIED REQUIREMENTS PREVIOUSLY MENTIONED SHALL NOT BE RE-LUBRICATED, EXCEPT BY THE MANUFACTURER.
8. FINGER SHIMMING SHALL BE USED UP TO 1/4 INCH WITHOUT RESTRICTION. SHIM REQUIREMENTS GREATER THAN 1/4 INCH SHALL BE SUBMITTED TO SIDEPLATE SYSTEMS INC FOR APPROVAL PRIOR TO USE.
9. WASHERS SHALL BE ASTM F436 ORDINARY THICKNESS AND SHALL BE USED UNDER THE NUT OF THE FASTENER ASSEMBLY SO AS TO PROVIDE A HARDENED NON-GALLING SURFACE OF THE TURNED ELEMENT. WHEN USING THE TURN-OF-NUT OR CALIBRATED WRENCH METHOD, THE TURNED ELEMENT MUST BE THE SAME AS WAS USED WHEN PERFORMING PREINSTALLATION VERIFICATION TESTING.

QUALITY CONTROL

- 1. THE FABRICATOR AND ERECTOR SHALL BE RESPONSIBLE FOR QUALITY CONTROL BY PROVIDING, AS A MINIMUM, IN-PROCESS VISUAL INSPECTION OF ALL FABRICATION AND ERECTION ACTIVITIES TO ENSURE THAT MATERIALS AND WORKMANSHIP MEET THE REQUIREMENTS OF THE CONTRACT DOCUMENTS AND SHALL INCLUDE WORK PERFORMED PRIOR TO ASSEMBLY. SUCH WORK SHALL INCLUDE, BUT NOT BE LIMITED TO, VERIFYING THAT EFFECTIVE PROCEDURES AND METHODS HAVE BEEN EMPLOYED IN THE FORM OF A DISTORTION CONTROL PROGRAM TO ACCOUNT FOR AND COUNTERACT THE EFFECTS OF WELD SHRINKAGE, EXISTING BEAM SWEEP AND CAMBER, AND CHANGES IN MOMENT FRAME GEOMETRY DUE TO STEEL AND CURVED DESIGN CONFIGURATIONS (AS OCCURS), TO ENSURE COMPLIANCE WITH SPECIFIED ERECTION AND ALIGNMENT TOLERANCES. QC INSPECTION SHALL INCLUDE HOLD POINTS FOR THE FOLLOWING:

- a. COLUMN TREE ASSEMBLY
1. VERIFICATION THAT ACTUAL COLUMN FLANGE WIDTH IS AT LEAST NOMINAL COLUMN FLANGE WIDTH WHERE THE SIDE PLATES (A) ARE TO BE INSTALLED. IN THE UNLIKELY EVENT ACTUAL COLUMN FLANGE WIDTH IS LESS THAN NOMINAL, BUT WITHIN AISC STANDARD MILL TOLERANCES (3/16 INCH MAX), CONTACT SIDEPLATE SYSTEMS, INC FOR APPROPRIATE RECOMMENDATIONS.
2. MINIMUM CLEAR DIMENSION SHALL BE VERIFIED AFTER PLACEMENT OF WELD (2), COOLING OF WELD (2), AND REMOVAL OF TEMPORARY SHOP CONSTRUCTION AID(S). VERIFY THAT A MINIMUM ACTUAL COLUMN FLANGE WIDTH DIMENSION OCCURS ANYWHERE IN BETWEEN THE SIDE PLATES (A) FROM TOP TO BOTTOM. THE SIDE PLATES SHALL BE PARALLEL TO ONE ANOTHER. IN NO CASE SHALL THEY BE LESS THAN THE ACTUAL COLUMN FLANGE WIDTH.
3. MAXIMUM SPREAD DIMENSION OF SIDE PLATE (A) SHALL NOT EXCEED ACTUAL COLUMN FLANGE WIDTH PLUS THE SCHEDULED SPREAD DIMENSION. THE FIELD CONSTRUCTION AID SHALL BE PLACED AND HOLD THE SIDE PLATES IN THIS FLARED CONDITION UNTIL THE BEAM HAS BEEN SAFELY ERECTED. IN NO CASE SHALL THE SPREAD CAUSE PERMANENT DEFORMATION IN THE SIDE PLATES.
4. VERIFICATION OF BOLT HOLE ELEVATION AND SPACING FOR POSITION OF SIDE PLATE (A) AND PROPER POSITION AND ELEVATION OF ANGLES (G).
b. BEAM ASSEMBLY
1. VERIFICATION OF PERPENDICULAR ALIGNMENT BETWEEN THE TOP COVER PLATE (B) AND BOTTOM ANGLES (H) TO THE WEB OF THE BEAM. TO MINIMIZE, IF NOT ELIMINATE, ANY MISALIGNMENT OF BOLT HOLES DUE TO BEAM FLANGE TILT WHEN THE BEAM HAS BEEN LOWERED INTO PLACE.
2. VERIFICATION OF BOLT HOLE SPACING AND POSITION ON COVER PLATE (B) AND ANGLES (H). CONSIDERATION SHALL BE GIVEN TO THE CURPING EFFECT OF THE TOP COVER PLATE (B), DUE TO WELD SHRINKAGE.
3. VERIFICATION OF THE DISTANCE BETWEEN EXTERIOR ANGLE (H) FACES AND THEIR RESPECTIVE BOLT HOLE PLACEMENT TO EACH OTHER (VERTICALLY AND HORIZONTALLY).
4. VERIFICATION THAT IN NO CASE SHALL THE OUTSIDE FACE OF VSE (F) EXTEND BEYOND THE OUTSIDE FACES OF THE LONGITUDINAL ANGLES (H).
5. CORRECTION THAT VERTICAL PLACEMENT OF VSE (F) IS IN THE CORRECT LOCATION.
2. FILLET WELD FIT-UP TOLERANCES
a. THE PARTS TO BE JOINED BY FILLET WELDS SHALL BE BROUGHT INTO AS CLOSE CONTACT AS PRACTICABLE, USING AS NECESSARY SUITABLE CLAMPING MEANS. THE ROOT OPENING (I.E. THE FIT-UP GAP) SHALL NOT EXCEED 1/4 INCH. FOR FILLET WELD ROOT GAPS GREATER THAN 1/16 INCH, THE LEG SIZE (I.E. THE SPECIFIED SIZE) OF FILLET WELD SHALL BE INCREASED BY THE AMOUNT OF THE ROOT OPENING.
3. THERMAL CUTTING:
a. THE ROUGHNESS OF ALL THERMAL CUT SURFACES SHALL BE NO GREATER THAN AN ANSI SURFACE ROUGHNESS VALUE OF 1000 MICRO-INCHES. ROUGHNESS EXCEEDING THIS VALUE AND NOTCHES OR GOUGES NOT MORE THAN 3/16 INCH DEEP SHALL BE REMOVED BY MACHINING OR GRINDING. NOTCHES OR GOUGES IN THE THERMALLY CUT EDGES DEEPER THAN 3/16 INCH SHALL BE REPAIRED PER AWS.
4. TENSION CALIBRATION FOR PRE-INSTALLATION:
a. TENSION CALIBRATION SHALL BE USED TO CONFIRM THE SUITABILITY OF THE COMPLETE FASTENER ASSEMBLY, AND THE PROCEDURE TO BE USED BY THE BOLTING CREW.

QUALITY ASSURANCE

- 1. WELDING
a. TO ASSURE THE PROPER AMPERAGE AND VOLTAGE OF THE WELDING PROCESS, THE USE OF HAND HELD CALIBRATED AMP AND VOLT METERS SHALL BE USED. THIS EQUIPMENT SHALL BE USED BY THE FABRICATOR AND THE INSPECTOR. AMPERAGE AND VOLTAGE SHALL BE MEASURED NEAR THE ARC. TRAVEL SPEED AND ELECTRODE STICK OUT SHALL BE VERIFIED TO BE IN COMPLIANCE WITH THE APPROVED WPS.
b. VISUAL INSPECTION SHALL BE PERFORMED ON ALL SHOP WELDS.
c. EACH WELDER EMPLOYED ON THE PROJECT SHALL UNDERSTAND ALL THE REQUIREMENTS OF THE WELDING PROCEDURE SPECIFICATION(S) BEFORE WELDING ON THE PROJECT.
d. AS-BUILT BEAM TO COLUMN GAP PER CONNECTION SCHEDULE IS ALLOWED TO BE INSTALLED WITH A TOLERANCE OF PLUS OR MINUS 1/2 INCH.
2. FINISH SURFACES:
a. THE SURFACES ADJACENT TO THE BOLT HEAD AND NUT SHALL BE FREE OF DIRT AND OTHER FOREIGN MATERIAL OTHER THAN THE SPECIFIED COATINGS.
b. FINISH SURFACES ARE PERMITTED TO BE UNCOATED AND COATED WITH ANY COATINGS OF ANY FORMULATION OR GALVANIZATION.
c. AFTER THE CONNECTIONS HAVE BEEN ASSEMBLED, VISUALLY ENSURE THAT THE PLIES OF THE CONNECTED ELEMENTS HAVE BEEN BROUGHT INTO AS CLOSE OF CONTACT AS PRACTICABLE WITH ONE ANOTHER. GAPS UP TO 1/8 INCH BETWEEN THE SURFACES SHALL BE ALLOWED. GAPS GREATER THAN 1/8 INCH UP TO 1/4 INCH SHALL HAVE FINGER SHIMS INSTALLED BEFORE PRETENSIONING. FOR GAPS GREATER THAN 1/4 INCH, CONTACT SIDEPLATE SYSTEMS, INC.

HOT DIPPED GALVANIZING

- 1. SIDEPLATE CONNECTIONS REQUIRING THIS TYPE OF FINISH SHALL FOLLOW THE SAME CONSTRUCTION SEQUENCING AS PREVIOUSLY OUTLINED WITH THE FOLLOWING MODIFICATIONS:
a. HORIZONTAL SHEAR PLATES (C) SHALL HAVE AN INCREASED CLIP SIZE WHICH SHALL BE 1.58 INCH BY 1.58 INCH TO PROVIDE ADEQUATE VENTILATION AND DRAINAGE. CONTACT SIDEPLATE SYSTEMS, INC. IN THE EVENT THAT THE GALVANIZING CONTRACTOR SPECIFICATIONS REQUIRE A LARGER OPENING THAN THAT SPECIFIED HEREIN.
b. SEAL WELDING SHALL BE ALLOWED ON THE PLATES (B) AND ANGLES.
c. ANY DEVIATIONS TO THESE MODIFICATIONS SHALL BE COORDINATED WITH SIDEPLATE SYSTEMS, INC. AND THE SEOR.
FIREPROOFING
1. WHEN REQUIRED BY THE GOVERNING CODE FOR CERTAIN TYPES OF CONSTRUCTION, SIDEPLATE CONNECTIONS SHALL HAVE A FIRE-RESISTANCE RATING LIKE THAT OF A STEEL "STRUCTURAL FRAME".
2. THE MINIMUM THICKNESS OF SPRAY-APPLIED FIRE-RESISTIVE MATERIAL (SFRM) FOR STEEL SIDEPLATE CONNECTIONS PLATES THAT ARE NOT ENCASED IN CONCRETE, SHALL BE DETERMINED JUST LIKE THAT OF A PIPE/TUBE COLUMN SECTION WITH A CONSTANT STEEL WALL THICKNESS USING THE THICKNESS OF SIDE PLATE (A) FOR EACH SIDEPLATE CONNECTION ID PER THE SIDEPLATE CONNECTION SCHEDULE, WHICH ARE UNIFORMLY HEATED AND PROTECTED (THE FIRE EXPOSURE OF A PIPE/TUBE COLUMN IS DIRECTLY ANALOGOUS TO A PLATE WITH A 1-SIDED FIRE EXPOSURE AND PROTECTION). THE SFRM SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ASTM E119 AND LISTED FOR FIRE RESISTIVE PIPE/TUBE COLUMN APPLICATIONS FOR NO LESS THAN THE REQUIRED RATED TIME.
3. AS REQUIRED, WHEN NO VERTICAL SHEAR ELEMENT (F) EXISTS IN THE BEAM, SPRAY THE MINIMUM THICKNESS OF SFRM BETWEEN INSIDE OF SIDE PLATE (A) AND BEAM WEB COVERING ALL SURFACES INCLUDING COLUMN FLANGE. NOTE: THIS DOES NOT NECESSITATE FILLING THE CAVITY FULL.
4. WHEN VERTICAL SHEAR ELEMENT (F) IS USED, THE CONTRACTOR SHALL PROVIDE THE MEANS, TYPICALLY DONE WITH A LAYERING TECHNIQUE, FOR FIREPROOFING ACROSS THE BOTTOM OF THE GAP.
5. SEE GRAPHIC NUMBER 10 IN FIELD ERECTION OF THE SIDEPLATE BOLTED SYSTEM FOR FIREPROOFING ACROSS THE BOTTOM OF THE GAP.

INTELLECTUAL PROPERTY

- 1. IN ORDER TO SAFEGUARD THE AUTHORIZED USE AND INTELLECTUAL PROPERTY OF THE PATENTED SIDEPLATE CONNECTION TECHNOLOGY, THE STEEL FABRICATION SUBCONTRACTOR SHALL SATISFY THE FOLLOWING REQUIREMENTS:
a. A NOTICE OF INTELLECTUAL PROPERTY, IDENTICAL TO THAT PROVIDED ON THIS SHEET, SHALL BE AFFIXED ON EACH SHEET OF SHOP DETAIL AND FIELD ERECTION DRAWINGS CONTAINING SIDEPLATE SYSTEM INFORMATION WHICH DISCLOSES IN ANY WAY THE SIDEPLATE CONNECTION CONCEPT PRIOR TO RELEASING SUCH INFORMATION FOR ITS INTENDED USE. SUCH NOTICE SHALL BE PROVIDED TO THE STEEL FABRICATION SUBCONTRACTOR BY SIDEPLATE SYSTEMS, INC. IN A FORMAT (E.G. WORD OR AUTOCAD) SUITABLE TO THE NEEDS OF THE STEEL FABRICATION SUBCONTRACTOR'S DETAILER.
b. PATENT LABELS SHALL BE APPLIED ON THE OUTSIDE FACE OF ONE OF THE TWO BOTTOM HORIZONTAL SHEAR PLATES (D) OF EACH MOMENT CONNECTION AND ON ONE END OF THE BEAM WEB IN COMPLIANCE WITH THE PATENT AND INTELLECTUAL PROPERTY LAWS.

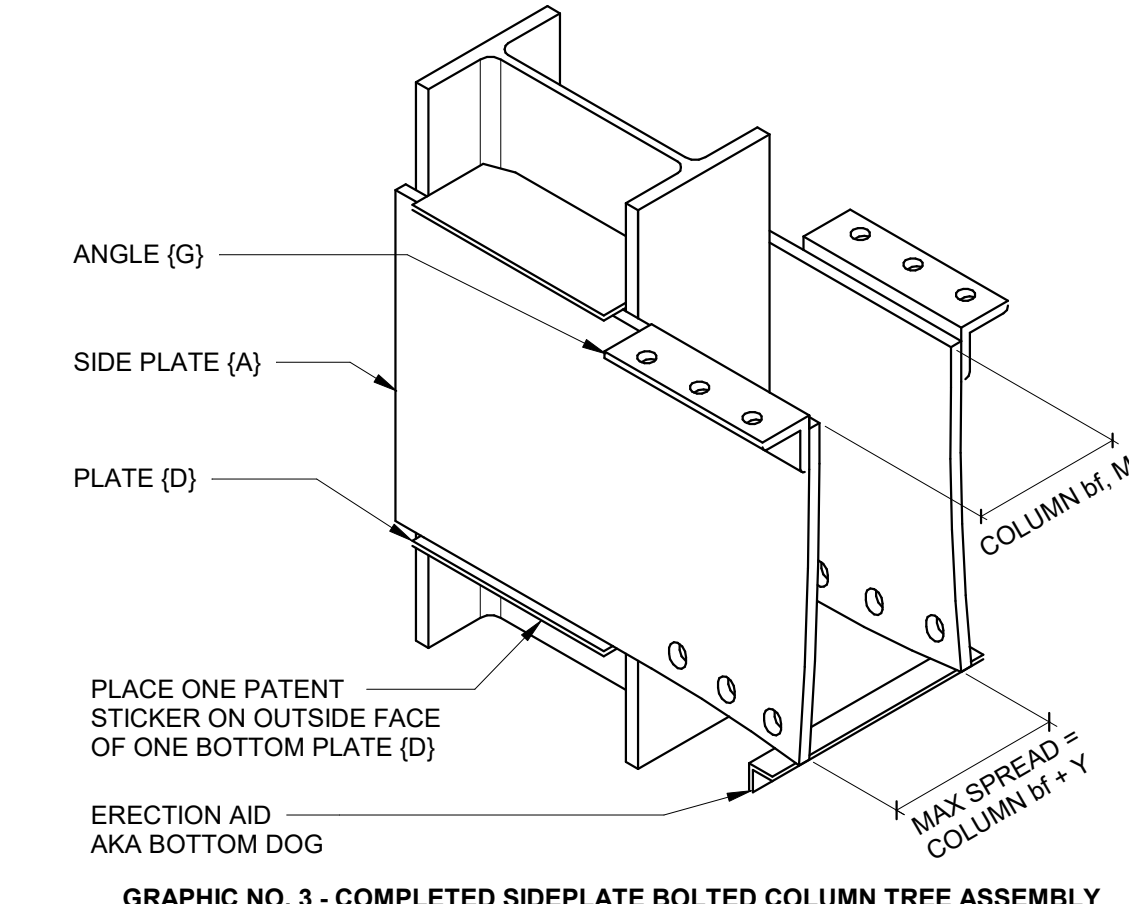
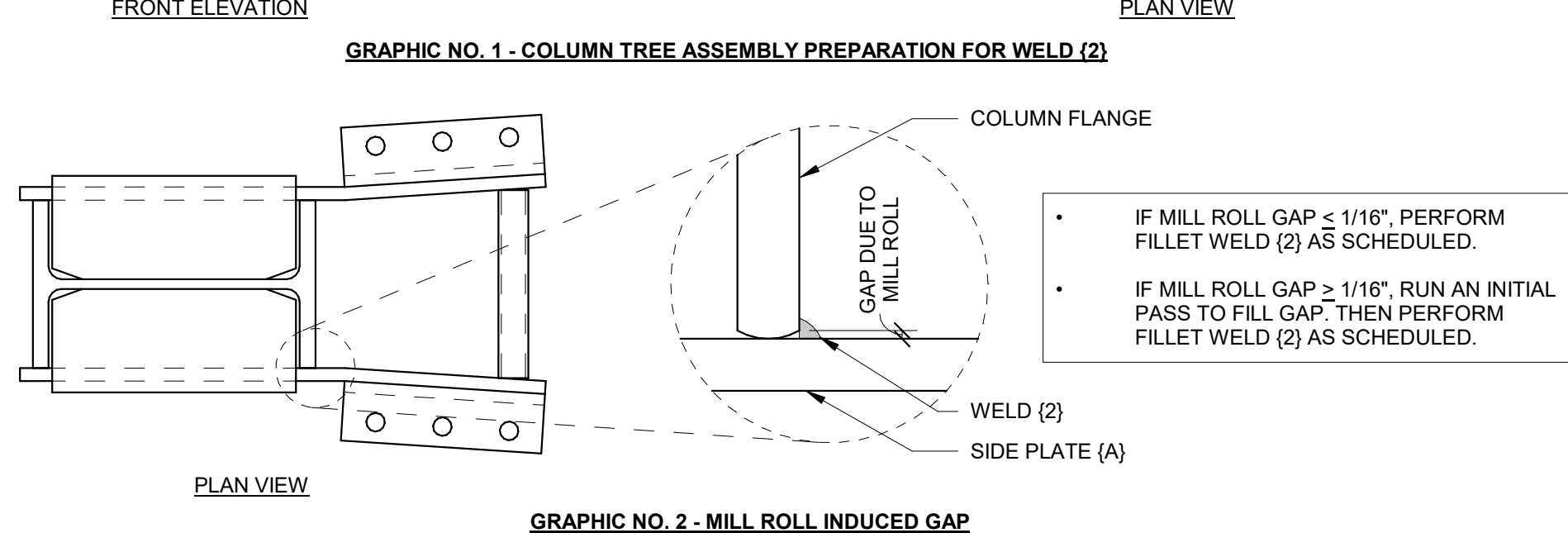
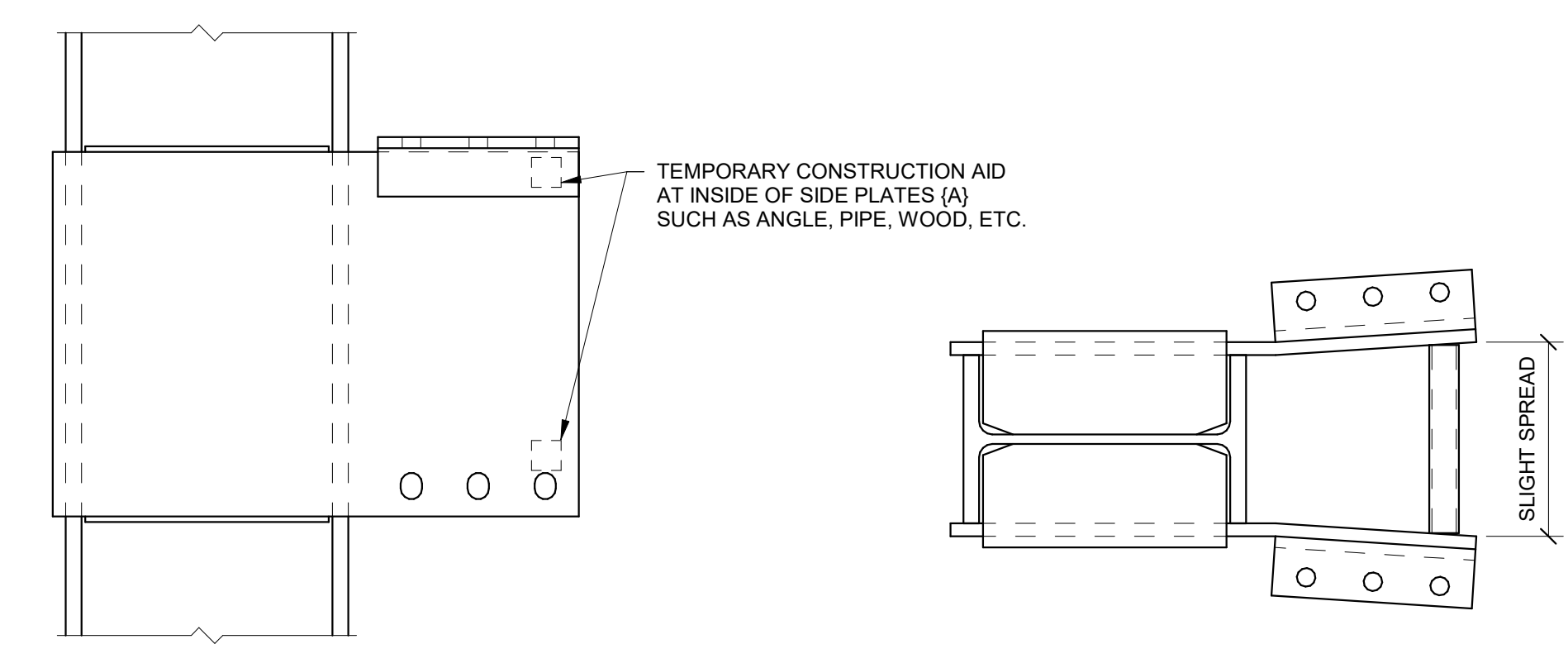
CONSTRUCTION GUIDELINES

- 1. THE CONTRACTOR SHALL ASSUME FULL AND COMPLETE RESPONSIBILITY FOR THE MEANS AND METHODS OF CONSTRUCTING THE STEEL FRAME USING THE SIDEPLATE BOLTED SYSTEM. CONSTRUCTION MEANS AND METHODS SHALL BE COMPLIANT WITH THE CURRENT PROVISIONS OF AWS D1.1, THE AISC 360 CODE OF STANDARD PRACTICE, THE RCSC HIGH-STRENGTH BOLTING SPECIFICATIONS, AND THE CONSTRUCTION GUIDELINES PROVIDED HEREIN AND SHALL INCLUDE, BUT ARE NOT LIMITED TO:
a. DIMENSIONAL VERIFICATION AND CONTROL
b. FABRICATION AND ERECTION PROCEDURES (INCLUDING METHODS FOR CONTROLLING DISTORTION DUE TO WELD SHRINKAGE, AND FOR CONTROLLING COMBINED MILL, FABRICATION AND ERECTION TOLERANCES)
c. CONSTRUCTION AIDS SUCH AS ERECTION RIGGING AND SHORING
d. PROPER BOLT HOLE ALIGNMENT
e. PROPER PRETENSIONING OF BOLTS
2. THE SEQUENCE OF CONSTRUCTION OPTIONS PROVIDED BELOW IN THESE CONSTRUCTION GUIDELINES HAVE PROVEN TO BE SUCCESSFUL BY STEEL FABRICATORS AND ERECTORS TO COST EFFICIENTLY CONSTRUCT THE BOLTED SIDEPLATE CONNECTION SYSTEM. VARIATIONS TO THESE CONSTRUCTION SEQUENCE OPTIONS PROVIDED BELOW SHALL BE SUBMITTED FOR REVIEW AND DISPOSITION TO SIDEPLATE SYSTEMS, INC.
3. A PRE-FABRICATION COORDINATION MEETING WITH A SIDEPLATE SYSTEMS, INC. REPRESENTATIVE IS REQUIRED FOR ALL PROJECTS. THE PRE-FABRICATION COORDINATION MEETING IS INTENDED TO SHARE BEST PRACTICES AND COMMON MISTAKES TO AVOID.

SHOP FABRICATION OF THE SIDEPLATE BOLTED SYSTEM

- 1. WATCH OUR SIDEPLATE COLUMN ASSEMBLY VIDEO AT

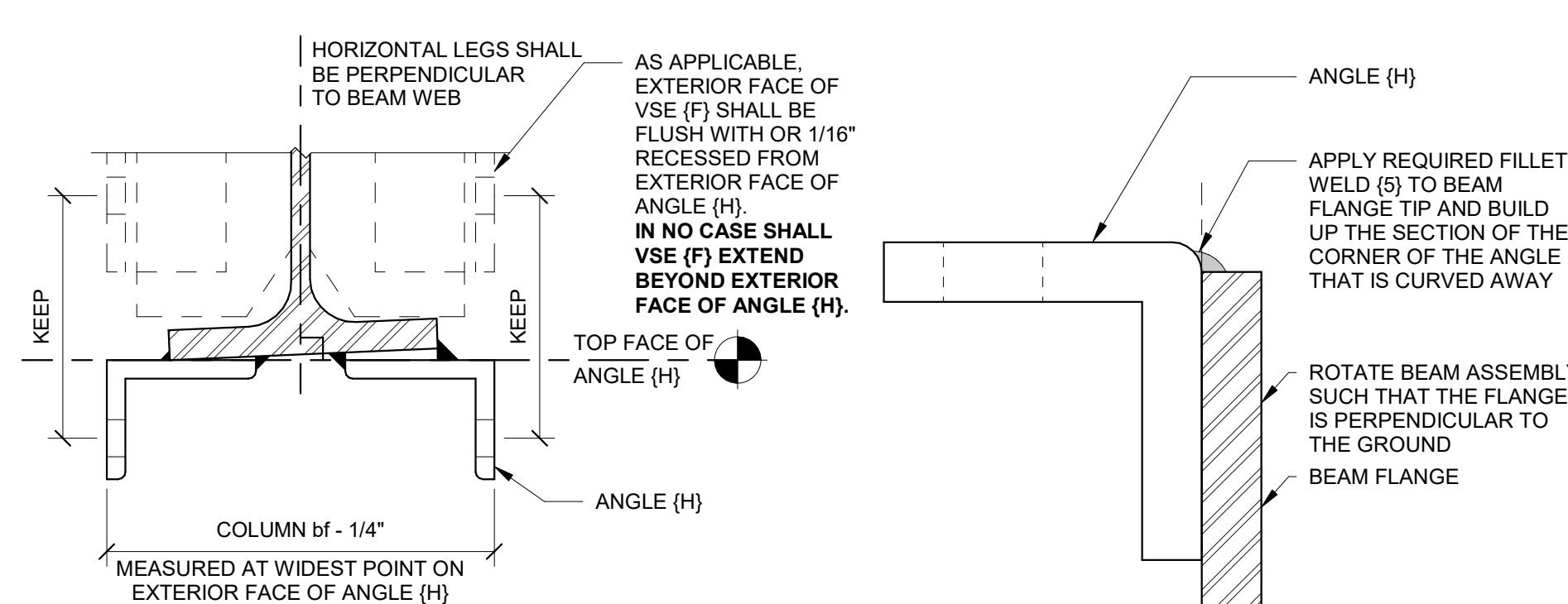
https://portal.sideplate.com/account/login



GRAPHIC NO. 3 - COMPLETED SIDEPLATE BOLTED COLUMN TREE ASSEMBLY

- 2. WATCH OUR SIDEPLATE BEAM ASSEMBLY VIDEO AT

https://portal.sideplate.com/account/login



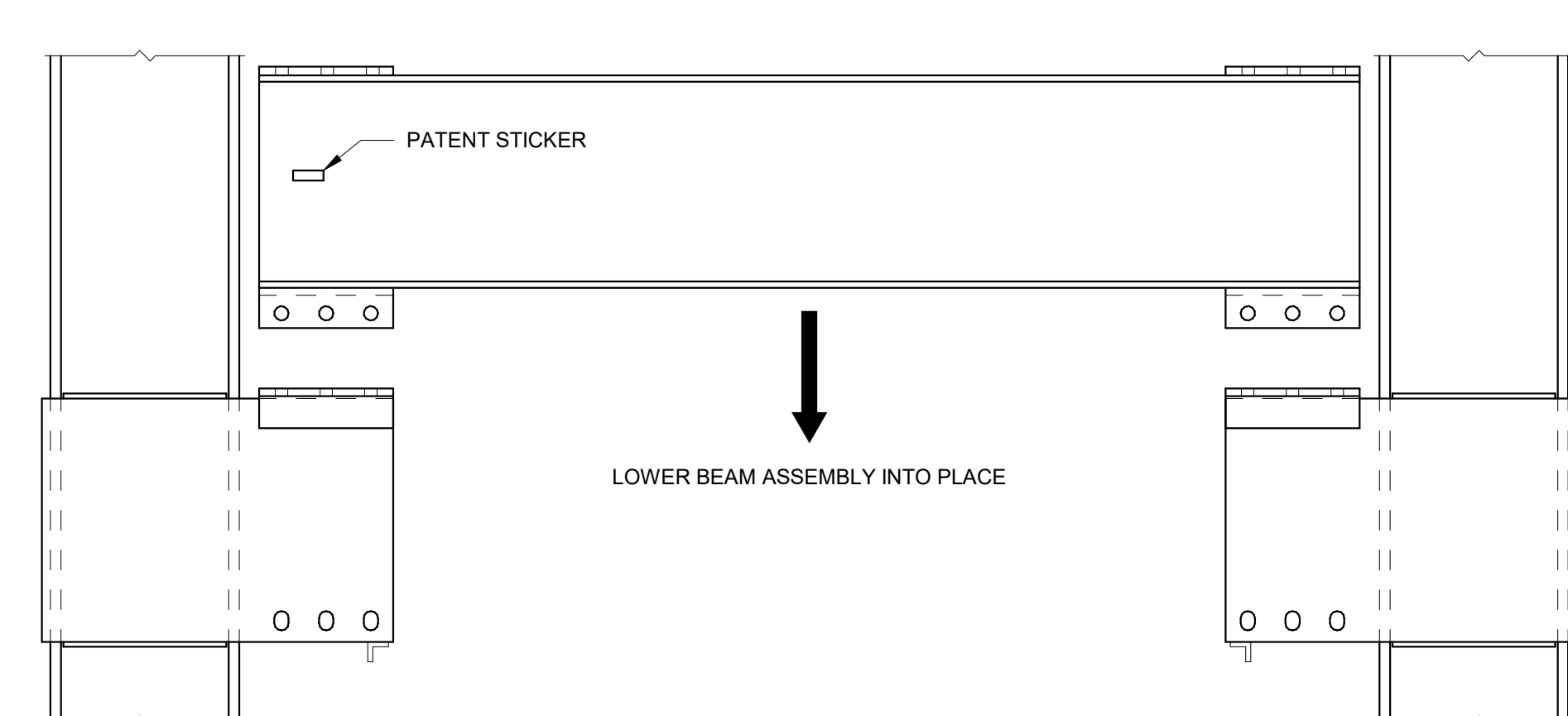
GRAPHIC NO. 4 - ANGLE (H) PLACEMENT

GRAPHIC NO. 5 - ROTATE BEAM ASSEMBLY TO FACILITATE WELD (6) MATERIAL AT MINIMAL EDGE DISTANCE

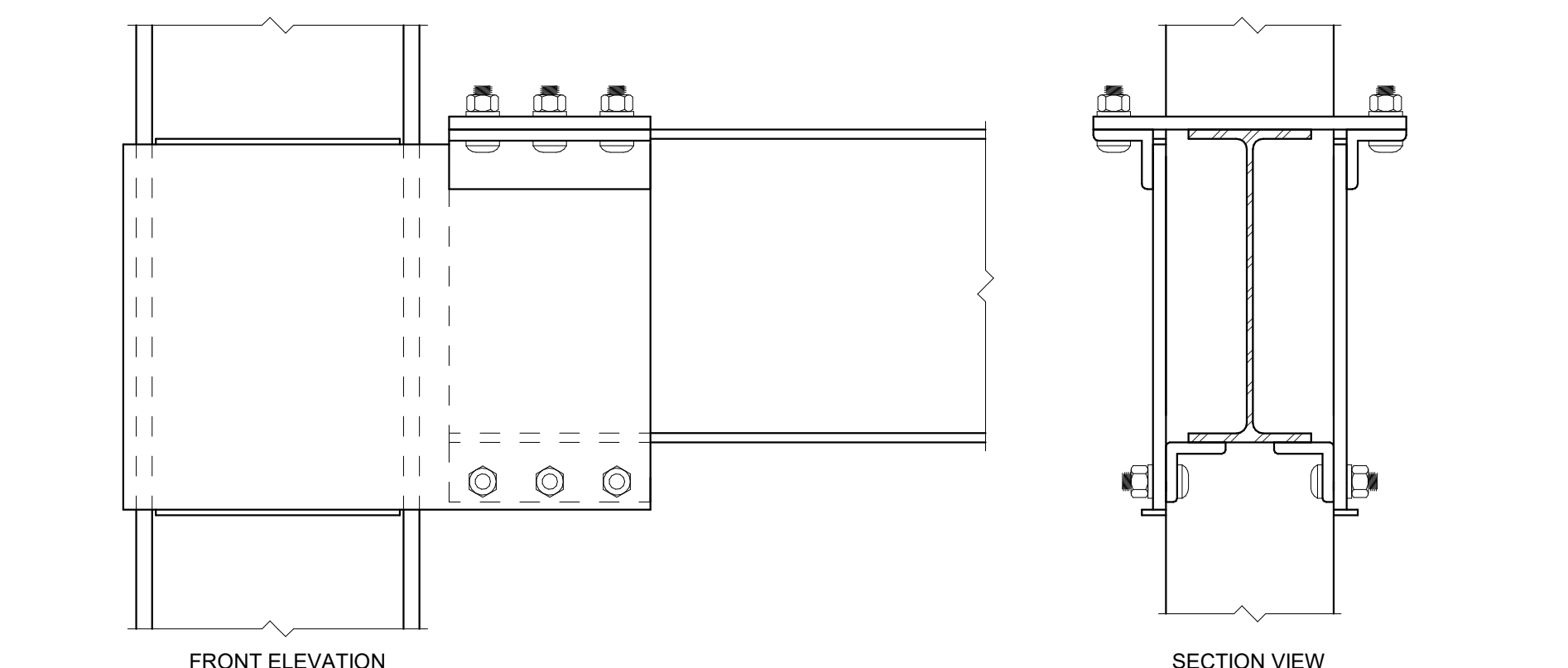
FIELD ERECTION OF SIDEPLATE BOLTED SYSTEM

- 3. WATCH OUR SIDEPLATE FIELD ERECTION VIDEO AT

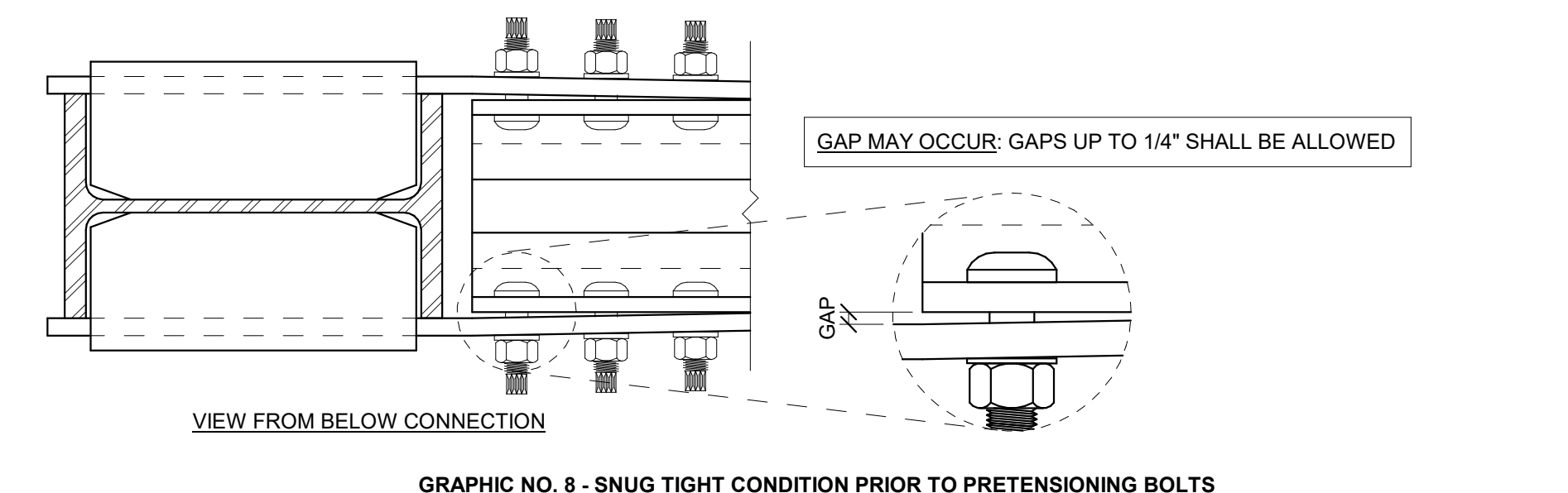
https://portal.sideplate.com/account/login



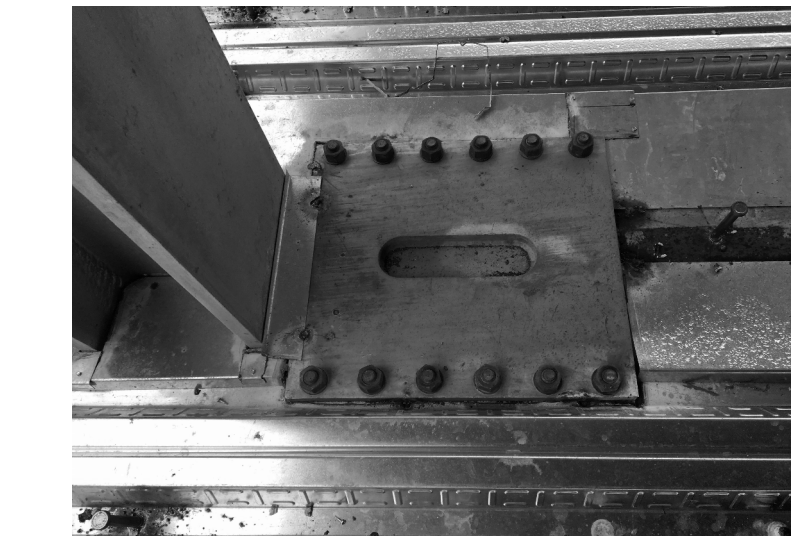
GRAPHIC NO. 6 - FIELD ERECTION OF SIDEPLATE BEAM ASSEMBLY



GRAPHIC NO. 7 - COMPLETED SIDEPLATE BOLTED CONNECTION



GRAPHIC NO. 8 - SNUG TIGHT CONDITION PRIOR TO PRETENSIONING BOLTS

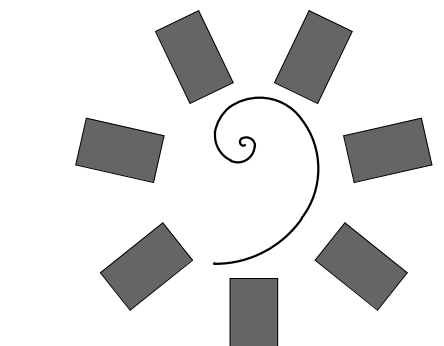


GRAPHIC NO. 9 - TYPICAL GAP CLOSURE AT THE TOP OF THE GAP

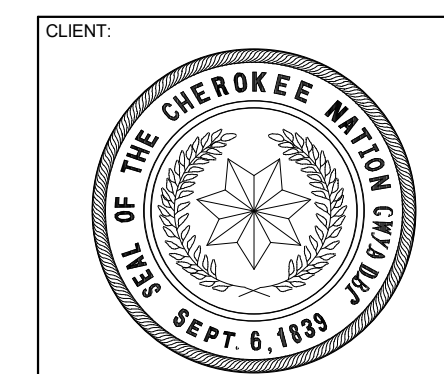
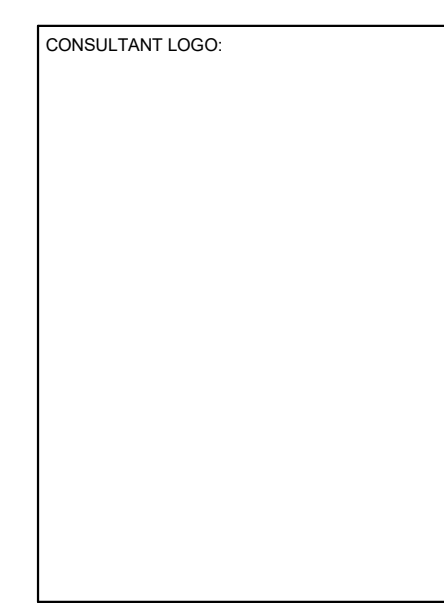
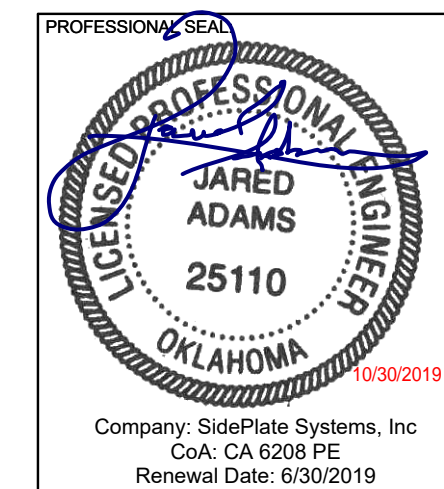


GRAPHIC NO. 10 - FIREPROOFING ACROSS THE BOTTOM OF THE GAP

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THE SIDEPLATE® steel frame connection system is covered by one or more of U.S. Pat. Nos. 6,138,427; 6,516,883; 6,589,157; 7,170,296; 6,122,077; 6,122,078; 6,122,079; 6,122,080; 6,122,081; 6,122,082; 6,122,083; 6,122,084; 6,122,085; and foreign applications pending.
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WILMA P. MANKILLER HEALTH CENTER EXPANSION
STILWELL, OKLAHOMA

Table with 2 columns: #, DATE, REVISIONS, DESCRIPTION. Includes PROJECT PHASE (BID PACKAGE 01) and SHEET NUMBER (S8.01).

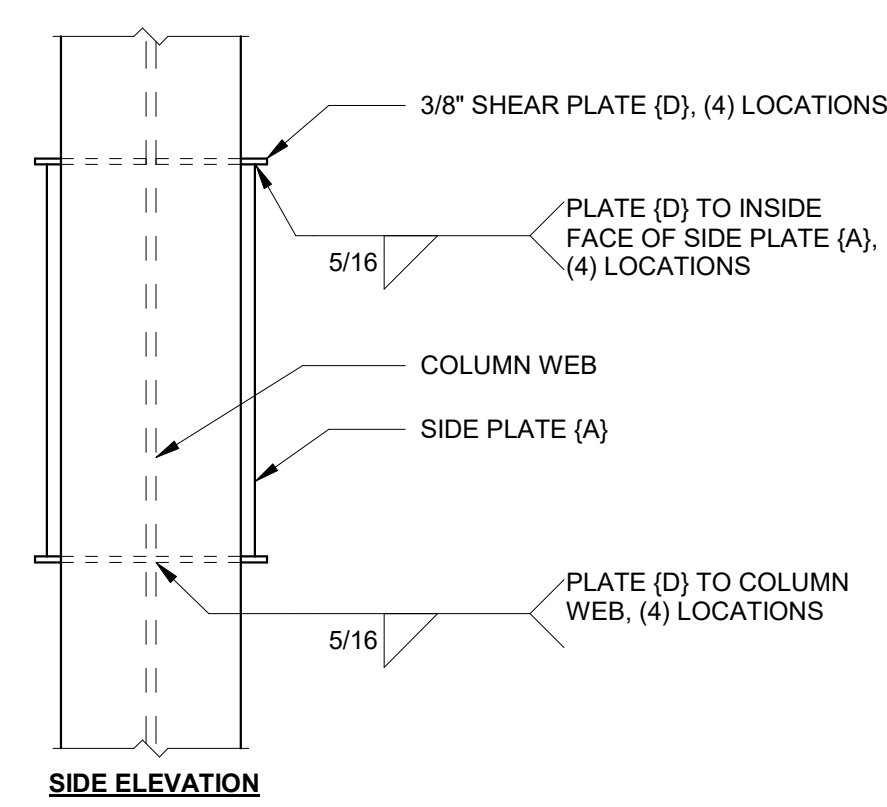
DATE: 11-01-19 JOB NUMBER: 18-01.01
SHEET NUMBER: S8.01
SIDEPLATE GENERAL NOTES AND CONSTRUCTION GUIDELINES

INTELLECTUAL PROPERTY RIGHTS NOTICE
 The SIDEPLATE® steel frame connection system is covered by one or more of U.S. Pat. Nos. 6,138,427; 6,516,583; 6,591,573; 7,178,296; 8,122,671; 8,122,672; 8,146,322; 8,176,706; 8,205,408; and 9,091,065 and foreign counterparts. Other U.S. and foreign applications pending.

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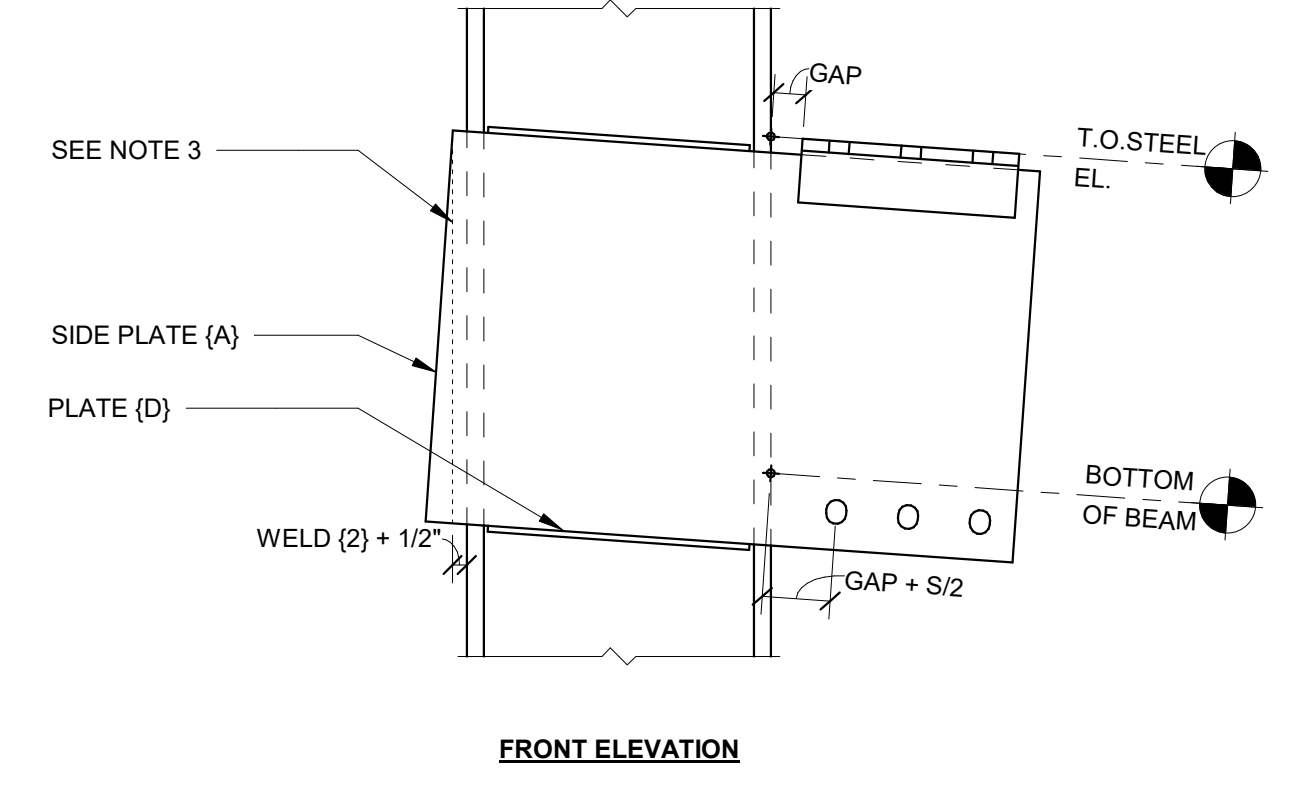
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18-01-01



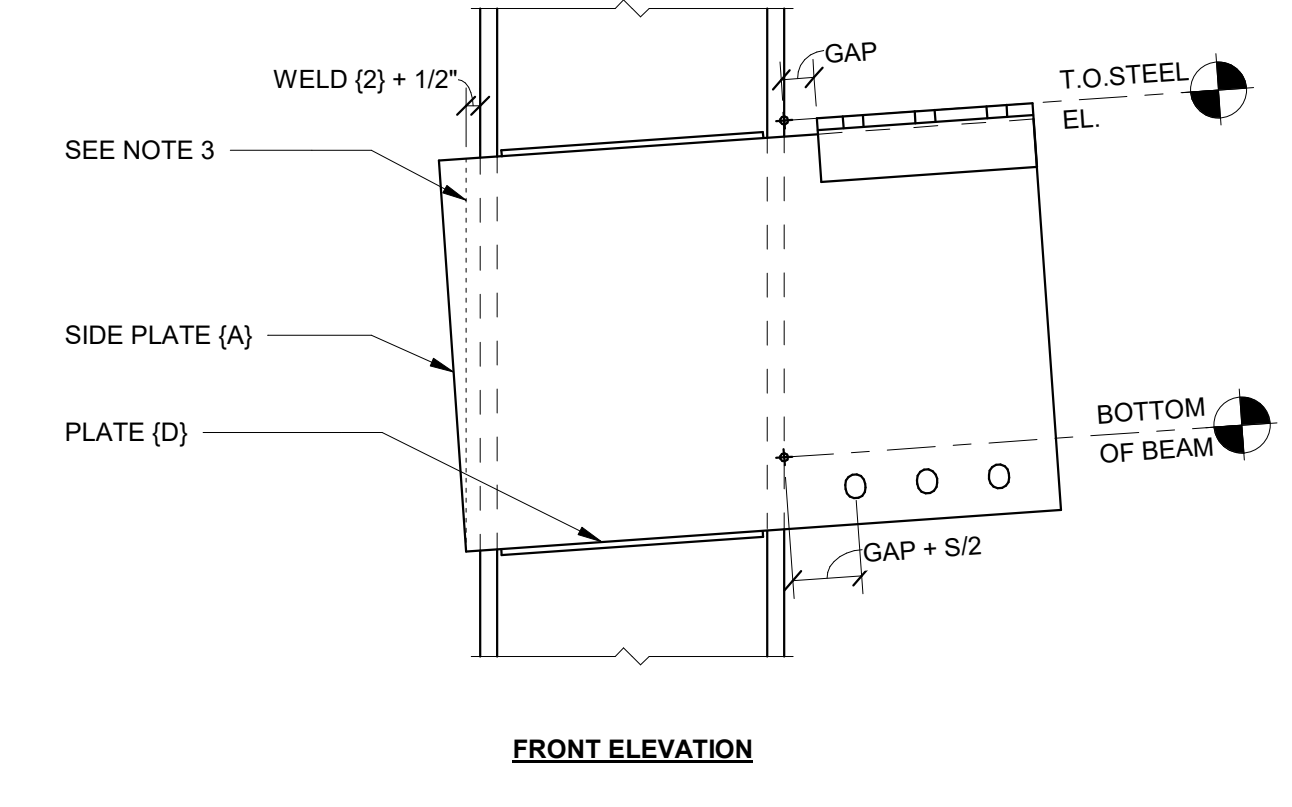
NOTE(S):
 1. LONGITUDINAL ANGLES (G) NOT SHOWN FOR CLARITY.

8 PLATE (D) DETAIL FOR SLOPED CONDITIONS
 N.T.S.



NOTE(S):
 1. FOR BEAM SLOPES > 1" PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC.
 2. COORDINATE PLATES, ANGLES, AND DIMENSIONS WITH RESPECT TO THE SLOPE OF THE CONNECTION.
 3. AT CONTRACTOR'S DISCRETION, SIDE PLATE (A) MAY BE CUT AS SHOWN.
 4. HORIZONTAL SHEAR PLATES (D) AND ASSOCIATED WELDS ARE REQUIRED FOR SLOPED SIDE PLATE CONDITIONS. SEE 8 / S8.02

4 SLOPED DOWN CONNECTION (AS APPLICABLE)
 N.T.S.

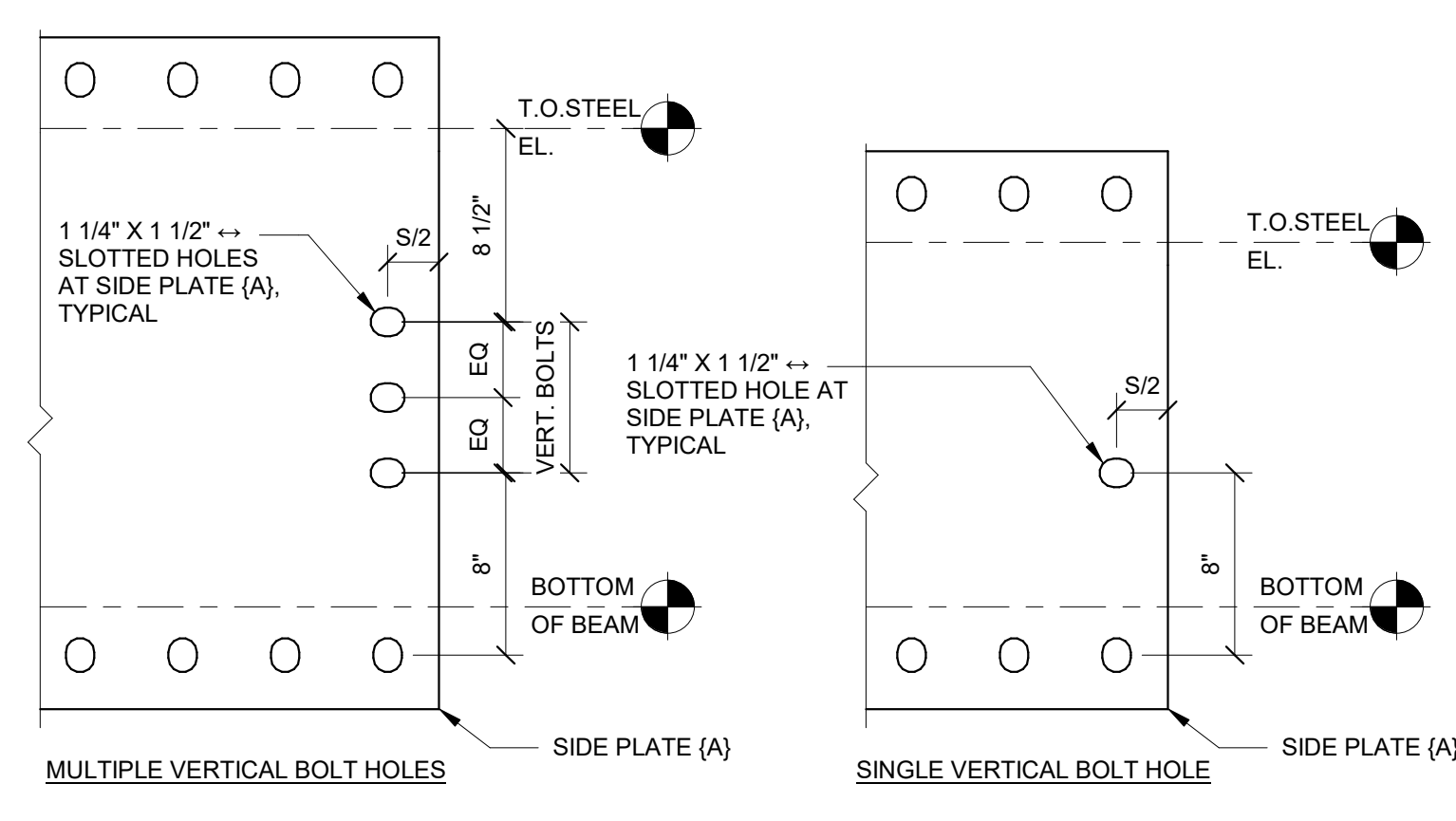


NOTE(S):
 1. FOR BEAM SLOPES > 1" PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC.
 2. COORDINATE PLATES, ANGLES, AND DIMENSIONS WITH RESPECT TO THE SLOPE OF THE CONNECTION.
 3. AT CONTRACTOR'S DISCRETION, SIDE PLATE (A) MAY BE CUT AS SHOWN.
 4. HORIZONTAL SHEAR PLATES (D) AND ASSOCIATED WELDS ARE REQUIRED FOR SLOPED SIDE PLATE CONDITIONS. SEE 8 / S8.02

3 SLOPED UP CONNECTION (AS APPLICABLE)
 N.T.S.

ID	COLUMN PANEL ZONE DESIGN (INCHES)				SIDE PLATE (A) EXTENSION DESIGN (INCHES)									
	SERIES	WELD SIZE	BEAM SHAPE	GAP	PLATE (A)			BOLT						
					THICKNESS	B	E	Y	DIAMETER	HORIZONTAL #	VERTICAL #	G	S	
A15	W14x	3/8	W24X68	2	5/8	31 3/4	1 3/8	2 1/2		1 1/8	4	2	2 1/8	4 1/2
A25	W14x	3/8	W24X94	2	5/8	32 1/4	1 3/8	3 5/8		1 1/8	5	2	2 1/8	4 1/2
A45	W14x	3/8	W36X160	2	5/8	44	1 3/8	5		1 1/8	6	3	2 1/8	4 1/2

6 A TYPE NARROW COLUMN CONNECTION SCHEDULE
 N.T.S.

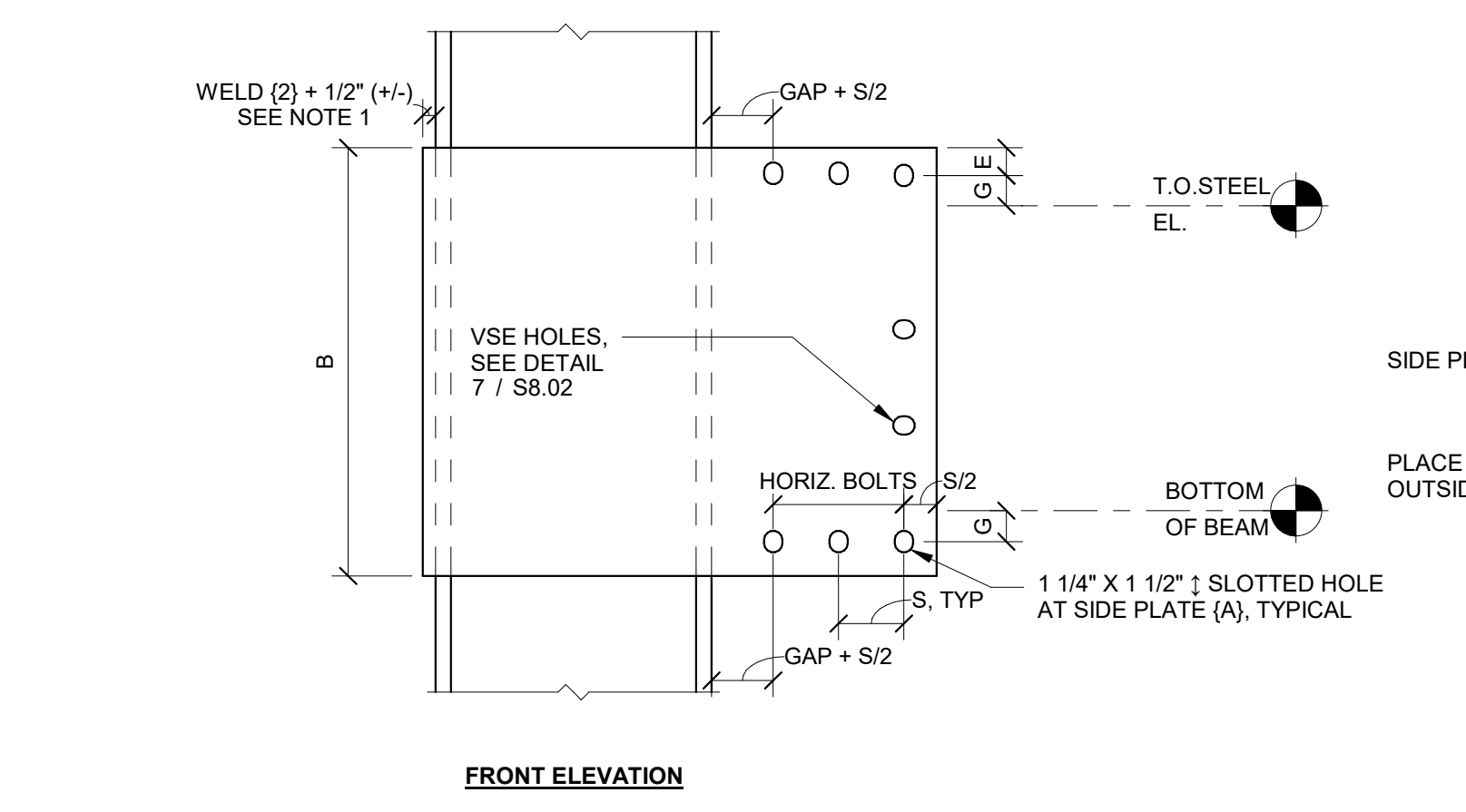
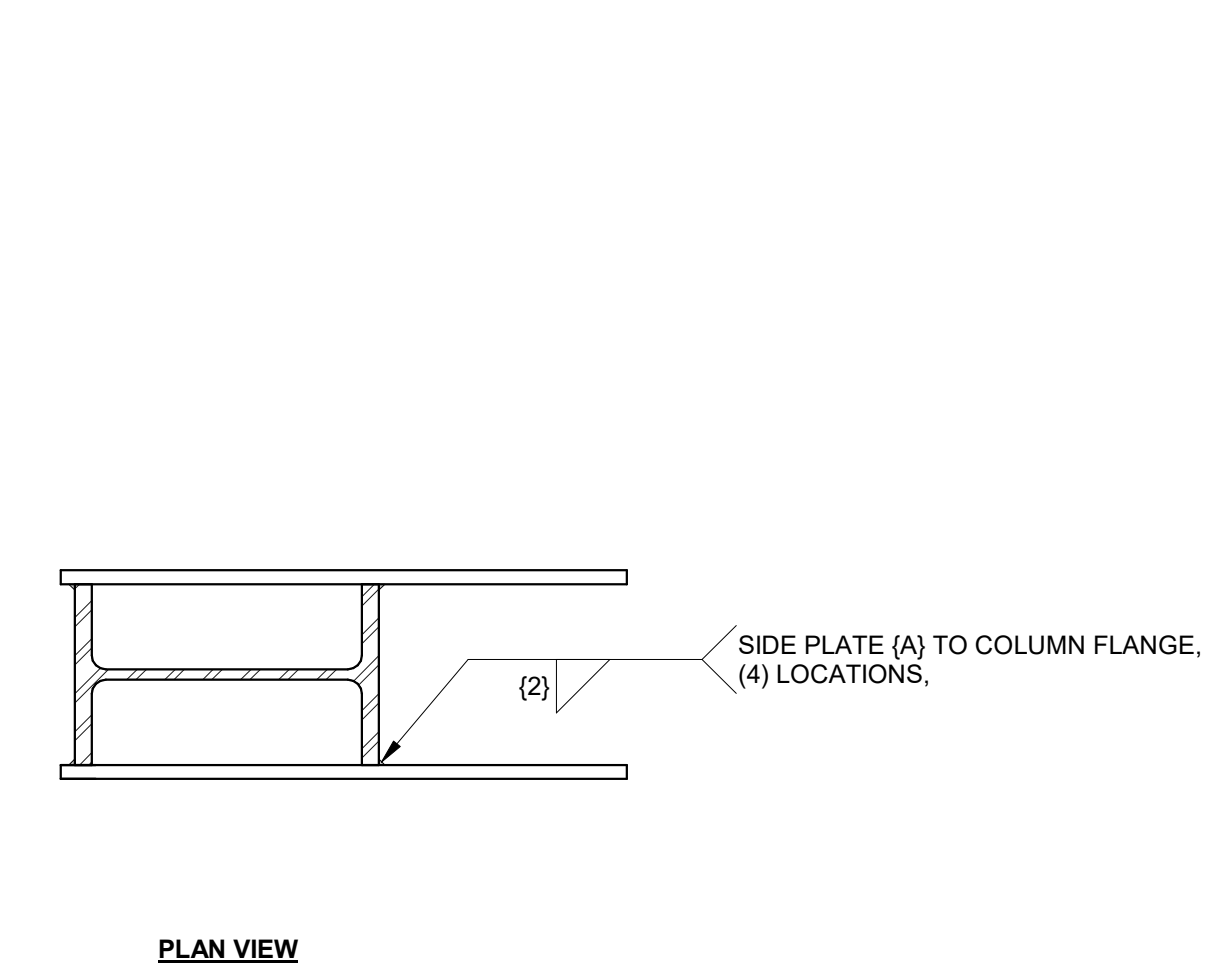


NOTE(S):
 1. SEE COLUMN SCHEDULE FOR BOLT QUANTITY.

7 SIDE PLATE (A) VSE BOLT HOLE DETAIL
 N.T.S.

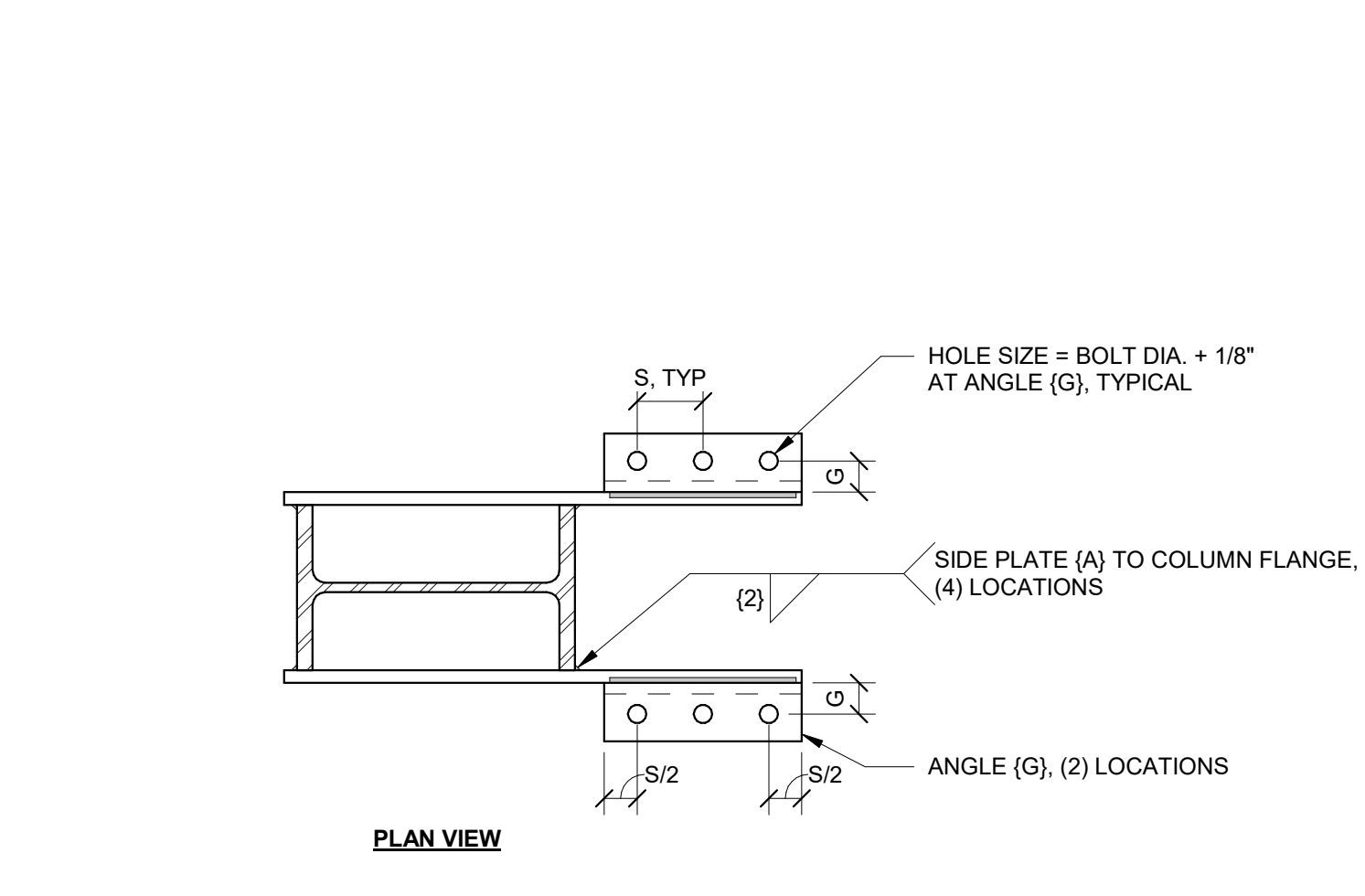
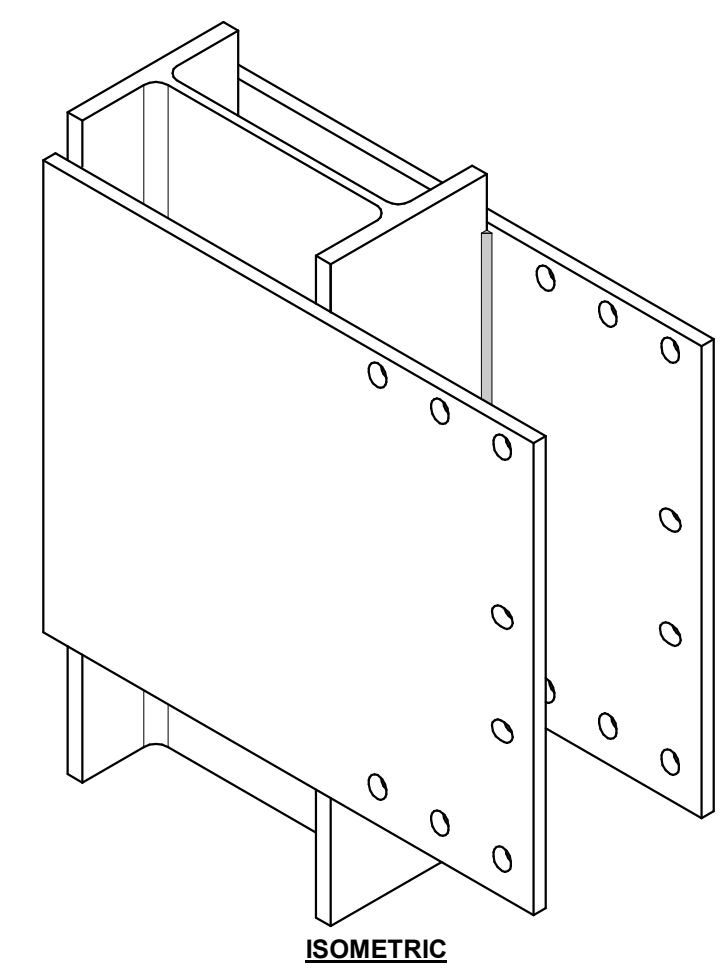
ID	COLUMN PANEL ZONE DESIGN (INCHES)				SIDE PLATE (A) EXTENSION DESIGN (INCHES)										
	SERIES	WELD SIZE	BEAM SHAPE	GAP	PLATE (A)			ANGLE (G)		WELD SIZE	BOLT				
					THICKNESS	B	Y	SUGGESTED SIZE	HORIZONTAL LEG		VERTICAL LEG	DIAMETER	HORIZONTAL #	G	S
A10, A11, A19	W14x	3/8	W24X68	2	5/8	27 1/4	2 1/2	L5X3-1/2X5/8	3-1/2 to 6	4 to 6	5/16	1 1/8	4	2 1/8	4 1/2
A12	W14x	7/16	W24X68	2 1/4	1	27 1/4	1 7/8	L5X3-1/2X5/8	3-1/2 to 6	4 to 6	5/16	1 1/8	4	2 1/8	4 1/2
A20	W14x	3/8	W24X94	2	7/8	27 3/4	2 7/8	L5X3-1/2X5/8	3-1/2 to 6	4 to 6	5/16	1 1/8	5	2 1/8	4 1/2
A30	W14x	3/8	W36X150	2	5/8	39 3/8	5	L5X3-1/2X5/8	3-1/2 to 6	4 to 6	5/16	1 1/8	6	2 1/8	4 1/2

2 A TYPE COLUMN CONNECTION SCHEDULE
 N.T.S.



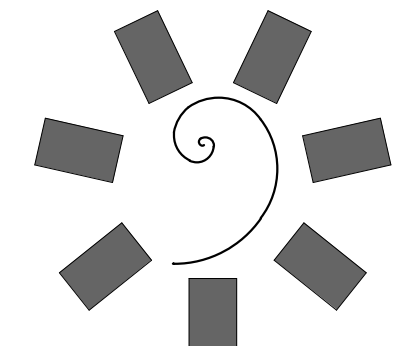
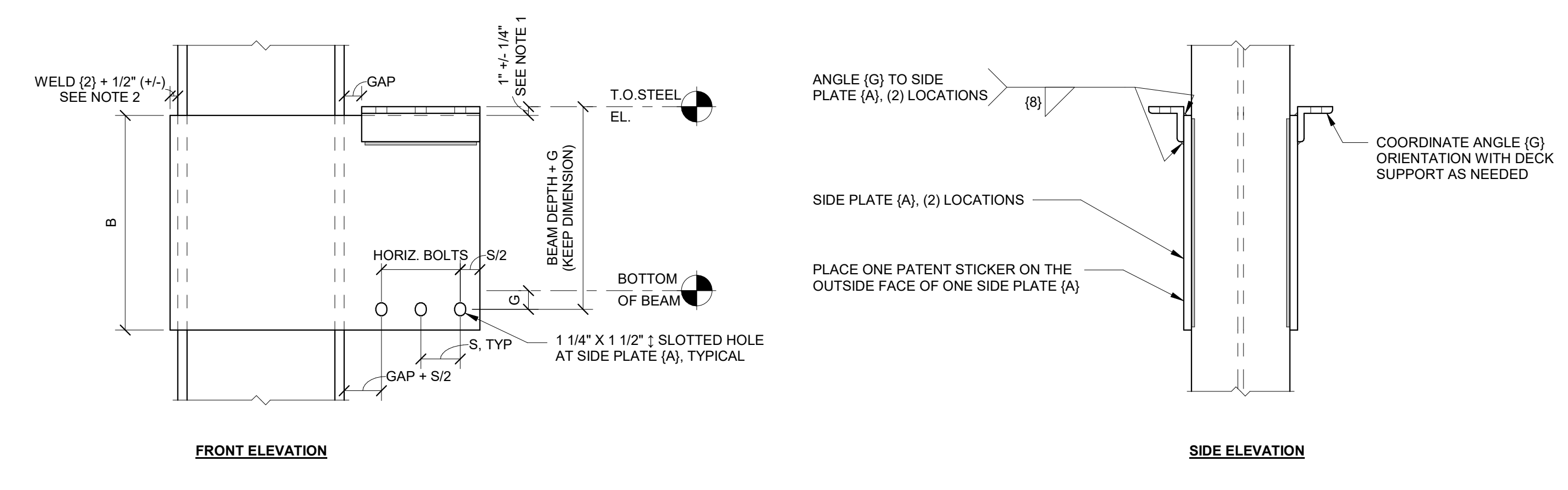
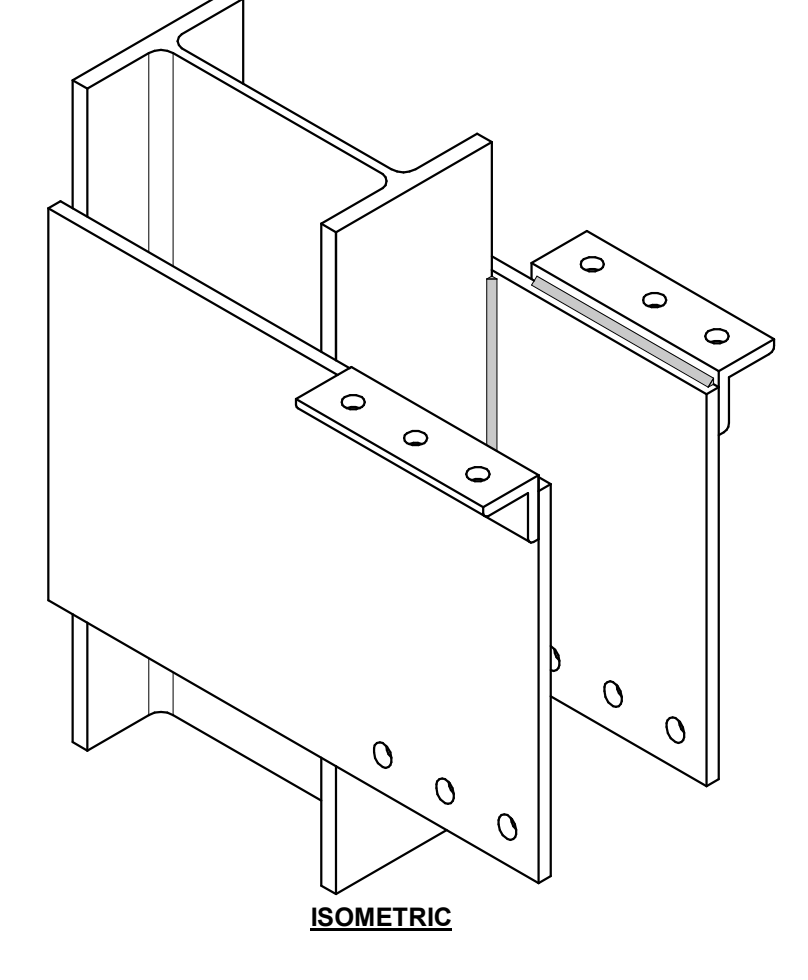
NOTE(S):
 1. THE 1/2 INCH OVERHANG ON THE SIDE PLATE (A) IS TO ENSURE SUFFICIENT ROOM FOR WELD (2). THE +/- TOLERANCE IS APPLIED SO THAT IF DESIRED, THE DETAILER CAN MAKE THE SIDE PLATES (A) THE SAME LENGTH WITH SLIGHTLY VARYING COLUMN DEPTHS WITHIN A GROUP OF THE SAME CONNECTION ID'S.

5 A TYPE NARROW BOLTED CONNECTION
 N.T.S.

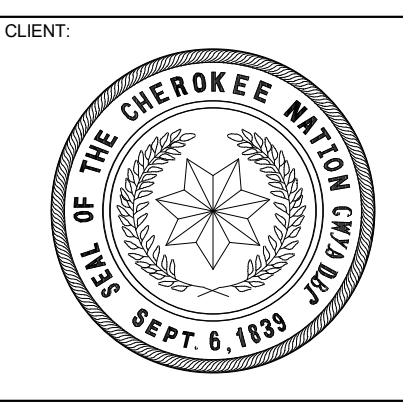


NOTE(S):
 1. THE +/- 1/4 INCH TOLERANCE FOR PLACEMENT OF ANGLES (G) IS TO ENSURE CORRECT TOP OF STEEL PLACEMENT RELATIVE TO THE CENTERLINE OF THE BOTTOM HORIZONTAL ROW OF BOLT HOLES. THE PLACEMENT OF ANGLES (G) SHALL NEVER BE MEASURED FROM THE BOTTOM EDGE OF SIDE PLATE (A) TO ESTABLISH THE CORRECT TOP OF STEEL.
 2. THE 1/2 INCH OVERHANG ON THE SIDE PLATE (A) IS TO ENSURE SUFFICIENT ROOM FOR WELD (2). THE +/- TOLERANCE IS APPLIED SO THAT IF DESIRED, THE DETAILER CAN MAKE THE SIDE PLATES (A) THE SAME LENGTH WITH SLIGHTLY VARYING COLUMN DEPTHS WITHIN A GROUP OF THE SAME CONNECTION ID'S.

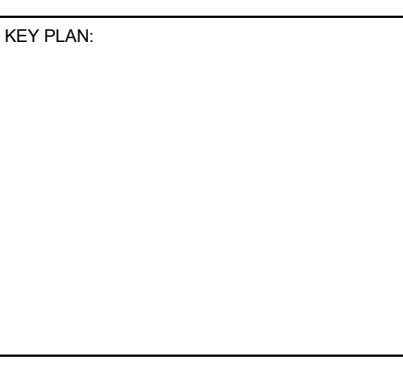
4 A TYPE BOLTED CONNECTION
 N.T.S.



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**WILMA P. MANKILLER HEALTH CENTER
 EXPANSION**
 STILWELL, OKLAHOMA



PROJECT PHASE:
 BID PACKAGE 01

#	DATE	REVISIONS DESCRIPTION

DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER: S8.02

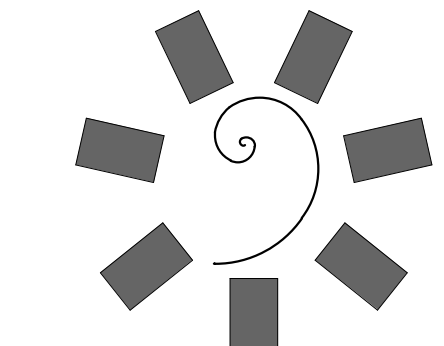
SIDEPLATE COLUMN DETAILS, A TYPE

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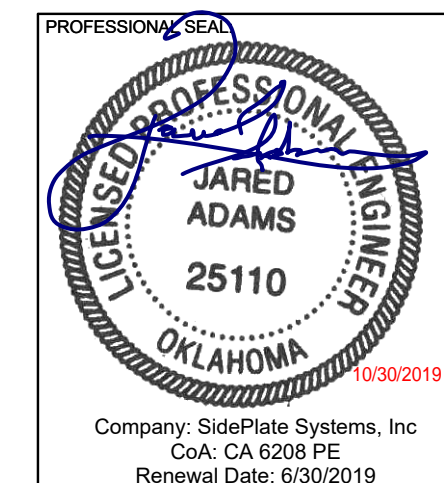
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S8.03

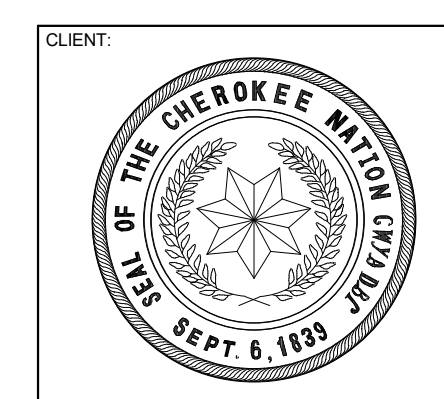


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CONSULTANT LOGO



WILMA P. MANKILLER HEALTH CENTER
 EXPANSION
 STILWELL, OKLAHOMA

KEY PLAN

PROJECT PHASE

BID PACKAGE 01

#	DATE	REVISIONS	DESCRIPTION

DATE: 11-01-19 JOB NUMBER: 18-01.01

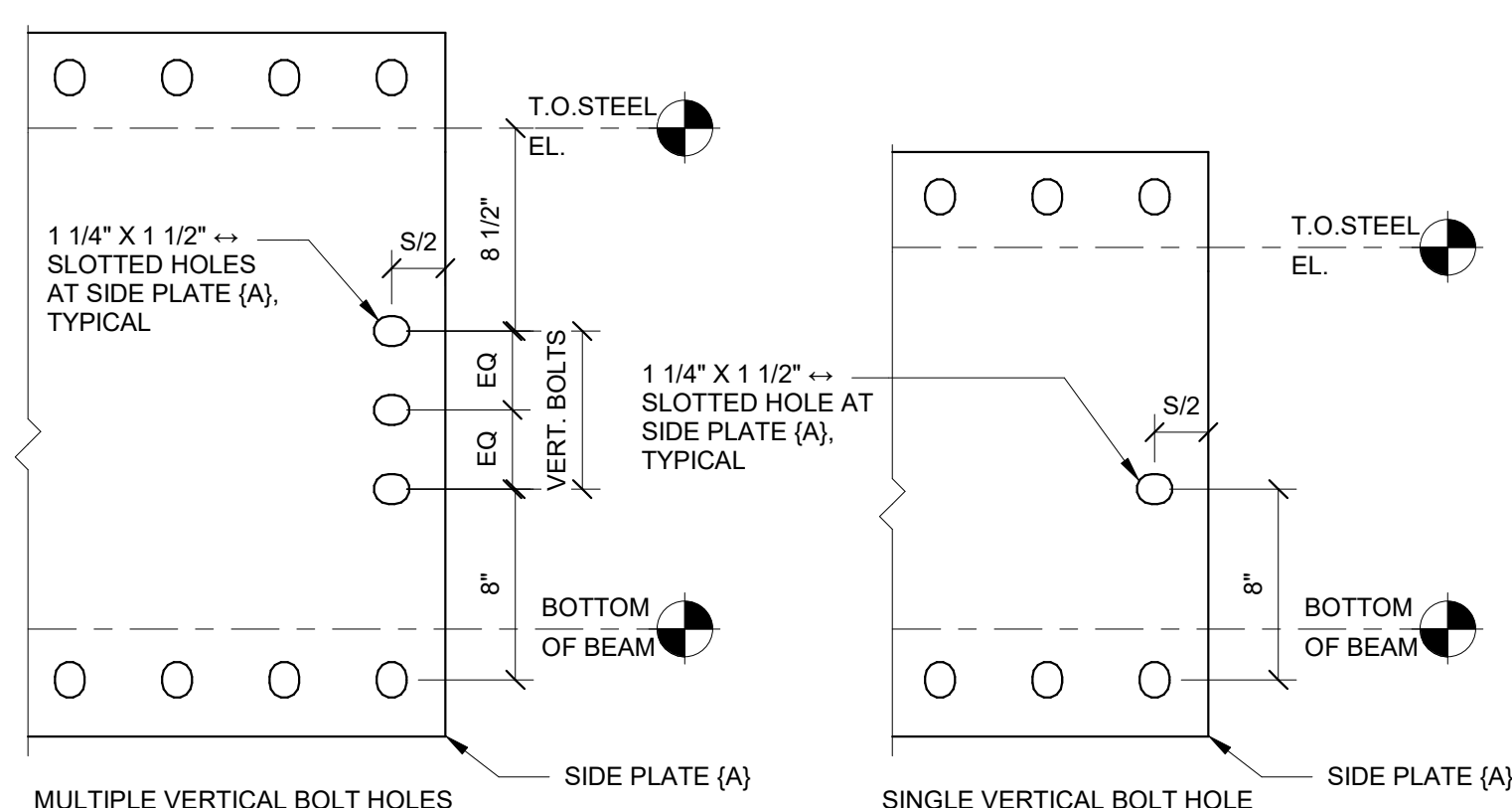
SHEET NUMBER:

S8.03

SIDEPLATE COLUMN
 DETAILS, B TYPE

ID	COLUMN PANEL ZONE DESIGN (INCHES)				SIDE PLATE (A) EXTENSION DESIGN (INCHES)								
	COLUMN	WELD	BEAM	PLATE (A)	BOLT			G	S				
	SERIES	SIZE	SHAPE		GAP	THICKNESS	HORIZONTAL #			VERTICAL #			
B15	W14x	3/8	W24X68	2	5/8	31 3/4	1 3/8	2 1/2	1 1/8	4	2	2 1/8	4 1/2
B25	W14x	1/2	W24X94	2	5/8	32 1/4	1 3/8	3 5/8	1 1/8	5	2	2 1/8	4 1/2
B45	W14x	1/2	W36X160	2	5/8	44	1 3/8	5	1 1/8	6	3	2 1/8	4 1/2

6 B TYPE NARROW COLUMN CONNECTION SCHEDULE
 N.T.S.

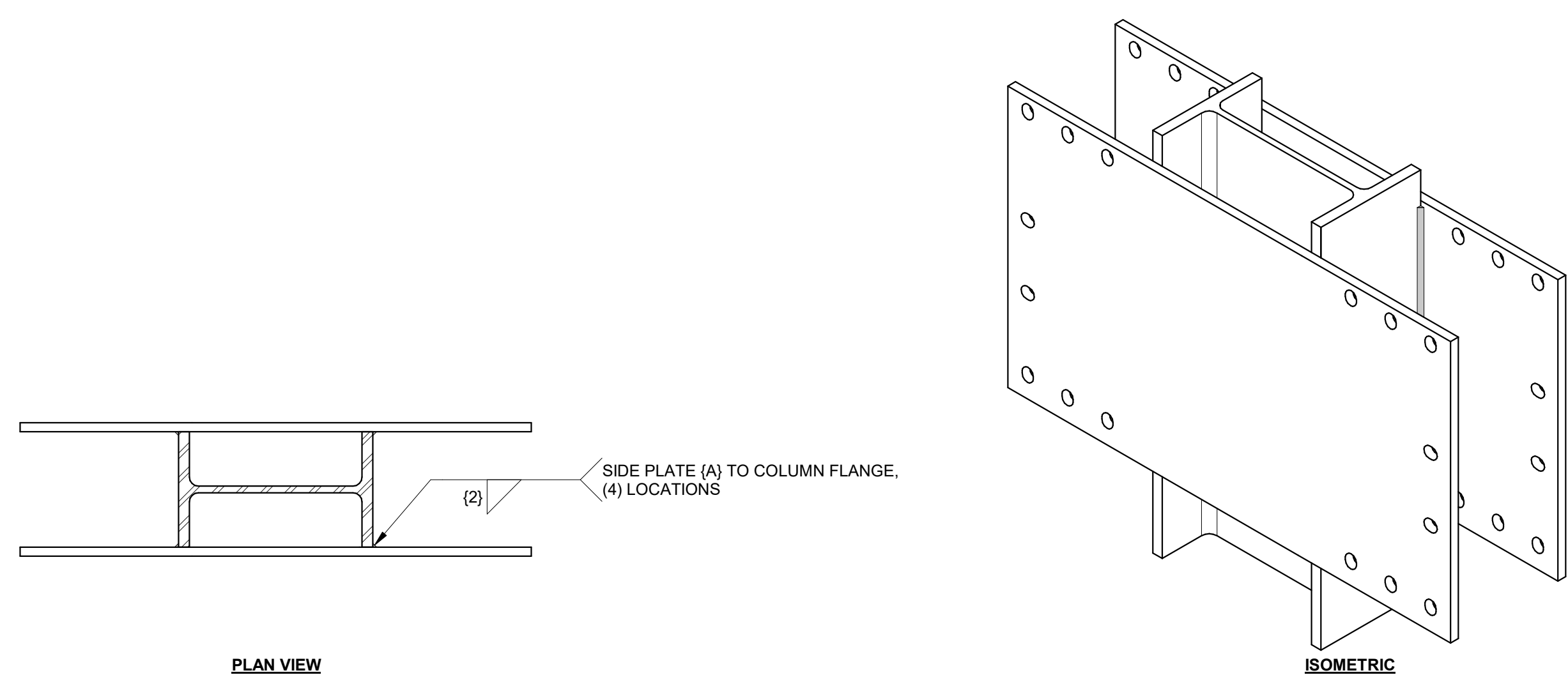


NOTE(S):
 1. SEE COLUMN SCHEDULE FOR BOLT QUANTITY.

7 SIDE PLATE (A) VSE BOLT HOLE DETAIL
 N.T.S.

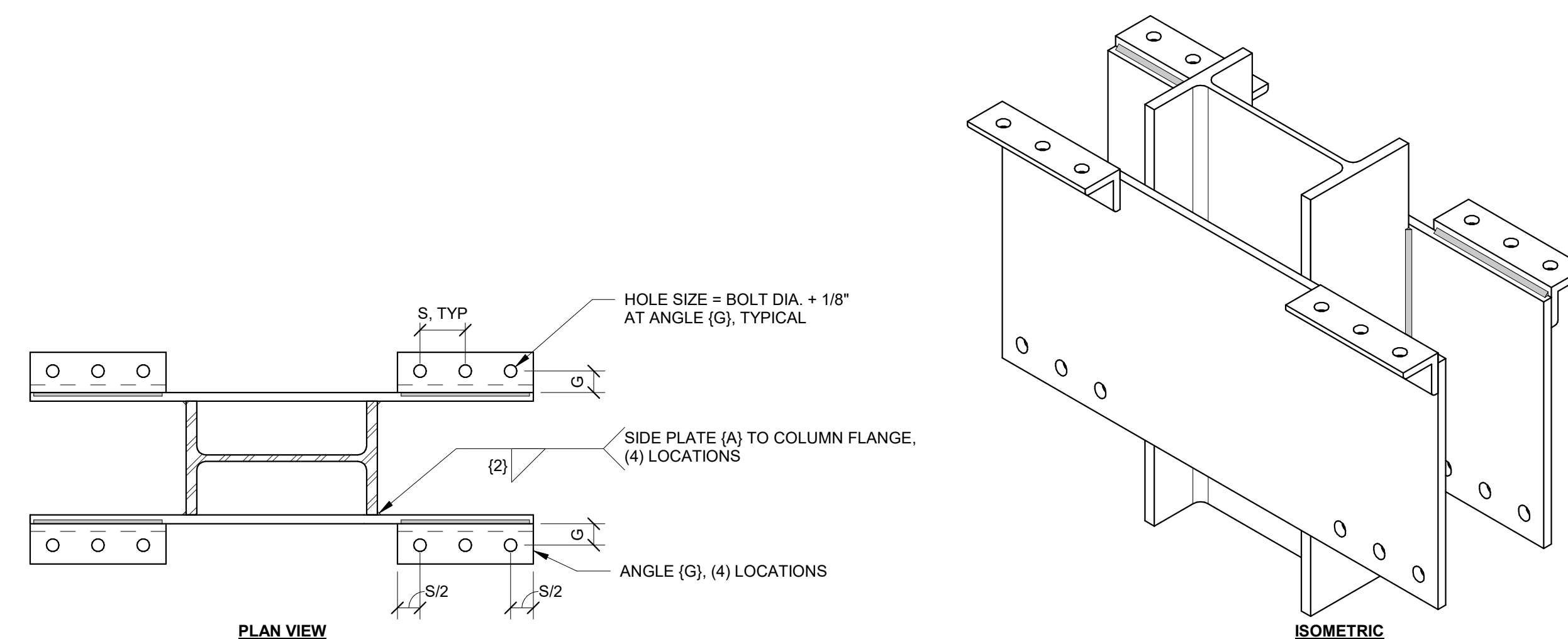
ID	COLUMN PANEL ZONE DESIGN (INCHES)				SIDE PLATE (A) EXTENSION DESIGN (INCHES)										
	COLUMN	WELD	BEAM	PLATE (A)	ANGLE			WELD (8)	BOLT						
	SERIES	SIZE	SHAPE		GAP	THICKNESS	B		Y	SUGGESTED SIZE	HORIZONTAL LEG	VERTICAL LEG	SIZE	DIAMETER	HORIZONTAL #
B11	W14x	3/8	W24X68	2	5/8	27 1/4	2 1/2	L5X3-1/2X5/8	3-1/2 to 6	4 to 6	5/16	1 1/8	4	2 1/8	4 1/2
B12	W14x	7/16	W24X68	2 1/4	1	27 1/4	1 7/8	L5X3-1/2X5/8	3-1/2 to 6	4 to 6	5/16	1 1/8	4	2 1/8	4 1/2
B19	W14x	1/2	W24X68	2	5/8	27 1/4	2 1/2	L5X3-1/2X5/8	3-1/2 to 6	4 to 6	5/16	1 1/8	4	2 1/8	4 1/2
B20	W14x	3/4	W24X94	2	7/8	27 3/4	2 7/8	L5X3-1/2X5/8	3-1/2 to 6	4 to 6	5/16	1 1/8	5	2 1/8	4 1/2
B30	W14x	1/2	W36X150	2	5/8	39 3/8	5	L5X3-1/2X5/8	3-1/2 to 6	4 to 6	5/16	1 1/8	6	2 1/8	4 1/2

2 B TYPE COLUMN CONNECTION SCHEDULE
 N.T.S.



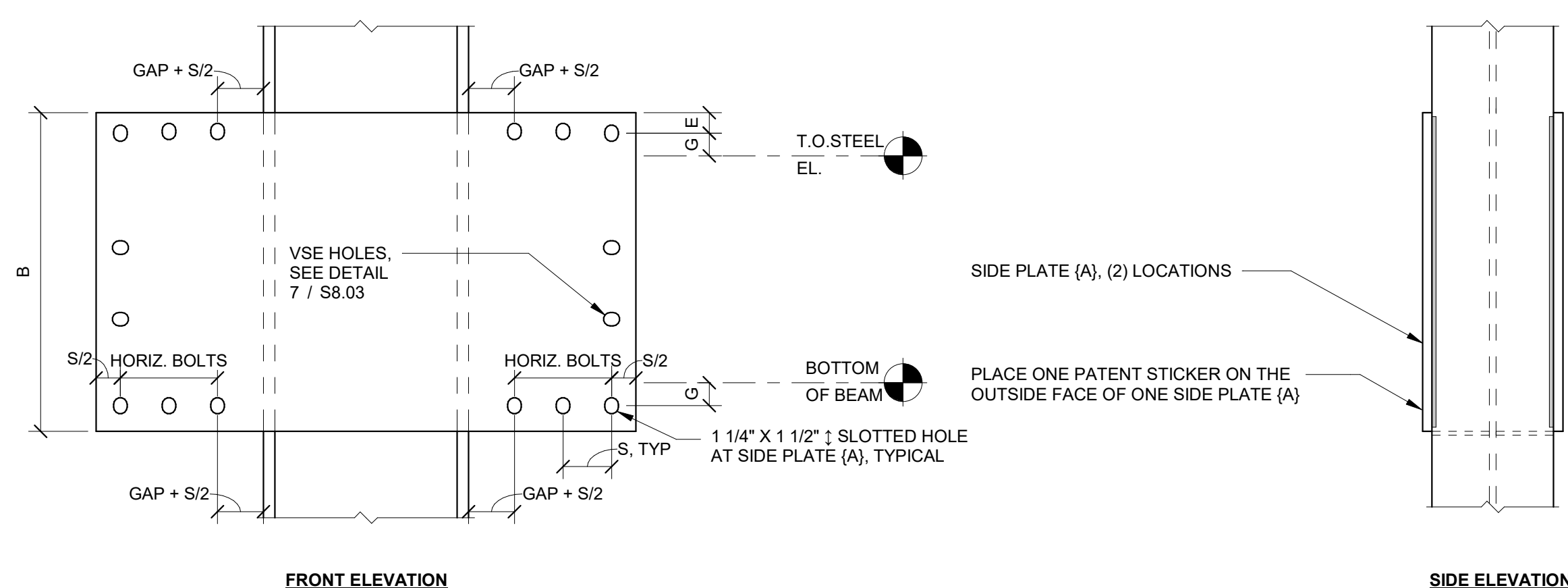
PLAN VIEW

ISOMETRIC



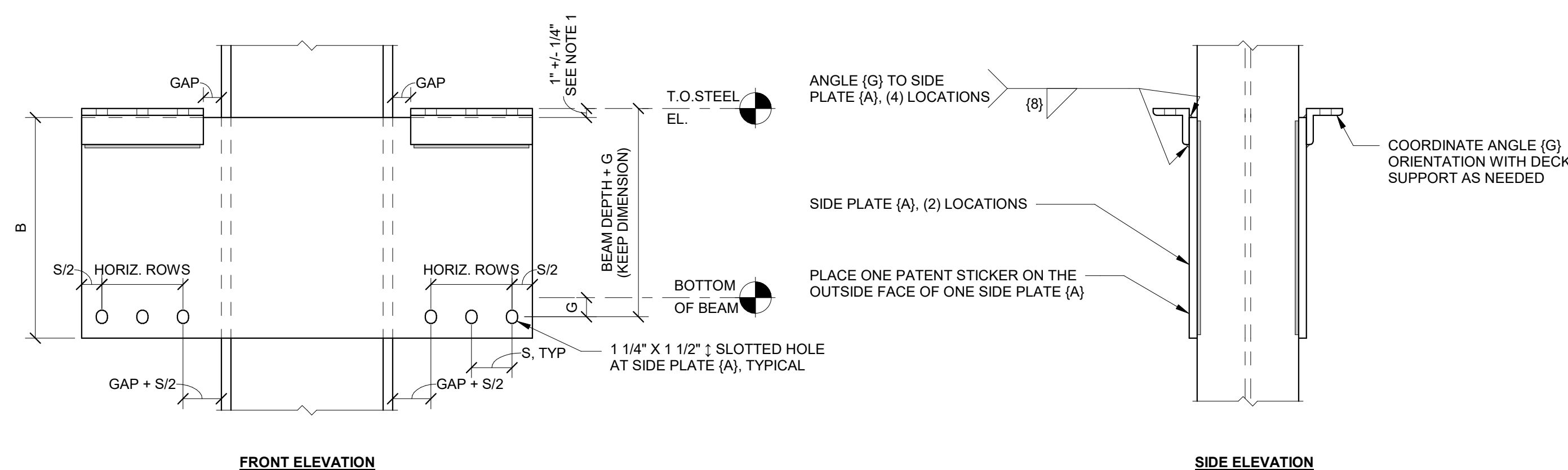
PLAN VIEW

ISOMETRIC



FRONT ELEVATION

SIDE ELEVATION



FRONT ELEVATION

SIDE ELEVATION

NOTE(S):
 T. THE +/- .14 INCH TOLERANCE FOR PLACEMENT OF ANGLES (G) IS TO ENSURE CORRECT TOP OF STEEL PLACEMENT RELATIVE TO THE CENTERLINE OF THE BOTTOM HORIZONTAL ROW OF BOLT HOLES. THE PLACEMENT OF ANGLES (G) SHALL NEVER BE MEASURED FROM THE BOTTOM EDGE OF SIDE PLATE (A) TO ESTABLISH THE CORRECT TOP OF STEEL.

5 B TYPE NARROW BOLTED CONNECTION
 N.T.S.

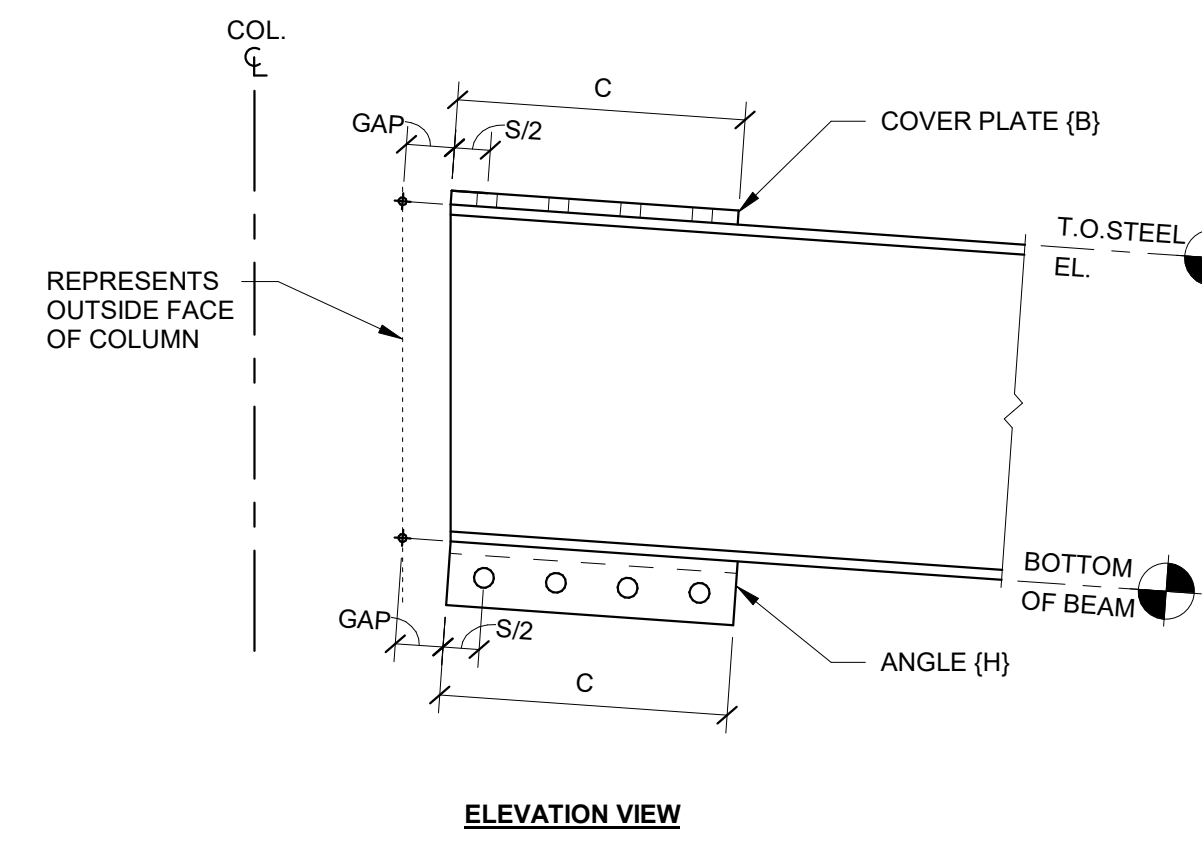
1 B TYPE BOLTED CONNECTION
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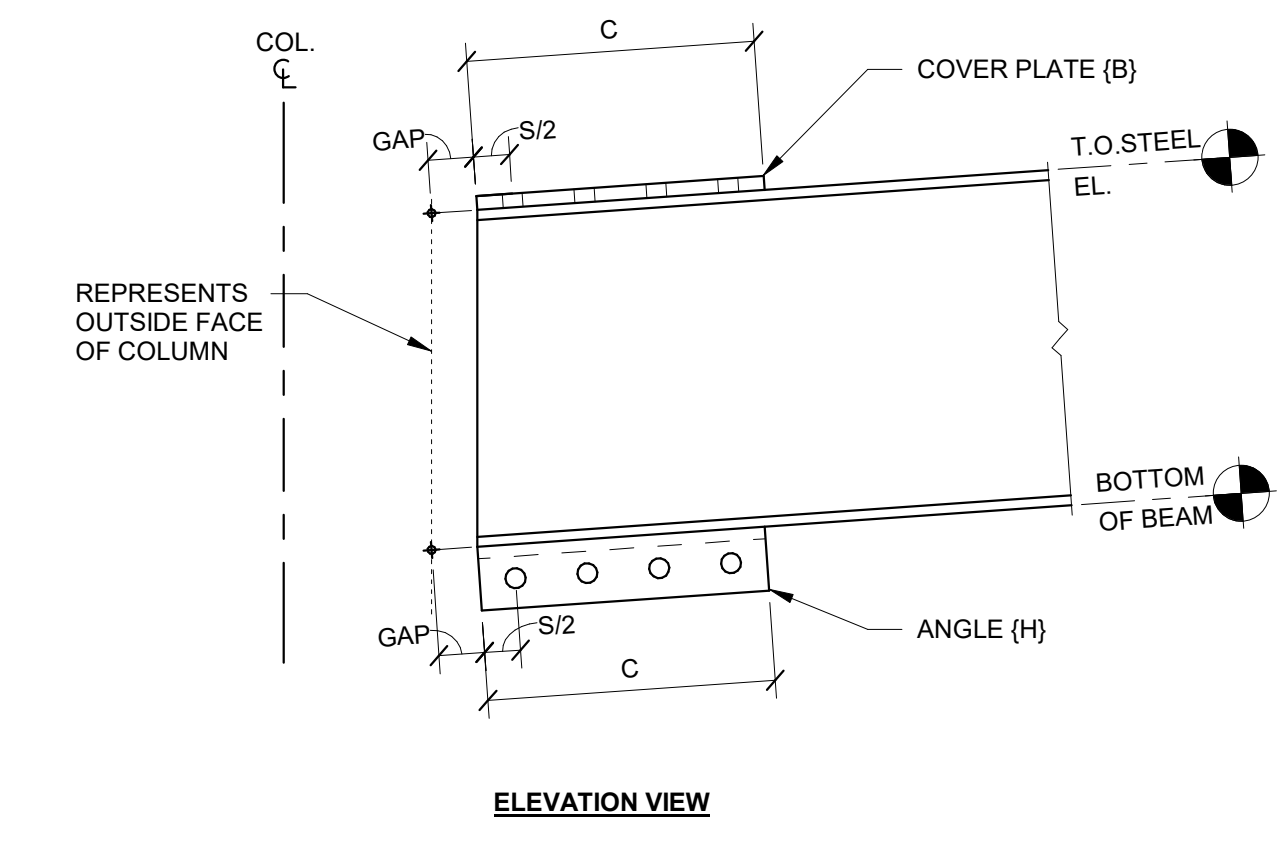
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vis.06.02 3/19/2019



NOTE(S):
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4 SLOPED DOWN BEAM END (AS APPLICABLE)
 N.T.S.

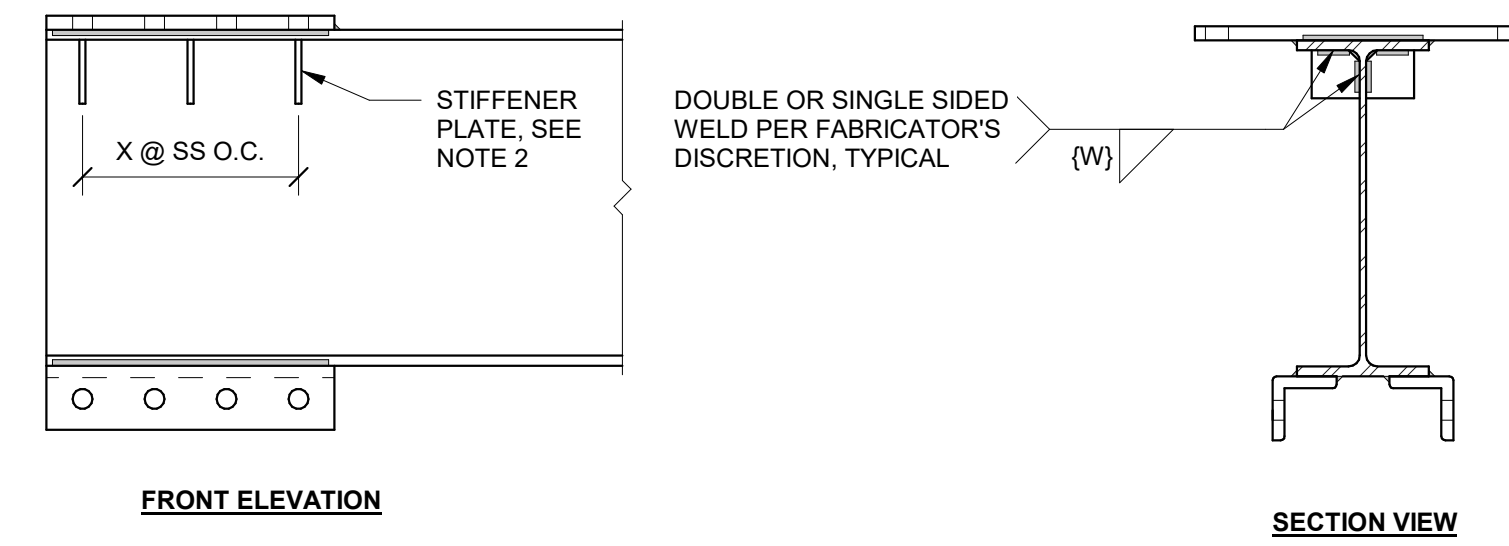


NOTE(S):
 1. FOR BEAM SLOPES > 1" PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC.

3 SLOPED UP BEAM END (AS APPLICABLE)
 N.T.S.

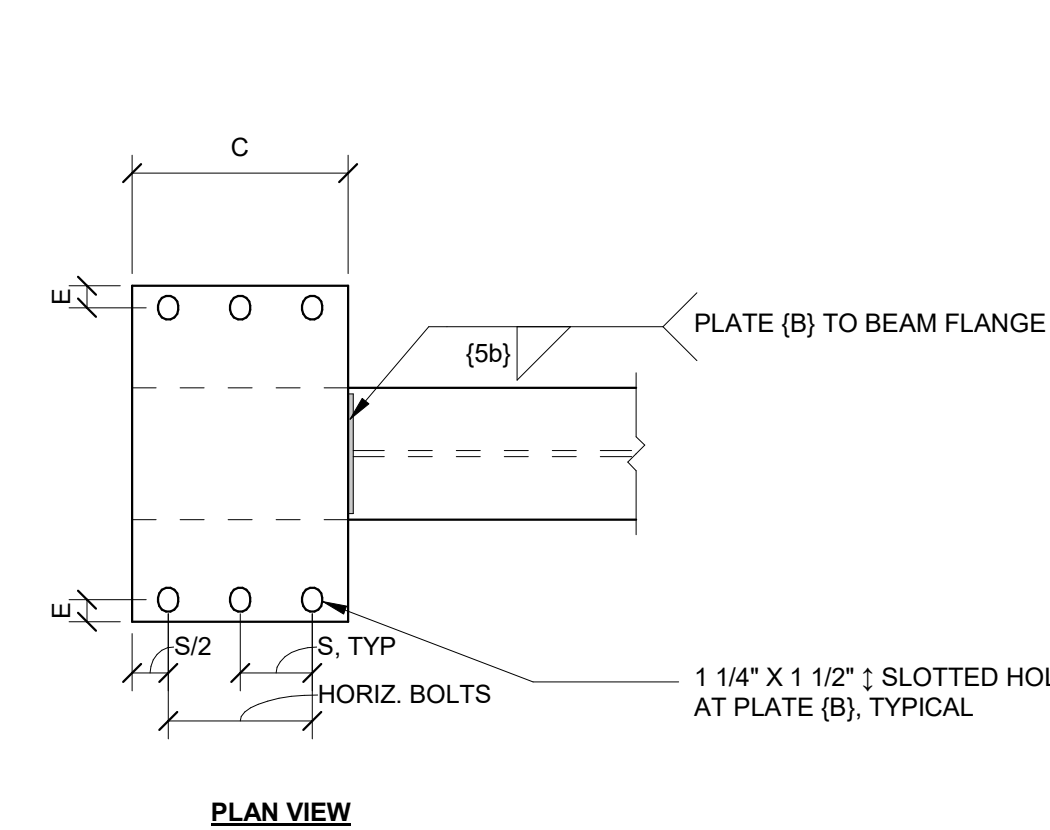
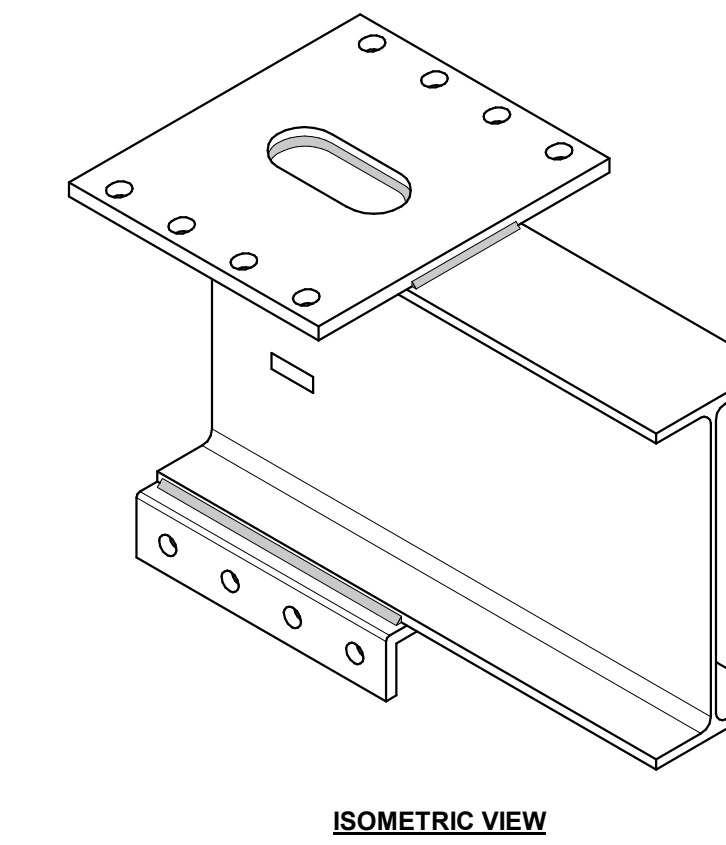
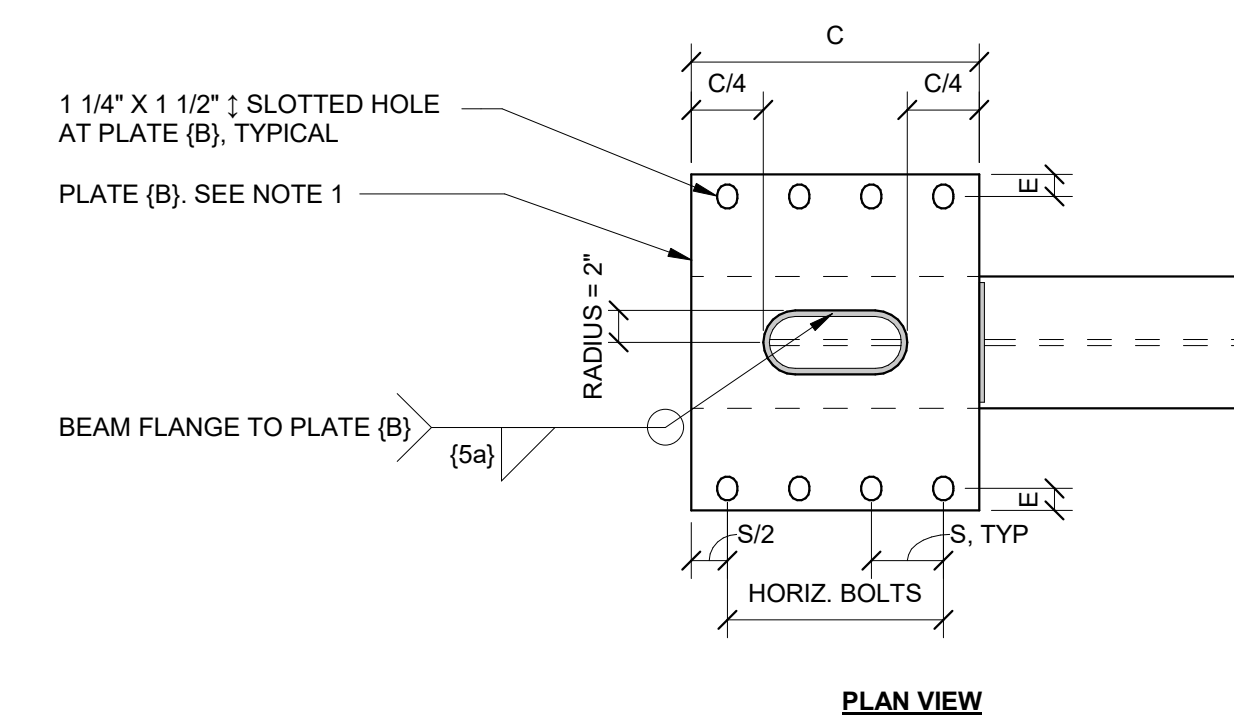
ID	BEAM	BEAM DESIGN (INCHES)																					
		SHAPE	GAP	PLATE				ANGLE				WELD				BOLT							
				COVER PLATE TYPE	THICKNESS	E	H	THICKNESS	LENGTH X WIDTH	X	SS	SUGGESTED SIZE	C	HORIZONTAL LEG	VERTICAL LEG	SIZE	SIZE	SIZE	SIZE (SINGLE)	SIZE (DOUBLE)	DIAMETER	HORIZONTAL #	G
A10	W24X68	2	Slotted	1 1/8	1 3/8	8 1/4	-	-	-	-	L6X4X5/8	18	6	4	5/16	5/16	5/16	-	-	1 1/8	4	2 1/8	4 1/2
A11	W24X68	2	Slotted	7/8	1 3/8	8 1/4	-	-	-	L7X4X5/8	18	7	4	5/16	5/16	5/16	-	-	1 1/8	4	2 1/8	4 1/2	
A12, B12	W24X68	2 1/4	Slotted	3/4	1 3/8	9	-	-	-	L7X4X5/8	18	7	4	5/16	5/16	5/16	-	-	1 1/8	4	2 1/8	4 1/2	
A19, B19	W24X68	2	Slotted	1	1 3/8	8 1/4	1/4	4 X 4	3	6 3/4	L6X4X5/8	18	6	4	5/16	5/16	5/16	1/4	1/8	1 1/8	4	2 1/8	4 1/2
A20, B20	W24X94	2	Slotted	1 1/4	1 3/8	8 3/4	-	-	-	L6X4X5/8	22 1/2	6	4	5/16	5/16	5/16	-	-	1 1/8	5	2 1/8	4 1/2	
A30, B30	W36X150	2	Slotted	1 1/4	1 3/8	8 1/4	-	-	-	L5X3-1/2X5/8	27	5	3 1/2	5/16	5/16	5/16	-	-	1 1/8	6	2 1/8	4 1/2	
B11	W24X68	2	Slotted	3/4	1 3/8	8 1/4	-	-	-	L7X4X5/8	18	7	4	5/16	5/16	5/16	-	-	1 1/8	4	2 1/8	4 1/2	

6 BEAM END SCHEDULE
 N.T.S.



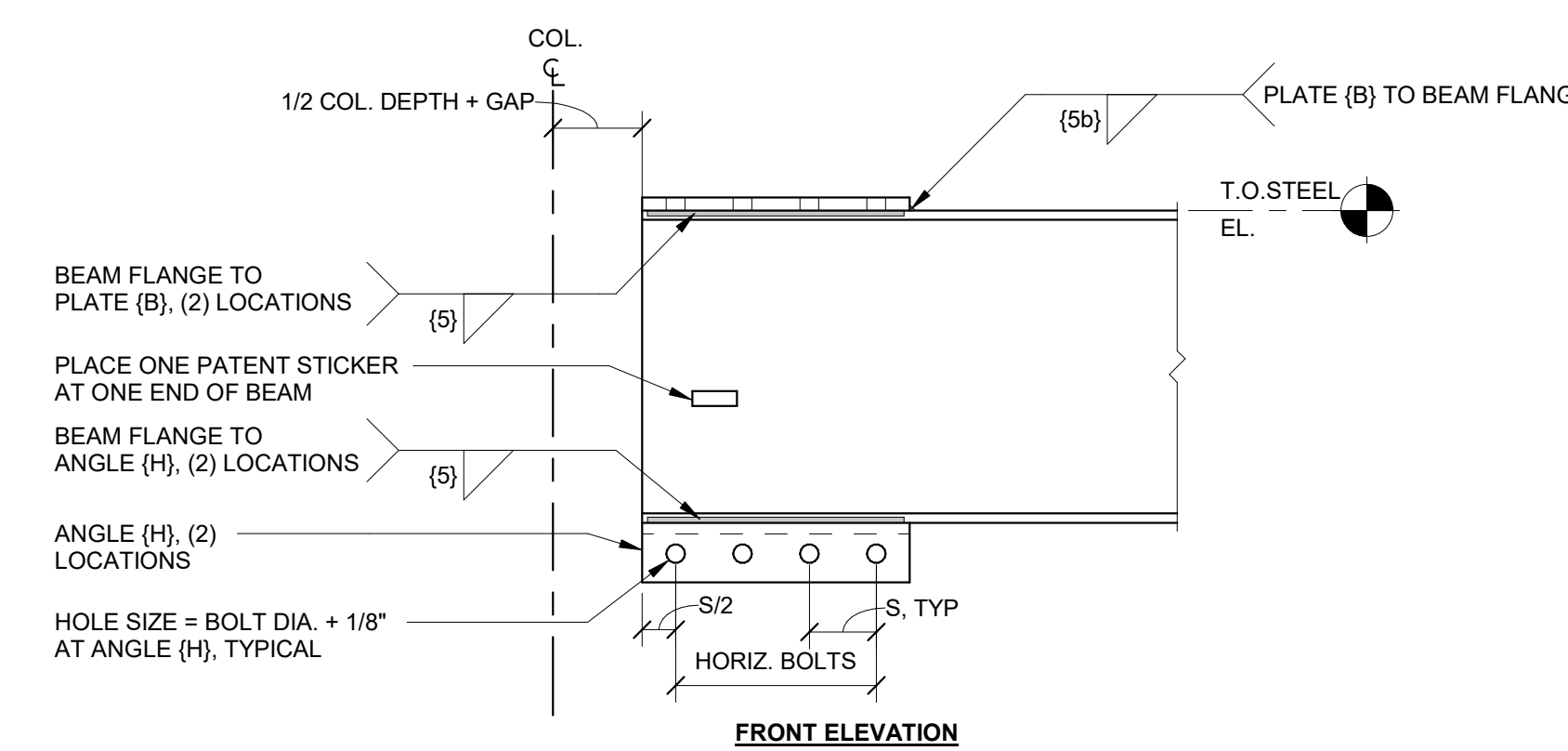
NOTE(S):
 1. SEE BEAM END SCHEDULE FOR QUANTITY, SPACING, AND WELDING OF STIFFENER PLATES.
 2. STIFFENER PLATES SHALL BE MADE OF GRADE 50 MATERIAL.
 3. STIFFENER PLATES AND WELDS ARE NOT CREATED BY SIDEPLATE CUSTOM COMPONENT TOOL.

6 STIFFENER PLATES
 N.T.S.



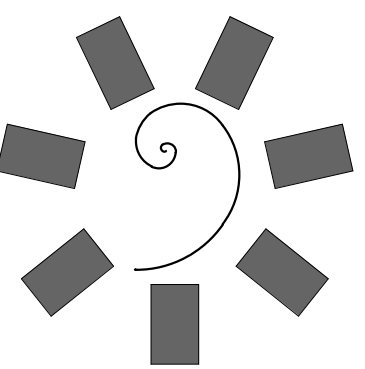
NOTE(S):
 1. FOR ITEMS NOT NOTED, SEE DETAIL 1 / S8.04

5 RECTANGULAR COVER PLATE (B)
 N.T.S.

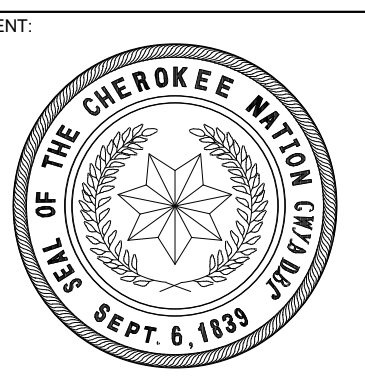


NOTE(S):
 1. USE SLOTTED OR RECTANGULAR COVER PLATE (B) PER SCHEDULE. FOR RECTANGULAR COVER PLATE, SEE DETAIL 5 / S8.04

1 BEAM END DETAIL
 N.T.S.



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 479-783-2450
 www.childersarchitect.com



WILMA P. MANKILLER HEALTH CENTER
 EXPANSION
 STILWELL, OKLAHOMA

PROJECT PHASE:
 BID PACKAGE 01

DATE: 11-01-19
 JOB NUMBER: 18-01.01

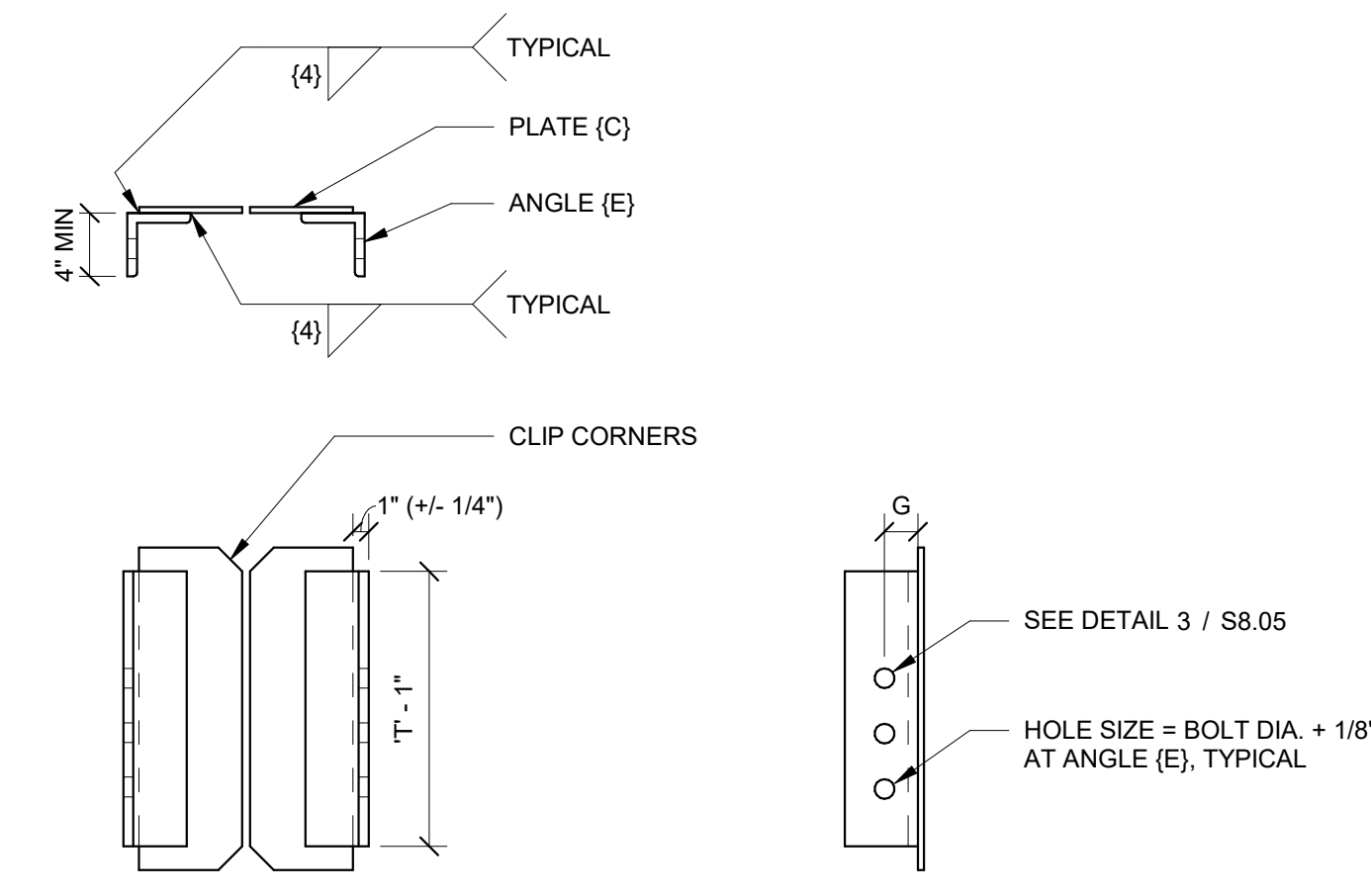
#	DATE	REVISIONS DESCRIPTION

DATE: 11-01-19
 JOB NUMBER: 18-01.01

SHEET NUMBER:
 S8.04

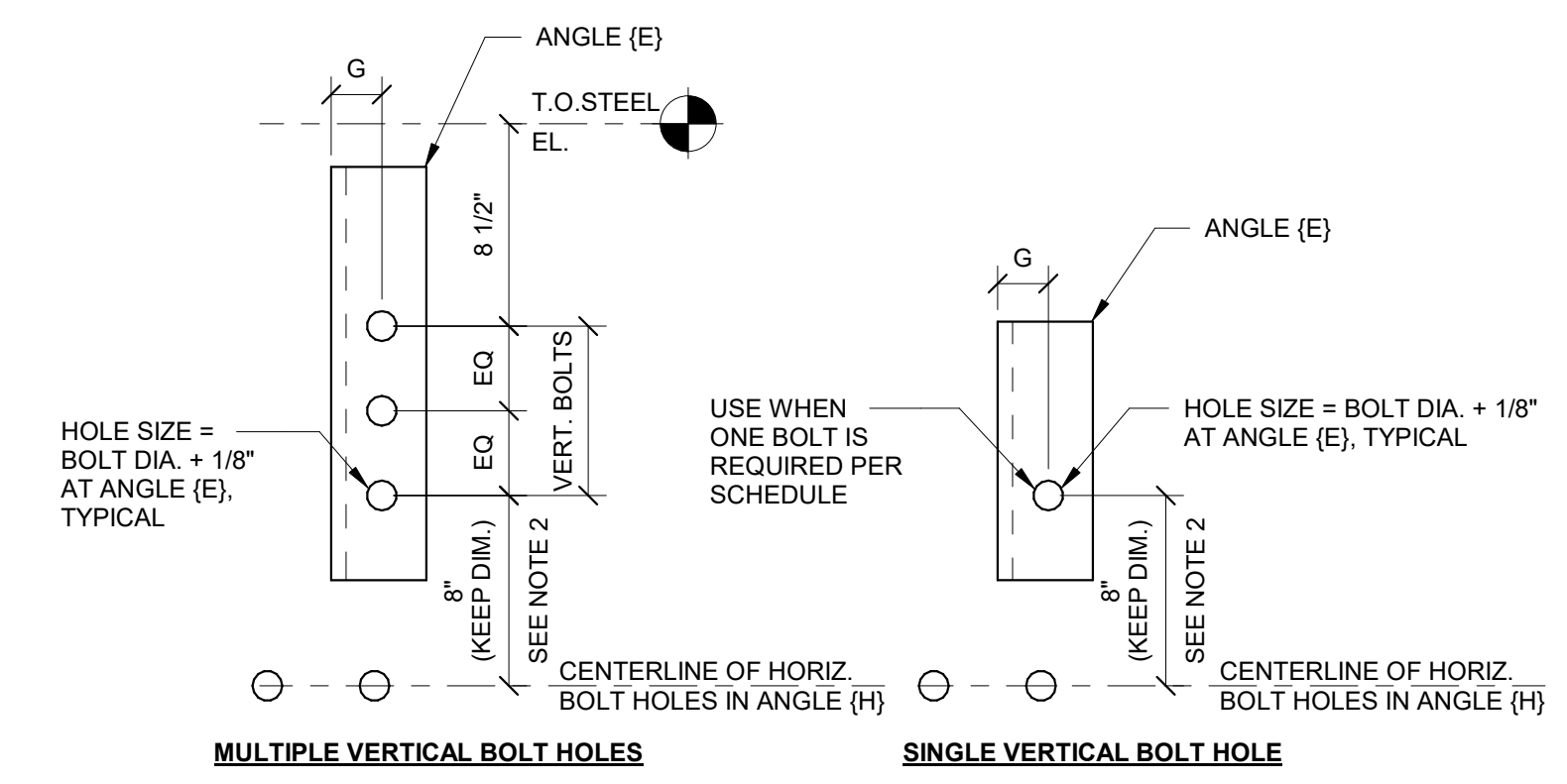
SIDEPLATE BEAM
 DETAILS

INTELLECTUAL PROPERTY RIGHTS NOTICE
 The SIDEPLATE® steel frame connection system is covered by one or more of U.S. Pat. Nos. 6,138,427; 6,516,583; 6,591,573; 7,178,296; 8,122,671; 8,122,672; 8,146,322; 8,176,706; 8,205,408; and 9,091,065 and foreign counterparts.
 Other U.S. and foreign applications pending.
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 v15.08.02



NOTE(S):
 1. SEE SIDEPLATE SCHEDULE FOR BOLT QUANTITY.

4 VSE (F) DETAIL
 N.T.S.

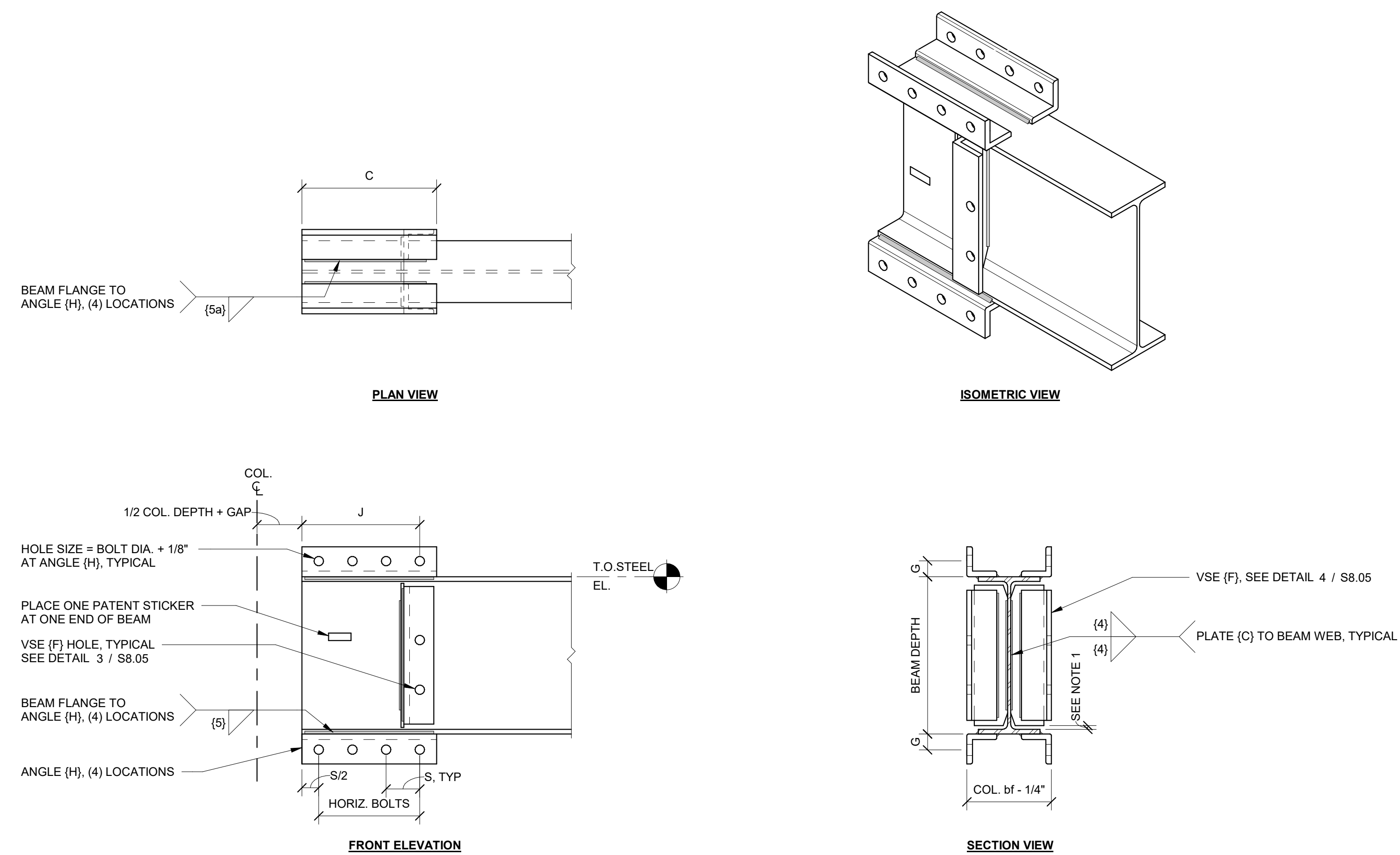


NOTE(S):
 1. SEE BEAM END SCHEDULE FOR BOLT QUANTITY.
 2. EFFECTS OF MILL AND FABRICATION TOLERANCES ARE ACCOUNTED FOR BY MEASURING FROM THE CENTERLINE OF THE HORIZONTAL ROW OF BOLTS IN THE BOTTOM ANGLES (H).

3 VSE (F) HOLE DETAIL
 N.T.S.

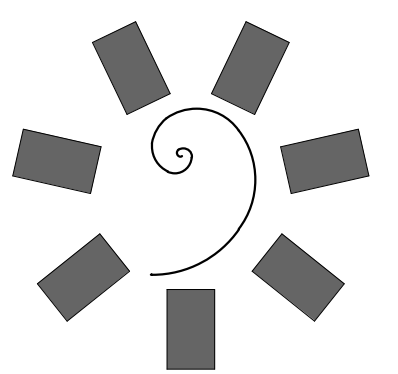
ID	BEAM DESIGN (INCHES)																
	BEAM		PLATE		ANGLE			WELD				BOLT					
	SHAPE	GAP	THICKNESS	SUGGESTED SIZE	C	HORIZONTAL LEG	VERTICAL LEG	SIZE	SIZE	SIZE	SIZE	DIAMETER	HORIZONTAL #	VERTICAL #	G	J	S
A15, B15	W24X68	2	3/8	L6X4X5/8	18	6	4	L4X4X1/2	1/4	5/16	5/16	1 1/8	4	2	2 1/8	15 3/4	4 1/2
A25, B25	W24X94	2	3/8	L6X4X5/8	22 1/2	6	4	L4X4X1/2	1/4	5/16	5/16	1 1/8	5	2	2 1/8	20 1/4	4 1/2
A45, B45	W36X160	2	3/8	L5X3-1/2X5/8	27	5	3 1/2	L4X4X1/2	1/4	5/16	5/16	1 1/8	6	3	2 1/8	24 3/4	4 1/2

2 NARROW BEAM END SCHEDULE
 N.T.S.

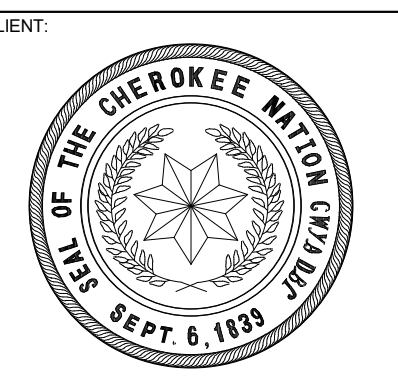


NOTE(S):
 1. DIMENSION BETWEEN PLATE (C) AND INSIDE FACE OF BEAM FLANGE SHALL NOT EXCEED 1/4 INCH, AND MAY VARY DEPENDING ON BEAM MILL TOLERANCES. PLATE (C) SHALL BE CENTERED ON THE DEPTH OF THE BEAM.

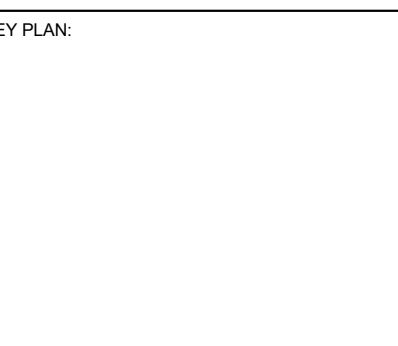
1 NARROW BEAM END DETAIL
 N.T.S.



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WILMA P. MANKILLER HEALTH CENTER
 EXPANSION
 STILWELL, OKLAHOMA



PROJECT PHASE:
 BID PACKAGE 01

#	DATE	REVISIONS DESCRIPTION

DATE: 11-01-19 JOB NUMBER: 18-01.01

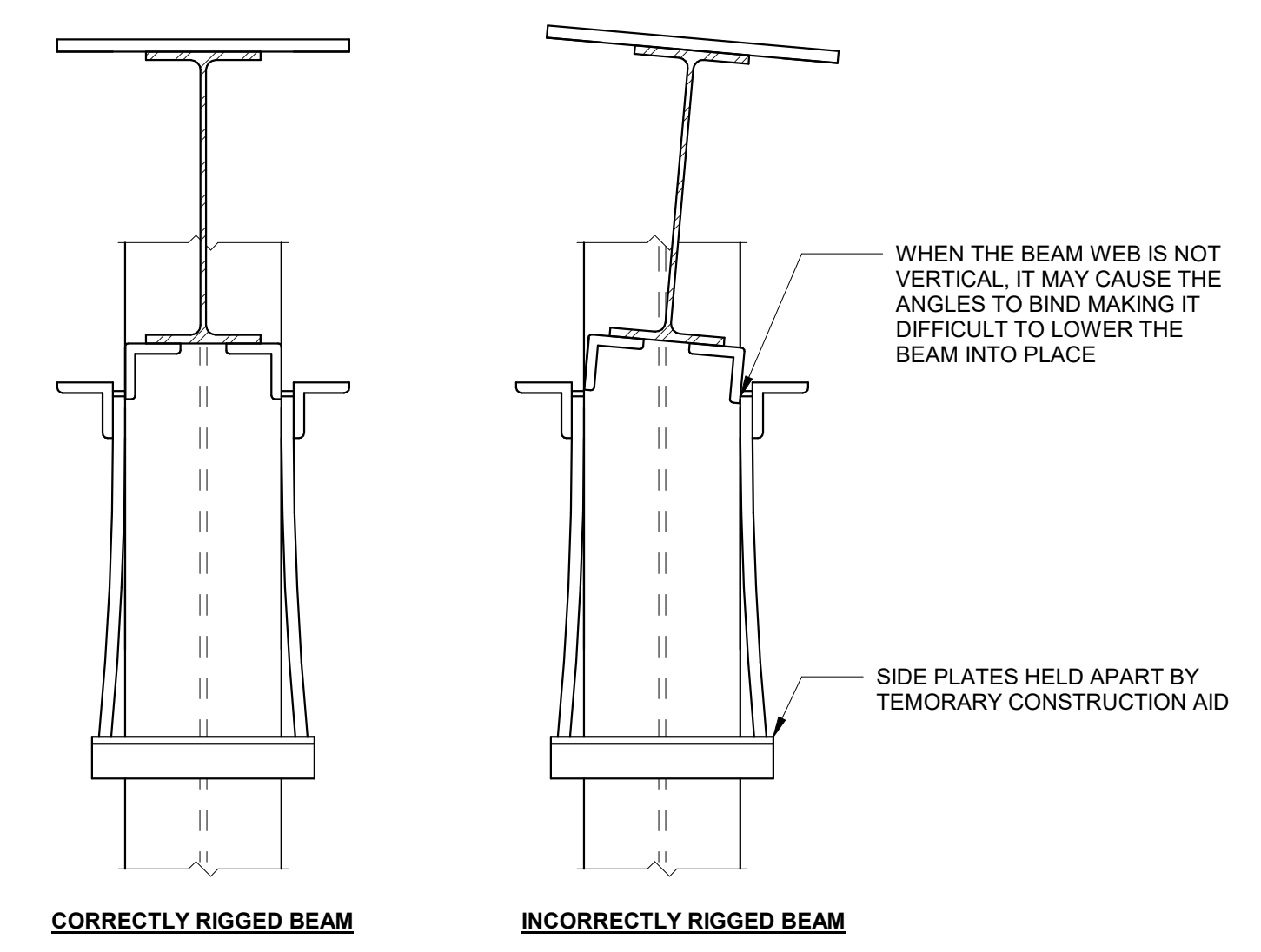
SHEET NUMBER:
 S8.05

SIDEPLATE BEAM
 DETAILS, NARROW

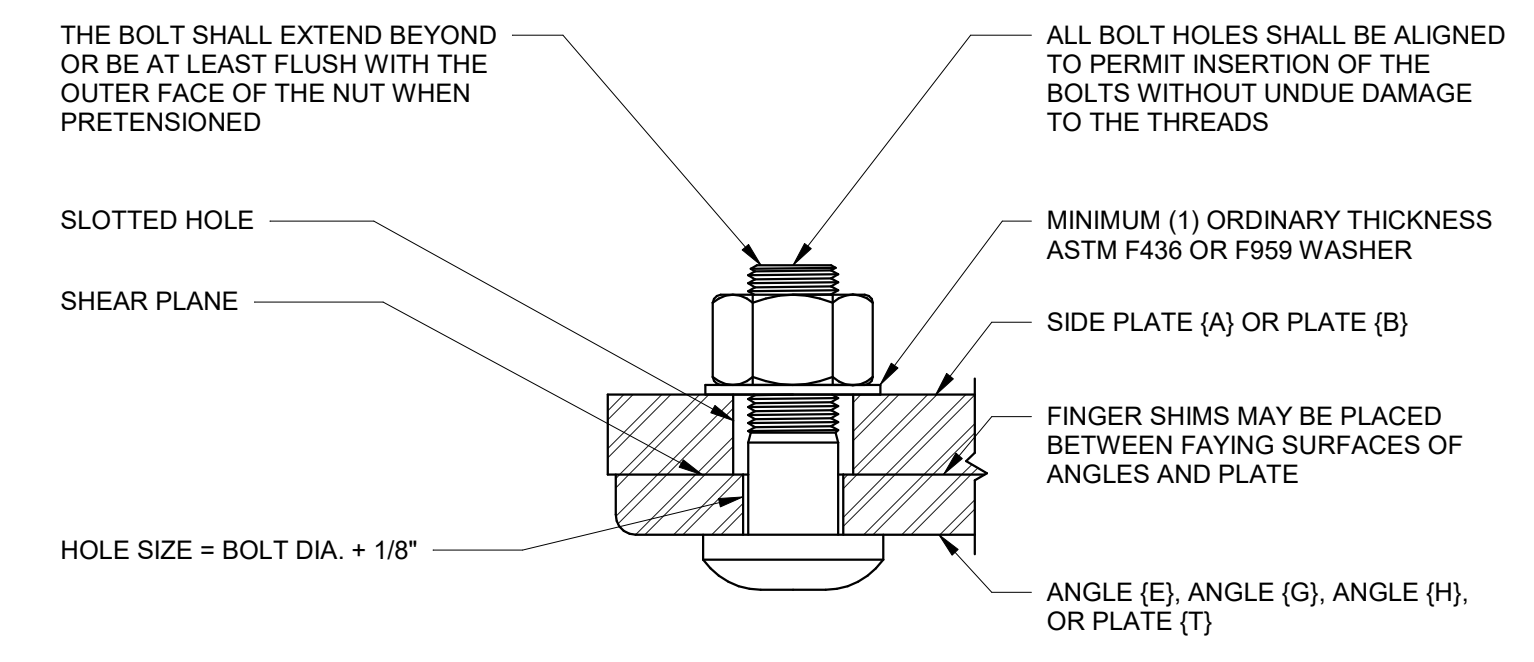
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v15.08.02
 N.P.E. REVISED



4 BEAM INSTALLATION DETAIL
 N.T.S.



NOTES:
 1. BOLTS SHALL BE INSTALLED AS SHOWN TO KEEP THREADS OUTSIDE OF SHEAR PLANE.
 2. BOLTS SHALL BE SYSTEMATICALLY INSTALLED AS OUTLINED IN THE BOLTING SPECIFICATIONS. FIRST TO A SNUG TIGHT CONDITION, AND THEN PRETENSIONED.
 3. THE USE OF FINGER SHIMS ARE ALLOWED FOR GAPS GREATER THAN 1/8 INCH UP TO 1/4 INCH. CONTACT SIDEPLATE SYSTEMS, INC. IF GAPS ARE GREATER THAN 1/4 INCH.
 4. NUT SHALL BE ASTM A563.
 5. THE BOLT/FASTENER ASSEMBLY SHALL BE COVERED IN A LIGHT PROTECTIVE OIL.
 6. FOLLOW QUALITY CONTROL SECTION FOR EXPOSURE LIMITATION ON BOLTS/FASTENERS.

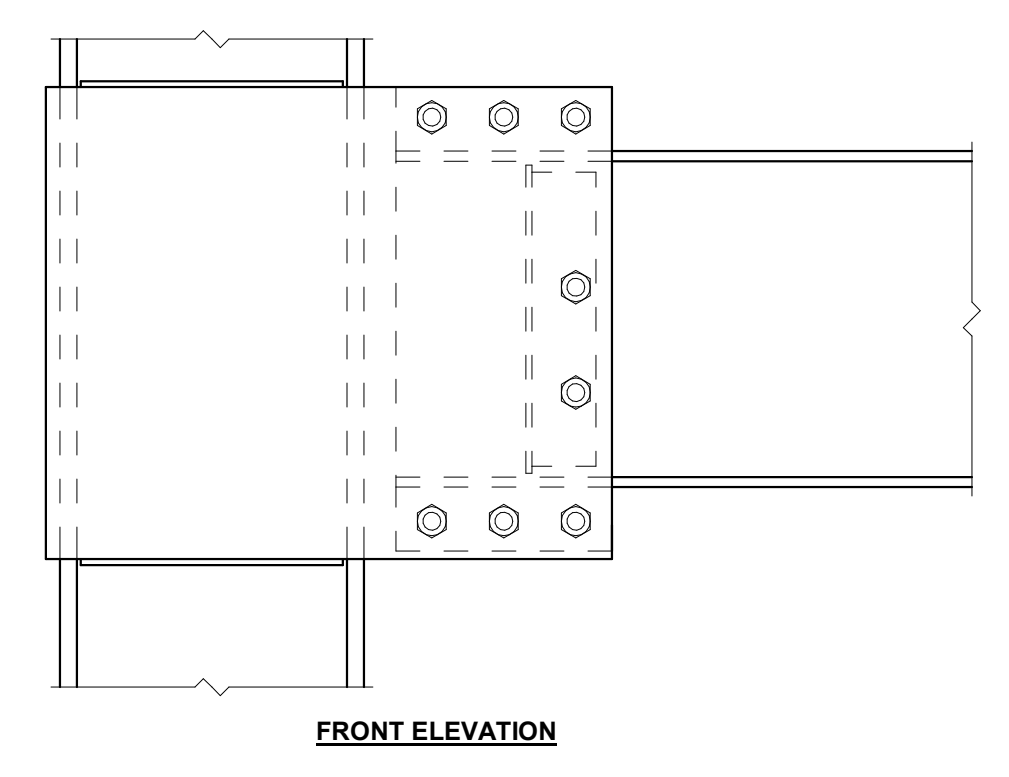
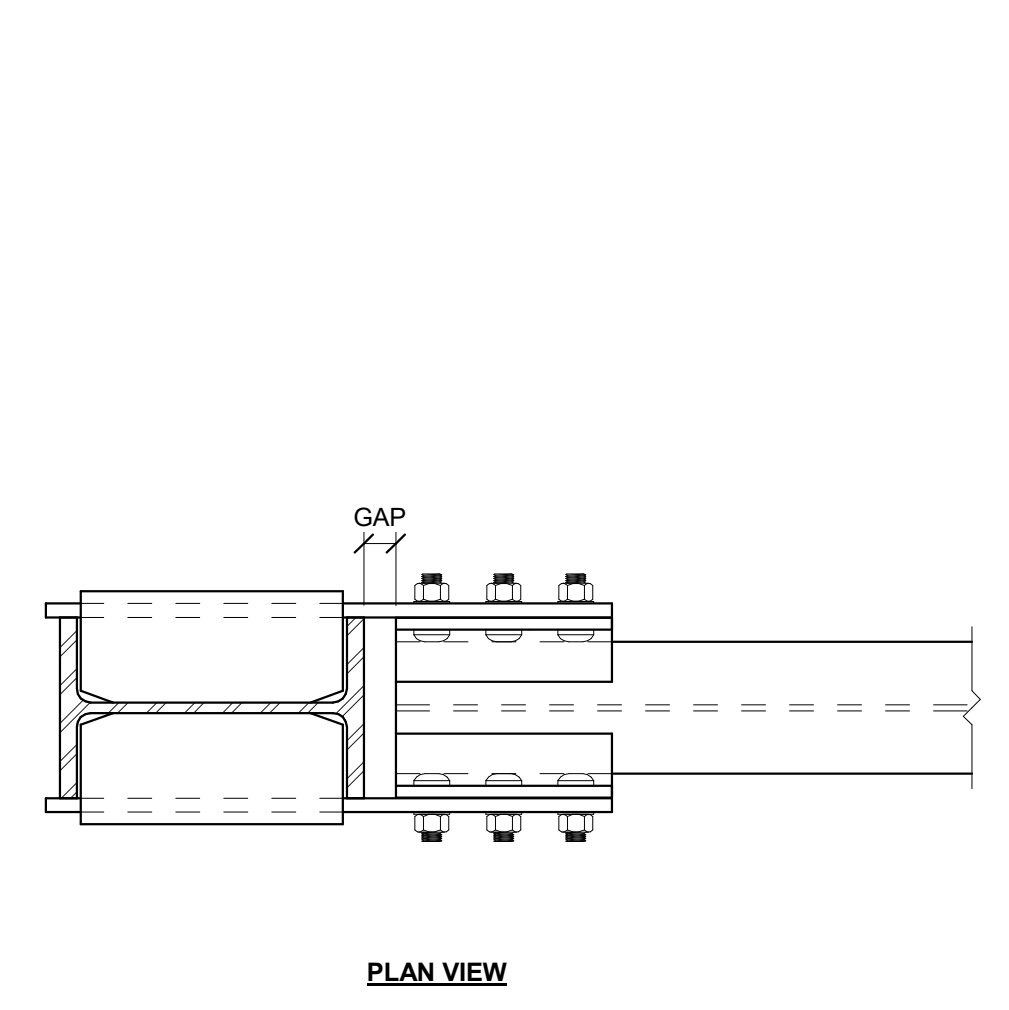
3 FIELD BOLTING DETAIL
 N.T.S.

ID	ERECTION DESIGN (INCHES)				
	BEAM		BOLT		
	SHAPE	DIAMETER	HORIZONTAL #	VERTICAL #	TOTAL # PER BEAM END
A15, B15	W24X88	1 1/8	4	2	20
A25, B25	W24X94	1 1/8	5	2	24
A45, B45	W36X160	1 1/8	6	3	30

6 NARROW BEAM ERECTION SCHEDULE
 N.T.S.

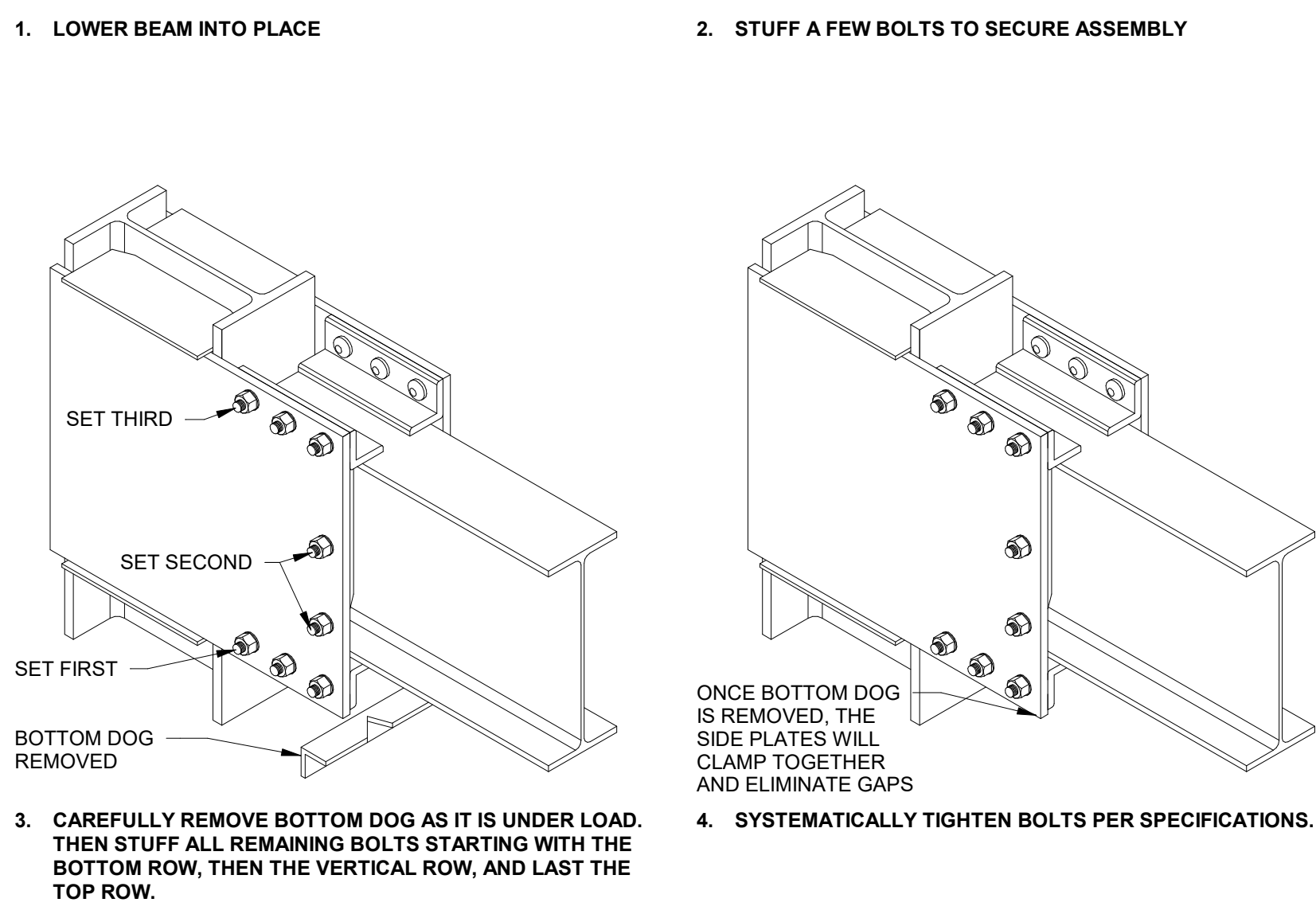
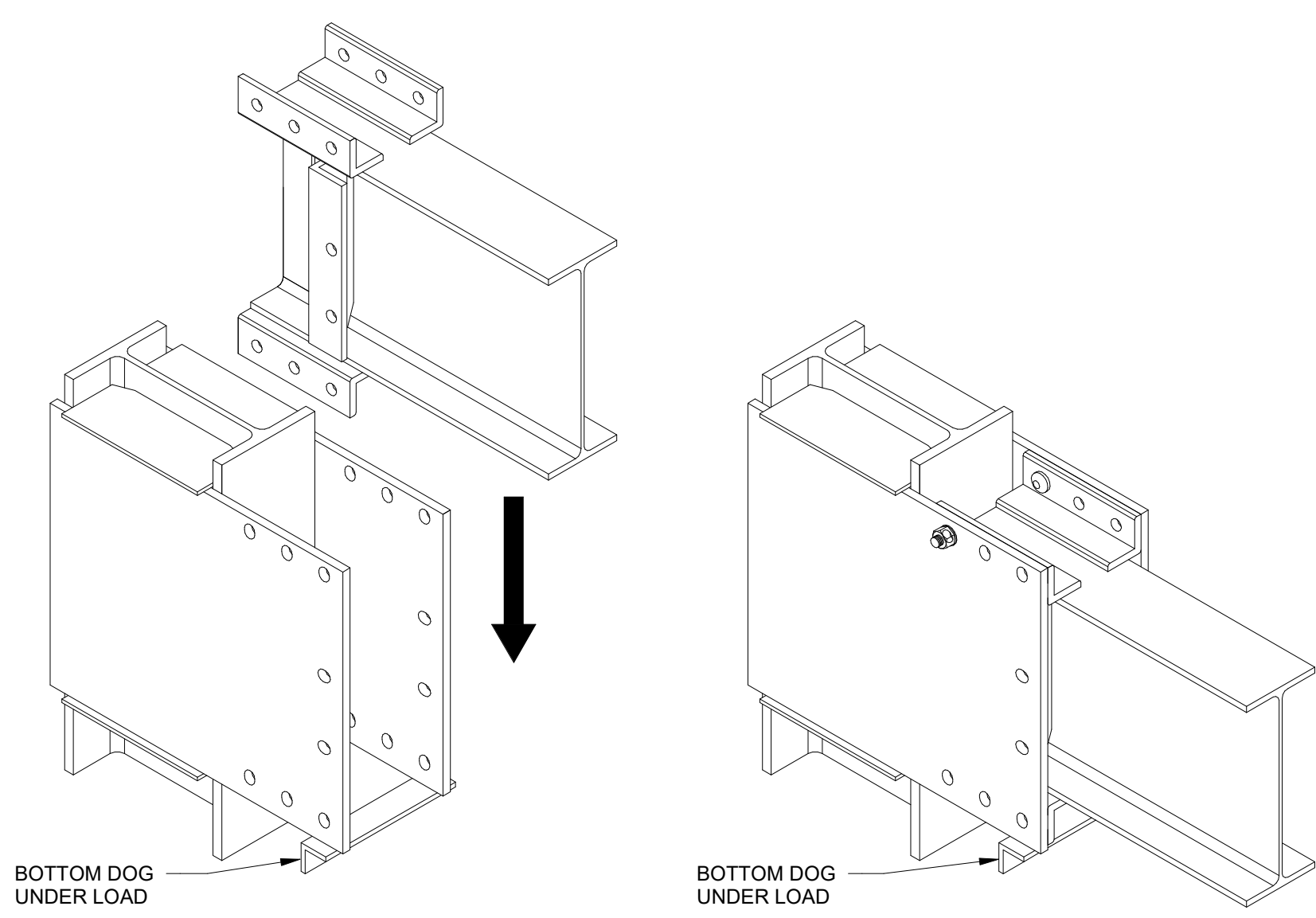
ID	ERECTION DESIGN (INCHES)			
	BEAM		BOLT	
	SHAPE	DIAMETER	HORIZONTAL #	TOTAL # PER BEAM END
A10, A11, A12, A19, B11, B12, B19	W24X88	1 1/8	4	16
A20, B20	W24X94	1 1/8	5	20
A30, B30	W36X150	1 1/8	6	24

2 BEAM ERECTION SCHEDULE
 N.T.S.

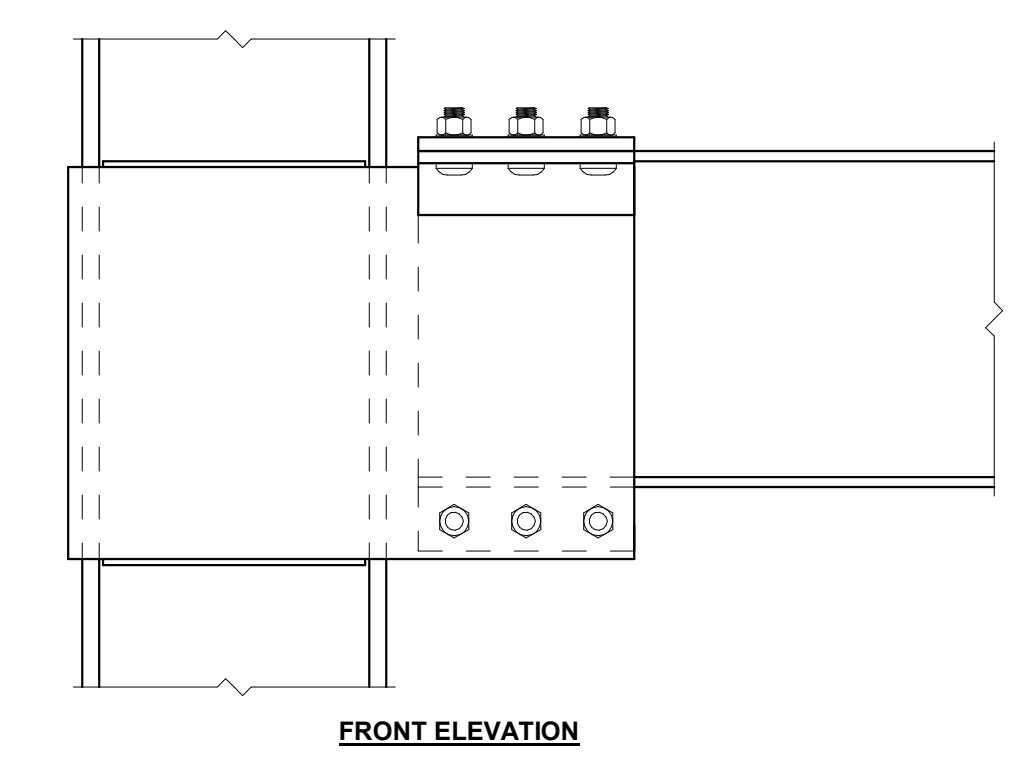
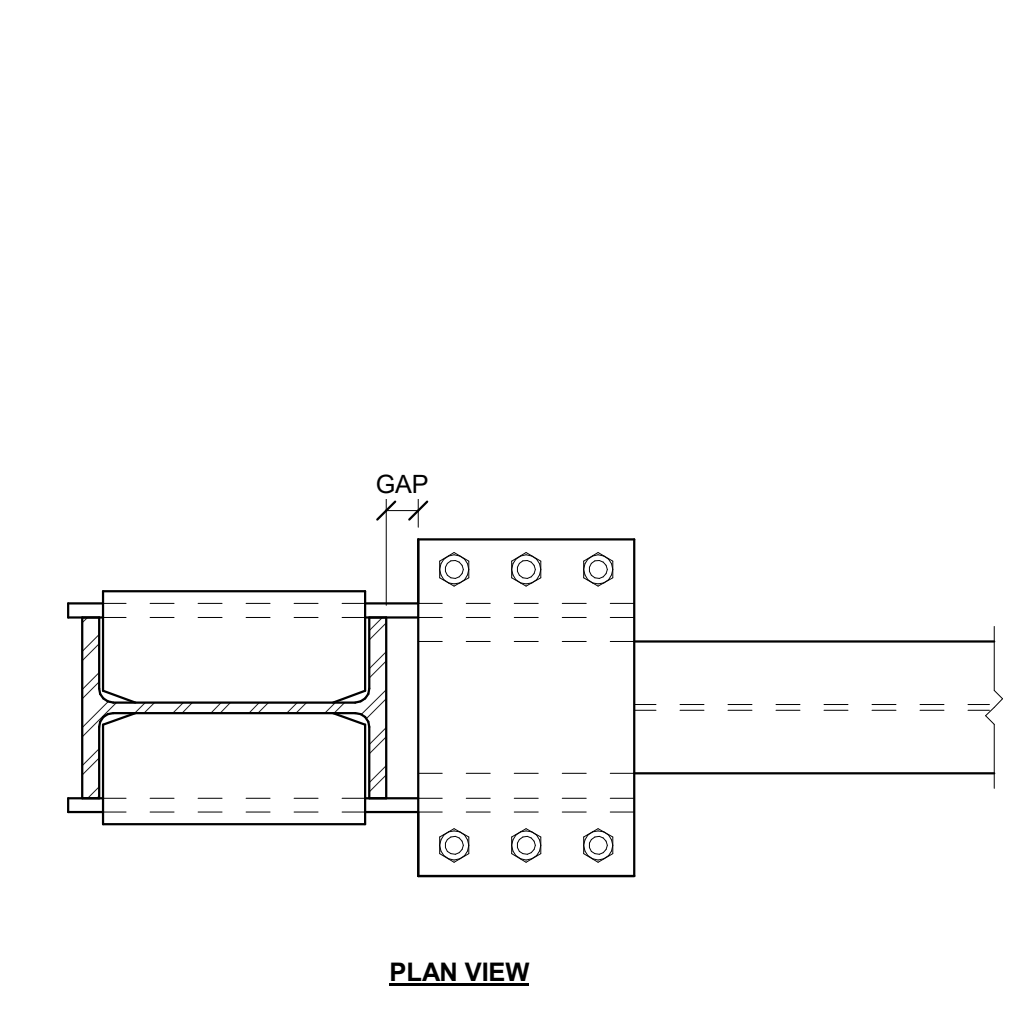


TYPICAL SEQUENCE OF ERECTION:
 1. LOWER THE BEAM INTO PLACE FROM ABOVE.
 2. STUFF A FEW BOLTS TO SECURE ASSEMBLY.
 3. **BOTTOM DOG SHALL BE REMOVED.** IT IS RECOMMENDED THAT IT BE REMOVED BY TORCH CUTTING A V SECTION OUT OF ONE OF THE ANGLE LEGS TO ALLEVIATE THE LOAD AND THEN PROCEED TO REMOVE IT. IT IS NOT RECOMMENDED TO USE A GRINDING WHEEL TO REMOVE THE WELDS WHILE THE DOG IS UNDER LOAD.
 4. BOLTS SHALL BE STUFFED INTO HOLES IN THE BEAM COVER PLATE (B) AND THE SIDE PLATES (A). SYSTEMATICALLY TIGHTEN BOLTS PER RCSC SPECIFICATIONS.
 5. THE WELD REMNANTS OF THE BOTTOM DOG MAY REMAIN IN PLACE AND DO NOT NEED TO BE GROUND SMOOTH.

5 NARROW BEAM ERECTION DETAIL
 N.T.S.

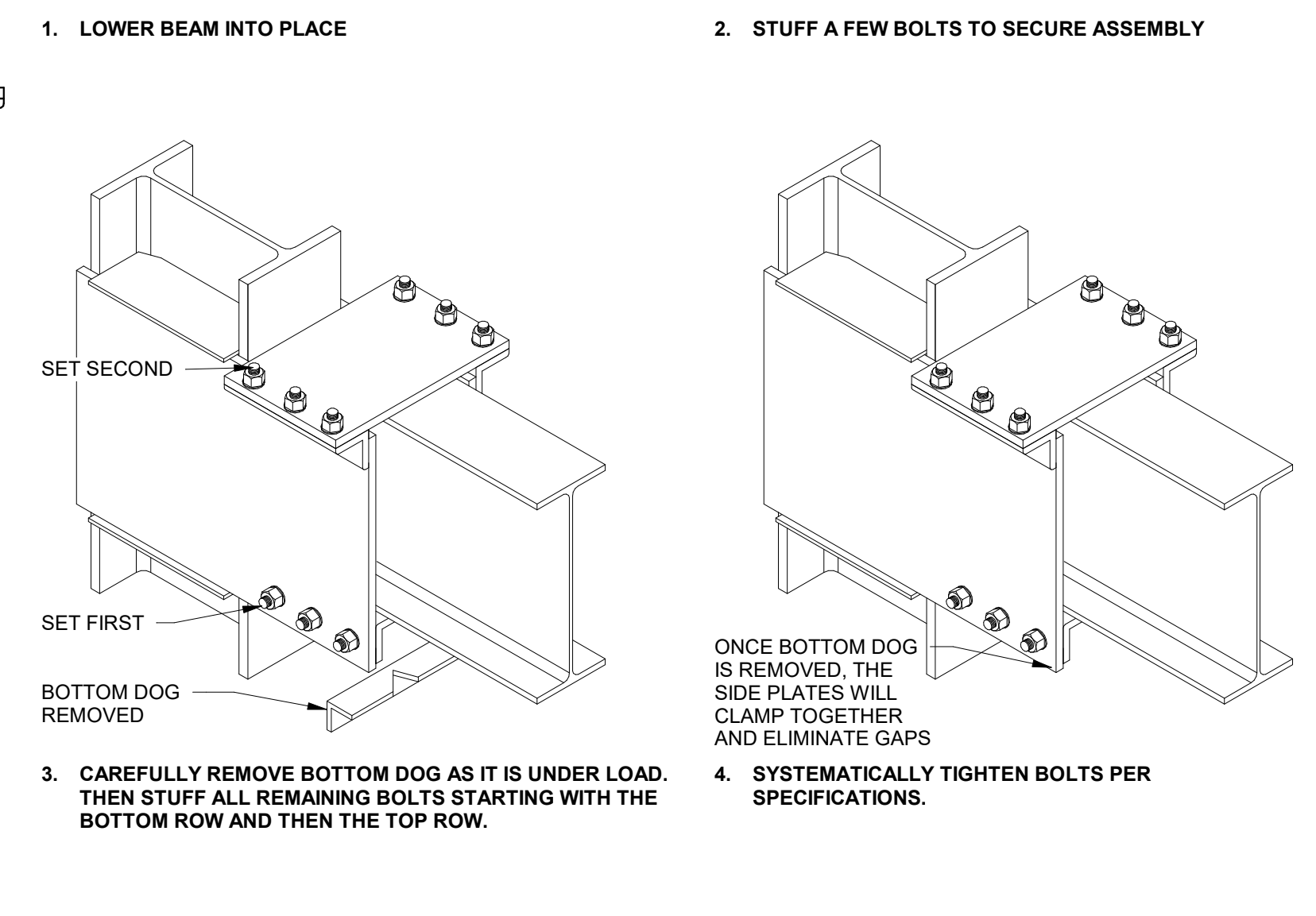
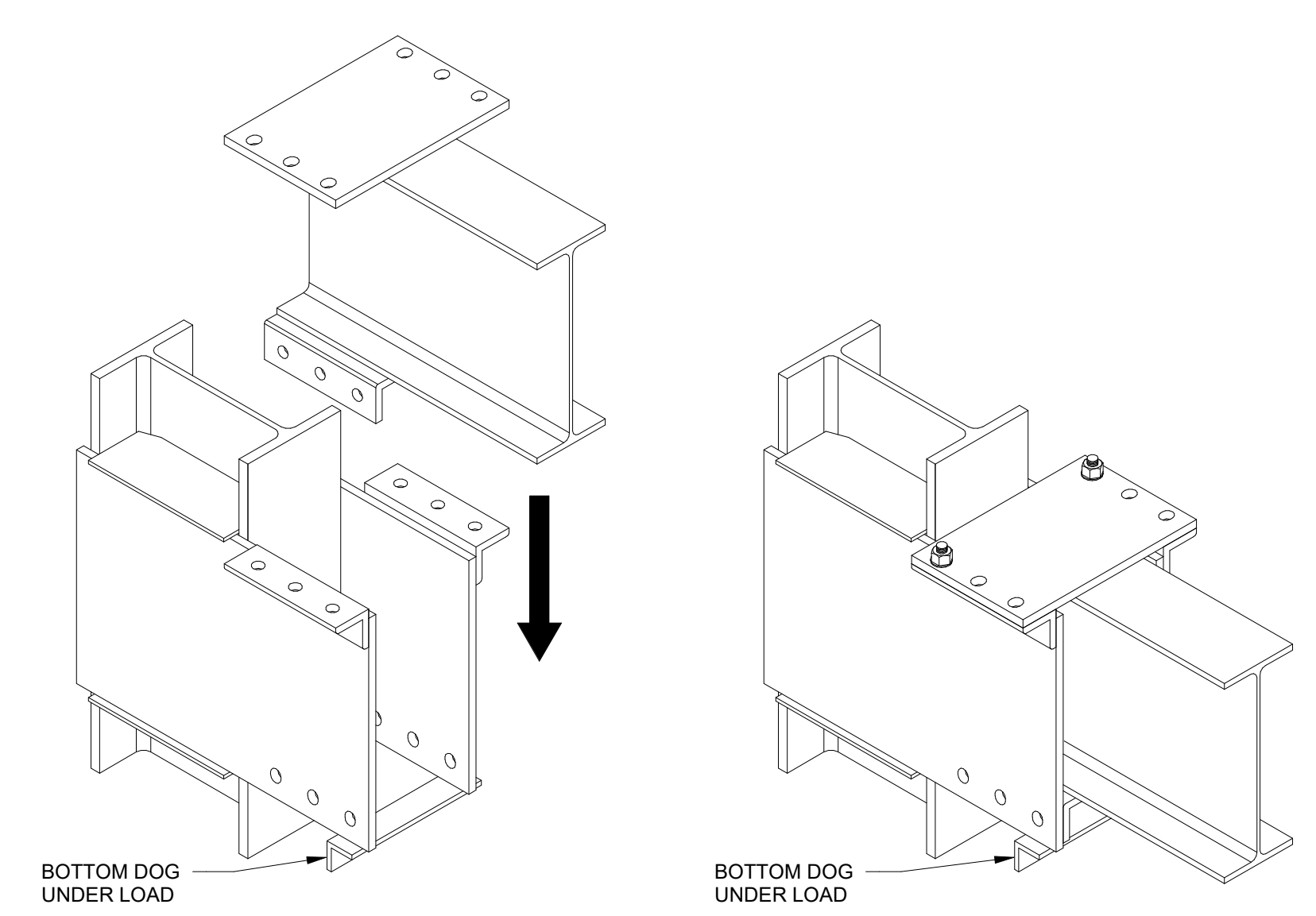


6 NARROW BEAM ERECTION SCHEDULE
 N.T.S.



TYPICAL SEQUENCE OF ERECTION:
 1. LOWER THE BEAM INTO PLACE FROM ABOVE.
 2. STUFF A FEW BOLTS TO SECURE ASSEMBLY.
 3. **BOTTOM DOG SHALL BE REMOVED.** IT IS RECOMMENDED THAT IT BE REMOVED BY TORCH CUTTING A V SECTION OUT OF ONE OF THE ANGLE LEGS TO ALLEVIATE THE LOAD AND THEN PROCEED TO REMOVE IT. IT IS NOT RECOMMENDED TO USE A GRINDING WHEEL TO REMOVE THE WELDS WHILE THE DOG IS UNDER LOAD.
 4. BOLTS SHALL BE STUFFED INTO HOLES IN THE BEAM COVER PLATE (B) AND THE SIDE PLATES (A). SYSTEMATICALLY TIGHTEN BOLTS PER RCSC SPECIFICATIONS.
 5. THE WELD REMNANTS OF THE BOTTOM DOG MAY REMAIN IN PLACE AND DO NOT NEED TO BE GROUND SMOOTH.

3 BEAM ERECTION DETAIL
 N.T.S.

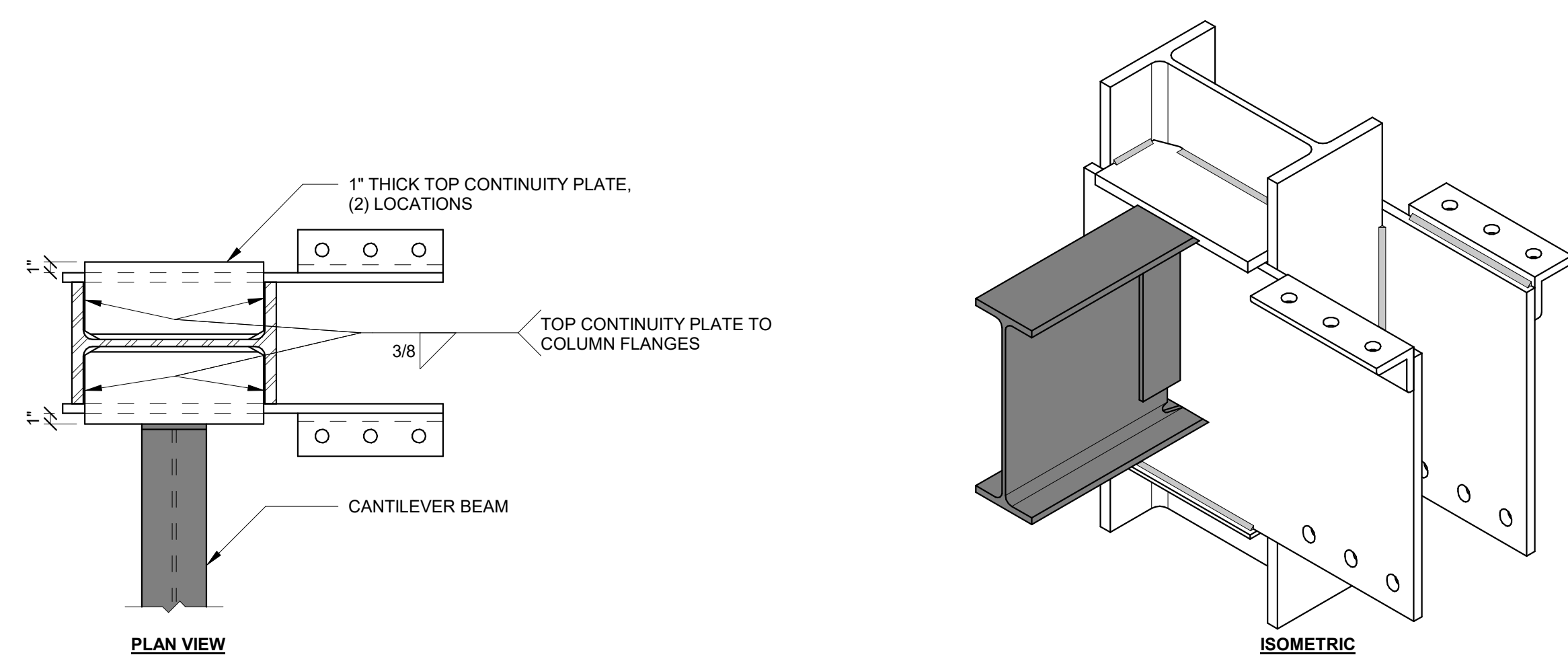


2 BEAM ERECTION SCHEDULE
 N.T.S.

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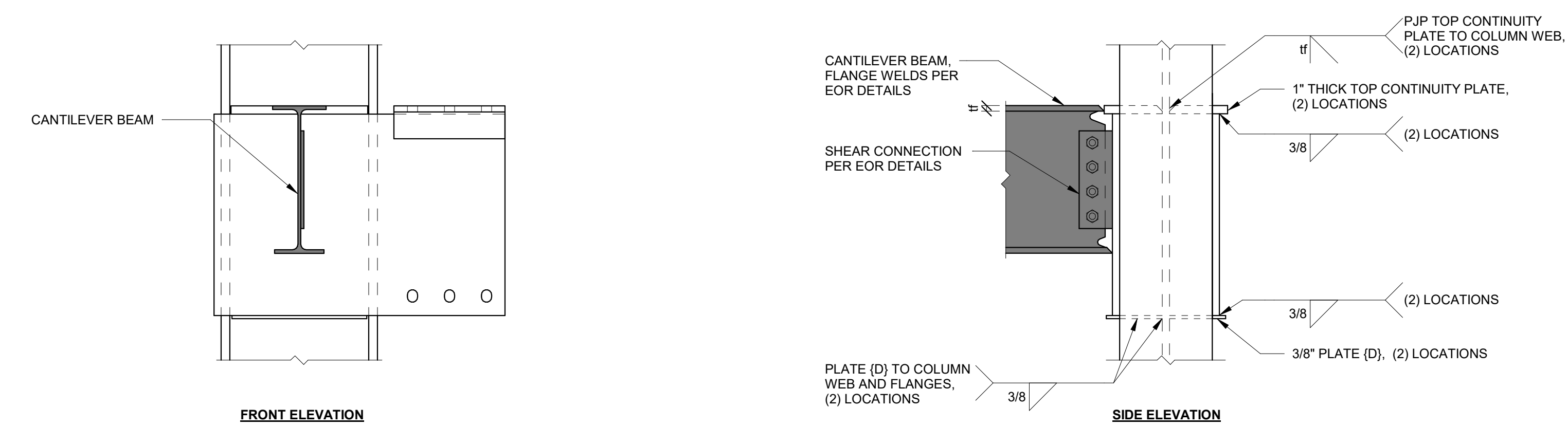
Misc ID	Coordinate with Detail
M1	11/S8.07

12 MISCELLANEOUS DETAILS SCHEDULE
 N.T.S.

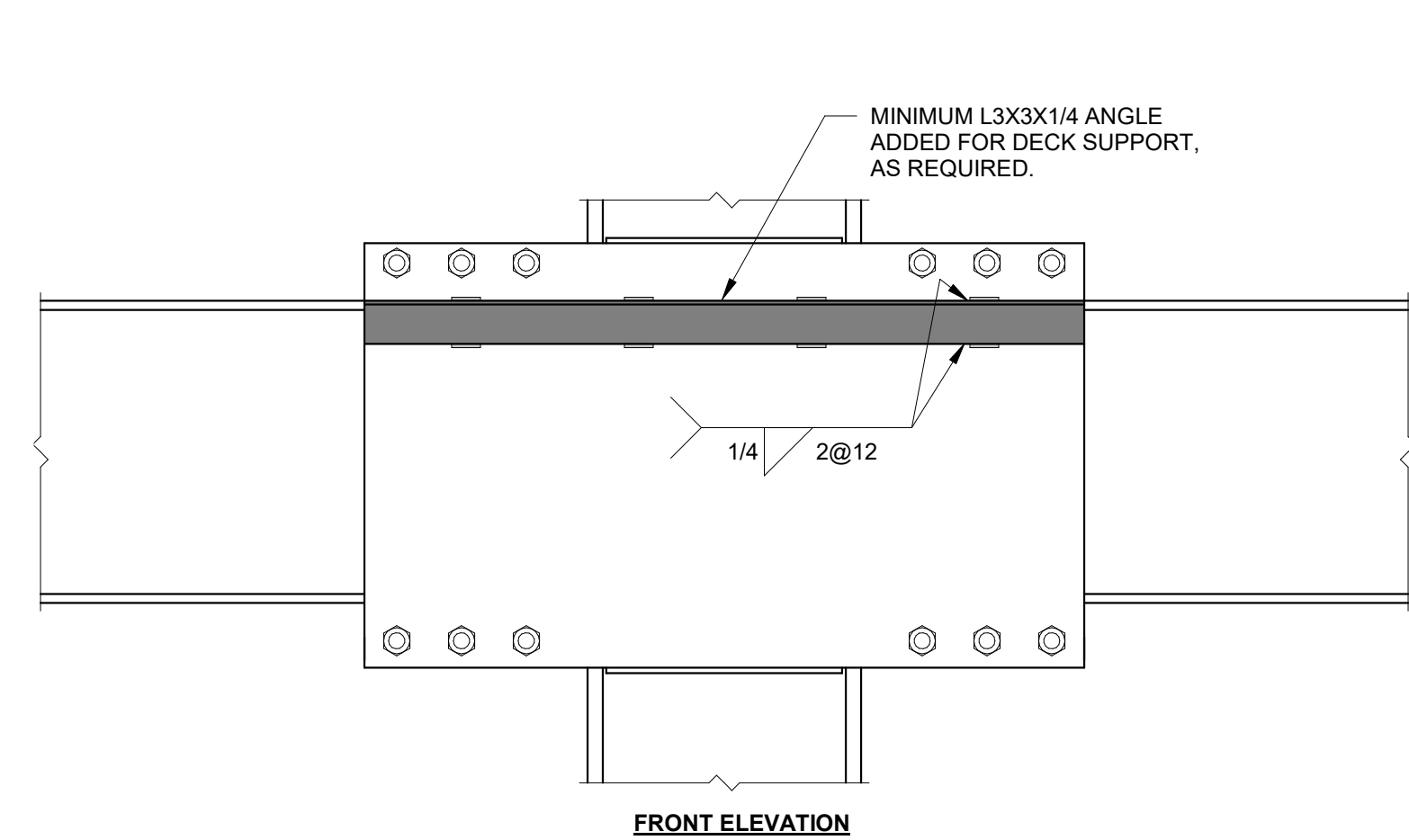


NOTES:
 1. ATTACHMENT SHOWN ON ONE SIDE OF SIDEPLATE CONNECTION FOR ILLUSTRATION. ATTACHMENT CAN OCCUR ON LEFT SIDE, RIGHT SIDE, OR BOTH SIDES OF CONNECTION AS APPLICABLE.

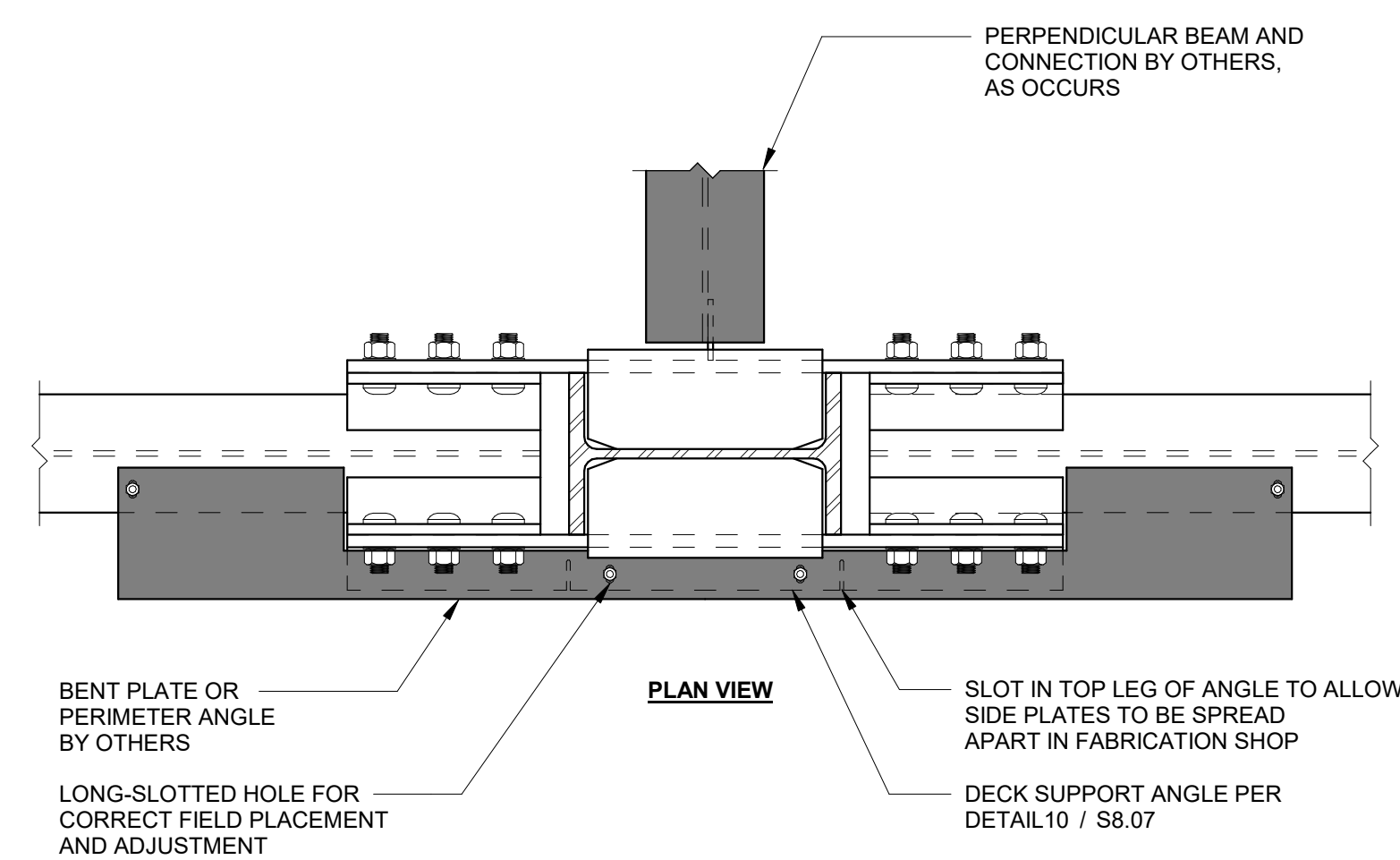
11 CANTILEVER TO SIDEPLATE CONNECTION
 N.T.S.



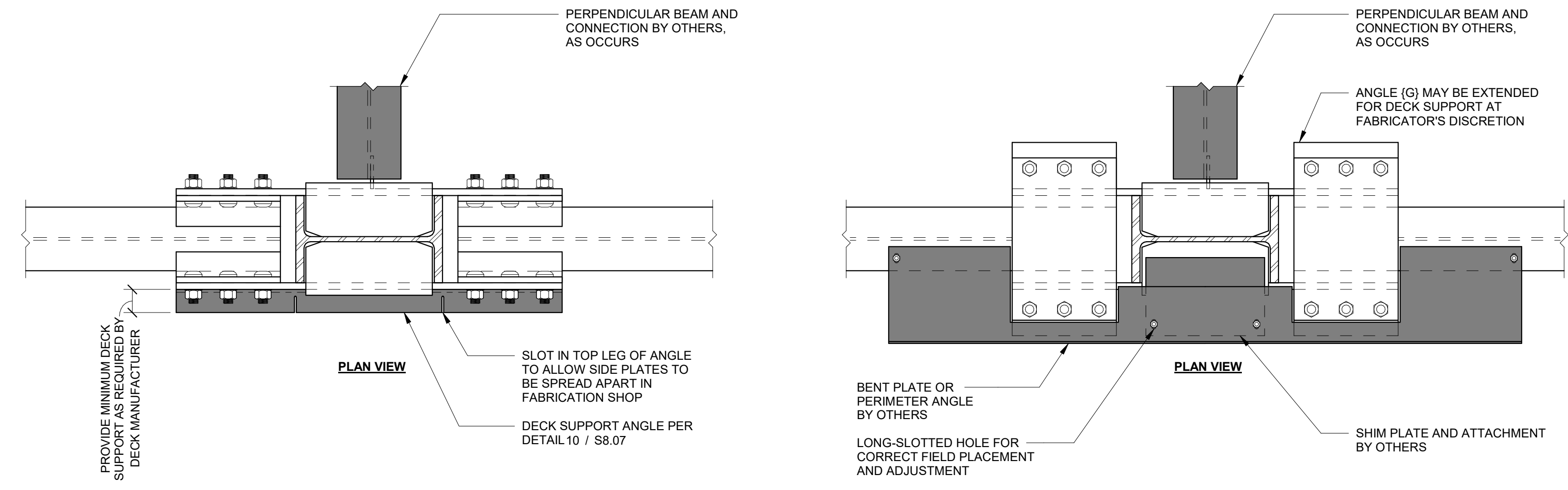
10 DECK SUPPORT ANGLE DETAIL
 N.T.S.



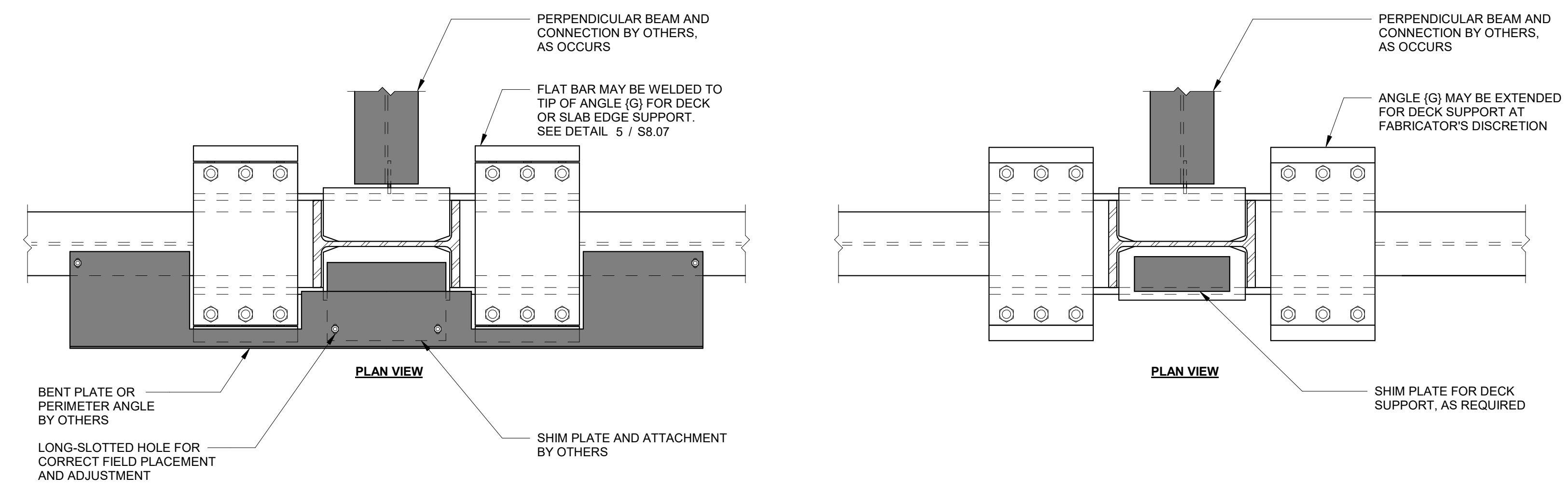
9 NARROW CONFIGURATION SLAB EDGE DETAIL
 N.T.S.



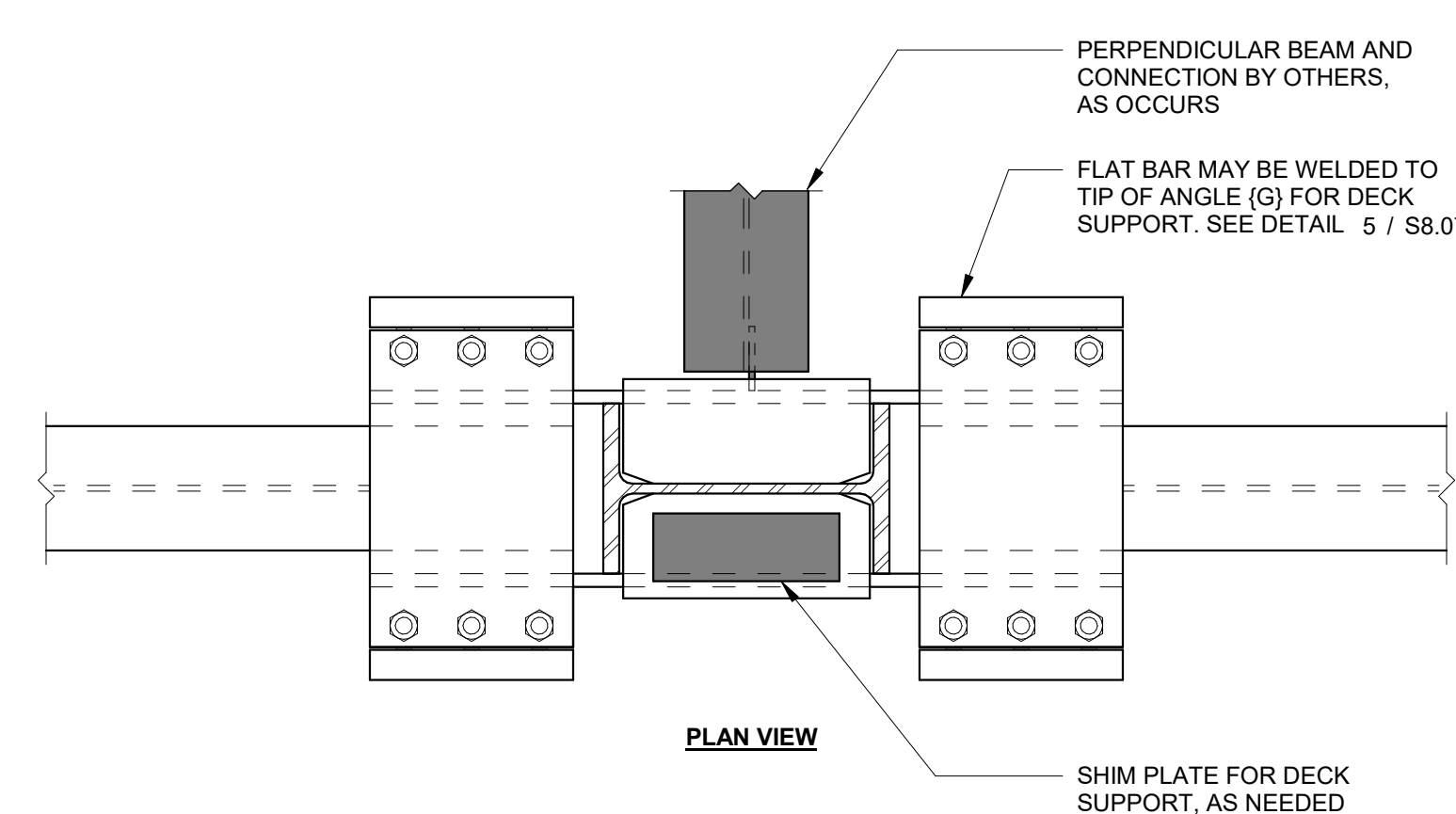
8 NARROW CONFIGURATION DECK SUPPORT DETAIL
 N.T.S.



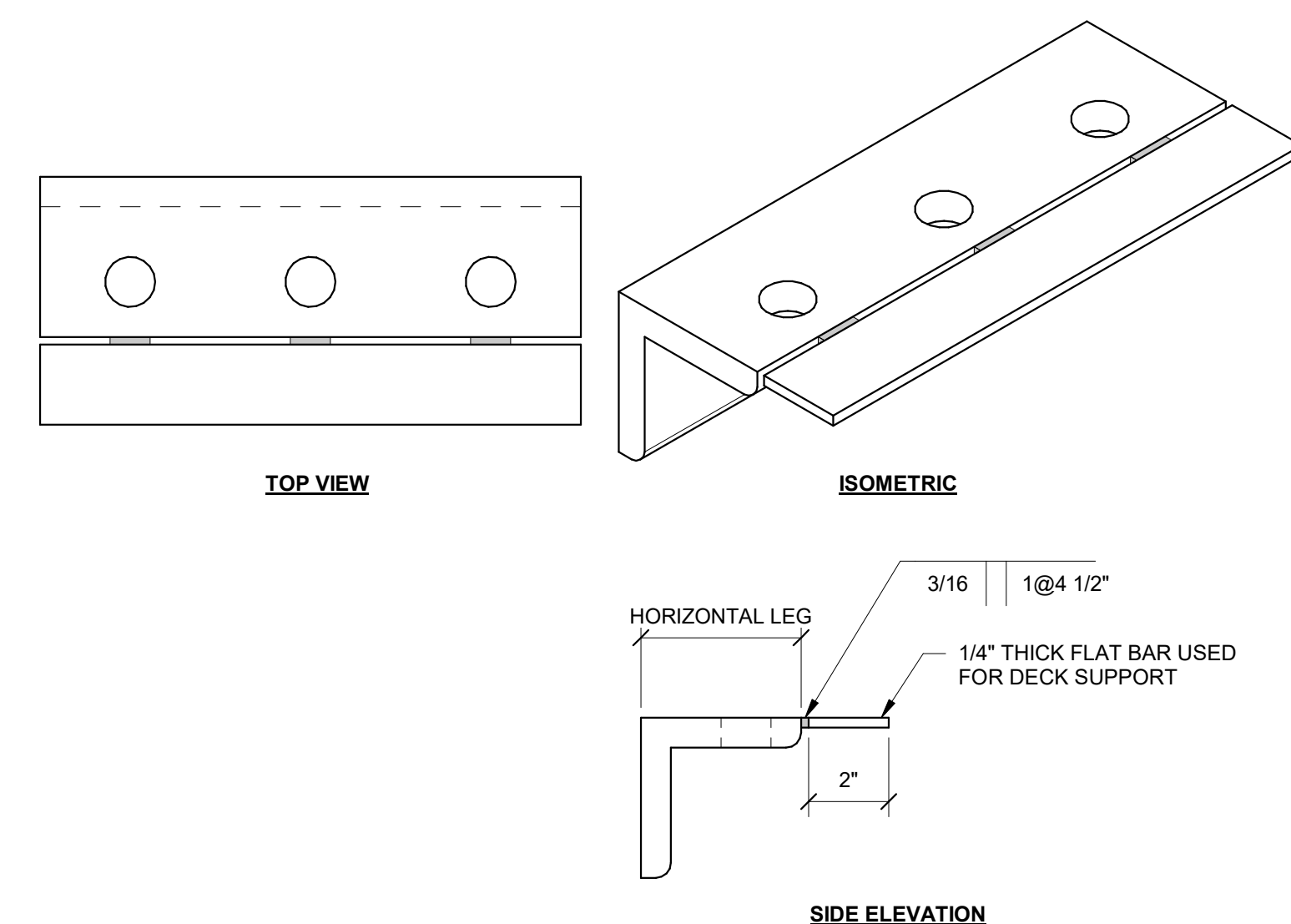
7 WELDED FLAT BAR FOR SLAB EDGE SUPPORT DETAIL
 N.T.S.



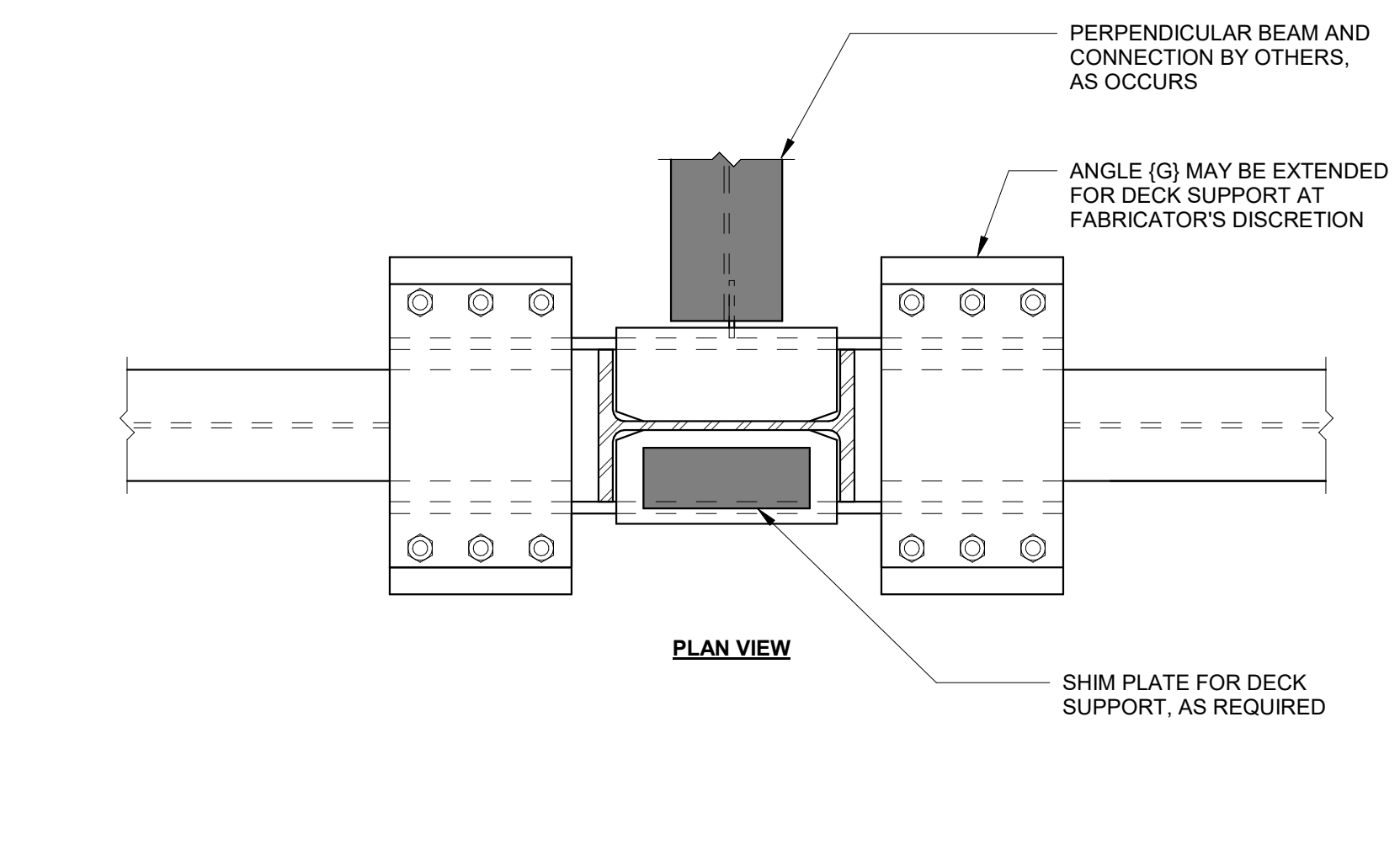
6 WELDED FLAT BAR DECK SUPPORT DETAIL
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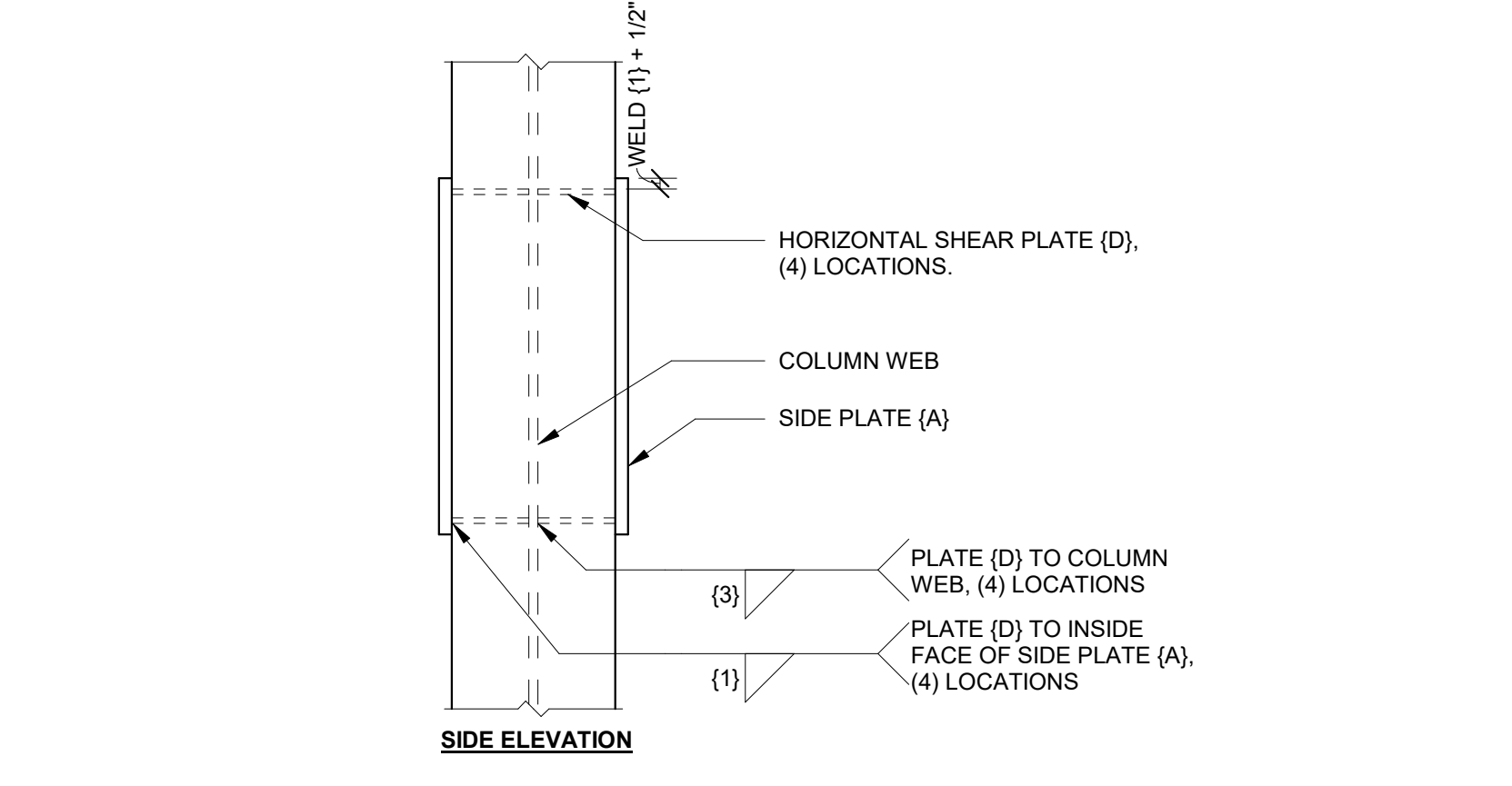
5 WELDED FLAT BAR TO ANGLE (G) FOR DECK SUPPORT
 N.T.S.



4 SLAB EDGE DETAIL
 N.T.S.

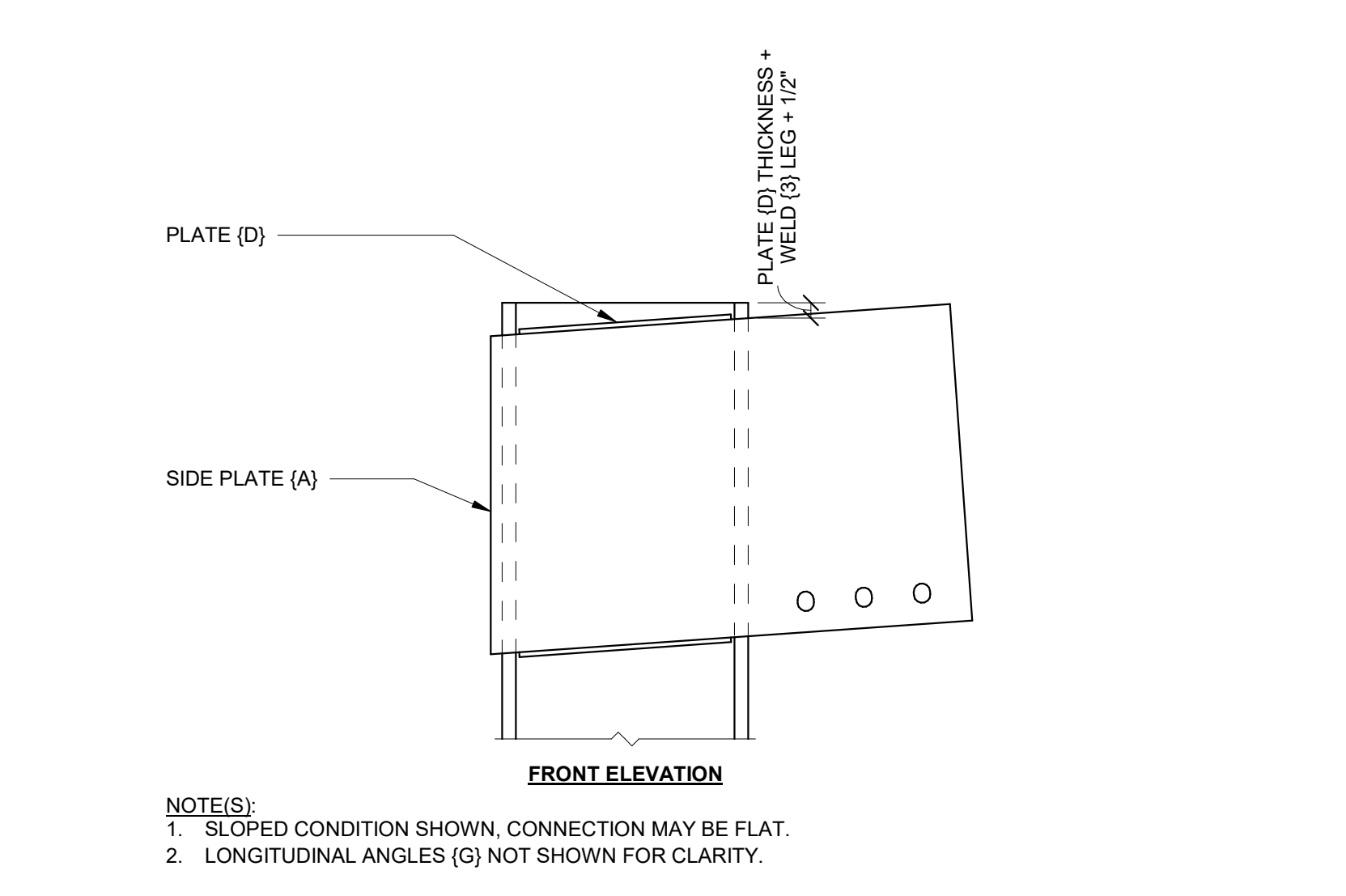


3 DECK SUPPORT DETAIL
 N.T.S.



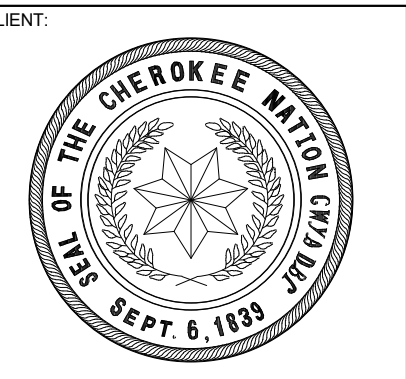
NOTE(S):
 1. LONGITUDINAL ANGLES (G) NOT SHOWN FOR CLARITY.

2 PLATE (D) ALTERNATE DETAIL
 N.T.S.



1 DISCONTINUOUS COLUMN DETAIL
 N.T.S.

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 479-783-2450
 www.childersarchitect.com



**WILMA P. MANKILLER HEALTH CENTER
 EXPANSION**
 STILWELL, OKLAHOMA

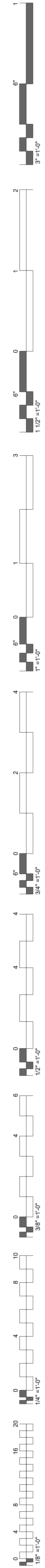
KEY PLAN:
 PROJECT PHASE:
 BID PACKAGE 01

#	DATE	REVISIONS DESCRIPTION

DATE: 11-01-19
 JOB NUMBER: 18-01.01

SHEET NUMBER:
S8.07

SIDEPLATE MISC
 DETAILS AND
 COORDINATION ITEMS

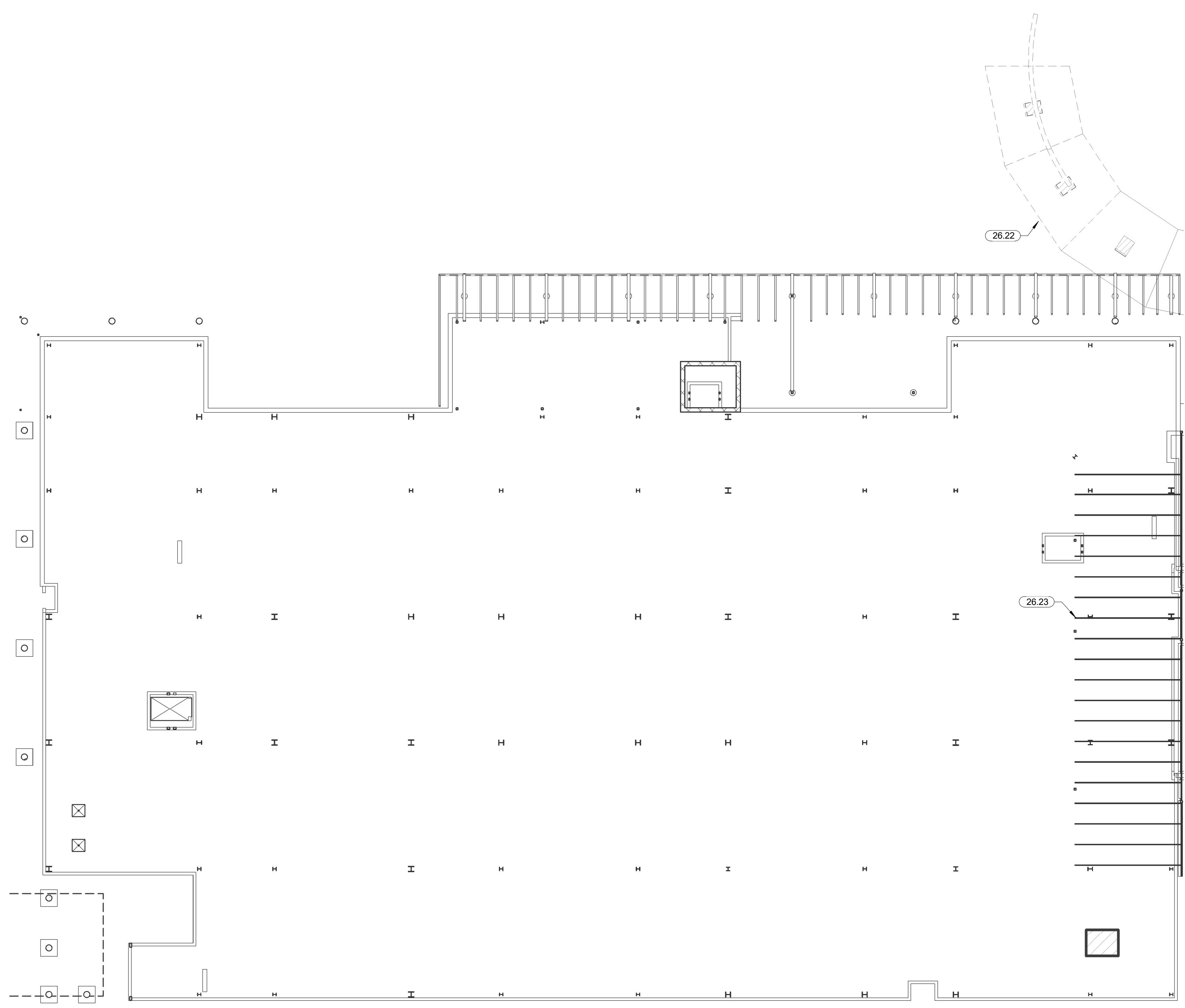


EXISTING ELECTRICAL AND DEMOLITION NOTES

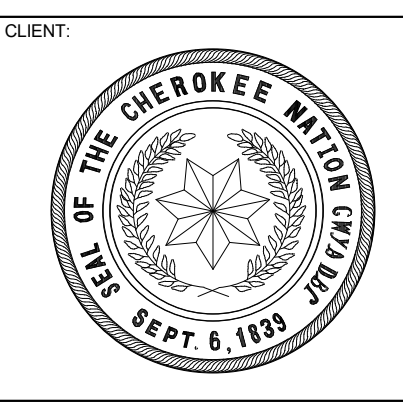
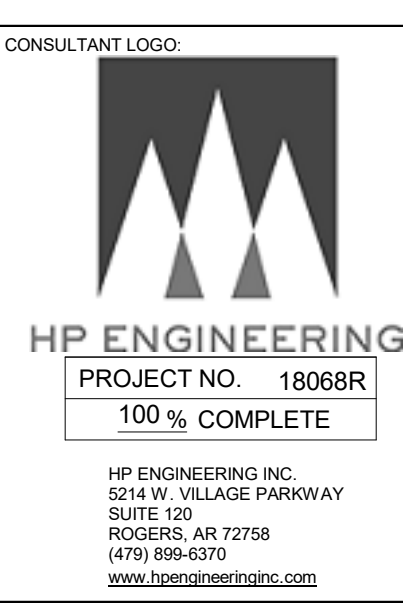
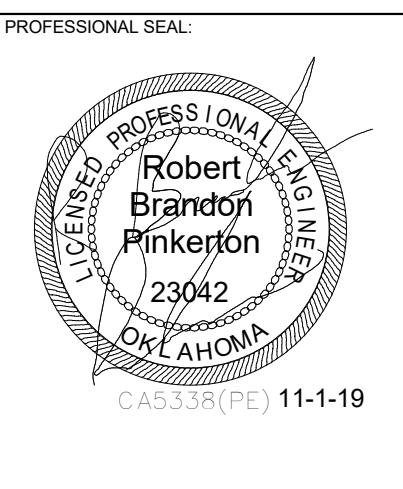
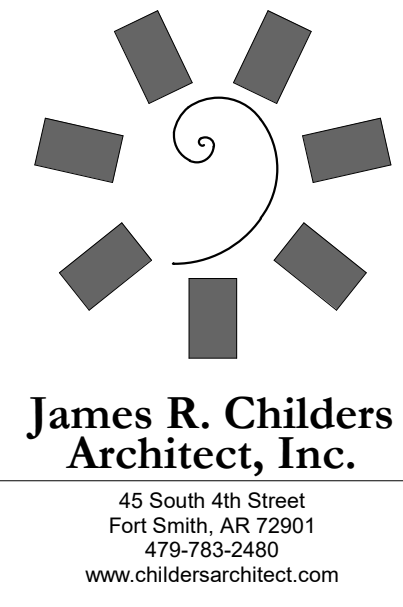
- 1 PRIOR TO SUBMITTING BID, VISIT THE JOB SITE AND BECOME FULLY ACQUAINTED WITH THE EXISTING CONDITIONS OF THE FACILITY AND RELATED SITE. REVIEW THE GENERAL NOTES AND ALL OTHER TRADE DRAWINGS FOR ADDITIONAL REQUIREMENTS THAT MAY NOT BE CALLED OUT IN THIS PORTION OF THE BID PACKAGE. NOTIFY ARCHITECT, ENGINEER OR OWNER, AS SPECIFIED, OF ANY CONFLICTS OR DISCREPANCIES PRIOR TO SUBMITTING BID.
- 2 FIELD VERIFY ALL EXISTING CONDITIONS AND CAREFULLY COORDINATE NEW WORK AND DEMOLITION WITH ALL OTHER DISCIPLINES AND EXISTING CONDITIONS.
- 3 PROVIDE ALL DEMOLITION OF EXISTING ELECTRICAL SYSTEMS. PROVIDE NEW ELECTRICAL SYSTEM MODIFICATIONS REQUIRED BECAUSE OF BUILDING REMODELING, AS NOTED ON THE DRAWINGS, OR NECESSARY FOR PROPER OPERATION AND NEW CONSTRUCTION.
- 4 COORDINATE INTERRUPTION OF ALL BUILDING SERVICES INCLUDING BUT NOT LIMITED TO BRANCH CIRCUITS, DATA, TELEPHONE, ETC WITH BUILDING OWNER PRIOR TO INTERRUPTION. PROVIDE LABOR AND MATERIALS AS REQUIRED TO REDUCE INTERRUPTIONS IN ORDER TO MAINTAIN EXISTING OPERATION.
- 5 PAY SPECIAL ATTENTION NOT TO DAMAGE THE FINISH OF EXISTING WALLS AND CEILINGS THAT ARE TO REMAIN. REPAIR ANY DAMAGE CAUSED DURING WORK AT NO EXTRA COST TO THE OWNER.

KEYNOTES

- 26.22 EXISTING FIXTURES SHALL BE REMOVED, CLEANED, REPAIRED, AND STORED, UNTIL SUCH TIME WHEN THEY CAN BE REINSTALLED IN NEW CANOPY SECTIONS ON NORTH SIDE OF EXISTING TO REMAIN CANOPY.
- 26.23 E.C. SHALL SURVEY EXISTING CONDITIONS AND CONFIRM THAT ALL LIGHTS, FIRE ALARM, SECURITY AND GENERAL POWER CIRCUITS SHALL NOT BE AFFECTED BY BUILDING TO BE DEMOLISHED. IF ANY ISSUES ARE DISCOVERED E.C. SHALL REPAIR OR RELOCATE SO CLINIC WILL NOT BE AFFECTED BY DEMOLITION.



OVERALL ELECTRICAL DEMO PLAN
1/16" = 1'-0"



**WILMA P. MANKILLER HEALTH CENTER
EXPANSION**
STILWELL, OKLAHOMA

KEY PLAN:

PROJECT PHASE:
BID PACKAGE 01

#	DATE	REVISIONS DESCRIPTION

DATE: 11-1-19 JOB NUMBER: 18-01.01

SHEET NUMBER:
DE1.0
OVERALL ELECTRICAL DEMO PLAN

Project Manual

Bid Package 01

Volume I

Divisions 00, Through 14, 31 through 33

Cherokee Nation WILMA P. MANKILLER HEALTH CENTER EXPANSION

Stilwell, Oklahoma

November 01, 2019



Tel: 479.783.2480

Fax: 479.783.4844

E-mail: breck@childersarchitect.com

www.childersarchitect.com

TABLE OF CONTENTS

LEGEND

First Column: **Current Date for Section**
 Second Column: Checked Indicates Section is Included in Current Issue
 Third Column: Section Number
 Fourth Column: Section Title

ISSUES

Bid Package 01 Demolition/Foundation/Steel Nov. 01, 2019

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1. DELETED INFORMATION IS INDICATED BY A STRIKETHROUGH (IE, ~~THIS IS DELETED~~).
2. NEW INFORMATION IS INDICATED BY A DOUBLE UNDERLINE (IE, THIS IS ADDED).
3. ALL REVISED INFORMATION IS FURTHER IDENTIFIED BY A HEAVY VERTICAL LINE TO THE RIGHT OF ALL REVISIONS IN EACH INDIVIDUAL SPECIFICATION SECTION (REFER TO HEAVY BOLD LINE TO THE RIGHT FOR AN EXAMPLE).

VOLUME 1

CIVIL, STRUCTURAL, ARCHITECTURAL



DIVISION 00 - PROCUREMENT AND CONTRACTING REQUIREMENT

	<input type="checkbox"/>	00 1115	Invitation to Bid
	<input type="checkbox"/>	00 2113	Instructions to Bidders
2019-11-01	<input checked="" type="checkbox"/>	00 3100	Available Project Information
	<input type="checkbox"/>	00 4100	Bid Form
	<input type="checkbox"/>	00 5200	Agreement Form
	<input type="checkbox"/>	00 6100	Bonds
2019-11-01	<input checked="" type="checkbox"/>	00 7200	General Conditions
	<input type="checkbox"/>	00 7300	Supplementary Conditions

11.01.2019



11.01.19

DIVISION 01 - GENERAL REQUIREMENTS

	<input type="checkbox"/>	01 0500	Design Selections
	<input type="checkbox"/>	01 0510	Exterior Design Selections
	<input type="checkbox"/>	01 0520	Interior Design Selections
	<input type="checkbox"/>	01 1000	Summary
	<input type="checkbox"/>	01 2100	Allowances
	<input type="checkbox"/>	01 2200	Unit Prices
	<input type="checkbox"/>	01 2300	Alternates



**18-01.01 WPMHC Expansion
 Childers Architect
 2019-11-01**

2019-11-01	<input checked="" type="checkbox"/>	01 2500	Substitution Procedures
2019-11-01	<input checked="" type="checkbox"/>	01 2500a	Substitution Procedure Form
2019-11-01	<input checked="" type="checkbox"/>	01 2600	Contract Modification Procedures
2019-11-01	<input checked="" type="checkbox"/>	01 2900	Payment Procedures
2019-11-01	<input checked="" type="checkbox"/>	01 2900a	Project Cost Summary Form
2019-11-01	<input checked="" type="checkbox"/>	01 3100	Project Management and Coordination
2019-11-01	<input checked="" type="checkbox"/>	01 3200	Construction Progress Documentation
	<input type="checkbox"/>	01 3233	Photographic Documentation
2019-11-01	<input checked="" type="checkbox"/>	01 3300	Submittal Procedures
2019-11-01	<input checked="" type="checkbox"/>	01 4000	Quality Requirements
	<input type="checkbox"/>	01 4200	References
2019-11-01	<input checked="" type="checkbox"/>	01 4323	Special Inspection
2019-11-01	<input checked="" type="checkbox"/>	01 4339	Visual Mock-Up Requirements
2019-11-01	<input checked="" type="checkbox"/>	01 4516	Field Test for Water Leakage
	<input type="checkbox"/>	01 4529	Testing Laboratory Services
	<input checked="" type="checkbox"/>	01 4534	Testing of Piping Systems
2019-11-01	<input checked="" type="checkbox"/>	01 5000	Temporary Facilities and Controls
2019-11-01	<input checked="" type="checkbox"/>	01 6000	Product Requirements
2019-11-01	<input checked="" type="checkbox"/>	01 7300	Execution
2019-11-01	<input checked="" type="checkbox"/>	01 7416	Clean Up (Site Maintenance)
2019-11-01	<input checked="" type="checkbox"/>	01 7419	Construction Waste Management and Disposal
	<input type="checkbox"/>	01 7420	LEED Construction Waste Management and Disposal
2019-11-01	<input checked="" type="checkbox"/>	01 7700	Closeout Procedures
2019-11-01	<input checked="" type="checkbox"/>	01 7823	Operations and Maintenance Data
2019-11-01	<input checked="" type="checkbox"/>	01 7839	Project Record Documents
2019-11-01	<input checked="" type="checkbox"/>	01 7900	Demonstration and Training
	<input type="checkbox"/>	01 8111	Sustainable Construction Requirements
	<input type="checkbox"/>	01 8112	LEED Construction Requirements
	<input type="checkbox"/>	01 8113	LEED Construction Requirements for New Construction and Major Renovations
	<input type="checkbox"/>	01 8123	LEED Construction Requirements for Commercial Interiors
	<input type="checkbox"/>	01 8133	LEED Construction Requirements for Core and Shell Development
	<input type="checkbox"/>	01 8143	LEED Construction Requirements for Schools
2019-11-01	<input checked="" type="checkbox"/>	01 9113	General Commissioning Requirements

DIVISION 02 - EXISTING CONDITIONS

2019-11-01	<input checked="" type="checkbox"/>	02 1113	Demolition
2019-11-01	<input checked="" type="checkbox"/>	02 4116	Building Demolition
2019-11-01	<input checked="" type="checkbox"/>	02 4119	Selective Demolition

DIVISION 03 - CONCRETE

	<input type="checkbox"/>	03 0150	Concrete Patching
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2019-11-01	<input checked="" type="checkbox"/>	03 1000	Concrete Forming and Accessories
	<input type="checkbox"/>	03 1500	Concrete Accessories
2019-11-01	<input checked="" type="checkbox"/>	03 2000	Concrete Reinforcing
	<input type="checkbox"/>	03 2100	Steel Reinforcement (Sidewalk)
2019-11-01	<input checked="" type="checkbox"/>	03 3000	Cast-In-Place Concrete
	<input type="checkbox"/>	03 3053	Cast-in Place Concrete (Site work)
	<input type="checkbox"/>	03 3500	Concrete Finishing
	<input type="checkbox"/>	03 3536	Concrete Finishing (Site work)
	<input type="checkbox"/>	03 3543	Polished Concrete
	<input type="checkbox"/>	03 3600	Special Concrete Finishes
	<input type="checkbox"/>	03 3800	Post-Tensioned Concrete
2019-11-01	<input checked="" type="checkbox"/>	03 4000	Precast Concrete
	<input type="checkbox"/>	03 4500	Architectural Precast Concrete
	<input type="checkbox"/>	03 4713	Tilt-Up Concrete
	<input type="checkbox"/>	03 4900	Glass-Fiber Reinforced Precast Concrete (GFRC)
	<input type="checkbox"/>	03 5216	Lightweight Insulating Concrete
	<input type="checkbox"/>	03 5300	Concrete Toppings
	<input type="checkbox"/>	03 5416	Hydraulic Cement Underlayment

DIVISION 04 – MASONRY

	<input type="checkbox"/>	04 2100	Masonry Veneer
2019-11-01	<input checked="" type="checkbox"/>	04 2200	Reinforced Unit Masonry
	<input type="checkbox"/>	04 2300	Glass Unit Masonry
	<input type="checkbox"/>	04 4200	Exterior Stone Cladding
	<input type="checkbox"/>	04 4216	Steel Supported Stone Cladding
	<input type="checkbox"/>	04 7200	Cast Stone Masonry
	<input type="checkbox"/>	04 7500	Adhered Masonry Veneer

DIVISION 05 – METALS

2019-11-01	<input checked="" type="checkbox"/>	05 1000	Structural Steel
2019-11-01	<input checked="" type="checkbox"/>	05 1213	Architecturally Exposed Structural Steel (AESS)
	<input type="checkbox"/>	05 1636	Barrier Cables
2019-11-01	<input checked="" type="checkbox"/>	05 2100	Steel Joists Framing
	<input type="checkbox"/>	05 5214	Ornamental & Misc. Metals
2019-11-01	<input checked="" type="checkbox"/>	05 3000	Steel Decking
	<input type="checkbox"/>	05 3123	Steel Roof Deck System
	<input type="checkbox"/>	05 3133	Permanent Metal Forming
2019-11-01	<input checked="" type="checkbox"/>	05 4000	Cold-Formed Steel Framing
	<input type="checkbox"/>	05 4300	Slotted Channel Framing
	<input type="checkbox"/>	05 5000	Metal Fabrications
2019-11-01	<input checked="" type="checkbox"/>	05 5100	Metal Stairs
	<input type="checkbox"/>	05 5213	Pipe and Tube Railings
	<input type="checkbox"/>	05 5300	Metal Gratings
	<input type="checkbox"/>	05 5813	Ornamental Metal Column Covers
	<input type="checkbox"/>	05 7000	Ornamental Metal
	<input type="checkbox"/>	05 7300	Ornamental Handrails and Railings

DIVISION 06 - WOOD, PLASTICS, AND COMPOSITES

<input type="checkbox"/>	06 1053	Miscellaneous Rough Carpentry
<input type="checkbox"/>	06 1643	Exterior Gypsum Sheathing
<input type="checkbox"/>	06 4023	Interior Architectural Woodwork
<input type="checkbox"/>	06 4223	Slatwall Paneling
<input type="checkbox"/>	06 6100	Simulated Stone Fabrications
<input type="checkbox"/>	06 6400	Plastic (FRP) Paneling
<input type="checkbox"/>	06 6413	Translucent Resin Panel Fabrications
<input type="checkbox"/>	06 6419	Simulated Stone Paneling
<input type="checkbox"/>	06 6713	Louvered Light Diffusers
<input type="checkbox"/>	06 6813	Plastic Gratings

DIVISION 07 - THERMAL AND MOISTURE PROTECTION

	<input type="checkbox"/>	07 0151	Preparation for Re-Roofing
	<input type="checkbox"/>	07 0152	Patching of Existing Roofing
	<input type="checkbox"/>	07 1114	Asphalt Mastic Dampproofing
	<input type="checkbox"/>	07 1328	Pre-Applied Sheet Waterproofing
	<input type="checkbox"/>	07 1352	Modified Bituminous Sheet Waterproofing
	<input type="checkbox"/>	07 1413	Hot Fluid-Applied Rubberized Asphalt Waterproofing
	<input type="checkbox"/>	07 1416	Cold Fluid Applied Waterproofing
	<input type="checkbox"/>	07 1616	Crystalline Waterproofing
	<input type="checkbox"/>	07 1700	Bentonite Waterproofing
	<input type="checkbox"/>	07 1800	Traffic Coatings
	<input type="checkbox"/>	07 1900	Water Repellents
	<input type="checkbox"/>	07 2100	Thermal Insulation
	<input type="checkbox"/>	07 2119	Spray-Applied Foam Insulation
	<input type="checkbox"/>	07 2400	EIFS
	<input type="checkbox"/>	07 2423	DEFS for Soffits
	<input type="checkbox"/>	07 2500	Mechanically Fastened Air and Water Barriers
2019-11-01	<input checked="" type="checkbox"/>	07 2600	Under-Slab Vapor Retarder
	<input type="checkbox"/>	07 2613	Rubberized Asphalt Vapor Retarders
	<input type="checkbox"/>	07 2713	Self-Adhering Air and Water Barriers
	<input type="checkbox"/>	07 3113	Asphalt Shingles
	<input type="checkbox"/>	07 3127	Simulated Slate Roofing
	<input type="checkbox"/>	07 3200	Roof Tiles
	<input type="checkbox"/>	07 4114	Metal Roof Panels
	<input type="checkbox"/>	07 4213	Formed Metal Wall Panels
	<input type="checkbox"/>	07 4229	Terra Cotta Wall Panels
	<input type="checkbox"/>	07 4243	Composite Metal Wall Panels
	<input type="checkbox"/>	07 4263	Insulated-Core Metal Wall Panels
	<input type="checkbox"/>	07 5013	Single-Ply Membrane Roofing
	<input type="checkbox"/>	07 5216	Modified Bituminous Membrane Roofing
	<input type="checkbox"/>	07 5556	Fluid-Applied Protected Membrane Roofing
	<input type="checkbox"/>	07 5563	Vegetated Protected Membrane Roofing
	<input type="checkbox"/>	07 6200	Flashing and Sheet Metal
	<input type="checkbox"/>	07 7200	Roof Accessories
	<input type="checkbox"/>	07 7600	Roof Pavers and Pedestal Assemblies

<input type="checkbox"/>	07 8116	Cementitious Fireproofing
<input type="checkbox"/>	07 8123	Intumescent Mastic Fireproofing
<input type="checkbox"/>	07 8413	Penetration Firestopping
<input type="checkbox"/>	07 8446	Fire-Resistive Joint Firestopping
<input type="checkbox"/>	07 9100	Preformed Joint Seals
<input type="checkbox"/>	07 9200	Joint Sealants
<input type="checkbox"/>	07 9500	Expansion Control

DIVISION 08 - OPENINGS

<input type="checkbox"/>	08 0610	Door Schedule
<input type="checkbox"/>	08 1113	Hollow Metal Doors and Frames
<input type="checkbox"/>	08 1114	Interior Hollow Metal Frames
<input type="checkbox"/>	08 1170	Steel Fire Door and Frame Assembly
<input type="checkbox"/>	08 1216	Interior Aluminum Frames
<input type="checkbox"/>	08 1416	Prefinished Flush Wood Doors
<input type="checkbox"/>	08 1433	Stile and Rail Wood Doors
<input type="checkbox"/>	08 3113	Access Doors and Frames
<input type="checkbox"/>	08 3213	Sliding Aluminum-Framed Glass Doors
<input type="checkbox"/>	08 3313	Coiling Counter Doors
<input type="checkbox"/>	08 3323	Overhead Coiling Doors
<input type="checkbox"/>	08 3326	Overhead Coiling Grilles
<input type="checkbox"/>	08 3338	Interior Side Coiling Grilles
<input type="checkbox"/>	08 3400	Special Function Doors
<input type="checkbox"/>	08 3513	Folding Doors
<input type="checkbox"/>	08 3515	Accordion Folding Fire Doors
<input type="checkbox"/>	08 3613	Sectional Overhead Doors
<input type="checkbox"/>	08 4110	Interior Storefront
<input type="checkbox"/>	08 4127	Exterior All-Glass Entrances and Storefronts
<input type="checkbox"/>	08 4128	Interior All-Glass Entrances and Storefronts
<input type="checkbox"/>	08 4213	Exterior Aluminum Entrance Doors
<input type="checkbox"/>	08 4216	Interior Aluminum Entrance Doors
<input type="checkbox"/>	08 4229	Automatic Entrances
<input type="checkbox"/>	08 4233	Revolving Entrance Doors
<input type="checkbox"/>	08 4243	Medical Specialty Sliding Entrances
<input type="checkbox"/>	08 4400	Glazed Aluminum Framing Systems
<input type="checkbox"/>	08 4426	Structural Glass Curtainwall
<input type="checkbox"/>	08 4500	Translucent Insulating Panel Assemblies
<input type="checkbox"/>	08 5113	Aluminum Windows
<input type="checkbox"/>	08 5619	Sliding Pass Windows
<input type="checkbox"/>	08 5656	Bullet-Resistive Windows
<input type="checkbox"/>	08 6200	Unit Skylights
<input type="checkbox"/>	08 6300	Metal-Framed Skylights
<input type="checkbox"/>	08 7100	Door Hardware
<input type="checkbox"/>	08 7121	Interior Automatic Door Operators for Staff Use
<input type="checkbox"/>	08 7122	Automatic Door Operators for the Disabled
<input type="checkbox"/>	08 8000	Glazing
<input type="checkbox"/>	08 8300	Unframed Mirrored Glazing
<input type="checkbox"/>	08 8816	Between Glass Blinds Units
<input type="checkbox"/>	08 8840	Switchable Privacy Glass Units
<input type="checkbox"/>	08 9100	Wall Louvers

DIVISION 09 – FINISHES

<input type="checkbox"/>	09 0565	Floor Preparation for Renovation Work
<input type="checkbox"/>	09 0600	Room Finish Schedule
<input type="checkbox"/>	09 2300	Gypsum Plastering
<input type="checkbox"/>	09 2400	Portland Cement Plastering
<input type="checkbox"/>	09 2600	Veneer Plastering
<input type="checkbox"/>	09 2613	Gypsum Veneer Plastering
<input type="checkbox"/>	09 2713	GFRG Fabrications
<input type="checkbox"/>	09 2900	Gypsum Board Assemblies
<input type="checkbox"/>	09 3000	Tiling
<input type="checkbox"/>	09 5113	Acoustical Panel Ceilings
<input type="checkbox"/>	09 5133	Acoustical Metal Pan Ceilings
<input type="checkbox"/>	09 5135	Snap-in Metal Pan Ceilings
<input type="checkbox"/>	09 5423	Linear Metal Ceilings
<input type="checkbox"/>	09 5436	Suspended Decorative Grids
<input type="checkbox"/>	09 5451	Linear Wood Wall and Ceiling Systems
<input type="checkbox"/>	09 6115	Concrete Floor Sealer
<input type="checkbox"/>	09 6116	Liquid Floor Hardener
<input type="checkbox"/>	09 6119	Moisture Floor Treatment
<input type="checkbox"/>	09 6340	Stone Flooring
<input type="checkbox"/>	09 6400	Wood Flooring
<input type="checkbox"/>	09 6500	Resilient Flooring
<input type="checkbox"/>	09 6513	Resilient Base and Accessories
<input type="checkbox"/>	09 6520	Interlocking Rubber Tile Flooring
<input type="checkbox"/>	09 6566	Resilient Athletic Flooring
<input type="checkbox"/>	09 6603	Precast Terrazzo Flooring for Stairs
<input type="checkbox"/>	09 6613	Thick-Set Terrazzo Flooring
<input type="checkbox"/>	09 6623	Thin-Set Terrazzo Flooring
<input type="checkbox"/>	09 6723	Resinous Flooring
<input type="checkbox"/>	09 6800	Carpeting
<input type="checkbox"/>	09 6900	Access Flooring
<input type="checkbox"/>	09 7200	Wall Covering
<input type="checkbox"/>	09 7213	Tackboard Wall Coverings
<input type="checkbox"/>	09 7500	Interior Stone Facing
<input type="checkbox"/>	09 7723	Fabric Wrapped Panels
<input type="checkbox"/>	09 8433	Acoustical Wall Panels
<input type="checkbox"/>	09 9100	Painting
<input type="checkbox"/>	09 9413	Textured Interior Coatings
<input type="checkbox"/>	09 9600	High-Performance Coatings
<input type="checkbox"/>	09 9613	Multicolored Interior Coatings
<input type="checkbox"/>	09 9653	Elastomeric Coatings
<input type="checkbox"/>	09 9663	Textured Acrylic Coating

DIVISION 10 - SPECIALTIES

<input type="checkbox"/>	10 1100	Visual Display Boards
<input type="checkbox"/>	10 1146	Visual Display Fabrics
<input type="checkbox"/>	10 1400	Interior Signage

- 10 1443 Photoluminescent Exit Path Marking System
- 10 1700 Telephone Specialties
- 10 2113 Toilet Compartments
- 10 2115 Cubicle Specialties
- 10 2213 Wire Mesh Partitions
- 10 2223 Accordion Folding Partitions
- 10 2238 Operable Panel Partition
- 10 2239 Vertically Folding Panel Partitions
- 10 2613 Wall and Corner Guards
- 10 2813 Toilet Accessories
- 10 2819 Shower Doors and Enclosures
- 10 4116 Emergency Key Cabinets
- 10 4400 Fire Protection Specialties
- 10 4450 Automated External Defibrillators (AED)
- 10 5113 Metal Lockers
- 10 5116 Wood Lockers
- 10 5503 USPS-Delivery Postal Specialties
- 10 5506 Private-Delivery Postal Specialties
- 10 5713 Wall Mounted Coat Rack and Shelf
- 10 7113 Exterior Sun Control Devices
- 10 7310 Aluminum Walkways and Canopies

- 10 7500 Flagpoles

DIVISION 11 - EQUIPMENT

- 11 1300 Loading Dock Equipment
- 11 2400 Building Maintenance Equipment
- 11 5213 Projection Screens
- 11 7000 Medical Equipment
- 11 7313 Wall-Mounted Fold-Up Writing Surface
- 11 7316 Wall-Mounted Chart Rack

DIVISION 12 - FURNISHINGS

- 12 2113 Horizontal Louver Blinds
- 12 2116 Vertical Louver Blinds
- 12 2413 Roller Window Shades
- 12 2500 Between Glass Blinds
- 12 3553 Laboratory Casework
- 12 3571 Stainless Steel Casework
- 12 3640 Stone Countertops
- 12 3661 Simulated Stone Countertops
- 12 4816 Entrance Floor Grilles
- 12 4843 Entrance Floor Mats
- 12 6300 Stadium Seating
- 12 9313 Bicycle Racks

DIVISION 13 - SPECIAL CONSTRUCTION

- 13 2817 Ballpark Netting and Supports
- 13 3448 Pre-Fabricated Rooftop Helipad

- 13 4900 Radiation Protection
- 13 4923 RF/MRI Modular Shielding Enclosure

DIVISION 14 - CONVEYING EQUIPMENT

- 14 1000 Dumbwaiters
- 14 2100 Electric Traction Elevators
- 14 2400 Hydraulic Elevators
- 14 3100 Escalators
- 14 9100 Chutes
- 14 9200 Pneumatic Tube Systems

DIVISION 31 - EARTHWORK

- 2019-11-01 31 1100 Cleaning and Grubbing
- 31 2119 Site Grading
- 2019-11-01 31 2300 Excavation & Fill
- 2019-11-01 31 2311 Earthwork for Building Construction
- 31 2333 Trenching
- 2019-11-01 31 2500 Erosion Control
- 2019-11-01 31 2573 Temporary Silt Fence
- 31 3116 Termite Control
- 31 4134 Excavation/Trench & Shore

- 31 6218 Mini-Piles
- 2019-11-01 31 6613 Aggregate Piers

DIVISION 32 - EXTERIOR IMPROVEMENTS

- 32 1123 Aggregate Base Course
- 32 1313 Concrete Paving
- 32 1413 Interlocking Precast Concrete Paving
- 32 1416 Brick unit Paving
- 32 1440 Stone Paving
- 32 1613 Concrete Curb & gutters
- 32 1614 Concrete Side Walk

- 32 1715 Parking Accessories
- 32 3113 Chain Link Fencing
- 32 3115 Tubular Steel Fencing
- 32 3117 Gate Operators
- 32 3121 Cable Guardrail System

DIVISION 33 - UTILITIES

- 33 0516 Manholes Vaults
- 33 0526 Utility Line Marking
- 33 0527 Connection to Existing Utilities
- 33 0533 Plastic Pipe (water & San. Swr.)
- 33 1113 HDPE Potable Water Pipe
- 33 1216 Valves

<input type="checkbox"/>	33 1219	Hydrants
<input type="checkbox"/>	33 1300	Disinfection of Waterlines
<input type="checkbox"/>	33 4100	Storm Drainage

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SECTION 00 3100

AVAILABLE PROJECT INFORMATION

PART 1 - GENERAL

1.1 SUMMARY

- A. This Section references other information relevant to the construction of this Project that is available project information.
- B. At the request of the Owner, the information identified below represents services that have been provided by others, not as an Architect's Consultant, regarding conditions that affect this Project that are beyond the responsibilities of the Architect and Architect's Consultants. Reference to such information herein is solely for the convenience of the Owner. Architect makes no representation, express or implied, as to the accuracy or validity of the information.
- C. Bidders are expected to examine the site and the information available from the Owner to determine for themselves the conditions to be encountered.
- D. If conditions other than those indicated in the information available from the Owner are encountered before or during construction, notify the Owner before work continues.

1.2 PROPERTY SURVEY

- A. The Owner's Surveyor has performed a property survey and the some of the survey information is included on the Drawings for the convenience of the Contractor.

1.3 GEOTECHNICAL REPORT

- A. The Owner's Geotechnical Consultant has made subsurface borings at the Project site, has performed an investigation of the geotechnical conditions, and has prepared a report of the investigation that contains specific requirements of the Contractor.
- B. A copy is being provided as a separate volume.

The Owner's Demolition Consultant has determined demolition provisions for the Owner's existing property and has prepared documents indicating demolition requirements.

1.4 MEDICAL EQUIPMENT DOCUMENTS

- A. The Owner's Medical Equipment Planning Consultant has developed medical equipment documents for the Owner's facility and has prepared construction documents.
- B. Copies of the documents have been included in the Contract Documents prepared by the Architect for the convenience of the Owner Contractor

1.5 DEMOLITION CONTRACT DOCUMENTS

- A. The Owner's Demolition Consultant has determined demolition provisions for the Owner's existing property and has prepared documents indicating demolition requirements.

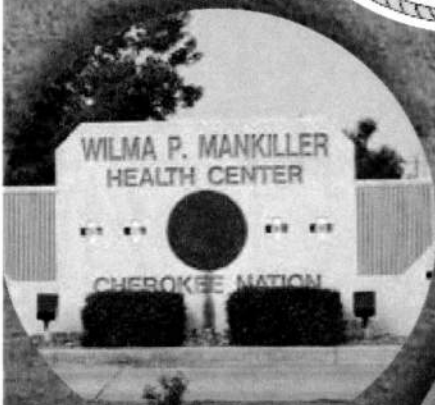
1.6 TAX EXEMPTION STATUS INFORMATION

- A. This Project qualifies for exemption from taxes as indicated in the letter from [], dated [], that follows this Section.

PART 2 - PRODUCTS (NOT USED)

PART 3 - EXECUTION (NOT USED)

END OF SECTION



REPORT OF SUBSURFACE EXPLORATION
AND PRELIMINARY GEOTECHNICAL EVALUATION
WILMA P MANKILLER CLINIC RENOVATIONS
STILWELL, OKLAHOMA
BUILDING & EARTH PROJECT NO.: OK180172

PREPARED FOR:



AUGUST 30, 2018



Geotechnical, Environmental, and Materials Engineers



Geotechnical, Environmental, and Materials Engineers

1403 South 70th East Avenue
Tulsa, OK 74112
Ph: (918) 439-9005
www.BuildingAndEarth.com

August 30, 2018



45 South 4th Street
Fort Smith, Arkansas 72901

Attention: Mr. Shane Boren, LEED AP, Associate AIA

Subject: Report of Subsurface Exploration and **Preliminary** Geotechnical Evaluation
Wilma P Mankiller Clinic Renovations
Stilwell, Oklahoma
Building & Earth Project No: OK180172

Dear Mr. Boren:

Building & Earth Sciences, Inc. has completed the authorized subsurface exploration and preliminary geotechnical engineering evaluation for the proposed renovations to the Wilma P Mankiller Clinic in Stilwell, Oklahoma.

It should be emphasized that the geotechnical recommendations presented in this report are to be considered preliminary until Building & Earth has had the opportunity to review civil and structural design development (DD) plans. Depending on the site plans, and grading and structural information, additional geotechnical analysis may be warranted in order to present final geotechnical recommendations prior to issuing 100% construction plans, signed and sealed by professional engineers registered in the state of Oklahoma.

The purpose of this exploration and evaluation was to determine general subsurface conditions at the site and to address applicable geotechnical aspects of the proposed construction and site development. The recommendations in this report are based on a physical reconnaissance of the site and observation and classification of samples obtained from five (5) borings. Confirmation of the anticipated subsurface conditions during construction is an essential part of geotechnical services.

We appreciate the opportunity to provide consultation services for the proposed project. If you have any questions regarding the information in this report or need any additional information, please call us.

Respectfully Submitted,

BUILDING & EARTH SCIENCES, INC.

Certificate of Authorization, #3975, Expires 6/30/2020

Dharmateja Maganti, E.I.
Project Manager

Marco V. Vicente Silvestre, P.E.
Regional Vice President (OK: 21903)



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APPENDIX

1.0 PROJECT & SITE DESCRIPTION

The project site is located about 1500 feet east from the intersection of Oklahoma 51 and Flint Street in Stilwell, Oklahoma. General information relative to the proposed site and the proposed development is listed in Table 1 below. Photographs depicting the current site condition are presented on the following page.

Development Item	Detail	Description
General Site	Size (Ac.)	~ 2 Acres
	Existing Development	Existing clinic building, pavement roads, parking lots, and concrete sidewalks,
	Vegetation	Some landscaped areas consisting of grass, shrubs, and small trees to the north of existing clinic building. Heavy tree cover was noted to the south and southwest.
	Slopes	To the north of the existing building – the site slopes down to the north towards the existing parking lot with a grade differential of approximately 5 feet. On the south of the building, the ground surface slopes up towards the south
	Retaining Walls	Along the east side of the clinic and access drive
	Drainage	Concrete line drainage channel to the east, west, and south
	Cuts & Fills	Fill materials ranging in thickness from 4 to 8 feet
Proposed Buildings	No. of Bldgs	One (1)
	Square Ft.	40,000 square feet (SF)
	Stories	Two (2)
	Construction	Steel framed with a masonry veneer (assumed)
	Column Loads	Up to 250 kips (provided)
	Wall Loads	0.5 to 3 kips per linear foot (klf)
	Preferred Foundation	Shallow foundations
	Preferred Slab	Slab-on-grade
Proposed Pavements	Traffic	Not provided, assumed passenger and maintenance vehicles, ambulances, delivery trucks, and garbage collection trucks
	Pavement Type	Rigid Pavement Sections

Table 1: Project and Site Description

References:

- Mankiller Geotechnical Services request document and proposed test hole locations document, prepared by Chavez-Grieves Consulting Engineers, Inc.
- A geotechnical report for Wilma P Mankiller Clinic Addition, prepared by Building & Earth (project No: OK13100, dated July 14, 2013)

Notes:

1. If final loading conditions exceed preliminary given loads, Building & Earth Sciences should be allowed to review the proposed structural design and its effects on our recommendations for foundation design.
2. When a grading plan is finalized, Building & Earth should be allowed to review the plan and its effects on our recommendations.
3. It should be emphasized that the geotechnical recommendations presented in this report are to be considered preliminary until Building & Earth has had the opportunity to review civil and structural design development (DD) plans. Depending on the site plans, and grading and structural information, additional geotechnical analysis may be warranted in order to present final geotechnical recommendations prior to issuing 100% construction plans, signed and sealed by professional engineers registered in the state of Oklahoma.



Figure 1: Google Earth Satellite Image, dated 10/27/2017

Based on the information provided to our office, we understand proposed renovations will include demolition of the west section of the existing building to make way for construction of a new two-story structure.

At the time of the subsurface exploration the proposed construction area was covered with concrete pavements, landscaping areas, and grass. Limestone outcroppings were noted to the south of the existing building. The area to the south of the existing building is relatively flat to where a concrete drainage channel is present and then slopes up towards a heavily wooded area beyond. The area to the north of the existing building slopes down towards an existing parking lot.

Dumpster and equipment pads were noted to the west of the existing building. A concrete drainage channel was noted to the east, west, and south of the existing building and pavement areas. Underground utility markings were noted with the west section of the project area.



Figure 2: Photo looking east, north of the building (image dated 7/25/18)



Figure 3: Photo looking east, south of the building (image dated 7/25/18)

2.0 SCOPE OF SERVICES

The authorized subsurface exploration was performed on July 25, 2018 in conformance with our proposal OK20267, dated July 19, 2018. Notice to proceed was provided by signing and returning our proposal document on July 20, 2018.

The purpose of the geotechnical exploration was to determine general subsurface conditions at specific boring locations and to gather data on which to base a geotechnical evaluation with respect to the proposed construction. The subsurface exploration for this project consisted of five (5) test borings. The site was drilled using an ATV-mounted CME-550 drill rig equipped with hollow stem augers and an automatic hammer.

The boring locations were determined in the field by a representative of our staff using a handheld GPS device. As such, the boring locations shown on the Boring Location Plan attached to this report should be considered approximate.

The samples recovered during our site investigation were visually classified and specific samples were selected by the project engineer for laboratory analysis. The laboratory analysis consisted of:

Test	ASTM	No. of Tests
Natural Moisture Content	D2216	13
Atterberg Limits	D4318	3
Material Finer Than No. 200 Sieve by Washing	D1140	1
Loss of Ignition Test	D2974	1

Table 2: Scope of Laboratory Tests

The results of the laboratory analysis are presented on the enclosed Boring Logs and in tabular form in the Appendix of this report. Descriptions of the laboratory tests that were performed are also included in the Appendix.

The information gathered from the exploration was evaluated to determine a suitable foundation type for the proposed structure. The information was also evaluated to help determine if any special subgrade preparation procedures will be required during the earthwork phase of the project.

The results of the work are presented within this report that addresses:

- Site geology.
- Summary of existing surface conditions.
- A description of the subsurface conditions encountered at the boring locations.
- A description of the groundwater conditions observed in the boreholes during drilling. Long-term monitoring was not included in the scope of work.
- Presentation of laboratory test results.
- Site preparation considerations including material types to be expected at the site, treatment of any encountered unsuitable soils, excavation considerations, and surface drainage.
- Recommendations to be used for foundation design, including appropriate foundation types, bearing pressures, and depths
- Presentation of expected total and differential settlements.
- Recommendations to be used for design of slabs-on-grade, including modulus of subgrade reaction.
- Recommendations to be used for design of retaining walls, including lateral earth pressures and coefficient of friction.
- Seismic Site Classification per IBC 2015 based on SPT test boring information only.
- Compaction requirements and recommended criteria to establish suitable material for structural backfill.
- Recommended typical minimum rigid pavement sections for assumed standard-duty and heavy-duty traffic conditions

3.0 GEOTECHNICAL SITE CHARACTERIZATION

The following discussion is intended to create a general understanding of the site from a geotechnical engineering perspective. It is not intended to be a discussion of every potential geotechnical issue that may arise, nor to provide every possible interpretation of the conditions identified. The following conditions and subsequent recommendations assume that significant changes in subsurface conditions do not occur between boreholes. However, anomalous conditions can occur due to variations in existing fill that is present at the site, or the geologic conditions at the site, and it will be necessary to evaluate the assumed conditions during site grading and foundation installation.

It should be understood that borings drilled to date were located around the existing structure that eventually will be demolished prior to start of new construction. The geotechnical recommendations assume that subsurface conditions similar to those encountered in the borings will be present within the footprint of the existing building. It is recommended that the geotechnical engineer evaluate the subgrade following demolition of the structure to verify the assumption. Unforeseen conditions may be encountered during construction when subsurface conditions within the existing building area differ from those encountered in the borings.

3.1 GEOLOGY

Based on published geologic literature, the project site is underlain by the early and middle Mississippian age Boone formation. The Boone Formation consists of gray, fine to coarse-grained fossiliferous limestone inter-bedded with chert and minor shale. Some sections of the formation may be predominantly limestone or chert. The chert is dark in color in the lower part and light in color in the upper part. The quantity of chert varies considerably both vertically and horizontally. The Boone Formation is well known for dissolutional features, such as sinkholes, caves, and enlarged fissures.

The subsurface conditions encountered at the boring locations correlate with the published site geology.

3.2 EXISTING SURFACE CONDITIONS

At the time of our subsurface exploration, the area proposed for construction was covered with grass to the north and south sides of the existing building, concrete paving in existing parking lot and drive areas, and some shrubs and trees were noted in grass areas.

Approximately 4 to 8 inches of topsoil was encountered in borings B-01, B-02, B-04, and B-05. The surface conditions apply only to the specific boring locations. It should be noted that topsoil conditions likely vary at unexplored locations of the project site. No testing has been performed to verify that soils meet the requirements of "topsoil". For the purpose of this report, topsoil is defined as the soil horizon which contains the root mat of the noted vegetation.

Boring B-03 drilled within the existing pavement encountered concrete with a thickness of about 3 inches. Aggregate base encountered below the concrete pavement was to be about 3 inches in thickness.

3.3 SUBSURFACE CONDITIONS

A generalized stratification summary, tabulated below, has been prepared using data from the test borings. This summary depicts general soil and rock conditions and strata types encountered during the field investigation.

Stratum No.	Typical Thickness	Description	Consistency
1	3.5' to 7.3'	Fill and Possible Fill consisting primarily of lean clays, lean to fat clays with varying amount and size of concrete and limestone fragments	Typically exhibited stiff to very stiff consistencies, with N-values ranging between 9 and 20. N-values ranging between 44 and 67 blows for 11 inches of penetration were recorded in Borings B-04 and B-05. Clay fill in Boring B-02 at a depth of about 2.5 feet exhibited medium stiff consistency (N-value of 6).
2	4.5' to 7.5'	Residual soils consisting of lean to fat clays (Borings B-01 and B-02 only)	Stiff to very stiff clays, with N-values of 8 and 16. An N-value of 23 (very stiff to hard consistency) was recorded in B-02 at depth of about 8.5 feet.
3	Termination Layer ¹	Boone Formation: Limestone (With exception of B-04)	Rock unit: moderately hard to hard

Table 3: Stratification Summary

Notes:

1. Auger refusal was encountered in all borings on limestone and apparent limestone unit at depths ranging between 4 and 13 feet.

For specific details on the information obtained from individual borings, refer to the Boring Logs included in the Appendix.

3.3.1 FILL AND POSSIBLE FILL

Fill and possible fill consisting of lean clays and lean to fat clays were noted below the topsoil and existing pavements. The fill and possible fill extended to depths ranging from about 4 to 8 feet below the ground surface. The fill soils were various shades and combinations of brown, gray, dark brown, dark grayish brown, brownish gray, dark gray, and bluish gray in color. The fill soils contained fine roots and fragments of concrete and limestone. The limestone encountered in borings B-04 and B-05 ranged in size from gravel to cobbles/boulders.

Fill encountered in the borings typically exhibited stiff to very stiff consistencies, with N values ranging between 9 and 20. The fill in Boring B-02 at a depth of about 2.5 feet exhibited a medium stiff consistency with an N-value of 6.

N-values ranging between 44 and 67 blows for 11 inches of penetration were recorded in Borings B-04 and B-05. The high blow counts are likely due to the presence of broken concrete rubble and limestone cobbles/boulders.

The clay fill typically exhibited moderate to high plasticity characteristics. Representative samples of the clay fill material selected for Atterberg Limits testing indicated liquid limit (LL) values ranging from 34 to 47, plastic limit (PL) values of 17 and 21, and plasticity index (PI) values ranging from 17 to 26. Sieve analysis performed on a select sample indicated approximately 70 percent fines (silt and clay fraction). Measured moisture contents of the fill and possible fill materials typically ranged from 8 to 22 percent.

3.3.2 RESIDUAL CLAY SOILS

Residual clay soils were encountered below the existing fill and possible fill materials in borings B-01 and B-02 and extended to a depth of about 12.5 feet below current grades. The residual soils consisted of lean to fat clays that were dark brown, brown, gray, yellowish brown, reddish brown, and some dark red in color. The residual clays exhibited stiff to hard consistencies, with N-values ranging from 8 to 23. Natural moisture contents typically ranged from 19 to 27 percent.

3.3.3 BOONE FORMATION

A limestone unit was encountered in the borings below the existing fill materials and residual clay soils at depths of about 4 to 12.5 feet below current grades. Limestone unit was light gray to gray in color and extended to boring termination depths of 4 to 13 feet. SPT N-values in the limestone unit ranged from 50 blows for 0.5 to 4 inches of penetration.

3.3.4 AUGER REFUSAL

Auger refusal is the drilling depth at which the borehole can no longer be advanced using soil drilling procedures. Auger refusal was encountered on the limestone unit. The following table summarizes the auger refusal depths.

Boring No.	Depth (ft)
B-01	13.0
B-02	13.0
B-03	4.0
B-04	5.5
B-05	9.0

Table 4: Auger Refusal Depths

3.3.5 GROUNDWATER

Groundwater was not encountered in the borings while drilling and the borings were dry upon completion of drilling and prior to backfilling. Fluctuations in the water level can occur due to seasonal rainfall. Water levels reported are accurate only for the time and date that the borings were drilled. Long term monitoring of the boreholes was not included as part of our subsurface exploration. The borings were backfilled the same day that they were drilled.

3.4 SEISMIC SITE CLASSIFICATION

Basis of Evaluation	Recommended Site Classification
2015 International Building Code (IBC) and ASCE 7, Chapter 20	C
This recommended seismic site classification is based on the 2015 Edition of the International Building Code, the subsurface conditions encountered in the borings, and our knowledge of the geologic conditions of the site. Our subsurface exploration extended to a maximum depth of about 13 feet; hence the seismic site classification should be re-evaluated in the event subsurface information is made available to a depth of 100 feet.	

Table 5: Seismic Site Classification

4.0 SITE DEVELOPMENT CONSIDERATIONS

Based on the information provided to our office, we understand that the finished floor elevation of the proposed two-story building will be relatively close to that of the existing building which will be demolished. We also understand that the footprint of the proposed building may be slightly expanded to the north, south, and west of the existing building.

A grading plan was not available at the time of this report. For the purpose of this report, we assume that fill depths on the order of up to 5 feet may be required to achieve design grades within the north portion of the planned building and minimal grade changes will be required within south and west portions.

When final site and grading plans become available, it will be necessary for Building & Earth to review the recommendations presented in this report to determine if any additional considerations would be appropriate based on the final grading and layout scheme.

Based on our evaluation of the subsurface conditions, and the provided foundation loads, we recommend the structure be constructed with a conventional shallow foundation after implementation of ground improvement measures using installation of stone columns. Site development recommendations presented below have been prepared to address foundation support using this type of system.

If a different type of foundation system is preferred, Building & Earth should be allowed to review the site development recommendations to verify they remain appropriate for the preferred foundation system.

Primary geotechnical considerations, identified by Building & Earth, affecting this project include:

- Existing clinic building and associated pavements, and concrete sidewalk were present within the planned building area at the time of our subsurface exploration.
- Fill materials were encountered throughout the project site, extending to depths of approximately 4 to 8 feet below existing ground surface. Broken concrete and possibly limestone fragments ranging in size from gravel to boulders were noted within the fill in borings B-04 and B-05.
- Portions of the onsite fill materials and underlying residual clay soils exhibited moderate to high plasticity characteristics that have a moderate to high shrink-swell potential.
- Clay fill encountered at depth of 2.5 feet in boring B-02 exhibited a medium stiff consistency.
- Limestone was encountered at depths ranging from 4 feet at the west side, 5 to 6.5 feet on the south side, and 12.5 feet on the north side.

Recommendations addressing the site conditions are presented in the following sections.

4.1 INITIAL SITE PREPARATION

The initial site preparation should commence with demolition of the existing building and pavements. All slabs, footings, below grade walls (if any), underground utility lines, pavement materials, etc. associated with the existing development should be removed from the proposed construction area prior to any fill placement or new construction. Soils disturbed during the process should be undercut to undisturbed material and replaced with structural fill.

All trees, vegetation, roots, topsoil, any other deleterious materials, and existing pavement materials should be removed from the proposed construction areas. Approximately 4 to 8 inches of topsoil and 3 inches of concrete were observed in the borings; however, topsoil and pavement thickness could extend to greater depths in unexplored areas of the site. Grubbing of trees should include removal of the tree stumps and the root systems. Desiccated clay soils may be present in the zone surrounding the trees. Desiccated clay soils should be undercut and replaced with structural fill.

Materials disturbed during clearing operations should be stabilized in place or, if necessary, undercut to undisturbed materials and backfilled with properly compacted, approved structural fill.

During site preparation activities, the contractor should identify borrow source materials that will be used as structural fill and provide samples to the testing laboratory so that conformance to the Structural Fill requirements outlined below and appropriate moisture-density relationship curves can be determined.

4.2 BUILDING PAD PREPARATION

Fill materials encountered in the borings exhibited moderate to high plasticity characteristics with a moderate to high shrink-swell potential. Standard Penetration (SPT) N values recorded within the fill ranged from 6 to SPT refusal of 67 blows for 11 inches of penetration. Concrete rubble and limestone cobbles/boulders were encountered within the fill in borings B-04 and B-05. In addition to the heterogenous composition of the fill, its thickness ranged from 4 to 8 feet.

On the north side of the building, the fill was underlain by residual clay soils that continued depth of about 12.5 feet, whereas on the west and south sides the fill was directly underlain by limestone at depths of 5 to 6.5 feet.

4.2.1 SOIL IMPROVEMENT USING STONE COLUMNS

Taking into consideration the above described subsurface conditions and the given structural loads for the proposed building, it is our opinion that the fill and residual soils need to be reinforced using stone columns extending to the top of limestone unit. The use of stone columns will allow for an increase in bearing capacity while maintaining a 1-inch settlement criterion and using conventional shallow footings for support of the proposed building. Further detailed recommendations are included in the *Foundation Recommendations* section presented later in this report.

4.2.2 PARTIAL UNDERCUTTING OF HIGHER PLASTICITY CLAYS

The existing clay fill and underlying residual clays exhibited higher plasticity characteristics. The potential vertical rise (PVR) of the clay soils encountered in the borings was evaluated using the Texas Department of Transportation's test method TEX-124-E, Potential Vertical Rise (PVR). This method estimates the potential vertical rise (PVR) of the soils based on the plasticity characteristics, soil moisture levels at the time of the subsurface exploration, thickness of the clay soil strata, and surcharge loads. For this project site, an active zone of 8 feet was used in the calculations.

The TexDOT method indicates a PVR on the order of 1¼ inches for the soil moisture contents encountered at the time of drilling. It is an accepted local practice to tolerate a maximum PVR of 1 inch for conventionally reinforced concrete slabs-on-grade. *In order to reduce the PVR to 1 inch or less, it is recommended that all grade supported slabs of the proposed building be supported on at least 18 inches of properly compacted and approved lower plasticity structural fill.* Where feasible, structural fill placed within the building area should extend at least 5 feet outside the perimeter of the proposed footprint.

Following the recommended undercut, the exposed subgrade should be proofrolled with a loaded tandem-axle dump truck to verify its stability and suitability of start of fill placement. Soft and unstable materials will need to be undercut in order to provide for a platform that is suitable for start of fill placement.

4.3 PAVEMENT SUBGRADE PREPARATION

Location of new pavement areas was not provided to us at the time of preparing this report. The following recommendations assume that clay fill and residual clay soils will be exposed at finished subgrade.

Clay subgrade within proposed pavement areas should be scarified, moisture conditioned and recompacted to a depth of 8 inches. The clay soils should be moisture conditioned within a range of 1 percent below to 3 percent above the material's optimum moisture content, and the subgrade soils recompacted to least 95 percent of the material's standard Proctor maximum dry density.

Where limestone is exposed at finished subgrade, it is recommended to undercut the limestone rock unit to a level that will allow for placement of at least 8 inches of structural fill to provide for uniform subgrade conditions across pavement areas.

We recommend that the project geotechnical engineer or a qualified representative evaluate the subgrade after the site is prepared. Some unsuitable or unstable areas may be present in unexplored areas of the site. All pavement areas should be carefully proofrolled with a heavy (25-ton minimum), rubber-tired vehicle at the following times.

- After an area has been stripped and undercut as recommended, prior to the placement of any fill.
- After grading an area to the finished subgrade elevation.
- After areas have been exposed to any precipitation, and/or have been exposed for more than 48 hours.

Some instability may exist during construction, depending on climatic and other factors immediately preceding and during construction. If any soft or otherwise unsuitable soils are identified during the proofrolling process, they must be undercut or stabilized prior to fill placement or pavement construction. All unsuitable material identified during the construction shall be removed and replaced in accordance with the Structural Fill section of this report.

4.4 STRUCTURAL FILL

The existing fill materials encountered across the site were heterogeneous in composition and comprised of lean clays, and lean to fat clays with varying amounts of concrete rubble and limestone cobbles/boulders.

Based on the laboratory test results, portions of the onsite, lean clay soils will meet the structural fill criteria set forth for this project; however, they will be difficult to differentiate and separate from the lean to fat clay soils.

As such, the onsite fill materials and residual soils are not considered suitable for use as structural fill within the proposed building area. The contractor will need to import lower plasticity material from an offsite borrow source to construct the building pad. Requirements for imported lower plasticity structural fill on this project are as follows:

Soil Type	USCS Classification	Property Requirements	Placement Location
Imported Lean Clay, Clayey Sand or Shale	CL, SC	LL < 40, PI < 18, $\gamma_d > 100$ pcf, P200 > 15%, Maximum 3" particle size in any dimension.	Lower Plasticity Structural Fill to be used for construction of building pad (min. 18" thick)

Table 6: Imported Structural Fill Requirements

Notes:

1. All structural fill should be free of vegetation, topsoil, and any other deleterious materials. The organic content of materials to be used for fill should be less than 3 percent.
2. LL indicates the soil Liquid Limit; PI indicates the soil Plasticity Index; P200 indicates the percent of material by weight that passes the #200 sieve; γ_d indicates the maximum dry density as defined by the density standard outlined in the table below.
3. Laboratory testing of the soils proposed for fill must be performed in order to verify their conformance with the above recommendations.
4. Any fill to be placed at the site should be reviewed by the geotechnical engineer.
5. The contractor needs to anticipate the need to import lower plasticity structural fill from an approved offsite borrow source for construction of building pad.

Placement requirements for structural fill are as follows:

Specification	Requirement
Lift Thickness	Maximum loose lift thickness of 8 to 12 inches, depending on type of compaction equipment used.
Density	95% of the standard Proctor (ASTM D698) maximum density
Moisture	±2% of the optimum moisture content as determined by ASTM D698
Density Testing Frequency	<p>Building and foundation areas: One test per 2,500 square feet (SF) per lift with a minimum of three tests performed per lift</p> <p>Pavement areas: One test per 5,000 square feet (SF) per lift with a minimum of three tests performed per lift</p> <p>Utility trenches: One test per 150 linear feet per lift with a minimum of two tests performed per lift</p>

Table 7: Structural Fill Placement Requirements

4.5 EXCAVATION DIFFICULTY

Excavations within the proposed building area will likely encounter a limestone unit at depths of about 4 to 12.5 feet below existing grades. Auger refusal was encountered within the limestone unit at depths of about 4 to 13 feet.

Based on the information gathered during our subsurface exploration, we anticipate that the overburden materials (fill and residuum) can be excavated using a backhoe in good working condition. It should be noted though that boulder-sized fragments of limestone and concrete may be encountered in the fill.

The contractor will need to anticipate rock excavation techniques. The ability to excavate hard limestone rock is a function of the material, the equipment used, the skill of the operator, the desired rate of removal and other factors. The contractor should review the borings logs and should use his own method to evaluate excavation difficulty.

4.6 TEMPORARY EXCAVATION CONSIDERATIONS

All excavations performed at the site should follow OSHA guidelines for temporary excavations. Excavated soils should be stockpiled according to OSHA regulations to limit the potential cave-in of soils.

Care should be exercised during undercutting of the building areas along the perimeter of the existing building to avoid possible influence on the bearing materials of the existing footings. The bearing materials of the foundation elements supporting the existing building should be protected during excavations. Depending on the bearing elevations of the existing footings and the undercutting depths for the proposed new building additions, the contractor may need to develop a shoring, bracing, or underpinning plan.

Excavations extending to depths greater than 4 feet should be cut to a stable slope or be temporarily braced. Temporary slopes should be constructed in strict compliance with current OSHA excavation regulations. Stockpiles should be placed well away from the edge of the excavations and their height controlled so that they do not surcharge the sides of the excavation. Surface drainage should be carefully controlled to prevent flow of water into the excavations.

Temporary slopes should be closely observed for signs of mass movement: tension cracks near the crest, bulging at the toe, etc. If potential stability problems are observed, a geotechnical engineer should be immediately contacted. The responsibility for excavation safety and stability of temporary construction slopes should lie solely with the contractor.

4.7 EXISTING SLOPES

Existing slopes are present immediately to the north of the existing building (sloping down towards the existing parking lot) and to the south (sloping up towards the wooded area on the south). The slope to the south appeared to be relatively steep. Global slope stability analysis of the existing slopes was not included in our scope of work.

Minimum slope setback requirements are identified in the International Building Code (IBC) Section 1808.7.1. The setbacks should be followed to assure that the footings have adequate vertical and lateral support, and maintains a minimum safe distance from the top of the slope.

4.8 BENCHING OF EXISTING SLOPES

Existing slopes within the project site steeper than 5 Horizontal to 1 Vertical, 5(H): 1(V), and located in fill areas should be benched prior to fill placement. Benching of the slopes provides interlocking between the new fill and on-site materials and facilitates compaction of the fill. Benches should be cut as the fill placement progresses and should have a maximum bench height of 2 to 3 feet. Special attention should be given to fractured limestone units within the cut slopes, which may require additional drainage measures to intercept and divert groundwater flow from the slope.

4.9 UTILITY TRENCH BACKFILL

All utility trenches must be backfilled and compacted in the manner specified above for structural fill. It may be necessary to reduce the lift thickness to 4 to 6 inches to achieve compaction using hand-operated equipment.

4.10 LANDSCAPING AND DRAINAGE CONSIDERATION

The potential for soil moisture fluctuations within building subgrades should be reduced to lessen the potential of subgrade movement. Site grading should include positive drainage away from buildings. Excessive irrigation of landscaping poses a risk of saturating soils below shallow footings resulting in potential differential movement of the foundations due to heave.

4.11 WET WEATHER CONSTRUCTION

Excessive movement of construction equipment across the site during wet weather may result in ruts, which will collect rainwater, prolonging the time required to dry the subgrade soils.

During rainy periods, additional effort will be required to properly prepare the site and establish/maintain an acceptable subgrade. The difficulty will increase in areas where clay or silty soils are exposed at the subgrade elevation. Grading contractors typically postpone grading operations during wet weather to wait for conditions that are more favorable. Contractors can typically disk or aerate the upper soils to promote drying during intermittent periods of favorable weather. When deadlines restrict postponement of grading operations, additional measures such as undercutting and replacing saturated soils or stabilization can be utilized to facilitate placement of additional fill material.

5.0 FOUNDATION RECOMMENDATIONS

Based on the information provided to our office, we understand that the individual column loads will be up to 250 kips. The wall loads for this project were assumed to be 0.5 to 3 kips per linear foot. ***If the above-mentioned structural loading information is incorrect, our office should be contacted, such that our recommendations can be reviewed.***

Considering the relatively high structural loads for the proposed building, the heterogenous composition of the fill, and the presence of a limestone unit at depths ranging from 12.5 feet on the north end to 4 feet on the south end, the subsurface conditions are not considered suitable for supporting the proposed building on conventional shallow footings.

Large footings (on the order of 10 to 12 feet square) with risk of excessive total and differential settlement would be needed to support the multi-story building on shallow footings.

Therefore, we recommend ground modification using a stone column system to improve the bearing capacity of the subsurface materials and to reduce the risk of total and differential settlement. Stone columns will reinforce the overburden soils to allow shallow foundations to be used for support of the structural loads of the proposed building. We recommend the perimeter continuous footings and isolated spread footings bear directly on stone column elements that bear on top of the limestone unit encountered at depths of about 4 to 12.5 feet below grades.

Stone columns are proprietary foundation systems and should be designed and constructed by a licensed company. The installer should provide detailed design calculations sealed by a professional engineer licensed in the State of Oklahoma. Estimates from the design calculations should demonstrate that soil reinforcement from the particular method selected will control long-term settlements to less than 1-inch total and ½ inch differential. The design parameters should be verified by a full-scale “load” test performed in the field. Building & Earth should be retained to monitor the modulus test and subsequent production stone column installations. Other factors, such as damage to near-by existing structures due to vibrations or displacement stresses, should also be addressed.

An allowable bearing pressure will be developed by the installer as part of the design of the stone column system; however, we anticipate an allowable bearing pressure on the order of 4,000 to 6,000 psf is feasible.

Though computed footing dimensions may be less, column footings should be at least 24 inches wide and strip footings should be at least 16 inches wide. These dimensions facilitate hand cleaning of footing subgrades disturbed by the excavation process, aides the placement of reinforcing steel, and reduces the potential for punching shear failure. Exterior footing bottoms should be at least 24 inches below the lowest adjacent finished grade. Lowest adjacent finished grade may be taken as the bottom of interior slab-on-grade or the finished exterior grade, excluding landscape topsoil, whichever is lower.

South and West Portions of the Building Area:

Depending on the final design grades, portions of the proposed building area (south and west portion of the planned building) could encounter auger refusal material (limestone) within 1 to 2 feet below the footing bearing elevation. Installation of stone columns is not practical at these locations. For these particular locations we recommend the fill and residual soils be undercut to the top of the limestone unit.

A Building & Earth representative must evaluate the condition of the exposed bearing materials in the bottom of the foundation excavation at the undercut level.

After completion of the recommended undercutting and evaluation by a Building & Earth representative, the footing excavations can be brought back up to design bearing elevation using lean concrete or compacted graded aggregate base. The aggregate must be placed in lifts with maximum thickness of 6 inches and compacted to at least 98 percent of the material's standard Proctor maximum dry density.

Footings founded directly on evaluated and approved limestone unit (auger refusal material), or on the recommended lean concrete or properly placed and compacted graded aggregate can be dimensioned for a maximum net allowable bearing pressure of 6,000 pounds per square foot (psf).

5.1 GENERAL FOUNDATION RECOMMENDATIONS

All exterior footings should bear at least 24 inches below the adjacent exterior grade and the edge of footing facing the slope must be at least one footing width behind the face of the slope at bearing level, as shown in the figure below.

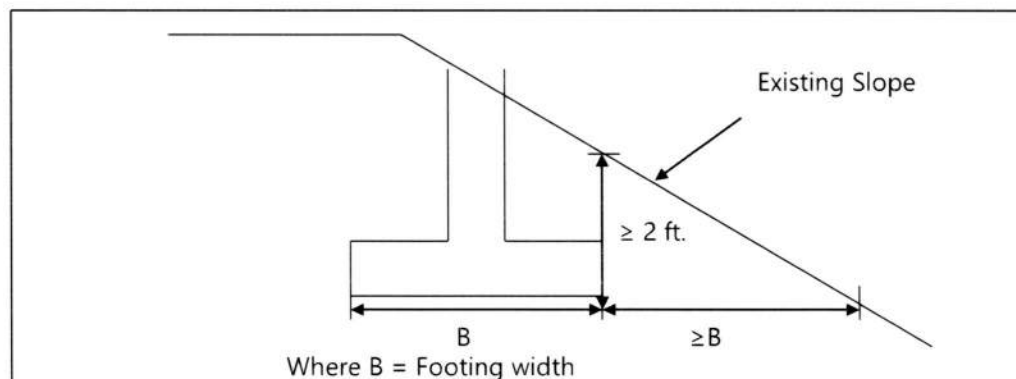


Figure 4: Recommendations for footing on slope

The following items should be considered during the preparation of construction documents and foundation installation:

- The geotechnical engineer of record should observe the exposed foundation bearing surfaces prior to concrete placement to verify that the conditions anticipated during the subsurface exploration are encountered.
- All bearing surfaces must be free of soft or loose soil prior to placing concrete.
- Concrete should be placed the same day the excavations are completed and bearing materials verified by the engineer. If the excavations are left open for an extended period, or if the bearing surfaces are disturbed after the initial observation, then the bearing surfaces should be re-evaluated prior to concrete placement.
- Water should not be allowed to pond in foundation excavations prior to concrete placement or above the concrete after the foundation is completed.
- Wherever possible, the foundation concrete should be placed “neat”, using the sides of the excavations as forms. Where this is not possible, the excavations created by forming the foundations must be backfilled with suitable structural fill and properly compacted.
- The building pad should be sloped to drain away from the building foundations.
- Roof drains should be routed away from the foundation soils. All drains should be collected in pipes or discharged on pavements to prevent drainage into the subsurface.
- Structural design should anticipate differential vertical movement of sidewalks adjacent to the structures and underground utility lines and their connections into the buildings.

6.0 FLOOR SLABS

Site development recommendations presented in this report should be followed to provide for subgrade conditions suitable for support of grade supported slabs. Floor slabs will be supported on well compacted, lower plasticity structural fill with a thickness of at least 18 inches.

Floor slabs for the proposed buildings should be supported on a minimum four (4) inches thick compacted layer of free-draining, granular material, such as ASSTM No. 57 stone. The purpose of this layer is to serve as a leveling course and act as a capillary break for moisture migration through the subgrade soil.

Depending on the proposed floor covering, consideration should be given to the use of a polyethylene vapor barrier. The slabs should be appropriately reinforced (if required) to support the proposed loads.

With addition of the granular material, an effective modulus of subgrade reaction of 130 pci can be used in the design of grade supported building floor slabs.

7.0 STEM AND RETAINING WALLS

We assumed that building stem walls and retaining walls may be constructed as part of the proposed construction to accommodate for grade changes. Specific details of the proposed stem/retaining walls were not provided at the time of preparing this report. For the purpose of this report we assumed that stem/retaining walls will have maximum wall heights on the order of 5 feet.

We anticipate that all stem/retaining walls will retain new structural fill. We recommended that all walls be backfilled with properly compacted and approved lower plasticity structural fill in accordance with *Structural Fill* section of this report.

The following equivalent fluid pressures should be used to design the proposed stem/retaining walls. At rest parameters, should be used to design walls that are not allowed to rotate or translate. Any surcharge loads must be included in the design.

Backfill Material	Soil Parameter Values		Equivalent Fluid Unit Weights for Active & At-Rest Lateral Earth Pressures (pcf)	
	Wet Unit Weight (pcf)	Effective Angle of Internal Friction	At-Rest Condition	Active Condition
Structural Fill	120	30°	60	40

Table 8: Soil Parameters and Lateral Earth Pressure Values (Drained Condition)

It is recommended the design provides for drainage behind the wall to eliminate hydrostatic forces that can result in substantial sliding and over-turning forces to be applied to walls.

Lateral pressures arising from surcharge loading should be added to the above earth pressures to determine the total lateral pressures. In addition, transient loads imposed on the stem/retaining walls by construction equipment during backfilling should be taken into consideration. Excessively heavy grading equipment (that could impose temporary excessive lateral pressures) should not be allowed within 5 feet (horizontally) of the walls.

Passive earth pressures of materials adjacent to wall footings as well as bearing material friction at the footing bases may be used to resist shear. The following table presents recommended ultimate friction coefficient and passive earth pressure values.

Friction Coefficient	Equivalent Fluid Unit Weight for Passive Condition Lateral Earth Pressures (pcf)
0.35	250

Table 9: Soil Parameters Values Resisting Shear

We further recommend that the passive earth pressure be neglected in the upper 2 feet because of the risk of disturbance and exposure to repeat freeze-thaw and wetting-drying cycles.

The above soil parameter and lateral earth pressure values assume the following:

- The wall backfill will be horizontal.
- The backfill will be compacted to 95 percent of standard Proctor maximum dry density.
- No factor of safety is included. The design of the stem walls should include a factor of safety of at least 1.5 against sliding and overturning using the above recommended values.
- Any surcharge is uniform.
- Wall friction is negligible

8.0 RIGID PAVEMENT CONSIDERATIONS

Based on the information provided to our office, we understand the proposed expansion will include construction of drives. Specific traffic information was not provided. For the purpose of this report we assumed that pavements will be subjected to passenger cars, maintenance vehicles, ambulances, light delivery box trucks, and trash collection trucks. Standard-duty sections are to be considered for parking stalls and heavy-duty sections for drives.

Subgrade preparation recommendations were presented earlier in this report. All subgrade, base and pavement construction operations should meet minimum requirements of the Oklahoma Department of Transportation (ODOT), Standard Specifications for Highway Construction, dated 2009. The applicable sections of the specifications are identified as follows:

Material	Specification Section
Portland Cement Concrete Pavement	414 & 701
Mineral Aggregate Base Materials	303 & 703.01

Table 10: ODOT Specification Sections

The following are recommended typical minimum rigid pavement sections for this project.

Minimum Recommended Thickness (in)		Material
Standard Duty Parking Stalls	Heavy Duty Drives	
5.0	6.0	Portland Cement Concrete, $f'_c=3,500$ psi
4.0	6.0	Crushed Aggregate Base (ODOT Type "A")

Table 11: Rigid Pavement Recommendations

The concrete should be protected against moisture loss, rapid temperature fluctuations, and construction traffic for several days after placement. All pavements should be sloped for positive drainage. We suggest that a curing compound be applied after the concrete has been finished.

For access drive approaches and loading areas frequently subjected to high traffic loads with frequent braking and turning of wheels, consideration should be given to using a rigid pavement section comprised of 7 inches Portland cement concrete over 6 inches of limestone aggregate base.

The recommended aggregate base course will serve as a leveling course, improve the subgrade support properties, and reduce the risk of pumping of fine-grained subgrade soils through the joints.

With the use of aggregate base course, the aggregate should have uniform thickness and the subgrade graded such as to provide positive drainage from the granular base. The aggregate base section should grade toward a storm sewer or drainage ditch to provide drainage from the aggregate base.

Although not referenced in the ODOT specifications, based on our experience with project sites in this region and anticipated traffic loads, we recommend Portland cement concrete should have a minimum 28-day compressive strength of 3,500 psi, maximum slump of 4 inches, and air content of 5 to 7 percent.

Consideration should be given to providing reinforcement each way. The civil engineer of record will need to determine the appropriate amount of reinforcing steel required for the various concrete pavements. Typically, two-way reinforcement will consist of No. 3 or No. 4 size reinforcing steel spaced at 12 to 24 inches on center depending on the anticipated traffic loading conditions.

For rigid pavements, we recommend a jointing plan be developed to control cracking and help preclude surficial migration of water into the base course and subgrade. Additionally, we recommend the joints be sealed in order to further preclude surficial moisture migration into the underlying supporting soils.

All pavements should be sloped, approximately $\frac{1}{4}$ inch per foot, to provide rapid surface drainage. Water allowed to pond on or adjacent to the pavement could saturate the subgrade and cause premature deterioration of the pavements as a result of loss of strength and stability. Periodic maintenance of the pavement should be anticipated. This should include sealing of cracks and joints and maintaining proper surface drainage to avoid ponding of water on or near the pavement areas.

9.0 SUBGRADE REHABILITATION

The subgrade soils often become disturbed during the period between initial site grading and construction of surface improvements. The amount and depth of disturbance will vary with soil type, weather conditions, construction traffic, and drainage.

The engineer should evaluate the subgrade soil during final grading and prior to stone placement to verify that the subgrade is suitable to receive base stone or floor slabs. The final evaluation may include proofrolling or density tests.

Subgrade rehabilitation can become a point of controversy when different contractors are responsible for mass and final grading. The construction documents should specifically state which contractor will be responsible for maintaining and rehabilitating the subgrade. Rehabilitation may include wetting, mixing, and re-compacting soils that have dried excessively or drying soils that have become wet.

10.0 CONSTRUCTION MONITORING

Field verification of site conditions is an essential part of the services provided by the geotechnical consultant. In order to confirm our recommendations, it will be necessary for Building & Earth personnel to make periodic visits to the site during site grading. Typical construction monitoring services are listed below.

- Periodic observations and consultations by a member of our engineering staff during site grading.
- Field density tests during structural fill placement on a continuous basis.
- Monitor the vibro-replacement stone columns installer's operations as a Quality Assurance (QA) service. Our services will supplement the installer's internal Quality Control (QC) program. Together, the QA and QC programs will monitor pertinent construction items that affect the performance of each installed ground improvement element.
- Observation and verification of the bearing surfaces exposed after foundation excavation.
- Reinforcing steel inspections.
- Molding and testing of concrete cylinders.

11.0 CLOSING AND LIMITATIONS

This report was prepared for Childers Architects for specific application to the subject project located in Stilwell, Oklahoma. The information in this report is not transferable. This report should not be used for a different development on the same property without first being evaluated by the engineer.

The recommendations in this report were based on the information obtained from our field exploration and laboratory analysis. The data collected is representative of the locations tested. Variations are likely to occur at other locations throughout the site. Engineering judgment was applied in regards to conditions between borings. It will be necessary to confirm the anticipated subsurface conditions during construction.

This report has been prepared in accordance with generally accepted standards of geotechnical engineering practice. No other warranty is expressed or implied. In the event that changes are made, or anticipated to be made, to the nature, design, or location of the project as outlined in this report, Building & Earth must be informed of the changes and given the opportunity to either verify or modify the conclusions of this report in writing, or the recommendations of this report will no longer be valid.

The scope of services for this project did not include any environmental assessment of the site or identification of pollutants or hazardous materials or conditions. If the owner is concerned about environmental issues Building & Earth would be happy to provide an additional scope of services to address those concerns.

This report is intended for use during design and preparation of specifications and may not address all conditions at the site during construction. Contractors reviewing this information should acknowledge that this document is for design information only.

An article published by the Geoprofessional Business Association (GBA), titled *Important Information About Your Geotechnical Report*, has been included in the Appendix. We encourage all individuals to become familiar with the article to help manage risk.

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GEOTECHNICAL INVESTIGATION METHODOLOGIES

The subsurface exploration, which is the basis of the recommendations of this report, has been performed in accordance with industry standards. Detailed methodologies employed in the investigation are presented in the following sections.

DRILLING PROCEDURES – STANDARD PENETRATION TEST (ASTM D1586)

At each boring location, soil samples were obtained at standard sampling intervals with a split-spoon sampler. The borehole was first advanced to the sample depth by augering and the sampling tools were placed in the open hole. The sampler was then driven 18 inches into the ground with a 140-pound automatic hammer free-falling 30 inches. The number of blows required to drive the sampler each 6-inch increment was recorded. The initial increment is considered the "seating" blows, where the sampler penetrates loose or disturbed soil in the bottom of the borehole.

The blows required to penetrate the final two (2) increments are added together and are referred to as the Standard Penetration Test (SPT) N-value. The N-value, when properly evaluated, gives an indication of the soil's strength and ability to support structural loads. Many factors can affect the SPT N-value, so this result cannot be used exclusively to evaluate soil conditions.

The SPT testing was performed using a drill rig equipped with an automatic hammer. Automatic hammers mechanically control the height of the hammer drop, and doing so, deliver higher energy efficiency (90 to 99 % efficiency) than manual hammers (60 % efficiency) which are dropped using a manually operated rope and cathead system. Because historic data correlations were developed based on use of a manual hammer, it is necessary to adjust the N-values obtained using an automatic hammer to make these correlations valid. Therefore, an energy correction factor of 1.3 was applied to the recorded field N-values from the automatic hammer for the purpose of our evaluation. The N-values discussed or mentioned in this report and shown on the boring logs are recorded field values.

Samples retrieved from the boring locations were labeled and stored in plastic bags at the jobsite before being transported to our laboratory for analysis. The project engineer prepared Boring Logs summarizing the subsurface conditions at the boring locations.

BORING LOG DESCRIPTION

Building & Earth Sciences, Inc. used the gINT software program to prepare the attached boring logs. The gINT program provides the flexibility to custom design the boring logs to include the pertinent information from the subsurface exploration and results of our laboratory analysis. The soil and laboratory information included on our logs is summarized below:

DEPTH AND ELEVATION

The depth below the ground surface and the corresponding elevation are shown in the first two columns.

SAMPLE TYPE

The method used to collect the sample is shown. The typical sampling methods include Split Spoon Sampling, Shelby Tube Sampling, Grab Samples, and Rock Core. A key is provided at the bottom of the log showing the graphic symbol for each sample type.

SAMPLE NUMBER

Each sample collected is numbered sequentially.

BLOWS PER INCREMENT, REC%, RQD%

When Standard Split Spoon sampling is used, the blows required to drive the sampler each 6-inch increment are recorded and shown in column 5. When rock core is obtained the recovery ratio (REC%) and Rock Quality Designation (RQD%) is recorded.

SOIL DATA

Column 6 is a graphic representation of four different soil parameters. Each of the parameters use the same graph, however, the values of the graph subdivisions vary with each parameter. Each parameter presented on column 6 is summarized below:

- **N-value**- The Standard Penetration Test N-value, obtained by adding the number of blows required to drive the sampler the final 12 inches, is recorded . The graph labels range from 0 to 50.
- **Qu** – Unconfined Compressive Strength estimate from the Pocket Penetrometer test in tons per square foot (tsf). The graph labels range from 0 to 5 tsf.
- **Atterberg Limits** – The Atterberg Limits are plotted with the plastic limit to the left, and liquid limit to the right, connected by a horizontal line. The difference in the plastic and liquid limits is referred to as the Plasticity Index. The Atterberg Limits test results are also included in the Remarks column on the far right of the boring log. The Atterberg Limits graph labels range from 0 to 100%.
- **Moisture** – The Natural Moisture Content of the soil sample as determined in our laboratory.

SOIL DESCRIPTION

The soil description prepared in accordance with ASTM D2488, Visual Description of Soil Samples. The Munsel Color chart is used to determine the soil color. Strata changes are indicated by a solid line, with the depth of the change indicated on the left side of the line and the elevation of the change indicated on the right side of the line. If subtle changes within a soil type occur, a broken line is used. The Boring Termination or Auger Refusal depth is shown as a solid line at the bottom of the boring.

GRAPHIC

The graphic representation of the soil type is shown. The graphic used for each soil type is related to the Unified Soil Classification chart. A chart showing the graphic associated with each soil classification is included.

REMARKS

Remarks regarding borehole observations, and additional information regarding the laboratory results and groundwater observations.

Major Divisions			Symbols		Group Name & Typical Description
			Lithology	Group	
Coarse Grained Soils More than 50% of material is larger than No. 200 sieve size	Gravel and Gravelly Soils More than 50% of coarse fraction is larger than No. 4 sieve	Clean Gravels (Less than 5% fines)		GW	Well-graded gravels, gravel – sand mixtures, little or no fines
				GP	Poorly-graded gravels, gravel – sand mixtures, little or no fines
		Gravels with Fines (More than 12% fines)		GM	Silty gravels, gravel – sand – silt mixtures
				GC	Clayey gravels, gravel – sand – clay mixtures
	Sand and Sandy Soils More than 50% of coarse fraction is smaller than No. 4 sieve	Clean Sands (Less than 5% fines)		SW	Well-graded sands, gravelly sands, little or no fines
				SP	Poorly-graded sands, gravelly sands, little or no fines
		Sands with Fines (More than 12% fines)		SM	Silty sands, sand – silt mixtures
				SC	Clayey sands, sand – clay mixtures
Fine Grained Soils More than 50% of material is smaller than No. 200 sieve size	Silts and Clays Liquid Limit less than 50	Inorganic		ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silt with slight plasticity
				CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays
		Organic		OL	Organic silts and organic silty clays of low plasticity
	Silts and Clays Liquid Limit greater than 50 sieve	Inorganic		MH	Inorganic silts, micaceous or diatomaceous fine sand, or silty soils
				CH	Inorganic clays of high plasticity
		Organic		OH	Organic clays of medium to high plasticity, organic silts
Highly Organic Soils				PT	Peat, humus, swamp soils with high organic contents

Table 1: Soil Classification Chart (based on ASTM D2487)

Building & Earth Sciences classifies soil in general accordance with the Unified Soil Classification System (USCS) presented in ASTM D2487. Table 1 and Figure 1 exemplify the general guidance of the USCS. Soil consistencies and relative densities are presented in general accordance with Terzaghi, Peck, & Mesri’s (1996) method, as shown on Table 2, when quantitative field and/or laboratory data is available. Table 2 includes Consistency and Relative Density correlations with N-values obtained using either a manual hammer (60 percent efficiency) or automatic hammer (90 percent efficiency). The *Blows Per Increment* and *SPT N-values* displayed on the boring logs are the unaltered values measured in the field. When field and/or laboratory data is not available, we may classify soil in general accordance with the Visual Manual Procedure presented in ASTM D2488.

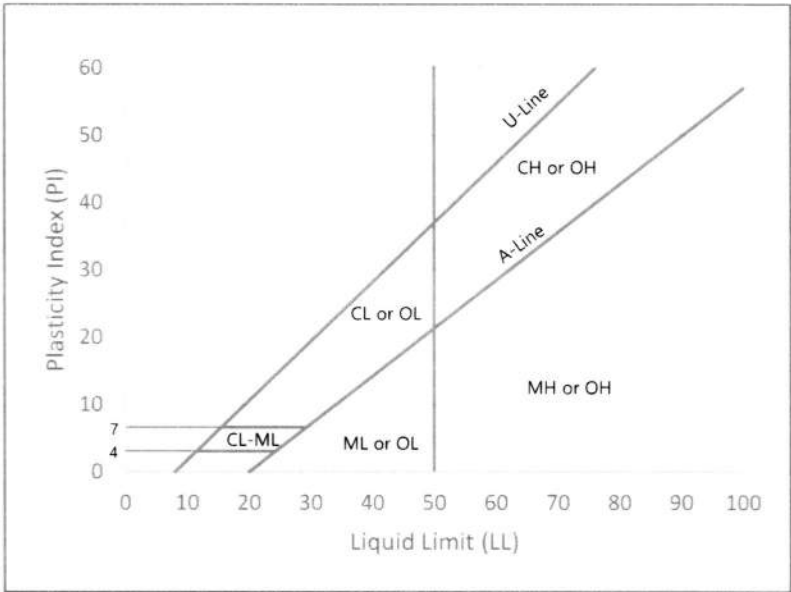


Figure 1: Plasticity Chart (based on ASTM D2487)

Non-cohesive: Coarse-Grained Soil		Cohesive: Fine-Grained Soil				
SPT Penetration (blows/foot)		Relative Density	SPT Penetration (blows/foot)		Consistency	Estimated Range of Unconfined Compressive Strength (tsf)
			Automatic Hammer*	Manual Hammer		
Automatic Hammer*	Manual Hammer		< 2	< 2	Very Soft	< 0.25
0 - 3	0 - 4	Very Loose	2 - 3	2 - 4	Soft	0.25 – 0.50
3 - 8	4 - 10	Loose	3 - 6	4 - 8	Medium Stiff	0.50 – 1.00
8 - 23	10 - 30	Medium Dense	6 - 12	8 - 15	Stiff	1.00 – 2.00
23 - 38	30 - 50	Dense	12 - 23	15 - 30	Very Stiff	2.00 – 4.00
> 38	> 50	Very Dense	> 23	> 30	Hard	> 4.00

Table 2: Soil Consistency and Relative Density (based on Terzaghi, Peck & Mesri, 1996)

* - Modified based on 90% hammer efficiency





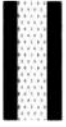



	Standard Penetration Test ASTM D1586 or AASHTO T-206		Dynamic Cone Penetrometer (Sower DCP) ASTM STP-399
	Shelby Tube Sampler ASTM D1587		No Sample Recovery
	Rock Core Sample ASTM D2113		Groundwater at Time of Drilling
	Auger Cuttings		Groundwater as Indicated

Table 1: Symbol Legend

Soil	Particle Size	U.S. Standard
Boulders	Larger than 300 mm	N.A.
Cobbles	300 mm to 75 mm	N.A.
Gravel	75 mm to 4.75 mm	3-inch to #4 sieve
Coarse	75 mm to 19 mm	3-inch to ¾-inch sieve
Fine	19 mm to 4.75 mm	¾-inch to #4 sieve
Sand	4.75 mm to 0.075 mm	#4 to #200 Sieve
Coarse	4.75 mm to 2 mm	#4 to #10 Sieve
Medium	2 mm to 0.425 mm	#10 to #40 Sieve
Fine	0.425 mm to 0.075 mm	#40 to #200 Sieve
Fines	Less than 0.075 mm	Passing #200 Sieve
Silt	Less than 5 µm	N.A.
Clay	Less than 2 µm	N.A.

Table 2: Standard Sieve Sizes





N-Value 	Standard Penetration Test Resistance calculated using ASTM D1586 or AASHTO T-206. Calculated as sum of original, field recorded values.	Atterberg Limits 	A measure of a soil's plasticity characteristics in general accordance with ASTM D4318. The soil Plasticity Index (PI) is representative of this characteristic and is bracketed by the Liquid Limit (LL) and the Plastic Limit (PL).
Qu 	Unconfined compressive strength, typically estimated from a pocket penetrometer. Results are presented in tons per square foot (tsf).	% Moisture 	Percent natural moisture content in general accordance with ASTM D2216.

Table 3: Soil Data

Hollow Stem Auger	Flights on the outside of the shaft advance soil cuttings to the surface. The hollow stem allows sampling through the middle of the auger flights.
Mud Rotary / Wash Bore	A cutting head advances the boring and discharges a drilling fluid to support the borehole and circulate cuttings to the surface.
Solid Flight Auger	Flights on the outside bring soil cuttings to the surface. Solid stem requires removal from borehole during sampling.
Hand Auger	Cylindrical bucket (typically 3-inch diameter and 8 inches long) attached to a metal rod and turned by human force.

Table 4: Soil Drilling Methods

Descriptor	Meaning
Trace	Likely less than 5%
Few	5 to 10%
Little	15 to 25%
Some	30 to 45%
Mostly	50 to 100%

Table 5: Descriptors

Manual Hammer	The operator tightens and loosens the rope around a rotating drum assembly to lift and drop a sliding, 140-pound hammer falling 30 inches.
Automatic Trip Hammer	An automatic mechanism is used to lift and drop a sliding, 140-pound hammer falling 30 inches.
Dynamic Cone Penetrometer (Sower DCP) ASTM STP-399	Uses a 15-pound steel mass falling 20 inches to strike an anvil and cause penetration of a 1.5-inch diameter cone seated in the bottom of a hand augered borehole. The blows required to drive the embedded cone a depth of 1-3/4 inches have been correlated by others to N-values derived from the Standard Penetration Test (SPT).

Table 6: Sampling Methods

Non-plastic	A 1/8-inch thread cannot be rolled at any water content.
Low	The thread can barely be rolled and the lump cannot be formed when drier than the plastic limit.
Medium	The thread is easy to roll and not much time is required to reach the plastic limit. The thread cannot be re-rolled after reaching the plastic limit. The lump crumbles when drier than the plastic limit.
High	It takes considerable time rolling and kneading to reach the plastic limit. The thread can be re-rolled several times after reaching the plastic limit. The lump can be formed without crumbling when drier than the plastic limit.

Table 7: Plasticity

Dry	Absence of moisture, dusty, dry to the touch.
Moist	Damp but no visible water.
Wet	Visible free water, usually soil is below water table.

Table 8: Moisture Condition

Stratified	Alternating layers of varying material or color with layers at least 1/2 inch thick.
Laminated	Alternating layers of varying material or color with layers less than 1/4 inch thick.
Fissured	Breaks along definite planes of fracture with little resistance to fracturing.
Slickensides	Fracture planes appear polished or glossy, sometimes striated.
Blocky	Cohesive soil that can be broken down into small angular lumps which resist further breakdown.
Lensed	Inclusion of small pockets of different soils, such as small lenses of sand scattered through a mass of clay.
Homogeneous	Same color and appearance throughout.

Table 9: Structure

Hatch	Description	Hatch	Description	Hatch	Description
	GW - Well-graded gravels, gravel – sand mixtures, little or no fines		Asphalt		Clay with Gravel
	GP - Poorly-graded gravels, gravel – sand mixtures, little or no fines		Aggregate Base		Sand with Gravel
	GM - Silty gravels, gravel – sand – silt mixtures		Topsoil		Silt with Gravel
	GC - Clayey gravels, gravel – sand – clay mixtures		Concrete		Gravel with Sand
	SW - Well-graded sands, gravelly sands, little or no fines		Coal		Gravel with Clay
	SP - Poorly-graded sands, gravelly sands, little or no fines		CL-ML - Silty Clay		Gravel with Silt
	SM - Silty sands, sand – silt mixtures		Sandy Clay		Limestone
	SC - Clayey sands, sand – clay mixtures		Clayey Chert		Chalk
	ML - Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silt with slight plasticity		Low and High Plasticity Clay		Siltstone
	CL - Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays		Low Plasticity Silt and Clay		Till
	OL - Organic silts and organic silty clays of low plasticity		High Plasticity Silt and Clay		Sandy Clay with Cobbles and Boulders
	MH - Inorganic silts, micaceous or diatomaceous fine sand, or silty soils		Fill		Sandstone with Shale
	CH - Inorganic clays of high plasticity		Weathered Rock		Coral
	OH - Organic clays of medium to high plasticity, organic silts		Sandstone		Boulders and Cobbles
	PT - Peat, humus, swamp soils with high organic contents		Shale		Soil and Weathered Rock

Table 1: Key to Hatches Used for Boring Logs and Soil Profiles

BORING LOCATION PLAN



Google Earth
2016-09-08

**REFERENCE USED
TO PRODUCE THIS
DRAWING:**

Google Earth Satellite
Imagery

BORING LOCATION PLAN

DATE: 8/8/2018

PROJECT NO.

OK180172

PROJECT NAME / LOCATION:

Wilma P Mankiller Clinic
Renovations
Stilwell, Oklahoma

SCALE:

As Shown

BUILDING & EARTH

Geotechnical, Environmental, and Materials Engineers

BORING LOGS



Geotechnical, Environmental, and Materials Engineers

LOG OF BORING

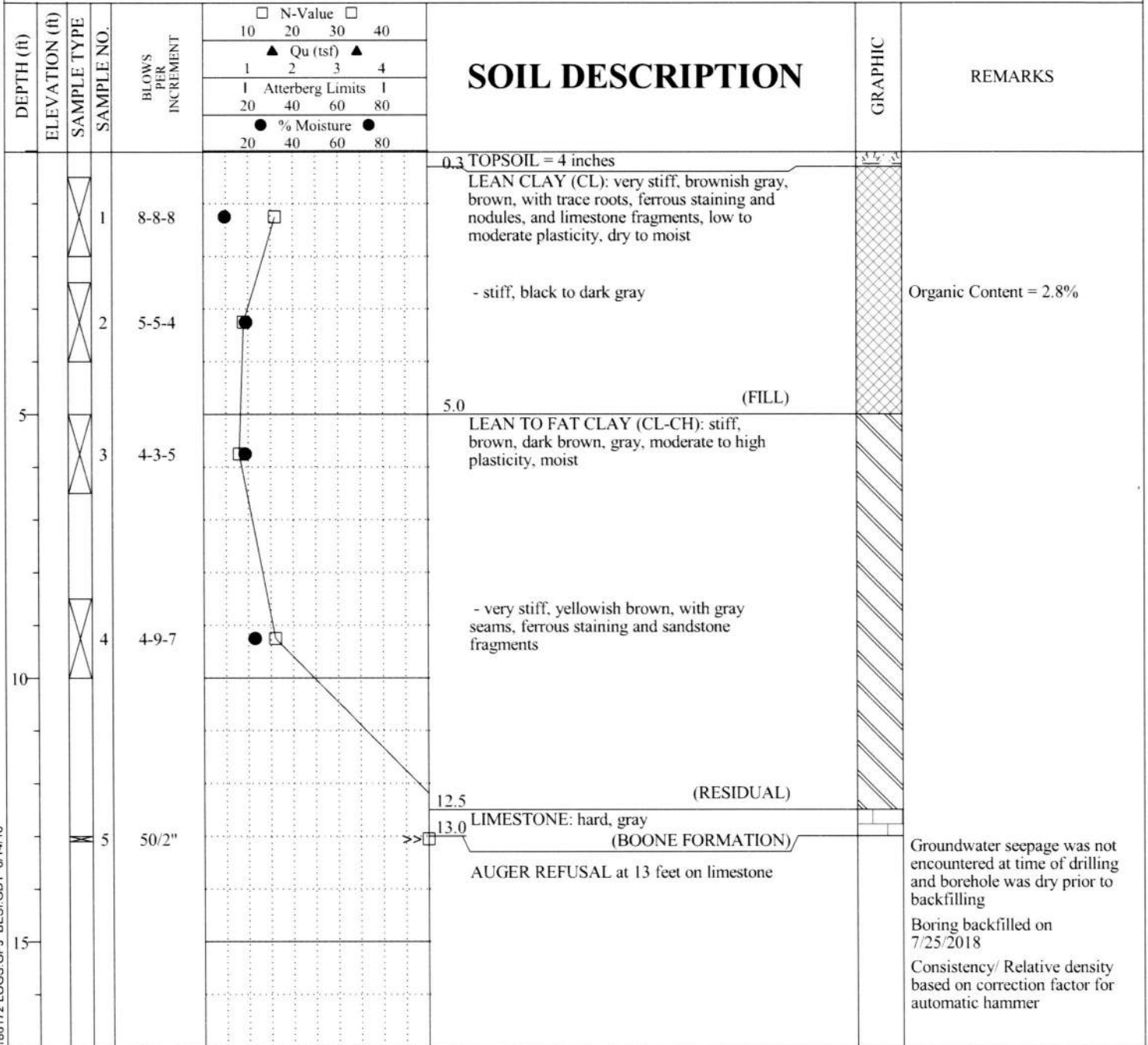
Designation: B-01

Sheet 1 of 1

1403 S. 70th East Avenue
Tulsa, OK 74112
Office: (918) 439-9005
Fax: (918) 439-9255
www.BuildingAndEarth.com

Project Name: Wilma P Mankiller Clinic Renovations
Project Number: OK180172
Drilling Method: Hollow Stem Auger
Equipment Used: CME-550
Hammer Type: Automatic
Boring Location: NE portion of the planned addition

Project Location: Stilwell, Oklahoma
Date Drilled: 7/25/18
Weather Conditions: cloudy
Surface Elevation:
Drill Crew: B & E
Logged By: Jacob Gatling



SAMPLE TYPE Split Spoon

N-VALUE STANDARD PENETRATION RESISTANCE (AASHTO T-206)
% MOISTURE PERCENT NATURAL MOISTURE CONTENT
 GROUNDWATER LEVEL IN THE BOREHOLE
Qu UNCONFINED COMPRESSIVE STRENGTH ESTIMATE FROM POCKET PENETROMETER TEST

REC RECOVERY
RQD ROCK QUALITY DESIGNATION
UD UNDISTURBED

LOG OF BORING 2 OK180172 LOGS.GPJ BESJ.GDT 8/14/18

Birmingham, AL ■ Huntsville, AL ■ Auburn, AL ■ Montgomery, AL ■ Columbus, GA

Raleigh, NC ■ Tulsa, OK ■ Little Rock, AR ■ Springdale, AR ■ New Orleans, LA ■ Louisville, KY



Geotechnical, Environmental, and Materials Engineers

LOG OF BORING

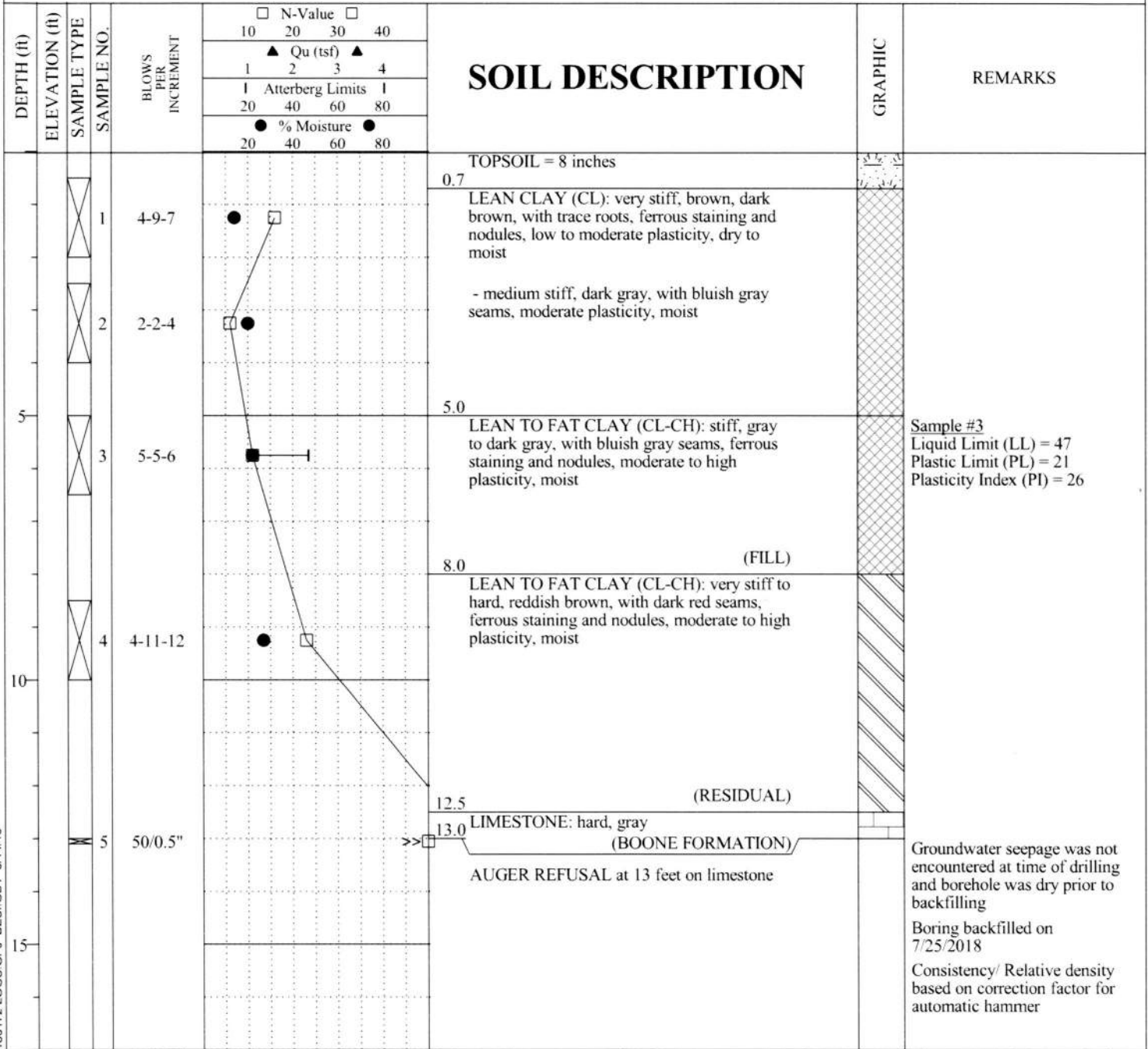
Designation: B-02

Sheet 1 of 1

1403 S. 70th East Avenue
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Office: (918) 439-9005
Fax: (918) 439-9255
www.BuildingAndEarth.com

Project Name: Wilma P Mankiller Clinic Renovations
Project Number: OK180172
Drilling Method: Hollow Stem Auger
Equipment Used: CME-550
Hammer Type: Automatic
Boring Location: NW portion of the planned addition

Project Location: Stilwell, Oklahoma
Date Drilled: 7/25/18
Weather Conditions: cloudy
Surface Elevation:
Drill Crew: B & E
Logged By: Jacob Gatling



SAMPLE TYPE Split Spoon

N-VALUE STANDARD PENETRATION RESISTANCE (AASHTO T-206) **REC** RECOVERY
% MOISTURE PERCENT NATURAL MOISTURE CONTENT **RQD** ROCK QUALITY DESIGNATION
 GROUNDWATER LEVEL IN THE BOREHOLE **UD** UNDISTURBED
Qu UNCONFINED COMPRESSIVE STRENGTH ESTIMATE FROM POCKET PENETROMETER TEST

Birmingham, AL ■ Huntsville, AL ■ Auburn, AL ■ Montgomery, AL ■ Columbus, GA

Raleigh, NC ■ Tulsa, OK ■ Little Rock, AR ■ Springdale, AR ■ New Orleans, LA ■ Louisville, KY

LOG OF BORING 2 OK180172 LOGS.GPJ BESJ.GDT 8/14/18



Geotechnical, Environmental, and Materials Engineers

LOG OF BORING

Designation: B-03

Sheet 1 of 1

1403 S. 70th East Avenue
Tulsa, OK 74112
Office: (918) 439-9005
Fax: (918) 439-9255
www.BuildingAndEarth.com

Project Name: Wilma P Mankiller Clinic Renovations
Project Number: OK180172
Drilling Method: Hollow Stem Auger
Equipment Used: CME-550
Hammer Type: Automatic
Boring Location: West portion of planned addition

Project Location: Stilwell, Oklahoma
Date Drilled: 7/25/18
Weather Conditions: cloudy
Surface Elevation:
Drill Crew: B & E
Logged By: Jacob Gatling

DEPTH (ft)	ELEVATION (ft)	SAMPLE TYPE	SAMPLE NO.	BLOWS PER INCREMENT	N-Value				SOIL DESCRIPTION	GRAPHIC	REMARKS
					10	20	30	40			
					<input type="checkbox"/> N-Value <input type="checkbox"/> 10 20 30 40 ▲ Qu (tsf) ▲ 1 2 3 4 Atterberg Limits 20 40 60 80 ● % Moisture ● 20 40 60 80						
								0.3 CONCRETE = 3 inches			
								0.5 AGGREGATE BASE = 3 inches			
		1		7-8-12	●			LEAN TO FAT CLAY (CL-CH): very stiff, dark gray, with some ferrous staining, limestone gravel, moderate to high plasticity, moist		Sample #1 Liquid Limit (LL) = 45 Plastic Limit (PL) = 20 Plasticity Index (PI) = 25	
		2		9-9-5	●						
								4.0 (FILL)			
								AUGER REFUSAL at 4 feet on apparent limestone			
5											
10											
15											

Groundwater seepage was not encountered at time of drilling and borehole was dry prior to backfilling
Boring backfilled on 7/25/2018
Consistency/ Relative density based on correction factor for automatic hammer

SAMPLE TYPE	<input checked="" type="checkbox"/> Split Spoon
N-VALUE	STANDARD PENETRATION RESISTANCE (AASHTO T-206)
% MOISTURE	PERCENT NATURAL MOISTURE CONTENT
<input type="checkbox"/>	GROUNDWATER LEVEL IN THE BOREHOLE
Qu	UNCONFINED COMPRESSIVE STRENGTH ESTIMATE FROM POCKET PENETROMETER TEST
REC	RECOVERY
RQD	ROCK QUALITY DESIGNATION
UD	UNDISTURBED

LOG OF BORING 2 OK180172 LOGS.GPJ BESJ.GDT 8/14/18



Geotechnical, Environmental, and Materials Engineers

LOG OF BORING

Designation: B-04

Sheet 1 of 1

1403 S. 70th East Avenue
Tulsa, OK 74112
Office: (918) 439-9005
Fax: (918) 439-9255
www.BuildingAndEarth.com

Project Name: Wilma P Mankiller Clinic Renovations
Project Number: OK180172
Drilling Method: Hollow Stem Auger
Equipment Used: CME-550
Hammer Type: Automatic
Boring Location: SW portion of the planned addition

Project Location: Stilwell, Oklahoma
Date Drilled: 7/25/18
Weather Conditions: cloudy
Surface Elevation:
Drill Crew: B & E
Logged By: Jacob Gatling

DEPTH (ft)	ELEVATION (ft)	SAMPLE TYPE	SAMPLE NO.	BLOWS PER INCREMENT	N-Value				SOIL DESCRIPTION	GRAPHIC	REMARKS
					10	20	30	40			
					▲ Qu (tsf) ▲ 1 2 3 4 Atterberg Limits 20 40 60 80 ● % Moisture ● 20 40 60 80						
						TOPSOIL = 7 inches 0.6 LEAN CLAY (CL): very stiff, brown, with trace roots, some ferrous staining, low to moderate plasticity, moist - hard, light gray with cobble- to boulder-sized limestone and concrete fragments below 2.5 feet					
		1	4-7-7								
		2	5-17-50/5"								
		3	50/4"								
5								5.0 (FILL)			
								5.5 LIMESTONE: hard, gray (BOONE FORMATION)			
								AUGER REFUSAL at 5.5 feet on limestone			
10											
15											

Groundwater seepage was not encountered at time of drilling and borehole was dry prior to backfilling
Boring backfilled on 7/25/2018
Consistency/ Relative density based on correction factor for automatic hammer

SAMPLE TYPE Split Spoon

N-VALUE STANDARD PENETRATION RESISTANCE (AASHTO T-206) **REC** RECOVERY
% MOISTURE PERCENT NATURAL MOISTURE CONTENT **RQD** ROCK QUALITY DESIGNATION
 GROUNDWATER LEVEL IN THE BOREHOLE **UD** UNDISTURBED
Qu UNCONFINED COMPRESSIVE STRENGTH ESTIMATE FROM POCKET PENETROMETER TEST

Birmingham, AL ■ Huntsville, AL ■ Auburn, AL ■ Montgomery, AL ■ Columbus, GA

Raleigh, NC ■ Tulsa, OK ■ Little Rock, AR ■ Springdale, AR ■ New Orleans, LA ■ Louisville, KY

LOG OF BORING 2 OK180172 LOGS.GPJ BESIG.DT 8/14/18



Geotechnical, Environmental, and Materials Engineers

LOG OF BORING

Designation: B-05

Sheet 1 of 1

1403 S. 70th East Avenue
Tulsa, OK 74112
Office: (918) 439-9005
Fax: (918) 439-9255
www.BuildingAndEarth.com

Project Name: Wilma P Mankiller Clinic Renovations
Project Number: OK180172
Drilling Method: Hollow Stem Auger
Equipment Used: CME-550
Hammer Type: Automatic
Boring Location: SW portion of the planned addition

Project Location: Stilwell, Oklahoma
Date Drilled: 7/25/18
Weather Conditions: cloudy
Surface Elevation:
Drill Crew: B & E
Logged By: Jacob Gatling

DEPTH (ft)	ELEVATION (ft)	SAMPLE TYPE	SAMPLE NO.	BLOWS PER INCREMENT	<input type="checkbox"/> N-Value <input type="checkbox"/> 10 20 30 40 <input type="checkbox"/> Qu (tsf) <input type="checkbox"/> 1 2 3 4 Atterberg Limits 20 40 60 80 <input type="checkbox"/> % Moisture <input type="checkbox"/> 20 40 60 80				SOIL DESCRIPTION	GRAPHIC	REMARKS	
									0.6	TOPSOIL = 7 inches		
		1		4-5-8						LEAN CLAY with SAND (CL): stiff to very stiff, brown to dark brown, with trace roots, some ferrous staining, and limestone fragments, moderate plasticity, moist (FILL)		Sample #1 Liquid Limit (LL) = 34 Plastic Limit (PL) = 17 Plasticity Index (PI) = 17 % Passing #200 = 70.4
		2		4-16-28						LEAN CLAY (CL): hard, dark brownish gray, with trace roots, with gravel-sized limestone fragments, and ferrous staining, moderate plasticity, moist - with cobble- and boulder-sized limestone fragments below 3.5 feet		
		3		7-21-28						(POSSIBLE FILL)		
		4		50/4"						LIMESTONE: moderately hard, light gray to gray, thin bedded and fragmented		
										(BOONE FORMATION)		
										AUGER REFUSAL at 9 feet on limestone		

Groundwater seepage was not encountered at time of drilling and borehole was dry prior to backfilling
Boring backfilled on 7/25/2018
Consistency/ Relative density based on correction factor for automatic hammer

SAMPLE TYPE Split Spoon

N-VALUE STANDARD PENETRATION RESISTANCE (AASHTO T-206) **REC** RECOVERY
% MOISTURE PERCENT NATURAL MOISTURE CONTENT **RQD** ROCK QUALITY DESIGNATION
 GROUNDWATER LEVEL IN THE BOREHOLE **UD** UNDISTURBED
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LOG OF BORING 2 OK180172 LOGS.GPJ BESJ.GDT 8/14/18

LABORATORY TEST PROCEDURES

A brief description of the laboratory tests performed is provided in the following sections.

DESCRIPTION OF SOILS (VISUAL-MANUAL PROCEDURE) (ASTM D2488)

The soil samples were visually examined by our engineer and soil descriptions were provided. Representative samples were then selected and tested in accordance with the aforementioned laboratory-testing program to determine soil classifications and engineering properties. This data was used to correlate our visual descriptions with the Unified Soil Classification System (USCS).

NATURAL MOISTURE CONTENT (ASTM D2216)

Natural moisture contents (M%) were determined on selected samples. The natural moisture content is the ratio, expressed as a percentage, of the weight of water in a given amount of soil to the weight of solid particles.

ATTERBERG LIMITS (ASTM D4318)

The Atterberg Limits test was performed to evaluate the soil's plasticity characteristics. The soil Plasticity Index (PI) is representative of this characteristic and is bracketed by the Liquid Limit (LL) and the Plastic Limit (PL). The Liquid Limit is the moisture content at which the soil will flow as a heavy viscous fluid. The Plastic Limit is the moisture content at which the soil is between "plastic" and the semi-solid stage. The Plasticity Index ($PI = LL - PL$) is a frequently used indicator for a soil's potential for volume change. Typically, a soil's potential for volume change increases with higher plasticity indices.

MATERIAL FINER THAN NO. 200 SIEVE BY WASHING (ASTM D1140)

Grain-size tests were performed to determine the partial soil particle size distribution. The amount of material finer than the openings on the No. 200 sieve (0.075 mm) was determined by washing soil over the No. 200 sieve. The results of wash #200 tests are presented on the boring logs included in this report and in the table of laboratory test results.

LOSS ON IGNITION TEST (LOI) (ASTM D2974)

LOI test was performed on select sample collected from the site, which was possibly mixed with organic material. The ash content of a peat or organic soil sample is determined by igniting an oven-dried sample of the soil in a muffle furnace at 440°C (Method C) or 750°C (Method D). The substance remaining after ignition is the ash. The organic content is expressed as a percentage of the mass of the oven-dried sample.

LABORATORY TEST RESULTS

The results of the laboratory testing are presented in the following table.

Boring Location	Sample Depth (ft)	LL	PL	PI	% Passing #200 Sieve	Organic Content (%)	Moisture Content (%)
B-01	2.5 – 4.0	---	---	---	---	2.83	18.5
B-02	2.5 – 4.0	47	21	26	---	---	19.6
B-03	0.5 – 2.0	45	20	25	---	---	6.3
B-05	0.5 – 2.0	34	17	17	70.4	---	14.1

Table A-1: General Soil Classification Test Results

Soils with a Liquid Limit (LL) greater than 50 and Plasticity Index (PI) greater than 25 usually exhibit significant volume change with varying moisture content and are considered to be highly plastic.

Important Information about This

Geotechnical-Engineering Report

Subsurface problems are a principal cause of construction delays, cost overruns, claims, and disputes.

While you cannot eliminate all such risks, you can manage them. The following information is provided to help.

Geotechnical Services Are Performed for Specific Purposes, Persons, and Projects

Geotechnical engineers structure their services to meet the specific needs of their clients. A geotechnical-engineering study conducted for a civil engineer may not fulfill the needs of a constructor — a construction contractor — or even another civil engineer. Because each geotechnical-engineering study is unique, each geotechnical-engineering report is unique, prepared *solely* for the client. No one except you should rely on this geotechnical-engineering report without first conferring with the geotechnical engineer who prepared it. *And no one — not even you — should apply this report for any purpose or project except the one originally contemplated.*

Read the Full Report

Serious problems have occurred because those relying on a geotechnical-engineering report did not read it all. Do not rely on an executive summary. Do not read selected elements only.

Geotechnical Engineers Base Each Report on a Unique Set of Project-Specific Factors

Geotechnical engineers consider many unique, project-specific factors when establishing the scope of a study. Typical factors include: the client's goals, objectives, and risk-management preferences; the general nature of the structure involved, its size, and configuration; the location of the structure on the site; and other planned or existing site improvements, such as access roads, parking lots, and underground utilities. Unless the geotechnical engineer who conducted the study specifically indicates otherwise, do not rely on a geotechnical-engineering report that was:

- not prepared for you;
- not prepared for your project;
- not prepared for the specific site explored; or
- completed before important project changes were made.

Typical changes that can erode the reliability of an existing geotechnical-engineering report include those that affect:

- the function of the proposed structure, as when it's changed from a parking garage to an office building, or from a light-industrial plant to a refrigerated warehouse;
- the elevation, configuration, location, orientation, or weight of the proposed structure;
- the composition of the design team; or
- project ownership.

As a general rule, *always* inform your geotechnical engineer of project changes—even minor ones—and request an

assessment of their impact. *Geotechnical engineers cannot accept responsibility or liability for problems that occur because their reports do not consider developments of which they were not informed.*

Subsurface Conditions Can Change

A geotechnical-engineering report is based on conditions that existed at the time the geotechnical engineer performed the study. *Do not rely on a geotechnical-engineering report whose adequacy may have been affected by: the passage of time; man-made events, such as construction on or adjacent to the site; or natural events, such as floods, droughts, earthquakes, or groundwater fluctuations. Contact the geotechnical engineer before applying this report to determine if it is still reliable. A minor amount of additional testing or analysis could prevent major problems.*

Most Geotechnical Findings Are Professional Opinions

Site exploration identifies subsurface conditions only at those points where subsurface tests are conducted or samples are taken. Geotechnical engineers review field and laboratory data and then apply their professional judgment to render an opinion about subsurface conditions throughout the site. Actual subsurface conditions may differ — sometimes significantly — from those indicated in your report. Retaining the geotechnical engineer who developed your report to provide geotechnical-construction observation is the most effective method of managing the risks associated with unanticipated conditions.

A Report's Recommendations Are Not Final

Do not overrely on the confirmation-dependent recommendations included in your report. *Confirmation-dependent recommendations are not final*, because geotechnical engineers develop them principally from judgment and opinion. Geotechnical engineers can finalize their recommendations *only* by observing actual subsurface conditions revealed during construction. *The geotechnical engineer who developed your report cannot assume responsibility or liability for the report's confirmation-dependent recommendations if that engineer does not perform the geotechnical-construction observation required to confirm the recommendations' applicability.*

A Geotechnical-Engineering Report Is Subject to Misinterpretation

Other design-team members' misinterpretation of geotechnical-engineering reports has resulted in costly

problems. Confront that risk by having your geotechnical engineer confer with appropriate members of the design team after submitting the report. Also retain your geotechnical engineer to review pertinent elements of the design team's plans and specifications. Constructors can also misinterpret a geotechnical-engineering report. Confront that risk by having your geotechnical engineer participate in prebid and preconstruction conferences, and by providing geotechnical construction observation.

Do Not Redraw the Engineer's Logs

Geotechnical engineers prepare final boring and testing logs based upon their interpretation of field logs and laboratory data. To prevent errors or omissions, the logs included in a geotechnical-engineering report should *never* be redrawn for inclusion in architectural or other design drawings. Only photographic or electronic reproduction is acceptable, *but recognize that separating logs from the report can elevate risk.*

Give Constructors a Complete Report and Guidance

Some owners and design professionals mistakenly believe they can make constructors liable for unanticipated subsurface conditions by limiting what they provide for bid preparation. To help prevent costly problems, give constructors the complete geotechnical-engineering report, *but* preface it with a clearly written letter of transmittal. In that letter, advise constructors that the report was not prepared for purposes of bid development and that the report's accuracy is limited; encourage them to confer with the geotechnical engineer who prepared the report (a modest fee may be required) and/or to conduct additional study to obtain the specific types of information they need or prefer. A prebid conference can also be valuable. *Be sure constructors have sufficient time* to perform additional study. Only then might you be in a position to give constructors the best information available to you, while requiring them to at least share some of the financial responsibilities stemming from unanticipated conditions.

Read Responsibility Provisions Closely

Some clients, design professionals, and constructors fail to recognize that geotechnical engineering is far less exact than other engineering disciplines. This lack of understanding has created unrealistic expectations that have led to disappointments, claims, and disputes. To help reduce the risk of such outcomes, geotechnical engineers commonly include a variety of explanatory provisions in their reports. Sometimes labeled "limitations," many of these provisions indicate where geotechnical engineers' responsibilities begin and end, to help

others recognize their own responsibilities and risks. *Read these provisions closely.* Ask questions. Your geotechnical engineer should respond fully and frankly.

Environmental Concerns Are Not Covered

The equipment, techniques, and personnel used to perform an *environmental* study differ significantly from those used to perform a *geotechnical* study. For that reason, a geotechnical-engineering report does not usually relate any environmental findings, conclusions, or recommendations; e.g., about the likelihood of encountering underground storage tanks or regulated contaminants. *Unanticipated environmental problems have led to numerous project failures.* If you have not yet obtained your own environmental information, ask your geotechnical consultant for risk-management guidance. *Do not rely on an environmental report prepared for someone else.*

Obtain Professional Assistance To Deal with Mold

Diverse strategies can be applied during building design, construction, operation, and maintenance to prevent significant amounts of mold from growing on indoor surfaces. To be effective, all such strategies should be devised for the *express purpose* of mold prevention, integrated into a comprehensive plan, and executed with diligent oversight by a professional mold-prevention consultant. Because just a small amount of water or moisture can lead to the development of severe mold infestations, many mold prevention strategies focus on keeping building surfaces dry. While groundwater, water infiltration, and similar issues may have been addressed as part of the geotechnical-engineering study whose findings are conveyed in this report, the geotechnical engineer in charge of this project is **not** a mold prevention consultant; *none of the services performed in connection with the geotechnical engineer's study were designed or conducted for the purpose of mold prevention. Proper implementation of the recommendations conveyed in this report will not of itself be sufficient to prevent mold from growing in or on the structure involved.*

Rely, on Your GBC-Member Geotechnical Engineer for Additional Assistance

Membership in the Geotechnical Business Council of the Geoprofessional Business Association exposes geotechnical engineers to a wide array of risk-confrontation techniques that can be of genuine benefit for everyone involved with a construction project. Confer with you GBC-Member geotechnical engineer for more information.



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SECTION 00 7200

GENERAL CONDITIONS

PART 1 - GENERAL

1.1 GENERAL

- A. The General Conditions of this Contract are the American Institute of Architects Document A201, "General Conditions of the Contract for Construction", 2007 Edition as amended by the Owner, hereinafter referred to as the "General Conditions."
- B. The General Conditions shall apply to each and every Section of the Work as though written in full therein and are made a part of the Contract Documents by reference.
- C. The General Conditions of this Contract are available from the Owner, hereinafter referred to as the "General Conditions."
- D. The General Conditions of this Contract are that which is referenced in the Owner-Contractor Agreement and hereinafter is referred to as the "General Conditions."

PART 2 - (NOT USED)

PART 3 - (NOT USED)

END OF SECTION

SECTION 01 2500

SUBSTITUTION PROCEDURES

PART 1 - GENERAL

1.1 SUMMARY

- A. Section includes administrative and procedural requirements for substitutions.

1.2 DEFINITIONS

- A. Substitutions: Changes in products, materials, equipment, and methods of construction from those required by the Contract Documents and proposed by Contractor.
 - 1. Substitutions for Cause: Changes proposed by Contractor that are required due to changed Project conditions, such as unavailability of product, regulatory changes, or unavailability of required warranty terms.
 - 2. Substitutions for Convenience: Changes proposed by Contractor or Owner that are not required in order to meet other Project requirements but may offer advantage to Contractor or Owner.

1.3 SUBMITTALS

- A. Substitution Requests: Submit electronic copies of each request for consideration. Identify product or fabrication or installation method to be replaced. Include Specification Section number and title and Drawing numbers and titles.
 - 1. Substitution Request Form: Use scanned PDF electronic file of form provided at end of this section or annotated PDF electronic file of electronic form received from Architect matching form provided at end of this section.
 - 2. Documentation: Show compliance with requirements for substitutions and the following, as applicable:
 - a. Statement indicating why specified product or fabrication or installation cannot be provided, if applicable.
 - b. Coordination information, including a list of changes or revisions needed to other parts of the Work and to construction performed by Owner and separate contractors that will be necessary to accommodate proposed substitution.
 - c. Detailed comparison of significant qualities of proposed substitution with those of the Work specified. Include annotated copy of applicable Specification Section. Significant qualities may include attributes such as performance, weight, size, durability, visual effect, sustainable design characteristics, warranties, and specific features and requirements indicated. Indicate deviations, if any, from the Work specified.
 - d. Product Data, including drawings and descriptions of products and fabrication and installation procedures.
 - e. Samples, where applicable or requested.
 - f. Certificates and qualification data, where applicable or requested.
 - g. List of similar installations for completed projects with project names and addresses and names and addresses of architects and owners.
 - h. Material test reports from a qualified testing agency indicating and interpreting test results for compliance with requirements indicated.
 - i. Research reports evidencing compliance with building code in effect for Project, from ICC-ES.

- j. Detailed comparison of Contractor's construction schedule using proposed substitution with products specified for the Work, including effect on the overall Contract Time. If specified product or method of construction cannot be provided within the Contract Time, include letter from manufacturer, on manufacturer's letterhead, stating date of receipt of purchase order, lack of availability, or delays in delivery.
 - k. Cost information, including a proposal of change, if any, in the Contract Sum.
 - l. Contractor's certification that proposed substitution complies with requirements in the Contract Documents except as indicated in substitution request, is compatible with related materials, and is appropriate for applications indicated.
 - m. Contractor's waiver of rights to additional payment or time that may subsequently become necessary because of failure of proposed substitution to produce indicated results.
3. Architect's Action: If necessary, with reasonable promptness, Architect will request additional information or documentation for evaluation. Architect will notify Contractor of acceptance or rejection of proposed substitution with reasonable promptness. Acceptance of proposed substitution does not constitute approval or inclusion in Contract Documents. Pay applications certification, change orders, and certificate of substantial completion will contain such qualification.

1.4 QUALITY ASSURANCE

- A. Compatibility of Substitutions: Investigate and document compatibility of proposed substitution with related products and materials. Engage a qualified testing agency to perform compatibility tests recommended by manufacturers.

1.5 PROCEDURES

- A. Coordination: Revise or adjust affected work as necessary to integrate work of the approved substitutions.

PART 2 - PRODUCTS

2.1 SUBSTITUTIONS

- A. Prior to starting Substitution Process, review proposed recommendations with Architect.
- B. Contractor's submittal and Architect's acceptance of Shop Drawings, Product Data or Samples for construction activities not complying with Contract Documents does not constitute acceptable or valid request for substitution, nor does it constitute approval.
- C. Contractor Representations: By making substitution request, Contractor:
 - 1. Recognizes burden of proof of equality for requested substitution rests with Contractor.
 - 2. Represents and warrants that Contractor has personally investigated requested substitution and determined that it is equal to or superior in all respects to specified Work.
 - 3. Represents and warrants that Contractor will provide same warranties for requested substitution that Contractor would for specified Work.
 - 4. Certifies that cost data presented is complete and includes all related costs under this Contract except for Architect's redesign cost, and waives all claims for additional costs related to requested substitution which may subsequently become apparent.
 - 5. Will coordinate installation of accepted substitution, making such other changes as may be required to make Work complete in all respects.

SUBSTITUTION PROCEDURES

6. Represents and warrants that accepted substitution will perform same as specified Work would have performed. Should accepted substitution fail to perform as required, Contractor shall replace accepted substitution with specified Work at no additional cost to Owner.
- D. Substitutions for Cause: Submit requests for substitution immediately on discovery of need for change.
1. Conditions: Architect will consider Contractor's request for substitution when the following conditions are satisfied. If the following conditions are not satisfied, Architect will return requests without action, except to record noncompliance with these requirements:
 - a. Requested substitution is consistent with the Contract Documents and will produce indicated results.
 - b. Requested substitution provides sustainable design characteristics that specified product provided.
 - c. Substitution request is fully documented and properly submitted.
 - d. Requested substitution will not adversely affect Contractor's construction schedule.
 - e. Requested substitution has received necessary approvals of authorities having jurisdiction.
 - f. Requested substitution is compatible with other portions of the Work.
 - g. Requested substitution has been coordinated with other portions of the Work.
 - h. Requested substitution provides specified warranty.
- E. Substitutions for Convenience:
1. Conditions: Architect will consider Contractor's request for substitution when the following conditions are satisfied. If the following conditions are not satisfied, Architect will return requests without action, except to record noncompliance with these requirements:
 - a. Requested substitution offers Owner a substantial advantage in cost, time, energy conservation, or other considerations, after deducting additional responsibilities Owner must assume. Owner's additional responsibilities may include compensation to Architect for redesign and evaluation services, increased cost of other construction by Owner, and similar considerations.
 - b. Requested substitution does not require extensive revisions to the Contract Documents.
 - c. Requested substitution is consistent with the Contract Documents and will produce indicated results.
 - d. Requested substitution provides sustainable design characteristics that specified product provided.
 - e. Substitution request is fully documented and properly submitted.
 - f. Requested substitution will not adversely affect Contractor's construction schedule.
 - g. Requested substitution has received necessary approvals of authorities having jurisdiction.
 - h. Requested substitution is compatible with other portions of the Work.
 - i. Requested substitution has been coordinated with other portions of the Work.
 - j. Requested substitution provides specified warranty.

PART 3 - EXECUTION (Not Used)

END OF SECTION

Substitution Request Form 012500a

To Contract Manager _____

Project No. _____ Project Name _____

Specified Item _____

Section	Page	Paragraph	Description
---------	------	-----------	-------------

The undersigned General Contractor requests consideration of the following:

Proposed Substitution _____
 (Include all product data as indicated in Specification Section 01 2500 and any supplemental information as requested by the Architect.)

The undersigned General Contractor warrants to the Architect and Owner that the following paragraphs, unless modified on attachments, are correct.

1. The Proposed Substitution does not affect dimensions shown on Drawings.
2. The cost reduction/increase indicated in item 5 below includes costs for changes to the building design, including engineering, design, detailing and construction costs caused by the requested Substitution. Any additional costs resulting from this substitution will be reimbursed from the cost savings in item 5 or, in it's absence, funded as a project cost.
3. The Proposed Substitution will have no adverse affect on other trades, the construction schedule, or specified warranty requirements.
4. Maintenance and service parts will be locally available for the Proposed Substitution.

The General Contractor further warrants to the Architect and Owner that the function and quality of the Proposed Substitution are equivalent or superior to the Specified Item. The General Contractor further warrants that specification section 01 2500, paragraph 2.1G intent has been met.

5. Total Cost Savings/Increase to the Owner: \$ _____
6. Compensation to the Architect / Consultant for related Additional Service Fee: \$ _____

Manufacturer's Certification of Equal Quality

I _____ represent the manufacturer of the Proposed Substitution item and hereby certify and warrant to the Architect and Owner that the function and quality of the Proposed Substitution are equivalent or superior to the Specified Item.

_____	_____	_____
Manufacturer's Representative	Date	Company

Acceptances

1.	_____	_____	_____
	General Contractor Acceptance	Date	Company

2.	_____	_____	_____
	Owner Acceptance	Date	Company

3.	_____	_____	_____
	Architect Acceptance	Date	Company

Recommend Acceptance: Yes No

SECTION 01 2600

CONTRACT MODIFICATION PROCEDURES

PART 1 - GENERAL

1.1 SUMMARY

- A. Section includes administrative and procedural requirements for handling and processing Contract modifications.

1.2 MINOR CHANGES IN THE WORK

- A. Architect will issue supplemental instructions authorizing minor changes in the Work, not involving adjustment to the Contract Sum or the Contract Time, on Architect's Form HKS-710 "Architect's Supplemental Instructions"; copy attached at the end of this Section.

1.3 PROPOSAL REQUESTS

- A. Owner-Initiated Proposed Change: Architect will issue a detailed description of proposed changes in the Work that may require adjustment to the Contract Sum or the Contract Time using Architect's Form HKS-709 "Proposed Change"; copy attached at the end of this Section. If necessary, the description will include supplemental or revised Drawings and Specifications.
 - 1. Proposed Changes issued by Architect are not instructions either to stop work in progress or to execute the proposed change.
 - 2. Within time specified in Proposed Change or with reasonable promptness, when not otherwise specified, after receipt of Proposed Change, submit a quotation estimating cost adjustments to the Contract Sum and the Contract Time necessary to execute the change.
 - a. Include a list of quantities of products required or eliminated and unit costs, with total amount of purchases and credits to be made. If requested, furnish survey data to substantiate quantities.
 - b. Indicate applicable taxes, delivery charges, equipment rental, and amounts of trade discounts.
 - c. Include costs of labor and supervision directly attributable to the change.
 - d. Include an updated Contractor's construction schedule that indicates the effect of the change, including, but not limited to, changes in activity duration, start and finish times, and activity relationship. Use available total float before requesting an extension of the Contract Time.
 - e. Include updated Submittal Schedule showing effect of the change.
- B. Contractor-Initiated Proposed Change: If latent or changed conditions require modifications to the Contract, Contractor may initiate a claim by submitting a request for a change to Architect using Contractor's Standard Form.
 - 1. Include a statement outlining reasons for the change and the effect of the change on the Work. Provide a complete description of the proposed change. Indicate the effect of the proposed change on the Contract Sum and the Contract Time.

2. Include a list of quantities of products required or eliminated and unit costs, with total amount of purchases and credits to be made. If requested, furnish survey data to substantiate quantities.
3. Indicate applicable taxes, delivery charges, equipment rental, and amounts of trade discounts.
4. Include costs of labor and supervision directly attributable to the change.
5. Include an updated Contractor's construction schedule that indicates the effect of the change, including, but not limited to, changes in activity duration, start and finish times, and activity relationship. Use available total float before requesting an extension of the Contract Time.
6. Include updated Submittal Schedule showing effect of the change.
7. Comply with requirements in Division 01 Section "Substitution Procedures" if the proposed change requires substitution of one product or system for product or system specified.

1.4 ADMINISTRATIVE CHANGE ORDERS

- A. Allowance Adjustment: If applicable, see Division 01 Section "Allowances" for administrative procedures for preparation of Proposed Change for adjusting the Contract Sum to reflect actual costs of allowances.
- B. Unit-Price Adjustment: If applicable, see Division 01 Section "Unit Prices" for administrative procedures for preparation of Proposed Change for adjusting the Contract Sum to reflect measured scope of unit-price work.
- C. Alternates: If applicable, see Division 01 Section "Alternates" for administrative procedures for preparation of Proposed Change for adjusting the Contract Sum to reflect measured scope of alternate work.

1.5 CHANGE ORDER PROCEDURES

- A. On Owner's approval of a Proposed Change, Architect will issue a Change Order for signatures of Owner and Contractor on Architects Form HKS-701 "Change Order"; copy attached at the end of this Section.

1.6 CONSTRUCTION CHANGE DIRECTIVE

- A. Construction Change Directive: Architect may issue a Construction Change Directive on Architects Form HKS-714 "Construction Change Directive"; copy attached at the end of this Section. Construction Change Directive instructs Contractor to proceed with a change in the Work, for subsequent inclusion in a Change Order.
 1. Construction Change Directive contains a complete description of change in the Work. It also designates method to be followed to determine change in the Contract Sum or the Contract Time.
- B. Documentation: Maintain detailed records on a time and material basis of work required by the Construction Change Directive.
 1. After completion of change, submit an itemized account and supporting data necessary to substantiate cost and time adjustments to the Contract.

PART 2 - PRODUCTS (Not Used)

PART 3 - EXECUTION (Not Used)

END OF SECTION

**18-01.01 WPMHC Expansion
Childers Architect
2019-11-01**

CONTRACT MODIFICATION PROCEDURES

01 2600 - 4

SECTION 01 2900

PAYMENT PROCEDURES

PART 1 - GENERAL

1.1 SUMMARY

- A. Section includes administrative and procedural requirements necessary to prepare and process Applications for Payment.

1.2 SCHEDULE OF VALUES

- A. Coordination: Coordinate preparation of the schedule of values with preparation of Contractor's construction schedule.
 - 1. Coordinate line items in the schedule of values with other required administrative forms and schedules, including the following:
 - a. Application for Payment forms with continuation sheets.
 - b. Submittal schedule.
 - c. Accepted Alternates.
 - 2. Submit the schedule of values to Architect at earliest possible date, but no later than seven days before the date scheduled for submittal of initial Applications for Payment.
 - 3. Subschedules for Phased Work: Where the Work is separated into phases requiring separately phased payments; provide subschedules showing values coordinated with each phase of payment.
 - 4. Subschedules for Separate Elements of Work: Where the Contractor's construction schedule defines separate elements of the Work; provide subschedules showing values coordinated with each element.
 - 5. Subschedules for Separate Design Contracts: Where the Owner has retained design professionals under separate contracts who will each provide certification of payment requests, provide subschedules showing values coordinated with the scope of each design services contract as described in Division 01 Section "Summary."
- B. Format and Content: Use Project Manual table of contents as a guide to establish line items for the schedule of values. Provide at least one line item for each Specification Section.
 - 1. Identification: Include the following Project identification on the schedule of values:
 - a. Project name and location.
 - b. Name of Architect.
 - c. Architect's project number.
 - d. Contractor's name and address.
 - e. Date of submittal.
 - 2. Arrange the schedule of values in tabular form, in format accepted by Architect, with separate columns to indicate the following for each item listed:
 - a. Related Specification Section or Division.
 - b. Description of the Work.

- c. Name of subcontractor.
 - d. Name of manufacturer or fabricator.
 - e. Name of supplier.
 - f. Change Orders.
 - g. Dollar value of the following, as a percentage of the Contract Sum to nearest one-hundredth percent, adjusted to total 100 percent.
 - 1) Labor.
 - 2) Materials.
 - 3) Equipment.
3. Provide a breakdown of the Contract Sum in enough detail to facilitate continued evaluation of Applications for Payment and progress reports. Coordinate with Project Manual table of contents. Provide multiple line items for principal subcontract amounts, where appropriate.
 4. Round amounts to nearest whole dollar; total shall equal the Contract Sum.
 5. Provide a separate line item in the schedule of values for each part of the Work where Applications for Payment may include materials or equipment purchased or fabricated and stored, but not yet installed.
 - a. Differentiate between items stored on-site and items stored off-site. If required, include evidence of insurance or bonded warehousing.
 6. Provide separate line items in the schedule of values for initial cost of materials, for each subsequent stage of completion, and for total installed value of that part of the Work.
 7. Allowances (If Applicable): Provide a separate line item in the schedule of values for each allowance. Show line-item value of unit-cost allowances (if applicable), as a product of the unit cost, multiplied by measured quantity. Use information indicated in the Contract Documents to determine quantities.
 8. Alternates (If Applicable): Provide a separate line item in the schedule of values for each accepted alternate.
 9. Change Orders: Provide a separate line item in the schedule of values for each change order.
 10. Separate Owner-Consultant Contracts: Provide a separate line item in the schedule of values for each separate Owner-Consultant related Work item.
 11. Purchase Contracts: When applicable, provide a separate line item in the schedule of values for each purchase contract. Show line-item value of purchase contract. Indicate owner payments or deposits, if any, and balance to be paid by Contractor.
 12. Each item in the schedule of values and Applications for Payment shall be complete. Include total cost and proportionate share of general overhead and profit for each item.
 - a. Temporary facilities and other major cost items that are not direct cost of actual work-in-place may be shown either as separate line items in the schedule of values or distributed as general overhead expense, at Contractor's option.
 13. Schedule Updating: Update and resubmit the schedule of values before the next Applications for Payment when Change Orders or Construction Change Directives result in a change in the Contract Sum.

1.3 ARCHITECTS COST DATA

- A. In addition to the Schedule of Values, submit itemized cost data reporting on Architect's Form "Project Cost Summary", included at the end of this section. Initial submission shall be included with contractors first Application for Payment. Final updated submission shall be included with contractors final Application for Payment.

1.4 APPLICATIONS FOR PAYMENT

- A. Each Application for Payment following the initial Application for Payment shall be consistent with previous applications and payments as certified by Architect and paid for by Owner.
 - 1. Initial Application for Payment, Application for Payment at time of Substantial Completion, and final Application for Payment involve additional requirements.
- B. Payment Application Times: The date for each progress payment is indicated in the Agreement between Owner and Contractor. The period of construction work covered by each Application for Payment is the period indicated in the Agreement.
 - 1. If the Agreement does not state payment dates, establish dates at preconstruction conference.
 - 2. Submit draft, or pencil, copy of Application for Payment seven days prior to due date for review by Architect.
- C. Application for Payment Forms: Unless directed otherwise by Owner, use AIA Document G702 and AIA Document G703 as form for Applications for Payment.
- D. Application Preparation: Complete every entry on form. Notarize and execute by a person authorized to sign legal documents on behalf of Contractor. Architect will return incomplete applications without action.
 - 1. Entries shall match data on the schedule of values and Contractor's construction schedule. Use updated schedules if revisions were made.
 - 2. Include amounts of Change Orders and Construction Change Directives issued before last day of construction period covered by application.
- E. Stored Materials: If accepted by Owner, include in Application for Payment amounts applied for materials or equipment purchased or fabricated and stored, but not yet installed. Differentiate between items stored on-site and items stored off-site.
 - 1. Provide certificate of insurance, evidence of transfer of title to Owner, and consent of surety to payment, for stored materials.
 - 2. Provide supporting documentation that verifies amount requested, such as paid invoices. Match amount requested with amounts indicated on documentation; do not include overhead and profit on stored materials.
 - 3. Provide summary documentation for stored materials indicating the following:
 - a. Value of materials previously stored and remaining stored as of date of previous Applications for Payment.
 - b. Value of previously stored materials put in place after date of previous Application for Payment and on or before date of current Application for Payment.
 - c. Value of materials stored since date of previous Application for Payment and remaining stored as of date of current Application for Payment.
- F. Transmittal: Submit 3 signed and notarized original copies of each Application for Payment to Architect by a method ensuring receipt within 24 hours. One copy shall include waivers of lien and similar attachments if required.
 - 1. Transmit each copy with a transmittal form listing attachments and recording appropriate information about application.

- G. Waivers of Mechanic's Lien: With each Application for Payment, submit waivers of mechanic's liens from General Contractor, subcontractors, sub-subcontractors, and suppliers for construction period covered by the previous application.
1. Submit partial waivers on each item for amount requested in previous application, after deduction for retainage, on each item.
 2. When an application shows completion of an item, submit conditional final or full waivers.
 3. Owner reserves the right to designate which entities involved in the Work must submit waivers.
 4. Waiver Delays: Submit each Application for Payment with Contractor's waiver of mechanic's lien for construction period covered by the application.
 - a. Submit final Application for Payment with or preceded by conditional final waivers from every entity involved with performance of the Work covered by the application who is lawfully entitled to a lien.
 5. Waiver Forms: Submit executed waivers of lien on forms, acceptable to Owner.
- H. Initial Application for Payment: Administrative actions and submittals that must precede or coincide with submittal of first Application for Payment include the following:
1. List of subcontractors.
 2. Schedule of values.
 3. Contractor's construction schedule (preliminary if not final).
 4. Products list (preliminary if not final).
 5. Schedule of unit prices.
 6. Submittal schedule (preliminary if not final).
 7. List of Contractor's staff assignments.
 8. List of Contractor's principal consultants.
 9. Copies of building permits.
 10. Copies of authorizations and licenses from authorities having jurisdiction for performance of the Work.
 11. Initial progress report.
 12. Report of preconstruction conference.
 13. Certificates of insurance and insurance policies.
 14. Performance and payment bonds.
- I. Application for Payment at Substantial Completion: After Architect issues the Certificate of Substantial Completion, submit an Application for Payment showing 100 percent completion for portion of the Work claimed as substantially complete.
1. Include documentation supporting claim that the Work is substantially complete and a statement showing an accounting of changes to the Contract Sum.
 2. When applicable, this application shall reflect Certificate(s) of **Partial** Substantial Completion issued previously for Owner occupancy of designated portions of the Work.
- J. Final Payment Application: Submit final Application for Payment with releases and supporting documentation not previously submitted and accepted, including, but not limited, to the following:
1. Evidence of completion of Project closeout requirements.
 2. Insurance certificates for products and completed operations where required and proof that taxes, fees, and similar obligations were paid.
 3. Updated final statement, accounting for final changes to the Contract Sum.
 4. AIA Document G706, "Contractor's Affidavit of Payment of Debts and Claims."

5. AIA Document G706A, "Contractor's Affidavit of Release of Liens."
6. AIA Document G707, "Consent of Surety to Final Payment."
7. Evidence that claims have been settled.
8. Final meter readings for utilities, a measured record of stored fuel, and similar data as of date of Substantial Completion or when Owner took possession of and assumed responsibility for corresponding elements of the Work.
9. If applicable, final liquidated damages settlement statement.

PART 2 - PRODUCTS (Not Used)

PART 3 - EXECUTION (Not Used)

END OF SECTION



Project Cost Summary

Project Name _____ Project No. _____ Latest Update _____

Start Construction Date _____ Substantial Completion Date _____

Submitted By _____

Building Type: (Select One Only)

- Entertainment Commercial Education Hospitality
 Healthcare Religion Residential

Structure Type: (Select One Only)

- Concrete Steel

Government/Justice

SITework CATEGORY	SF FOR EACH AREA	COSTS			
		BLDG. NEW CONST	GARAGE	SITework	BLDG. RENOVATION
1. Sitework/Excavation					
2. Paving/Walks/Curbs					
3. Hardscape					
TOTAL SITework		\$	\$	\$	\$

BUILDING/GARAGE CATEGORY	SF FOR EACH AREA	COSTS			
		BLDG. NEW CONST.	GARAGE	SITework	BLDG. RENOVATION
4. Masonry					
5. Stucco/Plaster					
6. Waterproofing					
7. Insulation/Roofing					
8. Glass Curtain Wall					
9. Stone Exterior					
10. Precast Concrete					
11. Lobby Finish					
12. Pavers					
13. Toilet Partitions					
14. Toilet Accessories					
15. Elevators & Escalators					
16. Specialties					
17. Resilient/Carpet					
TOTAL EACH AREA		\$	\$	\$	\$

BUILDING/GARAGE CATEGORY	REMARKS	COSTS			
		BLDG. NEW CONST. BLDG. NEW CONST. AREA _SF	GARAGE GARAGE AREA _SF	SITWORK SITWORK AREA _SF	BLDG. RENOVATION BLDG. RENOV. AREA _SF
18. Overhead/General Conditions					
19. General Contractor Fees					
20. Demolition					
21. Utilities					
22. Foundations					
23. Structural Frame					
24. Fireproofing					
25. Misc. Metals					
26. Carpentry/Millwork					
27. Doors and Frames					
28. Finish Hardware					
29. Gyp. Wallboard					
30. Painting					
31. Ceramic Tile					
32. Acoustical Ceiling					
33. Plumbing					
34. Fire Sprinklers					
35. Electrical					
36. Energy Management					
37. HVAC/Equipment					
38. Security System					
39. Kitchen Equipment					
40.					
41.					
42.					
43.					
44.					
45. Change Order Total					
TOTAL EACH AREA		\$	\$	\$	\$

TOTAL PROJECT COST \$ _____

SECTION 01 3100

PROJECT MANAGEMENT AND COORDINATION

PART 1 - GENERAL

1.1 SUMMARY

- A. Section includes administrative provisions for coordinating construction operations on Project including, but not limited to, the following:
 - 1. General coordination procedures.
 - 2. Coordination drawings.
 - 3. Requests for Information (RFIs).
 - 4. Project Web site.
 - 5. Project meetings.

1.2 DEFINITIONS

- A. Project communications documents shall be defined as the following:
 - 1. Letters.
 - 2. Memoranda.
 - 3. E-Mail Communications/Internet Communications/Project Management Software Communications.
 - 4. RFI (Request for Information - Contractor).
 - 5. RFI-A (Request for Information - Architect).

1.3 FORMAT

- A. Letters and Memoranda: Submit in formats acceptable to the Architect.
- B. E-Mail Communications/Internet Communications/Project Management Software Communications: Submit in forms and formats acceptable to and as approved by the Architect.
- C. RFI (Request for Information - Contractor): Submit on forms furnished by the Architect, or on other forms as approved by the Architect. Unless otherwise approved use Architect's Form HKS-750, "Request for Information"; copy attached at the end of this Section.
- D. RFI-A (Request for Information - Architect), will be submitted by Architect to Contractor on Architects standard form.

1.4 PROJECT COMMUNICATIONS DOCUMENTS

- A. Letters and Memoranda documents shall be submitted in a timely manner so as to facilitate project delivery and coordination. Routing of communications shall be as established in the Contract, the Contract Documents and the Pre-Construction Conference. Communications documents shall be transmitted or forwarded in a manner consistent with the schedule and progress of the work.

- B. E-Mail Communications, Internet Communications, and Project Management Software programs must be compatible with the Architect's and Owner's computer systems and equipment. The responsibility for all costs for management of these systems, including, but not limited to, licensing, onsite training or other training necessary for the proper operation of such systems, shall be by the Contractor. The Contractor shall keep written records and hard file copies of all electronic communications. Failure of the Contractor to keep such records shall waive the Contractor's right to rely on such communications and such communications shall be deemed to have not taken place.
- C. RFI (Request for Information - Contractor) shall be defined and limited to a request from the Contractor seeking interpretation or clarification of the requirements of the Contract Documents. Such requests shall comply with the following requirements:
1. RFI requests shall be submitted in a timely manner, well in advance of related work, and allow sufficient time for the resolution of issues relating to the request for interpretation or clarification. Contractor shall schedule the submission of RFI's so as to moderate and manage the flow of RFI requests. RFI's shall be submitted in a manner consistent with the schedule and progress of the work, and shall not be submitted in a sporadic and/or excessive manner.
 2. RFI requests shall be numbered in a sequential manner and contain a detailed description of the areas of work requiring interpretation or clarification. Include drawing and specification references, sketches, technical data, brochures, or other supporting data as deemed necessary by the Architect, for the Architect to provide the interpretations and clarifications requested.
 - a. The Contractor shall include a "Proposed Solution" to the issue requiring interpretation or clarification.
 3. RFI's submitted to the Contractor by Sub-Contractors, vendors, suppliers, or other parties to the work shall be reviewed by the Contractor prior to submission to the Architect. If the Architect deems that such RFI requests have not been adequately reviewed by the Contractor, such requests will be returned to the Contractor for further action. Sub-Contractor's RFI shall contain a "Proposed Solution".
 4. RFI requests shall not contain submittals, substitutions requests, routine communications, correspondence, memos, claims, or any information required by other areas of the Contract Documents. RFI requests containing such information will be returned to the Contractor without action by the Architect.
 5. RFI requests are limited to a request for interpretation or clarification of the requirements of the Contract Documents. Interpretations provided by the Architect shall not change the requirements of the Contract or the Contract Documents. If the Contractor determines that the Architect's response to an RFI gives cause for a change in the Contract or the Contract Documents, the Contractor shall promptly, within 5 working days, give written notice to the Architect of request for adjustments. Requests for adjustments to the Contract shall be submitted in a manner consistent with the terms and conditions of the Contract Documents.
 6. If the Architect, after review, determines that any RFI has been submitted in an incomplete manner, is unnecessary, or does not otherwise comply with the requirements of this Section, the RFI will be returned without action to the Contractor. The Contractor shall delete the original submittal date from the RFI log and enter a new submittal date at the time of re-submittal.
 7. RFI Log: Prepare, maintain, and submit a tabular log of RFIs organized by the RFI number. Submit log weekly. Use software log that is part of Project Web site. Software log with not less than the following:

- a. Project name.
 - b. Name and address of Contractor.
 - c. Name and address of Architect.
 - d. RFI number including RFIs that were returned without action or withdrawn.
 - e. RFI description.
 - f. Date the RFI was submitted.
 - g. Date Architect's response was received.
8. On receipt of Architect's action, update the RFI log and immediately distribute the RFI response to affected parties. Review response and notify Architect within seven days if Contractor disagrees with response.
- a. Identification of related Minor Change in the Work, Construction Change Directive, and Proposal Request, as appropriate.
- D. RFI-A (Request for Information - Architect) shall be defined as a request by the Architect for information relating to the obligations of the Contractor under the Contract.
- 1. After receipt of an RFI-A the Contractor shall provide a written response to the Architect within 5 working days. Responses shall be thorough, complete and shall contain all information requested by the Architect.
 - 2. An RFI-A shall be limited to a request by the Architect for information related to the project. The RFI-A shall not be construed as authorizing or directing a change in the Contract or the Contract Documents.
- E. Revisions to Construction Documents: Responses to requests for information (RFI) shall not serve as construction documents; and the Contractor shall not incorporate RFI responses into construction of the Project, unless such answers bear the seal and signature of a licensed design professional.

1.5 INFORMATIONAL SUBMITTALS

- A. Subcontract List: Prepare a written summary identifying individuals or firms proposed for each portion of the Work, including those who are to furnish products or equipment fabricated to a special design. Include the following information in tabular form:
- 1. Name, address, and telephone number of entity performing subcontract or supplying products.
 - 2. Number and title of related Specification Section(s) covered by subcontract.
 - 3. Drawing number and detail references, as appropriate, covered by subcontract.
- B. Key Personnel Names: Within 15 days of starting construction operations, submit a list of key personnel assignments, including superintendent and other personnel in attendance at Project site. Identify individuals and their duties and responsibilities; list addresses and telephone numbers, including home, office, and cellular telephone numbers and e-mail addresses. Provide names, addresses, and telephone numbers of individuals assigned as alternates in the absence of individuals assigned to Project.
- 1. Post copies of list in project meeting room, in temporary field office, and Project Web site. Keep list current at all times.

1.6 GENERAL COORDINATION PROCEDURES

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- A. Coordination: Coordinate construction operations included in different Sections of the Specifications to ensure efficient and orderly installation of each part of the Work. Coordinate construction operations, included in different Sections, that depend on each other for proper installation, connection, and operation.
 - 1. Schedule construction operations in sequence required to obtain the best results where installation of one part of the Work depends on installation of other components, before or after its own installation.
 - 2. Coordinate installation of different components to ensure maximum performance and accessibility for required maintenance, service, and repair.
 - 3. Make adequate provisions to accommodate items scheduled for later installation.
- B. Prepare memoranda for distribution to each party involved, outlining special procedures required for coordination. Include such items as required notices, reports, and list of attendees at meetings.
 - 1. Prepare similar memoranda for Owner and separate contractors if coordination of their Work is required.
- C. Administrative Procedures: Coordinate scheduling and timing of required administrative procedures with other construction activities to avoid conflicts and to ensure orderly progress of the Work. Such administrative activities include, but are not limited to, the following:
 - 1. Preparation of Contractor's construction schedule.
 - 2. Preparation of the schedule of values.
 - 3. Installation and removal of temporary facilities and controls.
 - 4. Delivery and processing of submittals.
 - 5. Progress meetings.
 - 6. Preinstallation conferences.
 - 7. Project closeout activities.
 - 8. Startup and adjustment of systems.
- D. Conservation: Coordinate construction activities to ensure that operations are carried out with consideration given to conservation of energy, water, and materials.

1.7 COORDINATION DRAWINGS

- A. Coordination Drawings, General: Prepare coordination drawings according to requirements in individual Sections, and additionally where installation is not completely shown on Shop Drawings, where limited space availability necessitates coordination, or if coordination is required to facilitate integration of products and materials fabricated or installed by more than one entity.
 - 1. Content: Project-specific information, drawn accurately to a scale large enough to indicate and resolve conflicts. Do not base coordination drawings on standard printed data. Include the following information, as applicable:
 - a. Use applicable Drawings as a basis for preparation of coordination drawings. Prepare sections, elevations, and details as needed to describe relationship of various systems and components.
 - b. Indicate functional and spatial relationships of components of architectural, structural, civil, mechanical, and electrical systems.

- c. Indicate space requirements for routine maintenance and for anticipated replacement of components during the life of the installation.
 - d. Show location and size of access doors required for access to concealed dampers, valves, and other controls.
 - e. Indicate required installation sequences.
 - f. Indicate dimensions shown on the Drawings. Specifically note dimensions that appear to be in conflict with submitted equipment and minimum clearance requirements. Provide alternate sketches to Architect indicating proposed resolution of such conflicts. Minor dimension changes and difficult installations will not be considered changes to the Contract.
- B. Coordination Digital Data Files: Prepare coordination digital data files according to the following requirements:
- 1. File Preparation Format: Same digital data software program, version, and operating system as original Drawings.
 - 2. File Submittal Format: Submit or post coordination drawing files using Portable Data File (PDF) format.
 - 3. BIM File Incorporation: When applicable, develop coordination drawing files from Building Information Model (BIM) established for Project.
 - a. Perform three-dimensional component conflict analysis as part of preparation of coordination drawings. Resolve component conflicts prior to submittal. Indicate where conflict resolution requires modification of design requirements by Architect.
 - 4. If approved by Owner, Architect will furnish Contractor one set of digital data files of Drawings for use in preparing coordination digital data files.
 - a. Architect makes no representations as to the accuracy or completeness of digital data files as they relate to Drawings.
 - b. Digital Data Software Program: Drawings are available in Autodesk Revit and/or Autocad; and compatible with Microsoft Windows operating system.
 - c. Distribution: Digital data files shall only be distributed via the HKS Thru site with acceptance of HKS data licensing agreement.

1.8 PROJECT WEB SITE

- A. Use Architect's Project Web site implementing Architect's electronic project management software system for purposes of managing project communication and documentation until Final Completion.
- B. Contractor, subcontractors, and other parties granted access to Project Web site shall execute a data licensing agreement in the form of AIA Document C106.

1.9 PROJECT MEETINGS

- A. General: Schedule and conduct meetings and conferences at Project site unless otherwise indicated.
 - 1. Attendees: Inform participants and others involved, and individuals whose presence is required, of date and time of each meeting. Notify Owner and Architect of scheduled meeting dates and times.

2. Agenda: Prepare the meeting agenda. Distribute the agenda to all invited attendees.
 3. Minutes: Record significant discussions and agreements achieved. Distribute the meeting minutes to everyone concerned, including Owner and Architect, within 3 days of the meeting.
 4. Attendance: Document attendance of all participants.
- B. Preconstruction Conference: Architect will schedule and conduct a preconstruction conference before starting construction.
1. Conduct the conference to review responsibilities and personnel assignments.
 2. Attendees: Authorized representatives of Owner, Architect, and their consultants; Contractor and its superintendent; major subcontractors; suppliers; and other concerned parties shall attend the conference. Participants at the conference shall be familiar with Project and authorized to conclude matters relating to the Work.
- C. Preinstallation Conferences: Conduct a preinstallation conference at Project site before each construction activity that requires coordination with other construction.
1. Attendees: Installer and representatives of manufacturers and fabricators involved in or affected by the installation and its coordination or integration with other materials and installations that have preceded or will follow, shall attend the meeting. Advise Architect of scheduled meeting dates.
 2. Agenda: Review progress of other construction activities and preparations for the particular activity under consideration, and coordination with adjacent activities. Prepare agenda appropriate to Work.
 3. Record significant conference discussions, agreements, and disagreements, including required corrective measures and actions.
 4. Reporting: Distribute minutes of the meeting to each party present and to other parties requiring information.
 5. Do not proceed with installation if the conference cannot be successfully concluded. Initiate whatever actions are necessary to resolve impediments to performance of the Work and reconvene the conference at earliest feasible date.
- D. Project Closeout Conference: Schedule and conduct a project closeout conference, at a time convenient to Owner and Architect, at a time to be decided prior to the scheduled date of Substantial Completion.
1. Conduct the conference to review requirements and responsibilities related to Project closeout.
 2. Attendees: Authorized representatives of Owner, Architect, and their consultants; Contractor and its superintendent; major subcontractors; suppliers; and other concerned parties shall attend the meeting. Participants at the meeting shall be familiar with Project and authorized to conclude matters relating to the Work.
 3. Agenda: Discuss items of significance that could affect or delay Project closeout, including the following:
 - a. Preparation of record documents.
 - b. Procedures required prior to inspection for Substantial Completion and for final inspection for acceptance.
 - c. Submittal of written warranties.
 - d. If applicable, requirements for completing sustainable design documentation.
 - e. Requirements for preparing operations and maintenance data.

- f. Requirements for delivery of material samples, attic stock, and spare parts.
 - g. Requirements for demonstration and training.
 - h. Preparation of Contractor's punch list.
 - i. Procedures for processing Applications for Payment at Substantial Completion and for final payment.
 - j. Submittal procedures.
 - k. If applicable, coordination of separate contracts.
 - l. If applicable, Owner's partial occupancy requirements.
 - m. Installation of Owner's furniture, fixtures, and equipment.
 - n. Responsibility for removing temporary facilities and controls.
4. Minutes: Entity conducting meeting will record and distribute meeting minutes.
- E. Progress Meetings: Conduct progress meetings at regular intervals.
- 1. Coordinate dates of meetings with preparation of payment requests.
 - 2. Attendees: In addition to representatives of Owner and Architect, each contractor, subcontractor, supplier, and other entity concerned with current progress or involved in planning, coordination, or performance of future activities shall be represented at these meetings. All participants at the meeting shall be familiar with Project and authorized to conclude matters relating to the Work.
 - 3. Agenda: Review and correct or approve minutes of previous progress meeting. Review other items of significance that could affect progress. Include topics for discussion as appropriate to status of Project.
 - a. Contractor's Construction Schedule: Review progress since the last meeting. Determine whether each activity is on time, ahead of schedule, or behind schedule, in relation to Contractor's construction schedule. Determine how construction behind schedule will be expedited; secure commitments from parties involved to do so. Discuss whether schedule revisions are required to ensure that current and subsequent activities will be completed within the Contract Time.
 - 1) Review schedule for next period.
 - b. Review present and future needs of each entity present, including the following or as needed:
 - 1) Interface requirements.
 - 2) Sequence of operations.
 - 3) If applicable, resolution of BIM component conflicts.
 - 4) Status of submittals.
 - 5) If applicable, status of sustainable design documentation.
 - 6) Deliveries.
 - 7) Off-site fabrication.
 - 8) Access.
 - 9) Site utilization.
 - 10) Temporary facilities and controls.
 - 11) Work hours.
 - 12) Hazards and risks.
 - 13) Progress cleaning.
 - 14) Quality and work standards.
 - 15) Status of correction of deficient items.

- 16) Field observations.
 - 17) Status of RFIs.
 - 18) Status of proposal requests.
 - 19) Pending changes.
 - 20) Status of Change Orders.
 - 21) Documentation of information for payment requests.
4. Minutes: Entity responsible for conducting the meeting will record and distribute the meeting minutes to each party present and to parties requiring information.
 - a. Schedule Updating: Revise Contractor's construction schedule after each progress meeting where revisions to the schedule have been made or recognized. Issue revised schedule concurrently with the report of each meeting.
- F. Coordination Meetings: Conduct Project coordination meetings on an as-needed basis. Project coordination meetings are in addition to specific meetings held for other purposes, such as progress meetings and preinstallation conferences.
1. Attendees: Each contractor, subcontractor, supplier, and other entity concerned with current progress or involved in planning, coordination, or performance of future activities shall be represented at these meetings. All participants at the meetings shall be familiar with Project and authorized to conclude matters relating to the Work.
 2. Agenda: Review items of significance that could affect progress. Include topics for discussion as appropriate to status of Project.
 - a. Review present and future needs of each contractor present, including the following:
 - 1) Interface requirements.
 - 2) Sequence of operations.
 - 3) If applicable, resolution of BIM component conflicts.
 - 4) Status of submittals.
 - 5) Deliveries.
 - 6) Off-site fabrication.
 - 7) Access.
 - 8) Site utilization.
 - 9) Temporary facilities and controls.
 - 10) Work hours.
 - 11) Hazards and risks.
 - 12) Progress cleaning.
 - 13) Quality and work standards.
 - 14) Change Orders.

PART 2 - PRODUCTS

2.1 ELECTRONIC PROJECT MANAGEMENT SOFTWARE

- A. General: So as to expedite electronic review process, process all documents through a web-based software service. Sending documents via email, FTP or paper will not be accepted.
 1. Basis of Design (Product Standard):

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- a. Newforma, Inc.; Newforma Project Cloud, web-based software.
 - 1) Website: www.NewformaProjectCloud.com
 - 2) E-mail: projectcloud@newforma.com
 - 3) Phone: (800) 303-4650

B. Performance Requirements:

1. Project License:
 - a. Cloud based (no hardware required).
 - b. Unlimited user accounts.
 - c. Functionality to support subcontractors, contractors, architects and consultants.
 - d. Provide access to data for all project team members at no cost to the individual users.
2. Training and Support:
 - a. Dedicated project training.
 - b. Phone support.
3. Archive:
 - a. Export all data to an offline archive at the completion of the project.
 - b. Provide archive to architect, contractor and owner.
 - c. Archive shall include all attachments, meta data, review comments and time stamp history.
4. Submittals and RFIs:
 - a. Customizable logs and reporting accessible by all users.
 - b. Logs shall automatically update as submittals and RFIs are processed.
 - c. Automated routing of submittals and RFIs to design team based on trade.
 - d. Automated email notifications when submittal or RFI has been assigned or returned to a user.
 - e. Automated weekly email to design team users of overdue items.
 - f. Automatic sequential numbering per spec section for submittals.
 - g. Two sets of due dates - one overall due date and a consultant due date.
 - h. Built-in web-based markup tools to support a concurrent review of submittal and RFI.
5. Submittal Register:
 - a. Software vendor shall take specifications and build the required list of submittals and import into the software.
6. Drawing Management:
 - a. Provide current set of drawings and specifications through a centralized index.
 - b. Automated association of PDFs to the centralized index.
 - c. Manage drawing revisions with customizable review states.
 - d. Drawings shall be accessible offline via mobile devices.

7. File Sharing:
 - a. Integrated file sharing tool (FTP) to transfer any miscellaneous files such as BIM and CAD files.
 - b. Access permissions (view/edit) at a folder level.

8. Punch List and other Field Task Management:
 - a. Unlimited customizable field task types including punch list.
 - b. Locate and assign tasks from a mobile device.
 - c. No additional fees to individual users to access mobile apps.
 - d. Data shall be accessible offline on mobile devices.

PART 3 - EXECUTION (Not Used)

END OF SECTION

SECTION 01 3200

CONSTRUCTION PROGRESS DOCUMENTATION

PART 1 - GENERAL

1.1 SUMMARY

- A. Section includes administrative and procedural requirements for documenting the progress of construction during performance of the Work.

1.2 DEFINITIONS

- A. Activity: A discrete part of a project that can be identified for planning, scheduling, monitoring, and controlling the construction project. Activities included in a construction schedule consume time and resources.
 - 1. Critical Activity: An activity on the critical path that must start and finish on the planned early start and finish times.
 - 2. Predecessor Activity: An activity that precedes another activity in the network.
 - 3. Successor Activity: An activity that follows another activity in the network.
- B. Major Area: A story of construction, a separate building, or a similar significant construction element.
- C. Milestone: A key or critical point in time for reference or measurement.
- D. CPM: Critical path method, which is a method of planning and scheduling a construction project where activities are arranged based on activity relationships. Network calculations determine when activities can be performed and the critical path of Project.
- E. Critical Path: The longest connected chain of interdependent activities through the network schedule that establishes the minimum overall Project duration and contains no float.
- F. Network Diagram: A graphic diagram of a network schedule, showing activities and activity relationships.
- G. Event: The starting or ending point of an activity.
- H. Float: The measure of leeway in starting and completing an activity.
 - 1. Float time is not for the exclusive use or benefit of either Owner or Contractor, but is a jointly owned, expiring Project resource available to both parties as needed to meet schedule milestones and Contract completion date.
 - 2. Free float is the amount of time an activity can be delayed without adversely affecting the early start of the successor activity.
 - 3. Total float is the measure of leeway in starting or completing an activity without adversely affecting the planned Project completion date.
- I. Fagnets: A partial or fragmentary network that breaks down activities into smaller activities for greater detail.

1.3 SUBMITTALS

- A. Format for Submittals: Submit required submittals in the following format:
 - 1. PDF electronic file.
- B. Startup construction schedule.
- C. Startup Network Diagram: Of size required to display entire network for entire construction period. Show logic ties for activities.
- D. Contractor's Construction Schedule: Initial schedule, of size required to display entire schedule for entire construction period.
- E. CPM Reports: Concurrent with CPM schedule, submit each of the following reports. Format for each activity in reports shall contain activity number, activity description, original duration, remaining duration, early start date, early finish date, late start date, late finish date, and total float in calendar days.
 - 1. Activity Report: List of all activities sorted by activity number and then early start date, or actual start date if known.
 - 2. Logic Report: List of preceding and succeeding activities for all activities, sorted in ascending order by activity number and then early start date, or actual start date if known.
 - 3. Total Float Report: List of all activities sorted in ascending order of total float.
 - 4. Earnings Report: Compilation of Contractor's total earnings from the Notice to Proceed until most recent Application for Payment.
- F. Construction Schedule Updating Reports: Submit with Applications for Payment.
- G. Daily Construction Reports: Submit at weekly intervals.
- H. Material Location Reports: Submit at monthly intervals.
- I. Site Condition Reports: Submit at time of discovery of differing conditions.
- J. Special Reports: Submit at time of unusual event.

1.4 COORDINATION

- A. Coordinate preparation and processing of schedules and reports with performance of construction activities and with scheduling and reporting of separate contractors.
- B. Coordinate Contractor's construction schedule with the schedule of values, list of subcontracts, submittal schedule, progress reports, payment requests, and other required schedules and reports.
 - 1. Secure time commitments for performing critical elements of the Work from entities involved.
 - 2. Coordinate each construction activity in the network with other activities and schedule them in proper sequence.

PART 2 - PRODUCTS

2.1 CONTRACTOR'S CONSTRUCTION SCHEDULE, GENERAL

- A. Time Frame: Extend schedule from date established for the Notice to Proceed to date of final completion.
1. Contract completion date shall not be changed by submission of a schedule that shows an early completion date, unless specifically authorized by Change Order.
- B. Activities: Treat each story or separate area as a separate numbered activity for each main element of the Work. Comply with the following:
1. Procurement Activities: Include procurement process activities for the following long lead items and major items, requiring a cycle of more than 60 days, as separate activities in schedule. Procurement cycle activities include, but are not limited to, submittals, approvals, purchasing, fabrication, and delivery.
 2. Submittal Review Time: Include review and resubmittal times indicated in Division 01 Section "Submittal Procedures" in schedule. Coordinate submittal review times in Contractor's construction schedule with submittal schedule.
 3. Startup and Testing Time: Include no fewer than 7 days for startup and testing.
 4. Substantial Completion: Indicate completion in advance of date established for Substantial Completion, and allow time for Architect's administrative procedures necessary for certification of Substantial Completion.
 5. Punch List and Final Completion: Include not more than 30 days for completion of punch list items and final completion.
- C. Constraints: Include constraints and work restrictions indicated in the Contract Documents and as follows in schedule (where applicable), and show how the sequence of the Work is affected.
1. Phasing: Arrange list of activities on schedule by phase.
 2. Work by Owner: Include a separate activity for each portion of the Work performed by Owner.
 3. Products Ordered in Advance: Include a separate activity for each product.
 4. Owner-Furnished Products: Include a separate activity for each product.
 5. Work Restrictions: Show the effect of the following items on the schedule:
 - a. Coordination with existing construction.
 - b. Limitations of continued occupancies.
 - c. Uninterruptible services.
 - d. Partial occupancy before Substantial Completion.
 - e. Use of premises restrictions.
 - f. Provisions for future construction.
 - g. Seasonal variations.
 - h. Environmental control.
 6. Work Stages: Indicate important stages of construction for each major portion of the Work, including, but not limited to, the following:
 - a. Subcontract awards.
 - b. Submittals.
 - c. Purchases.
 - d. Mockups.
 - e. Fabrication.

- f. Sample testing.
 - g. Deliveries.
 - h. Installation.
 - i. Tests and inspections.
 - j. Adjusting.
 - k. Curing.
 - l. Building flush-out.
 - m. Startup and placement into final use and operation.
7. Construction Areas: Identify each major area of construction for each major portion of the Work. Indicate where each construction activity within a major area must be sequenced or integrated with other construction activities to provide for the following:
- a. Structural completion.
 - b. Temporary enclosure and space conditioning.
 - c. Permanent space enclosure.
 - d. Completion of mechanical installation.
 - e. Completion of electrical installation.
 - f. Substantial Completion.
8. Other Constraints include but are not limited to the following:
- a. Roads.
 - b. Parking.
 - c. Landscape.
- D. Milestones: Include milestones indicated in the Contract Documents in schedule, including, but not limited to, the Notice to Proceed, punch list activities, Substantial Completion, and final completion.
- E. Recovery Schedule: When periodic update indicates the Work is 14 or more calendar days behind the current approved schedule, submit a separate recovery schedule indicating means by which Contractor intends to regain compliance with the schedule. Indicate changes to working hours, working days, crew sizes, and equipment required to achieve compliance, and date by which recovery will be accomplished.
- F. Computer Scheduling Software: Prepare schedules using current version of a program that has been developed specifically to manage construction schedules.

2.2 STARTUP CONSTRUCTION SCHEDULE

- A. Bar-Chart Schedule: Submit startup, horizontal, bar-chart-type construction schedule within 14 days of date established for the Notice of Award.
- B. Preparation: Indicate each significant construction activity separately. Identify first workday of each week with a continuous vertical line. Outline significant construction activities for first 60 days of construction. Include skeleton diagram for the remainder of the Work and a cash requirement prediction based on indicated activities.
- C. Startup Network Diagram may be submitted in lieu of Bar-Chart Schedule.

2.3 CONTRACTOR'S CONSTRUCTION SCHEDULE (BAR CHART/GANTT CHART)

- A. Bar Chart/Gantt Chart Schedule: Submit a comprehensive, fully developed, horizontal, Gantt-chart-type, Contractor's construction schedule within 30 days of date established for the Notice to Proceed. Base schedule on the startup construction schedule and additional information received since the start of Project.
- B. Preparation: Indicate each significant construction activity separately. Identify first workday of each week with a continuous vertical line.
 - 1. For construction activities that require three months or longer to complete, indicate an estimated completion percentage in 10 percent increments within time bar.

2.4 CONTRACTOR'S CONSTRUCTION SCHEDULE (CPM SCHEDULE)

- A. General: Prepare network diagrams using AON (activity-on-node) format.
- B. Startup Network Diagram: Submit diagram within 14 days of date established for the Notice to Proceed. Outline significant construction activities for the first 60 days of construction. Include skeleton diagram for the remainder of the Work and a cash requirement prediction based on indicated activities.
- C. CPM Schedule: Prepare Contractor's construction schedule using a time-scaled CPM network analysis diagram for the Work.
 - 1. Develop network diagram in sufficient time to submit CPM schedule so it can be accepted for use no later than 30 days after date established for the Notice to Proceed.
 - a. Failure to include any work item required for performance of this Contract shall not excuse Contractor from completing all work within applicable completion dates, regardless of Architect's approval of the schedule.
 - 2. Conduct educational workshops to train and inform key Project personnel, including subcontractors' personnel, in proper methods of providing data and using CPM schedule information.
 - 3. Establish procedures for monitoring and updating CPM schedule and for reporting progress. Coordinate procedures with progress meeting and payment request dates.
 - 4. Use "one workday" as the unit of time for individual activities. Indicate nonworking days and holidays incorporated into the schedule in order to coordinate with the Contract Time.
- D. CPM Schedule Preparation: Prepare a list of all activities required to complete the Work. Using the startup network diagram, prepare a skeleton network to identify probable critical paths.
 - 1. Activities: Indicate the estimated time duration, sequence requirements, and relationship of each activity in relation to other activities. Include estimated time frames for the following activities:
 - a. Preparation and processing of submittals.
 - b. Purchase of materials.
 - c. Delivery.
 - d. Fabrication.
 - e. Installation.
 - f. Punch list and final completion.

2. Critical Path Activities: Identify critical path activities, including those for interim completion dates. Scheduled start and completion dates shall be consistent with Contract milestone dates.
 3. Processing: Process data to produce output data on a computer-drawn, time-scaled network. Revise data, reorganize activity sequences, and reproduce as often as necessary to produce the CPM schedule within the limitations of the Contract Time.
 4. Format: Mark the critical path. Locate the critical path near center of network; locate paths with most float near the edges.
 - a. Subnetworks on separate sheets are permissible for activities clearly off the critical path.
- E. Contract Modifications: For each proposed contract modification and concurrent with its submission, prepare a time-impact analysis using a network fragment, fragnet, to demonstrate the effect of the proposed change on the overall project schedule.
- F. Initial Issue of Schedule: Prepare initial network diagram from a sorted activity list indicating straight "early start-total float." Identify critical activities. Prepare tabulated reports showing the following:
1. Contractor or subcontractor and the Work or activity.
 2. Description of activity.
 3. Main events of activity.
 4. Immediate preceding and succeeding activities.
 5. Early and late start dates.
 6. Early and late finish dates.
 7. Activity duration in workdays.
 8. Total float or slack time.
 9. Average size of workforce.
 10. Dollar value of activity (coordinated with the schedule of values).
- G. Schedule Updating: Concurrent with making revisions to schedule, prepare tabulated reports showing the following:
1. Identification of activities that have changed.
 2. Changes in early and late start dates.
 3. Changes in early and late finish dates.
 4. Changes in activity durations in workdays.
 5. Changes in the critical path.
 6. Changes in total float or slack time.
 7. Changes in the Contract Time.
- H. Value Summaries: Prepare two cumulative value lists, sorted by finish dates.
1. In first list, tabulate activity number, early finish date, dollar value, and cumulative dollar value.
 2. In second list, tabulate activity number, late finish date, dollar value, and cumulative dollar value.
 3. In subsequent issues of both lists, substitute actual finish dates for activities completed as of list date.
 4. Prepare list for ease of comparison with payment requests; coordinate timing with progress meetings.

- a. In both value summary lists, tabulate "actual percent complete" and "cumulative value completed" with total at bottom.
- b. Submit value summary printouts one week before each regularly scheduled progress meeting.

2.5 REPORTS

- A. Daily Construction Reports: Prepare a daily construction report recording the following information concerning events at Project site:
 1. List of subcontractors at Project site.
 2. List of separate contractors at Project site.
 3. Approximate count of personnel at Project site.
 4. High and low temperatures and general weather conditions, including presence of rain or snow.
 5. Accidents.
 6. Meetings and significant decisions.
 7. Unusual events (see special reports).
 8. Stoppages, delays, shortages, and losses.
 9. Meter readings and similar recordings.
 10. Emergency procedures.
 11. Orders and requests of authorities having jurisdiction.
 12. Change Orders received and implemented.
 13. Construction Change Directives received and implemented.
 14. Services connected and disconnected.
 15. Equipment or system tests and startups.
 16. Partial completions and occupancies.
 17. Substantial Completions authorized.
- B. Material Location Reports: At monthly intervals, prepare and submit a comprehensive list of materials delivered to and stored at Project site. List shall be cumulative, showing materials previously reported plus items recently delivered. Include with list a statement of progress on and delivery dates for materials or items of equipment fabricated or stored away from Project site.
- C. Site Condition Reports: Immediately on discovery of a difference between site conditions and the Contract Documents, prepare and submit a detailed report. Submit with a Request for Information. Include a detailed description of the differing conditions, together with recommendations for changing the Contract Documents.

2.6 SPECIAL REPORTS

- A. General: Submit special reports directly to Owner within one day(ε) of an occurrence. Distribute copies of report to parties affected by the occurrence.
- B. Reporting Unusual Events: When an event of an unusual and significant nature occurs at Project site, whether or not related directly to the Work, prepare and submit a special report. List chain of events, persons participating, response by Contractor's personnel, evaluation of results or effects, and similar pertinent information. Advise Owner in advance when these events are known or predictable.

PART 3 - EXECUTION

3.1 CONTRACTOR'S CONSTRUCTION SCHEDULE

- A. Scheduling Consultant: Engage a consultant to provide planning, evaluation, and reporting using CPM scheduling.
 - 1. In-House Option: Owner may waive the requirement to retain a consultant if Contractor employs skilled personnel with experience in CPM scheduling and reporting techniques. Submit qualifications.
 - 2. Meetings: Scheduling consultant shall attend all meetings related to Project progress, alleged delays, and time impact.
- B. Contractor's Construction Schedule Updating: At monthly intervals, update schedule to reflect actual construction progress and activities. Issue schedule one week before each regularly scheduled progress meeting.
 - 1. Revise schedule immediately after each meeting or other activity where revisions have been recognized or made. Issue updated schedule concurrently with the report of each such meeting.
 - 2. Include a report with updated schedule that indicates every change, including, but not limited to, changes in logic, durations, actual starts and finishes, and activity durations.
 - 3. As the Work progresses, indicate final completion percentage for each activity.
- C. Distribution: Distribute copies of approved schedule to Architect-Owner, separate contractors, testing and inspecting agencies, and other parties identified by Contractor with a need-to-know schedule responsibility.
 - 1. Post copies in Project meeting rooms and temporary field offices.
 - 2. When revisions are made, distribute updated schedules to the same parties and post in the same locations. Delete parties from distribution when they have completed their assigned portion of the Work and are no longer involved in performance of construction activities.

3.2 CONSTRUCTION PHOTOGRAPHS

- A. Photographer: Engage a qualified professional commercial photographer to take electronic construction photographs.
- B. Minimum Digital Camera Resolution: 1800 x 1200 dpi (dots per inch) @ 72 dpi resolution.
- C. Acceptable Electronic File Format: .jpg, .tif, .tiff, .tga, .jpe, or .png.
- D. Date Stamp: Unless otherwise indicated, date and time stamp each photograph as it is being taken so stamp is integral to photograph.
- E. Image File Naming Convention (separate by an underscore _):
 - 1. Project Job Number / Year-Month-Day / Image Number . file extension
- F. Print Format: 8 in (200 mm) by 10 in (250 mm) smooth surface matte prints on single-weight commercial-grade stock, mounted on linen or card stock to allow a 1 in (25 mm) wide margin and enclosed back to back in clear plastic sleeves that are punched for standard 3-ring binder.

- G. Print Identification: On back of each print, provide an applied label or rubber-stamped impression with the following information:
1. Name of Project.
 2. Name and address of photographer.
 3. Name of Architect.
 4. Name of Contractor.
 5. Date photograph was taken.
 6. Description of vantage point, indicating location, direction (by compass point), and elevation or story of construction.
- H. Preconstruction Photographs: Before starting construction, take 4 photographs of Project site and surrounding properties from different vantage points, as directed by Architect. Show existing conditions adjacent to property. Submit prints and CD ROMs with digital files as required under "Submittals" Article.
- I. Periodic Construction Photographs: Take 4 photographs monthly, coinciding with cutoff date associated with each Application for Payment. Photographer shall select vantage points to best show status of construction and progress since last photographs were taken. Submit prints and CD ROMs with digital files as required under "Submittals" Article.
1. Field Office Prints: In addition to prints required to be submitted under "Submittals" Article, make and retain in field office at Project site available at all times for reference, one set of prints of periodic construction photographs. Identify photographs the same as for those submitted to Architect.
- J. Final Completion Construction Photographs: Take 8 photographs after date of Substantial Completion for submission as Project Record Documents. Architect will direct photographer for desired vantage points. Submit prints and CD ROMs with digital files as required under "Submittals" Article.

END OF SECTION

**18-01.01 WPMHC Expansion
Childers Architect
2019-11-01**

**CONSTRUCTION PROGRESS
DOCUMENTATION**

SECTION 013300

SUBMITTAL PROCEDURES

PART 1 - GENERAL

1.1 SUMMARY

- A. Section includes requirements for the submittal schedule and administrative and procedural requirements for submitting Shop Drawings, Product Data, Samples, and other submittals.

1.2 DEFINITIONS

- A. Action Submittals: Written and graphic information and physical samples that require Architect's review. Architect's responsive action is required.
- B. Digital Signature: A digital signature or digital signature scheme is a mathematical scheme for demonstrating the authenticity of a digital message or document.
- C. Electronic Signature: An electronic signature is any legally recognized electronic means that indicates that a person adopts the contents of an electronic message.
- D. Informational Submittals: Written and graphic information other than action submittals that require Architect's review. Architect's responsive action is required on informational submittals that do not comply with the information given and design concept expressed in the Drawings and Specifications.
- E. Portable Document Format (PDF): An open standard file format licensed by Adobe Systems used for representing documents in a device-independent and display resolution-independent fixed-layout document format.

1.3 SUBMITTALS

- A. Submittal Schedule: Submit a schedule of submittals, arranged in chronological order by dates required by construction schedule. Include time required for review, ordering, manufacturing, fabrication, and delivery when establishing dates. Include additional time required for making corrections or revisions to submittals noted by Architect and additional time for handling and reviewing submittals required by those corrections.
 - 1. Coordinate submittal schedule with list of subcontracts, the schedule of values, and Contractor's construction schedule.
 - 2. Initial Submittal: Submit concurrently with startup construction schedule. Include submittals required during the first 60 days of construction. List those submittals required to maintain orderly progress of the Work and those required early because of long lead time for manufacture or fabrication.
 - 3. Final Submittal: Submit concurrently with the first complete submittal of Contractor's construction schedule.
 - a. Submit revised submittal schedule to reflect changes in current status and timing for submittals.

4. Format: Arrange the following information in a tabular format:
 - a. Scheduled date for first submittal.
 - b. Specification Section number and title.
 - c. Submittal category: Action; informational.
 - d. Name of subcontractor.
 - e. Description of the Work covered.
 - f. Scheduled date for Architect's final release or approval.

1.4 SUBMITTAL ADMINISTRATIVE REQUIREMENTS

- A. Architect's Digital Data Files: If approved by the by Owner, Architect will furnish Contractor one set of digital data files of Drawing files for use in preparing submittals. Architect makes no representations as to the accuracy or completeness of digital data files as they relate to Drawings. .
 1. Distribution: Digital data files shall only be distributed via the HKS Thru site with acceptance of HKS data licensing agreement.
- B. Coordination: Coordinate preparation and processing of submittals with performance of construction activities.
 1. Coordinate each submittal with fabrication, purchasing, testing, delivery, other submittals, and related activities that require sequential activity.
 2. Submit all submittal items required for each Specification Section concurrently unless partial submittals for portions of the Work are indicated on approved submittal schedule.
 3. Coordinate transmittal of different types of submittals for related parts of the Work so processing will not be delayed because of need to review submittals concurrently for coordination.
 - a. Architect reserves the right to withhold action on a submittal requiring coordination with other submittals until related submittals are received.
- C. Processing Time: Allow sufficient and reasonable time for submittal review, including time for resubmittals. Time for review shall commence on Architect's receipt of submittal. No extension of the Contract Time will be authorized because of failure to transmit submittals enough in advance of the Work to permit processing, including resubmittals.
 1. The Architect's action will be taken in accordance with the submittal schedule approved by the Architect or, in the absence of an approved submittal schedule, with reasonable promptness while allowing sufficient time in the Architect's professional judgment to permit adequate review.
- D. Paper Submittals: Architect reserves the right to require paper submittals.
- E. Electronic Submittals: Identify and incorporate information in each electronic submittal file as follows:
 1. Assemble complete submittal package into a single indexed file incorporating submittal requirements of a single Specification Section and transmittal form with links enabling navigation to each item.
 2. Name file with submittal number, including revision identifier.

- a. File Naming Convention (separate by dashes - or underscores _):
 - 1) Specification Number / Revision Number / Submittal Sequence (A, B, C, etc.).pdf
- 3. Provide means for insertion to permanently record Contractor's review and approval markings and action taken by Architect.
- 4. Transmittal Form for Electronic Submittals: Use software-generated form from electronic project management software or electronic form acceptable to Owner, containing the following information:
 - a. Project name.
 - b. Date.
 - c. Name and address of Architect.
 - d. Name of Contractor.
 - e. Name of firm or entity that prepared submittal.
 - f. Names of subcontractor, manufacturer, and supplier.
 - g. Category and type of submittal.
 - h. Submittal purpose and description.
 - i. Specification Section number and title.
 - j. Specification paragraph number or drawing designation and generic name for each of multiple items.
 - k. Drawing number and detail references, as appropriate.
 - l. Location(s) where product is to be installed, as appropriate.
 - m. Related physical samples submitted directly.
 - n. Indication of full or partial submittal.
 - o. Transmittal number, numbered consecutively.
 - p. Submittal and transmittal distribution record.
 - q. Other necessary identification.
 - r. Remarks.
- 5. Metadata: Include the following information as keywords in the electronic submittal file metadata:
 - a. HKS Job Number and Add Service Number (e.g. 14424.000)
 - b. Project name.
 - c. Number and title of appropriate Specification Section.
 - d. Manufacturer name.
 - e. Product name.
 - f. Submittal revision number.
- 6. Utilize electronic project management software program to process submittals when feasible with the type and extent of submittals. Refer to Division 01 Section "Project Management and Coordination" for description of electronic project management software.
- F. Options: Identify options requiring selection by Architect.
- G. Deviations and Additional Information: On page, prepared on Contractor's letterhead, record relevant information, requests for data, revisions other than those requested by Architect on previous submittals, and deviations from requirements in the Contract Documents, including minor variations and limitations. Include same identification information as related submittal.
- H. Resubmittals: Make resubmittals in same form as initial submittal.

1. Note date and content of previous submittal.
 2. Note date and content of revision in file name and clearly indicate extent of revision.
 3. Resubmit submittals until they are marked with notation from Architect's action stamp not requiring additional submittals.
- I. Distribution: Furnish electronic copies of final submittals to manufacturers, subcontractors, suppliers, fabricators, installers, authorities having jurisdiction, and others as necessary for performance of construction activities. Show distribution on transmittal forms.
- J. Use for Construction: Retain complete copies of submittals on Project site. Use only final action submittals that are marked with appropriate notation from Architect's action stamp indicating for construction. Retain a separate copy for Owner to be delivered to Owner with Project Closeout documents.

PART 2 - PRODUCTS

2.1 SUBMITTAL PROCEDURES

- A. General Submittal Procedure Requirements: Prepare and submit submittals required by individual Specification Sections. Types of submittals are indicated in individual Specification Sections.
1. Submit electronic submittals as PDF electronic files directly to Architect's Project Web site specifically established for Project.
 - a. Architect will return annotated file. Annotate and retain one copy of file as an electronic Project record document file.
 - b. Provide PDF electronic files from scanned paper originals at 300 dpi, minimum.
 2. Certificates and Certifications Submittals: Provide a statement that includes signature of entity responsible for preparing certification. Certificates and certifications shall be signed by an officer or other individual authorized to sign documents on behalf of that entity.
 - a. Provide a digital signature with digital certificate on electronically submitted certificates and certifications where indicated.
 - b. When one or more individual Specification Sections includes requirements for notarized signature on certificates and certifications, provide a notarized statement on original paper copy certificates and certifications where indicated.
- B. Product Data: Collect information into a single submittal for each element of construction and type of product or equipment.
1. If information must be specially prepared for submittal because standard published data are not suitable for use, submit as Shop Drawings, not as Product Data.
 2. Mark each copy of each submittal to show which products and options are applicable.
 3. Include the following information, as applicable:
 - a. Manufacturer's written recommendations.
 - b. Manufacturer's installation instructions.
 - c. Mill reports.
 - d. Standard product operating and maintenance manuals.

- e. Certification that products are appropriate for installation indicated.
 - f. Manufacturer's catalog cuts.
 - g. Manufacturer's product specifications.
 - h. Standard color charts.
 - i. Statement of compliance with specified referenced standards.
 - j. Testing by recognized testing agency.
 - k. Application of testing agency labels and seals.
 - l. Notation of coordination requirements.
 - m. Availability and delivery time information.
4. For equipment, include the following in addition to the above, as applicable:
- a. Wiring diagrams showing factory-installed wiring.
 - b. Printed performance curves.
 - c. Operational range diagrams.
 - d. Clearances required to other construction, if not indicated on accompanying Shop Drawings.
5. Submit Product Data before or concurrent with Samples.
6. Submit Product Data in the following format:
- a. PDF electronic file.
- C. Shop Drawings: Prepare Project-specific information, drawn accurately to scale. Do not base Shop Drawings on reproductions of the Contract Documents or standard printed data.
1. Preparation: Fully illustrate requirements in the Contract Documents. Include the following information, as applicable:
- a. Identification of products.
 - b. Dimensions.
 - c. Fabrication and installation drawings.
 - d. Roughing-in and setting diagrams.
 - e. Wiring diagrams showing field-installed wiring, including power, signal, and control wiring.
 - f. Shopwork manufacturing instructions.
 - g. Templates and patterns.
 - h. Design calculations.
 - i. Schedules.
 - j. Compliance with specified standards.
 - k. Notation of coordination requirements.
 - l. Notation of dimensions established by field measurement.
 - m. Relationship and attachment to adjoining construction clearly indicated.
 - n. Seal and signature of professional engineer.
2. Wiring Diagrams: Differentiate between manufacturer-installed and field-installed wiring.
3. Sheet Size: Except for templates, patterns, and similar full-size drawings, submit Shop Drawings on sheets at least 8-1/2 by 11 inches (212 by 275 mm), but no larger than 30 by 42 inches (750 by 1050 mm).
4. Submit Shop Drawings in the following format:
- a. PDF electronic file.

5. BIM File Incorporation: When applicable, develop and incorporate Shop Drawing files into Building Information Model established for Project.
 - a. Prepare Shop Drawings in the following format: Same digital data software program, version, and operating system as the original Drawings.
 - b. Refer to Division 01 Section "Project Management and Coordination" for requirements for coordination drawings.
- D. Samples: Submit Samples for review of kind, color, pattern, and texture for a check of these characteristics with other elements and for a comparison of these characteristics between submittal and actual component as delivered and installed.
 1. Transmit Samples that contain multiple, related components such as accessories together in one submittal package.
 2. Identification: Attach label on unexposed side of Samples that includes the following:
 - a. Generic description of Sample.
 - b. Product name and name of manufacturer.
 - c. Sample source.
 - d. Number and title of applicable Specification Section.
 - e. Specification paragraph number and generic name of each item.
 3. For projects where electronic submittals are required, provide corresponding electronic submittal of Sample transmittal, digital image file illustrating Sample characteristics, and identification information for record. This is in addition to physical samples.
 4. Disposition: Maintain sets of approved Samples at Project site, available for quality-control comparisons throughout the course of construction activity. Sample sets may be used to determine final acceptance of construction associated with each set.
 - a. Samples that may be incorporated into the Work are indicated in individual Specification Sections. Such Samples must be in an undamaged condition at time of use.
 - b. Samples not incorporated into the Work, or otherwise designated as Owner's property, are the property of Contractor.
 5. Samples for Initial Selection: Submit manufacturer's color charts consisting of units or sections of units showing the full range of colors, textures, and patterns available.
 - a. Number of Samples: Submit 4 full set(s) of available choices where color, pattern, texture, or similar characteristics are required to be selected from manufacturer's product line. Architect will return submittal with options selected.
 6. Samples for Verification: Submit full-size units or Samples of size indicated, prepared from same material to be used for the Work, cured and finished in manner specified, and physically identical with material or product proposed for use, and that show full range of color and texture variations expected. Samples include, but are not limited to, the following: partial sections of manufactured or fabricated components; small cuts or containers of materials; complete units of repetitively used materials; swatches showing color, texture, and pattern; color range sets; and components used for independent testing and inspection.

- a. Number of Samples: Submit 4 sets of Samples. Architect will retain 2 Sample sets; remainder will be returned. Mark up and retain one returned Sample set as a project record sample.
 - 1) Submit a single Sample where assembly details, workmanship, fabrication techniques, connections, operation, and other similar characteristics are to be demonstrated.
 - 2) If variation in color, pattern, texture, or other characteristic is inherent in material or product represented by a Sample, submit at least 4 sets of paired units that show approximate limits of variations.
- E. Coordination Drawing Submittals: Comply with requirements specified in Division 01 Section "Project Management and Coordination."
- F. Contractor's Construction Schedule: Comply with requirements specified in Division 01 Section "Construction Progress Documentation."
- G. Application for Payment and Schedule of Values: Comply with requirements specified in Division 01 Section "Payment Procedures."
- H. Test and Inspection Reports and Schedule of Tests and Inspections Submittals: Comply with requirements specified in Division 01 Section "Quality Requirements."
- I. Closeout Submittals and Maintenance Material Submittals: Comply with requirements specified in Division 01 Section "Closeout Procedures."
- J. Maintenance Data: Comply with requirements specified in Division 01 Section "Operation and Maintenance Data."
- K. Sustainable Construction Submittals: Where applicable, comply with requirements specified in Division 01 sustainable construction requirements Section.
- L. Qualification Data: Prepare written information that demonstrates capabilities and experience of firm or person. Include lists of completed projects with project names and addresses, contact information of architects and owners, and other information specified.
- M. Welding Certificates: Prepare written certification that welding procedures and personnel comply with requirements in the Contract Documents. Submit record of Welding Procedure Specification and Procedure Qualification Record on AWS forms. Include names of firms and personnel certified.
- N. Installer Certificates: Submit written statements on manufacturer's letterhead certifying that Installer complies with requirements in the Contract Documents and, where required, is authorized by manufacturer for this specific Project.
- O. Manufacturer Certificates: Submit written statements on manufacturer's letterhead certifying that manufacturer complies with requirements in the Contract Documents. Include evidence of manufacturing experience where required.
- P. Product Certificates: Submit written statements on manufacturer's letterhead certifying that product complies with requirements in the Contract Documents.

- Q. Material Certificates: Submit written statements on manufacturer's letterhead certifying that material complies with requirements in the Contract Documents.
- R. Material Test Reports: Submit reports written by a qualified testing agency, on testing agency's standard form, indicating and interpreting test results of material for compliance with requirements in the Contract Documents.
- S. Product Test Reports: Submit written reports indicating that current product produced by manufacturer complies with requirements in the Contract Documents. Base reports on evaluation of tests performed by manufacturer and witnessed by a qualified testing agency, or on comprehensive tests performed by a qualified testing agency.
- T. Research Reports: Submit written evidence, from a model code organization acceptable to authorities having jurisdiction, that product complies with building code in effect for Project. Include the following information:
 - 1. Name of evaluation organization.
 - 2. Date of evaluation.
 - 3. Time period when report is in effect.
 - 4. Product and manufacturers' names.
 - 5. Description of product.
 - 6. Test procedures and results.
 - 7. Limitations of use.
- U. Preconstruction Test Reports: Submit reports written by a qualified testing agency, on testing agency's standard form, indicating and interpreting results of tests performed before installation of product, for compliance with performance requirements in the Contract Documents.
- V. Compatibility Test Reports: Submit reports written by a qualified testing agency, on testing agency's standard form, indicating and interpreting results of compatibility tests performed before installation of product. Include written recommendations for primers and substrate preparation needed for adhesion.
- W. Field Test Reports: Submit written reports indicating and interpreting results of field tests performed either during installation of product or after product is installed in its final location, for compliance with requirements in the Contract Documents.
- X. Design Data: Prepare and submit written and graphic information, including, but not limited to, performance and design criteria, list of applicable codes and regulations, and calculations. Include list of assumptions and other performance and design criteria and a summary of loads. Include load diagrams if applicable. Provide name and version of software, if any, used for calculations. Include page numbers.

2.2 DELEGATED-ENGINEERING SERVICES

- A. Performance and Design Criteria: Where professional design services or certifications by a design professional are specifically required of Contractor by the Contract Documents, provide products and systems complying with specific performance and design criteria indicated.
 - 1. If criteria indicated are not sufficient to perform services or certification required, submit a written request for additional information to Architect.

- B. Delegated-Engineering Services Certification: In addition to Shop Drawings, Product Data, and other required submittals, submit digitally signed PDF electronic file and three paper copies of certificate, signed and sealed by the responsible design professional, for each product and system specifically assigned to Contractor to be designed or certified by a design professional.
 - 1. Indicate that products and systems comply with performance and design criteria in the Contract Documents. Include list of codes, loads, and other factors used in performing these services.
- C. BIM File Incorporation: Where applicable, incorporate delegated-design drawing and data files into Building Information Model established for Project.
 - 1. Prepare delegated-design drawings in the following format: Same digital data software program, version, and operating system as the original Drawings.

PART 3 - EXECUTION

3.1 CONTRACTOR'S REVIEW

- A. Action and Informational Submittals: Review each submittal and check for coordination with other Work of the Contract and for compliance with the Contract Documents. Note corrections and field dimensions. Mark with approval stamp before submitting to Architect.
- B. Project Closeout and Maintenance Material Submittals: See requirements in Division 01 Section "Closeout Procedures."
- C. Approval Stamp: Stamp each submittal with a uniform, approval stamp. Include Project name and location, submittal number, Specification Section title and number, name of reviewer, date of Contractor's approval, and statement certifying that submittal has been reviewed, checked, and approved for compliance with the Contract Documents.

3.2 ARCHITECT'S ACTION

- A. Action Submittals: Architect will review submittal, make marks to indicate corrections or revisions required, and return it. Architect will stamp and mark submittal appropriately, as follows:
 - 1. Final but Restricted Release: When submittals are marked "Accepted as Noted," the Work covered by the submittal may proceed provided it complies with both the Architect's notations and corrections on the submittal and requirements of the Contract Documents. Final acceptance will depend on that compliance.
 - 2. Returned for Resubmittal: When submittal is marked "Revise Resubmit," do not proceed with the Work covered by the submittal, including purchasing, fabrication, delivery, or other activity. Revise or prepare a new submittal in accordance with the Architect's notations. Resubmit without delay. Repeat if necessary to obtain a different action mark.
 - a. Do not permit submittals marked "Revise Resubmit" to be used at the Project site, or elsewhere where construction is in progress.
 - 3. Submittals Not Required: Where a submittal is primarily for other Contractor activity, the submittal will be returned, marked "Not Reviewed; Submittal not required by Contract Documents".

- B. Architect's acceptance of Shop Drawings, Samples or Product Data which deviates from the Contract Documents does not authorize changes to the Contract Sum. Submit in writing at the time of submission any changes to the Contract Sum affected by such Shop Drawings, Samples or Product Data, otherwise, claim for extras will not be considered.
- C. Informational Submittals: Architect will review submittal, and return it in accordance with submittal Processing Time indicated if it does not comply with requirements. Architect will stamp and mark submittal appropriately.
- D. Partial submittals prepared for a portion of the Work will be reviewed when use of partial submittals has received prior approval from Architect. Review shall not be final until complete submittal has been reviewed by Architect.
- E. Incomplete submittals are unacceptable, will be considered nonresponsive, and will be returned for resubmittal without review.
- F. Submittals not required by the Contract Documents may be returned by the Architect without action.
- G. Electronic File of Submittal Documents: Provide Architect with an independent electronic archive of project submittal documents using electronic project management software as defined in Division 01 Section "Project Management and Coordination".

END OF SECTION

SECTION 01 4000

QUALITY REQUIREMENTS

PART 1 - GENERAL

1.1 SUMMARY

- A. Section includes administrative and procedural requirements for quality assurance and quality control.
- B. Testing and inspecting services are required to verify compliance with requirements specified or indicated. These services do not relieve Contractor of responsibility for compliance with the Contract Document requirements.
 - 1. Specific quality-assurance and -control requirements for individual construction activities are specified in the Sections that specify those activities. Requirements in those Sections may also cover production of standard products.
 - 2. Specified tests, inspections, and related actions do not limit Contractor's other quality-assurance and -control procedures that facilitate compliance with the Contract Document requirements.
 - 3. Requirements for Contractor to provide quality-assurance and -control services required by Architect, Owner, or authorities having jurisdiction are not limited by provisions of this Section.

1.2 DEFINITIONS

- A. Quality-Assurance Services: Activities, actions, and procedures performed before and during execution of the Work to guard against defects and deficiencies and substantiate that proposed construction will comply with requirements.
- B. Quality-Control Services: Tests, inspections, procedures, and related actions during and after execution of the Work to evaluate that actual products incorporated into the Work and completed construction comply with requirements. Services do not include contract enforcement activities performed by Architect.
- C. Mockups: Full-size physical assemblies that are constructed on-site. Mockups are constructed to verify selections made under Sample submittals; to demonstrate aesthetic effects and, where indicated, qualities of materials and execution; to review coordination, testing, or operation; to show interface between dissimilar materials; and to demonstrate compliance with specified installation tolerances. Mockups are not Samples. Mockups establish the standard by which the Work will be judged.
 - 1. Integrated Exterior Mockups: Mockups of the exterior envelope erected separately from the building but on Project site, consisting of multiple products, assemblies, and subassemblies.
 - 2. Room Mockups: Mockups of typical interior spaces complete with wall, floor, and ceiling finishes, doors, windows, millwork, casework, specialties, furnishings and equipment, and lighting.
 - 3. Laboratory Mockups: Full-size physical assemblies constructed at testing facility to verify performance characteristics.

- D. Preconstruction Testing: Tests and inspections performed specifically for Project before products and materials are incorporated into the Work, to verify performance or compliance with specified criteria.
- E. Product Testing: Tests and inspections that are performed by an NRTL (Nationally Recognized Testing Laboratories), an NVLAP (National Voluntary Laboratory Accreditation Program), or a testing agency qualified to conduct product testing and acceptable to authorities having jurisdiction, to establish product performance and compliance with specified requirements.
- F. Source Quality-Control Testing: Tests and inspections that are performed at the source, e.g., plant, mill, factory, or shop.
- G. Field Quality-Control Testing: Tests and inspections that are performed on-site for installation of the Work and for completed Work.
- H. Testing Agency: An entity engaged to perform specific tests, inspections, or both. Testing laboratory shall mean the same as testing agency.
- I. Installer/Applicator/Erector: Contractor or another entity engaged by Contractor as an employee, Subcontractor, or Sub-subcontractor, to perform a particular construction operation, including installation, erection, application, and similar operations.
 - 1. Use of trade-specific terminology in referring to a trade or entity does not require that certain construction activities be performed by accredited or unionized individuals, or that requirements specified apply exclusively to specific trade(s).
- J. Experienced: When used with an entity or individual, "experienced" means having successfully completed a minimum of five previous projects similar in nature, size, and extent to this Project; being familiar with special requirements indicated; and having complied with requirements of authorities having jurisdiction and with the qualification requirements of individual specification section governing their work.

1.3 CONFLICTING REQUIREMENTS

- A. Referenced Standards: If compliance with two or more standards is specified and the standards establish different or conflicting requirements for minimum quantities or quality levels, comply with the most stringent requirement. Refer conflicting requirements to Architect for a decision before proceeding.
- B. Minimum Quantity or Quality Levels: The quantity or quality level shown or specified shall be the minimum provided or performed. The actual installation may comply exactly with the minimum quantity or quality specified, or it may exceed the minimum within reasonable limits. To comply with these requirements, indicated numeric values are minimum or maximum, as appropriate, for the context of requirements. Refer uncertainties to Architect for a decision before proceeding.

1.4 CONTRACTOR'S QUALITY-CONTROL PLAN

- A. Quality-Control Plan, General: Submit quality-control plan prior to preconstruction conference. Submit in format acceptable to Architect. Identify personnel, procedures, controls, instructions, tests, records, and forms to be used to carry out Contractor's quality-assurance and quality-control responsibilities. Coordinate with Contractor's construction schedule.

- B. Quality-Control Personnel Qualifications: Engage qualified full-time personnel trained and experienced in managing and executing quality-assurance and quality-control procedures similar in nature and extent to those required for Project.
 - 1. Project quality-control manager may be the Project superintendent or be an individual with no other Project responsibilities, as accepted by the Architect.
- C. Submittal Procedure: Describe procedures for ensuring compliance with requirements through review and management of submittal process. Indicate qualifications of personnel responsible for submittal review.
- D. Testing and Inspection: In quality-control plan, include a comprehensive schedule of Work requiring testing or inspection, including the following:
 - 1. Contractor-performed tests and inspections including subcontractor-performed tests and inspections. Include required tests and inspections and Contractor-elected tests and inspections.
 - 2. Special inspections required by authorities having jurisdiction and indicated on the "Statement of Special Inspections."
 - 3. Owner-performed tests and inspections indicated in the Contract Documents, including tests and inspections indicated to be performed by the Commissioning Authority when Commissioning is included in the Project.
- E. Continuous Inspection of Workmanship: Describe process for continuous inspection during construction to identify and correct deficiencies in workmanship in addition to testing and inspection specified. Indicate types of corrective actions to be required to bring work into compliance with standards of workmanship established by Contract requirements and approved mockups.
- F. Monitoring and Documentation: Maintain testing and inspection reports including log of approved and rejected results, including Owner acceptance of nonconforming work. Include work Architect has indicated as nonconforming or defective. Indicate corrective actions taken to bring nonconforming work into compliance with requirements. Comply with requirements of authorities having jurisdiction.

1.5 REPORTS AND DOCUMENTS

- A. Test and Inspection Reports: Prepare and submit certified written reports specified in other Sections. Include the following:
 - 1. Date of issue.
 - 2. Project title and number.
 - 3. Name, address, and telephone number of testing agency.
 - 4. Dates and locations of samples and tests or inspections.
 - 5. Names of individuals making tests and inspections.
 - 6. Description of the Work and test and inspection method.
 - 7. Identification of product and Specification Section.
 - 8. Complete test or inspection data.
 - 9. Test and inspection results and an interpretation of test results.
 - 10. Record of temperature and weather conditions at time of sample taking and testing and inspecting.
 - 11. Comments or professional opinion on whether tested or inspected Work complies with the Contract Document requirements.
 - 12. Name and signature of laboratory inspector.
 - 13. Recommendations on retesting and reinspecting.

- B. **Manufacturer's Technical Representative's Field Reports:** Prepare written information documenting manufacturer's technical representative's tests and inspections specified in other Sections. Include the following:
1. Name, address, and telephone number of technical representative making report.
 2. Statement on condition of substrates and their acceptability for installation of product.
 3. Statement that products at Project site comply with requirements.
 4. Summary of installation procedures being followed, whether they comply with requirements and, if not, what corrective action was taken.
 5. Results of operational and other tests and a statement of whether observed performance complies with requirements.
 6. Statement whether conditions, products, and installation will affect warranty.
 7. Statement whether conditions, products, and installation exceed manufacturer's statements.
 8. Other required items indicated in individual Specification Sections.
- C. **Factory-Authorized Service Representative's Reports:** Prepare written information documenting manufacturer's factory-authorized service representative's tests and inspections specified in other Sections. Include the following:
1. Name, address, and telephone number of factory-authorized service representative making report.
 2. Statement that equipment complies with requirements.
 3. Results of operational and other tests and a statement of whether observed performance complies with requirements.
 4. Statement whether conditions, products, and installation will affect warranty.
 5. Other required items indicated in individual Specification Sections.
- D. **Permits, Licenses, and Certificates:** For Owner's records, submit copies of permits, licenses, certifications, inspection reports, releases, jurisdictional settlements, notices, receipts for fee payments, judgments, correspondence, records, and similar documents, established for compliance with standards and regulations bearing on performance of the Work.

1.6 QUALITY ASSURANCE

- A. **General:** Qualifications paragraphs in this article establish the minimum qualification levels required; individual Specification Sections specify additional requirements.
- B. **Manufacturer Qualifications:** A firm experienced in manufacturing products or systems similar to those indicated for this Project and with a record of successful in-service performance, as well as sufficient production capacity to produce required units.
- C. **Fabricator Qualifications:** A firm experienced in producing products similar to those indicated for this Project and with a record of successful in-service performance, as well as sufficient production capacity to produce required units.
- D. **Installer Qualifications:** A firm or individual experienced in installing, erecting, or assembling work similar in material, design, and extent to that indicated for this Project, whose work has resulted in construction with a record of successful in-service performance.
- E. **Professional Engineer Qualifications:** A professional engineer who is legally qualified to practice in jurisdiction where Project is located and who is experienced in providing engineering services of the kind indicated. Engineering services are defined as those performed for installations of the system, assembly, or product that is similar in material, design, and extent to those indicated for this Project.

- F. Specialists: Certain Specification Sections require that specific construction activities shall be performed by entities who are recognized experts in those operations. Specialists shall satisfy qualification requirements indicated and shall be engaged for the activities indicated.
1. Requirements of authorities having jurisdiction shall supersede requirements for specialists.
- G. Testing Agency Qualifications: An NRTL, an NVLAP, or an independent agency with the experience and capability to conduct testing and inspecting indicated, as documented according to ASTM E 329; and with additional qualifications specified in individual Sections; and, where required by authorities having jurisdiction, that is acceptable to authorities.
1. NRTL: A nationally recognized testing laboratory according to 29 CFR 1910.7.
 2. NVLAP: A testing agency accredited according to NIST's National Voluntary Laboratory Accreditation Program.
- H. Manufacturer's Technical Representative Qualifications: An authorized representative of manufacturer who is trained and approved by manufacturer to observe and inspect installation of manufacturer's products that are similar in material, design, and extent to those indicated for this Project.
- I. Factory-Authorized Service Representative Qualifications: An authorized representative of manufacturer who is trained and approved by manufacturer to inspect installation of manufacturer's products that are similar in material, design, and extent to those indicated for this Project.
- J. Preconstruction Testing: Where testing agency is indicated to perform preconstruction testing for compliance with specified requirements for performance and test methods, comply with the following:
1. Contractor responsibilities include the following:
 - a. Provide test specimens representative of proposed products and construction.
 - b. Submit specimens in a timely manner with sufficient time for testing and analyzing results to prevent delaying the Work.
 - c. Provide sizes and configurations of test assemblies, mockups, and laboratory mockups to adequately demonstrate capability of products to comply with performance requirements.
 - d. Build site-assembled test assemblies and mockups using installers who will perform same tasks for Project.
 - e. Build laboratory mockups at testing facility using personnel, products, and methods of construction indicated for the completed Work.
 - f. When testing is complete, remove test specimens, assemblies, mockups, and laboratory mockups; do not reuse products on Project.
 2. Testing Agency Responsibilities: Submit a certified written report of each test, inspection, and similar quality-assurance service to Architect, with copy to Contractor. Interpret tests and inspections and state in each report whether tested and inspected work complies with or deviates from the Contract Documents.

- K. Mock-ups: Prior to fabrication and installation, build mock-up for each form of construction and finish required to verify selections made under sample Submittals and to demonstrate aesthetic effects and qualities of materials and execution. Build mock-up to comply with the following requirements, using materials indicated for the completed Work:
 - 1. Build mock-up in the location and of the size indicated or, if not indicated, as directed by Architect. Contractor shall provide structural support framework.
 - a. Show typical components, attachments to building structure, and requirements of installation.
 - 2. Clean exposed faces of mock-up.
 - 3. Notify Architect seven days in advance of the dates and times when mock-up will be installed.
 - 4. Demonstrate the proposed range of aesthetic effects and workmanship.
 - 5. Protect accepted mock-up from the elements with weather-resistant membrane.
 - 6. Obtain Architect's acceptance of mock-ups before starting fabrication.
 - 7. Maintain mock-ups during construction in an undisturbed condition as a standard for review of the completed Work.
 - 8. Acceptance of mock-ups does not constitute acceptance of deviations from the Contract Documents contained in mock-ups unless such deviations are specifically noted by Contractor, submitted to Architect in writing, and accepted by Architect in writing.
 - 9. Demolish and remove mock-ups when directed by Architect unless accepted to become part of the completed Work.
- L. Integrated Exterior Mockups: See Division 01 Section "Visual Mock-Up Requirements".
- M. Room Mockups: See Division 01 Section "Visual Mock-Up Requirements".
- N. Laboratory Mockups: See Division 01 Section "Testing Mock-Up for Building Enclosure System".

1.7 QUALITY CONTROL

- A. Owner Responsibilities: Where quality-control services are indicated as Owner's responsibility, Owner will engage a qualified testing agency to perform these services.
 - 1. Owner will furnish Contractor with names, addresses, and telephone numbers of testing agencies engaged and a description of types of testing and inspecting they are engaged to perform.
 - 2. Costs for retesting and reinspecting construction that replaces or is necessitated by work that failed to comply with the Contract Documents will be charged to Contractor.
- B. Contractor Responsibilities: Tests and inspections not explicitly assigned to Owner are Contractor's responsibility. Perform additional quality-control activities required to verify that the Work complies with requirements, whether specified or not.
 - 1. Unless otherwise indicated, provide quality-control services specified and those required by authorities having jurisdiction. Perform quality-control services required of Contractor by authorities having jurisdiction, whether specified or not.
 - 2. Where services are indicated as Contractor's responsibility, engage a qualified testing agency to perform these quality-control services.

- a. Contractor shall not employ same entity engaged by Owner, unless agreed to in writing by Owner.
 3. Notify testing agencies at least 48 hours in advance of time when Work that requires testing or inspecting will be performed.
 4. Where quality-control services are indicated as Contractor's responsibility, submit a certified written report, in duplicate, of each quality-control service.
 5. Testing and inspecting requested by Contractor and not required by the Contract Documents are Contractor's responsibility.
 6. Submit additional copies of each written report directly to authorities having jurisdiction, when they so direct.
- C. **Manufacturer's Field Services:** Where indicated, engage a factory-authorized service representative to inspect field-assembled components and equipment installation, including service connections. Report results in writing as specified in Division 01 Section "Submittal Procedures."
- D. **Manufacturer's Technical Services:** Where indicated, engage a manufacturer's technical representative to observe and inspect the Work. Manufacturer's technical representative's services include participation in preinstallation conferences, examination of substrates and conditions, verification of materials, observation of Installer activities, inspection of completed portions of the Work, and submittal of written reports.
- E. **Retesting/Reinspecting:** Regardless of whether original tests or inspections were Contractor's responsibility, provide quality-control services, including retesting and reinspecting, for construction that replaced Work that failed to comply with the Contract Documents.
- F. **Testing Agency Responsibilities:** Cooperate with Architect and Contractor in performance of duties. Provide qualified personnel to perform required tests and inspections.
1. Notify Architect and Contractor promptly of irregularities or deficiencies observed in the Work during performance of its services.
 2. Conduct and interpret tests and inspections and state in each report whether tested and inspected work complies with or deviates from requirements.
 3. Submit a certified written report, in duplicate, of each test, inspection, and similar quality-control service through Contractor.
 4. Do not release, revoke, alter, or increase the Contract Document requirements or approve or accept any portion of the Work.
 5. Do not perform any duties of Contractor.
- G. **Associated Services:** Cooperate with agencies performing required tests, inspections, and similar quality-control services, and provide reasonable auxiliary services as requested. Notify agency sufficiently in advance of operations to permit assignment of personnel. Provide the following:
1. Access to the Work.
 2. Incidental labor and facilities necessary to facilitate tests and inspections.
 3. Adequate quantities of representative samples of materials that require testing and inspecting. Assist agency in obtaining samples.
 4. Facilities for storage and field curing of test samples.
 5. Delivery of samples to testing agencies.
 6. Preliminary design mix proposed for use for material mixes that require control by testing agency.
 7. Security and protection for samples and for testing and inspecting equipment at Project site.

- H. Coordination: Coordinate sequence of activities to accommodate required quality-assurance and -control services with a minimum of delay and to avoid necessity of removing and replacing construction to accommodate testing and inspecting.
 - 1. Schedule times for tests, inspections, obtaining samples, and similar activities.
- I. Schedule of Tests and Inspections: Prepare a schedule of tests, inspections, and similar quality-control services required by the Contract Documents. Coordinate and submit concurrently with Contractor's construction schedule.
 - 1. Distribution: Distribute schedule to Owner, Architect, testing agencies, and each party involved in performance of portions of the Work where tests and inspections are required.
 - a. Prepare in tabular form and include the following:
 - 1) Specification Section number and title.
 - 2) Entity responsible for performing tests and inspections.
 - 3) Description of test and inspection.
 - 4) Identification of applicable standards.
 - 5) Identification of test and inspection methods.
 - 6) Number of tests and inspections required.
 - 7) Time schedule or time span for tests and inspections.
 - 8) Requirements for obtaining samples.
 - 9) Unique characteristics of each quality-control service.

1.8 SPECIAL TESTS AND INSPECTIONS

- A. Special Tests and Inspections: Owner may engage a qualified to conduct special tests and inspections required by authorities having jurisdiction as the responsibility of Owner, and as follows:
 - 1. Notifying Architect and Contractor promptly of irregularities and deficiencies observed in the Work during performance of its services.
 - 2. Submitting a certified written report of each test, inspection, and similar quality-control service to Architect with copy to Contractor and to authorities having jurisdiction.
 - 3. Submitting a final report of special tests and inspections at Substantial Completion, which includes a list of unresolved deficiencies.
 - 4. Interpreting tests and inspections and stating in each report whether tested and inspected work complies with or deviates from the Contract Documents.
 - 5. Retesting and reinspecting corrected work.

PART 2 - PRODUCTS (Not Used)

PART 3 - EXECUTION

3.1 TEST AND INSPECTION LOG

- A. Test and Inspection Log: Prepare a record of tests and inspections. Include the following:
 - 1. Date test or inspection was conducted.
 - 2. Description of the Work tested or inspected.
 - 3. Date test or inspection results were transmitted to Architect.

4. Identification of testing agency or special inspector conducting test or inspection.
- B. Maintain log at Project site. Post changes and revisions as they occur. Provide access to test and inspection log for Architect's reference during normal working hours.

3.2 REPAIR AND PROTECTION

- A. General: On completion of testing, inspecting, sample taking, and similar services, repair damaged construction and restore substrates and finishes.
 1. Provide materials and comply with installation requirements specified in other Specification Sections or matching existing substrates and finishes. Restore patched areas and extend restoration into adjoining areas in a manner that eliminates evidence of patching. Comply with the Contract Document requirements for cutting and patching in Division 01 Section "Execution."
- B. Protect construction exposed by or for quality-control service activities.
- C. Repair and protection are Contractor's responsibility, regardless of the assignment of responsibility for quality-control services.

END OF SECTION

SECTION 01 4323
SPECIAL INSPECTIONS

PART 1 - GENERAL

1.1 SUMMARY

- A. Section Includes: Special Inspections required by Authorities Having Jurisdiction including supplementary work necessary to complete inspections.

1.2 RESPONSIBILITY

- A. Special Inspections do not relieve the Contractor of the responsibility to provide construction in accordance with the Contract Documents.

1.3 INSPECTOR'S RESPONSIBILITY

- A. Special Inspector shall be acceptable to the Authorities Having Jurisdiction for this project.
- B. Special Inspector shall be completely familiar with the Contract Documents relating to inspection responsibility.

1.4 COOPERATION

- A. Contractor shall cooperate with and assist Special Inspector in performing Special Inspections. Special Inspector shall have access to the project without restriction.
 - 1. Contractor shall advise Special Inspector in advance of construction schedules and planned operations to assure timely and appropriate observation and inspection of items requiring Special Inspections.
 - 2. Contractor shall make available approved Shop Drawings to the Special Inspector at jobsite.

1.5 OWNER'S RESPONSIBILITY

- A. Cost of employing the Special Inspector shall be the responsibility of the Owner.
- B. Final interpretation of the Contract Documents shall rest with the Architect and Engineers of Record.

1.6 GENERAL DUTIES OF SPECIAL INSPECTOR

- A. Special Inspector shall cooperate with the Contractor on timely observations and inspections of Work. Special Inspector shall not supervise or direct the Work.
- B. Special Inspector shall immediately alert the Contractor of discrepancies and deviations from the Contract Documents and approved Shop Drawings.
- C. Special Inspection Reports shall be submitted within 24 hours of the time and date of the observation and inspection.

1. Special Inspector shall submit his reports to the Authorities Having Jurisdiction for the project. In addition, Special Inspector shall submit copies of reports to the Owner and to the Contractor.
2. Upon completion of the building and prior to the issuance of a Certificate of Occupancy, a signed and sealed statement by the Special Inspector shall be submitted to the Authorities Having Jurisdiction, stating the portion of the project subject to Special Inspections has been constructed in accordance with the Contract Documents. This statement shall be provided in accordance with the governing building codes and ordinances.

PART 2 - PRODUCTS - Not Used

PART 3 - EXECUTION - Not Used

END OF SECTION

SECTION 01 4339

VISUAL MOCK-UP REQUIREMENTS

PART 1 - GENERAL

1.1 SUMMARY

- A. This Section specifies full size mock-up requirements of building components to verify material selections, demonstrate aesthetic effects and review construction and workmanship.
- B. Approved mock-up shall establish the standard by which the Work will be judged.
- C. Acceptance of mock-up does not constitute approval of deviations from the Contract Documents in mock-up, unless such deviations are specifically approved by Architect in writing.
- D. Retain mock-ups during construction and maintain in an undisturbed condition. Do not demolish alter or remove mock-up until approved by Architect.
- E. Mock-up shall be used to demonstrate quality of materials, finish and workmanship as well as to show compliance with visual criteria.
- F. Submit shop drawings prior to fabrication of visual mock-up, showing plan, elevations and details of mock-up.

PART 2 - PRODUCTS

2.1 MATERIALS

- A. As specified in individual Specification Sections.

PART 3 - EXECUTION

3.1 INSTALLATION

- A. Use the same workers to do work in conjunction with construction of the mock-up as covered by the work of their respective Contracts.
- B. Provide mock-up complete with corners, splice joints, control joints, sealants, and complete finish with details identical to those proposed for use in the building and as indicated on the drawings.
- C. Do not use special measures or techniques, which are not representative of those to be used in the building. Finish the various components to show the maximum variation that will exist in the actual building construction between adjacent components.
- D. Notify Architect when construction of mock-up begins and when major components are to be installed.
- E. Complete the mock-up and obtain Architect's approval of each component of the mock-up prior to fabrication or purchase of products for the Project.

3.2 BUILDING EXTERIOR WALL

- A. Fabricate and erect a visual mock-up of the typical exterior wall condition with one outside corner returning to the window line.
 - 1. Mock-up size: As indicated on drawing elevations.
- B. Provide structural steel frame work for support of visual mock up. Frame work shall be designed by a registered professional or structural engineer licensed in the State where the Project is located.
- C. Coordinate with Architect and Owner for location of mock-up on project site. When directed, demolish mock-ups and remove from Project site.
- D. Construct mock-up in such a manner that each type of exterior finishes will be demonstrated in a layered fashion from one side to the other. The purpose in the layers is to facilitate the review of the multiple activities that are required to accomplish the final finish.
- E. Construct mock-up in phases so that proposed construction methodologies can be observed. Mock-up shall be completely constructed, just as if it were the finished exterior wall. Wall insulation and interior wall finishes are not required.
- F. In addition to specifics in the respective Specification Sections, the mock-up will be reviewed by the Owner and the Architect for the following evaluation purposes:
 - 1. Aesthetic: To review and verify selections made under submittals, as well as to show compliance with visual criteria. Acceptance criteria is for general and specific aesthetic qualities of construction, and includes, but is not limited to, the following:
 - a. Color, texture, and blending of masonry units.
 - b. Consistency of masonry mortar color.
 - c. Tooling of masonry mortar joints.
 - d. Color, texture and finishing of exterior insulation and finish system (EIFS).
 - e. Tooling of sealants.
 - f. Color consistency of aluminum window framing.
 - g. Color and clarity of glass.
 - h. Flatness and alignment of aluminum composite material panels.
 - 2. Installation Execution: To review and verify quality of workmanship, and compliance with Drawings, Specification and submittals. Acceptance criteria is for general and specific erection, installation and application qualities of construction, and includes, but is not limited to, the following:
 - a. Masonry bonding and mortar character.
 - b. Brick cavity cleanliness.
 - c. Brick cavity drainage function.
 - d. Masonry anchor and ties integrity.
 - e. Masonry flashing fabrication and embedment within the masonry.
 - f. Assembly of various components of the exterior insulation and finish system (EIFS), including color, texture and finishing.
 - g. Cold-formed metal framing fabrication and assemblage.
 - h. Glazed aluminum wall system fabrication, anchorage/attachment, and installation quality.
 - i. Glazing integrity.
 - j. Sealant profile consistency and bonding integrity.

- k. Attachment of aluminum composite material panels.
- l. Dimensional tolerances.

3.3 TYPICAL ROOM MOCK-UP

- A. Arrange for the construction of a typical room mock-up located in the building, per mutual agreement between Owner and Contractor.
 - 1. **Typical Restrooms:** At the first installation of a typical office restroom, complete finishes, fixtures and accessories in one restroom module as a visual mock-up.
 - 2. **Typical Patient Room:** At the first installation of a typical hospital patient room, complete finishes, fixtures and accessories in one patient room module as a visual mock-up.
- B. The materials trades Subcontractors shall do all work in conjunction with construction of the mock-up as covered by the work of their respective Contracts.
- C. Typical Room mock-up shall be complete with finishes, fixtures and details identical to those proposed for use in the Project. Do not use special measures or techniques, which are not representative of those to be used in the finish work. Finish the various components to show the quality of material and construction that will exist in the actual construction.
- D. Typical room mock-up shall be of adequate size to contain all samples and demonstrate quality of materials, finish and workmanship as well as to show compliance with visual criteria. Submit shop drawings to show layout of room prior to construction. Mock-up room shall be constructed as soon as sufficient structure is available to allow installation.
- E. Materials or workmanship not approved shall be removed and replaced with acceptable products or workmanship. Fabrication, purchase or installation of materials for the building shall not begin until approved in the mock-up room. Owner's approval of samples will be required on all visual items prior to any contractor's buy-out.
- F. Materials installed in the room mock-up shall include an example of all furnished assemblies, hardware, equipment or accessories required for the Project and shall include, but may not be necessarily limited to the following:
 - 1. Lavatory countertop, complete with sealant, sink, accessories, trim, etc.
 - 2. Flush Wood veneer door
 - 3. Hollow Metal Door Frame
 - 4. Door Hardware
 - 5. Ceramic floor and wall material
 - 6. Vinyl Wall Covering(s)
 - 7. Toilet Compartment complete with partition, side wall, pilaster, door and hardware.
 - 8. Toilet Room Accessories
 - 9. Water closet and trim
 - 10. Urinal and trim
 - 11. Lighting
 - 12. Electrical Devices and Accessories
 - 13. Access Doors or Panels

END OF SECTION

SECTION 01 4516

FIELD TEST FOR WATER LEAKAGE

PART 1 - GENERAL

1.1 SUMMARY

- A. Section Includes: Field test to determine resistance to water leakage in wall systems in accordance with performance requirements indicated including joints in adjacent construction which are designed to remain permanently closed and watertight.
- B. Contractor will pay costs of testing laboratory for this test.

1.2 DEFINITIONS

- A. Water Leakage: Any uncontrolled water that appears on any normally exposed interior surfaces, that is not contained or drained back to exterior, or that can cause damage to adjacent materials or finishes. Water contained within drained flashings, gutters, and sills is not considered water leakage.

1.3 SUBMITTALS

- A. Field Quality Control Reports: Written report of testing and inspection required by "Field Quality Control".
- B. Qualification Data: For testing agency.
 - 1. For firms and persons specified in "Quality Assurance" to demonstrate their capabilities and experience. Include lists of completed projects with project names and addresses, names and addresses of Architects and Owners, and other information specified.

1.4 QUALITY ASSURANCE

- A. Testing Agency Qualifications: Qualified according to ASTM E 699 for testing indicated.
- B. Field Water Spray Testing: Selective field testing will be conducted in accordance with AAMA Standard 501.2: "Quality Assurance and Diagnostic Water Leakage Field Check of Installed Storefronts, Curtainwalls, and Sloped Glazing Systems".
 - 1. Water Spray Test without Air Pressure Difference: Designated areas shall be tested according to AAMA 501.2 and shall not evidence water penetration.
 - 2. Test shall be performed prior to installation of interior finishes.

- C. Field Chamber Testing: Selective field testing will be conducted in accordance with ASTM E 1105 "Standard Test Method for Field Determination of Water Penetration of Installed Exterior Windows, Curtain Walls, and Doors by Uniform or Cyclic Static Air Pressure Difference".
 - 1. Water Penetration Test with Static Air Pressure Difference: Designated areas shall be tested according to ASTM E 1105 at a minimum static air pressure differential specified for laboratory testing in "Performance Requirements" and shall not evidence water penetration. Field test to full design static pressure differential with no reduction for field installation.
 - 2. Test chamber shall include adjacent joints and interior/exterior construction.

1.5 CONTRACTOR RESPONSIBILITIES

- A. Cooperate with Testing Agency. Provide labor, materials and equipment, including but not limited to scaffolding, equipment and water supply for use in testing by testing agency personnel.
 - 1. Contractor shall be responsible for coordinating and scheduling testing.
 - 2. Provide powered scaffold, hose, water supply, communication system and manpower to perform tests Schedule any work necessary, such as out of sequence sealant work, so that wall can be tested.
- B. Satisfactory results of this test do not relieve Contractor from conforming to requirements of Contract Documents, shop drawings, and project specifications, and installation work on remainder of building shall match area checked, unless otherwise instructed in writing.
- C. Pending actual performance, prevalence or absence of water leakage in initial water penetration test, and upon measures adopted by the Contractor to eliminate source of leakage, Architect/Consultant will determine necessity for, and scope of additional tests and test methods. Remedial measures shall maintain standards of quality and durability and are subject to approval.
- D. Remedial measures found necessary and effective in eliminating leakage in area checked shall be used in fabricating and installing remainder of wall on building.
- E. Notify Architect/Consultant and testing agency when wall construction is ready for testing.

PART 2 - PRODUCTS

2.1 PERFORMANCE REQUIREMENTS

- A. Refer to Technical Sections for performance requirements of each system or product to be tested.

PART 3 - EXECUTION

3.1 EXAMINATION

- A. Acceptance of Surfaces and Conditions: Examine substrates to receive products and systems and associated work for compliance with requirements and other conditions affecting performance. Proceed only when unsatisfactory conditions have been corrected in a manner complying with Contract Documents. Starting work within a particular area will be construed as acceptance of surface conditions.

3.2 INSTALLATION, GENERAL

- A. Installation Quality Standards: In addition to standards listed elsewhere, perform Work according to following, unless otherwise specified:
 - 1. Respective manufacturer's written installation instructions.
 - 2. Accepted submittals.
 - 3. Contract Documents.

3.3 PREPARATION

- A. General: Comply with manufacturer's instructions, recommendations, and specifications for cleaning and surface preparation. Surfaces shall have no defects, contaminants, or errors which would result in poor or potentially defective installation or would cause latent defects in Work.
- B. Exterior wall construction shall be completed, and shall be fully glazed to provide complete wall installation. Work shall be done in strict accord with approved shop drawings and job specifications.

3.4 FIELD QUALITY CONTROL TESTING

- A. Testing Services, General: Testing and inspecting of representative areas of exterior walls shall take place as installation proceeds to determine compliance of installed assemblies with specified performance requirements.
- B. Testing Agency Field Service: Engage a qualified independent testing agency employed by Contractor and approved by Architect to perform field quality control. Materials and installation failing to meet specified requirements shall be replaced at Contractor's expense. Retesting of materials and installations failing to meet specified requirements shall be done at Contractor's expense.
- C. Architect shall designate test procedure and areas of completed walls to be checked.
 - 1. Field Testing shall be performed at intervals and locations including but not limited to the following:
 - a. Field Water Spray Testing per AAMA 501.2: Perform tests at intervals and locations in each test area as directed by Architect; however, not less than 1-percent of total units or 5 units minimum, whichever is greater, shall be tested for each type of glazing system and configuration.
 - b. Field Chamber Testing per ASTM E 1105: Perform tests at intervals and locations in each test area as directed by Architect; however, not less than 3 static water test minimum, shall be tested for each type of glazing system and configuration.

2. Systems shall be tested in accordance with definitions and at performance requirements indicated in Technical Sections.
 3. Test wall at 15-percent, 50-percent, and 75-percent completion unless directed otherwise.
- D. All joints or other conditions within designated areas where leakage may occur will be tested.
- E. Indoor side of wall in this area shall be unfinished and left open and unobstructed, permitting full length of joints to be examined from indoor side.
- F. If operable joints such as those around doors and operable parts of windows occur within wall area involved, appropriate modifications both of procedure and performance requirements will be made in respect to such joints.

3.5 TEST REPORTS

- A. Testing laboratory shall be responsible for conducting and reporting tests, shall state in report whether or not test specimen conforms to requirements of Contract Documents approved drawings, and shall specifically note any deviations.
- B. Testing laboratory shall submit its report directly to Contractor and Architect. Necessary corrections shall be performed in presence of Architect. Tests shall be witnessed by Architect/Consultant. Approval of test assembly and test results rests with the Architect.
- C. Test Reports: Shall be prepared according to testing standard indicated.

3.6 REMEDIAL WORK BY CONTRACTOR

- A. In event of failure to initially meet test requirements called for hereinabove, Contractor and respective subcontractors shall, as required, redesign, rework, and/or re-fabricate, reship and re-erect assemblies until said requirements are met, at no additional cost to Owner.
- B. Wherever leakage has occurred, joints shall be made watertight in manner acceptable to Architect.
- C. Remedial work involving use of curing-type compounds shall be allowed to set for one week before it is re-checked for leakage.
- D. After necessary remedial work has been completed, and required curing time, if any, has elapsed, repaired joints shall be retested.
- E. Should leakage still be found, further remedial measures shall be taken and checking shall be repeated until joints in designated area are found to be satisfactory.
- F. Re-checking, when required, shall be performed by testing laboratory originally employed by Contractor.

END OF SECTION

SECTION 01 5000

TEMPORARY FACILITIES AND CONTROLS

PART 1 - GENERAL

1.1 SUMMARY

- A. Section includes requirements for temporary utilities, support facilities, and security and protection facilities.

1.2 USE CHARGES

- A. General: Installation and removal of and use charges for temporary facilities shall be included in the Contract Sum. Allow other entities to use temporary services and facilities without cost, including, but not limited to, Owner's construction forces, Architect, occupants of Project, testing agencies, and authorities having jurisdiction.
 - 1. Cost or use charges for temporary facilities are not chargeable to Owner or Architect.
- B. Sewer Service: Pay sewer-service use charges for sewer usage, indicated by utility company meter readings, by all entities for construction operations.
- C. Water Service: Pay water-service use charges for water used, indicated by utility company meter readings, by all entities for construction operations.
- D. Electric Power Service: Pay electric-power-service use charges for electricity used, indicated by utility company meter readings, by all entities for construction operations.

1.3 INFORMATIONAL SUBMITTALS

- A. Site Plan: Show temporary facilities, utility hookups, staging areas, and parking areas for construction personnel.
- B. Moisture-Protection Plan: Describe procedures and controls for protecting materials and construction from water absorption and damage.
 - 1. Describe delivery, handling, and storage provisions for materials subject to water absorption or water damage.
 - 2. Indicate procedures for discarding water-damaged materials, protocols for mitigating water intrusion into completed Work, and replacing water-damaged Work.
 - 3. Indicate sequencing of work that requires water, such as sprayed fire-resistive materials, plastering, and terrazzo grinding, and describe plans for dealing with water from these operations. Show procedures for verifying that wet construction has dried sufficiently to permit installation of finish materials.
- C. Dust- and HVAC-Control Plan at Renovation Work: Submit coordination drawing and narrative that indicates the dust- and HVAC-control measures proposed for use, proposed locations, and proposed time frame for their operation. Identify further options if proposed measures are later determined to be inadequate. Include the following:
 - 1. Locations of dust-control partitions at each phase of work.
 - 2. HVAC system isolation schematic drawing.

3. Location of proposed air-filtration system discharge.
 4. Waste handling procedures.
 5. Other dust-control measures.
- D. Temporary Utility Reports: Make available on request, reports of tests, inspections, meter readings, and similar procedures performed on temporary utilities.
- E. Implementation and Termination Schedule: Make available on request a schedule indicating implementation and termination of each temporary utility.

1.4 QUALITY ASSURANCE

- A. Standards: Comply with ANSI A10.6 "Requirements for Demolition Operations", NECA's "Temporary Electrical Facilities," and NFPA 241 "Standard for Safeguarding Construction, Alteration, and Demolition Operations".
1. Trade Jurisdictions: Assigned responsibilities for installation and operation of temporary utilities are not intended to interfere with trade regulations and union jurisdictions.
- B. Electric Service: Comply with NECA, NEMA, and UL standards and regulations for temporary electric service. Install service to comply with NFPA 70.
- C. Tests and Inspections: Arrange for authorities having jurisdiction to test and inspect each temporary utility before use. Obtain required certifications and permits.
- D. Accessible Temporary Egress at Renovation Work: Comply with applicable provisions in the U.S. Architectural & Transportation Barriers Compliance Board's ADA-ABA Accessibility Guidelines and ICC/ANSI A117.1.
- E. Regulations: Comply with industry standards and applicable laws and regulations of authorities having jurisdiction, including but not limited to, the following:
1. Building Code requirements.
 2. Health and safety regulations.
 3. Utility company regulations.
 4. Police, Fire Department and Rescue Squad rules.
 5. Environmental protection regulations.
 6. City ordinances and regulations.

1.5 PROJECT CONDITIONS

- A. Temporary Use of Permanent Facilities: Engage Installer of each permanent service to assume responsibility for operation, maintenance, and protection of each permanent service during its use as a construction facility before Owner's acceptance, regardless of previously assigned responsibilities.

PART 2 - PRODUCTS

2.1 MATERIALS

- A. General: Provide new materials. Undamaged, previously used materials in serviceable condition may be used if approved by Architect. Provide materials suitable for use intended.

- B. Materials and equipment may be new or used, but must be adequate in capacity for the required usage, must not create unsafe conditions, and must not violate requirements of applicable codes and standards.
- C. Chain-Link Fencing: Minimum 2 inch (50 mm), 0.148 inch (3.8 mm) thick, galvanized-steel, chain-link fabric fencing; minimum 6 feet (1.8 m) high with galvanized-steel pipe posts; minimum 2-3/8 inch (60 mm) OD line posts and 2-7/8 inch (73 mm) OD corner and pull posts, with 1-5/8 inch (42 mm) OD top rails.
- D. Portable Chain-Link Fencing: Minimum 2 inch (50 mm), 0.148 inch (3.8 mm) thick, galvanized-steel, chain-link fabric fencing; minimum 6 feet (1.8 m) high with galvanized-steel pipe posts; minimum 2-3/8 inch (60 mm) OD line posts and 2-7/8 inch (73 mm) OD corner and pull posts, with 1-5/8 inch (40 mm) OD top and bottom rails. Provide concrete or galvanized-steel bases for supporting posts.
- E. Wood Enclosure Fence: Plywood, 8 feet (2.4 m) high, framed with four 2 by 4 inch (50 by 100 mm) rails, with preservative-treated wood posts spaced not more than 8 feet (2.4 m) apart.
- F. Polyethylene Sheet: Reinforced, fire-resistive sheet, 10 mil (0.25 mm) minimum thickness, with flame-spread rating of 15 or less per ASTM E 84 and passing NFPA 701 Test Method 2.
- G. Polyethylene Sheet: Reinforced, fire-resistive sheet, 6 mil (0.14 mm) minimum thickness, with Class A flame-spread rating per ASTM E 84 and passing NFPA 701 Test Method 2.
 - 1. Basis of Design (Product Standard): Abatement Technologies, Inc.; SAFE-FLEX ICRA Awareness Barrier.
- H. Dust Containment Barrier for Doors: reinforced, fire-resistive polyethylene sheet, 10 mil (0.25 mm) minimum thickness with Class B flame-spread rating per ASTM E 84 and designed to be used for securing temporary construction doors so as to minimize and mitigate particle control during construction.
 - 1. Basis of Design (Product Standard): Abatement Technologies, Inc.; Aire Guardian Door Guard Reusable Barrier.
- I. Dust-Control Adhesive-Surface Walk-off Mats: Provide mats minimum 36 by 60 inches (900 by 1500 mm).
- J. Insulation: Unfaced mineral-fiber blanket, manufactured from glass, slag wool, or rock wool; with maximum flame-spread and smoke-developed indexes of 25 and 50, respectively.

2.2 TEMPORARY FACILITIES

- A. Field Offices, General: Prefabricated or mobile units with serviceable finishes, temperature controls, and foundations adequate for normal loading.
- B. Common-Use Field Office: Of sufficient size to accommodate needs of Owner, Architect, and construction personnel office activities and to accommodate Project meetings specified in other Division 01 Sections. Keep office clean and orderly. Furnish and equip offices as follows:
 - 1. Furniture required for Project-site.
 - 2. Conference room of sufficient size to accommodate. Provide electrical power service and 120-V ac duplex receptacles. Furnish room with conference table, chairs, and tack and marker boards.
 - 3. Drinking water and private toilet.

4. Heating and cooling equipment necessary to maintain a uniform indoor.
 5. Lighting fixtures capable of maintaining average illumination at desk height.
- C. Storage and Fabrication Sheds: Provide sheds sized, furnished, and equipped to accommodate materials and equipment for construction operations.
1. Store combustible materials apart from building.

2.3 EQUIPMENT

- A. Fire Extinguishers: Portable, UL rated; with class and extinguishing agent as required by locations and classes of fire exposures.
1. Comply with NFPA 10 and NFPA 241 for classification, extinguishing agent, and size required by location and class of fire exposure.
- B. Self-Contained Toilet Units: Single-occupant units of chemical, aerated recirculation, or combustion type; vented; fully enclosed with a glass-fiber-reinforced polyester shell or similar nonabsorbent material.
- C. Drinking-Water Fixtures: Containerized, tap-dispenser, bottled-water drinking-water units, including paper cup supply.
- D. HVAC Equipment: Unless Owner authorizes use of permanent HVAC system, provide vented, self-contained, liquid-propane-gas or fuel-oil heaters with individual space thermostatic control.
1. Use of gasoline-burning space heaters, open-flame heaters, or salamander-type heating units is prohibited.
 2. Heating Units: Listed and labeled for type of fuel being consumed, by a qualified testing agency acceptable to authorities having jurisdiction, and marked for intended location and application.
- E. Air-Filtration Units for Renovation Work: Primary and secondary HEPA-filter-equipped portable units with four-stage filtration. Provide single switch for emergency shutoff. Configure to run continuously.

PART 3 - EXECUTION

3.1 INSTALLATION, GENERAL

- A. Locate facilities where they will serve Project adequately and result in minimum interference with performance of the Work. Relocate and modify facilities as required by progress of the Work.
- B. Provide each facility ready for use when needed to avoid delay. Do not remove until facilities are no longer needed or are replaced by authorized use of completed permanent facilities.

3.2 TEMPORARY UTILITY INSTALLATION

- A. Locate temporary utilities where they will serve Project adequately and result in minimum interference with performance of the Work. Relocate and modify utilities as required.

- B. Provide each utility ready for use when needed to avoid delay. Maintain and modify as required. Do not remove until utilities are no longer needed or are replaced by authorized use of completed permanent utilities.
- C. Engage appropriate local utility company to install temporary service or connect to existing service. Where utility company provides only part of the service, provide the remainder with matching, compatible materials and equipment. Comply with utility company recommendations.
 - 1. Arrange with utility company, Owner, and existing users for time when service can be interrupted, if necessary, to make connections for temporary services.
 - 2. Provide adequate capacity at each stage of construction. Before temporary utility is available, provide trucked-in services.
 - 3. Obtain easements to bring temporary utilities to Project site where Owner's easements cannot be used for that purpose.
- D. Storm Sewers and Drainage: If sewers are available, provide temporary connections to remove effluent that can be discharged lawfully. If sewers are not available or cannot be used, provide drainage ditches, dry wells, stabilization ponds, and similar facilities. If neither sewers nor drainage facilities can be lawfully used for discharge of effluent, provide containers to remove and dispose of effluent off-site in a lawful manner.
 - 1. Filter out excessive soil, construction debris, chemicals, oils, and similar contaminants that might clog sewers or pollute waterways before discharge.
 - 2. Connect temporary sewers to municipal system as directed by sewer department officials.
 - 3. Maintain temporary sewers and drainage facilities in a clean, sanitary condition. After heavy use, restore normal conditions promptly.
 - 4. Provide temporary filter beds, settlement tanks, separators, and similar devices to purify effluent to levels acceptable to authorities having jurisdiction.
- E. Water Service: Install water service and distribution piping in sizes and pressures adequate for construction. Sterilize temporary water piping before use in accordance with requirements of authorities having jurisdiction.
- F. Sanitary Facilities: Provide temporary toilets, wash facilities, and drinking water for use of construction personnel. Comply with requirements of authorities having jurisdiction for type, number, location, operation, and maintenance of fixtures and facilities.
 - 1. Existing Toilets in Occupied Facilities: Use of Owner's existing toilet facilities will not be permitted or allowed.
- G. Heating and Cooling: Provide temporary heating and cooling required by construction activities for curing or drying of completed installations or for protecting installed construction from adverse effects of low temperatures or high humidity. Select equipment that will not have a harmful effect on completed installations or elements being installed.
- H. Isolation of Work Areas in Occupied Facilities: Prevent dust, fumes, and odors from entering occupied areas.
 - 1. Prior to commencing work, isolate the HVAC system in area where work is to be performed according to coordination drawings.
 - a. Disconnect supply and return ductwork in work area from HVAC systems servicing occupied areas.

- b. Maintain negative air pressure within work area using HEPA-equipped air-filtration units, starting with commencement of temporary partition construction, and continuing until removal of temporary partitions is complete.
 - 2. Maintain dust partitions during the Work. Use vacuum collection attachments on dust-producing equipment. Isolate limited work within occupied areas using portable dust-containment devices.
 - 3. Perform daily construction cleanup and final cleanup using approved, HEPA-filter-equipped vacuum equipment.
- I. Ventilation and Humidity Control: Provide temporary ventilation required by construction activities for curing or drying of completed installations or for protecting installed construction from adverse effects of high humidity. Select equipment that will not have a harmful effect on completed installations or elements being installed. Coordinate ventilation requirements to produce ambient condition required and minimize energy consumption.
 - 1. Provide dehumidification systems when required to reduce substrate moisture levels to level required to allow installation or application of finishes.
- J. Electric Power Service: Provide electric power service and distribution system of sufficient size, capacity, and power characteristics required for construction operations.
- K. Lighting: Provide temporary lighting with local switching that provides adequate illumination for construction operations, observations, inspections, and traffic conditions.
- L. Telephone Service: Provide temporary telephone service in common-use facilities for use by all construction personnel. Provide telephone line(s) for each field office.
- M. Electronic Communication Service: Provide internet access of not less than 15-Mbps download and 5-Mbps upload speed for use by Architect and Owner to access Project electronic documents and maintain electronic communications

3.3 SUPPORT FACILITIES INSTALLATION

- A. General: Comply with the following:
 - 1. Locate field offices, storage sheds, sanitary facilities, and other temporary construction and support facilities for easy access.
 - 2. Maintain support facilities until Architect schedules Substantial Completion inspection. Remove before Substantial Completion. Personnel remaining after Substantial Completion will be permitted to use permanent facilities, under conditions acceptable to Owner.
- B. Temporary Roads and Paved Areas: Construct and maintain temporary roads and paved areas adequate to support loads and to withstand exposure to traffic during construction period. Extend temporary roads and paved areas, within construction limits indicated, as necessary for construction operations.
 - 1. Provide dust-control that is nonpolluting and nontracking. Reapply as required to minimize dust.
- C. Traffic Controls: Provide temporary traffic controls at junction of temporary roads with public roads. Include warning signs for public traffic and "STOP" signs for entrance onto public roads. Comply with requirements of authorities having jurisdiction.

1. Protect existing site improvements to remain including curbs, pavement, and utilities.
 2. Maintain access for fire-fighting equipment and access to fire hydrants.
- D. Parking: Coordinated parking with Owner's requirements.
- E. Dewatering Facilities and Drains: Comply with requirements of authorities having jurisdiction. Maintain Project site, excavations, and construction free of water.
1. Dispose of rainwater in a lawful manner that will not result in flooding Project or adjoining properties or endanger permanent Work or temporary facilities.
 2. Before connection and operation of permanent drainage piping system, provide temporary drainage where roofing or similar construction is completed.
 3. Remove snow and ice as required to minimize accumulations.
- F. Project Signs: Coordinated signs with Owner's requirements and requirements of authorities having jurisdiction.
- G. Waste Disposal Facilities: Comply with requirements specified in Division 01 Section "Construction Waste Management and Disposal."
- H. Comply with progress cleaning requirements in Division 01 Section "Execution."
- I. Existing Elevator Use in Occupied Facilities: Use of Owner's existing elevators will be permitted, provided elevators are cleaned and maintained in a condition acceptable to Owner. At Substantial Completion, restore elevators to condition existing before initial use, including replacing worn cables, guide shoes, and similar items of limited life.
1. Do not load elevators beyond their rated weight capacity.
 2. Provide protective coverings, barriers, devices, signs, or other procedures to protect elevator car and entrance doors and frame. If, despite such protection, elevators become damaged, engage elevator Installer to restore damaged work so no evidence remains of correction work. Return items that cannot be refinished in field to the shop, make required repairs and refinish entire unit, or provide new units as required.
- J. Existing Stair Usage in Occupied Facilities: Use of Owner's existing stairs will be permitted, provided stairs are cleaned and maintained in a condition acceptable to Owner. At Substantial Completion, restore stairs to condition existing before initial use.
1. Provide protective coverings, barriers, devices, signs, or other procedures to protect stairs and to maintain means of egress. If stairs become damaged, restore damaged areas so no evidence remains of correction work.
- K. Temporary Use of Permanent Stairs: Use of new stairs for construction traffic will be permitted, provided stairs are protected and finishes restored to new condition at time of Substantial Completion.

3.4 SECURITY AND PROTECTION FACILITIES INSTALLATION

- A. Protection of Existing Facilities: Protect existing vegetation, equipment, structures, utilities, and other improvements at Project site and on adjacent properties, except those indicated to be removed or altered. Repair damage to existing facilities.
- B. Environmental Protection: Provide protection, operate temporary facilities, and conduct construction as required to comply with environmental regulations and that minimize possible air, waterway, and subsoil contamination or pollution or other undesirable effects.

1. Comply with work restrictions specified in Division 01 Section "Summary."
- C. Temporary Erosion and Sedimentation Control: Provide measures to prevent soil erosion and discharge of soil-bearing water runoff and airborne dust to undisturbed areas and to adjacent properties and walkways, according to requirements of 2003 EPA Construction General Permit or authorities having jurisdiction, whichever is more stringent.
- D. Stormwater Control: Comply with requirements of authorities having jurisdiction. Provide barriers in and around excavations and subgrade construction to prevent flooding by runoff of stormwater from heavy rains.
- E. Tree and Plant Protection: Install temporary fencing located as indicated or outside the drip line of trees to protect vegetation from damage from construction operations. Protect tree root systems from damage, flooding, and erosion.
- F. Pest Control: Engage pest-control service to recommend practices to minimize attraction and harboring of rodents, roaches, and other pests and to perform extermination and control procedures at regular intervals so Project will be free of pests and their residues at Substantial Completion. Perform control operations lawfully, using environmentally safe materials.
- G. Site Enclosure Fence: Before construction operations begin, furnish and install site enclosure fence in a manner that will prevent people and animals from easily entering site except by entrance gates.
 1. Extent of Fence: As required to enclose entire Project site or portion determined sufficient to accommodate construction operations.
- H. Barricades, Warning Signs, and Lights: Comply with requirements of authorities having jurisdiction for erecting structurally adequate barricades, including warning signs and lighting.
- I. Temporary Egress: Maintain temporary egress from existing occupied facilities as indicated and as required by authorities having jurisdiction.
- J. Covered Walkway: Erect protective, covered walkway for passage of individuals through or adjacent to Project site. Coordinate with entrance gates, other facilities, and obstructions. Comply with regulations of authorities having jurisdiction and requirements indicated on Drawings.
- K. Temporary Enclosures: Provide temporary, weathertight, enclosures for protection of construction, in progress and completed, including, but not limited to, vertical and horizontal openings, from exposure, foul weather, other construction operations, and similar activities.
- L. Temporary Partitions: Provide floor-to-ceiling dustproof partitions to limit dust and dirt migration and to separate areas from fumes and noise.
 1. Construct dustproof partitions with gypsum wallboard with joints taped on occupied side, and fire-retardant-treated plywood on construction operations side.
 2. Construct dustproof partitions with two layers of 6 mil (0.14 mm) polyethylene sheet on each side. Cover floor with two layers of 6 mil (0.14 mm) polyethylene sheet, extending sheets 18 inches (450 mm) up the sidewalls. Overlap and tape full length of joints. Cover floor with fire-retardant-treated plywood.
 - a. Construct vestibule and airlock at each entrance through temporary partition with not less than 48 inches (1200 mm) between doors. Maintain walk-off mats in vestibule, for dust control.

3. Where fire-resistance-rated temporary partitions are indicated or are required by authorities having jurisdiction, construct partitions according to the rated assemblies.
4. Seal joints and perimeter. Equip partitions with gasketed dustproof doors and security locks where openings are required.
5. Protect air-handling equipment.
6. Provide walk-off mats at each entrance through temporary partition.

M. Temporary Fire Protection: Install and maintain temporary fire-protection facilities of types needed to protect against reasonably predictable and controllable fire losses. Comply with NFPA 241 and authorities having jurisdiction; manage fire-prevention program.

3.5 MOISTURE CONTROL

A. Contractor's Moisture-Protection Plan: Avoid trapping water in finished work. Document visible signs of discoloration that may appear during construction.

B. Exposed Construction Phase: Before installation of weather barriers, when materials are subject to wetting and exposure, protect as follows:

1. Protect porous materials from water damage.
2. Protect stored and installed material from flowing or standing water.
3. Keep porous and organic materials from coming into prolonged contact with concrete.
4. Remove standing water from decks.
5. Keep deck openings covered or dammed.

C. Partially Enclosed Construction Phase: After installation of weather barriers but before full enclosure and conditioning of building, when installed materials are still subject to infiltration of moisture, protect as follows:

1. Do not load or install drywall or other porous materials or components, or items with high organic content, into partially enclosed building.
2. Keep interior spaces reasonably clean and protected from water damage.
3. Periodically collect and remove waste containing cellulose or other organic matter.
4. Discard or replace water-damaged material.
5. Do not install material that is wet.
6. Discard, replace, or clean stored or installed material that begins to show discoloration.
7. Perform work in a sequence that allows any wet materials adequate time to dry before enclosing the material in drywall or other interior finishes.

D. Controlled Construction Phase of Construction: After completing and sealing of the building enclosure but prior to the full operation of permanent HVAC systems, maintain as follows:

1. Control moisture and humidity inside building by maintaining effective dry-in conditions.
2. Use permanent HVAC system to control humidity.
3. Comply with manufacturer's written instructions for temperature, relative humidity, and exposure to water limits. Refer to technical specification sections for additional and more stringent criteria.

3.6 OPERATION, TERMINATION, AND REMOVAL

A. Supervision: Enforce strict discipline in use of temporary facilities. To minimize waste and abuse, limit availability of temporary facilities to essential and intended uses.

B. Maintenance: Maintain facilities in good operating condition until removal.

1. Maintain operation of temporary enclosures, heating, cooling, humidity control, ventilation, and similar facilities on a 24-hour basis where required to achieve indicated results and to avoid possibility of damage.
 2. Maintain markers for underground lines. Protect from damage during excavation operations.
- C. Temporary Facility Changeover: Do not change over from using temporary security and protection facilities to permanent facilities until Substantial Completion.
- D. Termination and Removal: Remove each temporary facility when need for its service has ended, when it has been replaced by authorized use of a permanent facility, or no later than Substantial Completion. Complete or, if necessary, restore permanent construction that may have been delayed because of interference with temporary facility. Repair damaged Work, clean exposed surfaces, and replace construction that cannot be satisfactorily repaired.
1. Materials and facilities that constitute temporary facilities are property of Contractor.
 2. Remove temporary roads and paved areas not intended for or acceptable for integration into permanent construction. Where area is intended for landscape development, remove soil and aggregate fill that do not comply with requirements for fill or subsoil. Remove materials contaminated with road oil, asphalt and other petrochemical compounds, and other substances that might impair growth of plant materials or lawns. Repair or replace street paving, curbs, and sidewalks at temporary entrances, as required by authorities having jurisdiction.
 3. At Substantial Completion, repair, renovate, and clean permanent facilities used during construction period. Comply with final cleaning requirements specified in Division 01 Section "Closeout Procedures."

END OF SECTION

SECTION 016000

PRODUCT REQUIREMENTS

PART 1 - GENERAL

1.1 SUMMARY

- A. Section includes administrative and procedural requirements for selection of products for use in Project; product delivery, storage, and handling; manufacturers' standard warranties on products; special warranties; and comparable products.

1.2 DEFINITIONS

- A. Products: Items obtained for incorporating into the Work, whether purchased for Project or taken from previously purchased stock. The term "product" includes the terms "material," "equipment," "system," and terms of similar intent.
 - 1. Named Products: Items identified by manufacturer's product name, including make or model number or other designation shown or listed in manufacturer's published product literature that is current as of date of the Contract Documents.
 - 2. New Products: Items that have not previously been incorporated into another project or facility. Products salvaged or recycled from other projects are not considered new products.
 - 3. Comparable Product: Product that is demonstrated and approved through submittal process to have the indicated qualities related to type, function, dimension, in-service performance, physical properties, appearance, and other characteristics that equal or exceed those of specified product.
- B. Substitutions: Changes in products, materials, and equipment from those required by the Contract Documents and proposed by Contractor. Refer to Division 01 Section "Substitution Procedures".
- C. Basis-of-Design Product Specification: Where a specific manufacturer's product is named and accompanied by the words "Product Standard," including make or model number or other designation, to establish the significant qualities related to type, function, dimension, in-service performance, physical properties, appearance, and other characteristics for purposes of evaluating comparable products of other manufacturers
- D. Hazardous Substances Prohibited by Law: Including, but not limited to, any product, material, element, constituent, chemical, substance, compound, or mixture, which is defined in, included under, or regulated by any environmental laws.

- E. Environmental Laws: Applicable local, state, and federal laws, rules, ordinances, codes, regulations, and requirements in effect at the time Contractor's services are rendered, any amendments for Contractor's services rendered after the effective date of any such amendments.

1.3 SUBMITTALS

- A. Comparable Product: Comply with requirements in Division 01 Section "Submittal Procedures." Show compliance with requirements. Submit request for consideration of each comparable product. Identify product or fabrication or installation method to be replaced. Include Specification Section number and title and Drawing numbers and titles.
- B. Basis-of-Design Product Specification Submittal: Comply with requirements in Division 01 Section "Submittal Procedures." Show compliance with requirements.
- C. Contractor shall submit an affidavit on construction company letterhead signed by an officer of the company, notarized by a notary public, which certifies compliance with the environmental laws controlling hazardous substances for the construction of this Project.

1.4 QUALITY ASSURANCE

- A. Compatibility of Options: If Contractor is given option of selecting between two or more products for use on Project, select product compatible with products previously selected, even if previously selected products were also options.
- B. Compliance: Contractor shall take whatever measures deemed necessary to insure that all employees, suppliers, vendors, fabricators, subcontractors, or their assigns, to comply with hazardous substance requirements.

1.5 PRODUCT DELIVERY, STORAGE, AND HANDLING

- A. Deliver, store, and handle products using means and methods that will prevent damage, deterioration, and loss, including theft and vandalism. Comply with manufacturer's written instructions.
- B. Delivery and Handling:
 - 1. Schedule delivery to minimize long-term storage at Project site and to prevent overcrowding of construction spaces.
 - 2. Coordinate delivery with installation time to ensure minimum holding time for items that are flammable, hazardous, easily damaged, or sensitive to deterioration, theft, and other losses.
 - 3. Deliver products to Project site in an undamaged condition in manufacturer's original sealed container or other packaging system, complete with labels and instructions for handling, storing, unpacking, protecting, and installing.
 - 4. Inspect products on delivery to ensure compliance with the Contract Documents and to ensure that products are undamaged and properly protected.
- C. Storage:
 - 1. Store products to allow for inspection and measurement of quantity or counting of units.
 - 2. Store materials in a manner that will not endanger Project structure.

3. Store products that are subject to damage by the elements, under cover in a weathertight enclosure above ground, with ventilation adequate to prevent condensation.
4. Protect foam plastic from exposure to sunlight, except to extent necessary for period of installation and concealment.
5. Comply with product manufacturer's written instructions for temperature, humidity, ventilation, and weather-protection requirements for storage.
6. Protect stored products from damage and liquids from freezing.

1.6 PRODUCT WARRANTIES

- A. Warranties specified in other Sections shall be in addition to, and run concurrent with, other warranties required by the Contract Documents. Manufacturer's disclaimers and limitations on product warranties do not relieve Contractor of obligations under requirements of the Contract Documents.
 1. **Manufacturer's Warranty:** Written warranty furnished by individual manufacturer for a particular product, required by the Contract Documents to provide specific rights for Owner, and specifically endorsed by manufacturer to Owner.
 2. **Warranties:** Prepare a written document, on manufacturer's standard form, modified to include Project-specific information, that contains appropriate terms and identification, properly executed.
- B. **Submittal Time:** Comply with requirements in Division 01 Section "Closeout Procedures."

PART 2 - PRODUCTS

2.1 PRODUCT SELECTION PROCEDURES

- A. **General Product Requirements:** Provide products that comply with the Contract Documents, are undamaged and, unless otherwise indicated, are new at time of installation.
 1. Provide products complete with accessories, trim, finish, fasteners, and other items needed for a complete installation and indicated use and effect.
 2. **Standard Products:** If available, and unless custom products or nonstandard options are specified, provide standard products of types that have been produced and used successfully in similar situations on other projects.
 3. Owner reserves the right to limit selection to products with warranties not in conflict with requirements of the Contract Documents.
 4. Where products are accompanied by the term "as selected," Architect will make selection.
 5. Descriptive, performance, and reference standard requirements in the Specifications establish salient characteristics of products.
 6. Products and materials brought onto the Project Site, and products and materials incorporated into the Work, shall comply with environmental laws.
- B. **Product Selection Procedures:**
 1. **Product:** Where Specifications name a single manufacturer and product, provide the named product that complies with requirements. Comparable products or substitutions for Contractor's convenience will not be considered.
 2. **Manufacturer/Source:** Where Specifications name a single manufacturer or source, provide a product by the named manufacturer or source that complies with requirements.

Comparable products or substitutions for Contractor's convenience will not be considered.

3. Products:
 - a. Restricted List (Acceptable Manufacturers/Fabricators and Products): Where Specifications include a list of names of both manufacturers and products, provide one of the products listed that complies with requirements. Comparable products or substitutions for Contractor's convenience will be considered unless otherwise indicated.
 - b. Nonrestricted List (Available Manufacturers/Fabricators and Products): Where Specifications include a list of names of both available manufacturers and products, provide one of the products listed, or an unnamed product, that complies with requirements. Comply with requirements in "Comparable Products" Article for consideration of an unnamed product.
 4. Manufacturers:
 - a. Restricted List (Acceptable Manufacturers/Fabricators): Where Specifications include a list of manufacturers' names, provide a product by one of the manufacturers listed that complies with requirements. Comparable products or substitutions for Contractor's convenience will be considered unless otherwise indicated.
 - b. Nonrestricted List (Available Manufacturers/Fabricators): Where Specifications include a list of available manufacturers, provide a product by one of the manufacturers listed, or a product by an unnamed manufacturer, that complies with requirements. Comply with requirements in "Comparable Products" Article for consideration of an unnamed manufacturer's product.
 5. Basis-of-Design Product (Product Standard): Where Specifications name a product, or refer to a product indicated on Drawings, and include a list of manufacturers, provide the specified or indicated product or a comparable product by one of the other named manufacturers. Drawings and Specifications indicate sizes, profiles, dimensions, and other characteristics that are based on the product named. Comply with requirements in "Comparable Products" Article for consideration of an unnamed product by one of the other named manufacturers, or unnamed manufacturer's product.
- C. Descriptive Specification Requirements: Where Specifications describe a product, or assembly, listing exact characteristics required, without use of a brand or trade name, provide a product, material or assembly that provides the characteristics and otherwise complies with Contract requirements.
- D. Performance Specification Requirements: Where Specifications require compliance with performance requirements, provide products that comply with these requirements, and are recommended by the manufacturer for the application indicated. General overall performance of a product is implied where the product or material is specified for a specific application.
1. Manufacturer's recommendations may be contained in published product literature, or by the manufacturer's certification of performance.
- E. Compliance with Standards, Codes and Regulations: Where Specifications only require compliance with imposed code, standard or regulation, select product that complies with standards, codes or regulations specified.

- F. Visual Matching Specification: Where Specifications require "match Architect's sample", provide a product that complies with requirements and matches Architect's sample. Architect's decision will be final on whether a proposed product matches.
 - 1. If no product available within specified category matches and complies with other specified requirements, comply with requirements in Division 01 Section "Substitution Procedures" for proposal of product.
- G. Visual Selection Specification: Where Specifications include the phrase "as selected by Architect from manufacturer's color, gloss, pattern, density, or texture" or similar phrase, select a product (and manufacturer) that complies with other specified requirements.
 - 1. Standard Range: Where Specifications include the phrase "standard range of colors, patterns, textures" or similar phrase, Architect will select color, pattern, or texture from manufacturer's product line that does not include premium items.
 - 2. Custom Range: Where Specifications include the phrase "custom range of colors, patterns, textures" or similar phrase, Architect will select color, pattern, or texture from manufacturer's product line that includes both standard and premium items.
 - 3. Special Custom Range: Where Specifications include the phrase "special custom range of colors patterns, textures" or similar phrase, Architect will select a new color, pattern, or texture different from those normally produced by the manufacturer.
- A. Allowances (If Applicable): Refer to provisions of individual Specification Sections and of Division 01 Section "Allowance" for allowances that control product selection and for procedures required for processing such selections.

2.2 COMPARABLE PRODUCTS

- A. Conditions for Consideration: Architect will consider Contractor's request for comparable product when the following conditions are satisfied. If the following conditions are not satisfied, Architect may return requests without action, except to record noncompliance with these requirements:
 - 1. Evidence that the proposed product does not require revisions to the Contract Documents; that it is consistent with the Contract Documents and will produce the indicated results, and that it is compatible with other portions of the Work.
 - 2. Detailed comparison of significant qualities of proposed product with those named in the Specifications. Significant qualities include attributes such as performance, weight, size, durability, visual effect, and specific features and requirements indicated.
 - 3. Evidence that proposed product provides specified warranty.
 - 4. List of similar installations for completed projects with project names and addresses and names and addresses of architects and owners, if requested.
 - 5. Samples, if requested.

PART 3 - EXECUTION

3.1 RESTRICTION OF HAZARDOUS SUBSTANCES

- A. Contractor agrees that it shall not knowingly after reasonable diligence and effort, incorporate into the Work any hazardous substance other than as may be lawfully contained within products, except in accordance with applicable environmental laws. Further, in performing any of its obligations hereunder, Contractor shall not cause any release of hazardous substances into, or contamination of, the environment, including soil, the atmosphere, any watercourse or ground water, except in accordance with applicable environmental laws. In the event that Contractor engages in any of the activities prohibited in this paragraph, to the fullest extent permitted by law, Contractor hereby indemnifies and holds harmless Owner and its partners, members, officers, directors, agents, employees and consultants from and against any and all claims, damages, losses, causes of action, suits and liabilities of every kind, including, but not limited to, expenses of litigation, court costs, punitive damages and attorney's fees, arising out of, incidental to or resulting from the activities prohibited.
- B. In the event Contractor observes on the Project Site any substance which Contractor reasonably believes to be a hazardous substance, and which is being introduced into the Work, or exists on the Project Site, in a manner violative of any applicable environmental laws, Contractor shall immediately notify Owner and report the condition to Owner in writing. The Work in the affected area shall not thereafter be resumed except by written authorization of Owner if in fact a hazardous substance has been encountered and has not been rendered harmless. In the event that Contractor fails to give Owner proper notification hereunder, upon knowingly observing a hazardous substance at the Project Site, to the fullest extent permitted by the law, Contractor hereby indemnifies and holds harmless Owner, and all of its partners, members, officers, directors, agents, employees and consultants from and against all claims, damages, losses, causes of action, suits and liabilities of every kind, including, but not limited to, expenses of litigation, court costs, punitive damages and attorneys' fees, arising out of, incidental to, or resulting from Contractor's failure to stop the Work.
- C. If Owner believes that hazardous substances may have been located, generated, manufactured, used or disposed of on or about the Project Site by Contractor or any of its employees, agents, subcontractors, suppliers, or invitees, Owner may have environmental studies of the Project Site conducted as it deems appropriate, and Contractor shall be responsible for the cost of such studies to the extent that Contractor or any of its employees, agents, subcontractors, suppliers or invitees are responsible for the presence of any hazardous substances.

END OF SECTION

SECTION 01 7300

EXECUTION

PART 1 - GENERAL

1.1 SUMMARY

- A. Section includes general administrative and procedural requirements governing execution of the Work including, but not limited to, the following:
 - 1. Construction layout.
 - 2. Field engineering and surveying.
 - 3. Installation of the Work.
 - 4. Cutting and patching.
 - 5. Coordination of Owner-installed products.
 - 6. Progress cleaning.
 - 7. Starting and adjusting.
 - 8. Protection of installed construction.
 - 9. Correction of the Work.

1.2 DEFINITIONS

- A. Cutting: Removal of in-place construction necessary to permit installation or performance of other work.
- B. Patching: Fitting and repair work required to restore construction to original conditions after installation of other work.

1.3 SUBMITTALS

- A. Qualification Data: For land surveyor or professional engineer.
- B. Certificates: Submit certificate signed by land surveyor or professional engineer certifying that location and elevation of improvements comply with requirements.
- C. Cutting and Patching Plan: Submit plan describing procedures at least 10 days prior to the time cutting and patching will be performed, requesting approval to proceed. Include the following information:
 - 1. Extent: Describe reason for and extent of each occurrence of cutting and patching.
 - 2. Changes to In-Place Construction: Describe anticipated results. Include changes to structural elements and operating components as well as changes in building appearance and other significant visual elements.
 - 3. Products: List products to be used for patching and firms or entities that will perform patching work.
 - 4. Dates: Indicate when cutting and patching will be performed.

5. Utilities and Mechanical and Electrical Systems: List services and systems that cutting and patching procedures will disturb or affect. List services and systems that will be relocated and those that will be temporarily out of service. Indicate length of time permanent services and systems will be disrupted.
 - a. Include description of provisions for temporary services and systems during interruption of permanent services and systems.
 6. Structural Elements: Where cutting and patching involve adding reinforcement to structural elements, submit details and engineering calculations showing integration of reinforcement with original structure.
 7. Architect's Approval: Obtain approval of cutting and patching proposal before cutting and patching. Approval does not waive right to later require removal and replacement of unsatisfactory work.
- D. Certified Surveys: Submit two copies signed by land surveyor or professional engineer.
- E. Final Property Survey: Submit 6 copies showing the Work performed and record survey data.

1.4 QUALITY ASSURANCE

- A. Land Surveyor Qualifications: A professional land surveyor or engineer who is legally qualified to practice in jurisdiction where Project is located and who is experienced in providing land-surveying services of the kind indicated.
- B. Cutting and Patching: Comply with requirements for and limitations on cutting and patching of construction elements.
1. Structural Elements: Do not cut and patch structural elements in a manner that could change their load-carrying capacity or increase deflection
 2. Operational Elements: Do not cut and patch operating elements and related components in a manner that results in reducing their capacity to perform as intended or that results in increased maintenance or decreased operational life or safety.
 3. Miscellaneous Elements: Do not cut and patch other construction elements or components in a manner that could change their load-carrying capacity, that results in reducing their capacity to perform as intended, or that results in increased maintenance or decreased operational life or safety.
 4. Visual Elements: Do not cut and patch construction in a manner that results in visual evidence of cutting and patching. Do not cut and patch exposed construction on the exterior or in occupied spaces in a manner that would, in Architect's opinion, reduce the building's aesthetic qualities. Remove and replace construction that has been cut and patched in a visually unsatisfactory manner.
 - a. If possible, retain original Installer or fabricator to cut and patch exposed Work listed below. If it is impossible to engage original Installer or fabricator, engage another recognized, experienced, and specialized firm.
- C. Cutting and Patching Conference: Before proceeding, meet at Project site with parties involved in cutting and patching, including mechanical and electrical trades. Review areas of potential interference and conflict. Coordinate procedures and resolve potential conflicts before proceeding.
- D. Manufacturer's Installation Instructions: Obtain and maintain on-site manufacturer's written recommendations and instructions for installation of products and equipment.

1.5 WARRANTY

- A. Existing Warranties: Remove, replace, patch, and repair materials and surfaces cut or damaged during cutting and patching operations, by methods and with materials so as not to void existing warranties.

PART 2 - PRODUCTS

2.1 MATERIALS

- A. General: Comply with requirements specified in other Sections.
 - 1. For projects requiring compliance with sustainable design and construction practices and procedures, use products for patching that comply with requirements in Division 01 sustainable construction requirements Section.
- B. In-Place Materials: Use materials for patching identical to in-place materials. For exposed surfaces, use materials that visually match in-place adjacent surfaces to the fullest extent possible.
 - 1. If identical materials are unavailable or cannot be used, use materials that, when installed, will provide a match acceptable to Architect for the visual and functional performance of in-place materials.

PART 3 - EXECUTION

3.1 EXAMINATION

- A. Existing Conditions: The existence and location of site improvements, utilities, and other construction indicated as existing are not guaranteed. Before beginning work, investigate and verify the existence and location of mechanical and electrical systems and other construction affecting the Work.
 - 1. Before construction, verify the location and points of connection of utility services.
- B. Existing Conditions: The existence and location of underground and other utilities and construction indicated as existing are not guaranteed. Before beginning sitework, investigate and verify the existence and location of underground utilities, and other construction affecting the Work.
 - 1. Before construction, verify the location and invert elevation at points of connection of sanitary sewer, storm sewer, and water-service piping; underground electrical services, and other utilities.
 - 2. Furnish location data for work related to Project that must be performed by public utilities serving Project site.

- C. Acceptance of Surfaces and Conditions: Examine substrates to receive products and systems and associated work for compliance with requirements and other conditions affecting performance. Proceed only when unsatisfactory conditions have been corrected in a manner complying with Contract Documents. Starting work within a particular area will be construed as acceptance of surface conditions. Record observations.
 - 1. Examine roughing-in for mechanical and electrical systems to verify actual locations of connections before equipment and fixture installation.
 - 2. Examine walls, floors, and roofs for suitable conditions where products and systems are to be installed.
 - 3. Verify compatibility with and suitability of substrates, including compatibility with existing finishes or primers.

3.2 INSTALLATION, GENERAL

- A. Installation Quality Standards: In addition to standards listed elsewhere, perform Work according to following, unless otherwise specified:
 - 1. Respective manufacturer/fabricator's written installation instructions.
 - 2. Accepted submittals.
 - 3. Contract Documents.
- B. Control of Corrosion: Prevent galvanic action and other forms of corrosion by isolating metals and other materials from direct contact with incompatible materials.

3.3 PREPARATION

- A. General: Comply with manufacturer's instructions, recommendations, and specifications for cleaning and surface preparation. Surfaces shall have no defects, contaminants, or errors which would result in poor or potentially defective installation or would cause latent defects in Work.
- B. Existing Utility Information: Furnish information to local utility that is necessary to adjust, move, or relocate existing utility structures, utility poles, lines, services, or other utility appurtenances located in or affected by construction. Coordinate with authorities having jurisdiction.
- A. Existing Utility Interruptions at Renovation Work: Do not interrupt utilities serving facilities occupied by Owner or others unless permitted under the following conditions and then only after arranging to provide temporary utility services according to requirements indicated:
 - 1. Notify Owner not less than 72 hours in advance of proposed utility interruptions.
 - 2. Do not proceed with utility interruptions without Owner's written permission.
- D. Field Measurements: Take field measurements as required to fit the Work properly. Recheck measurements before installing each product. Where portions of the Work are indicated to fit to other construction, verify dimensions of other construction by field measurements before fabrication. Coordinate fabrication schedule with construction progress to avoid delaying the Work.
- E. Space Requirements: Verify space requirements and dimensions of items shown diagrammatically on Drawings.
- F. Review of Contract Documents and Field Conditions: Immediately on discovery of the need for clarification of the Contract Documents caused by differing field conditions outside the control of

Contractor, submit a request for information to Architect according to requirements in Division 01 Section "Project Management and Coordination."

3.4 CONSTRUCTION LAYOUT

- A. Verification: Before proceeding to lay out the Work, verify layout information shown on Drawings, in relation to the property survey and existing benchmarks. If discrepancies are discovered, notify Architect promptly.
- B. General: Engage a land surveyor or professional engineer to lay out the Work using accepted surveying practices.
 - 1. Establish benchmarks and control points to set lines and levels at each story of construction and elsewhere as needed to locate each element of Project.
 - 2. Establish dimensions within tolerances indicated. Do not scale Drawings to obtain required dimensions.
 - 3. Inform installers of lines and levels to which they must comply.
 - 4. Check the location, level and plumb, of every major element as the Work progresses.
 - 5. Notify Architect when deviations from required lines and levels exceed allowable tolerances.
 - 6. Close site surveys with an error of closure equal to or less than the standard established by authorities having jurisdiction.
- C. Site Improvements: Locate and lay out site improvements, including pavements, grading, fill and topsoil placement, utility slopes, and rim and invert elevations.
- D. Building Lines and Levels: Locate and lay out control lines and levels for structures, building foundations, column grids, and floor levels, including those required for mechanical and electrical work. Transfer survey markings and elevations for use with control lines and levels. Level foundations and piers from two or more locations.
- E. Record Log: Maintain a log of layout control work. Record deviations from required lines and levels. Include beginning and ending dates and times of surveys, weather conditions, name and duty of each survey party member, and types of instruments and tapes used. Make the log available for reference by Architect.

3.5 FIELD ENGINEERING

- A. Reference Points: Locate existing permanent benchmarks, control points, and similar reference points before beginning the Work. Preserve and protect permanent benchmarks and control points during construction operations.
 - 1. Do not change or relocate existing benchmarks or control points. Report lost or destroyed permanent benchmarks or control points promptly. Report the need to relocate permanent benchmarks or control points to Architect before proceeding.
 - 2. Replace lost or destroyed permanent benchmarks and control points promptly. Base replacements on the original survey control points.
- B. Benchmarks: Establish and maintain a minimum of 2 permanent benchmarks on Project site, referenced to data established by survey control points. Comply with authorities having jurisdiction for type and size of benchmark.
 - 1. Record benchmark locations, with horizontal and vertical data, on Project Record Documents.

2. Where the actual location or elevation of layout points cannot be marked, provide temporary reference points sufficient to locate the Work.
 3. Remove temporary reference points when no longer needed. Restore marked construction to its original condition.
- C. Certified Survey: 30 days after completion of each work component/activity, including, but not limited to, foundation walls, major site improvements, and other work requiring field-engineering services, prepare a certified survey showing dimensions, locations, angles, and elevations of construction and sitework.
- D. Final Property Survey: Engage a land surveyor or professional engineer to prepare a final property survey showing significant features (real property) for Project. Include on the survey a certification, signed by land surveyor or professional engineer, that principal metes, bounds, lines, and levels of Project are accurately positioned as shown on the survey.
1. Show boundary lines, monuments, streets, site improvements and utilities, existing improvements and significant vegetation, adjoining properties, acreage, grade contours, and the distance and bearing from a site corner to a legal point.
 2. Recording: At Substantial Completion, have the final property survey recorded by or with authorities having jurisdiction as the official "property survey."

3.6 INSTALLATION

- A. General: Locate the Work and components of the Work accurately, in correct alignment and elevation, as indicated.
1. Make vertical work plumb and make horizontal work level.
 2. Where space is limited, install components to maximize space available for maintenance and ease of removal for replacement.
 3. Conceal pipes, ducts, and wiring in finished areas unless otherwise indicated.
- B. Comply with manufacturer's written instructions and recommendations for installing products in applications indicated, unless indicated otherwise in the Contract Documents.
- C. Install products at the time and under conditions that will ensure the best possible results. Maintain conditions required for product performance until Substantial Completion.
- D. Conduct construction operations so no part of the Work is subjected to damaging operations or loading in excess of that expected during normal conditions of occupancy.
- E. Sequence the Work and allow adequate clearances to accommodate movement of construction items on site and placement in permanent locations.
- F. Tools and Equipment: Do not use tools or equipment that produce harmful noise levels.
- G. Templates: Obtain and distribute to the parties involved templates for work specified to be factory prepared and field installed. Check Shop Drawings of other work to confirm that adequate provisions are made for locating and installing products to comply with indicated requirements.
- H. Attachment: Provide blocking and attachment plates and anchors and fasteners of adequate size and number to securely anchor each component in place, accurately located, aligned, and coordinated with other portions of the Work. Where size and type of attachments are not indicated, verify size and type required for load conditions.

1. Mounting Heights: Where mounting heights are not indicated, mount components at heights directed by Architect.
 2. Allow for building movement, including thermal expansion and contraction.
 3. Coordinate installation of anchorages. Furnish setting drawings, templates, and directions for installing anchorages, including sleeves, concrete inserts, anchor bolts, and items with integral anchors, that are to be embedded in concrete or masonry. Deliver such items to Project site in time for installation.
- I. Joints: Make joints of uniform width. Where joint locations in exposed work are not indicated, arrange joints for the best visual effect. Fit exposed connections together to form hairline joints.
- J. Hazardous Materials: Use products, cleaners, and installation materials that are not considered hazardous.

3.7 CUTTING AND PATCHING

- A. Cutting and Patching, General: Employ skilled workers to perform cutting and patching. Proceed with cutting and patching at the earliest feasible time, and complete without delay.
1. Cut in-place construction to provide for installation of other components or performance of other construction, and subsequently patch as required to restore surfaces to their original condition.
- B. Existing Warranties: Remove, replace, patch, and repair materials and surfaces cut or damaged during installation or cutting and patching operations, by methods and with materials so as not to void existing warranties.
- C. Temporary Support: Provide temporary support of work to be cut.
- D. Protection: Protect in-place construction during cutting and patching to prevent damage. Provide protection from adverse weather conditions for portions of Project that might be exposed during cutting and patching operations.
- E. Adjacent Occupied Areas: Where interference with use of adjoining areas or interruption of free passage to adjoining areas is unavoidable, coordinate cutting and patching according to requirements in Division 01 Section "Summary."
- F. Existing Utility Services and Mechanical/Electrical Systems: Where existing services/systems are required to be removed, relocated, or abandoned, bypass such services/systems before cutting to prevent interruption to occupied areas.
- G. Cutting: Cut in-place construction by sawing, drilling, breaking, chipping, grinding, and similar operations, including excavation, using methods least likely to damage elements retained or adjoining construction. If possible, review proposed procedures with original Installer; comply with original Installer's written recommendations.
1. In general, use hand or small power tools designed for sawing and grinding, not hammering and chopping. Cut holes and slots neatly to minimum size required, and with minimum disturbance of adjacent surfaces. Temporarily cover openings when not in use.
 2. Finished Surfaces: Cut or drill from the exposed or finished side into concealed surfaces.
 3. Concrete and Masonry: Cut using a cutting machine, such as an abrasive saw or a diamond-core drill.
 4. Excavating and Backfilling: Comply with requirements in applicable Division 31 Sections where required by cutting and patching operations.

5. Mechanical and Electrical Services: Cut off pipe or conduit in walls or partitions to be removed. Cap, valve, or plug and seal remaining portion of pipe or conduit to prevent entrance of moisture or other foreign matter after cutting.
 6. Proceed with patching after construction operations requiring cutting are complete.
- H. Patching: Patch construction by filling, repairing, refinishing, closing up, and similar operations following performance of other work. Patch with durable seams that are as invisible as practicable. Provide materials and comply with installation requirements specified in other Sections, where applicable.
1. Inspection: Where feasible, test and inspect patched areas after completion to demonstrate physical integrity of installation.
 2. Exposed Finishes: Restore exposed finishes of patched areas and extend finish restoration into retained adjoining construction in a manner that will minimize evidence of patching and refinishing.
 - a. Clean piping, conduit, and similar features before applying paint or other finishing materials.
 - b. Restore damaged pipe covering to its original condition.
 3. Floors and Walls: Where walls or partitions that are removed extend one finished area into another, patch and repair floor and wall surfaces in the new space. Provide an even surface of uniform finish, color, texture, and appearance. Remove in-place floor and wall coverings and replace with new materials, if necessary, to achieve uniform color and appearance.
 - a. Where patching occurs in a painted surface, prepare substrate and apply primer and intermediate paint coats appropriate for substrate over the patch, and apply final paint coat over entire unbroken surface containing the patch. Provide additional coats until patch blends with adjacent surfaces.
 - b. Patch fire rated assemblies with materials to match existing and maintain assembly fire rating.
 4. Ceilings: Patch, repair, or rehang in-place ceilings as necessary to provide an even-plane surface of uniform appearance.
 5. Exterior Building Enclosure: Patch components in a manner that restores enclosure to a weathertight condition and ensures thermal and moisture integrity of building enclosure.
- I. Cleaning: Clean areas and spaces where cutting and patching are performed. Remove paint, mortar, oils, putty, and similar materials from adjacent finished surfaces.

3.8 OWNER-INSTALLED PRODUCTS

- A. Site Access: As applicable, provide access to Project site for Owner's construction personnel.
- B. Coordination: Coordinate construction and operations of the Work with work performed by Owner's construction personnel.
 1. Construction Schedule: Inform Owner of Contractor's preferred construction schedule for Owner's portion of the Work. Adjust construction schedule based on a mutually agreeable timetable. Notify Owner if changes to schedule are required due to differences in actual construction progress.
 2. Preinstallation Conferences: Include Owner's construction personnel at preinstallation conferences covering portions of the Work that are to receive Owner's work. Attend preinstallation conferences conducted by Owner's construction personnel if portions of the Work depend on Owner's construction.

3.9 PROGRESS CLEANING

- A. General: Clean Project site and work areas daily, including common areas. Enforce requirements strictly. Dispose of materials lawfully.
 - 1. Comply with requirements in NFPA 241 for removal of combustible waste materials and debris.
 - 2. Do not hold waste materials more than 7 days during normal weather or 3 days if the temperature is expected to rise above 80 deg F (27 deg C).
 - 3. Containerize hazardous and unsanitary waste materials separately from other waste. Mark containers appropriately and dispose of legally, according to regulations.
 - a. Use containers specifically intended for holding types of waste materials identified where applicable, e.g. blue colored containers with labeling and symbols for bio-waste.
- B. Site: Maintain Project site free of waste materials and debris.
- C. Work Areas: Clean areas where work is in progress to the level of cleanliness necessary for proper execution of the Work.
 - 1. Remove liquid spills immediately.
 - 2. Where dust would impair proper execution of the Work, broom-clean or vacuum the entire work area, as appropriate.
- D. Installed Work: Keep installed work clean. Clean installed surfaces according to written instructions of manufacturer or fabricator of product installed, using only cleaning materials specifically recommended. If specific cleaning materials are not recommended, use cleaning materials that are not hazardous to health or property and that will not damage exposed surfaces.
- E. Concealed Spaces: Remove debris from concealed spaces before enclosing the space.
- F. Exposed Surfaces in Finished Areas: Clean exposed surfaces and protect as necessary to ensure freedom from damage and deterioration at time of Substantial Completion.
- G. Waste Disposal: Do not bury or burn waste materials on-site. Do not wash waste materials down sewers or into waterways. Comply with waste disposal requirements in Division 01 Section "Temporary Facilities and Controls." and Division 01 Section "Construction Waste Management and Disposal", whichever is the more restrictive.
- H. Remove construction markings not required and graffiti immediately, repairing or replacing damaged material.
- I. During handling and installation, clean and protect construction in progress and adjoining materials already in place. Apply protective covering where required to ensure protection from damage or deterioration at Substantial Completion.
- J. Clean and provide maintenance on completed construction as frequently as necessary through the remainder of the construction period. Adjust and lubricate operable components to ensure operability without damaging effects.
- K. Limiting Exposures: Supervise construction operations to assure that no part of the construction, completed or in progress, is subject to harmful, dangerous, damaging, or otherwise deleterious exposure during the construction period.

3.10 STARTING AND ADJUSTING

- A. As applicable, coordinate startup and adjusting of equipment and operating components with commissioning requirements in Division 01 specification sections.
- B. Start equipment and operating components to confirm proper operation. Remove malfunctioning units, replace with new units, and retest.
- C. Adjust equipment for proper operation. Adjust operating components for proper operation without binding.
- D. Test each piece of equipment to verify proper operation. Test and adjust controls and safeties. Replace damaged and malfunctioning controls and equipment.
- E. Manufacturer's Field Service: Comply with qualification requirements in Division 01 Section "Quality Requirements."

3.11 PROTECTION OF INSTALLED CONSTRUCTION

- A. Provide final protection and maintain conditions that ensure installed Work is without damage or deterioration at time of Substantial Completion.
- B. Comply with manufacturer's written instructions for temperature and relative humidity.

END OF SECTION

SECTION 01 74 16 – CLEANUP(SITE MAINTENANCE)

1.0 GENERAL

1.1 DURING CONSTRUCTION

The Contractor shall at all times keep the job site as free from all materials, debris, and rubbish, as is practicable and shall remove same from any portion of the job site, when, in the opinion of the Owner, it becomes objectionable or interferes with the progress of the project.

1.2 FINAL

Upon completion of the work, the Contractor shall remove from the site, material, tools and equipment belonging to him, and leave the site with an appearance acceptable to the Engineer.

1.2.1 Clean Equipment and Materials: The Contractor shall thoroughly clean all equipment and materials installed by him and shall deliver all such materials and equipment in a bright, clean, polished and new-appearing condition.

1.2.2 Restoration of Landscape Damage: Any landscape feature scarred or damaged by the Contractor's equipment or operations shall be restored as nearly as possible to its original condition at the Contractor's expense. The Owner will decide what method of restoration shall be used.

1.2.3 Post-Construction Cleanup and Obliteration: The Contractor shall obliterate all signs of temporary construction facilities such as haul roads, work areas, structures, foundations of temporary structures, stockpiles of excess or waste materials or any other vestiges of construction, as directed by the Owner.

1.2.4 Restoration of Roads/pavements: The Contractor shall be responsible for restoring all roads to driveable condition, equal to or greater than original condition.

1.2.5 Restoration of Fence: The Contractor shall be responsible for reconstructing all fences, removed or damaged during construction, to original condition.

END OF SECTION

SECTION 01 7419

CONSTRUCTION WASTE MANAGEMENT AND DISPOSAL

PART 1 - GENERAL

1.1 SUMMARY

- A. Section includes administrative and procedural requirements for the following:
 - 1. Salvaging and/or recycling nonhazardous demolition and construction waste.
 - 2. Disposing of nonhazardous construction waste.

1.2 DEFINITIONS

- A. Construction Waste: Building and site improvement materials and other solid waste resulting from construction, remodeling, renovation, or repair operations. Construction waste includes packaging.
- B. Disposal: Removal off-site of demolition and construction waste and subsequent sale, recycling, reuse, or deposit in landfill or incinerator acceptable to authorities having jurisdiction.
- C. Salvage / Recycle: Recovery of demolition or construction waste for subsequent processing in preparation for reuse.

1.3 PERFORMANCE REQUIREMENTS

- A. General: Achieve end-of-Project rates for salvage/recycling of 50 percent by weight of total non-hazardous solid waste generated by the Work. Practice efficient waste management in the use of materials in the course of the Work. Use all reasonable means to divert construction and demolition waste from landfills and incinerators. Facilitate recycling and salvage of materials.

1.4 SUBMITTALS

- A. Waste Management Plan: Submit plan within 30 days of date established for commencement of the Work.
- B. Waste Reduction Progress Reports: Concurrent with each Application for Payment, submit report. Include the following information:
 - 1. Material category.
 - 2. Generation point of waste.
 - 3. Total quantity of waste in tons (tonnes).
 - 4. Total quantity of waste recovered (salvaged plus recycled) in tons (tonnes).
 - 5. Total quantity of waste recovered (salvaged plus recycled) as a percentage of total waste.
- C. Waste Reduction Calculations: Before request for Substantial Completion, submit calculated end-of-Project rates for salvage, recycling, and disposal as a percentage of total waste generated by the Work.

- D. Recycling and Processing Facility Records: Indicate receipt and acceptance of recyclable waste by recycling and processing facilities licensed to accept them. Include manifests, weight tickets, receipts, and invoices.
- E. Landfill and Incinerator Disposal Records: Indicate receipt and acceptance of waste by landfills and incinerator facilities licensed to accept them. Include manifests, weight tickets, receipts, and invoices.
- F. Submittal: Letter signed by Contractor, tabulating total waste material, quantities diverted and means by which it is diverted, and statement that requirements have been met.

1.5 QUALITY ASSURANCE

- A. Regulatory Requirements: Comply with hauling and disposal regulations of authorities having jurisdiction.
- B. Waste Management Conference: Conduct conference at Project site to comply with requirements in Division 01 Section "Project Management and Coordination." Review methods and procedures related to waste management including, but not limited to, the following:
 1. Review and discuss waste management plan.
 2. Review requirements for documenting quantities of each type of waste and its disposition.
 3. Review and finalize procedures for materials separation and verify availability of containers and bins needed to avoid delays.
 4. Review procedures for periodic waste collection and transportation to recycling and disposal facilities.
 5. Review waste management requirements for each trade.

1.6 WASTE MANAGEMENT PLAN

- A. General: Develop a waste management plan according to ASTM E 1609 and requirements of this Section. Plan shall consist of waste identification, waste reduction work plan, and cost/revenue analysis. Indicate quantities by weight or volume, but use same units of measure throughout waste management plan.
- B. Waste Identification: Indicate anticipated types and quantities of site-clearing and construction waste generated by the Work. Include estimated quantities and assumptions for estimates.
- C. Waste Reduction Work Plan: List each type of waste and whether it will be salvaged, recycled, or disposed of in landfill or incinerator. Include points of waste generation, total quantity of each type of waste, quantity for each means of recovery, and handling and transportation procedures.
 1. Recycled Materials: Include list of local receivers and processors and type of recycled materials each will accept. Include names, addresses, and telephone numbers.
 2. Disposed Materials: Indicate how and where materials will be disposed of. Include name, address, and telephone number of each landfill and incinerator facility.
 3. Handling and Transportation Procedures: Include method that will be used for separating recyclable waste including sizes of containers, container labeling, and designated location on Project site where materials separation will be located.

- D. Cost/Revenue Analysis: Indicate total cost of waste disposal as if there was no waste management plan and net additional cost or net savings resulting from implementing waste management plan. Include the following:
1. Total quantity of waste.
 2. Estimated cost of disposal (cost per unit). Include hauling and tipping fees and cost of collection containers for each type of waste.
 3. Total cost of disposal (with no waste management).
 4. Revenue from salvaged / recycled materials.
 5. Savings in hauling and tipping fees that are avoided.
 6. Handling and transportation costs. Include cost of collection containers for each type of waste.
 7. Net additional cost or net savings from waste management plan.

PART 2 - PRODUCTS (Not Used)

PART 3 - EXECUTION

3.1 PLAN IMPLEMENTATION

- A. General: Implement approved waste management plan. Provide handling, containers, storage, signage, transportation, and other items as required to implement waste management plan during the entire duration of the Contract.
1. Comply with Division 01 Section "Temporary Facilities and Controls" for operation, termination, and removal requirements.
- B. Site Access and Temporary Controls: Conduct waste management operations to ensure minimum interference with roads, streets, walks, walkways, and other adjacent occupied and used facilities.
1. Designate and label specific areas on Project site necessary for separating materials that are to be salvaged, recycled, reused, donated, and sold.
 2. Comply with Division 01 Section "Temporary Facilities and Controls" for controlling dust and dirt, environmental protection, and noise control.

3.2 RECYCLING CONSTRUCTION WASTE

- A. Preparation of Waste: Prepare and maintain recyclable waste materials according to recycling or reuse facility requirements. Maintain materials free of dirt, adhesives, solvents, petroleum contamination, and other substances deleterious to the recycling process.

3.3 DISPOSAL OF WASTE

- A. General: Except for items or materials to be salvaged, recycled, or otherwise reused, remove waste materials from Project site and legally dispose of them in a landfill or incinerator acceptable to authorities having jurisdiction.
1. Except as otherwise specified, do not allow waste materials that are to be disposed of accumulate on-site.

2. Remove and transport debris in a manner that will prevent spillage on adjacent surfaces and areas.
- B. Burning: Do not burn waste materials.
 - C. Disposal: Transport waste materials off Owner's property and legally dispose of them.

END OF SECTION

SECTION 01 7700

CLOSEOUT PROCEDURES

PART 1 - GENERAL

1.1 SUMMARY

- A. Section includes administrative and procedural requirements for contract closeout, including, but not limited to, the following:
 - 1. Substantial Completion procedures.
 - 2. Final completion procedures.
 - 3. Warranties.
 - 4. Final cleaning.
 - 5. Repair of the Work.

1.2 SUBMITTALS

- A. Contractor's List of Incomplete Items: Initial submittal at Substantial Completion.
- B. Certified List of Incomplete Items: Final submittal at Final Completion.
- C. Certificates of Release: From authorities having jurisdiction.
- D. Certificate of Insurance: For continuing coverage.
- E. Field Report: For pest control inspection.
- F. Schedule of Maintenance Material Items: For maintenance material submittal items specified in other Sections.

1.3 SUBSTANTIAL COMPLETION PROCEDURES

- A. Contractor's List of Incomplete Items: Prepare and submit a list of items to be completed and corrected (Contractor's punch list), indicating the value of each item on the list and reasons why the Work is incomplete.
- B. Submittals Prior to Substantial Completion: Complete the following prior to requesting inspection for determining date of Substantial Completion. List items below that are incomplete at time of request.
 - 1. Certificates of Release: Obtain and submit releases from authorities having jurisdiction permitting Owner unrestricted use of the Work and access to services and utilities. Include occupancy permits, operating certificates, and similar releases.
 - 2. Submit closeout submittals specified in other Division 01 Sections, including project record documents, operation and maintenance manuals, final completion construction photographic documentation, damage or settlement surveys, property surveys, and similar final record information.
 - 3. Submit closeout submittals specified in individual Divisions 02 through 33 Sections, including specific warranties, workmanship bonds, maintenance service agreements, final certifications, and similar documents.

4. Submit maintenance material submittals specified in individual Divisions 02 through 33 Sections, including tools, spare parts, extra materials, and similar items, and deliver to location designated by Architect. Label with manufacturer's name and model number where applicable.
 - a. Schedule of Maintenance Material Items: Prepare and submit schedule of maintenance material submittal items, including name and quantity of each item and name and number of related Specification Section. Obtain Owner's signature for receipt of submittals.
 5. Submit test/adjust/balance records.
- C. Procedures Prior to Substantial Completion: Complete the following prior to requesting inspection for determining date of Substantial Completion. List items below that are incomplete at time of request.
1. Advise Owner of pending insurance changeover requirements.
 2. Make final changeover of permanent locks and deliver keys to Owner. Advise Owner's personnel of changeover in security provisions.
 3. Complete startup and testing of systems and equipment.
 4. Perform preventive maintenance on equipment used prior to Substantial Completion.
 5. Instruct Owner's personnel in operation, adjustment, and maintenance of products, equipment, and systems. Submit demonstration and training video recordings specified in Division 01 Section "Demonstration and Training."
 6. Advise Owner of changeover in heat and other utilities.
 7. Participate with Owner in conducting inspection and walkthrough with local emergency responders.
 8. Terminate and remove temporary facilities from Project site, along with mockups, construction tools, and similar elements.
 9. Complete final cleaning requirements, including touchup painting.
 10. Touch up and otherwise repair and restore marred exposed finishes to eliminate visual defects.
- D. Inspection: Submit a written request for inspection to determine Substantial. On receipt of request, Architect will either proceed with inspection or notify Contractor of unfulfilled requirements. Architect will prepare the Certificate of Substantial Completion after inspection or will notify Contractor of items, either on Contractor's list or additional items identified by Architect, that must be completed or corrected before certificate will be issued.
1. Reinspection: Request, in writing, reinspection when the Work identified in previous inspections as incomplete is completed or corrected.
 2. Results of completed inspection will form the basis of requirements for final completion.
- E. Warranties required by the Contract Documents shall commence on the date of Substantial Completion of the Work or designated portion thereof unless otherwise provided in the Certificate of Substantial Completion.

1.4 FINAL COMPLETION PROCEDURES

- A. Submittals Prior to Final Completion: Before requesting final inspection for determining final completion, complete the following:
1. Submit a final Application for Payment according to Division 01 Section "Payment Procedures."

2. Certified List of Incomplete Items: Submit certified copy of Architect's Substantial Completion inspection list of items to be completed or corrected (punch list). Certified copy of the list shall state that each item has been completed or otherwise resolved for acceptance.
 3. Certificate of Insurance: Submit evidence of final, continuing insurance coverage complying with insurance requirements.
 4. Submit pest-control final inspection report.
- B. Inspection: Submit a written request for final inspection for acceptance. On receipt of request, Architect will either proceed with inspection or notify Contractor of unfulfilled requirements. Architect will prepare a final Certificate for Payment after inspection or will notify Contractor of construction that must be completed or corrected before certificate will be issued.
1. Reinspection: Request reinspection when the Work identified in previous inspections as incomplete is completed or corrected.

1.5 LIST OF INCOMPLETE ITEMS (PUNCH LIST)

- A. Organization of List: Include name and identification of each space and area affected by construction operations for incomplete items and items needing correction including, if necessary, areas disturbed by Contractor that are outside the limits of construction
1. Organize list of spaces in sequential order, starting with exterior areas first and proceeding from lowest floor to highest floor.
 2. Organize items applying to each space by major element, including categories for ceiling, individual walls, floors, equipment, and building systems.
 3. Include the following information at the top of each page:
 - a. Project name.
 - b. Date.
 - c. Name of Architect.
 - d. Name of Contractor.
 - e. Page number.
 4. Submit list of incomplete items in the format agreed upon by the Owner and Architect.

1.6 SUBMITTAL OF PROJECT WARRANTIES

- A. Time of Submittal: Submit written warranties for designated portions of the Work where commencement of warranties other than date of Substantial Completion is indicated, or when delay in submittal of warranties might limit Owner's rights under warranty.
- B. Partial Occupancy: Submit properly executed warranties within minimum number days, as required by the Contract, of completion of designated portions of the Work that are completed and occupied or used by Owner during construction period by separate agreement with Contractor.
- C. Organize warranty documents into an orderly sequence based on the table of contents of Project Manual.
1. Warranty Electronic File: Scan warranties and bonds and assemble complete warranty and bond submittal package into a single indexed electronic PDF file with links enabling navigation to each item. Provide bookmarked table of contents at beginning of document.

- D. Provide additional copies of each warranty to include in operation and maintenance manuals.

PART 2 - PRODUCTS

2.1 MATERIALS

- A. Cleaning Agents: Use cleaning materials and agents recommended by manufacturer or fabricator of the surface to be cleaned. Do not use cleaning agents that are potentially hazardous to health or property or that might damage finished surfaces.
 - 1. Use cleaning products that comply with Green Seal's GS-37, or if GS-37 is not applicable, use products that comply with the California Code of Regulations maximum allowable VOC levels.

PART 3 - EXECUTION

3.1 FINAL CLEANING

- A. General: Perform final cleaning. Conduct cleaning and waste-removal operations to comply with local laws and ordinances and Federal and local environmental and antipollution regulations.
- B. Cleaning: Employ experienced workers or professional cleaners for final cleaning. Clean each surface or unit to condition expected in an average commercial building cleaning and maintenance program. Comply with manufacturer's written instructions.
 - 1. Complete the following cleaning operations, as applicable, before requesting inspection for certification of Substantial Completion for entire Project or for a portion of Project:
 - a. Clean Project site, yard, and grounds, in areas disturbed by construction activities, including landscape development areas, of rubbish, waste material, litter, and other foreign substances.
 - b. Sweep paved areas broom clean. Remove petrochemical spills, stains, and other foreign deposits.
 - c. Rake grounds that are neither planted nor paved to a smooth, even-textured surface.
 - d. Remove tools, construction equipment, machinery, and surplus material from Project site.
 - e. Remove snow and ice to provide safe access to building.
 - f. Clean exposed exterior and interior hard-surfaced finishes to a dirt-free condition, free of stains, films, and similar foreign substances. Avoid disturbing natural weathering of exterior surfaces. Restore reflective surfaces to their original condition.
 - g. Remove debris and surface dust from limited access spaces, including roofs, plenums, shafts, trenches, equipment vaults, manholes, attics, and similar spaces.
 - h. Sweep concrete floors broom clean in unoccupied spaces.
 - i. Vacuum carpet and similar soft surfaces, removing debris and excess nap; clean according to manufacturer's recommendations if visible soil or stains remain.
 - j. Clean transparent materials, including mirrors and glass in doors and windows. Remove glazing compounds and other noticeable, vision-obscuring materials. Polish mirrors and glass, taking care not to scratch surfaces.
 - k. Remove labels that are not permanent.

- l. Remove all graffiti and construction writing.
 - m. Wipe surfaces of mechanical and electrical equipment and similar equipment. Remove excess lubrication, paint and mortar droppings, and other foreign substances.
 - n. Clean plumbing fixtures to a sanitary condition, free of stains, including stains resulting from water exposure.
 - o. Replace disposable air filters and clean permanent air filters. Clean exposed surfaces of diffusers, registers, and grills.
 - p. Clean ducts, blowers, and coils if units were operated without filters during construction or that display contamination with particulate matter on inspection.
 - q. Clean light fixtures, lamps, globes, and reflectors to function with full efficiency.
 - r. Leave Project clean and ready for occupancy.
- C. Pest Control: Comply with pest control requirements in Division 01 Section "Temporary Facilities and Controls." Prepare written report.
- D. Construction Waste Disposal: Comply with waste disposal requirements in Division 01 Section "Temporary Facilities and Controls." and Division 01 Section "Construction Waste Management and Disposal", whichever is the more restrictive and as follows:
- 1. Comply with safety standards for cleaning. Do not burn waste materials. Do not bury debris or excess materials on Owner's property. Do not discharge volatile, harmful, or dangerous materials into drainage systems. Remove waste materials from Project site and dispose of lawfully.

3.2 REPAIR OF THE WORK

- A. Complete repair and restoration operations before requesting inspection for determination of Substantial Completion.
- B. Repair or remove and replace defective construction. Repairing includes replacing defective parts, refinishing damaged surfaces, touching up with matching materials, and properly adjusting operating equipment. Where damaged or worn items cannot be repaired or restored, provide replacements. Remove and replace operating components that cannot be repaired. Restore damaged construction and permanent facilities used during construction to specified condition.
- 1. Remove and replace chipped, scratched, and broken glass, reflective surfaces, and other damaged transparent materials.
 - 2. Touch up and otherwise repair and restore marred or exposed finishes and surfaces. Replace finishes and surfaces that that already show evidence of repair or restoration.
 - a. Do not paint over "UL" and other required labels and identification, including mechanical and electrical nameplates. Remove paint applied to required labels and identification.
 - 3. Replace parts subject to operating conditions during construction that may impede operation or reduce longevity.
 - 4. Replace all lamps and starters to comply with requirements for new fixtures.
- C. All Warranties remain in effect.

END OF SECTION

SECTION 01 7823

OPERATION AND MAINTENANCE DATA

PART 1 - GENERAL

1.1 SUMMARY

- A. Section includes administrative and procedural requirements for preparing operation and maintenance manuals, including the following:
 - 1. Operation and maintenance documentation directory.
 - 2. Emergency manuals.
 - 3. Operation manuals for systems, subsystems, and equipment.
 - 4. Product maintenance manuals.
 - 5. Systems and equipment maintenance manuals.

1.2 DEFINITIONS

- A. System: An organized collection of parts, equipment, or subsystems united by regular interaction.
- B. Subsystem: A portion of a system with characteristics similar to a system.

1.3 CLOSEOUT SUBMITTALS

- A. Manual Content: Operations and maintenance manual content is specified in individual Specification Sections to be reviewed at the time of Section submittals. Submit reviewed manual content formatted and organized as required by this Section.
- B. Format: Submit operations and maintenance manuals in the following format:
 - 1. PDF electronic file. Assemble each manual into a composite electronically indexed file. Submit on digital media acceptable to Owner.
 - a. Name each indexed document file in composite electronic index with applicable item name. Include a complete electronically linked operation and maintenance directory.
 - b. Enable inserted reviewer comments on draft submittals.
- C. Initial Manual Submittal: Submit draft copy of each manual at least 30 days before commencing demonstration and training. Architect will comment on whether general scope and content of manual are acceptable.
- D. Final Manual Submittal: Submit each manual in final form prior to requesting inspection for Substantial Completion and before commencing demonstration and training. Architect will return copy with comments.
 - 1. Correct or revise each manual to comply with Architect's comments. Submit copies of each corrected manual prior to commencing demonstration and training.

PART 2 - PRODUCTS

2.1 OPERATION AND MAINTENANCE DOCUMENTATION DIRECTORY

- A. Directory: Prepare a single, comprehensive directory of emergency, operation, and maintenance data and materials, listing items and their location to facilitate ready access to desired information. Include a section in the directory for each of the following:
 - 1. List of documents.
 - 2. List of systems.
 - 3. List of equipment.
 - 4. Table of contents.
- B. List of Systems and Subsystems: List systems alphabetically. Include references to operation and maintenance manuals that contain information about each system.
- C. List of Equipment: List equipment for each system, organized alphabetically by system. For pieces of equipment not part of system, list alphabetically in separate list.
- D. Tables of Contents: Include a table of contents for each emergency, operation, and maintenance manual.
- E. Identification: In the documentation directory and in each operation and maintenance manual, identify each system, subsystem, and piece of equipment with same designation used in the Contract Documents. If no designation exists, assign a designation according to ASHRAE Guideline 4, "Preparation of Operating and Maintenance Documentation for Building Systems."

2.2 REQUIREMENTS FOR EMERGENCY, OPERATION, AND MAINTENANCE MANUALS

- A. Organization: Unless otherwise indicated, organize each manual into a separate section for each system and subsystem, and a separate section for each piece of equipment not part of a system. Each manual shall contain the following materials, in the order listed:
 - 1. Title page.
 - 2. Table of contents.
 - 3. Manual contents.
- B. Title Page: Include the following information:
 - 1. Subject matter included in manual.
 - 2. Name and address of Project.
 - 3. Name and address of Owner.
 - 4. Date of submittal.
 - 5. Name and contact information for Contractor.
 - 6. Name and contact information for Architect.
 - 7. Names and contact information for major consultants to the Architect that designed the systems contained in the manuals.
 - 8. Cross-reference to related systems in other operation and maintenance manuals.

- C. Table of Contents: List each product included in manual, identified by product name, indexed to the content of the volume, and cross-referenced to Specification Section number in Project Manual.
 - 1. If operation or maintenance documentation requires more than one volume to accommodate data, include comprehensive table of contents for all volumes in each volume of the set.
- D. Manual Contents: Organize into sets of manageable size. Arrange contents alphabetically by system, subsystem, and equipment. If possible, assemble instructions for subsystems, equipment, and components of one system into a single binder.
- E. Manuals, Electronic Files: Submit manuals in the form of a multiple file composite electronic PDF file for each manual type required.
 - 1. Electronic Files: Use electronic files prepared by manufacturer where available. Where scanning of paper documents is required, configure scanned file for minimum readable file size.
 - 2. File Names and Bookmarks: Enable bookmarking of individual documents based on file names. Name document files to correspond to system, subsystem, and equipment names used in manual directory and table of contents. Group documents for each system and subsystem into individual composite bookmarked files, then create composite manual, so that resulting bookmarks reflect the system, subsystem, and equipment names in a readily navigated file tree. Configure electronic manual to display bookmark panel on opening file.

2.3 EMERGENCY MANUALS

- A. Content: Organize manual into a separate section for each of the following:
 - 1. Type of emergency.
 - 2. Emergency instructions.
 - 3. Emergency procedures.
- B. Type of Emergency: Where applicable for each type of emergency indicated below, include instructions and procedures for each system, subsystem, piece of equipment, and component:
 - 1. Fire.
 - 2. Flood.
 - 3. Gas leak.
 - 4. Water leak.
 - 5. Power failure.
 - 6. Water outage.
 - 7. System, subsystem, or equipment failure.
 - 8. Chemical release or spill.
- C. Emergency Instructions: Describe and explain warnings, trouble indications, error messages, and similar codes and signals. Include responsibilities of Owner's operating personnel for notification of Installer, supplier, and manufacturer to maintain warranties.

D. Emergency Procedures: Include the following, as applicable:

1. Instructions on stopping.
2. Shutdown instructions for each type of emergency.
3. Operating instructions for conditions outside normal operating limits.
4. Required sequences for electric or electronic systems.
5. Special operating instructions and procedures.

2.4 OPERATION MANUALS

A. Content: In addition to requirements in this Section, include operation data required in individual Specification Sections and the following information:

1. System, subsystem, and equipment descriptions. Use designations for systems and equipment indicated on Contract Documents.
2. Performance and design criteria if Contractor has delegated design responsibility.
3. Operating standards.
4. Operating procedures.
5. Operating logs.
6. Wiring diagrams.
7. Control diagrams.
8. Piped system diagrams.
9. Precautions against improper use.
10. License requirements including inspection and renewal dates.

B. Descriptions: Include the following:

1. Product name and model number. Use designations for products indicated on Contract Documents.
2. Manufacturer's name.
3. Equipment identification with serial number of each component.
4. Equipment function.
5. Operating characteristics.
6. Limiting conditions.
7. Performance curves.
8. Engineering data and tests.
9. Complete nomenclature and number of replacement parts.

C. Operating Procedures: Include the following, as applicable:

1. Startup procedures.
2. Equipment or system break-in procedures.
3. Routine and normal operating instructions.
4. Regulation and control procedures.
5. Instructions on stopping.
6. Normal shutdown instructions.
7. Seasonal and weekend operating instructions.
8. Required sequences for electric or electronic systems.
9. Special operating instructions and procedures.

D. Systems and Equipment Controls: Describe the sequence of operation, and diagram controls as installed.

- E. Piped Systems: Diagram piping as installed, and identify color-coding where required for identification.

2.5 PRODUCT MAINTENANCE MANUALS

- A. Content: Organize manual into a separate section for each product, material, and finish. Include source information, product information, maintenance procedures, repair materials and sources, and warranties and bonds, as described below.
- B. Source Information: List each product included in manual, identified by product name and arranged to match manual's table of contents. For each product, list name, address, and telephone number of Installer or supplier and maintenance service agent, and cross-reference Specification Section number and title in Project Manual and drawing or schedule designation or identifier where applicable.
- C. Product Information: Include the following, as applicable:
 - 1. Product name and model number.
 - 2. Manufacturer's name.
 - 3. Color, pattern, and texture.
 - 4. Material and chemical composition.
 - 5. Reordering information for specially manufactured products.
- D. Maintenance Procedures: Include manufacturer's written recommendations and the following:
 - 1. Inspection procedures.
 - 2. Types of cleaning agents to be used and methods of cleaning.
 - 3. List of cleaning agents and methods of cleaning detrimental to product.
 - 4. Schedule for routine cleaning and maintenance.
 - 5. Repair instructions.
- E. Repair Materials and Sources: Include lists of materials and local sources of materials and related services.
- F. Warranties and Bonds: Include copies of warranties and bonds and lists of circumstances and conditions that would affect validity of warranties or bonds.
 - 1. Include procedures to follow and required notifications for warranty claims.

2.6 SYSTEMS AND EQUIPMENT MAINTENANCE MANUALS

- A. Content: For each system, subsystem, and piece of equipment not part of a system, include source information, manufacturers' maintenance documentation, maintenance procedures, maintenance and service schedules, spare parts list and source information, maintenance service contracts, and warranty and bond information, as described below.
- B. Source Information: List each system, subsystem, and piece of equipment included in manual, identified by product name and arranged to match manual's table of contents. For each product, list name, address, and telephone number of Installer or supplier and maintenance service agent, and cross-reference Specification Section number and title in Project Manual and drawing or schedule designation or identifier where applicable.

- C. **Manufacturers' Maintenance Documentation:** Manufacturers' maintenance documentation including the following information for each component part or piece of equipment:
 - 1. Standard maintenance instructions and bulletins.
 - 2. Drawings, diagrams, and instructions required for maintenance, including disassembly and component removal, replacement, and assembly.
 - 3. Identification and nomenclature of parts and components.
 - 4. List of items recommended to be stocked as spare parts.

- D. **Maintenance Procedures:** Include the following information and items that detail essential maintenance procedures:
 - 1. Test and inspection instructions.
 - 2. Troubleshooting guide.
 - 3. Precautions against improper maintenance.
 - 4. Disassembly; component removal, repair, and replacement; and reassembly instructions.
 - 5. Aligning, adjusting, and checking instructions.
 - 6. Demonstration and training video recording, if available.

- E. **Maintenance and Service Schedules:** Include service and lubrication requirements, list of required lubricants for equipment, and separate schedules for preventive and routine maintenance and service with standard time allotment.
 - 1. **Scheduled Maintenance and Service:** Tabulate actions for daily, weekly, monthly, quarterly, semiannual, and annual frequencies.
 - 2. **Maintenance and Service Record:** Include manufacturers' forms for recording maintenance.

- F. **Spare Parts List and Source Information:** Include lists of replacement and repair parts, with parts identified and cross-referenced to manufacturers' maintenance documentation and local sources of maintenance materials and related services.

- G. **Maintenance Service Contracts:** Include copies of maintenance agreements with name and telephone number of service agent.

- H. **Warranties and Bonds:** Include copies of warranties and bonds and lists of circumstances and conditions that would affect validity of warranties or bonds.
 - 1. Include procedures to follow and required notifications for warranty claims.

PART 3 - EXECUTION

3.1 MANUAL PREPARATION

- A. **Emergency Manual:** Assemble a complete set of emergency information indicating procedures for use by emergency personnel and by Owner's operating personnel for types of emergencies indicated.

- B. **Product Maintenance Manual:** Assemble a complete set of maintenance data indicating care and maintenance of each product, material, and finish incorporated into the Work.

- C. Operation and Maintenance Manuals: Assemble a complete set of operation and maintenance data indicating operation and maintenance of each system, subsystem, and piece of equipment not part of a system.
 - 1. Engage a factory-authorized service representative to assemble and prepare information for each system, subsystem, and piece of equipment not part of a system.
 - 2. Prepare a separate manual for each system and subsystem, in the form of an instructional manual for use by Owner's operating personnel.

- D. Manufacturers' Data: Where manuals contain manufacturers' standard printed data, include only sheets pertinent to product or component installed. Mark each sheet to identify each product or component incorporated into the Work. If data include more than one item in a tabular format, identify each item using appropriate references from the Contract Documents. Identify data applicable to the Work and delete references to information not applicable.
 - 1. Prepare supplementary text if manufacturers' standard printed data are not available and where the information is necessary for proper operation and maintenance of equipment or systems.

- E. Drawings: Prepare drawings supplementing manufacturers' printed data to illustrate the relationship of component parts of equipment and systems and to illustrate control sequence and flow diagrams. Coordinate these drawings with information contained in record Drawings to ensure correct illustration of completed installation.
 - 1. Do not use original project record documents as part of operation and maintenance manuals.
 - 2. Comply with requirements of newly prepared record Drawings in Division 01 Section "Project Record Documents."

- F. Comply with Division 01 Section "Closeout Procedures" for schedule for submitting operation and maintenance documentation.

END OF SECTION

18-01.01 WPMHC Expansion
Childers Architect
2019-11-01

OPERATION AND MAINTENANCE DATA

01 7823 - 8

SECTION 01 7839

PROJECT RECORD DOCUMENTS

PART 1 - GENERAL

1.1 SUMMARY

- A. Section includes administrative and procedural requirements for project record documents, including the following:
 - 1. Record Drawings.
 - 2. Record Specifications.
 - 3. Record Product Data.
 - 4. Miscellaneous record submittals.

1.2 SUBMITTALS

- A. Record Drawings: Comply with the following:
 - 1. Number of Copies: Submit copies of record Drawings as follows:
 - a. Initial Submittal:
 - 1) Submit PDF electronic files of scanned record.
 - 2) Architect will indicate whether general scope of changes, additional information recorded, and quality of drafting are acceptable.
 - b. Final Submittal:
 - 1) Submit PDF electronic files of scanned record.
- B. Record Specifications: Submit annotated PDF electronic files of Project's Specifications, including addenda and contract modifications.
- C. Record Product Data: Submit annotated PDF electronic files and directories of each submittal.
- D. Miscellaneous Record Submittals: See other Specification Sections for miscellaneous record-keeping requirements and submittals in connection with various construction activities. Submit annotated PDF electronic files and directories of each submittal.

PART 2 - PRODUCTS

2.1 ELECTRONIC PROJECT MANAGEMENT SOFTWARE

- A. Electronic File of Project Record Documents: Provide Architect with an independent electronic archive of accepted project record documents using electronic project management software as defined in Division 01 Section "Project Management and Coordination", in addition to the printed documents described elsewhere in this Section.

2.2 RECORD DRAWINGS

- A. Record Prints: Maintain one set of marked-up paper copies of the Contract Drawings and Shop Drawings, incorporating new and revised drawings as modifications are issued.
1. Preparation: Mark record prints to show the actual installation where installation varies from that shown originally. Require individual or entity who obtained record data, whether individual or entity is Installer, subcontractor, or similar entity, to provide information for preparation of corresponding marked-up record prints.
 - a. Give particular attention to information on concealed elements that would be difficult to identify or measure and record later.
 - b. Accurately record information in an acceptable drawing technique.
 - c. Record data as soon as possible after obtaining it.
 - d. Record and check the markup before enclosing concealed installations.
 - e. Cross-reference record prints to corresponding archive photographic documentation.
 2. Content: Types of items requiring marking include, but are not limited to, the following:
 - a. Dimensional changes to Drawings.
 - b. Revisions to details shown on Drawings.
 - c. Depths of foundations below first floor.
 - d. Locations and depths of underground utilities.
 - e. Revisions to routing of piping and conduits.
 - f. Revisions to electrical circuitry.
 - g. Actual equipment locations.
 - h. Duct size and routing.
 - i. Locations of concealed internal utilities.
 - j. Changes made by Change Order or Construction Change Directive.
 - k. Changes made following Architect's written orders.
 - l. Details not on the original Contract Drawings.
 - m. Field records for variable and concealed conditions.
 - n. Record information on the Work that is shown only schematically.
 3. Mark the Contract Drawings and Shop Drawings completely and accurately. Use personnel proficient at recording graphic information in production of marked-up record prints.
 4. Mark record sets with erasable, red-colored pencil. Use other colors to distinguish between changes for different categories of the Work at same location.
 5. Mark important additional information that was either shown schematically or omitted from original Drawings.
 6. Note Construction Change Directive numbers, alternate numbers, Change Order numbers, and similar identification, where applicable.

2.3 RECORD SPECIFICATIONS

- A. Preparation: Mark Specifications to indicate the actual product installation where installation varies from that indicated in Specifications, addenda, and contract modifications.
1. Give particular attention to information on concealed products and installations that cannot be readily identified and recorded later.
 2. Mark copy with the proprietary name and model number of products, materials, and equipment furnished, including substitutions and product options selected.

3. Record the name of manufacturer, supplier, Installer, and other information necessary to provide a record of selections made.
4. Note related Change Orders, record Product Data, and record Drawings where applicable.

B. Format: Submit record Specifications as scanned PDF electronic file(s) of marked-up paper copy of Specifications.

2.4 RECORD PRODUCT DATA

A. Preparation: Mark Product Data to indicate the actual product installation where installation varies substantially from that indicated in Product Data submittal.

1. Give particular attention to information on concealed products and installations that cannot be readily identified and recorded later.
2. Include significant changes in the product delivered to Project site and changes in manufacturer's written instructions for installation.
3. Note related Change Orders, record Specifications, and record Drawings where applicable.

B. Format: Submit record Product Data as scanned PDF electronic file(s) of marked-up paper copy of Product Data.

1. Include record Product Data directory organized by Specification Section number and title, electronically linked to each item of record Product Data.

2.5 MISCELLANEOUS RECORD SUBMITTALS

A. Assemble miscellaneous records required by other Specification Sections for miscellaneous record keeping and submittal in connection with actual performance of the Work. Bind or file miscellaneous records and identify each, ready for continued use and reference.

B. Format: Submit miscellaneous record submittals as scanned PDF electronic file(s) of marked-up miscellaneous record submittals.

1. Include miscellaneous record submittals directory organized by Specification Section number and title, electronically linked to each item of miscellaneous record submittals.

PART 3 - EXECUTION

3.1 RECORDING AND MAINTENANCE

A. Recording: Maintain one copy of each submittal during the construction period for project record document purposes. Post changes and revisions to project record documents as they occur; do not wait until end of Project.

B. Maintenance of Record Documents and Samples: Store record documents and Samples in the field office apart from the Contract Documents used for construction. Do not use project record documents for construction purposes. Maintain record documents in good order and in a clean, dry, legible condition, protected from deterioration and loss. Provide access to project record documents for Architect's reference during normal working hours.

END OF SECTION

**18-01.01 WPMHC Expansion
Childers Architect
2019-11-01**

PROJECT RECORD DOCUMENT

01 7839 - 4

SECTION 01 7900

DEMONSTRATION AND TRAINING

PART 1 - GENERAL

1.1 SUMMARY

- A. Section includes administrative and procedural requirements for instructing Owner's personnel, including the following:
 - 1. Demonstration of operation of systems, subsystems, and equipment.
 - 2. Training in operation and maintenance of systems, subsystems, and equipment.

1.2 SUBMITTALS

- A. Instruction Program: Submit outline of instructional program for demonstration and training, including a list of training modules and a schedule of proposed dates, times, length of instruction time, and instructors' names for each training module. Include learning objective and outline for each training module.
 - 1. Indicate proposed training modules using manufacturer-produced demonstration and training video recordings for systems, equipment, and products.
- B. Qualification Data: For instructor.
- C. Attendance Record: For each training module, submit list of participants and length of instruction time.
- D. Evaluations: For each participant and for each training module, submit results and documentation of performance-based test.

1.3 QUALITY ASSURANCE

- A. Instructor Qualifications: A factory-authorized service representative, complying with requirements in Division 01 Section "Quality Requirements," experienced in operation and maintenance procedures and training.
- B. Preinstruction Conference: Conduct conference at Project site to comply with requirements in Division 01 Section "Project Management and Coordination." Review methods and procedures related to demonstration and training including, but not limited to, the following:
 - 1. Inspect and discuss locations and other facilities required for instruction.
 - 2. Review and finalize instruction schedule and verify availability of educational materials, instructors' personnel, audiovisual equipment, and facilities needed to avoid delays.
 - 3. Review required content of instruction.
 - 4. For instruction that must occur outside, review weather and forecasted weather conditions and procedures to follow if conditions are unfavorable.

1.4 COORDINATION

- A. Coordinate instruction schedule with Owner's operations. Adjust schedule as required to minimize disrupting Owner's operations and to ensure availability of Owner's personnel.
- B. Coordinate instructors, including providing notification of dates, times, length of instruction time, and course content.
- C. Coordinate content of training modules with content of approved emergency, operation, and maintenance manuals.

PART 2 - PRODUCTS

2.1 INSTRUCTION PROGRAM

- A. Program Structure: Develop an instruction program that includes individual training modules for each system and for equipment not part of a system, as required by individual Specification Sections.
- B. Training Modules: Develop a learning objective and teaching outline for each module. Include a description of specific skills and knowledge that participant is expected to master. For each module, include instruction for the following as applicable to the system, equipment, or component:
 - 1. Basis of System Design, Operational Requirements, and Criteria: Include the following:
 - a. System, subsystem, and equipment descriptions.
 - b. Performance and design criteria if Contractor is delegated design responsibility.
 - c. Operating standards.
 - d. Regulatory requirements.
 - e. Equipment function.
 - f. Operating characteristics.
 - g. Limiting conditions.
 - h. Performance curves.
 - 2. Documentation: Review the following items in detail:
 - a. Emergency manuals.
 - b. Operations manuals.
 - c. Maintenance manuals.
 - d. Project record documents.
 - e. Identification systems.
 - f. Warranties and bonds.
 - g. Maintenance service agreements and similar continuing commitments.
 - 3. Emergencies: Include the following, as applicable:
 - a. Instructions on meaning of warnings, trouble indications, and error messages.
 - b. Instructions on stopping.
 - c. Shutdown instructions for each type of emergency.
 - d. Operating instructions for conditions outside of normal operating limits.
 - e. Sequences for electric or electronic systems.
 - f. Special operating instructions and procedures.

4. Operations: Include the following, as applicable:
 - a. Startup procedures.
 - b. Equipment or system break-in procedures.
 - c. Routine and normal operating instructions.
 - d. Regulation and control procedures.
 - e. Control sequences.
 - f. Safety procedures.
 - g. Instructions on stopping.
 - h. Normal shutdown instructions.
 - i. Operating procedures for emergencies.
 - j. Operating procedures for system, subsystem, or equipment failure.
 - k. Seasonal and weekend operating instructions.
 - l. Required sequences for electric or electronic systems.
 - m. Special operating instructions and procedures.
5. Adjustments: Include the following:
 - a. Alignments.
 - b. Checking adjustments.
 - c. Noise and vibration adjustments.
 - d. Economy and efficiency adjustments.
6. Troubleshooting: Include the following:
 - a. Diagnostic instructions.
 - b. Test and inspection procedures.
7. Maintenance: Include the following:
 - a. Inspection procedures.
 - b. Types of cleaning agents to be used and methods of cleaning.
 - c. List of cleaning agents and methods of cleaning detrimental to product.
 - d. Procedures for routine cleaning
 - e. Procedures for preventive maintenance.
 - f. Procedures for routine maintenance.
 - g. Instruction on use of special tools.
8. Repairs: Include the following:
 - a. Diagnosis instructions.
 - b. Repair instructions.
 - c. Disassembly; component removal, repair, and replacement; and reassembly instructions.
 - d. Instructions for identifying parts and components.
 - e. Review of spare parts needed for operation and maintenance.

PART 3 - EXECUTION

3.1 PREPARATION

- A. Assemble educational materials necessary for instruction, including documentation and training module. Assemble training modules into a training manual organized in coordination with requirements in Division 01 Section "Operations and Maintenance Data."
- B. Set up instructional equipment at instruction location.

3.2 INSTRUCTION

- A. Engage qualified instructors to instruct Owner's personnel to adjust, operate, and maintain systems, subsystems, and equipment not part of a system.
 - 1. Owner will furnish Contractor with names and positions of participants.
- B. Scheduling: Provide instruction at mutually agreed on times. For equipment that requires seasonal operation, provide similar instruction at start of each season.
 - 1. Schedule training with Owner with at least seven days' advance notice.
- C. Training Location and Reference Material: Conduct training on-site in the completed and fully operational facility using the actual equipment in-place. Conduct training using final operation and maintenance data submittals.
- D. Evaluation: At conclusion of each training module, assess and document each participant's mastery of module by use of a demonstration performance-based test.
- E. Cleanup: Collect used and leftover educational materials and remove from Project site. Remove instructional equipment. Restore systems and equipment to condition existing before initial training use.

END OF SECTION

SECTION 01 9113

GENERAL COMMISSIONING REQUIREMENTS

PART 1 - GENERAL

1.1 RELATED DOCUMENTS

- A. OPR and BoD documentation are included by reference for information only.

1.2 SUMMARY

- A. Section includes general requirements that apply to implementation of commissioning without regard to specific systems, assemblies, or components.
- B. Related Documents:
 - 1. OPR and BoD documentation are included by reference for information only.

1.3 DEFINITIONS

- A. BoD: Basis of Design. A document that records concepts, calculations, decisions, and product selections used to meet the OPR and to satisfy applicable regulatory requirements, standards, and guidelines. The document includes both narrative descriptions and lists of individual items that support the design process.
- B. Commissioning Plan: A document that outlines the organization, schedule, allocation of resources, and documentation requirements of the commissioning process.
- C. CxA: Commissioning Authority.
- D. OPR: Owner's Project Requirements. A document that details the functional requirements of a project and the expectations of how it will be used and operated. These include Project goals, measurable performance criteria, cost considerations, benchmarks, success criteria, and supporting information.
- E. Systems, Subsystems, Equipment, and Components: Where these terms are used together or separately, they shall mean "as-built" systems, subsystems, equipment, and components.

1.4 COMMISSIONING TEAM

- A. Members Appointed by Contractor(s): Individuals, each having the authority to act on behalf of the entity he or she represents, explicitly organized to implement the commissioning process through coordinated action. The commissioning team shall consist of, but not be limited to, representatives of Contractor, including Project superintendent and subcontractors, installers, suppliers, and specialists deemed appropriate by the CxA.
- B. Members Appointed by Owner:
 - 1. CxA: The designated person, company, or entity that plans, schedules, and coordinates the commissioning team to implement the commissioning process. Owner will engage the CxA under a separate contract.

2. Representatives of the facility user and operation and maintenance personnel.
3. Architect and engineering design professionals.

1.5 OWNER'S RESPONSIBILITIES

- A. Provide the OPR documentation to the CxA and Contractor for information and use.
- B. Assign operation and maintenance personnel and schedule them to participate in commissioning team activities.
- C. Provide the BoD documentation, prepared by Architect and approved by Owner, to the CxA and Contractor for use in developing the commissioning plan, systems manual, and operation and maintenance training plan.

1.6 CONTRACTOR'S RESPONSIBILITIES

- A. Contractor shall assign representatives with expertise and authority to act on its behalf and shall schedule them to participate in and perform commissioning process activities including, but not limited to, the following:
 1. Evaluate performance deficiencies identified in test reports and, in collaboration with entity responsible for system and equipment installation, recommend corrective action.
 2. Cooperate with the CxA for resolution of issues recorded in the Issues Log.
 3. Attend commissioning team meetings held as follows:
 - a. Monthly.
 4. Integrate and coordinate commissioning process activities with construction schedule.
 5. Review and accept construction checklists provided by the CxA.
 6. Complete construction checklists as Work is completed and provide to the Commissioning Authority as follows:
 - a. Format:
 - 1) Paper.
 - 2) Electronic.
 - b. Submit:
 - 1) Weekly.
 7. Review and accept commissioning process test procedures provided by the Commissioning Authority.
 8. Complete commissioning process test procedures.

1.7 CxA'S RESPONSIBILITIES

- A. Organize and lead the commissioning team.
- B. Provide commissioning plan.
- C. Convene commissioning team meetings.
- D. Provide Project-specific construction checklists and commissioning process test procedures.

- E. Verify the execution of commissioning process activities using random sampling. The sampling rate may vary from 1 to 100 percent. Verification will include, but is not limited to, equipment submittals, construction checklists, training, operating and maintenance data, tests, and test reports to verify compliance with the OPR. When a random sample does not meet the requirement, the CxA will report the failure in the Issues Log.
- F. Prepare and maintain the Issues Log.
- G. Prepare and maintain completed construction checklist log.
- H. Witness systems, assemblies, equipment, and component startup.
- I. Compile test data, inspection reports, and certificates; include them in the systems manual and commissioning process report.

PART 2 - PRODUCTS (Not Used)

PART 3 - EXECUTION (Not Used)

END OF SECTION

SECTION 02 41 13 - DEMOLITION

1.0 GENERAL

1.1 DESCRIPTION

The section covers the demolition or removal of any existing structures, pavement, curb & gutter, or other improvements that may interfere with the proposed construction and/or designated to be removed.

2.0 EXECUTION

2.1 DEMOLITION

The items identified or conflicting shall be removed before performing final site grading. Demolition shall be performed in a manner to insure no interruption in service to the existing facilities. Contractor shall be responsible for insuring that any temporary services or replacement services have been installed prior to removals. Maintenance of access shall be coordinated with the Owner.

Items to be salvaged or delivered to the Owner shall be removed in such a way to minimize damage and maintain functionality for future use.

All paving shall be removed by full depth sawcuts, unless otherwise noted, to provide a flush surface for new adjoining construction.

2.2 DISPOSAL

Contractor shall be responsible for removal and disposal of all existing equipment, structures, paving, and other materials scheduled for demolition or removal. Contractor shall be responsible for transporting and proper disposal of items scheduled to be removed from site. Disposal shall be in accordance with all Federal, State, and Local regulations that may govern.

Any equipment or items to be retained by the Owner shall be removed and placed at a location designated by the Owner. Disposal of concrete and other debris scheduled to be completely removed shall be the responsibility of the Contractor.

Items that are to be removed and salvaged and/or delivered to Owner shall be either delivered to a storage location on-site or to a location to be determined by the Owner. Salvaged material/items shall be removed, handled, and transported in a manner to minimize damage.

2.3 DEPTH OF DEMOLITION OR REMOVAL

It shall be the responsibility of the Contractor to insure that all items to be demolished or removed are done so to a level below grade in such a manner as to permit the construction of all improvements.

END OF SECTION

SECTION 02 4116

BUILDING DEMOLITION

PART 1 - GENERAL

1.1 SUMMARY

- A. Work required for this section includes demolition and removal of buildings, structures, and site improvements, including capping, sealing or removing of site utilities.

1.2 DEFINITIONS

- A. Remove: Detach items from existing construction and legally dispose of them off-site unless indicated to be removed and salvaged or recycled.
- B. Remove and Salvage: Detach items from existing construction and deliver them to Owner's designated storage area.
- C. Existing to Remain: Existing items of construction that are not to be removed and that are not otherwise indicated to be removed, removed and salvaged, or recycled.

1.3 SUBMITTALS

- A. Landfill Records: Indicate receipt and acceptance of hazardous wastes by a landfill facility licensed to accept hazardous wastes.
- B. Statement of Refrigerant Recovery: Signed by refrigerant recovery technician responsible for recovering refrigerant, stating that all refrigerant that was present was recovered and that recovery was performed according to EPA regulations. Include name and address of technician and date refrigerant was recovered.

1.4 QUALITY ASSURANCE

- A. Demolition Firm Qualifications: An experienced firm that has specialized in demolition work similar in material and extent to that indicated for this Project.
- B. Refrigerant Recovery Technician Qualifications: Certified by EPA-approved certification program.
- C. Professional Engineer Qualifications: Professional engineer legally authorized to practice in jurisdiction where project is located and experienced in providing engineering services of kind indicated for demolitions similar to this project and has a record of successful in-service performance.
- D. Regulatory Requirements: Comply with governing EPA notification regulations before beginning demolition. Comply with hauling and disposal regulations of authorities having jurisdiction.
- E. Standards: Comply with ANSI A10.6 and NFPA 241.
- F. Perform work in conformity with requirements of applicable local ordinances, regulations and codes.

- G. Pre-demolition Conference: Conduct conference at Project site to comply with requirements in Division 1 Section "Project Management and Coordination." Review methods and procedures related to building demolition including, but not limited to, the following:
1. Inspect and discuss condition of construction to be demolished.
 2. Review structural load limitations of existing structures.
 3. Review and finalize building demolition schedule.
 4. Verify availability of personnel, equipment, and facilities needed to avoid delays.
 5. Review and finalize protection requirements.

1.5 PROJECT CONDITIONS

- A. Buildings to be demolished will be vacated and their use discontinued before start of Work.
- B. Owner will occupy another building immediately adjacent to demolition area. Conduct building demolition so Owner's operations will not be disrupted.
1. Provide not less than 72 hours' notice to Owner of activities that will affect Owner's operations.
 2. Maintain access to existing walkways, exits, and other adjacent occupied or used facilities.
 3. Do not close or obstruct walkways, exits, or other occupied or used facilities without written permission from authorities having jurisdiction.
- C. Owner assumes no responsibility for conditions of buildings and structures to be demolished. Conditions existing at time of inspection for bidding purpose will be maintained by Owner as far as practical.
- D. Hazardous Materials: It is not expected that hazardous materials will be encountered in the Work. If materials suspected of containing hazardous materials are encountered, do not disturb; immediately notify Architect and Owner. Hazardous materials will be removed by Owner under a separate contract.
- E. Storage or sale of removed items or materials on-site is not permitted.

PART 2 - PRODUCTS

2.1 MATERIALS

- A. Satisfactory Soils: ASTM D 2487 soil classification groups GW, GP, GM, SW, SP, and SM, or a combination of these group symbols; free of rock or gravel larger than 3 inches in any dimension, debris, waste, frozen materials, vegetation, and other deleterious matter.
- B. Explosives: Use of explosives is not permitted.

PART 3 - EXECUTION

3.1 EXAMINATION

- A. Survey existing conditions and correlate with requirements indicated to determine extent of building demolition required.

- B. Review Project Record Documents of existing construction if available. Owner does not guarantee that existing conditions are the same as those indicated in Project Record Documents.
- C. Inventory and record the condition of items to be removed and salvaged.
- D. When unanticipated elements are encountered, investigate and measure the nature and extent of the element. Promptly submit a written report to Architect.
- E. Engage a professional engineer to perform an engineering survey of condition of building to determine whether removing any element might result in structural deficiency or unplanned collapse of any portion of structure or adjacent structures during building demolition operations.
- F. Verify that hazardous materials do not exist or have been removed before proceeding with building demolition operations.

3.2 PREPARATION

- A. Refrigerant: Remove and store refrigerant according to 40 CFR 82 and regulations of authorities having jurisdiction.
- B. Existing Utilities: Locate, identify, disconnect, and seal or cap off utilities serving buildings and structures to be demolished.
 - 1. Arrange to shut off utilities with utility companies.
 - 2. Provide temporary utilities that bypass buildings and structures to be demolished and that maintain continuity of service to other buildings and structures.
 - 3. Cut off pipe or conduit a minimum of 24 inches below grade. Cap, valve, or plug and seal remaining portion of pipe or conduit after bypassing.
- C. Temporary Shoring: Provide and maintain interior and exterior shoring, bracing, or structural support to preserve stability and prevent unexpected movement or collapse of construction being demolished. Strengthen or add new supports when required during progress of demolition.
- D. Removed and Salvaged Items: Comply with the following:
 - 1. Clean salvaged items of dirt and demolition debris.
 - 2. Pack or crate items after cleaning. Identify contents of containers.
 - 3. Store items in a secure area until delivery to Owner.
 - 4. Transport items to Owner's storage area designated by Owner.
 - 5. Protect items from damage during transport and storage.

3.3 PROTECTION

- A. Existing Utilities: Maintain utility services indicated to remain and protect them against damage during demolition operations.
 - 1. Do not interrupt existing utilities serving adjacent occupied or operating facilities unless authorized in writing by Owner and authorities having jurisdiction.
 - 2. Provide temporary services during interruptions to existing utilities, as acceptable to Owner and to authorities having jurisdiction.
 - a. Provide at least 72 hours' notice to Owner if shutdown of service is required during changeover.

- B. Temporary Protection: Erect temporary protection, such as walks, fences, railings, canopies, and covered passageways, where required by authorities having jurisdiction and as indicated. Comply with requirements in Division 1 Section "Temporary Facilities and Controls."
1. Protect existing site improvements, appurtenances, and landscaping to remain.
 2. Erect a plainly visible fence around drip line of individual trees or around perimeter drip line of groups of trees to remain.
 3. Provide temporary barricades and other protection required to prevent injury to people and damage to adjacent buildings and facilities to remain.
 4. Provide protection to ensure safe passage of people around building demolition area and to and from occupied portions of adjacent buildings and structures.
 5. Protect walls, windows, roofs, and other adjacent exterior construction that are to remain and that are exposed to building demolition operations.

3.4 DEMOLITION

- A. General: Demolish indicated existing buildings, structures and site improvements completely. Use methods required to complete the Work within limitations of governing regulations and as follows:
1. Do not use cutting torches until work area is cleared of flammable materials. Maintain portable fire-suppression devices during flame-cutting operations.
 2. Maintain adequate ventilation when using cutting torches.
 3. Locate building demolition equipment and remove debris and materials so as not to impose excessive loads on supporting walls, floors, or framing.
- B. Engineering Surveys: Perform surveys as the Work progresses to detect hazards that may result from building demolition activities.
- C. Site Access and Temporary Controls: Conduct building demolition and debris-removal operations to ensure minimum interference with roads, streets, walks, walkways, and other adjacent occupied and used facilities.
1. Do not close or obstruct streets, walks, walkways, or other adjacent occupied or used facilities without permission from Owner and authorities having jurisdiction. Provide alternate routes around closed or obstructed traffic ways if required by authorities having jurisdiction.
 2. Use water mist and other suitable methods to limit spread of dust and dirt. Comply with governing environmental-protection regulations. Do not use water when it may damage adjacent construction or create hazardous or objectionable conditions, such as ice, flooding, and pollution.
- D. Proceed with demolition of structural framing members systematically, from higher to lower level. Complete building demolition operations above each floor or tier before disturbing supporting members on the next lower level.
- E. Remove debris from elevated portions by chute, hoist, or other device that will convey debris to grade level in a controlled descent.
1. Remove structural framing members and lower to ground by method suitable to minimize ground impact or dust generation.
- F. Adjacent Construction: Protect adjacent construction that is to remain in place.
1. Concrete: Cut concrete full depth at junctures with construction indicated to remain, using power-driven saw, then remove concrete between saw cuts.

2. Masonry: Cut masonry at junctures with construction indicated to remain, using power-driven saw, then remove masonry between saw cuts.
 3. Concrete Slabs-on-Grade: Saw-cut perimeter of area to be demolished at junctures with construction indicated to remain, then break up and remove.
- G. Below-Grade Construction: Demolish foundation walls and other below-grade construction, as follows:
1. Remove below-grade construction, including foundation walls, to at least 12 inches below grade.
 2. Remove below-grade construction, including foundation walls and footings, to the depths indicated.
 3. Completely remove below-grade construction, including foundation walls and footings.
 4. Break up and remove below-grade concrete slabs, unless indicated to remain.
 5. Break up below-grade concrete slabs into sections no larger than 24 inches square and leave in place.
 6. Remove finish materials, walls, doors, equipment and nonstructural members. Basement walls and floor shall remain in place to support retained earth. Drill or break opening in basement floor for drainage of water.
- H. Existing Utilities: Demolish existing utilities and below-grade utility structures that are within 5 feet outside of footprint indicated for new construction. Abandon utilities outside this area.
1. Fill abandoned utility structures with satisfactory soil materials according to backfill requirements for below grade areas.

3.5 SITE RESTORATION

- A. Below-Grade Areas: Completely fill below-grade areas and voids resulting from building demolition operations with satisfactory soil materials.
1. Prior to placement of fill materials, ensure that areas to be filled are free of standing water, frost, frozen material, trash, and debris.
 2. Place fill materials in horizontal layers not exceeding 8 inches in loose depth.
 3. Compact each layer at optimum moisture content of fill material to density equal to original adjacent ground, but not less than 90 percent density when tested in accordance with ASTM D 1556.
- B. Site Grading: Uniformly rough grade area of demolished construction to a smooth surface, free from irregular surface changes. Provide a smooth transition between adjacent existing grades and new grades.

3.6 REPAIRS

- A. General: Promptly repair damage to adjacent construction caused by building demolition operations.
- B. Where repairs to existing surfaces are required, patch to produce surfaces suitable for new materials.
- C. Restore exposed finishes of patched areas and extend restoration into adjoining construction in a manner that eliminates evidence of patching and refinishing.

3.7 DISPOSAL OF DEMOLISHED MATERIALS

- A. General: Except for items or materials indicated to be reused, salvaged, reinstalled, or otherwise indicated to remain Owner's property, remove demolished materials from Project site and legally dispose of them in an EPA-approved landfill.
 - 1. Do not allow demolished materials to accumulate on-site.
 - 2. Remove and transport debris in a manner that will prevent spillage on adjacent surfaces and areas.
- B. Burning: Do not burn demolished materials on site.
- C. Disposal: Transport demolished materials off Owner's property and legally dispose of them.

3.8 CLEANING

- A. Clean adjacent structures and improvements of dust, dirt, and debris caused by building demolition operations. Return adjacent areas to condition existing before building demolition operations began.

3.9 SCHEDULE OF EQUIPMENT AND MATERIAL RETAINED BY OWNER

- A. Refer to the drawings.

END OF SECTION

SECTION 024119

SELECTIVE DEMOLITION

PART 1 - GENERAL

1.1 SUMMARY

- A. Section includes selective removal and subsequent offsite disposal of portions of existing building indicated on drawings and as required to accommodate new construction.

1.2 DEFINITIONS

- A. Remove: Detach items from existing construction and legally dispose of them off-site, unless indicated to be removed and salvaged or removed and reinstalled.
- B. Remove and Salvage: Detach items from existing construction and deliver them to Owner's designated storage area.
- C. Remove and Reinstall: Detach items from existing construction, prepare them for reuse, and reinstall them where indicated.
- D. Existing to Remain: Existing items of construction that are not to be removed and that are not otherwise indicated to be removed, removed and salvaged, or removed and reinstalled.

1.3 MATERIALS OWNERSHIP

- A. Except for items or materials indicated to be reused, salvaged, reinstalled, or otherwise indicated to remain Owner's property, demolished materials shall become Contractor's property and shall be removed from Project site.

1.4 INFORMATIONAL SUBMITTALS

- A. Qualification Data:
 - 1. For firms and persons specified in "Quality Assurance" to demonstrate their capabilities and experience. Include list of completed projects.
- B. Proposed Dust-Control and Noise-Control Measures: Submit statement or drawing that indicates the measures proposed for use, proposed locations, and proposed time frame for their operation. Identify options if proposed measures are later determined to be inadequate.
- C. Schedule of Selective Demolition Activities: Indicate the following:
 - 1. Detailed sequence of selective demolition and removal work, with starting and ending dates for each activity. Ensure Owner's on-site operations are uninterrupted.
 - 2. Interruption of utility services.
 - 3. Coordination for shutoff, capping, and continuation of utility services.
 - 4. Use of elevator and stairs.
 - 5. Locations of temporary partitions and means of egress.
 - 6. Coordination of Owner's continuing occupancy of portions of existing building and of Owner's partial occupancy of completed Work.

- D. Inventory: After selective demolition is complete, submit a list of items that have been removed and salvaged.
- E. Predemolition Photographs or Videotape: Show existing conditions of adjoining construction and site improvements, including finish surfaces, that might be misconstrued as damage caused by selective demolition operations. Submit before Work begins.
- F. Landfill Records: Indicate receipt and acceptance of hazardous wastes by a landfill facility licensed to accept hazardous wastes.

1.5 QUALITY ASSURANCE

- A. Demolition Firm Qualifications: An experienced firm that has specialized in demolition work similar in material and extent to that indicated for this Project.
- B. Professional Engineer Qualifications: Professional engineer legally authorized to practice in jurisdiction where Project is located and experienced in providing engineering services of kind indicated for demolitions similar to this Project and has a record of successful in-service performance.
- C. Regulatory Requirements: Comply with governing EPA notification regulations before beginning selective demolition. Comply with hauling and disposal regulations of authorities having jurisdiction. Comply with applicable regulations, codes and ordinances.
- D. Standards: Comply with ANSI A10.6 and NFPA 241.
- E. Predemolition Conference: Conduct conference at Project site to comply with requirements in Division 01 Section "Project Management and Coordination." Review methods and procedures related to selective demolition including, but not limited to, the following:
 1. Inspect and discuss condition of construction to be selectively demolished.
 2. Review structural load limitations of existing structure.
 3. Review and finalize selective demolition schedule.
 4. Review requirements of work performed by other trades that rely on substrates exposed by selective demolition operations.

1.6 PROJECT CONDITIONS

- A. Occupied Buildings:
 1. Owner will occupy portions of building immediately adjacent to selective demolition area. Conduct selective demolition so Owner's operations will not be disrupted. Provide not less than 72 hours' notice to Owner of activities that will affect Owner's operations.
 2. Maintain access to existing walkways, corridors, and other adjacent occupied or used facilities.
- B. Owner assumes no responsibility for condition of areas to be selectively demolished. Conditions existing at time of inspection for bidding will be maintained by Owner as far as practical.
- C. Hazardous Materials: If materials suspected of containing hazardous materials are encountered, do not disturb; immediately notify Architect and Owner. Hazardous materials will be removed by Owner under a separate contract.

- D. Storage or sale of removed items or materials on-site will not be permitted.
- E. Utility Service: Maintain existing utilities indicated to remain in service and protect them against damage during selective demolition operations.
 - 1. Maintain fire-protection facilities in service during selective demolition operations.

1.7 WARRANTY

- A. Existing Warranties: Remove, replace, patch, and repair materials and surfaces cut or damaged during selective demolition, by methods and with materials so as not to void existing warranties.

PART 2 - PRODUCTS

2.1 REPAIR MATERIALS

- A. Use repair materials identical to existing materials.
 - 1. If identical materials are unavailable or cannot be used for exposed surfaces, use materials that visually match existing adjacent surfaces to the fullest extent possible.
 - 2. Use materials whose installed performance equals or surpasses that of existing materials.
- B. Comply with material and installation requirements specified in individual Specification Sections.

PART 3 - EXECUTION

3.1 EXAMINATION

- A. Verify that utilities have been disconnected and capped.
- B. Survey existing conditions and correlate with requirements indicated to determine extent of selective demolition required.
- C. Inventory and record the condition of items to be removed and reinstalled and items to be removed and salvaged.
- D. When unanticipated mechanical, electrical, or structural elements that conflict with intended function or design are encountered, investigate and measure the nature and extent of conflict. Promptly submit a written report to Architect.
- E. Engage a professional engineer to survey condition of building to determine whether removing any element might result in structural deficiency or unplanned collapse of any portion of structure or adjacent structures during selective demolition operations.
- F. Perform surveys as the Work progresses to detect hazards resulting from selective demolition activities.

3.2 UTILITY SERVICES

- A. Existing Utilities: Maintain services indicated to remain and protect them against damage during selective demolition operations.
- B. Occupied Buildings: Do not interrupt existing utilities serving occupied or operating facilities unless authorized in writing by Owner and authorities having jurisdiction. Provide temporary services during interruptions to existing utilities, as acceptable to Owner and to authorities having jurisdiction.
 - 1. Provide at least 72 hours' notice to Owner if shutdown of service is required during changeover.
- C. Utility Requirements: Locate, identify, disconnect, and seal or cap off indicated utilities serving areas to be selectively demolished.
 - 1. Arrange with Owner to shut off indicated utilities.
 - 2. If utility services are required to be removed, relocated, or abandoned, before proceeding with selective demolition provide temporary utilities that bypass area of selective demolition and that maintain continuity of service to other parts of building.
 - 3. Cut off pipe or conduit in walls or partitions to be removed. Cap, valve, or plug and seal remaining portion of pipe or conduit after bypassing.

3.3 PREPARATION

- A. Site Access and Temporary Controls: Conduct selective demolition and debris-removal operations to ensure minimum interference with roads, streets, walks, walkways, and other adjacent occupied and used facilities.
 - 1. Do not close or obstruct streets, walks, walkways, or other adjacent occupied or used facilities without permission from Owner and authorities having jurisdiction. Provide alternate routes around closed or obstructed traffic ways if required by governing regulations.
 - 2. Erect temporary protection, such as walks, fences, railings, canopies, and covered passageways, where required by authorities having jurisdiction.
 - 3. Protect existing site improvements, appurtenances, and landscaping to remain.
- B. Temporary Facilities: Provide temporary barricades and other protection required to prevent injury to people and damage to adjacent buildings and facilities to remain.
 - 1. Provide protection to ensure safe passage of people around selective demolition area and to and from occupied portions of building.
 - 2. Provide temporary weather protection, during interval between selective demolition of existing construction on exterior surfaces and new construction, to prevent water leakage and damage to structure and interior areas.
 - 3. Protect walls, ceilings, floors, and other existing finish work that are to remain or that are exposed during selective demolition operations.
 - 4. Cover and protect furnishings, and equipment that have not been removed.
- C. Temporary Enclosures: Provide temporary enclosures for protection of existing building and construction, in progress and completed, from exposure, foul weather, other construction operations, and similar activities. Provide temporary weathertight enclosure for building exterior.

1. Where heating or cooling is needed and permanent enclosure is not complete, provide insulated temporary enclosures. Coordinate enclosure with ventilating and material drying or curing requirements to avoid dangerous conditions and effects.
- D. Temporary Partitions: Erect and maintain dustproof partitions and temporary enclosures to limit dust and dirt migration and to separate areas from fumes and noise.
- E. Temporary Shoring: Provide and maintain shoring, bracing, or structural support to preserve stability and prevent movement, settlement, or collapse of construction to remain, and to prevent unexpected or uncontrolled movement or collapse of construction being demolished. Strengthen or add new supports when required during progress of selective demolition.

3.4 POLLUTION CONTROLS

- A. Dust Control: Use water mist, temporary enclosures, and other suitable methods to limit spread of dust and dirt. Comply with governing environmental-protection regulations.
 1. Do not use water when it may damage existing construction or create hazardous or objectionable conditions, such as ice, flooding, and pollution.
 2. Wet mop floors to eliminate trackable dirt and wipe down walls and doors of demolition enclosure. Vacuum carpeted areas.
- B. Disposal: Remove and transport debris in a manner that will prevent spillage on adjacent surfaces and areas.
 1. Remove debris from elevated portions of building by chute, hoist, or other device that will convey debris to grade level in a controlled descent.
- C. Cleaning: Clean adjacent structures and improvements of dust, dirt, and debris caused by selective demolition operations. Return adjacent areas to condition existing before selective demolition operations began.

3.5 SELECTIVE DEMOLITION

- A. General: Demolish and remove existing construction only to the extent required by new construction and as indicated. Use methods required to complete the Work within limitations of governing regulations and as follows:
 1. Proceed with selective demolition systematically, from higher to lower level. Complete selective demolition operations above each floor or tier before disturbing supporting members on the next lower level.
 2. Neatly cut openings and holes plumb, square, and true to dimensions required. Use cutting methods least likely to damage construction to remain or adjoining construction.
 3. Use hand tools or small power tools designed for sawing or grinding, not hammering and chopping, to minimize disturbance of adjacent surfaces. Temporarily cover openings to remain.
 4. Cut or drill from the exposed or finished side into concealed surfaces to avoid marring existing finished surfaces.
 5. Do not use cutting torches until work area is cleared of flammable materials. At concealed spaces, verify condition and contents before starting flame-cutting operations.
 6. Maintain portable fire-suppression devices during flame-cutting operations.
 7. Maintain adequate ventilation when using cutting torches.
 8. Remove decayed, vermin-infested, or otherwise dangerous or unsuitable materials and promptly dispose of off-site.

9. Remove structural framing members and lower to ground by method suitable to avoid free fall and to prevent ground impact or dust generation.
 10. Locate selective demolition equipment and remove debris and materials so as not to impose excessive loads on supporting walls, floors, or framing.
 11. Dispose of demolished items and materials promptly.
 12. Return elements of construction and surfaces that are to remain to condition existing before selective demolition operations began.
- B. Existing Facilities: Comply with building manager's requirements for using and protecting elevators, stairs, walkways, loading docks, building entries, and other building facilities during selective demolition operations.
- C. Removed and Salvaged Items: Comply with the following:
1. Clean salvaged items.
 2. Pack or crate items after cleaning. Identify contents of containers.
 3. Store items in a secure area until delivery to Owner.
 4. Transport items to Owner's storage area designated by Owner.
 5. Protect items from damage during transport and storage.
- D. Removed and Reinstalled Items: Comply with the following:
1. Clean and repair items to functional condition adequate for intended reuse. Paint equipment to match new equipment.
 2. Pack or crate items after cleaning and repairing. Identify contents of containers.
 3. Protect items from damage during transport and storage.
 4. Reinstall items in locations indicated. Comply with installation requirements for new materials and equipment. Provide connections, supports, and miscellaneous materials necessary to make item functional for use indicated.
- E. Existing Items to Remain: Protect construction indicated to remain against damage and soiling during selective demolition. When permitted by Architect, items may be removed to a suitable, protected storage location during selective demolition and reinstalled in their original locations after selective demolition operations are complete.

3.6 PATCHING AND REPAIRS

- A. General: Promptly repair damage to adjacent construction caused by selective demolition operations.
- B. Repairs: Where repairs to existing surfaces are required, patch to produce surfaces suitable for new materials.
1. Completely fill holes and depressions in existing concrete or masonry that are to remain with an approved masonry patching material applied according to manufacturer's written recommendations.

3.7 DISPOSAL OF DEMOLISHED MATERIALS

- A. General: Promptly dispose of demolished materials. Do not allow demolished materials to accumulate on-site.
- B. Burning: Do not burn demolished materials.

- C. Disposal: Transport demolished materials off Owner's property and legally dispose of them.

3.8 SELECTIVE DEMOLITION SCHEDULE

- A. Refer to the drawings.

END OF SECTION

SECTION 03 1000

CONCRETE FORMING AND ACCESSORIES

PART 1 - GENERAL

1.1 WORK INCLUDED

- A. This section includes formwork for cast-in-place concrete, including water stops, and installation of embedded items.

1.2 RELATED WORK SPECIFIED ELSEWHERE

- A. Concrete Reinforcement - Section 03 2000
- B. Cast-In-Place Concrete - Section 03 3000
- C. Under-Slab Vapor Retarder – Section 07 2600

1.3 QUALITY ASSURANCE

- A. Comply with the American Concrete Institute Standard, ACI 347-04, Recommended Practice for Concrete Formwork.

1.4 REFERENCE STANDARDS

- A. American Society for Testing and Materials (ASTM), latest versions:
 - 1. ASTM D 226 Specification for Asphalt - Saturated Organic Felt used in Roofing and Waterproofing"
 - 2. ASTM D 1751 Specification for Preformed Expansion Joint Filler for Concrete Paving and Structural Construction (Nonextruding and Resilient Bituminous Types)

PART 2 - PRODUCTS

2.1 MATERIALS

- A. Forms for Exposed Finish Concrete: Plywood complying with Voluntary Product Standard PS 1-07 "B-B (Concrete Form) Plywood", Class I, Exterior Grade or better or metal, metal-framed plywood or other acceptable panel-type materials. Plywood shall be mill-oiled and edge-sealed, with each piece bearing legible inspection trademark. Furnish in largest practicable sizes to minimize number of joints. Provide form material with sufficient thickness to withstand pressure of newly placed concrete without bow or deflection.
- B. Forms for Unexposed Finish Concrete: Use plywood, lumber, metal or other acceptable material. Provide lumber dressed on at least 2 edges and one side for tight fit.
- C. Form Coatings: Commercial formulation that will not bond with, stain, or adversely affect concrete surfaces, and will not impair subsequent treatments of concrete surfaces.
- D. Chamfer Strips: ¾ inch by ¾ inch wood, PVC, or rubber.

- E. Preformed Construction Joint: 24-gage steel, galvanized, shaped to form a continuous tongue and groove key.
- F. Preformed Control Joint: Rigid plastic or metal strip with removable top section.
- G. Expansion Joint Material: Asphalt saturated fiberboard, ½ inch thick, meeting the requirements of ASTM D 1751.
- H. Felt: Asphalt-saturated organic felt, weighing 30 pounds per 100 square feet, meeting the requirements of ASTM D 226.
- I. Water stops: Volclay RX manufactured by Colloid Environmental Technologies Co. (CETCO).
- J. Recycled Content: Minimum 5 percent post-consumer content, or minimum 20 percent pre-consumer recycled content at contractor's option.

PART 3 - EXECUTION

3.1 COORDINATION

- A. Coordinate the installation of joint materials and moisture barriers with placement of forms and reinforcing steel. Set screeds accurately. Embedded items shall be accurately aligned and adequately supported. Verify installation of mechanical, plumbing, and electrical items to be embedded in concrete. Correct any unsatisfactory condition before proceeding further.

3.2 PREPARATION

- A. Form Coating: Coat contact surfaces of forms with a form coating compound before reinforcement is placed. Thin form-coating compounds with thinning agent and apply as specified in manufacturer's instructions. Do not allow excess form-coating material to accumulate in forms or to come into contact with concrete surfaces against which fresh concrete will be placed.

3.3 INSTALLATION

- A. Formwork: Formwork shall support vertical and lateral loads that are applied until such loads can be supported by concrete structure. Formwork shall be readily removable without impact, shock or damage to cast-in-place concrete surfaces and adjacent materials. Construct forms to sizes, shapes, lines and dimensions shown. Perform surveys to obtain accurate alignment. Provide for recesses, chamfers, blocking, anchorages, inserts, and other features required in work. Select materials to obtain required finishes. Butt joints solidly and provide backup at joints to prevent leakage of cement paste.
- B. Chamfer Strips: Provide at exposed corners and edges.
- C. Form Ties: Use factory fabricated, adjustable-length, removable or snap-off metal form ties, designed to prevent form deflection and to prevent spalling concrete surfaces upon removal.
- D. Cleaning and Tightening: Thoroughly clean forms and adjacent surfaces to receive concrete. Remove chips, wood, sawdust, dirt, or other debris before concrete is placed. Retighten forms and bracing after concrete placement as required to eliminate mortar leaks and maintain proper alignment.

3.4 INSTALLATION OF EMBEDDED ITEMS

- A. General: Set anchorage devices and other embedded items accurately. Use setting drawings, diagrams, templates and printed instructions provided by supplier. Secure embedded items such that they are not displaced during placement of concrete.
- B. Water stops: Install according to manufacturers printed instructions. Splice water stop sections using square cut butt joints and fuse sections together with indirect heat from preheated splicing iron. Use of direct flame is prohibited.
 - 1. Place water stops in all concrete construction joints in basement walls around the building perimeter that are exposed to soil, weather, or moisture, and in any other construction joints that have the potential to allow water infiltration into the building.

3.5 JOINTS

- A. Construction Joints in Elevated Slabs and Beams: Construction joints in Elevated Slabs, Beams, Grade Beams, and other flexural members shall only be made as shown in the contract drawings or as approved by the Engineer of Record. Joints shall be constructed in accordance with ACI 318 Section 6.4 with provisions made for the transfer of shear and other forces. Reinforcement shall be continuous through these joints unless noted otherwise.
- B. Construction Joints in Walls, Foundations, and Slabs on Grade: Provide keyways at least 1 ½ inches deep in vertical construction joints in walls and construction joints in slabs on grade and foundations. Discontinue every other horizontal bar through slab on grade construction joints unless noted otherwise.
- C. Preformed Construction Joint for Slabs on Grade: Secure with galvanized steel stakes, 1/8 inch thick by 1-1/8 inches wide with ½ inch deep rib and tapered point. Splice adjoining joints with 24 gage steel, galvanized splice plates.
- D. Isolation Joints in Slabs on Grade: Construct isolation joints in interior slabs using 30 lb. felt. Provide isolation joints at points of contact between slabs on grade and vertical surfaces, such as column pedestals, foundation walls, grade beams and elsewhere as indicated. Construct isolation joints on exterior slabs abutting vertical surfaces with ½ inch thick expansion joint material.
- E. Control Joints in Slabs-on-Grade:
 - 1. Preformed Strip: Insert premolded rigid plastic, or metal strip into fresh concrete. Cut groove for strip using 10-foot long straight edge cutting tool. Depths of strip shall be one fourth of slab thickness. Press strip into groove such that top of strip is level with the concrete surface. Pull off removable top section, if any, prior to troweling.
 - 2. Saw Cut: Contractor may saw cut control joints instead of using preformed strips. Saw cut joints shall be 1/8 inch wide. Saw cut depth should equal 1/4 of slab depth. Cut joints after concrete has hardened sufficiently to prevent raveling; usually 4 to 12 hours after slab has been cast and finished. Use diamond or silicone-carbide blades.
- F. Control Joints in Walls: Create weakened planes in cantilevered retaining walls at 25 feet on center. Use preformed strips, placed vertically, full height in each face of wall. Depth of strips shall be one inch.

3.6 REMOVAL OF FORMWORK

- A. General: Prevent excessive deflection, distortion, and damage to concrete when forms are stripped. Provide crush plates or wrecking plates where stripping may damage cast concrete surfaces.
- B. Formwork and supports at sides of concrete shall remain in place for 24 hours after concrete placement. This period represents cumulative number of hours, not necessarily consecutive,

during which the temperature of the air surrounding the concrete is above 50 degrees F. Formwork and shoring which support the weight of concrete shall not be removed until concrete has attained its specified compressive strength.

- C. Ensure safety of the structure. Do not superimpose any load on concrete until forms are removed and concrete is cured.

3.7 RE-USE OF FORMS

- A. General: Clean and repair surfaces of forms to be re-used in work. Split, frayed, delaminated, or otherwise damaged form facing material will not be acceptable for exposed surfaces. Apply new form coating compound as specified for new formwork.

When forms are intended for successive concrete placement, thoroughly clean surfaces and remove fins and latence. Align and secure joints to avoid offsets. Do not use "patched" forms for exposed concrete surfaces.

END OF SECTION

SECTION 03 2000

CONCRETE REINFORCEMENT

PART 1 - GENERAL

1.1 WORK INCLUDED

- A. This section includes fabrication and installation of deformed bar and welded wire fabric reinforcing steel.

1.2 RELATED WORK SPECIFIED ELSEWHERE

- A. Concrete Forming and Accessories - Section 03 1000.
- B. Cast In Place Concrete - Section 03 3000.

1.3 QUALITY ASSURANCE

- A. Reference Standards:
 - 1. American Concrete Institute (ACI), latest versions:
 - a. ACI 301 Specifications for Structural Concrete for Buildings
 - b. ACI 315 Details and Detailing of Concrete Reinforcement
 - c. ACI 318 Building Code Requirements for Structural Concrete
 - 2. American Society for Testing and Materials (ASTM), latest versions:
 - a. ASTM A 82/ Standard Specification for Steel Wire, plain, for Concrete Reinforcement
A82M
 - b. ASTM A 185/ Standard Specification for Steel Welded Wire Reinforcement, Plain, for Concrete
A185M
 - c. ASTM A 615/ Standard Specification for Deformed and Plain Carbon-Steel Bars for Concrete
A 615M Reinforcement
 - 3. Concrete Reinforcing Steel Institute (CRSI). Design Handbook, latest version

1.4 SUBMITTALS

- A. Shop Drawings: Submit shop drawings for reinforcing steel. Comply with ACI 315 requirements showing layout, bar schedules, stirrup spacing, diagrams of bent bars, and arrangement of reinforcing steel. Shop Drawings shall not be made by reproduction of the Contract Drawings.

PART 2 - PRODUCTS

2.1 MATERIALS

- A. Reinforcing Bars: ASTM A 615, Grade 60. Stirrups and ties may be Grade 40.

- B. Welded Wire Fabric: ASTM A 185, flat sheets.
- C. Steel Wire: ASTM A 82, 16 gage.
- D. Supports for Reinforcing Steel: Wire bar type and precast concrete block type meeting the requirements of CRSI Manual of Standard Practice.

2.2 FABRICATION

- A. Fabricate reinforcing steel in accordance with fabricating tolerances in ACI 315.
- B. Do not fabricate reinforcing steel until shop drawings are approved.

PART 3 - EXECUTION

3.1 PLACING BAR SUPPORTS

- A. General: Provide bar supports meeting the requirements of CRSI Specification for Placing Bar Supports.
- B. Slabs-on-grade: Use supports with sand plates or precast concrete blocks or horizontal runners where base material will not support chair legs.

3.2 PLACING REINFORCING STEEL

- A. General: Comply with CRSI Code of Standard Practice for "Placing Reinforcing Bars".
- B. Clean reinforcing steel of loose rust and mill scale, earth, ice, and other materials, which reduce or destroy bond with concrete.
- C. Accurately position, support and secure reinforcing steel against displacement by formwork, construction, or concrete placement operations. Place reinforcing steel to obtain minimum coverages. Arrange, space and securely tie bars and bar supports to hold reinforcing steel in position during concrete placement operations. Set wire ties so ends are directed into concrete, not toward exposed concrete surfaces.

Concrete Cover:

Concrete cast against and permanently exposed to earth 3 inches

Concrete exposed to earth or weather:

Bars larger than No. 5

2 inches

Bars No. 5 or smaller.

1 ½ inches

- D. Rebar Splices: Locate at points of minimum stress or as shown on contract drawings. Unless noted otherwise, provide lap splices 30 bar diameters (18 inches minimum) in length.
- E. Welded Wire Fabric Splices: Lap one complete wire spacing.
- F. Corner Reinforcing: Provide corner bars of same size and spacing as horizontal reinforcing steel. Lap with horizontal reinforcing 30 bar diameters or 18 inches minimum length.
- G. Reinforcing at Construction/Control Joints: Continue reinforcing steel through construction joints unless noted otherwise. Discontinue reinforcing steel 2 inches from preformed construction joints in slabs-on-grade. Cut alternate longitudinal bars at weakened plane control joints in walls.

END OF SECTION

SECTION 03 3000

CAST IN PLACE CONCRETE

PART 1 - GENERAL

1.1 WORK INCLUDED

- A. This section covers cast-in-place concrete including finishing, surface repair and curing.

1.2 RELATED WORK SPECIFIED ELSEWHERE

- A. Concrete Forming and Accessories - Section 03 1000
- B. Concrete Reinforcement - Section 03 2000
- C. Under Slab Vapor Retarder – Section 07 2600

1.3 QUALITY ASSURANCE

- A. Reference Standards: Meet the requirements of the following codes, specifications and standards.

- 1. American Concrete Institute (ACI) Publications, latest versions;

- a. ACI 301 Specifications for Structural Concrete for Buildings
- b. ACI 306.1 Standard Specification for Cold Weather Concreting
- c. ACI 318 Building Code Requirements for Structural Concrete.

- 2. ASTM International (ASTM), latest version:

- a. ASTM C 31/ C31M Standard Practice for Making and Curing Concrete Test Specimens in the Field
- b. ASTM C 33/C33M Standard Specification for Concrete Aggregates
- c. ASTM C 39/C39M Standard Test Method for Compressive Strength of Cylindrical Concrete Specimens
- d. ASTM C 94/C 94M Standard Specification for Ready-Mixed Concrete
- e. ASTM C 131 Standard Test Method for Resistance to Degradation of Small-Size Coarse Aggregate by Abrasion and Impact in the Los Angeles Machine
- f. ASTM C 136 Standard Test Method for Sieve Analysis of Fine and Coarse Aggregates
- g. ASTM C 143/C 143M Standard Test Method for Slump of Hydraulic Cement Concrete
- h. ASTM C 150/C150M Standard Specification for Portland Cement
- i. ASTM C 171 Standard Specification for Sheet Materials for Curing Concrete
- j. ASTM C 172/C172M Standard Practice for Sampling Freshly Mixed Concrete
- k. ASTM C 173/ C 173M Standard Test Method for Air Content of Freshly Mixed Concrete by the Volumetric Method
- l. ASTM C 231/ C231M Standard Test Method for Air Content of Freshly Mixed Concrete by the Pressure Method

- m. ASTM C 260/C260M Standard Specification for Air Entraining Admixtures for Concrete
- n. ASTM C 309 Standard Specification for Liquid Membrane-Forming Compounds for Curing Concrete
- o. ASTM C 330/330M Standard Specification for Lightweight Aggregates for Structural Concrete
- p. ASTM C 494/ C 494M Standard Specification for Chemical Admixtures for Concrete
- q. ASTM C 567 Standard Test Method for Determining Density of Structural Lightweight Concrete
- r. ASTM C 618 Standard Specification for Coal Fly Ash and Raw or Calcined Natural Pozzolan for Use in Concrete
- s. ASTM D 4318 Standard Test Method for Liquid Limit, Plastic Limit, and Plasticity Index of Soils

B. Environmental Requirements: Manufacturer and Contractor shall conform to Federal, State, and Local V.O.C. (Volatile Organic Compound) Regulations in area where Project is located. Notify A/E in writing if variations to Specifications herein are required.

- 1. V.O.C. content shall be a maximum 250 (55) gm/liter, unless more stringent codes or laws apply.

1.4 SUBMITTALS

A. Product Data: Submit manufacturer's product data with application and installation instructions for proprietary materials and admixtures.

B. Concrete Mix Design:

- 1. Submit mix design in accordance with ACI-301, Section 4.
- 2. Submit with mix design results of laboratory tests performed within previous 12 months indicating aggregates from the proposed source comply with the requirements of ASTM C 33 or C 330 as applicable.
- 3. Submit the proposed area of use for each mix design submitted (footings, stemwalls, slabs, walls, columns, etc.).

C. Granular Base Course: Submit gradation, plasticity index, and wear information.

D. Test Reports: Submit copies of test reports for concrete compressive strength, air content, temperature and slump. Submit copies of granular base course test reports.

1.5 QUALITY ASSURANCE

A. Manufacturer Qualifications: A firm experienced in manufacturing ready-mixed concrete products and that complies with ASTM C 94/C 94M requirements for production facilities and equipment.

Manufacturer certified according to NRMCA's "Certification of Ready Mixed Concrete Production Facilities."

B. Environmental Requirements: Manufacturer and Contractor shall conform to Federal, State, and Local V.O.C. (Volatile Organic Compound) Regulations in area where Project is located. Notify A/E in writing if variations to Specifications herein are required.

- 1. V.O.C. content shall be a maximum 250 (55) gm/liter, unless more stringent codes or laws apply.

PART 2 - PRODUCTS

2.2 MATERIALS

- A. Portland Cement: ASTM C 150, Type I or II, low alkali. Use one brand of cement throughout project.
- B. Normal Weight Aggregates: ASTM C 33. Provide aggregates from a single source for exposed concrete.
- C. Water: Potable.
- D. Air-Entraining Admixture: ASTM C 260.
- E. Water Reducing Admixture: ASTM C 494.
- F. Fly-Ash: ASTM C 618, Class C.
- G. Moisture-Retaining Cover: Provide waterproof paper, polyethylene film, or polyethylene-coated burlap meeting the requirements of ASTM C 171.
- H. Liquid Membrane-Forming Curing Compound: Liquid type membrane-forming curing compound meeting the requirements of ASTM C 309; Type 1-D with fugitive dye for interior concrete and foundations; Type 2, white pigmented, for exposed exterior concrete except exposed exterior Architectural concrete, use Type 1-D.

Curing compound shall NOT be used on interior slabs, except exposed integrally colored concrete slabs. Curing compound to be used on integrally colored concrete slabs shall be approved by the manufacturer of the color.

- I. Vapor Retarder shall comply with Section 07 26 00 of these Specifications.
- J. Granular base shall meet the following grading requirements when tested in accordance with ASTM C 136.

Granular base shall meet the gradation and material properties requirements as listed in the General Structural Notes.

The plasticity Index shall be no greater than 3 when tested in accordance with ASTM D 4318. The coarse aggregate shall have a percent wear of 50 or less when tested in accordance with ASTM C 131.

2.3 PROPORTIONING AND DESIGN OF MIXES

- A. Prepare design mixes for each type and strength of concrete by either laboratory trial mixture or field experience methods as specified in ACI 301, Section 4. If trial mixture method is used, employ an independent testing facility, acceptable to Architect, for preparing and reporting proposed mix designs.
- B. Submit written reports to Architect, or Engineer, of each proposed mix for each class of concrete at least 15 days prior to start of work. Do not begin concrete production until mixes have been approved.
- C. Refer to the General Structural Notes for concrete strengths.
- D. Slabs-on-ground or on vapor retarder shall have a water/total cementitious ratio not to exceed 0.45.

- E. Use 20% fly ash of cementitious weight in all concrete.
- F. Admixtures
 - 1. Use water reducing admixture conforming to ASTM C 494, Type A, in all concrete unless approved otherwise by the Structural Engineer.
 - 2. All other admixtures shall have the written approval of the Architect or Structural Engineer.
 - 3. Calcium chloride is not permitted.
 - 4. All admixtures, except high range water reducers, shall be added to the concrete at the batch plant.

PART 3 - EXECUTION

3.1 COORDINATION

- A. Coordinate the installation of joint materials and moisture barriers with placement of forms and reinforcing steel. Set screeds accurately. Embedded items shall be accurately aligned and adequately supported. Verify installation of mechanical, plumbing, and electrical items to be embedded in concrete. Correct any unsatisfactory condition before proceeding further.

3.2 PREPARATION

- A. Before placing concrete, clean and roughen surface of previously placed concrete. Clean reinforcing steel. Remove debris, providing clean-outs at bottom of forms when necessary. Moisten surfaces to receive concrete unless otherwise prepared. Remove excess water before placing concrete.
- B. The subgrade for floor slab support shall be graded uniformly flat using a laser device immediately prior to concrete placement.

3.3 CONCRETE PLACEMENT

- A. General: Comply with ACI 301.
- B. Place concrete continuously in layers not deeper than 24 inches. Concrete shall not be placed against concrete which has hardened sufficiently to cause the formation of seams or planes of weakness. If a section cannot be placed continuously, provide construction joints. Deposit concrete as nearly as practicable to its final location to avoid segregation. Do not use vibrators to transport concrete.
- C. Maintain reinforcing in proper position during concrete placement operations.
- D. Consolidate concrete, immediately after placing, by mechanical vibrating equipment supplemented by hand-spading, rodding or tamping. Use equipment and procedures for consolidation of concrete in accordance with ACI recommended practices.
- E. Bring slab surfaces to correct level with straightedge and strikeoff. Use bull floats or darbies to smooth surface. Do not disturb slab surfaces prior to beginning finishing operations.
- F. Cold Weather Concreting: Protect concrete work from physical damage or reduced strength caused by frost, freezing or low temperatures. Comply with ACI 306.1.
- G. Hot Weather Concreting: When hot weather conditions exist that would impair quality and strength of concrete, reduce delivery time of ready mix concrete, lower the temperature of

materials, or add retarder to ensure that the concrete is plastic. Retempering with water is not allowed. Comply with ACI 305R.

- H. Performance Requirements: Provide polished flooring that has been selected, manufactured and installed to achieve:
- I. ASTM C779 Method A – Standard Test Method for abrasion resistance of horizontal concrete services.

3.4 FINISH OF FORMED SURFACES

- A. Rough Form Finish: Provide where formed concrete surfaces are not exposed to view. Tie holes and surface imperfections shall be repaired and patched and fins and other projections exceeding ¼ inch in height rubbed down or chipped off.

3.5 FINISH OF HORIZONTAL SURFACES

- A. At tops of foundation walls and grade beams finish with a texture matching adjacent formed surfaces unless otherwise indicated.

3.6 SLAB FINISHES

- A. Float Finish: Begin floating when surface water has disappeared and when concrete has stiffened sufficiently to permit operation of power-driven or hand floats. Consolidate surface with power-driven floats, or by hand-floating if area is small or inaccessible to power units. Check and level surface plane to a tolerance not exceeding ¼ inch in 10 feet when tested with a 10 foot straightedge.
- B. Scratch Finish: Apply scratch finish to slab surfaces that are to receive floor topping. Roughen surface before final set, using stiff brushes, or brooms.
- C. Trowel Finish: Apply trowel finish to all slab surfaces unless noted otherwise. After floating, begin first trowel finish using a power-driven or hand trowel. Finish concrete surface by a final hand-trowel operation, free of trowel marks, and uniform in texture and appearance. The final surface finish for slabs-on-grade shall have a minimum FF = 25 and a minimum FL = 20 per ACI requirements. For polished concrete FF=50(Overall), FF=35 (Local) and a minimum FL=20 (Local)]. The final surface finish for elevated slabs shall have a minimum FF = 25. Verify with Architectural requirements.
- D. Polished concrete, if required, see architectural finish schedule:
 - a. Hard-Steel Troweled (3 passes) Concrete: No burn marks. Finish to ACI 302.1R, Class 5 floor.
 - b. When placing edges use a 3 foot metal or wooden 2x4 screed and run parallel with a form or edge after initial screed and before floating.
- E. Broom Finish: Apply on exterior slabs, ramps, steps, and sidewalks. Immediately after concrete has received a float finish, draw a broom or burlap belt across the surface to give a coarse transverse scored texture.

3.7 CONCRETE CURING AND PROTECTION

- A. General: Protect freshly placed concrete from premature drying and excessive cold or hot temperatures. Start initial curing as soon as free water has disappeared from concrete surface after placing and finishing. Continue curing for at least 7 days.
- B. Moisture-retaining Cover curing: All interior concrete slabs, except exposed integrally colored concrete slabs, are to be cured with a moisture retaining cover for the first 7 days. After that time, the cover shall be removed and the slab should be allowed to dry. Cover concrete

surfaces with moisture-retaining cover for curing concrete, placed in widest practicable width with sides and ends lapped at least 3 inches and sealed. Repair any holes or tears in cover during curing period.

- C. Curing compound: At contractor's option, exterior concrete slabs may be cured using curing compound. All vertical concrete (walls, beams, etc...) shall be cured using curing compound – apply compound to the vertical surface as soon as the forms are removed. Apply curing compound uniformly in accordance with the manufacturer's printed instructions. Curing compound shall NOT be used on interior slabs, except exposed integrally colored concrete slabs.
- D. Exposed integrally colored concrete slabs: Use curing compound recommended by the concrete supplier. Apply with and airless sprayer.

3.8 CONCRETE SURFACE REPAIRS

- A. Patching Surface Imperfections: Remove loose material and patch surface imperfections and holes left by tie rods with cement mortar. Surface imperfections include honeycomb, excessive air voids, sand streaking and cracks.

3.9 FOR EXPOSED-TO-VIEW SURFACES

- A. Blend white portland cement and standard portland cement so that, when dry, patching mortar will match color surrounding. Provide test areas at inconspicuous location to verify mixture and color match before proceeding with patching. Compact mortar in place and strike-off slightly higher than surrounding surface.

3.10 FIELD QUALITY CONTROL

- A. The Owner shall employ the services of a qualified testing laboratory to perform tests and submit test reports.
- B. Sampling Fresh Concrete: ASTM C 172.
- C. Slump: ASTM C 143; one test for each set of compressive strength test specimens.
- D. Air Content: ASTM C 173 or C 231 for each set of compressive strength test specimens.
- E. Concrete Temperature: Test hourly when air temperature is 40 degrees F. and below, when 80 degrees F and above; and when compression test specimens are made.
- F. Compression Test Specimen: ASTM C 31, one set of 4 standard cylinders for each compressive strength test, unless otherwise directed. Mold and store cylinders for laboratory cured test specimens except when field cure test specimens are required. Mold one set of standard cylinders for volume of concrete specified below or fraction thereof.
 - 1. Slabs on Grade or Metal Deck 30 cubic yards
 - 2. Footings and stem walls 50 cubic yards
 - 3. All other locations (unless noted otherwise) 30 cubic yards
- G. Compressive Strength Tests: ASTM C 39; test 1 specimen at 7 days, 2 specimens at 28 days, and retain one specimen in reserve for later testing. Additional Tests: The testing laboratory will make additional tests of in-place concrete when test results indicate specified concrete strengths and other characteristics have not been attained in the structure as directed by the Architect. The testing laboratory may conduct tests to determine adequacy of concrete by cored cylinders complying with ASTM C 42 or by other methods as directed by the Architect or

Engineer. The Owner shall pay for such tests conducted, and any other additional testing as may be required, when unacceptable concrete is verified.

- H. Granular Base Course: ASTM C 136 and ASTM D 4318 for every 500 square yards of building slab area.

END OF SECTION

SECTION 03 4000

PRECAST CONCRETE

PART 1 - GENERAL

1.1 WORK INCLUDED

- A. This section covers the design, fabrication, transportation and erection of precast concrete members. These include columns, beams, girders, purlins, wall panels, grout packing, connection and supporting devices.

1.2 RELATED WORK SPECIFIED ELSEWHERE

- A. Concrete Reinforcement - Section 03 2000

1.3 QUALITY ASSURANCE

- A. Reference Standards, latest versions.

1. American Concrete Institute (ACI) Standards

- | | | |
|----|---------|--|
| a. | ACI 301 | Specifications for Structural Concrete for Buildings |
| b. | ACI 315 | Details and Detailing of Concrete Reinforcement |
| c. | ACI 318 | Building Code Requirements for Structural Concrete |

2. ASTM International (ASTM)

- | | | |
|----|-----------------------|--|
| a. | ASTM A 82/
A82M | Standard Specification for Steel Wire, Plain,
for Concrete Reinforcement |
| b. | ASTM A 615/
A615M | Standard Specification for Deformed and
Plain Carbon-Steel Bars for Concrete
Reinforcement |
| c. | ASTM C 31/
C 31M | Standard Practice for Making and Curing
Concrete Test -Specimens in the Field |
| d. | ASTM C 33/
C 33M | Standard Specification for Concrete
Aggregates |
| e. | ASTM C 39/
C 39M | Standard Test Method for Compressive
Strength of Cylindrical Concrete Specimens |
| f. | ASTM C 94/
C 94M | Standard Specification for Ready-Mix
Concrete |
| g. | ASTM C 150/
C150M | Standard Specification for Portland Cement |
| h. | ASTM C 260/
C260M | Standard Specification for Air-Entraining
Admixtures for Concrete |
| i. | ASTM C 1107/
C1107 | Specification for Packaged Dry, Hydraulic-
Cement Grout (Non-shrink) |

3. Concrete Reinforcing Steel Institute (CRSI)

Design Handbook- latest Edition

4. Prestressed Concrete Institute (PCI)

PCI MNL 116 Manual for Quality for Plants and Production of Precast Concrete Products.

- B. Fabricator Qualifications

1. Firms shall have 2 years successful experience in the fabrication of precast units similar to the units required for this project. Fabricator must have sufficient production capacity to produce the required units without causing delay of the work.
2. Produce precast units at fabricating plant engaged primarily in manufacturing similar units. Precast Concrete shall be manufactured by a PCI certified plant or at a plant preapproved by the Structural Engineer.

1.4 SUBMITTALS

A. Shop Drawings

1. Content
 - a. Dimensions, reveal locations, and finishes.
 - b. Reinforcing for in-place condition plus any additional reinforcing required for lifting.
 - c. Locations of penetrations through members.
 - d. Locations of embedded plates and lifting devices as well as embeds required for other construction. Furnish templates if required for accurate placement.
 - e. Identification: Identify each precast unit corresponding to sequence and procedure of installation.

PART 2 - PRODUCTS

2.1 MATERIALS

- A. Portland Cement
 1. Conform to ASTM C 150, (Type I/II)
 2. Use same brand, type and source of supply throughout.
- B. Aggregates: Conform to ASTM C 33.
- C. Air Entraining Agent: Conform to ASTM C 260.
- D. Water Reducing Admixture: Conform to ASTM C 494.
- E. Water: Potable.
- F. Reinforcing Steel: Conform to ASTM A 615, Grade 60.
- G. Non-Metallic, Non-Shrink Grout: Meets the requirements of ASTM C 1107.
- H. Admixtures: Admixtures containing chlorides are prohibited.

2.2 CONCRETE MIX

- A. Concrete mixes shall be designed in accordance with ACI 301, Chapter 3, to have the following properties:
- B. Compressive strength, f_c equal to 5000 psi minimum at 28 days as determined by cylinders made in accordance with ASTM C 31 and tested in accordance with ASTM C 39.

2.3 READY-MIXED CONCRETE

- A. Ready-mixed concrete shall conform to the requirements of this section and to ASTM 94.

PART 3 - EXECUTION

3.1 FABRICATION

- A. Provide forms and, where required, form facing materials of metal, fiberglass, wood or other acceptable material that is non-reactive with concrete and will produce required finish surfaces.
- B. Accurately construct forms, mortar-tight, of sufficient strength to withstand pressures due to concrete placing operations, and temperature changes. Maintain formwork to provide completed precast concrete units of shapes, lines, and dimensions indicated, within fabrication tolerances specified in PCI MNL 116.
- C. All reinforcing steel shall be free of mud, grease, or other coatings which may adversely affect or reduce bond to concrete.
- D. Ensure exposed-to-view finish surfaces of units are uniform in color and appearance.

3.2 ERECTION

- A. Fit and align concrete units straight, plumb, level, and square.
- B. Brace all units until all supporting connections have been made.
- C. Grout spaces at elements as shown on the drawings using non-shrink grout. Grout shall be placed to completely fill the designated spaces with no voids.
- D. Provide bearing pads under units as indicated on the drawings.

3.3 CUTTING OPENINGS

- A. Small openings (less than 6" in any dimension) may be cored in precast wall panels as required for work of other trades. Location of openings shall be approved by Architect or Engineer to avoid areas of high stresses.

3.4 REPAIR

- A. Members damaged during or after erection shall be repaired, providing such damage has not impaired structural adequacy or appearance. All repairs shall be made by the Contractor at his cost in a manner and with results satisfactory to the Architect.

3.5 DEFECTIVE MEMBERS

- A. Members which are defective, or which are damaged during transportation or erection to an extent that repairs satisfactory to the Architect are not, in his opinion, possible, shall be removed and replaced by the Contractor at his cost.

END OF SECTION 03 4000

SECTION 04 2200

REINFORCED UNIT MASONRY

PART 1 - GENERAL

1.1 WORK INCLUDED

- A. This section includes the construction of reinforced hollow core unit masonry, masonry veneer and special shapes. It includes all split face units and smooth face units, as well as masonry mortar and grout.

1.2 RELATED WORK SPECIFIED ELSEWHERE

- A. Concrete Reinforcement - Section 03 2000
- B. Division 07 Section "Water Repellents" for water repellents applied to unit masonry assemblies.
- C. Division 07 Section "Sheet Metal Flashing and Trim" for exposed sheet metal flashing.
- D. Division 07 Section "Fire stopping" for fire stopping at tops of masonry walls and at openings in masonry walls.
- E. Division 08 Section "Louvers and Vents" for wall vents (brick vents).
- F. Products furnished, but not installed, under this Section include the following:
 - 1. Anchor sections of adjustable masonry anchors for connecting to structural frame, installed under Division 05 Section "Structural Steel" and Division 13 Section "Metal Building Systems".
- G. Products installed, but not furnished, under this Section include the following:
 - 1. Cast-stone trim, furnished under Division 04 Section "Cast Stone".
 - 2. Steel lintels for unit masonry, furnished under Division 05 Section "Metal Fabrications".
 - 3. Manufactured reglets in masonry joints for metal flashing, furnished under Division 07 Section "Sheet Metal Fabrications".
 - 4. Hollow-metal frames in unit masonry openings, furnished under Division 08 Section "Steel Doors and Frames".

1.3 QUALITY ASSURANCE

- A. Reference Standards
 - 1. ASTM International (ASTM), latest versions:
 - a. ASTM A 615/ A615M Standard Specification for Deformed and Plain Carbon-Steel Bars for Concrete Reinforcement
 - b. ASTM C 90 Standard Specification for Load bearing Concrete Masonry Units
 - c. ASTM C 780 Standard Test Methods for Preconstruction and Construction Evaluation of Mortars for Plain and reinforced Masonry
 - d. ASTM C 270 Standard Specification for Mortar for Unit Masonry
 - e. ASTM C 476 Standard Specification for Grout for Masonry
 - f. ASTM C 1019 Standard Test Method for Sampling and Testing Grout

2. American Concrete Institute (ACI), latest version:
 - a. ACI 530.1 Specification for Masonry Structures

1.4 SUBMITTALS

- A. Product Data: Submit sample of exposed masonry unit of each color and texture to be used to complete the work. Submit copies of test reports performed within last 12 months for representative specimens to be used in accordance with ASTM C 140 for strength, absorption and moisture content, and ASTM C 426 for drying shrinkage.
- B. Test Reports: Submit copies of test reports for masonry units, mortar and grout.

1.5 PRODUCT DELIVERY, STORAGE, AND HANDLING

- A. Store masonry units above ground on level platforms, which allows air circulation under stacked units.
- B. Cover and protect against wetting prior to use.
- C. Handle units on pallets or flat bed barrows.
- D. Store cementitious ingredients in weather-tight enclosures.
- E. Waste Management and Disposal: As specified in Division 01 Section "Construction Waste Management" and as follows:
 1. Separate and recycle waste materials in accordance with the Waste Management Plan and to the maximum extent economically feasible.
 - a. Fold up metal banding; flatten and place in designated area for recycling.
 - b. Collect wood packing shims and pallets; place in designated area.
 2. Recycling: Undamaged, excess masonry materials are Contractor's property and shall be removed from the Project site for his use.
 3. Disposal as Fill Material: Dispose of clean masonry waste, including broken masonry units, waste mortar, and excess or soil contaminated sand, by crushing and mixing with fill material as fill is placed.
 - a. Crush masonry waste to less than 2 inches in greatest dimension.
 - b. Mix masonry waste with at least 2 parts specified fill material for each part masonry waste. Fill material is specified in Division 31 Section "Earth Moving"
 - c. Do not dispose of masonry waste as fill within 18 inches of finished grade.
 4. Excess Masonry Waste: Remove excess, clean masonry waste that cannot be used as fill, as described above, and other masonry waste and legally dispose of off Owner's property.

PART 2 - PRODUCTS

2.1 MATERIALS

- A. Hollow Core Units: ASTM C90.

- B. Burnished or Sandblasted: Coordinate with architectural finish schedule.
- C. Aggregate: Scoria, natural color at exposed block.
- D. Aggregate: Natural color at concealed block.
- E. Mortar: ASTM C 270 "Standard Specification for Mortar for Unit Masonry," Type S, $f'c = 1800\text{psi}$.
- F. Grout: ASTM C 476 "Standard Specification for Grout for Masonry."
- G. Cell Reinforcing: ASTM A 615 "Standard Specification for Deformed and Plain Billet-Steel Bars for Concrete Reinforcement," Grade 60. Comply with Section 03 2000.
- H. Bond Beam and Lintel Reinforcing: ASTM A 615, Grade 60. Comply with Section 03 2000.
- I. Joint Reinforcing: Hot Dipped Galvanized, Standard Ladder Type 9 Gage Wire Dur-O-Wal or approved equal.
- J. Control Joint Material: Rubber, neoprene or PVC joint material for use with standard sash block by Dur-O-Wal or approved equal.
- K. Vertical Bar Positioner: Steel by Dur-O-Wal or approved equal.
- L. Mortar Plasticizer: Easy Spread by American Colloid Company or approved equal.

PART 3 - EXECUTION

3.1 GENERAL

- A. Provide jamb, header, lintel, bond beam, etc. units as required to complete the work. Lay only dry and unfrozen masonry units.
- B. All exposed masonry shall be scoria aggregate, split face, scored finish unless noted otherwise on the drawings. Masonry not exposed to view may be smooth finished.
- C. Discard any broken, chipped, or discolored masonry units.
- D. Use masonry saws to cut and fit masonry units.
- E. Lay units in running bond pattern with vertical joints located at center of masonry units in alternate course below.
- F. Set units plumb, true to line and with level courses accurately spaced.
- G. Adjust masonry unit to final position while mortar is soft and plastic.
- H. Anchors, flashing accessories and similar devices shall be built in as masonry progresses.

3.2 MORTAR

- A. Mix all cementitious materials and sand in a mechanical batch mixer for a minimum of 5 minutes. Adjust the consistency of the mortar to the satisfaction of the mason, but add only as much water as is compatible with convenience in using the mortar. If the mortar begins to stiffen from evaporation or from absorption of a pat if the mixing water, re-temper the mortar immediately by adding water, and remix the mortar.

- B. Mortar for exterior walls shall have waterproofing added in accordance with the manufacturer's recommendations.
- C. Addition of admixtures or re-tempering of mortar at the mixer to extend its use will not be permitted.

3.3 RE-TEMPERING

- A. All mortar shall be used within 2-1/2 hours of initial mixing and no mortar shall be used after it has begun to set. Re-tempering of mortar in which setting has saturated will not be permitted. However, mortar shall be re-tempered, except as above qualified, as necessary to keep it plastic.

3.4 JOINTS

- A. Provide joints 3/8 inch nominal thickness and tooled unless shown otherwise on drawings.
- B. Construct uniform joints.
- C. Units shall be placed with sufficient pressure to extrude mortar and provide a tight joint.

3.5 REINFORCEMENT

- A. Reinforcement shall be secured against displacement prior to grouting at a spacing not greater than 4 feet.
- B. Provide rebar lap lengths specified in the General Structural Notes on the drawings. Provide 6 inches minimum lap for all ladder type joint reinforcing.

3.6 GROUTING

- A. Grout all cells, which are below grade.
- B. Grout lintel blocks over masonry openings and each jamb of masonry openings.
- C. Grout pours shall not exceed 5 feet in height.
- D. Grout all cells solid, which contain reinforcing.

Grout shall have a slump range of 8 to 11 inches tested in accordance with ASTM C143.

Consolidate grout pours 12 inches or less in height by mechanical vibration or by puddling. Consolidate pours exceeding 12 inches in height by mechanical vibration and reconsolidate by mechanical vibration after initial water loss and settlement has occurred. Typically this occurs within 2-4 minutes of placement of grout.

Place grout within 1-1/2 hours from introducing water in the mixture and prior to initial set.

3.7 POINTING AND CLEANING

- A. At completion of unit masonry work, fill holes in joints and tool.
- B. Cut out and repoint defective joints.
- C. Dry brush masonry surface after mortar has set, at end of each day's work and after final pointing.
- D. Leave work and surrounding surfaces clean and free of mortar spots and droppings.

3.8 PROTECTION OF WORK

- A. Protect sills, ledges, and offsets from mortar drippings or other damage during construction.
- B. Remove misplaced mortar or grout immediately.
- C. Cover top of walls with non-staining waterproof coverings when work is not in progress.
- D. Provide adequate bracing during construction to prevent damage from wind loads.

3.9 WEATHER CONDITIONS

- A. Do not place concrete masonry units when air temperature is below 20 degrees F.
- B. For temperatures between 20 degrees F and 40 degrees F, sand and mixing water shall be heated to produce mortar temperatures between 40 degrees F and 120 degrees F. Mortar shall be maintained above 32 degrees F during placement.
- C. Masonry shall be protected from freezing for 24 hours after placement.

3.10 FIELD QUALITY CONTROL

- A. The Owner shall employ the services of a qualified testing laboratory to perform tests and submit test reports.
- B. Concrete Masonry Units (CMU): Test in accordance with ASTM C 140. "Standard Test Methods of Sampling and Testing Concrete Masonry Units." Six units shall be sampled and tested for each lot of 10,000 units or less delivered to the job site. Twelve units shall be sampled from each lot of more than 10,000 units and less than 100,000 units.
- C. Mortar: By proportions according to ASTM C 780 "Standard Test Method for Preconstruction and Construction Evaluation of Mortars for Plain and Reinforced Masonry."
- D. Grout: Mold and test 4 test specimens in accordance with ASTM C 1019 "Test Method for Sampling and Testing Grout" from each day's grout placement. Test grout slump prior to each day's grouting process. Submit slump value with test specimen results. See General Structural Notes for required strength.

END OF SECTION

SECTION 05 1000
STRUCTURAL STEEL

PART 1 - GENERAL

1.1 WORK INCLUDED

- A. This section includes the fabrication and erection of structural steel.

1.2 RELATED WORK SPECIFIED ELSEWHERE

- A. Architecturally Exposed Structural Steel – Section 051 1213
- B. Steel Joists - Section 05 2100
- C. Metal Deck - Section 05 3000
- D. Painting and Coating – Section 09 9000

1.3 QUALITY ASSURANCE

- A. Qualifications of Fabricator: Fabricator shall have a minimum of 5 years experience in the fabrication of structural steel of structures of similar size. Fabricator shall have AISC or IAS certification or other certification as approved by the building official and the engineer of record. If the fabricator does not have approved certification, special inspection shall be done on the fabrication process and on the fabricated material as required by Section 1704.2, Inspection of Fabricators of the International Building Code. The non-certified fabricator shall engage a special inspector that meets the requirements of IBC section 1704.1 and is acceptable to the building official and the engineer of record. Provide documentation verifying certification or provide special inspector information for approval prior to issuance of a building permit.
- B. Qualifications of Erector: Erector shall have a minimum of 5 years experience in the erection of structural steel of structures of similar size.
- C. Qualifications of Field Welders: Welders shall be certified in accordance with AWS D1.1 within the last 12 months.
- D. Reference Standards:
 - 1. ASTM International (ASTM), latest versions:
 - a. ASTM A 36/ A36M Standard Specification for Carbon Structural Steel
 - b. ASTM A 53/ A 53M Standard Specification for Pipe, Steel, Black and Hot-Dipped, Zinc-coated Welded and Seamless
 - c. ASTM A 61/A6M Standard Specification for General Requirements for Rolled Structural Steel Bars, Plates, Shapes, and Sheet Piling.
 - d. ASTM A 307 Standard Specification for Carbon Steel Bolts and Studs, 60,000 PSI Tensile Strength
 - e. ASTM A 325 Standard Specification for Structural Bolts, Steel, Heat Treated, 120/105 ksi Minimum Tensile Strength
 - f. ASTM A 490 Specification for Structural Bolts, Alloy Steel, Heat Treated, 150 ksi Minimum Tensile Strength

- g. ASTM A 500/ A500M Standard Specification for Cold-Formed Welded and Seamless Carbon Steel Structural Tubing in Rounds and Shapes
 - h. ASTM A 992/A 992M Standard Specification for Structural Steel Shapes
 - i. ASTM C 1107/ C1107M Standard Specification for Packaged Dry, Hydraulic-Cement Grout (non-shrink)
 - j. ASTM F1554-07 Standard Specification for Anchor Bolts, Steel, 36, 55, and 105-ksi Yield Strength.
2. American Welding Society (AWS), latest edition.
 - a. AWS D1.1 Structural Welding Code-Steel
 3. American Institute of Steel Construction (AISC), Steel Construction Manual, latest edition.
 - a. Specification for Structural Steel Buildings
 - b. AISC Code of Standard Practice
 - c. Specification for Structural Joints Using ASTM A 325 or A 490 Bolts.

1.4 SUBMITTALS

- A. Shop Drawings: Submit shop drawings including erection plans, complete details and schedules for fabrication and assembly of structural steel members. Include details of cuts, connections, camber, holes, and other pertinent data. Indicate welds by standard AWS symbols, and show size, length, and type of each weld. Shop drawings shall not be made by reproduction of the Contract Drawings.
- B. Provide setting drawings and directions for installation of anchor bolts and other anchorages to be installed by others.
- C. Welder Certification: Submit affidavit stating that all welders are certified in accordance with AWS and provide copies of welder's certificates.

1.5 PRODUCT DELIVERY, STORAGE, AND HANDLING

- A. Support structural steel above ground on skids, pallets, platforms, or other supports.
- B. Protect steel from damage.
- C. Store packaged materials in original unbroken package or container.
- D. Do not store materials on structure in a manner that might cause distortion or damage to members or supporting structures.
- E. Replace damaged shapes or members.
- F. Waste Management and Disposal; As specified in Division 01 Section "Construction Waste Management" and as follows: Collect cut offs and scrap and place in designated area for recycling in accordance with the Waste Management Plan and local recycler standards.

PART 2 - PRODUCTS

18-01.01 WPMHC Expansion
 Chavez-Grievs Consulting Engrs.
 2019-11-01

05 1000 - 2

STRUCTURAL STEEL

2.1 MATERIALS

- A. All Wide Flange Shapes shall conform to ASTM A 992, Grade 50 unless noted otherwise.
- B. All Angles, Channels, Plates, and Bars: ASTM A 36.
- C. Structural Steel Pipe: ASTM A 53, Type E or S, Grade B Fy=35 ksi
- D. Rectangular or Square Hollow Structural Section: ASTM A 500, Grade B, Fy = 46 ksi.
- E. Round Hollow Structural Sections: ASTM A 500, Grade B, Fy-42 ksi.
- F. Anchor Bolts: ASTM F1554, Grade 36
- G. High Strength Tension Control Threaded Fasteners: Meet requirements of ASTM A 325 or ASTM A 490.
- H. Headed Anchor Shear Studs: By the Nelson Division of TRW.
- I. Welding Electrodes: E 70 Series.
- J. Shop Primer Paint: Fabricators standard rust inhibitive primer.
- K. Non-Metallic, Non-Shrink Grout: Meets the requirements of ASTM C 1107.
- L. Nonmetallic, Shrinkage-Resistant Grout: ASTM C 1107, factory-packaged, nonmetallic aggregate grout, noncorrosive, non-staining, mixed with water to consistency suitable for application and a 30-minute working time. Grout shall have a minimum 28 day compressive strength of 6,000 psi.
 - 1. Subject to compliance with requirements, provide products by one of the following or an approved equal:
 - a. Five Star Fluid Grout 100; Five Star Products, Inc., Fairfield, Connecticut.
 - b. Crystex; L&M Construction Chemicals, Inc. Omaha, Nebraska.
 - c. Sure-Grip High Performance Grout; Dayton superior Corp., Miamisburg, Ohio.
 - d. Sonnogrout 10K; Sonneborn Building Products, Shakopee, Minnesota.
 - e. Sealight Pac-It Grout; W.R. Meadows, Inc., Hampshire, Illinois.
 - f. Enduro 50; Conspec Marketing & Manufacturing Co., Inc, Kansas City, Kansas.

2.2 FABRICATION

- A. Fabrication shall be in accordance with the AISC "Code of Standard Practice for Buildings and Bridges".
- B. Connections: Weld or bolt shop connections as indicated on the approved shop drawings. Design connections to support reactions and forces where indicated on the drawings.
- C. Shop Welds: Shall be visually inspected by the Fabricator's quality control department.

2.3 SHOP PAINTING

- A. General: Shop paint structural steel, except those members or portions of members to be embedded in concrete, mortar or to receive sprayed on fireproofing. Paint embedded steel, which is partially exposed on exposed portions and initial 2 inch of embedded areas only.

- B. Do not paint surfaces, which are to be welded or high-strength bolted with friction-type connections.
- C. Surface Preparation: After inspection and before shipping, clean steel work to be painted. Remove loose rust, loose mill scale, and spatter, slag or flux deposits. Clean steel in accordance with Steel Structures Painting Council (SSPC) as follows:
 - 1. SP-1 "Solvent Cleaning"
 - 2. SP-2 "Hand Tool Cleaning"
 - 3. For Architecturally Exposed Structural Steel, AESS, see Architectural drawings for locations and specification Architecturally Exposed Structural Steel Section 05 1213.
- D. Painting: After surface preparation, apply structural steel primer paint in accordance with manufacturer's instructions. Provide one coat.

PART 3 - EXECUTION

3.1 COORDINATION

- A. Field Measurements: Verify all elevations, locations, and dimensions of surfaces to receive structural steel.
- B. Anchor Bolts and Other Embedded Items: Verify locations and positions of anchor bolts and other embedded items used to support structural steel.

All Anchor bolts for column base plates, anchors and bearing plates for beams shall be located prior to installation by a Registered Professional surveyor. The Professional Surveyor shall use project control points, such as bench marks, grid lines, or building corners established and accurately maintained by the General Contractor for vertical and horizontal control of location. Templates shall be used to locate groupings of bolts or anchors and shall be confirmed as to orientation and hole geometry accuracy.

Anchor bolts and bearing plates with anchors shall be stabilized against movement, vertical and horizontal, prior to and during concrete casting of concrete supporting these devices.

Upon completion of the concrete casting the Professional Surveyor shall verify vertical and horizontal locations and orientation of anchor bolts or bearing plates with anchors. A report shall be furnished to the Engineer of Record (through the General Contractor and Architect) noting non-compliant locations. The EOR, will furnish remedial actions required to correct the non-compliant anchor bolt or bearing plate locations. Allow ten days for the EOR's report on remedial actions necessary.

It shall be the General Contractor's responsibility to have this work performed.

- C. Correct any unsatisfactory conditions prior to erection of structural steel.

3.2 PREPARATION

- B. Clean surfaces to receive structural steel prior to erection.

3.3 ERECTION

- A. General: Erect structural steel in accordance with AISC "Code of Standard Practice for Steel Buildings and Bridges".

- B. Field Assembly: Assemble structural steel accurately to the lines and elevations shown on the drawings. Align and adjust components accurately before fastening.
- C. Temporary Bracing: Provide temporary bracing or guys to secure structural steel against wind, seismic, or construction loads. It is the responsibility of the Contractor to maintain stability of the structure during erection.
- D. Field Bolted Connections: Install high strength tension control bolts in accordance with AISC Specifications for Structural Joints Using ASTM A325 and A490 Bolts and the manufacturer's instructions. Where clearance within a connection does not permit the use of tension control bolts, standard A325 bolts shall be used and inspected in accordance with the AISC Specification for Structural Joints.
- E. Field Welding: Perform all welds in accordance with AWS.
- F. Welded Connections: Field welds shall be visually inspected according to AWS D1.1/D1.1M.
 - i. In addition to visual inspection, field welds will be tested and inspected according to AWS D1.1/D1.1M and the following inspection procedures, at testing agency's option:
 - 1. Liquid Penetrant Inspection: ASTM E 165.
 - 2. Magnetic Particle Inspection: ASTM E 709; performed on root pass and on finished weld. Cracks or zones of incomplete fusion or penetration will not be accepted.
 - 3. Ultrasonic Inspection: ASTM E 164.
 - 4. Radiographic Inspection: ASTM E 94
- G. Gas Cutting: Do not use gas-cutting torches in field to cut structural framing.
- H. Do not enlarge unfair holes by burning. Ream holes that must be enlarged to admit bolts.
- I. Field Touch-up Painting (Primer): Paint all exterior exposed bolts, washers, and nuts after connections have been tightened and checked. Paint all exterior exposed field welds. Paint all exterior exposed abrasions in shop coat. Use same paint as for shop painting.
- J. Grout Placement: Comply with the manufacturer's instructions.
- K. Tighten anchor bolts after supported members have been positioned and plumbed.

END OF SECTION

SECTION 05 1213
ARCHITECTURALLY EXPOSED STRUCTURAL STEEL FRAMING

PART 1 - GENERAL

1.1 RELATED DOCUMENTS

- A. Drawings and general provisions of the Contract, including General and Supplementary Conditions and Division 01 Specification Sections, apply to this Section.

1.2 SUMMARY

- B. Section includes architecturally exposed structural-steel (AESS).
 - 1. Division 5 - "Structural Steel Framing" also apply to AESS.
- C. Related Requirements:
 - 1. Division 5 - "Structural Steel Framing" for additional requirements applicable to AESS.
 - 2. Division 5 – "Metal Fabrications" for steel lintels and shelf angles not attached to structural-steel frame, miscellaneous steel fabrications, and other metal items not defined as structural steel.
 - 3. Division 9 - "Painting" and "High-Performance Coatings"

1.3 DEFINITIONS

- D. AESS: Structural steel designated as "architecturally exposed structural steel" or "AESS" in the Contract Documents.

1.4 COORDINATION

- E. Coordinate selection of shop primers with topcoats to be applied per Division 9 - "Painting" and "High-Performance Coatings". Comply with paint and coating manufacturers' written recommendations to ensure that shop primers and topcoats are compatible with one another.

1.5 PREINSTALLATION MEETINGS

- F. Preinstallation Conference: See Division 5 - "Structural Steel Framing"

1.6 ACTION SUBMITTALS

- G. Shop Drawings: Show fabrication of AESS components. Shop Drawings for structural steel may be used for AESS provided items of AESS are specifically identified and requirements below are met for AESS.

1. Include details of cuts, connections, splices, camber, holes, and other pertinent data.
 2. Include embedment Drawings.
 3. Indicate welds by standard AWS symbols, distinguishing between shop and field welds, and show size, length, and type of each weld. Show backing bars that are to be removed and supplemental fillet welds where backing bars are to remain. Indicate grinding, finish, and profile of welds.
 4. Indicate type, size, and length of bolts, distinguishing between shop and field bolts. Identify pretensioned and slip-critical, high-strength bolted connections. Indicate orientation of bolt heads.
 5. Indicate exposed surfaces and edges and surface preparation being used.
 6. Indicate special tolerances and erection requirements.
- H. Samples: Submit Samples of AESS to set quality standards for exposed welds.
1. Two steel plates, 3/8 by 8 by 4 inches (9.5 by 200 by 100 mm), with long edges joined by a groove weld and with weld ground smooth.
 2. Steel plate, 3/8 by 8 by 8 inches (9.5 by 200 by 200 mm), with one end of a short length of rectangular steel tube, 4 by 6 by 3/8 inches (100 by 150 by 9.5 mm), welded to plate with a continuous fillet weld and with weld ground smooth and blended.
 3. Round steel tube or pipe, minimum 8 inches (200 mm) in diameter, with end of another round steel tube or pipe, approximately 4 inches (100 mm) in diameter, welded to its side at a 45-degree angle with a continuous fillet weld and with weld ground smooth and blended.

1.7 INFORMATIONAL SUBMITTALS

- I. Qualification Data:
1. Installer/Erector.
 2. Fabricator.
 3. Welder.
- J. Paint Compatibility Certificates: From manufacturers of topcoats applied over shop primers, certifying that shop primers are compatible with topcoats.

1.8 QUALITY ASSURANCE

1. Fabricator Qualifications:
 - a. A qualified fabricator that participates in the AISC Quality Certification Program and is designated an AISC-Certified Plant, Category BU: Certified Building Fabricator.
 - b. Minimum of 5 years' experience fabricating steel for jobs of similar size and complexity.
 2. Installer Qualifications:
 - a. Minimum of 5 years' experience erecting structural steel for jobs of similar size and complexity.
 3. Welding Qualifications: Qualify procedures and personnel according to AWS D1.1/D1.1M, "Structural Welding Code - Steel."
- K. -Painting Applicators: Qualified according to AISC's Sophisticated Paint Endorsement P1 or SSPC-QP 3, "Standard Procedure for Evaluating Qualifications of Shop Painting Applicators."

- L. Mockups: Build mockups of AESS to set quality standards for fabrication and installation.
 - 1. Build mockup of typical portion of AESS as shown on Drawings.
 - 2. Coordinate painting requirements with Division 9 - "Paintings" and "High-Performance Coatings".
 - 3. Approved mockups may become part of the completed Work if undisturbed at time of Substantial Completion.

1.9 DELIVERY, STORAGE, AND HANDLING

- M. Use special care in handling to prevent twisting, warping, nicking, and other damage. Store materials to permit easy access for inspection and identification. Keep steel members off ground and spaced by using pallets, dunnage, or other supports and spacers. Protect steel members and packaged materials from corrosion and deterioration.
 - 1. Do not store materials on structure in a manner that might cause distortion, damage, or overload to members or supporting structures. Repair or replace damaged materials or structures as directed.

1.10 FIELD CONDITIONS

- N. Field Measurements: Where AESS is indicated to fit against other construction, verify actual dimensions by field measurements before fabrication.

PART 2 - PRODUCTS

2.1 BOLTS, CONNECTORS, AND ANCHORS

- A. Tension-Control, High-Strength Bolt-Nut-Washer Assemblies: ASTM F 1852, Type 1, round-head assemblies, consisting of steel structural bolts with splined ends, heavy-hex carbon-steel nuts, and hardened carbon-steel washers.
 - 1. Finish: Plain unless noted otherwise.

2.2 FILLER

- B. Filler: Polyester filler intended for use in repairing dents in automobile bodies.

2.3 PRIMER

- C. Primer: Comply with Division 9 - "Paints and Coatings".
- D. Galvanizing Repair Paint: ASTM A 780/A 780M.
- E. Shop Primer for Galvanized Steel: MPI#26, cementitious galvanized metal primer.

2.4 FABRICATION

- F. Shop fabricate and assemble AESS to the maximum extent possible. Locate field joints at concealed locations if possible. Detail assemblies to minimize handling and to expedite erection.
- G. In addition to special care used to handle and fabricate AESS, comply with the following:
1. Fabricate with exposed surfaces smooth, square, and free of surface blemishes including pitting, rust, scale, and roughness.
 2. Grind sheared, punched, and flame-cut edges of AESS to remove burrs and provide smooth surfaces and edges.
 3. Fabricate AESS with exposed surfaces free of mill marks, including rolled trade names and stamped or raised identification.
 4. Fabricate AESS with exposed surfaces free of seams to maximum extent possible.
 5. Remove blemishes by filling or grinding or by welding and grinding, before cleaning, treating, and shop priming.
 6. Fabricate with piece marks fully hidden in the completed structure or made with media that permits full removal after erection.
 7. Fabricate AESS to the tolerances specified in AISC 303 for steel that is designated AESS.
 8. Fabricate AESS to the tolerances specified in AISC 303 for steel that is not designated AESS.
 9. Seal-weld open ends of hollow structural sections with 3/8-inch (9.5-mm) closure plates for AESS.
- H. Curved Members: Fabricate indicated members to curved shape by rolling to final shape in fabrication shop.
1. Distortion of webs, stems, outstanding flanges, and legs of angles shall not be visible from a distance of 20 feet (6 m) under any lighting conditions.
 2. Tolerances for walls of hollow steel sections after rolling shall be approximately 1/2 inch (13 mm).
- I. Coping, Blocking, and Joint Gaps: Maintain uniform gaps of 1/8 inch (3.2 mm) with a tolerance of 1/32 inch (0.8 mm) for AESS.
- J. Bolt Holes: Cut, drill, or punch standard bolt holes perpendicular to metal surfaces.
- K. Holes: Provide holes required for securing other work to structural steel and for other work to pass through steel members.
1. Cut, drill, or punch holes perpendicular to steel surfaces. Do not thermally cut bolt holes or enlarge holes by burning.
 2. Baseplate Holes: Cut, drill, mechanically thermal cut, or punch holes perpendicular to steel surfaces.
 3. Weld threaded nuts to framing and other specialty items indicated to receive other work.

2.5 SHOP CONNECTIONS

- L. High-Strength Bolts: Shop install high-strength bolts according to RCSC's "Specification for Structural Joints Using ASTM A 325 or A 490 Bolts" for type of bolt and type of joint specified.

1. Joint Type: Snug tightened unless noted otherwise.
- M. Weld Connections: Comply with AWS D1.1/D1.1M for tolerances, appearances, welding procedure specifications, weld quality, and methods used in correcting welding work, and comply with the following:
1. Assemble and weld built-up sections by methods that will maintain true alignment of axes without exceeding specified tolerances.
 2. Use weld sizes, fabrication sequence, and equipment for AESS that limit distortions to allowable tolerances.
 3. Provide continuous, sealed welds at angle to gusset-plate connections and similar locations where AESS is exposed to weather.
 4. Provide continuous welds of uniform size and profile where AESS is welded.
 5. Make butt and groove welds flush to adjacent surfaces within tolerance of plus 1/16 inch, minus zero inch (plus 1.5 mm, minus zero mm) for AESS. Do not grind unless required for clearances or for fitting other components, or unless directed to correct unacceptable work.
 6. Remove backing bars or runoff tabs; back-gouge and grind steel smooth for AESS.
 7. At locations where welding on the far side of an exposed connection of AESS occurs, grind distortions and marking of the steel to a smooth profile aligned with adjacent material.
 8. Make fillet welds for AESS oversize and grind to uniform profile with smooth face and transition.
 9. Make fillet welds for AESS of uniform size and profile with exposed face smooth and slightly concave. Do not grind unless directed to correct unacceptable work.

2.6 GALVANIZING

- N. Hot-Dip Galvanized Finish: Apply zinc coating by the hot-dip process to structural steel according to ASTM A 123/A 123M.
1. Do not quench or apply post-galvanizing treatments that might interfere with paint adhesion.
 2. Fill vent and drain holes that are exposed in the finished Work, unless indicated to remain as weep holes, by plugging with zinc solder and filing off smooth.
 3. Galvanize. As shown on drawings.

2.7 SHOP PRIMING

- O. Shop prime steel surfaces except the following:
1. Surfaces embedded in concrete or mortar. Extend priming of partially embedded members to a depth of 2 inches (50 mm).
 2. Surfaces to be field welded.
 3. Surfaces to be high-strength bolted with slip-critical connections.
 4. Surfaces to receive sprayed fire-resistive materials.
 5. Galvanized surfaces.
- P. Surface Preparation: Clean surfaces to be painted. Remove loose rust and mill scale and spatter, slag, or flux deposits. Prepare surfaces according to the following specifications and standards:
1. SSPC-SP 3, "Power Tool Cleaning."

2. SSPC-SP 6/NACE No. 3, "Commercial Blast Cleaning."
- Q. Preparing Galvanized Steel for Shop Priming: After galvanizing, thoroughly clean steel of grease, dirt, oil, flux, and other foreign matter, and treat with etching cleaner.
 - R. Priming: Immediately after surface preparation, apply primer according to manufacturer's written instructions per Division 9 - "Painting" and "High-Performance Coatings". Use priming methods that result in full coverage of joints, corners, edges, and exposed surfaces.

PART 3 - EXECUTION

3.1 EXAMINATION

- A. Verify, with steel erector present, elevations of concrete- bearing surfaces and locations of anchor rods, bearing plates, and other embedments for compliance with requirements.
 1. Prepare a certified survey of bearing surfaces, anchor rods, bearing plates, and other embedments showing dimensions, locations, angles, and elevations.
- B. Examine AESS for twists, kinks, warping, gouges, and other imperfections before erecting.
- C. Proceed with installation only after unsatisfactory conditions have been corrected.

3.2 PREPARATION

- D. Provide temporary shores, guys, braces, and other supports during erection to keep AESS secure, plumb, and in alignment against temporary construction loads and loads equal in intensity to design loads. Remove temporary supports when permanent structural steel, connections, and bracing are in place unless otherwise indicated.
 1. If possible, locate welded tabs for attaching temporary bracing and safety cabling where they will be concealed from view in the completed Work.

3.3 ERECTION

- E. Set AESS accurately in locations and to elevations indicated and according to AISC 303 and AISC 360.
 1. Erect AESS to the tolerances specified in AISC 303 for steel that is designated AESS.
 2. Erect AESS to the tolerances specified in AISC 303 for steel that is not designated AESS.
- F. Do not use thermal cutting during erection.

3.4 FIELD CONNECTIONS

- G. High-Strength Bolts: Install high-strength bolts according to RCSC's "Specification for Structural Joints Using ASTM A 325 or A 490 Bolts" for type of bolt and type of joint specified.

1. Joint Type: Snug tightened unless noted otherwise.
 2. Orient bolt heads in same direction for each connection and to maximum extent possible in same direction for similar connections.
- H. Weld Connections: Comply with requirements in "Weld Connections" Paragraph in "Shop Connections" Article.
1. Remove backing bars or runoff tabs; back-gouge and grind steel smooth for AESS.
 2. Remove erection bolts in AESS, fill holes, and grind smooth.
 3. Fill weld access holes in AESS and grind smooth.

3.5 FIELD QUALITY CONTROL

- I. Testing Agency: Owner will engage a qualified independent testing and inspecting agency to inspect AESS as specified in Division 5 "Structural Steel Framing." The testing agency is not responsible for enforcing requirements relating to aesthetic effect.
- J. Architect will observe AESS in place to determine acceptability relating to aesthetic effect.

3.6 REPAIRS AND PROTECTION

- K. Remove welded tabs that were used for attaching temporary bracing and safety cabling and that are exposed to view in the completed Work. Grind steel smooth.
- L. Galvanized Surfaces: Clean field welds, bolted connections, and abraded areas and repair galvanizing to comply with ASTM A 780/A 780M.

END OF SECTION

SECTION 05 2100

STEEL JOISTS

PART 1 - GENERAL

1.1 WORK INCLUDED

- A. This section includes the fabrication and erection of open web steel joists.

1.2 RELATED WORK SPECIFIED ELSEWHERE

- A. Structural Steel - Section 05 1000
- B. Painting and Coating – Section 09 9000

1.3 QUALITY ASSURANCE

- A. Qualification of Fabricator: Fabricator shall be a member of the Steel Joist Institute.
- B. Qualification of Field Welders: Welders shall be certified in accordance with AWS D1.1 within the last 12 months.
- C. Reference Standards:
 - 1. ASTM International, latest versions
 - a. ASTM A 307 Standard Specification for Carbon Steel Bolts and Studs, 60 000 PSI Tensile Strength
 - 2. Steel Joist Institute (SJI) Standard Specifications, Load Tables and Weight Tables for Steel Joists and Joist Girders, 44th Edition, 2010.
 - a. Standard Specifications and Load Tables, Open Web Steel Joists, K-Series. SJI, 2010
 - b. Standard Specifications and Load Tables for Longspan Steel Joists, LH-Series and Deep Longspan Steel Joists, DHL Series SJI, 2010

1.4 SUBMITTALS

- A. Certification: Submit manufacturer's certification that joists comply with SJI Specifications.
- B. Shop Drawings: Submit detailed drawings showing layout of joist units, special connections, jointing and accessories. Include mark, number, type, location and spacing of joists and bridging. Shop Drawings shall not be made by reproduction of the Contract Drawings. Shop drawings shall be coordinated with all other disciplines and the complete set of contract documents.

1.5 PRODUCT DELIVERY, STORAGE, AND HANDLING

- A. Support structural steel above ground on skids, pallets, platforms, or other supports
- B. Protect steel from damage.

- C. Store packaged materials in original unbroken package or container.
- D. Do not store materials on structure in a manner that might cause distortion or damage to members or supporting structures.
- E. Replace damaged shapes or members.
- F. Waste Management and Disposal: As specified in Division 01 Section "Construction Waste Management" and as follows:
 - 1. Collect off cuts and scrap and place in designated area for recycling in accordance with the Waste Management Plan and local recycler standards.

PART 2 - PRODUCTS

2.1 MATERIALS

- A. Steel: Comply with SJI Specifications.
- B. Unfinished Threaded Fasteners: ASTM A 307, Grade A, regular hexagon type, low carbon steel.
- C. Steel Prime Paint: Comply with SJI Specifications.

2.2 FABRICATION

- A. Fabricate steel joists in accordance with SJI Specification.
- B. Extended Ends: Provide extended ends on joists as required complying with applicable SJI Specifications and load tables.
- C. Ceiling Extensions: Provide ceiling extensions in areas having ceilings attached directly to joist bottom chord. Provide an extended bottom chord element of sufficient strength to support ceiling construction. Extend ends to within ½ inch of finished wall surface unless otherwise indicated.
- D. Bridging: Provide horizontal or diagonal type bridging for "open web" joists, as required by SJI Specifications.
- E. Shop Painting: Remove loose scale, heavy rust, and other foreign materials from fabricated joists and accessories before application of shop paint.
- F. Apply one shop coat of primer paint to steel joists and accessories, by spray, dipping, or other method to provide a continuous dry paint film thickness of not less than 0.50 mil.

PART 3 - EXECUTION

3.1 COORDINATION

- A. Verify all elevation locations and dimensions of surfaces to receive steel joists. Furnish plates, angles, etc. as required to secure steel joists.

3.2 ERECTION

- A. Place and secure steel joists in accordance with SJI Specifications, approved shop drawings, and as herein specified.

- B. Placing Joists: Do not start placement of steel joists until supporting work is in place and secured. Place joists on supporting work, adjust and align in accurate locations and spacing before permanently fastening.
- C. Bridging: Install bridging simultaneously with joist erection, before construction loads are applied. Anchor ends of bridging lines at top and bottom chords where terminating at walls or beams.
- D. Fastening Joists: Field weld joists to supporting steel framework in accordance with SJI Specifications for type of joists used. Coordinate welding sequence and procedure with placing of joists unless noted on contract drawings. Bolt joists to supporting steel framework where required by SJI Specifications.
- E. Touch-Up Painting: After joist installation, paint field bolt heads and nuts, and welded areas, abraded or rusty surfaces on joists and steel supporting members. Wire brush surfaces and clean with solvent before painting. Use same type of paint as used for shop painting.

END OF SECTION

SECTION 05 3000

METAL DECKING

PART 1 - GENERAL

1.1 WORK INCLUDED

- A. Provide all metal decking complete in place as shown on the drawings, specified herein, and needed for a complete and proper installation.

1.2 RELATED WORK SPECIFIED ELSEWHERE

- A. Structural Steel - Section 05 1000
- B. Steel Joists - Section 05 2100
- C. Cold-Formed Metal Framing - Section 05 4000
- D. Painting and Coating – Section 09 9000

1.3 QUALITY ASSURANCE

- A. Reference Standards:
 - 1. Qualification of Field Welders: Welders shall be certified in accordance with AWS D1.3 within the last 12 months.
 - 2. ASTM International, latest versions:
 - a. ASTM A 653/
A653M Standard Specification for Steel Sheet, Zinc-Coated (Galvanized) or Zinc-Iron Alloy-Coated (Galvannealed) by the Hot-Dip Process
 - b. A 1008/
A 1008M Standard Specification for Steel, Sheet, Cold-Rolled, Carbon, Structural, High-Strength Low Alloy, High-Strength Low Alloy with Improved Formability
 - c. ASTM A 1011/
A 1011M Standard Specification for Steel, Sheet and Strip, Hot Rolled, Carbon, Structural, High-Strength Low-Alloy, High-Strength Low Alloy with Improved Formability, and Ultra- High-Strength
 - 3. American Welding Society (AWS), latest edition.
 - a. D1.3 Structural Welding Code - Sheet Steel
 - 4. Steel Deck Institute.
 - a. SDI Design Manual for Floor Decks, Form Decks and Roof Decks
 - b. SDI Diaphragm Design Manual Third Edition

1.4 SUBMITTALS

- A. Product Data: Submit manufacturer's specifications and installation instructions for each type of decking and accessories.
- B. Shop Drawings: Submit detailed drawings showing layout and types of deck panels, anchorage details, and conditions requiring closure panels, supplementary framing, sump pans, cant strips, cut openings, special jointing or other accessories. Shop Drawings shall not be made by reproduction of the Contract Drawings.

1.5 PRODUCT DELIVERY, STORAGE, AND HANDLING

- A. Support metal deck above ground on skids, pallets, platforms or other supports.
- B. Protect metal deck from damage.
- C. Store packaged materials in original unbroken package or container.
- D. Waste Management and Disposal: As specified in Division 01 Section "Construction Waste Management" and as follows:
 - 1. Collect off cuts and scrap and place in designated area for recycling in accordance with the Waste Management Plan and local recycler standards.

PART 2 - PRODUCTS

2.1 MATERIALS

- A. Metal Roof Deck
 - 1. 1 ½" Type B, ASTM A 1008, Grade C. See plans for type, size and finish. Metal deck used in fire rated assemblies shall meet the requirements of UL. The UL mark on the product will be accepted as evidence of compliance.
- B. Metal Floor Deck: ASTM A 1011 with galvanized finish. See plans for type and size.
- C. Finishes:
 - 1. Painted: Manufacturer's baked-on, rust-inhibitive paint.
 - 2. Galvanized: Conform to ASTM A 653, G60.

PART 3 - EXECUTION

3.1 COORDINATION

- A. All edge angle shall be in place with proper attachment prior to installation of metal deck. All roof and floor opening frames shall be installed prior to deck installation.

3.2 INSTALLATION

- A. General: Install deck units and accessories in accordance with manufacturer's recommendations and final shop drawings, and as specified herein. Locate deck bundles to prevent overloading of structural members.

- B. Place deck units on supporting steel framework and adjust to final position with ends accurately aligned and bearing on supporting members before being permanently fastened. Do not stretch or contract side lap interlocks.
- C. Place deck units in straight alignment for entire length of run.
- D. Place deck units flat and square secured to adjacent framing without warp or excessive deflection.
- E. Lap ends of deck units a minimum of 2 inches over supports.
- F. Place deck units to permit proper attachment to the perimeter deck angle.
- G. Do not use deck units for storage or working platforms until permanently secured.
- H. Cut and neatly fit deck units and accessories around other work projecting through or adjacent to the decking, as shown.
- I. Fasten deck units to steel supporting members as shown on the structural drawings.
- J. Fasten side laps of units as called for on the structural drawings.
- K. Care shall be exercised in the selection of electrodes and amperage to provide positive welds and to prevent high amperage blowholes.
- L. Comply with AWS D1.3 requirements and procedures.
- M. Provide additional metal reinforcement and closure pieces as required for strength, continuity of decking and support of other work shown.
- N. Install closure strips at all locations as recommended by the manufacturer to provide a complete installation.
- O. Provide cleaning and touch-up painting of field welds, abraded areas and rust spots, as required for all exposed areas after erection and before proceeding with field painting.

END OF SECTION

SECTION 05 4000

COLD FORMED METAL FRAMING

PART 1 GENERAL

1.1 WORK INCLUDED

- A. This section includes all lightgauge studs, joists and track, 20 gage or heavier, including bridging, and related accessories as indicated on the Contract Drawings and specified herein.

1.2 RELATED WORK SPECIFIED ELSEWHERE

- A. Structural Steel - Section 05 1000
- B. Steel Joists - Section 05 2100
- C. Painting and Coating – Section 09 9000

1.3 QUALITY ASSURANCE

- A. Reference Standards:
 - 1. American Iron and Steel Institute (AISI) North American Specification for the Design of Cold-Formed Steel Structural Members, latest version.
 - 2. American Welding Society of (AWS) D1.3, Structural Welding Code-Sheet Steel. ASTM International, latest versions:
 - a. ASTM A 653/ A653M Standard Specification for Steel Sheet, Zinc-Coated (Galvanized) or Zinc-Iron Alloy-Coated (Galvannealed) by the Hot-Dip Process
 - b. A 1008/ A 1008M Standard Specification for Steel, Sheet, Cold-Rolled, Carbon, Structural, High-Strength Low Alloy, High-Strength Low Alloy with Improved Formability
 - c. ASTM A 1011/ A 1011M Standard Specification for Steel, Sheet and Strip, Hot Rolled, Carbon, Structural, High-Strength Low-Alloy, High-Strength Low Alloy with Improved Formability, and Ultra-High-Strength
 - 3. Qualifications of Erector: Erector shall have a minimum of 5-years experience in the erection of structural steel of structures of similar size.
 - 4. Qualifications of Field Welders: Welders shall be certified in accordance with AWS D1.1 within the last 12 months.

1.4 SUBMITTALS

- A. Submit stamped and signed calculations by a professional engineer registered in the state of the project.
- B. Submit manufacturer's product information and installation instructions for each item of lightgauge framing. Submit shop drawings for all prefabricated light gage systems.

1.5 PRODUCT DELIVERY, STORAGE, AND HANDLING

- A. Protect metal framing units from rusting and damage. Deliver to project site in manufacturer's unopened containers or bundles, fully identified with name, brand, type, and grade. Store off ground in a dry ventilated space or protect with suitable waterproof coverings.
- B. Waste Management and Disposal: As specified in Division 01 Section "Construction Waste Management" and as follows:
 - 1. Collect off cuts and scrap and place in designated area for recycling in accordance with the Waste Management Plan and local recycler standards.

PART 2 PRODUCTS

2.1 MATERIALS

- A. Metal Framing:
 - 1. All 12, 14, and 16 gage steel studs and joists shall be formed from steel that meets the requirements of one of the following standards with a minimum yield strength of 50,000 psi:
 - a. Painted Material - ASTM A 1011, Grade 50.
 - b. Galvanized Material - ASTM A 653 Grade 50.
 - 2. All 18 and 20 gage steel studs and joists; all track, bridging and accessories shall be formed from steel that meets the requirements of one of the following with a minimum yield strength of 33,000 psi:
 - a. Painted Material - ASTM A 1008, Grade C.
 - b. Galvanized Material - ASTM A 653.
- B. Material Finishes: All stud and joist components shall be primed with paint meeting the performance requirements of TT-P-1636C, or shall be formed from steel having a G-60 galvanized coating or better.

2.2 FABRICATION

- A. Framing components may be prefabricated into panels prior to erection. Prefabricated panels shall be square, with components attached to prevent racking. Handling and lifting of panels shall be done in a manner as to not cause distortion in any member.
- B. All framing components shall be cut squarely for attachment to perpendicular members, or as required for an angular fit against abutting members. Members shall be held positively in place until properly fastened.

PART 3 EXECUTION

3.1 INSTALLATION

- A. Install metal framing systems in accordance with manufacturer's printed instructions and recommendations, unless otherwise indicated on Contract Drawings.
- B. Install and align tracks accurately to layout at base and tops of studs. Secure tracks as indicated on Contract Drawings. Provide fasteners at corners and ends of tracks.

- C. Install supplementary framing, blocking and bracing in metal framing system to support fixtures, equipment, etc. Comply with stud manufacturer's recommendations and industry standards, considering weight and loading of each item.
- D. Secure studs to top and bottom tracks at both inside and outside flanges.
- E. Frame wall openings larger than 2 foot-0 inches square with double studs at each jamb of frame except where more than 2 are either shown or indicated in manufacturer's instructions. Install tracks and jack studs above and below wall openings. Anchor tracks to jamb studs with stud shoes or by welding, and space jack studs same as full-height studs of wall. Secure stud system wall opening frame in manner indicated.
- F. All components of build-up stud sections, including jack studs, full height studs, columns, headers, etc. shall be attached intermittently together the full height of each flange to flange connection.
- G. Install horizontal bridging in stud system, spaced (vertical distance) at no more than 4 foot – 0 inches o.c.
- H. Touch-up shop-applied protective coatings damaged during handling and installation. Use compatible primer for prime coated surfaces; use galvanizing repair paint for galvanized surfaces, such as zinc-rich paint.

END OF SECTION

SECTION 05 5100

METAL STAIRS

PART 1 - GENERAL

1.1 SUMMARY – DESCRIPTION OF WORK

- A. Section Includes: This Section specifies prefabricated metal stairs and railings.
- B. Related Requirements:
 - 1. Section 03 3000 - Cast-in-Place Concrete
 - 2. Section 05 1000 - Structural Steel
 - 3. Section 05 1217 – Architecturally Exposed Structural Steel
 - 4. Section 05 5000 – Metal Fabrications

1.2 QUALITY ASSURANCE

- C. Reference Standards:
 - 1. ASTM International (ASTM), latest versions:
 - a. ASTM A36 Standard Specification for Carbon Structural Steel.
 - b. ASTM A53 Standard Specification for Pipe, Steel, Black and Hot-Dipped, Zinc-Coated, Welded and Seamless.
 - c. ASTM A500 Standard Specification for Cold-Formed Welded and Seamless Carbon Steel Structural Tubing in Rounds and Shapes.
 - d. ASTM A513 Standard Specification for Electric-Resistance-Welded Carbon and Alloy Steel Mechanical Tubing.
 - e. ASTM A786 Standard Specification for Hot-Rolled Carbon, Low-Alloy, High-Strength Low-Alloy, and Alloy Steel Floor Plates.
 - f. ASTM A1008 Standard Specification for Steel, Sheet, Cold-Rolled, Carbon, Structural, High-Strength Low-Alloy, High-Strength Low-Alloy with Improved Formability, Solution Hardened, and Bake Hardenable.
 - g. ASTM A1011 Standard Specification for Steel, Sheet and Strip, Hot-Rolled, Carbon, Structural, High-Strength Low-Alloy, High-Strength Low-Alloy with Improved Formability, and Ultra-High Strength.
 - 2. American Welding Society (AWS):
 - a. AWS D1.1 Structural Welding Code - Steel.
 - b. AWS D1.3 Structural Welding Code - Sheet Steel.
 - 3. American National Standards Institute (ANSI):
 - a. ANSI A117.1 Accessible and Usable Buildings and Facilities Standards.
 - 4. The Society for Protective Coatings (SSPC):
 - a. SSPC-SP3 Power Tool Cleaning.

1.3 ADMINISTRATIVE REQUIREMENTS

- D. Coordination: Coordinate work of this Section with work of other trades for proper time and sequence to avoid construction delays. Comply with Section 01 3100 - Project Management and Coordination, if applicable.
- E. Sequencing: Sequence work of this section in accordance with Section 01 1216 - Work Sequence and manufacturer's written recommendations for sequencing construction operations, if applicable.
- F. Scheduling: Schedule work of this Section in accordance with Section 01 3213 - Scheduling of Work, if applicable.

1..4 ACTION SUBMITTALS

- G. General: Submit listed submittals in accordance with Contract Conditions and Section 01 3300 - Submittal Procedures.
- H. Product Data: Submit specified products as follows:
 - 1. Manufacturer's product data.
 - 2. Manufacturer's installation instructions.
- I. Shop Drawings: Indicate information on shop drawings as follows:
 - 1. Stair plans, elevations, details, methods of installation and anchoring.
 - a. Show members, sizes and thickness, anchorage locations and accessory items.
 - b. Furnish setting diagrams for anchorage installation as required.
 - c. Include calculations stamped by a structural engineer registered in the jurisdiction in which the project is located.
- J. Samples: Submit as follows:
 - 1. Two samples, minimum size 6 inches (152 mm) square, representing actual product, finish and patterns for each finished tread product specified.

1.5 INFORMATION SUBMITTALS

- K. General: Submit listed submittals in accordance with Contract Conditions and Section 01 3300 - Submittal Procedures.
- L. Manufacturer's Instructions: Submit manufacturer's storage and installation instructions.
- M. Source Quality Control: Submit documentation verifying that components and materials specified in this Section are from single manufacturer.
- N. Qualification Statements:
 - 1. Submit certificate verification that manufacturer is American Institute of Steel Construction (AISC) Certified for Standard Steel Building Structures.
 - 2. Submit letter of verification for Installer's Qualifications.

1.6 QUALITY ASSURANCE

- O. Qualifications:

1. Manufacturer:
 - a. American Institute of Steel Construction (AISC) Certified firm having 10 years experience manufacturing components similar to or exceeding requirements specified in scope of project.
 - b. Having sufficient capacity to produce and deliver required materials without causing delay in work.
2. Installer: Acceptable to manufacturer.

1.7 DELIVERY, STORAGE & HANDLING

P. Delivery and Acceptance Requirements:

1. Deliver material in accordance with Section 01 6100 - Common Product Requirements and in accordance with manufacturer's written instructions.
2. Deliver materials in manufacturer's original packaging with identification labels intact and in sizes to suit project.

Q. Storage and Handling Requirements:

1. Store materials protected from exposure to harmful weather conditions and at temperatures recommended by manufacturer.

R. Packaging Waste Management:

1. Separate waste materials for reuse and recycling in accordance with Section 01 7419 Construction Waste Management and Disposal.
2. Remove packaging materials from site and dispose of at appropriate recycling facilities.
3. Collect and separate for disposal paper, plastic, polystyrene, corrugated cardboard, packaging material in appropriate onsite bins for recycling.
4. Fold metal and plastic banding; flatten and place in designated area for recycling.
5. Remove:
 - a. Pallets from site and return to supplier or manufacturer.

PART 2 - PRODUCTS

2.1 METAL STAIRS

A. Manufacturer:

1. Single Source Responsibility: Provide components and materials specified in this section from a single American Institute of Steel Construction (AISC) Certified manufacturer.
2. Substitution Limitations:
 - a. Substitutions: In accordance with Section 01 2500 - Substitution Procedures.

B. Description:

1. Regulatory Requirements:
 - a. In accordance with Section 01 4100 - Regulatory Requirements if applicable.

2. Sustainability Characteristics:
 - a. In accordance with general project requirements.
 3. Compatibility:
 - a. Ensure components and materials are compatible with specified accessories and adjacent materials.
- C. Design Criteria:
1. Structural Performance of Stairs: Stairs shall withstand the following structural loads without exceeding the allowable design working stress of materials, including anchors and connections. Apply each load to produce the maximum stress in each component:
 - a. Treads and Platforms of Metal Stairs: Capable of withstanding a uniform load of 100 psf (4.8 kN/m²) and concentrated load of 300 lbf (1.33 kN) applied on an area of 4 square inches (2581 square mm). Concentrated and uniform loads need not be assumed to act concurrently.
 - b. Stair Framing: Capable of withstanding stresses resulting from loads specified, in addition to stresses resulting from railing system loads.
 - c. Limit Deflection of Treads, Platforms and Framing Members: To L/240.
 2. Structural Performance of Handrails and Railings: Handrails and railings shall withstand the following structural loads without exceeding the allowable design working stress of materials, including handrails, railings, anchors and connections.
 - a. Top Rail of Guardrail: Capable of withstanding a concentrated load of 200 lbf (0.89 kN) applied in any direction and a uniform load of 50 psf (2.39 kN/m²) applied in any direction. Concentrated and uniform loads need not be assumed to act concurrently.
- D. Standard Stair and Rail System:
1. Manufacturer's standard prefabricated, pre-engineered straight run stair and landing system, consisting of hot rolled steel sheet stringers, risers, treads, landings, fasteners/supports and railings.
 - a. Stringers:
 - 1) Steel channel with side mounted prefabricated railings.
 - 2) Minimum thickness or gage as determined by structural design calculations, structural grade steel plate or channel.
 2. Risers: Closed riser, minimum 14 gage (1.9 mm) hot rolled mild steel sheet, sloped maximum 1 1/2 inches (38.1 mm) and conforming to Americans with Disabilities Act (ADA) nosing requirements.
 3. Treads: Manufacturer's standard concrete pan system, field poured. Tread pans to be minimum of 14 gage (1.9 mm), or as determined by design calculations. Pan depth 1 1/2 inches (38.1 mm). Exposed welds from the bottom side of flight assemblies will not be allowed. All welds to be from topside of tread pans as recommended by manufacturer.
 4. Mid Landings: Minimum of 12 gage (2.7 mm) hot-rolled mild steel sheets, formed for a minimum 2 1/2 inches (64 mm) concrete fill, with 11 gage channel supports and bracing welded to perimeter frame at 12 inches (305 mm) on center.

5. Fasteners and Supports: Sized by the manufacturer to meet structural design criteria. If hanger rod connections are applicable to any of the landing connections, they shall be a minimum of 5/8 inch (15.9 mm) diameter steel rod, with actual size based on stair load.
6. Manufacturer's standard welded steel tube railing system complying with the following requirements:
 - a. Rails: 1 1/2 inches (38.1 mm) diameter x 13 gage (2.3 mm) minimum round steel tube, continuous multi-strand type, equally spaced with not more than 3 15/16 inches (100 mm) clearance between strands and with a minimum extension per code at top and bottom risers. Wrap rail continuously past space between flights to form guardrail as required by building code. Terminate rail ends with radiused returns, newel posts or safety terminations approved by local code. Provide not less than 1 1/2 inches (38.1 mm) clearing between rail and wall.
 - b. Rail Posts: 1 1/2 inches (38.1 mm) square x 11 gage (3 mm) tubing. Rail posts to fasten to side of plate stringers per manufacturer's shop drawings. Manufacturer to pre-weld erection aid to rail post for proper height to aid stair erector. Erection aid (setting block) to be removed and weld-ground smooth after installation.
 - c. Fabrication:
 - 1) Use preformed or prefabricated bends.
 - 2) Butt weld tee and cross intersections in tubing. Cope and weld intersections in pipe. Miter elbows.
 - 3) Mechanically fasten internal sleeves and fittings.
 - 4) Provide minimum 12 gage (2.7 mm) welded steel plate closures or hemispherical closure fittings on all exposed rail ends.

E. Custom Stair and Rail System:

1. Support System: Provide landing support with manufacturer's standard system. Comply with details indicated on Drawings.
 - a. Hanger rod landing supports.
 - b. Tube strut landing supports.
 - c. Shelf angle landing supports.
 - d. Knockdown (KD) landing supports.
2. Rail System: Provide rail system. Comply with details indicated on Drawings.
 - a. Standard 34 inch (864 mm) height handrail system with 42 inch (1067 mm) guardrails at landings and openings.
 - 1) Rail Type: Full mesh panel rail, Picket style rail, 5-Line sweep rail, or Perforated panel rail.
 - b. Standard 36 inch (914 mm) height handrail system with 42 inch (1067 mm) guardrails at landings and openings.
 - 1) Rail Type: Full mesh panel rail, Picket style rail, 6-Line sweep rail, or Perforated panel rail.
 - c. Standard 42 inch (1067 mm) height guard rail system with 34 inch (864 mm) ADA Grab and with 42 inch (1067 mm) guardrails at landings and openings.
 - 1) Rail Type: Full mesh panel guard rail with hand rail, Picket style guard rail with handrail, 7-Line sweep guard rail with handrail, or Perforated panel guard rail with handrail.
3. Wall Handrails: Match stair handrails. Provide manufacturer's standard pressed steel wall brackets with anchors suitable for supporting construction.
4. Tread Construction: Comply with details indicated on Drawings.

- a. 1 1/2 inch (38.1 mm) pan type treads for field-poured concrete.
 - 1) Acceptable Material: Field-Poured Concrete Treads.
 - 5. Barrier Gates: Manufacturer's standard swing gate assembly with steel spring hinges and rubber bumper between barrier/gate assembly and rail post.
- F. Materials:
- 1. Steel Shapes and Plates: To ASTM A36.
 - 2. Steel Pipe: To ASTM A53 Type E or S, Grade B.
 - 3. Steel Tubing:
 - a. Structural Use: To ASTM A500, Grade B or C.
 - b. Non-Structural Use: To ASTM A513, hot rolled or coiled rolled (mill option).
 - 4. Steel Sheet:
 - a. Structural Use: To ASTM A1011 (hot rolled).
 - b. Non-Structural Use: To ASTM A786, ASTM A1008.
 - 5. Fasteners: As recommended by manufacturer.
 - 6. Welding Rods: In accordance with AWS code and AWS filler metal specifications for material being welded.
 - 7. Primer: HAPS-free, solvent-based, rust inhibitive primer containing less than 3.5 lb/gal (1.6 kg/L) Volatile Organic Compounds (VOC) and compatible with conventional alkyds topcoats.
- G. Fabrication:
- 1. Use same material and finish as parts being joined. Use stainless steel between dissimilar metals and non-corrosive fasteners at exterior connections or joints.
 - 2. Provide fasteners of sufficient strength to support connected members and loads, and to develop full strength of parts fastened or connected.
 - 3. Construct stairs and rails with all components necessary for support and anchorage, and for a complete installation.
- H. Finishes
- 1. Rails and Stair Components: Completely remove oil, grease, dirt, mill scale, rust, corrosion products, oxides, paint or other foreign matter from steel surface in accordance with SSPC SP3.
 - 2. Shop Primer: Immediately after fabrication and cleaning, spray apply primer to dry film thickness recommended by the primer manufacturer, but not less than 2.0 mil thickness. Apply one coat High Solids Red Oxide Anticorrosive primer meeting SSPC-15 Paint.

2.2 ACCESSORIES

- A. Anchor bolts, clip angles, hanger rods, hardware and incidental materials required for complete installation, as recommended by the manufacturer.

PART 3 - EXECUTION

3.1 EXAMINATION

18-01.01 WPMHC Expansion
 Chavez-Grievs Consulting Engrs.
 2019-11-01

METAL STAIRS

- A. Verification of Conditions: Verify that conditions of substrates previously installed under other sections or contracts are acceptable for product installation in accordance with manufacturer's instructions prior to metal stair and railing installation.
 - 1. Inform Architect of unacceptable conditions immediately upon discovery.
 - 2. Proceed with installation only after unacceptable conditions have been remedied and after receipt of written approval from Architect.

3.2 PREPARATION

- A. Ensure structure or substrate is adequate to support metal stairs and railings.

3.3 INSTALLATION

- A. Coordinate installation of metal stairs and railings in accordance with Section 01 73 19 - Installation.
- B. Coordinate metal stairs and railings work with work of other trades for proper time and sequence to avoid construction delays.
- C. Install stairs, landings and handrails in accordance with manufacturer's instructions. Install square, plumb, straight and true to line and level, with neatly fitted joints and intersections.
 - 1. Do not cut or alter structural components without written authorization.
 - 2. Field welding and joining shall conform to AWS D1.1 and AWS D1.3.
 - 3. Grind all exposed welds smooth and touch-up shop-primed areas with same primer as used by manufacturer.

3.4 ADJUSTING

- A. Adjust components and systems for correct function and operation in accordance with manufacturer's written instructions. Coordinate with Section 01 7500 - Starting and Adjusting.

3.5 CLEANING

- A. Perform cleanup in accordance with Section 01 7400 - Cleaning and Waste Management and Section 01 7413 - Progress Cleaning.
- B. Upon completion, remove surplus materials, rubbish, tools and equipment in accordance with Section 01 7423 - Final Cleaning.
- C. Waste Management:
 - 1. Coordinate recycling of waste materials with Section 01 7419 - Construction Waste Management and Disposal.
 - 2. Collect recyclable waste and dispose of or recycle field generated construction waste created during demolition, construction or final cleaning.
 - 3. Remove recycling containers and bins from site.

END OF SECTION

SECTION 07 2600

UNDER-SLAB VAPOR RETARDER FOR CONCRETE SLABS-ON-GRADE

PART 1 – GENERAL

1.1 SUMMARY

- A. Products Supplied Under This Section
 - 1. Vapor Retarder, seam tape, mastic, pipe boots for installation under concrete slabs.

1.2 RELATED WORK SPECIFIED ELSEWHERE

- A. Cast-in-place Concrete - Section 03 3000
- B. Concrete Forming and Accessories - Section 03 2000
- C. Earthwork for Building Construction - Section 31 2311

1.3 REFERENCES

- A. American Society for Testing and Materials (ASTM), latest versions:
 - 1. ASTM E 96/ E96M Standard Test Methods for Water Vapor Transmission of Materials
 - 2. ASTM E 154 Standard Test Methods for Water Vapor Retarders Used in Contact with Earth Under Concrete Slabs
 - 3. ASTM E 1643 Standard Practice for Selection, Design, Installation and Inspection of Water Vapor Retarders Used in Contact with Earth or Granular Fill Under Concrete Slabs
 - 4. ASTM E 1745 Standard Specification for Plastic Water Vapor Retarders Used in Contact with Soil or Granular Fill Under Concrete Slabs
- B. American Concrete Institute (ACI), latest version
 - 1. ACI 302.2R, Guide for Concrete Slabs that Receive Moisture-Sensitive Flooring Materials.

1.4 SUBMITTALS

- A. Quality Control / Assurance
 - 1. Comply with Section 01 3300 – Submittal Procedures.
 - 2. Independent laboratory test results showing compliance with ASTM & ACI Standards.
 - 3. Manufacturer's samples, literature
 - 4. Manufacturer's installation instructions for placement, seaming and pipe boot installation
- B. Delivery, Storage, and Handling
 - 1. Deliver materials to site in manufacturer's original, unopened containers and packaging, with labels clearly identifying product name and manufacturer.
 - 2. Store materials in a clean dry area in accordance with manufacturer's instructions.
 - 3. Stack membrane on smooth ground or wood platform to eliminate warping.

4. Protect materials during handling and application to prevent damage or contamination.
 5. Ensure membrane is stamped with manufacturer's name, product name and membrane thickness at intervals of no more than 85" (220 cm).
- C. Environmental requirements
1. Product not intended for uses subject to abuse or permanent exposure to the elements.
 2. Do not apply on frozen ground.

PART 2 – PRODUCTS

2.1 MATERIALS

- A. Vapor Retarder (Performance-Based Specifications)
1. Vapor Retarder must have the following qualities at minimum and meet floor finish manufacturer's warranty requirements.
 - a. Water Vapor Retarder ASTM E1745: Meets or exceeds Class A
 - b. Maximum Permeance ASTM E96: 0.01 Perms or as required to meet Flooring Manufacturer's Warranties.
 - c. Tensile Strength ASTM E154, Section 9: not less than 45 LBS. Force/Inch
 - d. Puncture Resistance ASTM D1709, Method B.
 - e. Thickness of Retarder (plastic) ACI 302.1R-96: Not less than 15 mils
 - f. Material: Virgin Polyethylene or Polyolefin
 2. Vapor Retarder Products, may be by one of the following manufacturers or an approved equal, as long as the requirements above are met.
 - a. Epro, <http://eproserv.com>
 - b. Fortifiber, <http://www.fortifiber.com>
 - c. Stego Industries, <http://www.stegoindustries.com>
 - d. W.R. Meadows, <http://www.wrmeadows.com>
 - e. Raven Industries, <http://www.vaporblock.com>
 - f. Reef Industries, <http://www.reefindustries.com>

2.2 ACCESSORIES

- A. Seam Tape
1. Tape must have the following qualities:
 - a. Water Vapor Transmission Rate ASTM E 96 0.3 perms or lower
- B. Vapor Proofing Mastic
1. Mastic must have the following qualities:
 - a. Water Vapor Transmission Rate ASTM E 96 0.3 perms or lower
- C. Pipe Boots
1. Construct pipe boots from vapor Retarder material, pressure sensitive tape and/or mastic per manufacturer's instructions.

PART 3 – EXECUTION

3.1 EXAMINATION

- A. Examine surfaces to receive membrane. Ensure compaction requirements have been completed and geotechnical firm has confirmed compaction requirements have been met. Notify Architect if surfaces are not acceptable. Do not begin surface preparation or application until unacceptable conditions have been corrected.

3.2 SURFACE PREPARATION

- A. Prepare surfaces in accordance with manufacturer's instructions.

3.3 INSTALLATION

- A. Install Vapor Retarder:
 - 1. Installation shall be in accordance with manufacturer's instructions and ASTM E 1643.
 - a. Unroll Vapor Retarder with the longest dimension parallel with the direction of the pour.
 - b. Lap Vapor Retarder over footings and seal to foundation walls.
 - c. Overlap joints 6 inches and seal with manufacturer's tape.
 - d. Seal all penetrations (including pipes) per manufacturer's instructions.
 - e. No penetration of the Vapor Retarder is allowed except for reinforcing steel and permanent utilities.
 - f. Repair damaged areas by cutting patches of Vapor Retarder, overlapping damaged area 6 inches and taping all four sides with tape.

END OF SECTION

SECTION 31 11 00 - CLEARING AND GRUBBING

PART 1: GENERAL

1.1 SUMMARY

- A. Section Includes: Clearing of vegetation, and grubbing of stumps, roots, and debris; disposal of unutilized materials; and other incidental work related to preparing the site for later use.

1.2 DEFINITIONS

- A. Clearing: Clearing shall consist of the felling, trimming, and cutting of obstructions such as trees into sections and the satisfactory disposal of the trees and other surface vegetation designated for removal, including down timber, trees, brush, and rubbish occurring in the areas to be cleared.
- B. Grubbing: Grubbing shall consist of the removal and disposal of below-surface stumps, roots larger than 3-inches (75 millimeters) in diameter, and matted roots from the designated grubbing areas.
- C. Hazardous Waste: Substance likely to cause death or injury by reason of being explosive, flammable, poisonous, corrosive, oxidizing, irritating, or otherwise harmful; and includes, but is not limited to flammable dust, flammable fiber, combustible liquid, dangerous chemical, flammable gas, liquified flammable gas, and flammable liquid.

1.3 PROJECT/SITE CONDITIONS

- A. Work Limits: Area to be cleared and grubbed will be the excavation area or disturbed area identified by the project work. Scalping of topsoil during clearing operations will not be permitted.
- B. Burning of Debris: Shall not be permitted, unless specifically authorized. Contractor shall be responsible for obtaining all necessary permits, licenses and approvals from jurisdictions having authority.
- C. Landscape Preservation: Protect vegetation outside the work limits from injury. Existing trees and shrubs shall not be disturbed or damaged.

1.4 SUBMITTALS

- A. Tree Wound Paint: Submit three copies of manufacturer's literature and one container of tree wound paint that is to be used on this project.
- B. Herbicide: Submit three copies of manufacturer's literature and one container of herbicide to be used on this project.
- C. Records: Submit three copies of written permission to dispose of materials other than salable timber on private property.

PART 2: PRODUCTS

2.1 PREPARED PRODUCTS

- A. Tree Wound Paint: Bituminous based material of standard manufacture specially formulated for tree wounds.

- B. Herbicide: Comply with Federal Insecticide, Fungicide, and Rodenticide Act, Title 7 U.S.C. Section 136, for requirements on Contractor licensing, certification, and record keeping.

PART 3: EXECUTION

3.1 PROTECTION

- A. Roads and Walks: Keep roads and walks free of dirt and debris at all times. Damage to existing surfaces shall be repaired at no expense to the Owner.
- B. Utility Lines: Protect existing utility lines that are indicated to remain in service from damage. Notify the Owner's Representative immediately of damage to or an encounter with an unknown existing utility line. Contractor shall be responsible for contacting utility companies in the area to determine location of their respective utilities. When utility lines which are to be removed are encountered within the area of operations, the Contractor shall notify the Owner's Representative 72 hours prior to interruption of the service. Utility service interruption shall only occur after the approval of the Owner's Representative and the utility owner.

3.2 CLEARING

- A. Requirements: Clear trees, stumps, roots, brush, and other vegetation in areas to be graded; cut off flush with or below the original ground surface, except such trees and vegetation indicated or directed to be left standing. Trees designated to be left standing within the cleared areas shall be trimmed of dead branches 1-1/2 inches (40 mm) and shall be trimmed of all branches the heights indicated on the drawings. Limbs and branches to be trimmed shall be neatly cut close to the bole of the tree or main branches. Cuts more than 1-1/2 inches (40 mm) in diameter shall be painted with an approved tree-wound paint. Trees and vegetation to be left standing shall be protected from damage incident to clearing, grubbing, and construction operations by the erection of barriers or by such other means as the circumstances require. Clearing shall also include the removal and disposal of existing obstructions that obstruct, encroach upon, or otherwise obstruct the work.

3.3 GRUBBING

- A. Requirements: Material to be grubbed, together with logs and other organic or metallic debris not suitable for foundation purposes, shall be removed to a depth of not less than 18 inches (450 mm) below the original surface level of the ground in areas indicated to be grubbed and in areas indicated as construction areas under this contract, such as areas for buildings, and areas to be paved. Depressions made by grubbing shall be filled with suitable material and compacted to make the surface conform with the original adjacent surface of the ground.
- B. Low Embankment Areas: When the finished subgrade is less than 3 feet (1 m) above the original ground, remove stumps, roots, and debris to a minimum of 6 inches (150 mm) below the original ground. Backfill stump and root holes with approved material and compact before placing embankment material.
- C. High Embankment Areas: When the finished subgrade is 4 feet or more from the original ground, stumps may be cut flush and left in place. Removal of undisturbed stumps and roots and nonperishable solid objects will not be required. The surface of the original ground shall be scarified before starting the embankment operation.

3.4 EXPLOSIVES

- A. Requirements: The use of explosives is prohibited.

3.5 SALVAGE

Items to be salvaged, if any, shall be identified on the plan sheets.

3.6 DISPOSAL

- A. Requirements: Material that is not to be salvaged shall be removed from the project site and legally disposed of by a combination of burying and removal. Burning will not be permitted without approval.

END OF SECTION

SECTION 31 23 00 - EXCAVATION AND FILL

PART 1: GENERAL

1.1 SUMMARY

- A. Section Includes: Excavation, borrow excavation, embankment construction, placement, and disposal of materials as shown on the drawings.
- B. Related Sections
 - Testing Laboratory Services Section 01 45 29
 - Clearing and Grubbing Section 31 11 00

1.2 DEFINITIONS

- A. Unclassified Excavation: Consists of the material excavation and placement regardless of its nature. All excavation and fill shall be considered Unclassified unless types B through E, listed below, are specifically listed in the Bid Schedule.
- B. Rock Excavation: Removal of material is rock excavation when it consists of igneous, metamorphic, and sedimentary rock which cannot be excavated without blasting or the use of a tractor having a power rating in excess of 195 net horsepower, with a rear-mounted, heavy-duty, single-tooth, ripping attachment.
- C. Common Excavation: Removal of materials which can be excavated using a rear-mounted, heavy-duty, single-tooth ripping attachment mounted on a crawler tractor with a power rating of 195 net horsepower or less shall be considered common excavation.
- D. Muck Excavation: Shall consist of the removal and disposal of deposits of saturated or unsaturated mixtures of soils and organic matter not suitable for foundation material regardless of moisture content.
- E. Fill Material: Shall be suitable mineral soil free from organics, frozen material, brush, trees, roots over 2 inches (50 mm) in diameter and rocks over 6 inches (200 mm) in greatest diameter obtained from site during excavation. Fill material quality may be specified to meet certain project requirements. Refer to plan notes and project geotechnical report for specific requirements.
- C. Topsoil: Surface soil approximately 6 inches (150 mm) in depth that supports growth of vegetation and contains organic matter. Topsoil shall be free from subsoil, debris, and stones larger than 1 inch (25 mm) in diameter.

1.3 PROJECT/SITE CONDITIONS

- A. Excess Material: Usable excess material excavated shall be used in the embankment construction before the use of borrow is allowed. Borrow wasting is not permitted.
- B. Borrow Material Source: Furnish the borrow material source, unless an a source or sources shown on the drawings, subject to use restrictions or requirements as noted.
- C. Fencing: When fencing is removed, replace the fencing to the same condition as it was before removal. The Contractor shall be responsible for the livestock confinement when a portion of a fence is removed.

- D. Drainage of Borrow Pits: Borrow pits, gravel pits, quarry sites, and waste or disposal areas shall be excavated so that water will not collect and stand.

PART 2: PRODUCTS

2.1 MATERIALS

- A. General: See definitions.
- B. Fill Material: Shall be obtained from the unclassified excavation, or from approved borrow areas. Material from excavation shall be used unless it contains frozen earth, debris, does not comply with any quality restrictions, or is specified in other sections to be replaced. Materials removed in clearing and grubbing shall not be used for backfill or embankment.

PART 3: EXECUTION

3.1 PREPARATION

- A. Clearing and Grubbing: The area to be excavated, and the surface area to be covered by fill material, shall be thoroughly cleared and stripped of vegetative matter, brush, trees, stumps, roots, loose rocks, and other objectionable materials including sand, gravel, silt, and debris in channels within the foundation areas. Dispose of debris.
- B. Borrow Areas: Notify the Owner's Representative sufficiently(14 Days) in advance of opening borrow areas. This allows cross section determination of elevations and measurements of the ground surface after stripping. also, the borrow material can be tested before being used. Allow sufficient time for testing the borrow(14 Days). Borrow areas shall be bladed and left in such shape as to permit accurate measurements after excavation has been completed. Do not excavate beyond the dimensions and elevations established, and no material shall be removed prior to staking out and cross-sectioning of the site. The finished borrow areas shall be approximately true to the line and grade established by agreement.
- C. Prewatering: Excavation areas and borrow pits may be prewatered before excavating the material. The area to be excavated shall be moistened to the full depth from the surface to the bottom of the excavation. The application of water shall be controlled so that the excavated material will be near the optimum moisture content as specified in paragraph 3.3A Testing, below. When necessary, prewatering shall be supplemented to assure that embankment material and backfill material moisture content comply with paragraph 3.3A Testing, below.
- D. Preparation for Grading: Prior to beginning embankment operations in an area, necessary clearing and grubbing in that area shall have been completed and accepted by the Owners Representative. No embankment materials shall be placed upon an unapproved surface.
 - 1. Embankment Greater Than 4 feet (1200 mm) High: When embankments greater than 4 feet (1200 mm) are to be constructed, the cleared surface shall be completely broken up by plowing or scarifying to a minimum depth of 6 inches (150 mm). Scarifying or furrows shall be a maximum of 3 feet (900 mm) apart, and shall be parallel to the centerline of the embankment. This area shall then be compacted beginning with the first lift, as specified in paragraphs 3.3B Moisture Content and 3.3C Compaction.
 - 2. Embankment 4 feet (1200 mm) High or Less: When embankments 4 feet (1200 mm) high or less are to be constructed, sod and vegetation shall be removed from the surface upon which the embankment is to be placed, and the surface of the original ground shall be scarified as described in paragraph 3.1D1, and shall be compacted as described in paragraphs 3.3B Moisture Content and 3.3C Compaction, before starting the embankment operation.

3.2 INSTALLATION

- A. Explosives: The use of explosives is prohibited.

- B. Excavation: Excavate where shown on the drawings and typical sections, unless staked otherwise. Excavate on a straight grade between the control points shown on the drawings unless staked otherwise. Existing structures which are disturbed or damaged by construction activity shall be reset, repaired, or replaced at the Contractor's expense.
- C. Subgrade Compaction: In areas where earthfill, or granular embankments are to be constructed and in areas where excavation exposes the subgrade surface upon which improvements will be constructed, the subgrade shall be compacted as specified in paragraphs 3.3B Moisture Content and 3.3C Compaction below, before embankment construction begins.
- D. Disposal of Materials: Excess and unsuitable material, including rock and boulders that cannot be used in embankments, shall be disposed of off-site unless otherwise approved by the Owners Representative.
- E. Use of Borrow Material: Do not use borrow material until after the suitable materials obtained from required excavation have been placed in the fill, unless otherwise directed by the Owners Representative. When more borrow is placed than is required, and required excavation is wasted, the waste shall be replaced in the borrow area at the Contractor's expense.
- F. Overburden: Overburden shall be stripped from borrow pits, aggregate pits, and quarries, and stockpiled for later use. After a pit has served its purpose, waste material outside of the pit shall be moved back into the pit. The pit shall be neatly sloped and trimmed. Side slopes shall be flattened to a 4:1 slope. Stockpiled overburden material shall then be spread uniformly over the sides and bottom of the pit area, and vegetation established.
- G. NOT USED
- H. Fill Material: Slope surfaces shall be horizontally benched before placement of fill on slopes. Fill shall be spread over the full width of the cross section of the embankment to a maximum loose thickness of 8 inches (200 mm), and shall be compacted as specified in paragraphs 3.3B Moisture Content and 3.3C Compaction below, before the next layer is spread. Use sheepsfoot, vibratory, pneumatic tire, or smooth-steel wheel rollers to obtain the required degree of compaction. The in-place materials shall be free of lenses, pockets, streaks, or layers of materials differing substantially in texture or gradation from the surrounding materials. Proof rolling shall be performed as indicated on the plans.
- I. Frozen Material: Shall not be placed, nor be placed upon a frozen surface.

3.3 FIELD QUALITY CONTROL

- A. Testing: Furnish necessary equipment, labor, and materials to conduct the testing. Testing shall be conducted in the presence of the Owners Representative who shall be given 48 hours notice before any test is to be conducted. Arrange for a certified independent testing laboratory, according to the requirements of Section 01 45 29 - Testing Laboratory Services, to perform the required testing, recording, and distributing of the results.
- B. Moisture Content: During placing and compacting of fill material, the optimum moisture content (-2 percent to +2 percent) as determined by AASHTO T 217 or AASHTO T 239, unless otherwise approved, shall be maintained by wetting or drying. Moisture range may vary depending upon material. Contractor shall place material in accordance with recommendations provided by Geotechnical Engineers recommendations, if available.
- C. Compaction: The fill material shall be compacted to a minimum density of 95 percent of the maximum dry density as determined by AASHTO T 99, Method C unless noted otherwise.

END OF SECTION

SECTION 31 2311

EARTHWORK FOR BUILDING CONSTRUCTION

PART 1 - GENERAL

1.1 WORK INCLUDED

- A. The work covered by this Section consists of furnishing all plant, labor, equipment, appurtenances and material in performing all operations, hauling, placing, spreading, watering, processing, compacting and shaping earth sections, within the building limits, complete in place in accordance with the Project Manual and Drawings.

1.2 RELATED WORK ELSEWHERE

- A. Clearing - Section 31 1000
- B. Under-Slab Vapor Retarder – Section 07 2600
- C. General Foundation Notes on Drawings.
- D. Project Soils Report – shall be completely reviewed and understood by the contractor. In case of conflict or omission, the Project Soils Report shall govern.

1.3 SUBSURFACE SOIL DATA

- A. Subsurface soil investigations have been made and the results are available for examination by the Contractor. This is not a warranty of conditions the Contractor is expected to examine the site and determine for himself the character of materials to be encountered.
- B. No additional allowance will be made for rock removal, site clearing and grading, filling, compaction, disposal, or removal of any unclassified materials.

1.4 REFERENCES

- A. ASTM International, latest version:
 - 1. ASTM D 1556 Standard Test Method for Density of Soil in Place by the Sand-Cone Method
 - 2. ASTM D 698 Standard Test Methods for Laboratory Compaction Characteristics of Soil Using Standard effort (12,400 ft-lbf/ft³(600Kn-M/M)³)
 - 3. ASTM D 4318 Test Methods for Liquid Limit, Plastic Limit, and Plasticity Index of Soils
 - 4. ASTM D 6938 Standard Test Method for In-Place Density and Water Content of Soil and Soil-Aggregate by Nuclear Methods (Shallow Depth)

1.5 SUBMITTALS

- A. Submit copies of materials certificates and test results for materials in accordance with type of tests, frequencies and remarks as outlined in the sampling and testing schedule.

1.6 TESTING AND INSPECTION

- A. General: The Owner shall employ the services of a registered, licensed Geotechnical Engineer to observe all controlled earthwork soil testing. The testing laboratory shall provide continuous on-site observation by experienced personnel during construction of fill material. The Contractor shall notify the testing laboratory at least two working days in advance of any field operations of controlled earthwork, or of any resumption of operations after stoppages.
- B. Report of Field Density Tests
 - 1. The Geotechnical Engineer shall submit, daily, the results of field density tests required by these specifications.
- C. Costs of Tests and Inspection
 - 1. The cost of testing, inspecting and engineering, as specified in this section of the specifications, shall be borne by the Owner.
- D. Lines and Grades: Alignment and grade of all elements shall be made on true tangents and curves. Grades shall conform to the elevations indicated on Drawings, with minor adjustments, to provide a smooth approach at building lines, at connections to existing paving and to provide proper drainage. Correct irregularities at no cost to the Owner.

1.7 WEATHER LIMITATIONS

- A. Controlled fill shall not be constructed when the atmospheric temperature is below 35 degrees F. When the temperature falls below 35 degrees, it shall be the responsibility of the Contractor to protect all areas of completed work against any detrimental effects of ground freezing by methods approved by the testing laboratory. Any areas that are damaged by freezing shall be reconditioned, reshaped, and compacted by the Contractor in conformance with the requirements of this specification without additional cost to the Owner.

PART 2 - PRODUCTS

2.1 STRUCTURAL FILL MATERIAL

- A. On-site soils are **NOT** suitable for use as structural fill within the proposed building area.
- B. Material shall consist of soils that conform to the geotechnical report and as summarized with the following physical characteristics:

Sieve Size Sq. Openings	Percent Passing By Weight
3 inch	100
No. 200	> 15%.

- C. Liquid Limit < 40; Plasticity Index <18; Maximum Dry Density >100pcf
- D. Lower plasticity structural fill: Lean Clay, Clayey Sand or Shale.

PART 3 - EXECUTION

3.1 PREPARATION

- A. Clearing and Grubbing: Prior to placing structural fill all borrow areas and areas to receive structural fill shall be stripped of vegetation and deleterious materials. Strippings shall be

hauled offsite or stockpiled for subsequent use in landscaped areas or non-structural fill areas as designated by the Owner or his representative and approved by the Geotechnical Engineer.

3.2 CONSTRUCTION AREA TREATMENT

- A. Site Preparation - Fill Areas: Prior to placing structural fill the areas to be filled shall be scarified to a depth of eight inches and moisture conditioned as described below. The area to be filled shall then be compacted to a minimum of 95 percent of maximum density as determined in accordance with ASTM D 698. Any soft or "spongy" areas shall be removed as directed by the Geotechnical Engineer and replaced with structural fill as described herein.
- B. Site Preparation - Cut Areas: Following excavation to rough grade all building and pavement areas shall be scarified to a depth of eight inches and moisture conditioned as described below. All building and paved areas shall be compacted to a minimum of 95 percent of maximum density as determined by ASTM D 698.

3.3 EQUIPMENT AND METHODS

- A. In areas not accessible to heavy equipment, distribute by and compact with hand operated vibratory compactors.

3.4 BORROW

- A. The Contractor shall provide sufficient material for fill to the lines, elevations and cross sections as shown on the contract drawings from borrow areas.
- B. The Contractor shall obtain from the Owners of said borrow areas the right to excavate material, shall pay all royalties and other charges involved, and shall pay all expenses in developing the source including the cost of right-of-way required for hauling the material.

3.5 COMPACTION

- A. Fill shall be spread in layers not exceeding 8 inches, watered as necessary, and compacted. Moisture content at time of compaction shall plus/minus 2 percent of optimum moisture. A density of not less than 95 percent of maximum dry density shall be obtained within the building pads.
- B. Optimum moisture content and maximum dry density for each soil type used shall be determined in accordance with ASTM D 698.
- C. Compaction of the fill shall be by mechanical means only. Where vibratory compaction equipment is used, it shall be the Contractor's responsibility to ensure that the vibrations do not damage nearby buildings or other adjacent property. Where vibratory compaction is not possible, pneumatic rolling equipment shall be used.

MATERIAL	MINIMUM PERCENT COMPACTION
Structural & granular fill in construction area	95
Subgrade below structural fill	95
Structural fill under exterior walls	95
Miscellaneous backfill	90

3.6 MOISTURE CONTROL

- A. The material, while being compacted, shall be within the moisture range of 2 percent below to 2 percent above optimum, well distributed throughout the layer.

3.7 DENSITY REQUIREMENTS

- A. Density of undisturbed soils, in-place fill and backfill shall be determined in accordance with the procedures of ASTM D 1556 or ASTM D 6938. If tests indicate that the density of in-place soil is less than required, the material shall be scarified, moistened or dried as necessary to obtain proper moisture content and recompacted as necessary to achieve the proper densities. Sufficient density tests shall be made and reports submitted by the Testing Laboratory indicating all cut and fill areas were compacted and graded in accordance with the requirements.

3.8 SLOPE PROTECTION & DRAINAGE

- A. Berming and grading shall be done as may be necessary to prevent surface water from flowing into and out of the construction area. Any water accumulating therein shall be removed by pumping or by other methods.

3.9 SOIL EROSION PROTECTION

- A. The Contractor shall ensure that no soil erodes or blows from the site into public right-of-way or onto private property.
- B. The Contractor shall promptly clean up any material which erodes or blows into the public right-of-way or onto private property.

3.10 PRESERVATION OF PROPERTY

- A. Provide temporary fences, barricades, coverings, or other protections to preserve existing items indicated to remain and to prevent injury or damage to persons or property. Apply protections to adjacent properties as required.
- B. Restore damaged work to condition existing prior to start of work, unless otherwise directed.

3.11 EXISTING UTILITIES

- A. The Contractor shall verify the location of any utility lines, pipelines, or underground utility lines in or near the area of the work in advance of and during Earthwork. The Contractor is fully responsible for any and all damage caused by failure to locate, identify and preserve any and all existing utilities, pipelines and underground utility lines. Repair damaged utilities to the satisfaction of the utility owner at no expense to the Owner.
- B. Should uncharted or incorrectly charted piping or other utilities be encountered during grading, consult the Architect immediately for directions as to procedures.
- C. Cooperate with the Owner and public or private utility companies in keeping service and facilities in operation.

3.12 WASTE

- A. Dispose of all waste off Owner's property.
- B. Burning of waste will not be permitted.

3.13 AIR POLLUTION

- A. Use water sprinkling, temporary enclosures, and other suitable methods to limit dust and dirt air pollution. Comply with governing regulations pertaining to environmental protection.

**SAMPLING AND TESTING SCHEDULE
FOR EARTHWORK**

FIELD QUALITY CONTROL

MATERIAL	TEST FOR	FREQUENCY	REMARKS
NATURAL GROUND	Compaction in accordance with ASTM D 1556 or ASTM D 6938	1 per 2500 square feet of surface	Conduct a minimum of 3 tests on each section.
EMBANKMENT AND/OR SUBGRADE	Soil Conditions Moisture-Density in accordance with ASTM D 698	Test 1 per soil classification	
	Compaction control in accordance with ASTM D 1556 or ASTM D 6938	1 per each lift every 2500 square feet of surface	Immediately after placing, Conduct a minimum of 3 tests per section
		1 per each lift for each 2500 square feet of fill	

END OF SECTION

SECTION 31 25 00-EROSION CONTROL

1.0 GENERAL

1.1 DESCRIPTION

The work under this section consists of providing all work, materials, labor, equipment, and supervision necessary to provide and construct erosion control measures necessary to protect property and the environment. Included are the following topics:

Provide erosion & stormwater control in accordance with the following references:

- Oklahoma Department of Environmental Quality Stormwater General Permit for Construction Activities, Latest revision or amendment;

Or for projects on Federal/Indian Trust Lands,

- U.S. EPA Construction General Permit
<http://cfpub.epa.gov/npdes/stormwater/cgp.cfm> , Latest revision or amendment;

1.2 SUBMITTALS & DOCUMENTATION

The Contractor will submit or provide the following to the Owner:

- Construction Site Erosion Control Plan, if required. Contractor shall mark-up or create additional versions of the Erosion Control Plan that is included in these documents showing additional or alternate erosion control measures as needed due to the Contractors means and methods throughout all phases of construction.
- Submittals for materials used to implement the erosion control plan.

1.3 EROSION CONTROL PLAN/SWP3

For project disturbing one acre of land or more:

The contractor shall provide a Stormwater Pollution Prevention Plan (SWP3) in accordance with Oklahoma Department of Environmental Quality and/or Environmental Protection Agency stormwater requirements. The contractor shall complete and submit any required documents, including but not limited to the Notice of Intent(NOI), Notice of Termination(NOT) to applicable agency having jurisdiction.

Contractor shall be responsible for insuring the completion of the SWP3 and compliance with the applicable General Permit.

Contractor shall comply with all the requirements of the erosion control plan, and the requirements of the General Permit to Discharge. Erosion control and storm water management practices shall be installed and maintained in accordance with the erosion control plan and General Permits.

Contractor shall provide all erosion control practices necessary to protect property and the environment. Erosion control and storm water management practices shall be installed and maintained in accordance with the erosion control plan and the applicable General Permit. The contractor shall update and modify the erosion control plan as needed for phasing of work. A copy of the current erosion control plan shall be maintained at the project site.

2.0 MATERIALS

2.1 GENERAL

When the design or contract includes permanent erosion control or stormwater control features, the contractor may employ these items to control erosion and stormwater during construction activities. However, these features shall be fully cleaned and restored to the original design providing full function for the intended permanent use prior to acceptance of the work.

2.2 TEMPORARY SILT DIKES

Temporary silt dikes shall be triangular-shaped, having a height of at least eight to ten inches (8" - 10") in the center with equal sides and a sixteen- to twenty-inch (16" - 20") base. The triangular-shaped inner material shall be urethane foam. The outer cover shall be a woven geotextile fabric placed around the inner material and allowed to extend beyond both sides of the triangle two to three (2' - 3') feet. Standard length of each dike will be seven feet (7') unless otherwise indicated on the plans. The dikes shall be attached to the ground with wire staples. The staples shall be No. 11 gauge wire and be at least six to eight (6" - 8") inches long. Staples shall be placed as indicated on the installation detail.

2.3 SILT SCREEN FENCE

Fabric shall be a woven polypropylene geotextile fabric

Silt screen fences shall be installed at least 6 feet from the right-of-way or property line, unless otherwise noted. Framework supports shall have a maximum spacing of 10 feet center to center, or as shown on the construction plans. Filter material shall be secured to the ground between supports to prevent the material from curling or rolling up. Silt screen fences may be removed or left in place as determined by the Engineer.

Silt screen fences shall be kept free from debris and sediment build up. Sediment removal shall be removed at the direction of the Engineer. Material removed shall be disposed of in a manner and location approved by the Engineer. After the sediment removal has been completed, the control devices shall be left in operable condition. Any damage to the control devices shall be replaced at the Contractor's expense.

2.4 EROSION MAT

Erosion mat shall consist of a machine produced wood excelsior mat with interlocking fibers that form a continuous web. Cover the mesh on one side with an extruded plastic netting with mesh size no greater than 1" x 2".

2.5 STAPLES

Permanent installations: Use biodegradable staples in accordance with manufacturer's recommendations for materials being anchored. Wood and metal staples are not allowed for permanent installations.

2.6 RIPRAP

Riprap shall be the class specified and shall conform to Oklahoma Department of Transportation Standard Specifications for Highway Construction Section 713, Type I Riprap. 12 inch stone size.

2.7 COMPOST FILTER SOCK

Compost filter sock is a biodegradable wood compost filled tube used to prevent erosion. Provide filter sock in accordance with AASHTO specification MP 9-06. Minimum 12 inch size.

2.8 TRACKING PAD STONE

The aggregate for tracking pads shall be 3 to 6 inch clear or washed stone. All material shall be retained on a 3-inch sieve.

- 2.9 SOIL STABILIZERS
Soil stabilizers shall be non-asphalt-based products of the type approved by Owner.
- 2.10 SOIL TACKIFIERS
Soil tackifiers shall be non-asphalt-based products of the type approved by Owner.
- 2.11 POLYMERS
Polymers used to settle suspended sediment shall meet the requirements of the WDNR Technical Standards.

3.0 EXECUTION

3.1 GENERAL

Install and maintain erosion control measures as required by the erosion control plan throughout phases of the project. Notify Construction Representative of modifications to the erosion control plan as dictated by Contractor's means and methods, construction phasing or by differing site conditions.

Contractor shall provide all erosion control measures necessary to prevent and manage polluted runoff from the construction site and discharge of sediment onto adjacent property, into storm sewers or waters of the state.

Perform all work in accordance with manufacturer's instruction where these specifications do not specify a higher requirement.

3.2 GRADING AND EARTHWORK

Install temporary or permanent erosion control measures applicable to each phase of grading or land disturbance prior commencing on that phase.

Clear only those areas designated for the placement of improvements or earthwork before placement of the final cover. Perform stripping of vegetation, grading, excavation, or other land disturbing activities in phases to minimizing exposure of bare soil. Do not clear the site of topsoil, trees, and other natural ground covers before the commencement of construction. Retain natural vegetation and protect until the final ground cover is placed.

Do not stockpile soil within 25 feet of any roadway, parking lot, paved area, or drainage structure or channel. Provide temporary stabilization and erosion control measures on disturbed areas and soil stockpiles which will remain for a period of more than 7 consecutive calendar days.

Remove surplus excavation materials from the site immediately after rough grading. The disposal site for the surplus excavation materials shall also be subject to these erosion control requirements.

3.3 DRAINAGE

Divert roof drainage and runoff from all undisturbed areas upslope of the site around disturbed areas. Minimize runoff on exposed soil. Provide measures to remove sediment, and debris.

Convey clean or treated runoff to the nearest adequate stormwater facility. Do not discharge water in a manner that will cause erosion or sedimentation of the site or receiving stormwater facility.

Protect storm sewer inlets and catch basins with inlet protection devices.

Provide ditch checks or silt dikes in swales or ditches to reduce the velocity of water in the channel.

Dewatering discharge shall be routed to a sedimentation basin or sedimentation vessel to reduce the discharge of sediments. Do not discharge water in a manner that will cause erosion or sedimentation of the site or receiving stormwater facility.

3.4 TRACKING CONTROL

Construct and maintain Tracking Pads in accordance with the Technical Standards. Provide each entrance to the site with a stone tracking pad at least 50 feet in length with a minimum thickness of 12 inches. The tracking pad shall be the full width of the egress point. Inspect tracking pads on a daily basis and replace aggregate when no longer effective.

If necessary, provide a crushed aggregate paved parking area.

If applicable, wash water shall be discharged to sedimentation basins, sedimentation vessels, or other such control areas. Untreated wash water shall not be routed to storm sewers or waters of the state.

3.5 MAINTENANCE

Contractor shall inspect all erosion control measures within 24 hours of the end of each rainfall event that exceeds 0.25", or daily during periods of prolonged rainfall, or weekly during periods without rainfall. Immediately repair and/or replace any and all damaged, failed, or inadequate erosion control measures.

Re-apply soil stabilizers, tackifiers, polymers and anionic polyacrylamides as needed to prevent erosion of exposed soil.

Maintain records of all inspections and any remedial actions taken on-site.

Remove any sediment reaching a public or private roadway, parking lot, sidewalk, or other pavement. Do not remove tracked sediments by flushing. Completely remove any accumulations not requiring immediate attention at least once daily at the end of the workday.

Frequently dispose of all waste and unused construction materials in licensed solid waste or wastewater facilities. Do not bury, dump, or discharge, any garbage, debris, cleaning wastes, toxic materials, or hazardous materials on the site, on the land surface or in detention basins, or otherwise allow materials to be carried off the site by runoff onto adjacent lands or into receiving waters or storm sewer systems.

SECTION 31 25 73 – TEMPORARY SILT FENCE

PART 1 - GENERAL

1.1 RELATED DOCUMENTS

- A. Drawings, Bidding Requirements, Contract Forms, Conditions of the Contract and Division 1 - General Requirements apply to the work of this section.

1.2 DESCRIPTION OF WORK

- A. This work shall consist of furnishing, installing, maintaining and removing a barrier-fence designed to remove suspended particles from the water passing through it. The quantities of temporary silt fence shown on the plans may be increased or decreased based on weather, construction procedures, and actual site conditions that occur during construction of the project. Such variations in quantity will not be considered as alterations in the details of construction or a change in the character of the work.
- B. The contractor shall install a silt fence construction of geotextiles. Hay-bales may be substituted if approved by Engineer.

PART 2 - PRODUCTS

2.1 HAY BALES

- A. Rectangular, commercially sized bales, with densely packed hay, securely bound with wire or plastic strips.

2.2 GEOTEXTILES

- A. This specification provides criteria for wire-supported geotextile silt fence as well as a self-supporting geotextile silt fence.
- B. Fibers used in the manufacture of geotextiles shall consist of long-chain synthetic polymers, composed of at least 85% by weight polyolefins, polyesters, or polyamides. They shall be formed into a network such as that the filaments or yarns retain dimensional stability relative to each other, including selvages. The geotextile shall be specific for its intended purpose. The geotextile shall be free from any treatment or coating which might adversely alter its physical properties after installation.
- C. Geotextile rolls shall be furnished with suitable wrapping for protection against moisture and extended ultraviolet exposure prior to placement. Each roll shall be labeled or tagged to provide product identification sufficient for inventory and quality control purposes. Rolls shall be stored in a manner which protects them from the elements.
- D. Posts: Either wood, steel or synthetic posts may be used. Posts shall have a minimum length of 36 in. plus burial depth and be of sufficient strength to resist damage during installation and to support applied loads.
- E. Support Fence: Wire or other support fences shall be at least 32 in. high and strong enough to support applied loads.
- F. Prefabricated Fence: Prefabricated fence systems may be used provided they meet all of the above material requirements.

PART 3 - EXECUTION

3.1 CONSTRUCTION REQUIREMENTS

The Contractor shall install a temporary silt fence as shown on the plans, and at other locations as required. Fence construction shall be adequate to handle the stress from sediment loading.

- A. Hay-Bales: Shall be installed per the details shown on the plans.
- B. Geotextile Fence: Shall be installed per the details shown on the plans.
- C. It is the Contractor's responsibility to maintain the integrity of silt fences as long as they are necessary to contain sediment runoff. The Contractor shall inspect all temporary silt fences immediately after each rainfall and at least daily during prolonged rainfall. Any deficiencies shall be immediately corrected by the Contractor. In addition, the Contractor shall make a daily review of the location of the silt fences in areas where construction activities have changed the natural contour and drainage runoff to ensure that the silt fences are properly located for effectiveness. Where deficiencies exist, additional silt fences shall be installed. Should the silt fence become damaged or otherwise ineffective while the barrier is still necessary, it shall be repaired promptly.
- D. Sediment deposits shall either be removed when the deposit reaches approximately one-half of the height of the silt fence or a second silt fence shall be installed.
- E. The silt fence shall remain in place as long as needed. Upon removal, the Contractor shall remove and dispose of any excess silt accumulations, dress the area to give a pleasing appearance, and vegetate all bare areas. The fence materials will remain the property of the Contractor and may be used at other locations provided the materials meet their intended purpose.

END OF SECTION

SECTION 31 6613

STONE COLUMNS/AGGREGATE PIERS

PART 1 - GENERAL

1.1 RELATED DOCUMENTS

- A. Drawings and general provisions of Contract, including Supplemental General Conditions which apply to work of this section.

1.2 SUMMARY

- A. Extent of stone columns/aggregate piers is to be determined by the installer of the piers and shown on drawings, including locations, diameters of shafts, bottom elevations, and details of construction.

1.3 SUBSURFACE SOIL DATA

- A. Subsurface soil investigations have been made and the results are available for examination by the Contractor. The Contractor is expected to examine the site and determine for himself the character of the materials to be encountered.

1.4 SUBMITTALS

- A. Reports: Submit the following reports directly to Engineer, with copy to others as designated.
 - 1. Submit qualifications of installer of stone columns/aggregate piers. Installer shall have not less than three years experience installing similar piers in similar site conditions.
 - 2. Calculations and Design of stone columns/aggregate piers, including number, diameter, length and location of piers necessary to provide allowable bearing at bottom of column and wall footings of 6,000 psf with a calculated maximum long-term settlement not to exceed 1-inch total and 1/2-inch differential.

1.5 JOB CONDITIONS

- A. Site Information: Data on indicated subsurface conditions are not intended as representations or warranties of continuity of such conditions. It is expressly understood that Owner will not be responsible for interpretations or conclusions drawn there from by Contractor. Data are made available for convenience of Contractor and are not guaranteed to represent conditions that may be encountered.
- B. Additional test borings and other exploratory operations may be made by Contractor at no additional cost to Owner.

1.6 EXISTING UTILITIES

- A. Locate existing underground utilities by careful hand excavation before starting pier excavation operations. If utilities are to remain in place, provide protection from damage during drilled pier operations.
- B. Should uncharted or incorrectly charted piping or other utilities be encountered during excavation, consult Engineer immediately for directions as to procedure. Cooperate with Owner and or utility companies in keeping services and facilities in operation. Repair damaged utilities to satisfaction of utility Owner.

- C. Do not interrupt existing utilities serving facilities occupied and used by Owner or others, except when permitted in writing by Owner and after acceptable temporary utility services have been provided.

PART 2 - PRODUCTS

Not Used.

PART 3 - EXECUTION

3.1 DRILLED PIER EXCAVATION

- A. General: Excavate holes for stone columns/aggregate piers to elevation as shown on submittal-drawings. Stone columns/aggregate pier design dimensions shown on submittal drawings are minimums.
- B. Obstructions: If rock, boulders, or other unforeseen obstructions are encountered which cannot be removed by standard pier excavation methods, and if such obstructions are not indicated by available subsurface data, removal of such obstructions will be paid for in accordance with terms of contract relative to changes in work.

3.2 FIELD QUALITY CONTROL

- A. Quality Control Testing During Construction: Quality control during construction shall be the responsibility of the installation Contractor:
 - 1. The installer shall conduct a "full-scale" load test performed in the field and monitored by a representative of the geotechnical engineer. Provide a report in writing of the load test result within two days of the load test to the Architect and Engineer for review.
 - 2. The geotechnical engineer and/or their representative shall monitor the subsequent production stone columns/aggregate pier installations. Report results in writing to Engineer and Contractor on the day after installations are made. Include in reports project identification name and number, date of pier construction.

END OF SECTION



AIA[®] Document G710[™] – 2017

Architect's Supplemental Instructions

PROJECT: *(name and address)*
Wilma P. Mankiller Health Center
Expansion
Stilwell, OK

CONTRACT INFORMATION:
Contract For: CMAR

Date:

ASI INFORMATION:
ASI Number: Bid Package 01 - ASI 001

Date: 01/10/20

OWNER: *(name and address)*
Cherokee Nation Property Management,
LLC

ARCHITECT: *(name and address)*
James R Childers Architect Inc.

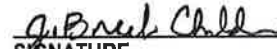
CONTRACTOR: *(name and address)*
M Ross Inc

The Contractor shall carry out the Work in accordance with the following supplemental instructions without change in Contract Sum or Contract Time. Proceeding with the Work in accordance with these instructions indicates your acknowledgment that there will be no change in the Contract Sum or Contract Time.
(Insert a detailed description of the Architect's supplemental instructions and, if applicable, attach or reference specific exhibits.)
See attached narrative from Chavez Grieves

ISSUED BY THE ARCHITECT:

James R Childers Architect Inc.

ARCHITECT *(Firm, name)*



SIGNATURE

Breck Childers

PRINTED NAME AND TITLE

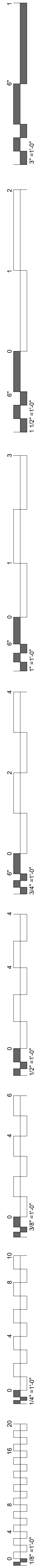
01/10/20

DATE

Bid Package 01- ASI 01 – Wilma P. Mankiller Health Center Expansion

Chavez-Grieves would like to incorporate the following revisions into the drawings for the above referenced project.

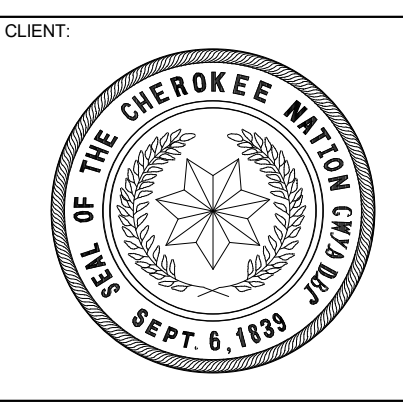
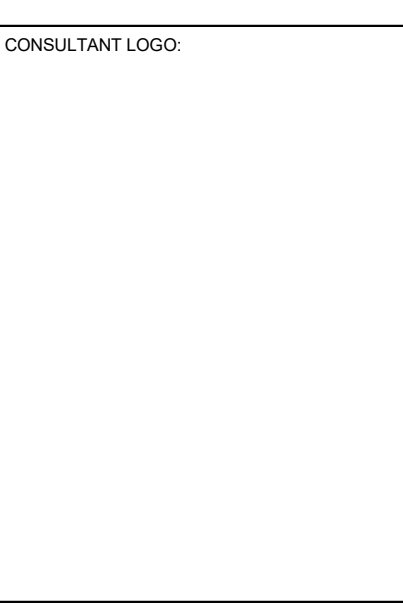
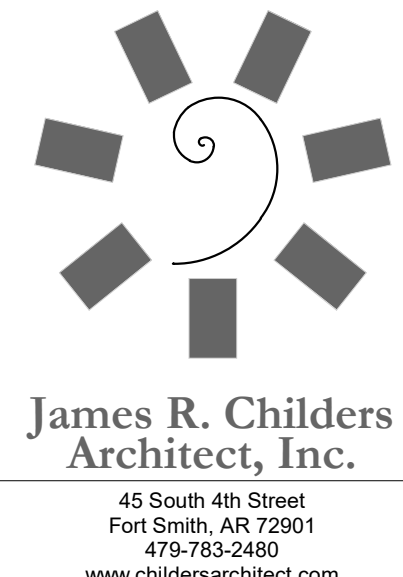
<u>Sheet</u>	<u>Description</u>
S2.01	SidePlate connection designations updated for elevations A4, A5, and C4.
S8.01	SidePlate information updated.
S8.02	SidePlate information updated.
S8.04	SidePlate information updated.
S8.06	SidePlate information updated.
S8.07	SidePlate information and sheet title updated.
S8.08	Sheet added.



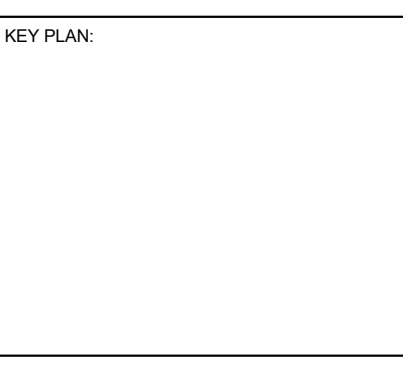
WILMA P. MANKILLER HEALTH CENTER EXPANSION

BID PACKAGE 01 (DEMOLITION / STEEL / FOUNDATIONS)

SHEET NUMBER	SHEET NAME	11-01-19 - BID PACKAGE 01	11-22-19 - BID PACKAGE 01 - ADDENDUM 01	12-10-19 - BID PACKAGE 01 - ADDENDUM 02	01-10-20 - BID PACKAGE 01 - AS01
GENERAL					
G0.01	COVER / INDEX				
CIVIL					
C002	GENERAL NOTES				
CS100	EXISTING SITE PLAN				
CS101	DEMOLITION PLAN				
CS102	DEMOLITION PLAN				
CE100	EROSION CONTROL SITE PLAN				
CE500	EROSION CONTROL DETAILS				
ARCHITECTURAL					
A0.01	OVERALL BUILDING DEMOLITION PLAN				
STRUCTURAL					
S0.01	ABBREVIATIONS AND LEGENDS				
S0.02	GENERAL STRUCTURAL NOTES				
S0.03	GENERAL STRUCTURAL NOTES AND SPECIAL INSPECTIONS				
SD0.01	DEMOLITION GENERAL STRUCTURAL NOTES				
SD1.01	DEMOLITION PLANS - SECTOR 1				
SD2.01	DEMOLITION SECTIONS				
S1.00	OVERALL PLAN - FOUNDATION				
S1.01	FOUNDATION PLAN SECTOR 1				
S1.02	FOUNDATION PLAN SECTOR 2				
S1.10	OVERALL PLAN - FLOOR FRAMING				
S1.11	FLOOR FRAMING PLAN - SECTOR 1				
S1.12	FLOOR FRAMING PLAN - SECTOR 2				
S1.13	LOW ROOF FRAMING PLAN				
S1.20	OVERALL PLAN - ROOF FRAMING				
S1.21	ROOF FRAMING PLAN - SECTOR 1				
S1.22	ROOF FRAMING PLAN - SECTOR 2				
S2.01	MOMENT FRAME AND BRACED FRAME ELEVATIONS				
S3.01	WALL SECTIONS				
S3.02	WALL SECTIONS				
S3.03	WALL SECTIONS				
S3.04	WALL SECTIONS				
S3.11	FOUNDATION SECTIONS				
S3.12	FOUNDATION SECTIONS				
S3.21	FLOOR FRAMING SECTIONS				
S3.31	ROOF FRAMING SECTIONS				
S4.01	ENLARGED PLANS				
S5.21	MASONRY FRAMING SECTIONS AND DETAILS				
S6.41	VERTICAL CIRCULATION DETAILS				
S5.51	STEEL DETAILS				
S5.52	STEEL DETAILS				
S5.53	STEEL DETAILS				
S5.54	STEEL DETAILS				
S6.01	SCHEDULES				
S7.11	TYPICAL CONCRETE DETAILS				
S7.21	TYPICAL MASONRY DETAILS				
S7.31	TYPICAL COLD-FORMED DETAILS				
S7.41	TYPICAL STEEL DETAILS				
S7.42	TYPICAL STEEL DETAILS				
S8.01	SIDEPLATE GENERAL NOTES AND CONSTRUCTION GUIDELINES				
S8.02	SIDEPLATE COLUMN DETAILS, A TYPE				
S8.03	SIDEPLATE COLUMN DETAILS, B TYPE				
S8.04	SIDEPLATE BEAM DETAILS				
S8.05	SIDEPLATE BEAM DETAILS, NARROW				
S8.06	SIDEPLATE FIELD ERECTION DETAILS				
S8.07	SIDEPLATE COORDINATION ITEMS				
S8.08	SIDEPLATE MISCELLANEOUS DETAILS				
ELECTRICAL					
E0.01	ELECTRICAL DEMOLITION PLAN				
Grand total: 66					



**WILMA P. MANKILLER HEALTH CENTER
EXPANSION**
 STILWELL, OKLAHOMA



PROJECT PHASE:
BID PACKAGE 01

#	DATE	REVISIONS	DESCRIPTION
1	11/22/19	BID PACKAGE 01 - ADD 01	
2	12/10/19	BID PACKAGE 01 - ADD 02	
3	1/10/20	BID PACKAGE 01 - AS01	

DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER:
G0.01

COVER / INDEX



1836 SOUTH BALTIMORE AVE.
TULSA, OK 74119
(539) 664-4618

MECHANICAL / ELECTRICAL / PLUMBING ENGINEER



3902 UNIVERSITY BOULEVARD
DURANT, OK 74701
(580) 931-9045

CIVIL ENGINEER



4700 LINCOLN ROAD NE, SUITE 102
ALBUQUERQUE, NM 87109
(505) 344-4080

STRUCTURAL ENGINEER



808 TRAVIS STREET, SUITE 200
HOUSTON, TX 77002
(281) 589-5900

FIRE PROTECTION / LIFE SAFETY

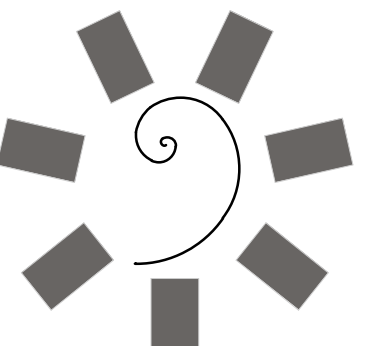


1316 E 35TH PLACE, SUITE 100
TULSA, OK 74105
(918) 382-9120

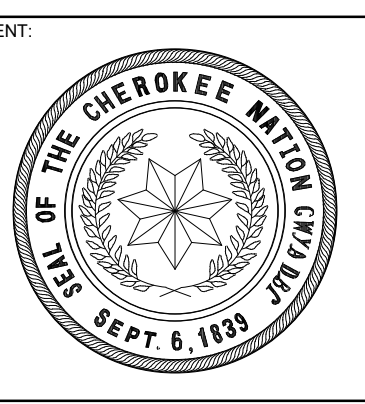
EQUIPMENT PLANNER

SIDEPLATE CONNECTION NOTES

- (AXX) INDICATES SIDEPLATE CONNECTION PER SHEETS S8.01 - S8.07.
- (MX) INDICATES MISCELLANEOUS SIDEPLATE DETAIL PER SHEETS S8.01 - S8.07.



James R. Childers
Architect, Inc.
45 South 4th Street
Fort Smith, AR 72901
479-783-2450
www.childersarchitect.com



**WILMA P. MANKILLER HEALTH CENTER
EXPANSION**
STILWELL, OKLAHOMA

KEY PLAN:

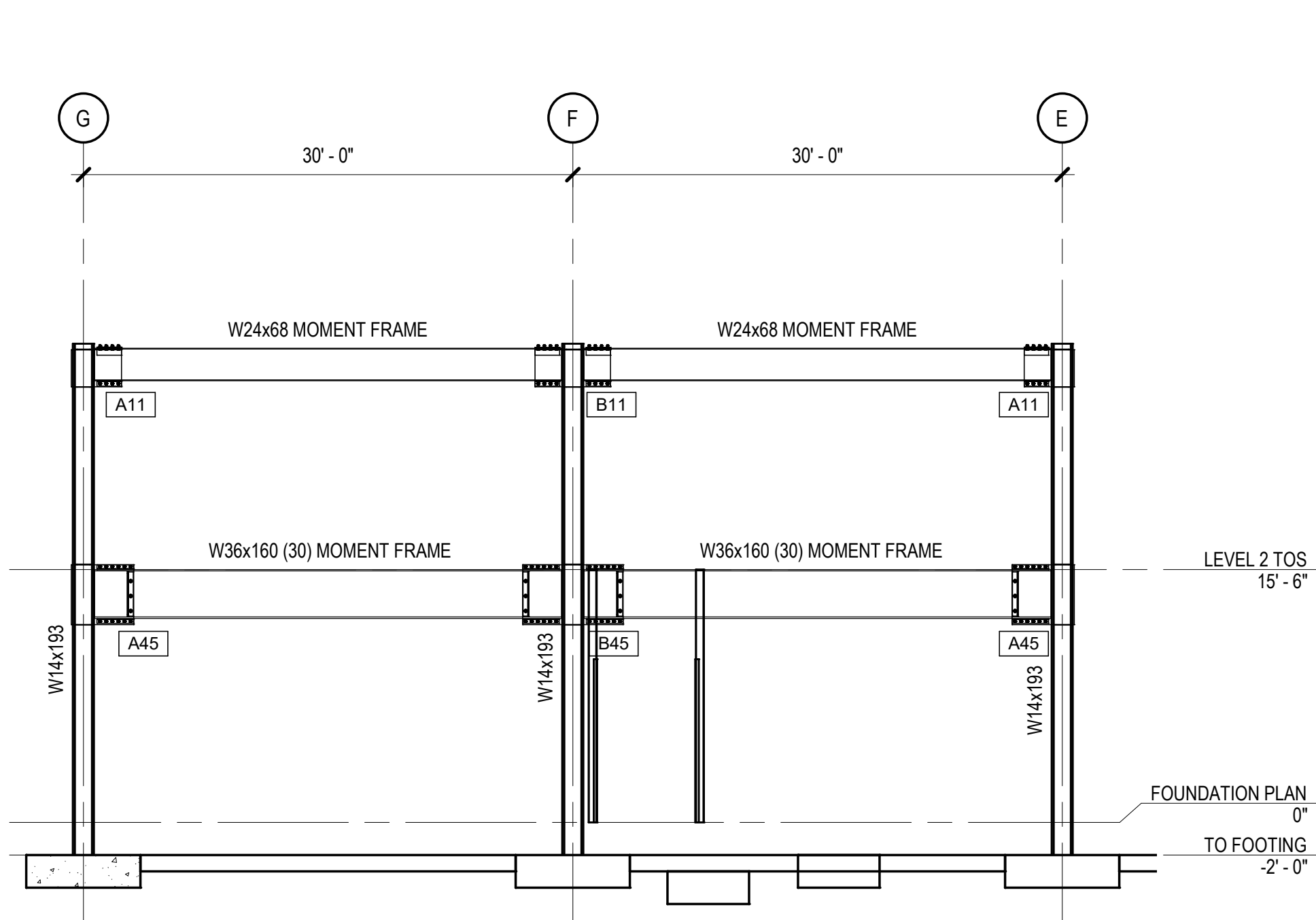
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BID PACKAGE 01

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3	01/16/20	BID PACKAGE 01 - ASH 01	

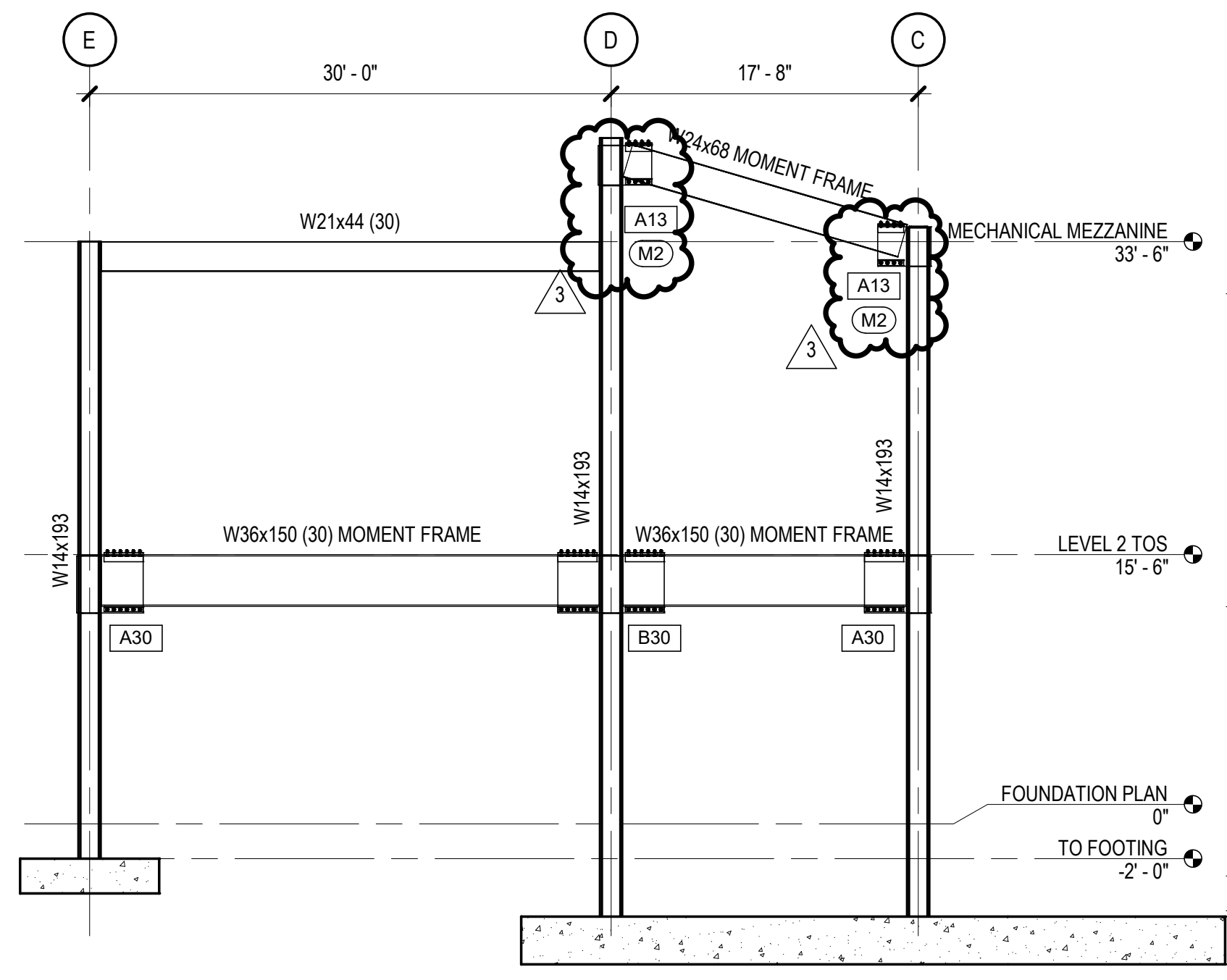
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JOB NUMBER: 18-01.01

SHEET NUMBER:
S2.01

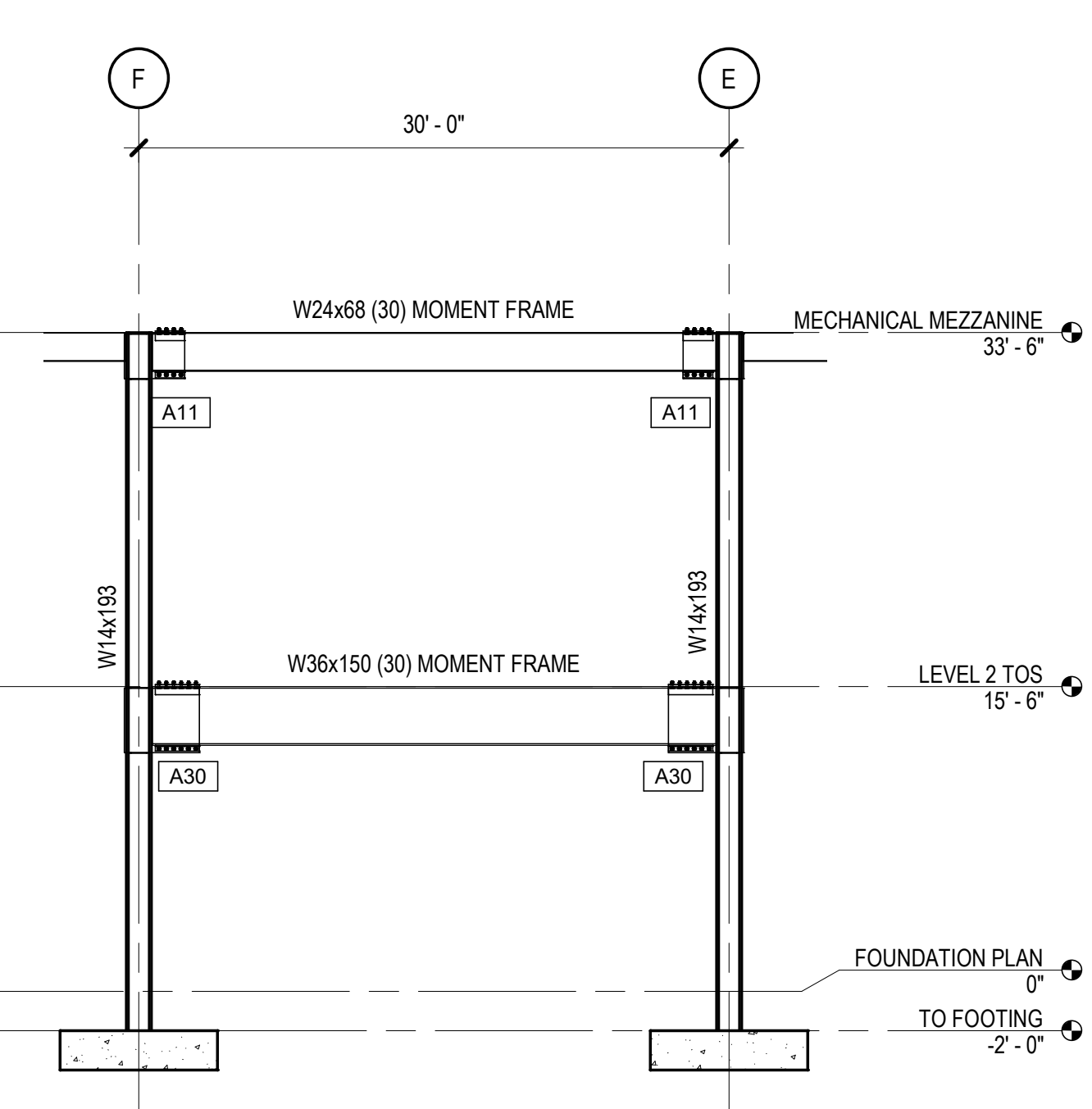
MOMENT FRAME ELEVATIONS



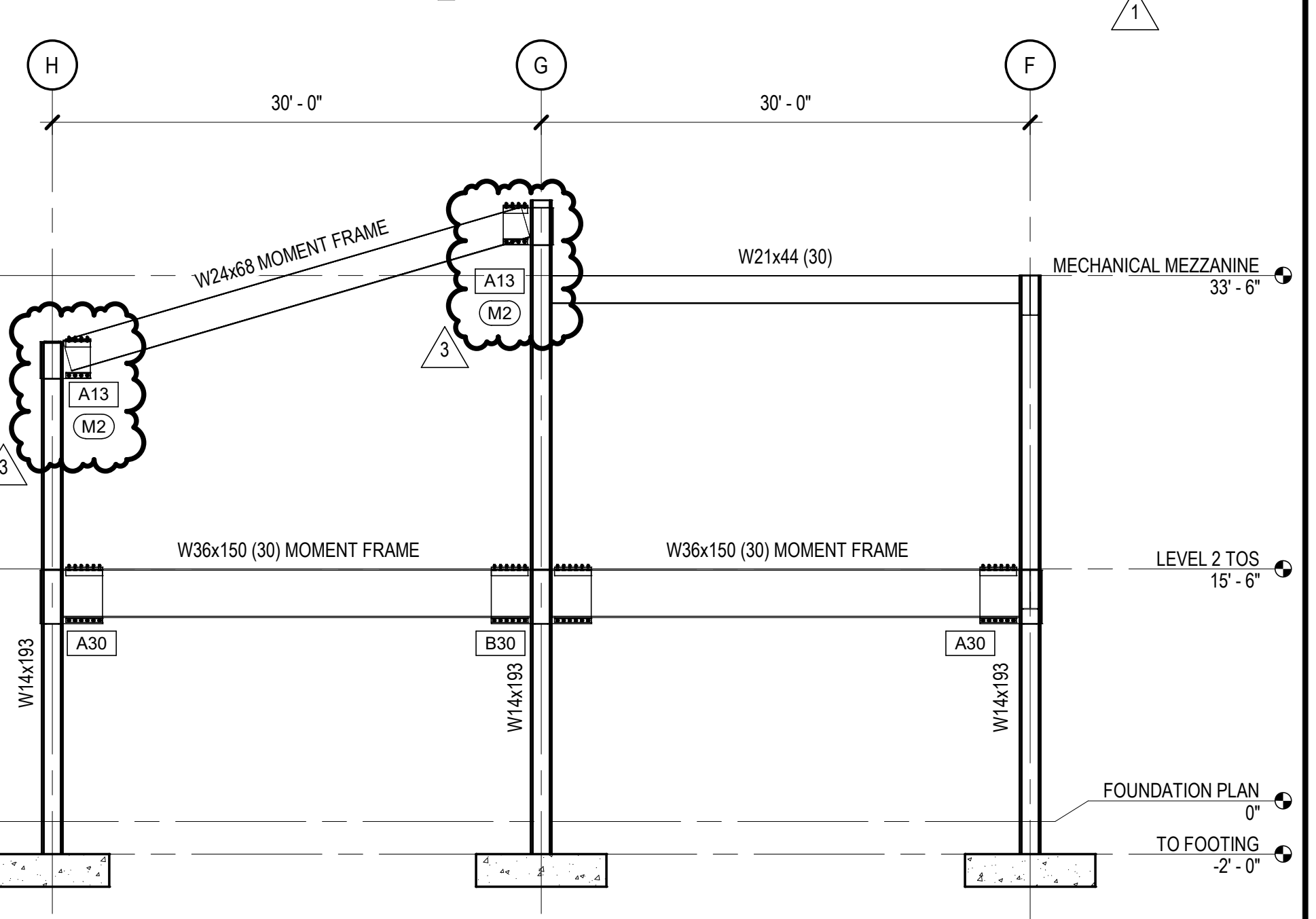
C2 MOMENT FRAME ELEVATION
SCALE: 1/8" = 1'-0"



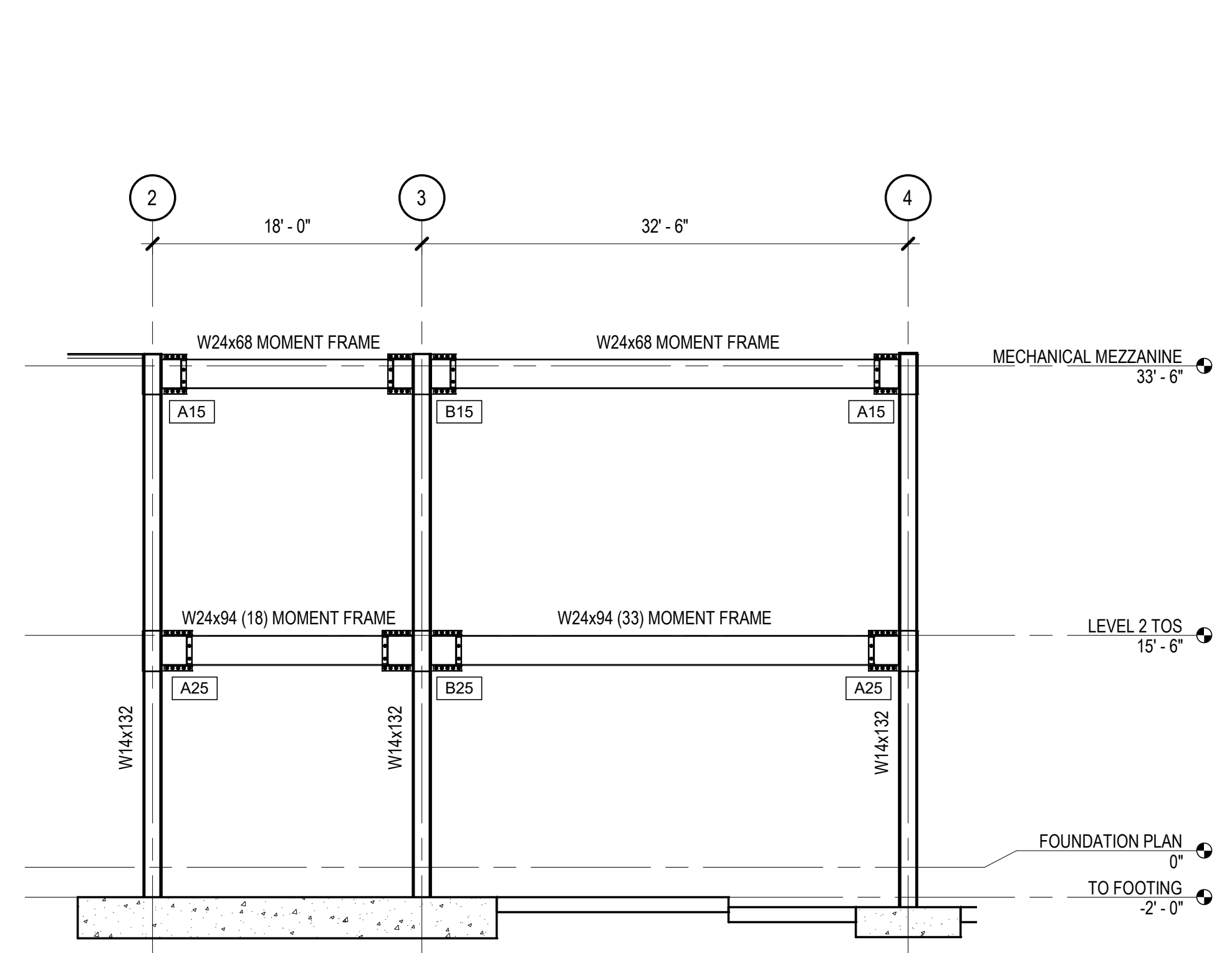
C4 MOMENT FRAME ELEVATION
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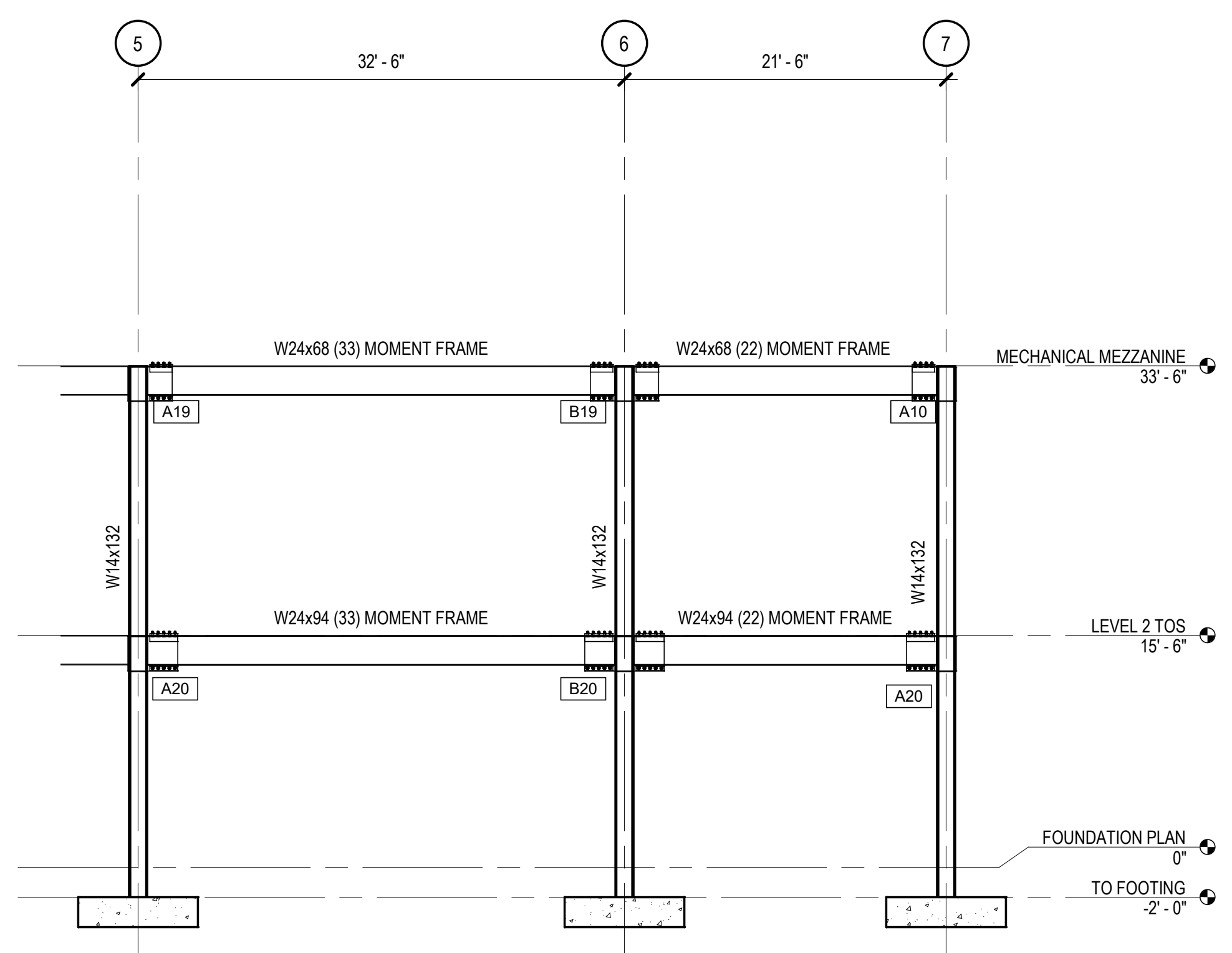
B4 MOMENT FRAME ELEVATION
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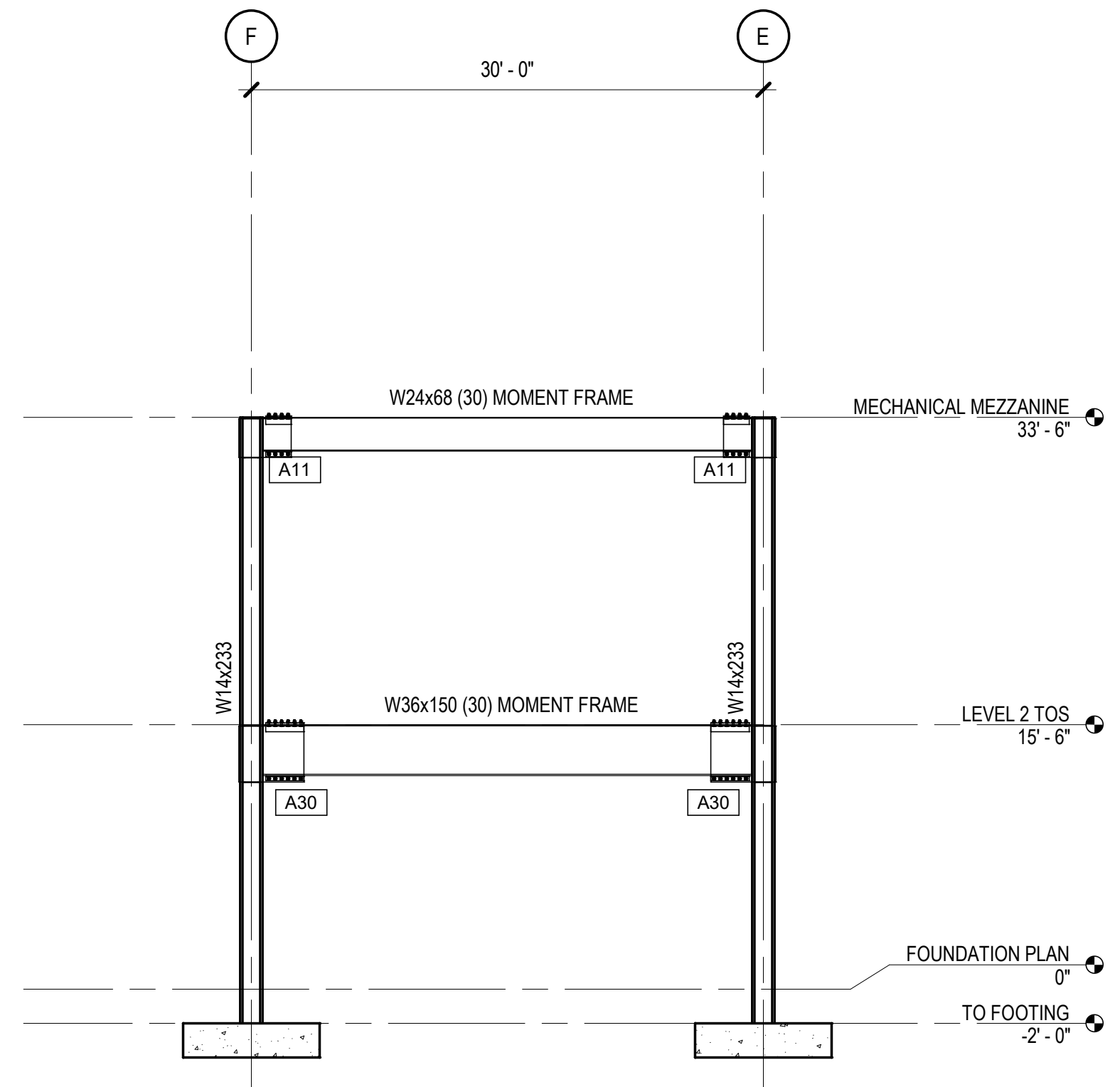
A4 MOMENT FRAME ELEVATION
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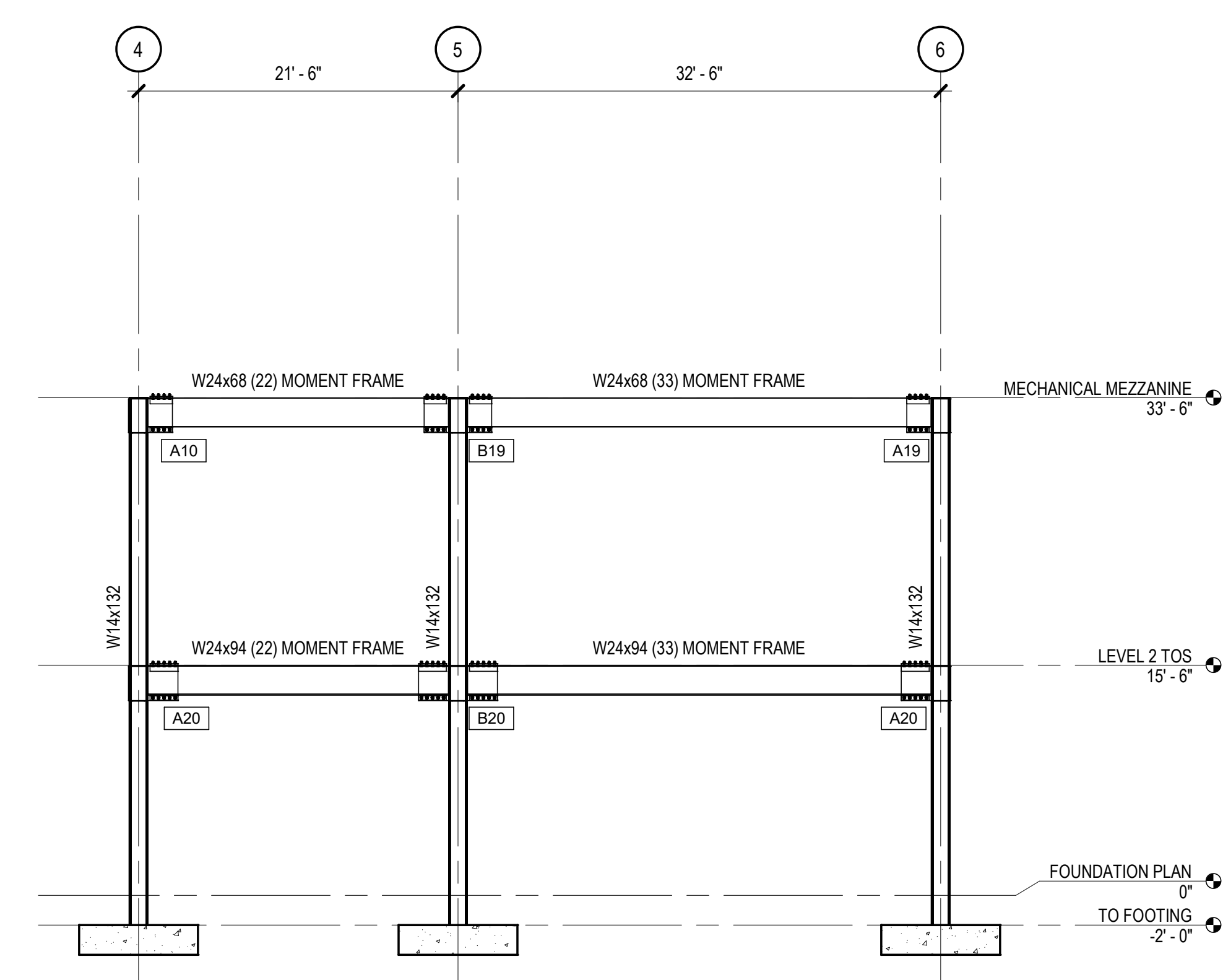
B1 MOMENT FRAME ELEVATION
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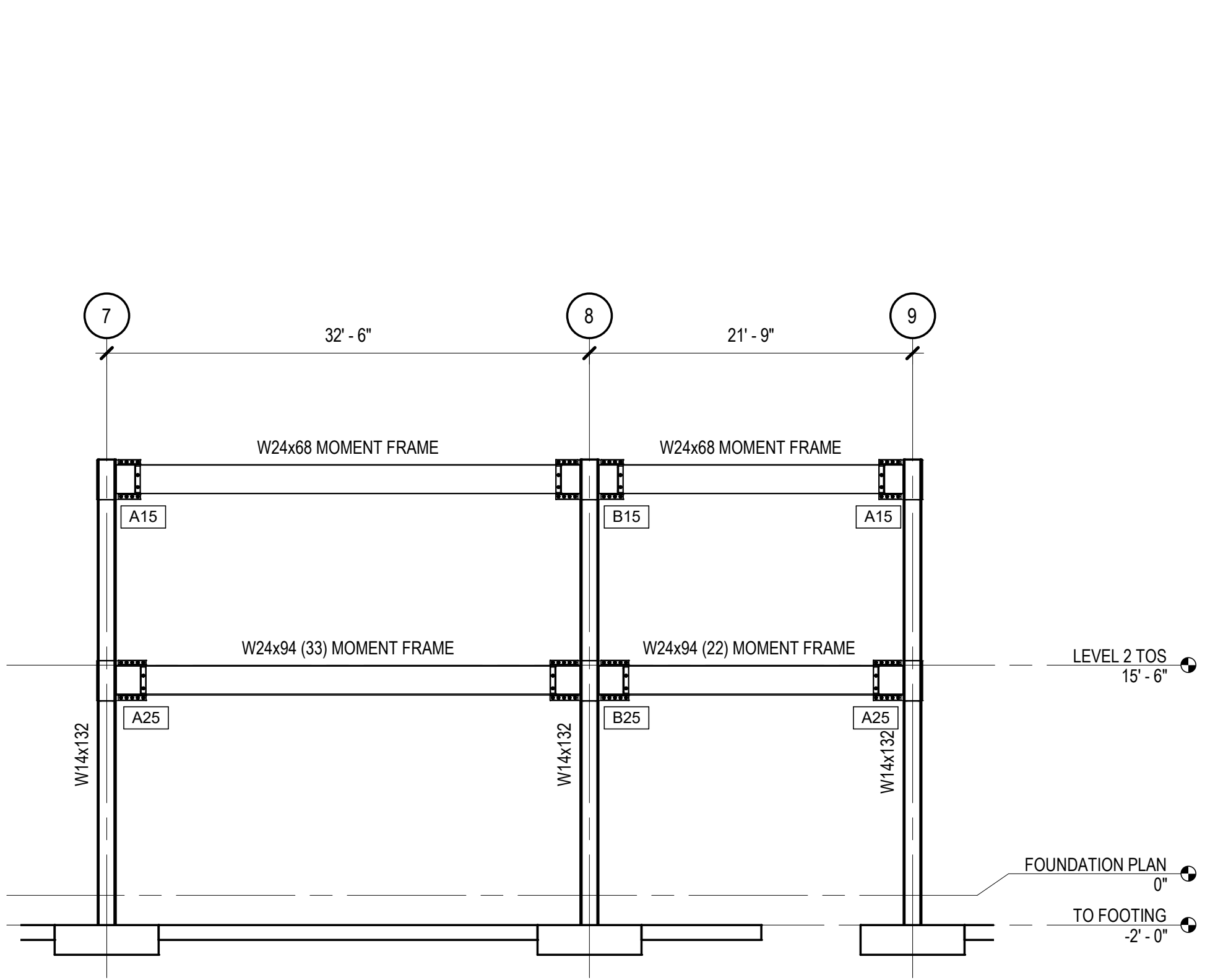
B2 MOMENT FRAME ELEVATION
SCALE: 1/8" = 1'-0"



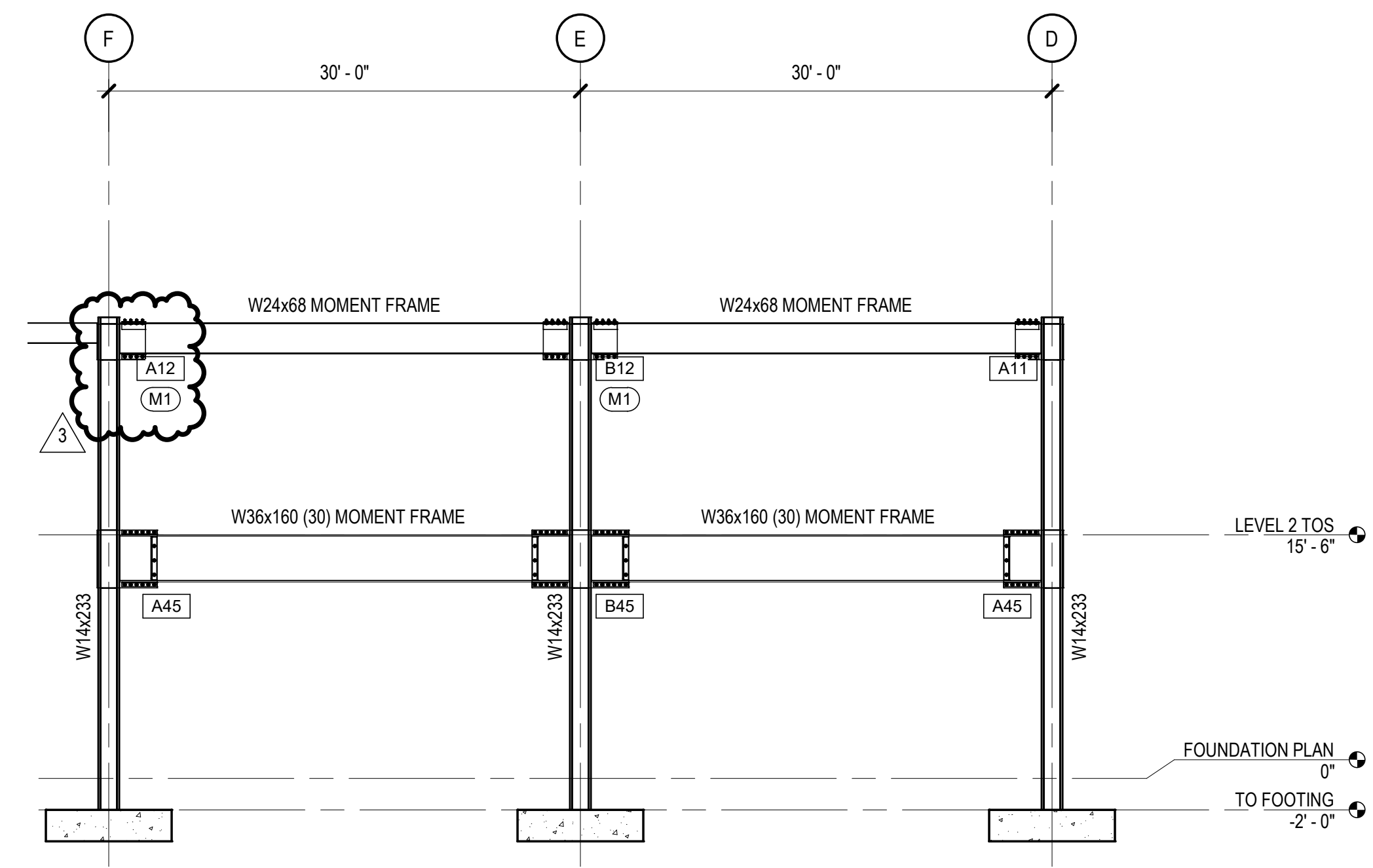
B5 MOMENT FRAME ELEVATION
SCALE: 1/8" = 1'-0"



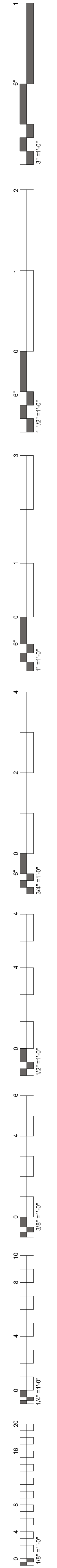
A1 MOMENT FRAME ELEVATION
SCALE: 1/8" = 1'-0"



A2 MOMENT FRAME ELEVATION
SCALE: 1/8" = 1'-0"



A5 MOMENT FRAME ELEVATION
SCALE: 1/8" = 1'-0"



PROJECT SPECIFIC INFORMATION

PROJECT NUMBER: 18250
SUBMITTAL NUMBER:
SUBMITTAL DATE: 8-03-2017
PROJECT NAME: STILWELL HEALTH CLINIC
LOCATION: STILWELL, OK
CONNECTION TYPE: SIDEPLATE
NUMBER OF BUILDINGS: 1
APPROX. TOTAL GROSS SQUARE FOOTAGE: 104714
NUMBER OF STORIES: 2

NOTES

- 1. THERE MAY BE eDATA AVAILABLE FOR YOUR PROJECT WHICH IS AVAILABLE FOR DOWNLOAD AT WWW.SIDEPLATE.COM. eDATA MAY INCLUDE:
a. eSTIMATE FILE IN EXCEL FORMAT FOR USE IN AFFIRMING SIDEPLATE CONNECTION MATERIAL QUANTITIES.
b. eCOMPRIEML FILE FOR USE IN ASSISTING DETAILING EFFORTS.
c. ESTIMATED NUMBER OF SIDEPLATE JOINTS FOR THIS PROJECT = 54
d. ESTIMATED NUMBER OF SIDEPLATE JOINTS FOR THIS PROJECT THAT ARE NOT SUPPORTED BY eDATA = (10)
e. MISCELLANEOUS DETAILS, TYPICALLY DESIGNATED BY MR. ARE NOT SUPPORTED.

INSTRUCTIONS TO STEEL FABRICATOR

- 1. SIDEPLATE LICENSE FEE
a. THE STEEL FABRICATOR'S BID PRICE FOR PROCUREMENT, FABRICATION AND ERECTION OF STRUCTURAL AND MISCELLANEOUS STEEL SHALL INCLUDE THE SIDEPLATE LICENSE FEE FOR THE PROJECT. EACH PROSPECTIVE STEEL FABRICATOR WHO BIDS THE PROJECT SHALL FORMALLY REQUEST THE SIDEPLATE LICENSE FEE BY ACCESSING THE SIDEPLATE WEBSITE (http://www.sideplate.com).
b. UPON THE SUCCESSFUL STEEL FABRICATOR SIGNING A CONTRACT TO FABRICATE STRUCTURAL STEEL FOR THIS PROJECT, THE STEEL FABRICATOR SHALL SUBMIT A PURCHASE ORDER (PO) TO SIDEPLATE SYSTEMS, INC. FOR THE TOTAL AMOUNT OF THE SIDEPLATE LICENSE FEE AND SHALL INCLUDE SAID FEE IN ITS FIRST CONSTRUCTION DRAW.
c. THE STEEL FABRICATOR SHALL MAKE PAYMENT OF THE SIDEPLATE LICENSE FEE DIRECTLY TO:

SIDEPLATE SYSTEMS, INC.
25909 PALA, SUITE 200
MISSION VIEJO, CA 92691
TEL: 949-238-9900

SUBMITTALS

- 1. IN ADDITION TO THE REQUIRED SUBMITTALS SPECIFIED BY THE BALANCE OF THE CONTRACT DOCUMENTS, THE FOLLOWING SUBMITTALS SHALL BE SENT TO SIDEPLATE SYSTEMS, INC. ELECTRONICALLY VIA THE STRUCTURAL ENGINEER OF RECORD FOR THEIR REVIEW AND DISPOSITION:
a. QUALITY CONTROL PROGRAM (REQUIRED IF NOT AISC CERTIFIED)
b. ONE ELECTRONIC COPY OF ALL STRUCTURAL STEEL DRAWINGS THAT EITHER DIRECTLY PERTAINS TO AND/OR AFFECTS THE SHOP FABRICATION OR FIELD ERECTION OF THE SIDEPLATE STEEL FRAME CONNECTION SYSTEM, INCLUDING THE INITIAL SUBMITTAL AND ALL CORRECTED RE-SUBMITTALS OF AFFECTED DRAWINGS. SIDEPLATE SYSTEMS, INC. SHALL BE GIVEN, AS A MINIMUM, THE SAME SPECIFIED REVIEW TIME (NOT LESS THAN SEVEN BUSINESS DAYS) AS THE ENGINEER OF RECORD.

MEETINGS

- 1. PRE-DETAILING MEETING
a. PRIOR TO THE START OF DETAILING OF THE SHOP DRAWINGS, THE FABRICATION CONTRACTOR SHALL FORMALLY REQUEST A PRE-DETAILING MEETING FROM SIDEPLATE SYSTEMS, INC. THIS MEETING IS TYPICALLY A WEBINAR TO DISCUSS BEST PRACTICES FOR THE DETAILING OF THE SIDEPLATE CONNECTIONS, AND TO CREATE A PROACTIVE FORUM TO ANSWER ANY QUESTIONS.
2. PRE-FABRICATION MEETING
a. PRIOR TO THE START OF FABRICATION, THE FABRICATION CONTRACTOR SHALL FORMALLY REQUEST A PRE-FABRICATION MEETING FROM SIDEPLATE SYSTEMS, INC. THIS MEETING IS TYPICALLY A WEBINAR TO DISCUSS BEST PRACTICES FOR THE FABRICATION OF THE SIDEPLATE CONNECTIONS, AND TO CREATE A PROACTIVE FORUM TO ANSWER ANY QUESTIONS.
3. PRE-ERECTION MEETING
a. PRIOR TO THE START OF STEEL ERECTION, THE ERECTION CONTRACTOR SHALL FORMALLY REQUEST A PRE-ERECTION MEETING FROM SIDEPLATE SYSTEMS, INC. THIS MEETING IS TYPICALLY A WEBINAR TO DISCUSS BEST PRACTICES FOR FIELD ERECTION OF THE SIDEPLATE BEAMS AND COLUMNS, AND TO CREATE A PROACTIVE FORUM TO ANSWER ANY QUESTIONS.

GENERAL

- 1. THE GOVERNING CODES SHALL CONSIST OF ANSIAWS D1.1-2010 (AWS D1.1), AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES (APRIL 14, 2010), 2009 RCSC SPECIFICATIONS FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS, AND ALL APPLICABLE BUILDING AND JURISDICTIONAL CODES AND PROJECT STANDARDS SPECIFIED IN THE PROJECT SPECIFICATION STRUCTURAL STEEL SECTION WHERE THE REQUIREMENTS DIFFER BETWEEN SIDEPLATE CONNECTION NOTES, THE GENERAL STRUCTURAL NOTES, AND THE GOVERNING CODES, THE MORE STRINGENT SECTION CRITERIA SHALL CONTROL.
2. ALPHA AND NUMERIC DESIGNATORS (A) & (H) USED HEREIN TO SIMPLIFY THE IDENTIFICATION OF PLATES, ANGLES, AND WELDS ARE DEFINED BELOW.

- (A) SIDE PLATE FOR UNAXIAL CONNECTIONS
(B) BEAM FLANGE COVER PLATE, AS REQUIRED
(C) VERTICAL SHEAR PLATE OR FLAT BAR, AS REQUIRED
(D) HORIZONTAL SHEAR PLATE OR FLAT BAR, AS REQUIRED
(E) VERTICAL ANGLE WELDED TO THE VERTICAL SHEAR PLATE (C), AS REQUIRED
(F) VERTICAL SHEAR ELEMENT (VSE) WHICH CONSISTS OF PLATE (C) AND ANGLE (E) MATERIAL, AS REQUIRED

- (G) LONGITUDINAL ANGLE WELDED TO THE OUTSIDE FACE OF SIDE PLATE (A), AS REQUIRED
(H) LONGITUDINAL ANGLE WELDED TO THE BOTTOM BEAM FLANGE (OR TOP BEAM FLANGE AS REQUIRED)
(I) HORIZONTAL PLATE WELDED TO THE OUTSIDE FACE OF SIDE PLATE (A), AS REQUIRED

- (1) FILLET WELD CONNECTING SIDE PLATE (A) TO HORIZONTAL SHEAR PLATE (D) OR COLUMN
(2) FILLET (AND/OR FLARE BEVEL) WELD CONNECTING INSIDE FACE OF SIDE PLATE (A) TO COLUMN
(3) FILLET WELD CONNECTING HORIZONTAL SHEAR PLATE (D) TO COLUMN, AS REQUIRED
(4) FILLET WELD TO CONSTRUCT VSE (F) AND TO CONNECT IT TO THE WEB OF THE BEAM, AS REQUIRED
(5) FILLET (AND/OR PJP) WELD CONNECTING BEAM FLANGE TIPS TO COVER PLATE (B) AND/OR LONGITUDINAL ANGLE (H), AS REQUIRED
(5a) FILLET WELD CONNECTING OUTSIDE FACE OF BEAM FLANGE TO COVER PLATE (B) AND/OR LONGITUDINAL ANGLE (H), AS REQUIRED
(5b) FILLET WELD CONNECTING COVER PLATE (B) EDGE TO TOP FACE OF BEAM FLANGE, ACROSS ITS WIDTH
(5c) PJP WELD CONNECTING ANGLE (H) TO BEVELED BEAM FLANGE
(8) FILLET (AND/OR PJP) WELD CONNECTING LONGITUDINAL ANGLE (G) (AND/OR PLATE (I)) TO SIDE PLATE (A), AS REQUIRED
(8b) PJP WELD CONNECTING PLATE (I) TO SIDE PLATE (A) AND/OR CONNECTING BUILT UP ANGLE (H) PLATES TOGETHER, AS REQUIRED
(9) FILLET WELD CONNECTING SIDE PLATE (A) TO COLUMN FACE, WRAPPED AROUND THREE SIDES OF SIDE PLATE (A)
(10) FILLET WELD TO CONSTRUCT SIDE PLATE SLOTTED INTERLOCK ASSEMBLY
(10a) PJP WELD TO CONSTRUCT SIDE PLATE SLOTTED INTERLOCK ASSEMBLY
(10b) REINFORCING FILLET WELD TO CONSTRUCT SIDE PLATE SLOTTED INTERLOCK ASSEMBLY

- 3. ALPHA DESIGNATORS, USED HEREIN TO SIMPLIFY THE IDENTIFICATION OF DIMENSIONS OF THE SIDEPLATE CONNECTIONS, ARE DEFINED BELOW:

- GAP PHYSICAL SEPARATION BETWEEN THE END OF THE MOMENT FRAME BEAM AND THE ADJOINING FACE OF THE COLUMN FLANGE
B DEPTH OF SIDE PLATE (A)
C LENGTH OF COVER PLATE (B) AND/OR LONGITUDINAL ANGLE (H)
D LENGTH OF SLOT FROM THE TOE OF THE RADIUS IN THE COVER PLATE (B), AS REQUIRED
E EDGE DISTANCE OF BOLT HOLES IN COVER PLATE (B), AS REQUIRED
G GAGE DISTANCE TO CENTERLINE OF BOLT HOLES IN ANGLES (G) AND (H), AND PLATE (I), AS REQUIRED
H ADDED DIMENSION TO COLUMN FLANGE WIDTH TO DEFINE TOTAL COVER PLATE (B) WIDTH
J DISTANCE FROM END OF THE BEAM TO CENTERLINE OF VERTICAL BOLT HOLES IN VSE (F), AS REQUIRED
R RADIUS OF SLOT DIMENSION IN COVER PLATE (B)
S HORIZONTAL SPACING BETWEEN BOLT HOLES
Y ADDED DIMENSION TO COLUMN FLANGE WIDTH FOR ALLOWABLE SPREAD OF SIDE PLATES (A)

MATERIAL

- 1. PLATE, FLAT BAR, AND ANGLE MATERIAL:
a. ALL PLATE MATERIAL SHALL HAVE A MINIMUM YIELD STRENGTH (Fy) OF 50 KSI.
b. ANGLE AND BAR MATERIAL SHALL HAVE A HIGH STRENGTH STEEL SPECIFICATION AND SHALL HAVE A MINIMUM YIELD STRENGTH (Fy) OF 50 KSI.
2. HIGH STRENGTH BOLTS/FASTENERS:
a. BOLTS SHALL BE TYPE 1 OR TYPE 3 AND SHALL BE A490X HEAVY HEX, F2280 TWIST-OFF-TYPE TENSION-CONTROL BOLT ASSEMBLIES, OR F3148 FIXED SPLINE BOLT ASSEMBLIES. THE BOLT HEAD SHALL BE DISTINCTIVELY MARKED WITH A MINIMUM MARKING OF A490, A490TC, OR 144 RESPECTIVELY, AN ALTERNATIVE DESIGN THAT MEETS THE REQUIREMENTS OF RCSC SECTION 2.8 MAY BE USED, WITH THE WRITTEN APPROVAL FROM SIDEPLATE SYSTEMS, INC.
b. WASHERS SHALL BE ORDINARY THICKNESS AND ASTM F436 TYPE 1 OR TYPE 3.
c. NUTS SHALL BE ASTM A563 GRADE DH OR DH3.
d. THE BOLT ASSEMBLY SHALL BE COVERED IN A LIGHT PROTECTIVE OIL, F2280 AND F3148 ASSEMBLIES SHALL ONLY BE LUBRICATED BY THE SUPPLIER.
e. THE MILL TEST REPORT (MTR) MUST HAVE DOCUMENTED LOT TRACEABILITY, STATEMENT OF DIMENSIONAL RESULTS, FULL CHEMICAL AND MECHANICAL TEST RESULTS TO THE SPECIFICATIONS ABOVE.
f. THE USE OF FINGER SHIMS ARE ACCEPTABLE PER BOLTING SECTION 8.
3. ROLLED SHAPES:
a. ALL ROLLED SHAPES USED FOR COLUMNS AND BEAMS IN CONSTRUCTING SIDEPLATE MOMENT FRAMES SHALL BE ASTM A992 GRADE 50 UNO.
4. HSS TUBE SHAPES:
a. ALL HSS SHAPES USED FOR COLUMNS AND BEAMS IN CONSTRUCTING SIDEPLATE MOMENT FRAMES SHALL, AS A MINIMUM, BE ASTM A500 GRADE B OR GRADE C OR ASTM1085.

PREPARATION

- 1. THE STEEL FABRICATION AND ERECTION SUBCONTRACTORS SHALL EMPLOY A DISTORTION CONTROL PROGRAM PRIOR TO THE START OF SIDEPLATE MOMENT FRAME FABRICATION. THE DISTORTION CONTROL PROGRAM SHALL BE IN ACCORDANCE WITH THE PROVISIONS OF AWS D1.1 SECTION 5.21 AND 5.22 TO ENSURE THAT THE FOLLOWING ARE MAINTAINED:
a. DIMENSIONAL ACCURACY
b. FRAMING AND ALIGNMENT TOLERANCES
c. COMPLIANCE WITH AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES, SECTION 7.0, ERECTION PROVISIONS
d. CONTROL OF DISTORTION AND WELD SHRINKAGE

WELDING

- 1. WELDER QUALIFICATION: THE PERFORMANCE OF ALL WELDERS, WELDING OPERATORS AND TACK WELDERS SHALL BE QUALIFIED IN CONFORMANCE WITH AWS D1.1, SECTION 4, PART C TO DEMONSTRATE ABILITY TO PRODUCE SOUND WELDS.

BOLTING

- 1. BOLTS/FASTENERS SHALL BE INSTALLED TO PRETENSIONED CONDITION USING ONE OF THE METHODS PRESCRIBED HERE: TURN-OF-NUT (A490), CALIBRATED WRENCH (A490), TWIST-OFF-TYPE TENSION-CONTROL BOLT (F2280), OR TORQUE AND ANGLE METHOD (F3148).
2. FOR ALL PRETENSIONING METHODOLOGIES, ALL FASTENER ASSEMBLIES WITHIN THE JOINT SHALL FIRST BE BROUGHT TO A SNUG TIGHT CONDITION, FOLLOWED BY A SYSTEMATIC PRETENSIONING PROCESS. PRETENSIONING SHALL BEGIN AT THE MOST RIGID PART OF THE JOINT AND CONTINUE IN A MANNER THAT WILL MAINTAIN THE RELAXATION OF PREVIOUSLY PRETENSIONED FASTENERS, UNTIL THE CONNECTED PLIES ARE IN AS FIRM CONTACT AS POSSIBLE.

- 3. REUSE OF A490, F2280, AND F3148 BOLT ASSEMBLIES SHALL NOT BE ALLOWED. TOUCHING UP OR RE-TIGHTENING BOLTS THAT MAY HAVE BEEN LOOSENEED BY THE INSTALLATION OF ADJACENT BOLTS SHALL NOT BE CONSIDERED TO BE A REUSE.
4. ALL BOLT HOLES SHALL BE ALIGNED TO PERMIT INSERTION OF THE BOLTS WITHOUT UNDUE DAMAGE TO THE THREADS.
5. THE BOLT LENGTH USED SHALL BE SUCH THAT THE BOLT THREAD EXTENDS BEYOND OR IS AT LEAST FLUSH WITH THE OUTER FACE OF THE NUT WHEN PROPERLY INSTALLED.
6. FASTENER COMPONENTS SHALL BE PROTECTED FROM DIRT AND MOISTURE IN CLOSED CONTAINERS AT THE SITE OF INSTALLATION.
7. F2280 OR F3148 ASSEMBLIES AND ALTERNATIVE DESIGN FASTENERS THAT MEET THE SPECIFIED REQUIREMENTS PREVIOUSLY MENTIONED SHALL NOT BE RE-LUBRICATED, EXCEPT BY THE MANUFACTURER.
8. FINGER SHIMMING SHALL BE USED UP TO 1/4 INCH WITHOUT RESTRICTION. SHIM REQUIREMENTS GREATER THAN 1/4 INCH SHALL BE SUBMITTED TO SIDEPLATE SYSTEMS INC FOR APPROVAL PRIOR TO USE.
9. WASHERS SHALL BE ASTM F436 ORDINARY THICKNESS AND SHALL BE USED UNDER THE NUT OF THE FASTENER ASSEMBLY SO AS TO PROVIDE A HARDENED-NON-GALLING SURFACE OF THE TURNED ELEMENT. WHEN USING THE TURN-OF-NUT OR CALIBRATED WRENCH METHOD, THE TURNED ELEMENT MUST BE THE SAME AS WAS USED WHEN PERFORMING PREINSTALLATION VERIFICATION TESTING.

QUALITY CONTROL

- 1. THE FABRICATOR AND ERECTOR SHALL BE RESPONSIBLE FOR QUALITY CONTROL BY PROVIDING, AS A MINIMUM, IN-PROCESS VISUAL INSPECTION OF ALL FABRICATION AND ERECTION ACTIVITIES TO ENSURE THAT MATERIALS AND WORKMANSHIP MEET THE REQUIREMENTS OF THE CONTRACT DOCUMENTS AND SHALL INCLUDE WORKMANSHIP. SUCH WORK SHALL INCLUDE, BUT NOT BE LIMITED TO, VERIFYING THAT EFFECTIVE PROCEDURES AND METHODS HAVE BEEN EMPLOYED IN THE FORM OF A DISTORTION CONTROL PROGRAM TO ACCOUNT FOR AND COUNTERACT THE EFFECTS OF WELD SHRINKAGE, EXISTING BEAM SWEEP AND CAMBER, AND CHANGES IN MOMENT FRAME GEOMETRY DUE TO SKID AND CURVED DESIGN CONFIGURATIONS (AS OCCURS), TO ENSURE COMPLIANCE WITH SPECIFIED ERECTION AND ALIGNMENT TOLERANCES. QC INSPECTION SHALL INCLUDE HOLD POINTS FOR THE FOLLOWING:

- a. COLUMN TREE ASSEMBLY
1. VERIFICATION THAT ACTUAL COLUMN FLANGE WIDTH IS AT LEAST NOMINAL COLUMN FLANGE WIDTH WHERE THE SIDE PLATES (A) ARE TO BE INSTALLED. IN THE UNLIKELY EVENT ACTUAL COLUMN FLANGE WIDTH IS LESS THAN NOMINAL, BUT WITHIN AISC STANDARD MILL TOLERANCES (3/16 INCH MAX), CONTACT SIDEPLATE SYSTEMS, INC FOR APPROPRIATE RECOMMENDATIONS.
2. MINIMUM CLEAR DIMENSION SHALL BE VERIFIED AFTER PLACEMENT OF WELD (2), COOLING OF WELD (2), AND REMOVAL OF TEMPORARY SHOP CONSTRUCTION AID(S). VERIFY THAT A MINIMUM ACTUAL COLUMN FLANGE WIDTH DIMENSION OCCURS ANYWHERE IN BETWEEN THE SIDE PLATES (A) FROM TOP TO BOTTOM. THE SIDE PLATES SHALL BE PARALLEL TO ONE ANOTHER. IN NO CASE SHALL THEY BE LESS THAN THE ACTUAL COLUMN FLANGE WIDTH.
3. MAXIMUM SPREAD DIMENSION OF SIDE PLATE (A) SHALL NOT EXCEED ACTUAL COLUMN FLANGE WIDTH PLUS THE SCHEDULED SPREAD DIMENSION. THE FIELD CONSTRUCTION AID SHALL BE PLACED AND HOLD THE SIDE PLATES IN THIS FLARED CONDITION UNTIL THE BEAM HAS BEEN SAFELY ERECTED. IN NO CASE SHALL THE SPREAD CAUSE PERMANENT DEFORMATION IN THE SIDE PLATES.
4. VERIFICATION OF BOLT HOLE ELEVATION AND SPACING FOR POSITION OF SIDE PLATE (A) AND PROPER POSITION AND ELEVATION OF ANGLES (G).
b. BEAM ASSEMBLY
1. VERIFICATION OF PERPENDICULAR ALIGNMENT BETWEEN THE TOP COVER PLATE (B) AND BOTTOM ANGLES (H) TO THE WEB OF THE BEAM. TO MINIMIZE, IF NOT ELIMINATE, ANY MISALIGNMENT OF BOLT HOLES DUE TO BEAM FLANGE TILT WHEN THE BEAM HAS BEEN LOWERED INTO PLACE.
2. VERIFICATION OF BOLT HOLE SPACING AND POSITION ON COVER PLATE (B) AND ANGLES (H). CONSIDERATION SHALL BE GIVEN TO THE CURPING EFFECT OF THE TOP COVER PLATE (B), DUE TO WELD SHRINKAGE.
3. VERIFICATION OF THE DISTANCE BETWEEN EXTERIOR ANGLE (H) FACES AND THEIR RESPECTIVE BOLT HOLE PLACEMENT TO EACH OTHER (VERTICALLY AND HORIZONTALLY).
4. VERIFICATION THAT IN NO CASE SHALL THE OUTSIDE FACE OF VSE (F) EXTEND BEYOND THE OUTSIDE FACES OF THE LONGITUDINAL ANGLES (H).
5. VERIFICATION THAT VERTICAL PLACEMENT OF VSE (F) IS IN THE CORRECT LOCATION.

- 2. FILLET WELD FIT-UP TOLERANCES
a. THE PARTS TO BE JOINED BY FILLET WELDS SHALL BE BROUGHT INTO AS CLOSE CONTACT AS PRACTICABLE, USING AS NECESSARY SUITABLE CLAMPING MEANS. THE ROOT OPENING (I.E. THE FIT-UP GAP) SHALL NOT EXCEED 1/4 INCH. FOR FILLET WELD ROOT GAPS GREATER THAN 1/16 INCH, THE LEG SIZE (I.E. THE SPECIFIED SIZE) OF FILLET WELD SHALL BE INCREASED BY THE AMOUNT OF THE ROOT OPENING.
3. THERMAL CUTTING:
a. THE ROUGHNESS OF ALL THERMAL CUT SURFACES SHALL BE NO GREATER THAN AN ANSI SURFACE ROUGHNESS VALUE OF 1000 MICRO-INCHES. ROUGHNESS EXCEEDING THIS VALUE AND NOTCHES OR GOUGES NOT MORE THAN 3/16 INCH DEEP SHALL BE REMOVED BY MACHINING OR GRINDING. NOTCHES OR GOUGES IN THE THERMALLY CUT EDGES DEEPER THAN 3/16 INCH SHALL BE REPAIRED PER AWS.
4. TENSION CALIBRATION FOR PRE-INSTALLATION:
a. TENSION CALIBRATION SHALL BE USED TO CONFIRM THE SUITABILITY OF THE COMPLETE FASTENER ASSEMBLY, AND THE PROCEDURE TO BE USED BY THE BOLTING CREW.

- QUALITY ASSURANCE
IN ADDITION TO ALL OTHER QUALITY ASSURANCE INSPECTION ACTIVITIES, THE OWNER'S VERIFICATION INSPECTOR SHALL BE RESPONSIBLE FOR:
1. WELDING:
a. TO ASSURE THE PROPER AMPERAGE AND VOLTAGE OF THE WELDING PROCESS, THE USE OF HAND HELD CALIBRATED AMP AND VOLT METERS SHALL BE USED. THIS EQUIPMENT SHALL BE USED BY THE FABRICATOR AND THE INSPECTOR. AMPERAGE AND VOLTAGE SHALL BE MEASURED NEAR THE ARC. TRAVEL SPEED AND ELECTRODE STICK OUT SHALL BE VERIFIED TO BE IN COMPLIANCE WITH THE APPROVED WPS.
b. VISUAL INSPECTION SHALL BE PERFORMED ON ALL SHOP WELDS.
c. EACH WELDER EMPLOYED ON THE PROJECT SHALL UNDERSTAND ALL THE REQUIREMENTS OF THE WELDING PROCEDURE SPECIFICATION(S) BEFORE WELDING ON THE PROJECT.
d. AS-BUILT BEAM TO COLUMN GAP PER CONNECTION SCHEDULE IS ALLOWED TO BE INSTALLED WITH A TOLERANCE OF PLUS OR MINUS 1/2 INCH.
2. FINISH SURFACES:
a. THE SURFACES ADJACENT TO THE BOLT HEAD AND NUT SHALL BE FREE OF DIRT AND OTHER FOREIGN MATERIAL OTHER THAN THE SPECIFIED COATINGS.
b. FINISH SURFACES ARE PERMITTED TO BE UNCOATED AND COATED WITH ANY COATINGS OF ANY FORMULATION OR GALVANIZATION.
c. AFTER THE CONNECTIONS HAVE BEEN ASSEMBLED, VISUALLY ENSURE THAT THE PLIES OF THE CONNECTED ELEMENTS HAVE BEEN BROUGHT INTO AS CLOSE OF CONTACT AS PRACTICABLE WITH ONE ANOTHER. GAPS UP TO 1/8 INCH BETWEEN THE SURFACES SHALL BE ALLOWED. GAPS GREATER THAN 1/8 INCH UP TO 1/4 INCH SHALL HAVE FINGER SHIMS INSTALLED BEFORE PRETENSIONING. FOR GAPS GREATER THAN 1/4 INCH, CONTACT SIDEPLATE SYSTEMS, INC.

HOT DIPPED GALVANIZING

- 1. SIDEPLATE CONNECTIONS REQUIRING THIS TYPE OF FINISH SHALL FOLLOW THE SAME CONSTRUCTION SEQUENCING AS PREVIOUSLY OUTLINED WITH THE FOLLOWING MODIFICATIONS:
a. HORIZONTAL SHEAR PLATES (C) SHALL HAVE AN INCREASED CLIP SIZE WHICH SHALL BE 1.58 INCH BY 1.58 INCH TO PROVIDE ADEQUATE VENTILATION AND DRAINAGE. CONTACT SIDEPLATE SYSTEMS, INC. IN THE EVENT THAT THE GALVANIZING CONTRACTOR SPECIFICATIONS REQUIRE A LARGER OPENING THAN THAT SPECIFIED HEREIN.
b. SEAL WELDING SHALL BE ALLOWED ON THE PLATES (B) AND ANGLES.
c. ANY DEVIATIONS TO THESE MODIFICATIONS SHALL BE COORDINATED WITH SIDEPLATE SYSTEMS, INC. AND THE SEOR.

FIREPROOFING

- 1. WHEN REQUIRED BY THE GOVERNING CODE FOR CERTAIN TYPES OF CONSTRUCTION, SIDEPLATE CONNECTIONS SHALL HAVE A FIRE-RESISTANCE RATING LIKE THAT OF A STEEL "STRUCTURAL FRAME".
2. THE MINIMUM THICKNESS OF SPRAY-APPLIED FIRE-RESISTIVE MATERIAL (SFRM) FOR STEEL SIDEPLATE CONNECTIONS PLATES THAT ARE NOT ENCASED IN CONCRETE, SHALL BE DETERMINED JUST LIKE THAT OF A PIPE/TUBE COLUMN SECTION WITH A CONSTANT STEEL WALL THICKNESS USING THE THICKNESS OF SIDE PLATE (A) FOR EACH SIDEPLATE CONNECTION ID PER THE SIDEPLATE CONNECTION SCHEDULE, WHICH ARE UNIFORMLY HEATED AND PROTECTED (THE FIRE EXPOSURE OF A PIPE/TUBE COLUMN IS DIRECTLY ANALOGOUS TO A PLATE WITH A 1-SIDED FIRE EXPOSURE AND PROTECTION). THE SFRM SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ASTM E119 AND LISTED FOR FIRE RESISTIVE PIPE/TUBE COLUMN APPLICATIONS FOR NO LESS THAN THE REQUIRED RATED TIME.
3. AS REQUIRED, WHEN NO VERTICAL SHEAR ELEMENT (F) EXISTS IN THE BEAM, SPRAY THE MINIMUM THICKNESS OF SFRM BETWEEN INSIDE OF SIDE PLATE (A) AND BEAM WEB COVERING ALL SURFACES INCLUDING COLUMN FLANGE. NOTE: THIS DOES NOT NECESSITATE FILLING THE CAVITY FULL.
4. WHEN VERTICAL SHEAR ELEMENT (F) IS USED, THE CONTRACTOR SHALL PROVIDE THE MEANS, TYPICALLY DONE WITH A LAYERING TECHNIQUE, FOR FIREPROOFING ACROSS THE BOTTOM OF THE GAP.
5. SEE GRAPHIC NUMBER 10 IN FIELD ERECTION OF THE SIDEPLATE BOLTED SYSTEM FOR FIREPROOFING ACROSS THE BOTTOM OF THE GAP.

INTELLECTUAL PROPERTY

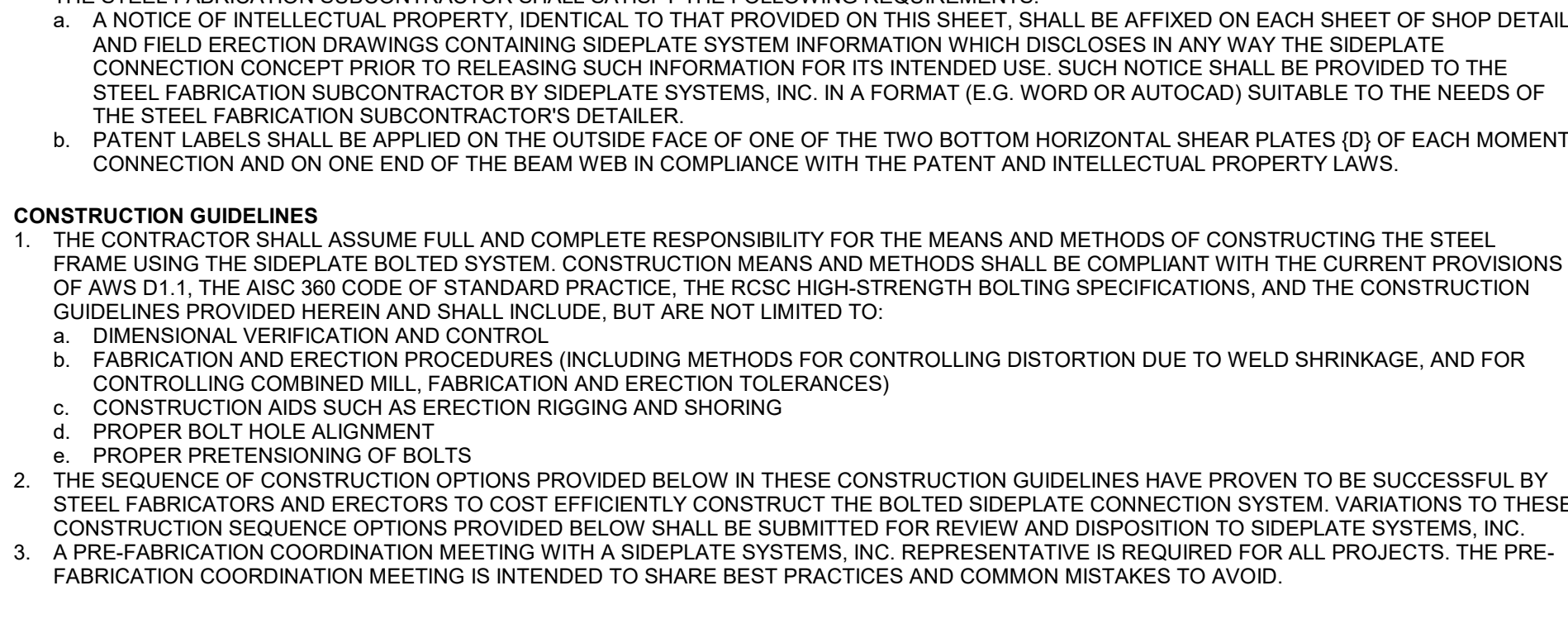
- 1. IN ORDER TO SAFEGUARD THE AUTHORIZED USE AND INTELLECTUAL PROPERTY OF THE PATENTED SIDEPLATE CONNECTION TECHNOLOGY, THE STEEL FABRICATION SUBCONTRACTOR SHALL SATISFY THE FOLLOWING REQUIREMENTS:
a. A NOTICE OF INTELLECTUAL PROPERTY, IDENTICAL TO THAT PROVIDED ON THIS SHEET, SHALL BE AFFIXED ON EACH SHEET OF SHOP DETAIL AND FIELD ERECTION DRAWINGS CONTAINING SIDEPLATE SYSTEM INFORMATION WHICH DISCLOSES IN ANY WAY THE SIDEPLATE CONNECTION CONCEPT PRIOR TO RELEASING SUCH INFORMATION FOR ITS INTENDED USE. SUCH NOTICE SHALL BE PROVIDED TO THE STEEL FABRICATION SUBCONTRACTOR BY SIDEPLATE SYSTEMS, INC. IN A FORMAT (E.G. WORD OR AUTOCAD) SUITABLE TO THE NEEDS OF THE STEEL FABRICATION SUBCONTRACTOR'S DETAILER.
b. PATENT LABELS SHALL BE APPLIED ON THE OUTSIDE FACE OF ONE OF THE TWO BOTTOM HORIZONTAL SHEAR PLATES (D) OF EACH MOMENT CONNECTION AND ON ONE END OF THE BEAM WEB IN COMPLIANCE WITH THE PATENT AND INTELLECTUAL PROPERTY LAWS.

CONSTRUCTION GUIDELINES

- 1. THE CONTRACTOR SHALL ASSUME FULL AND COMPLETE RESPONSIBILITY FOR THE MEANS AND METHODS OF CONSTRUCTING THE STEEL FRAME USING THE SIDEPLATE BOLTED SYSTEM. CONSTRUCTION MEANS AND METHODS SHALL BE COMPLIANT WITH THE CURRENT PROVISIONS OF AWS D1.1, THE AISC 360 CODE OF STANDARD PRACTICE, THE RCSC HIGH-STRENGTH BOLTING SPECIFICATIONS, AND THE CONSTRUCTION GUIDELINES PROVIDED HEREIN AND SHALL INCLUDE, BUT ARE NOT LIMITED TO:
a. DIMENSIONAL VERIFICATION AND CONTROL
b. FABRICATION AND ERECTION PROCEDURES (INCLUDING METHODS FOR CONTROLLING DISTORTION DUE TO WELD SHRINKAGE, AND FOR CONTROLLING COMBINED MILL, FABRICATION AND ERECTION TOLERANCES)
c. CONSTRUCTION AIDS SUCH AS ERECTION RIGGING AND SHORING
d. PROPER BOLT HOLE ALIGNMENT
e. PROPER PRETENSIONING OF BOLTS
2. THE SEQUENCE OF CONSTRUCTION OPTIONS PROVIDED BELOW IN THESE CONSTRUCTION GUIDELINES HAVE PROVEN TO BE SUCCESSFUL BY STEEL FABRICATORS AND ERECTORS TO COST EFFICIENTLY CONSTRUCT THE BOLTED SIDEPLATE CONNECTION SYSTEM. VARIATIONS TO THESE CONSTRUCTION SEQUENCE OPTIONS PROVIDED BELOW SHALL BE SUBMITTED FOR REVIEW AND DISPOSITION TO SIDEPLATE SYSTEMS, INC.
3. A PRE-FABRICATION COORDINATION MEETING WITH A SIDEPLATE SYSTEMS, INC. REPRESENTATIVE IS REQUIRED FOR ALL PROJECTS. THE PRE-FABRICATION COORDINATION MEETING IS INTENDED TO SHARE BEST PRACTICES AND COMMON MISTAKES TO AVOID.

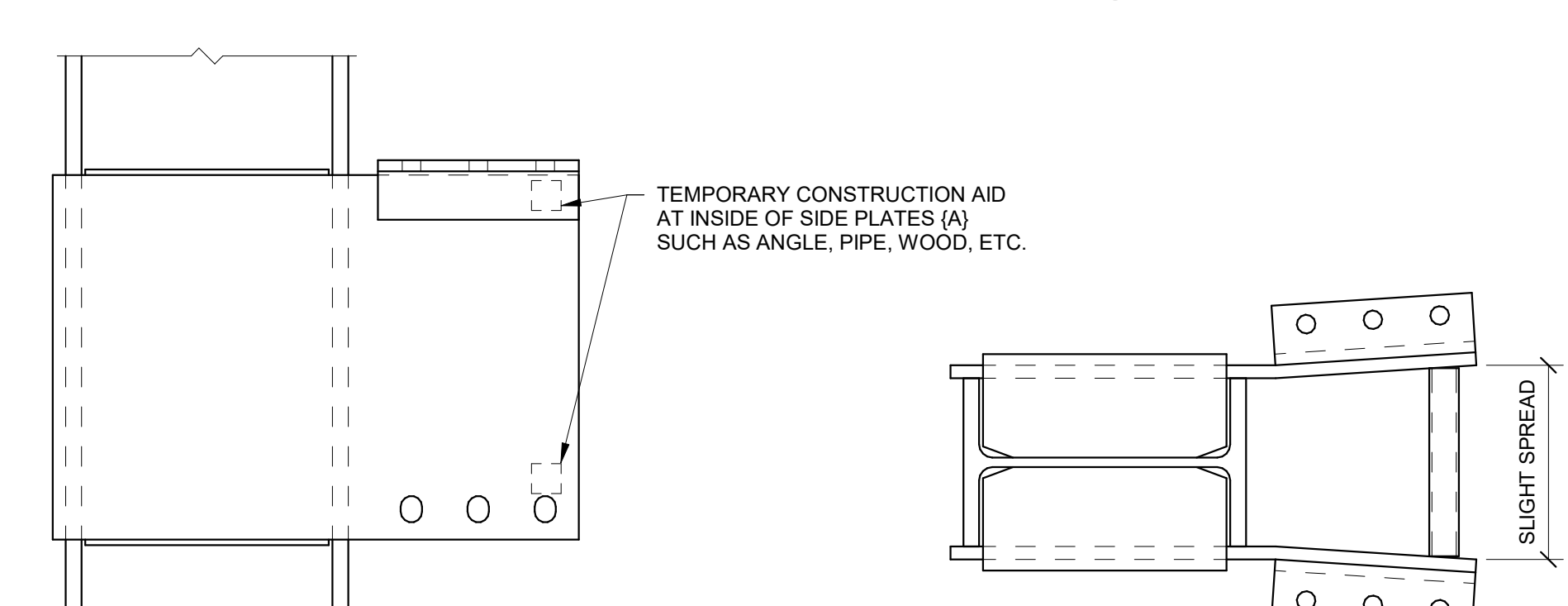
SHOP FABRICATION OF THE SIDEPLATE BOLTED SYSTEM

- 1. WATCH OUR SIDEPLATE COLUMN ASSEMBLY VIDEO AT

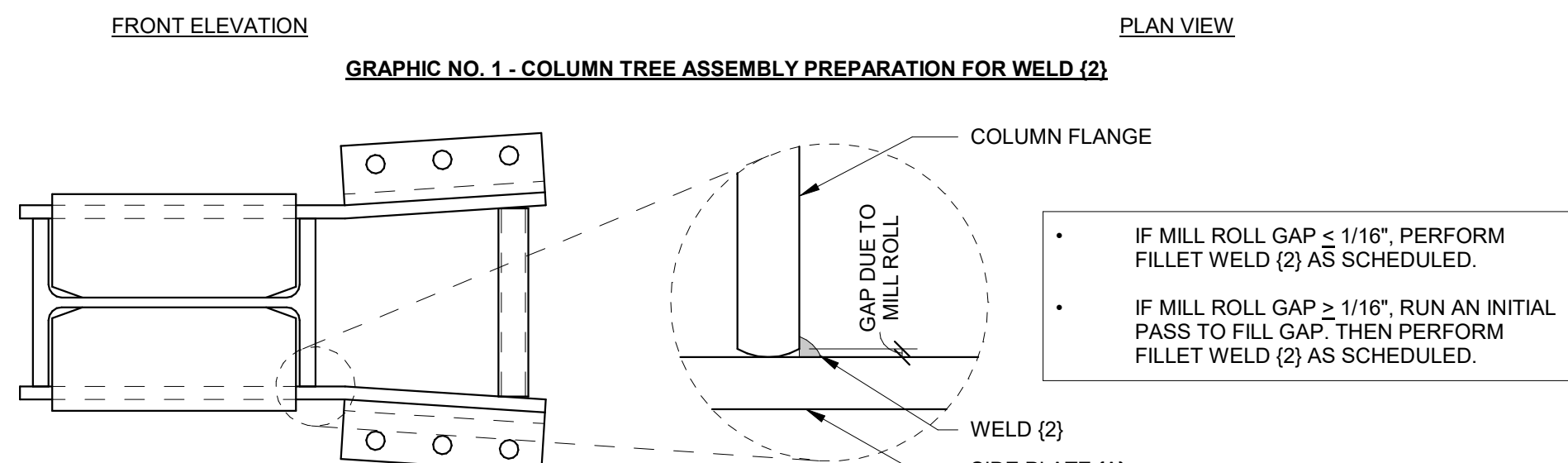


GRAPHIC NO. 1 - COLUMN TREE ASSEMBLY PREPARATION FOR WELD (2)

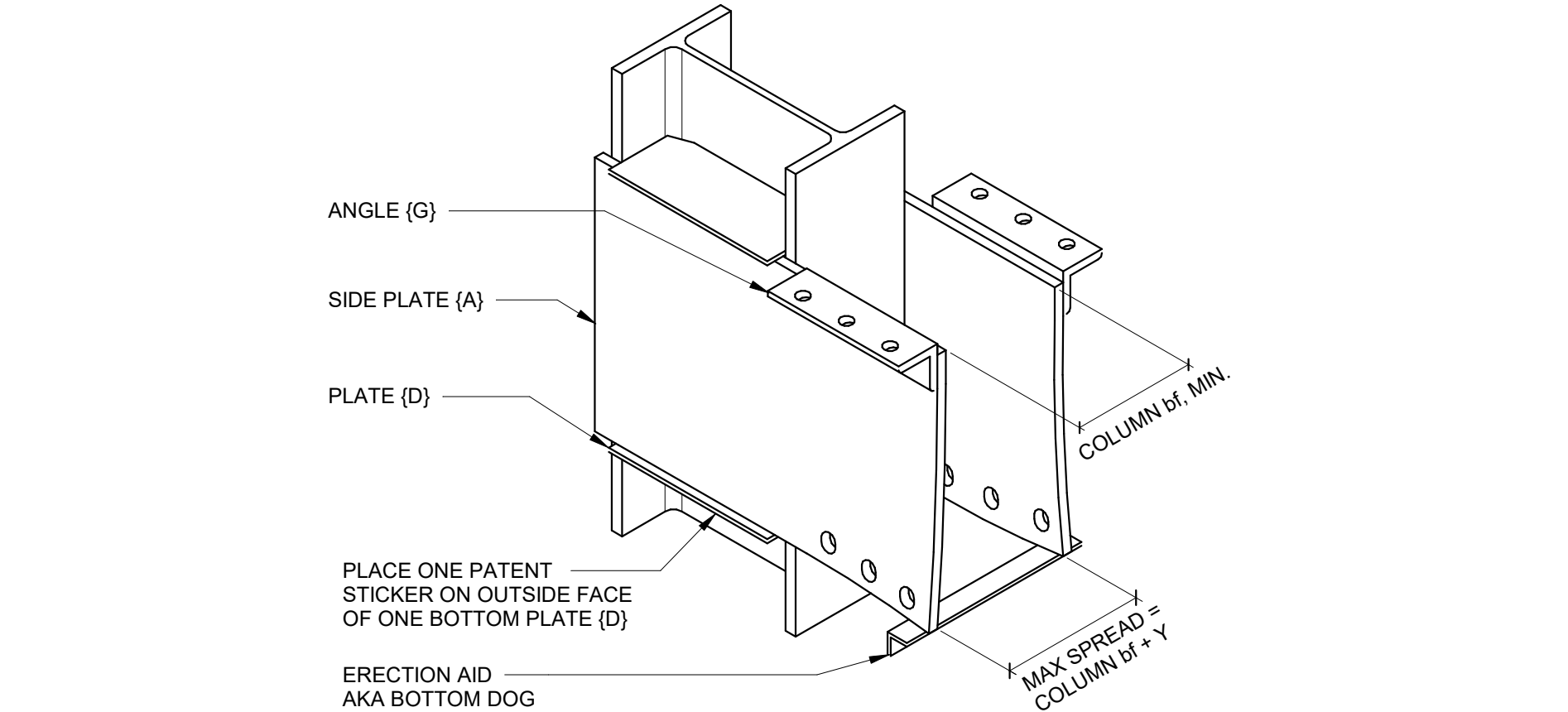
IF MILL ROLL GAP < 1/16", PERFORM FILLET WELD (2) AS SCHEDULED.
IF MILL ROLL GAP > 1/16", RUN AN INITIAL PASS TO FILL GAP, THEN PERFORM FILLET WELD (2) AS SCHEDULED.



GRAPHIC NO. 2 - MILL ROLL INDUCED GAP



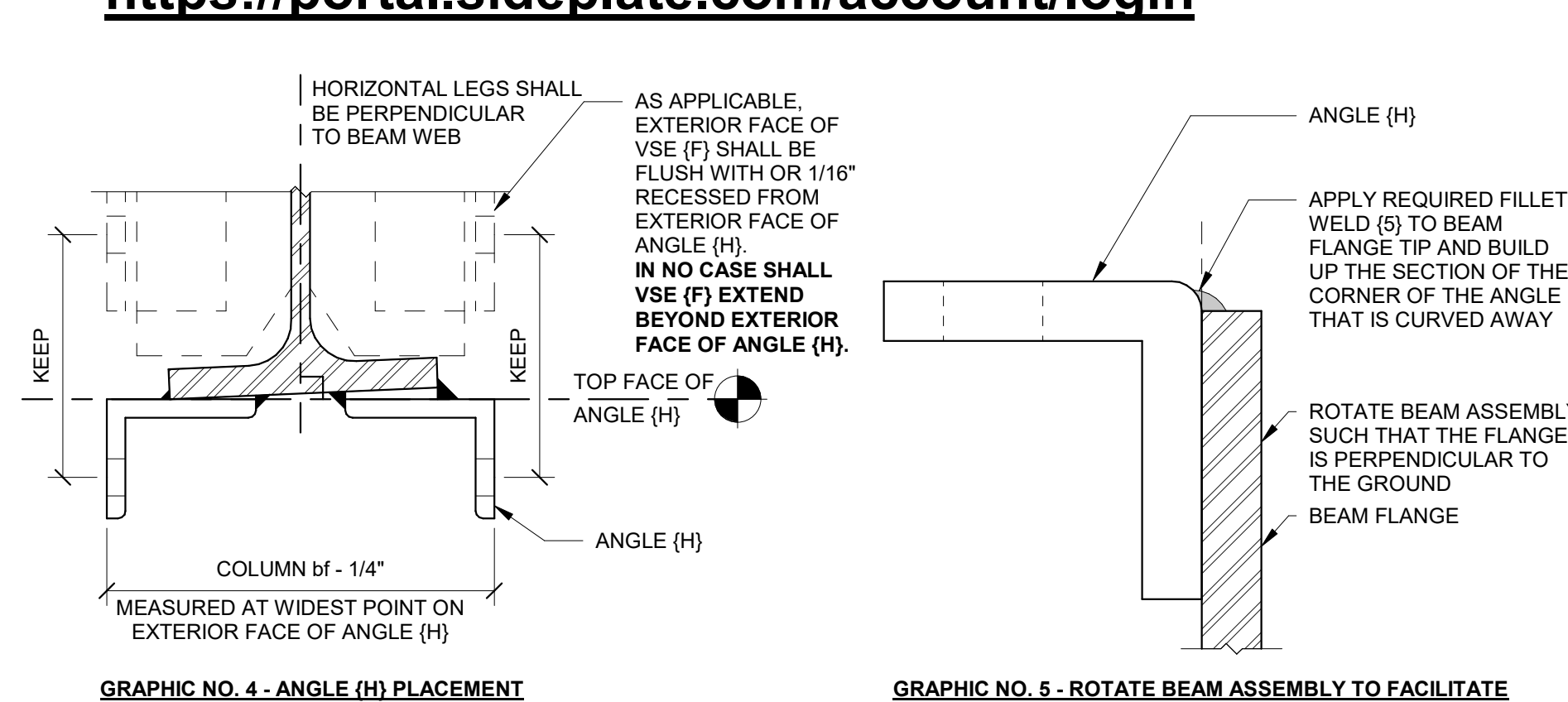
GRAPHIC NO. 3 - COMPLETED SIDEPLATE BOLTED COLUMN TREE ASSEMBLY



GRAPHIC NO. 4 - ANGLE (H) PLACEMENT

WATCH OUR SIDEPLATE BEAM ASSEMBLY VIDEO AT

https://portal.sideplate.com/account/login

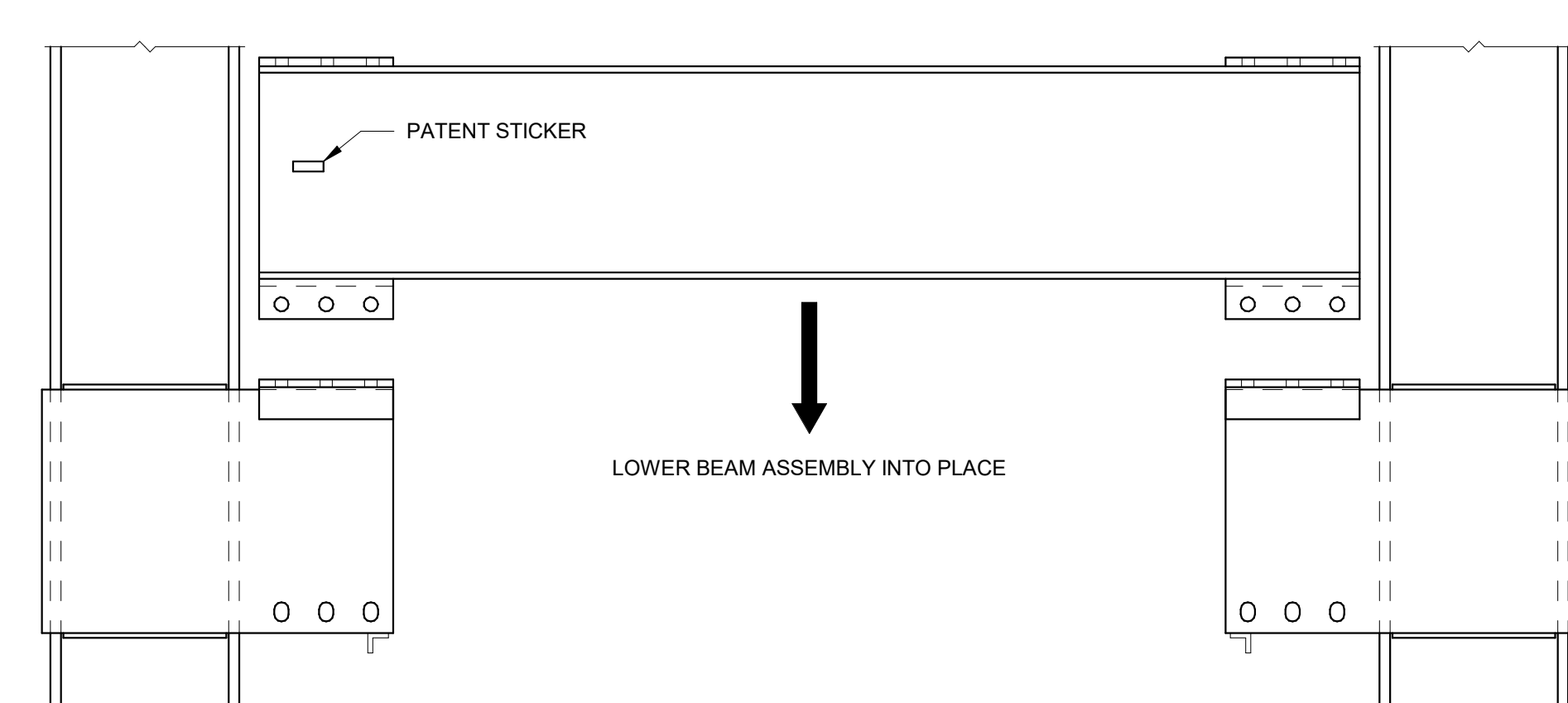


GRAPHIC NO. 5 - ROTATE BEAM ASSEMBLY TO FACILITATE WELD (6) MATERIAL AT MINIMAL EDGE DISTANCE

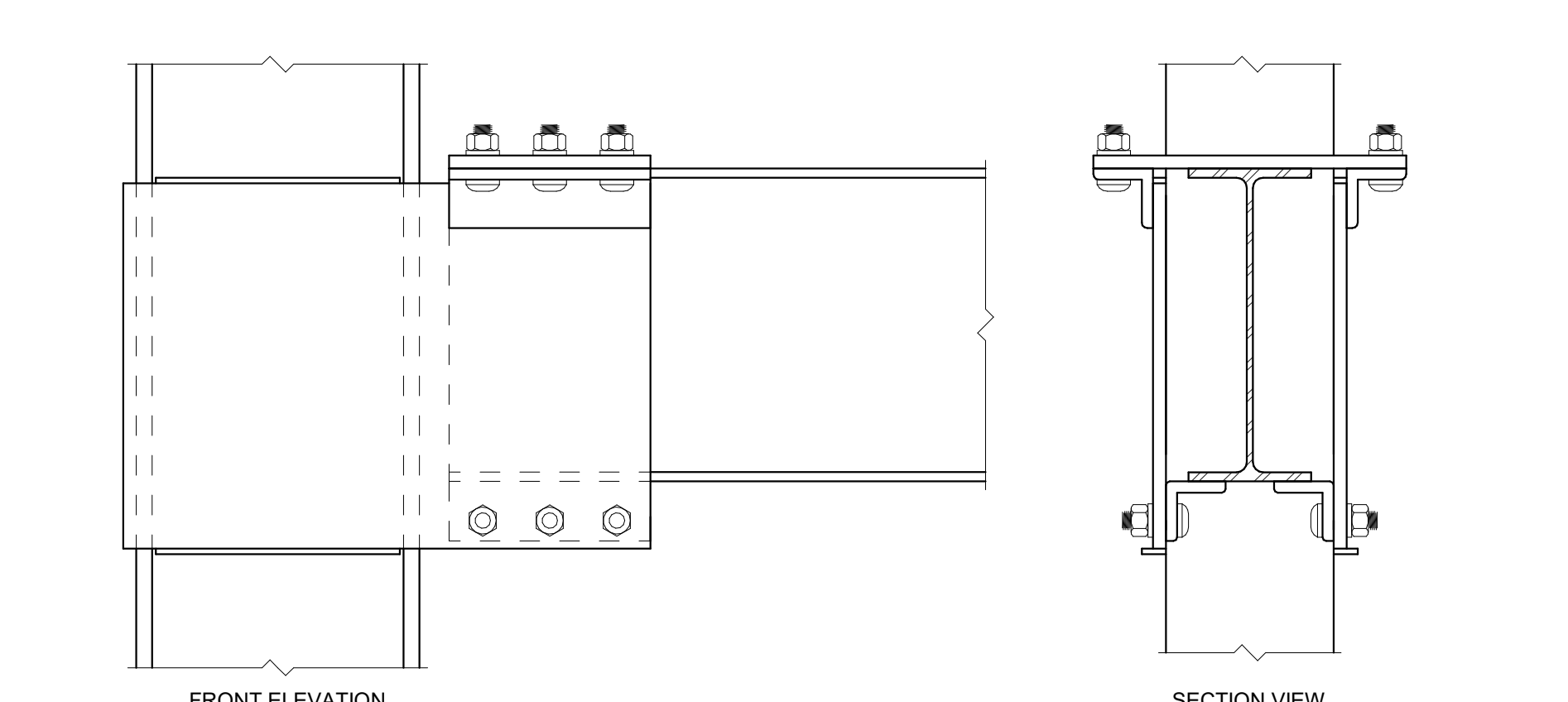
FIELD ERECTION OF SIDEPLATE BOLTED SYSTEM

- 3. WATCH OUR SIDEPLATE FIELD ERECTION VIDEO AT

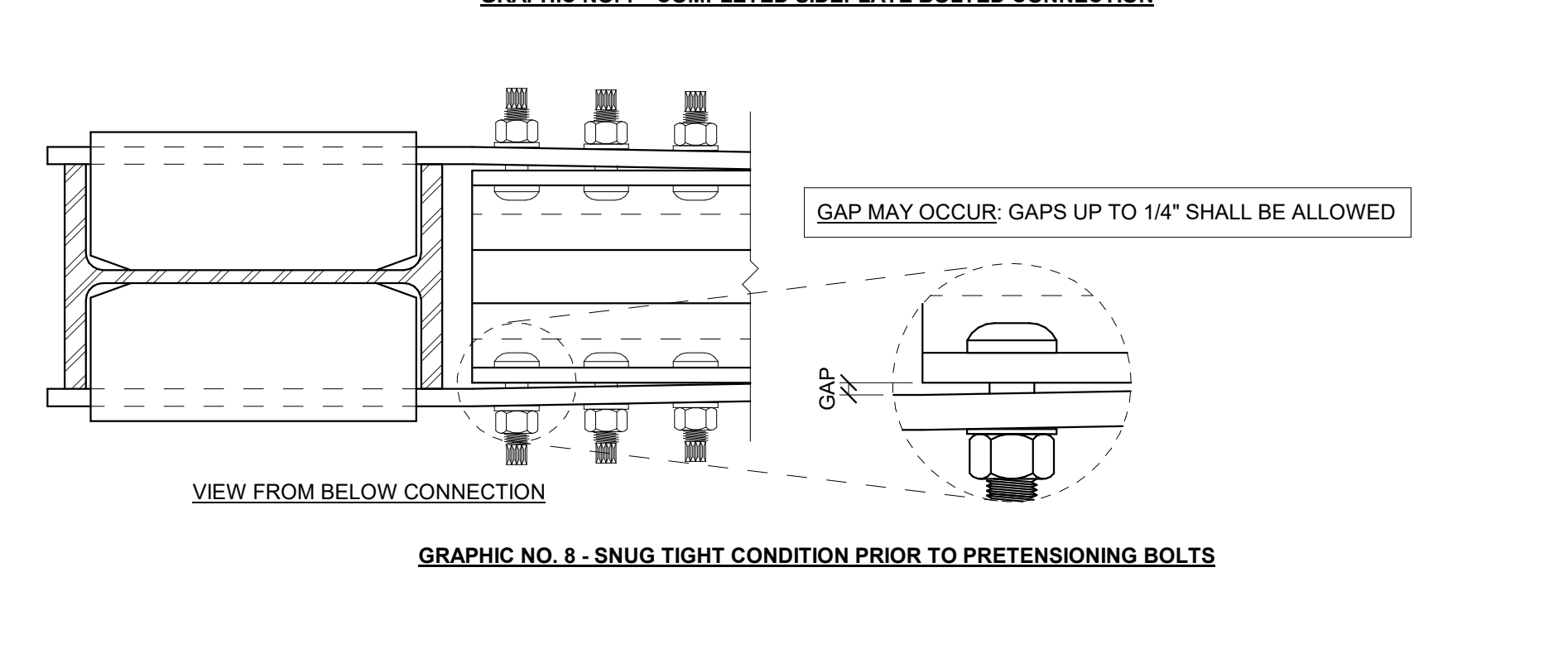
https://portal.sideplate.com/account/login



GRAPHIC NO. 6 - FIELD ERECTION OF SIDEPLATE BEAM ASSEMBLY



GRAPHIC NO. 7 - COMPLETED SIDEPLATE BOLTED CONNECTION



GRAPHIC NO. 8 - SNUG TIGHT CONDITION PRIOR TO PRETENSIONING BOLTS



GRAPHIC NO. 9 - TYPICAL GAP CLOSURE AT THE TOP OF THE GAP

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STILWELL, OKLAHOMA

KEY PLAN

PROJECT PHASE
STEEL DETAILS

REVISIONS
DATE DESCRIPTION
1 01/10/20 BID PACKAGE 01_ASI 01

DATE: 11-01-19
JOB NUMBER: 18-01.01

SHEET NUMBER: S8.01

SIDEPLATE GENERAL NOTES AND CONSTRUCTION GUIDELINES

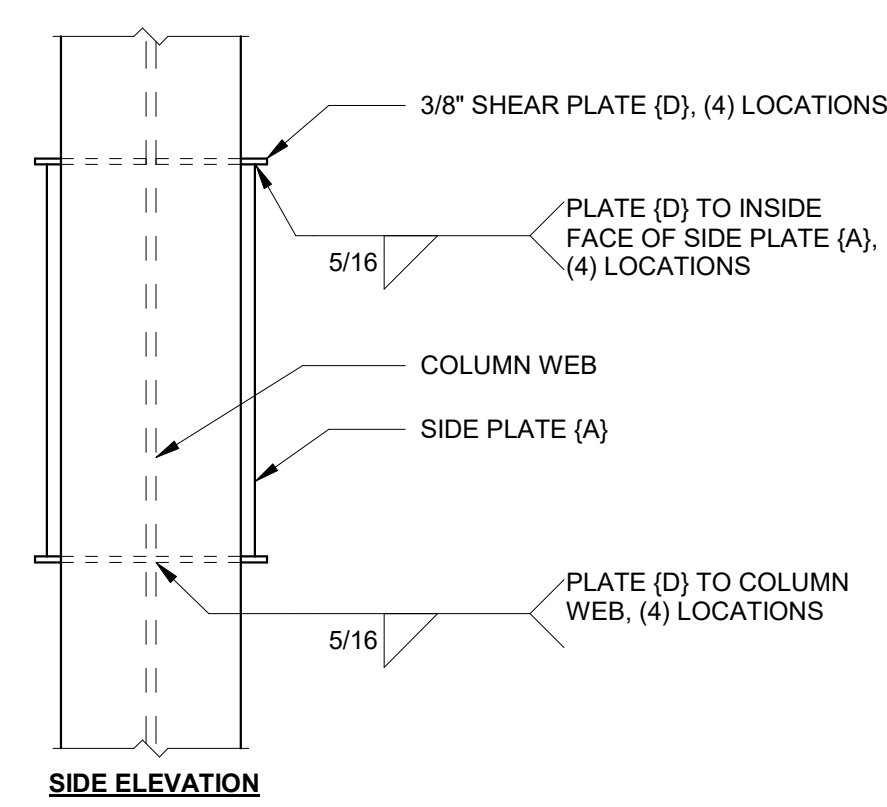
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THE SIDEPLATE® steel frame connection system is covered by one or more of U.S. Pat. Nos. 6,138,427; 6,516,883; 6,589,157; 7,170,296; 6,122,077; 6,122,078; 6,122,079; 6,122,080; 6,122,081; 6,122,082; 6,122,083; 6,122,084; 6,122,085; and foreign applications pending.
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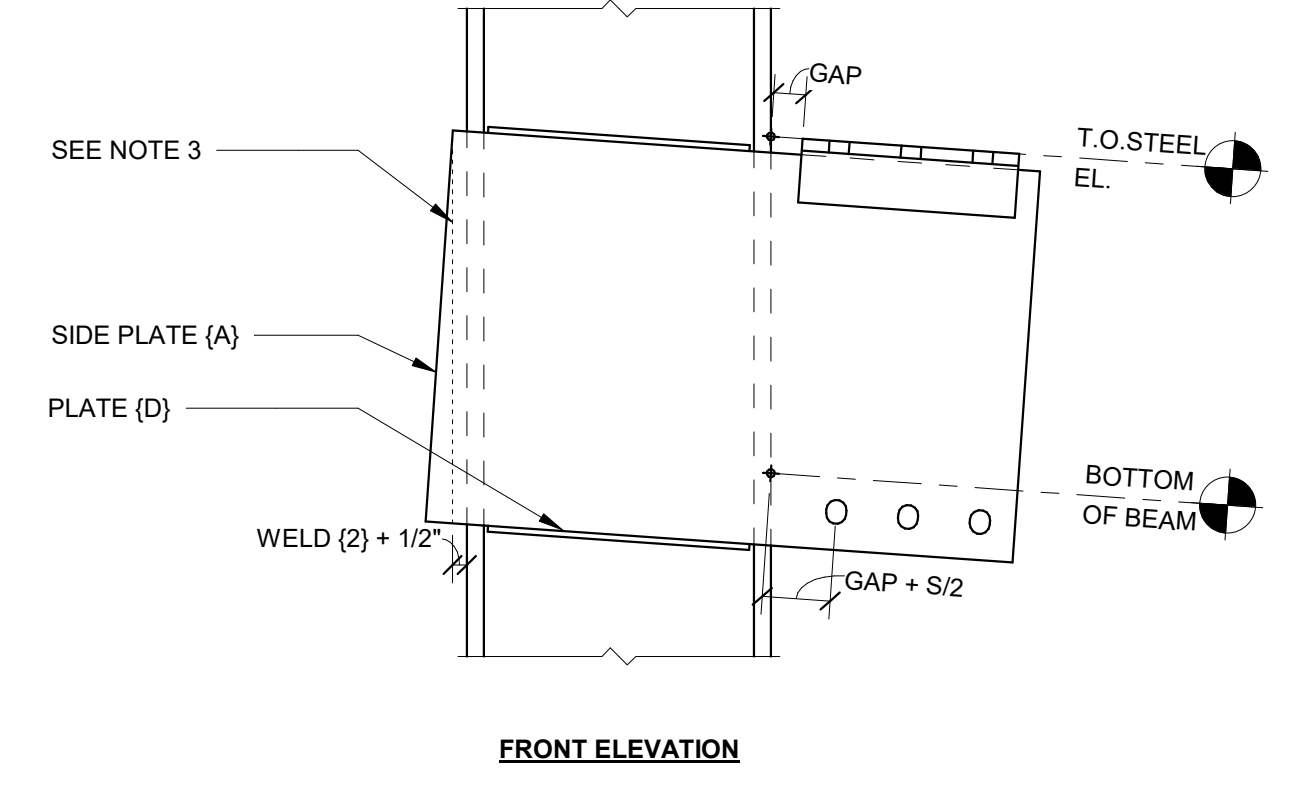
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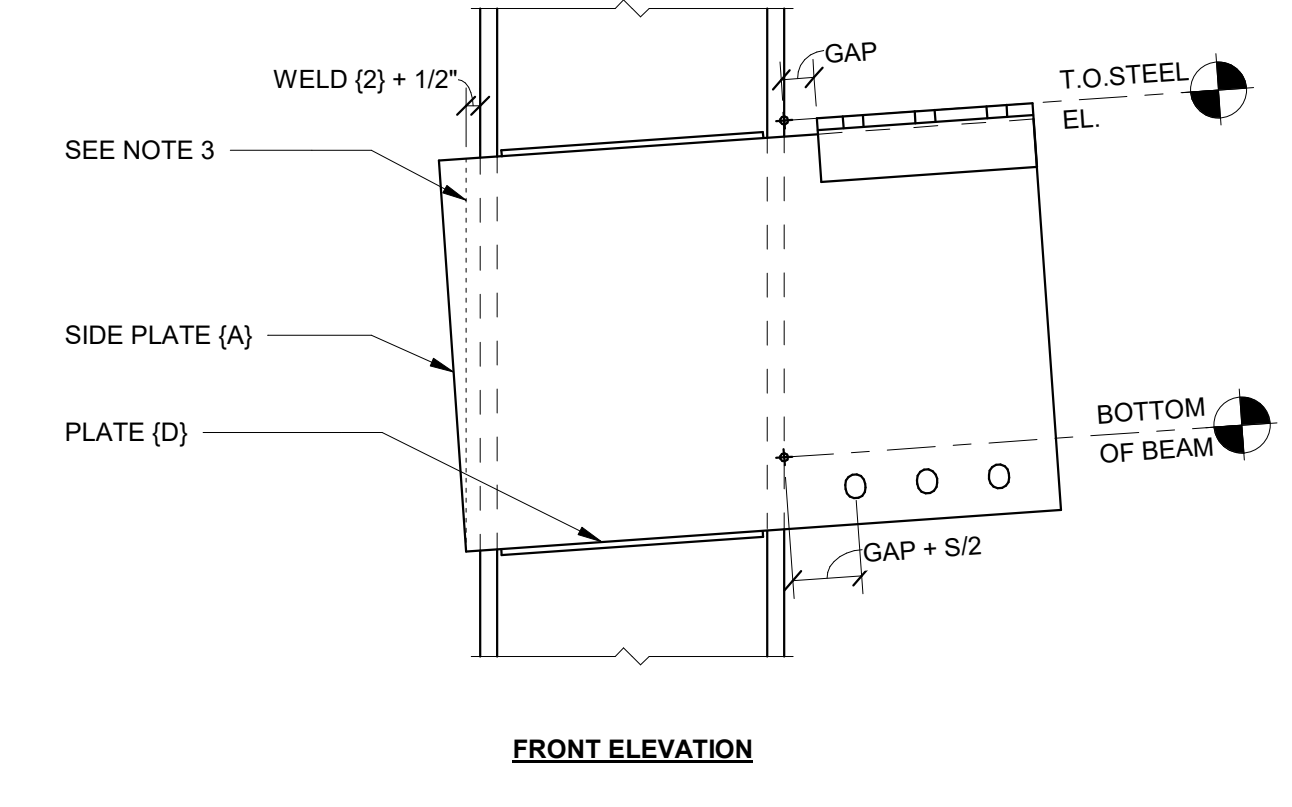
NOTE(S):
 1. LONGITUDINAL ANGLES (G) NOT SHOWN FOR CLARITY.

8 PLATE (D) DETAIL FOR SLOPED CONDITIONS
 N.T.S.



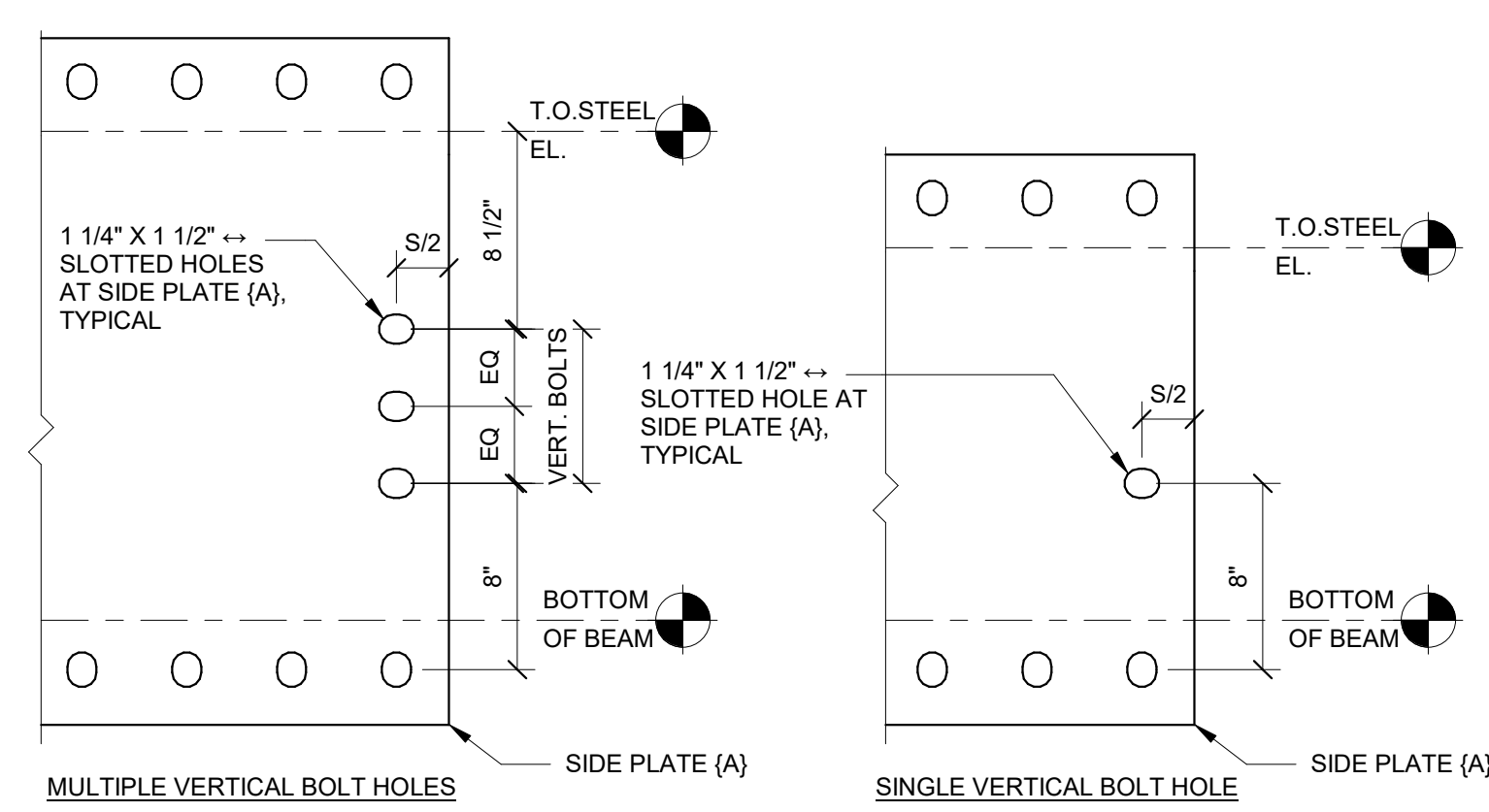
NOTE(S):
 1. FOR BEAM SLOPES > 1" PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC.
 2. COORDINATE PLATES, ANGLES, AND DIMENSIONS WITH RESPECT TO THE SLOPE OF THE CONNECTION.
 3. AT CONTRACTOR'S DISCRETION, SIDE PLATE (A) MAY BE CUT AS SHOWN.
 4. HORIZONTAL SHEAR PLATES (D) AND ASSOCIATED WELDS ARE REQUIRED FOR SLOPED SIDE PLATE CONDITIONS. SEE 8 / S8.02

4 SLOPED DOWN CONNECTION (AS APPLICABLE)
 N.T.S.



NOTE(S):
 1. FOR BEAM SLOPES > 1" PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC.
 2. COORDINATE PLATES, ANGLES, AND DIMENSIONS WITH RESPECT TO THE SLOPE OF THE CONNECTION.
 3. AT CONTRACTOR'S DISCRETION, SIDE PLATE (A) MAY BE CUT AS SHOWN.
 4. HORIZONTAL SHEAR PLATES (D) AND ASSOCIATED WELDS ARE REQUIRED FOR SLOPED SIDE PLATE CONDITIONS. SEE 8 / S8.02

3 SLOPED UP CONNECTION (AS APPLICABLE)
 N.T.S.



NOTE(S):
 1. SEE COLUMN SCHEDULE FOR BOLT QUANTITY.

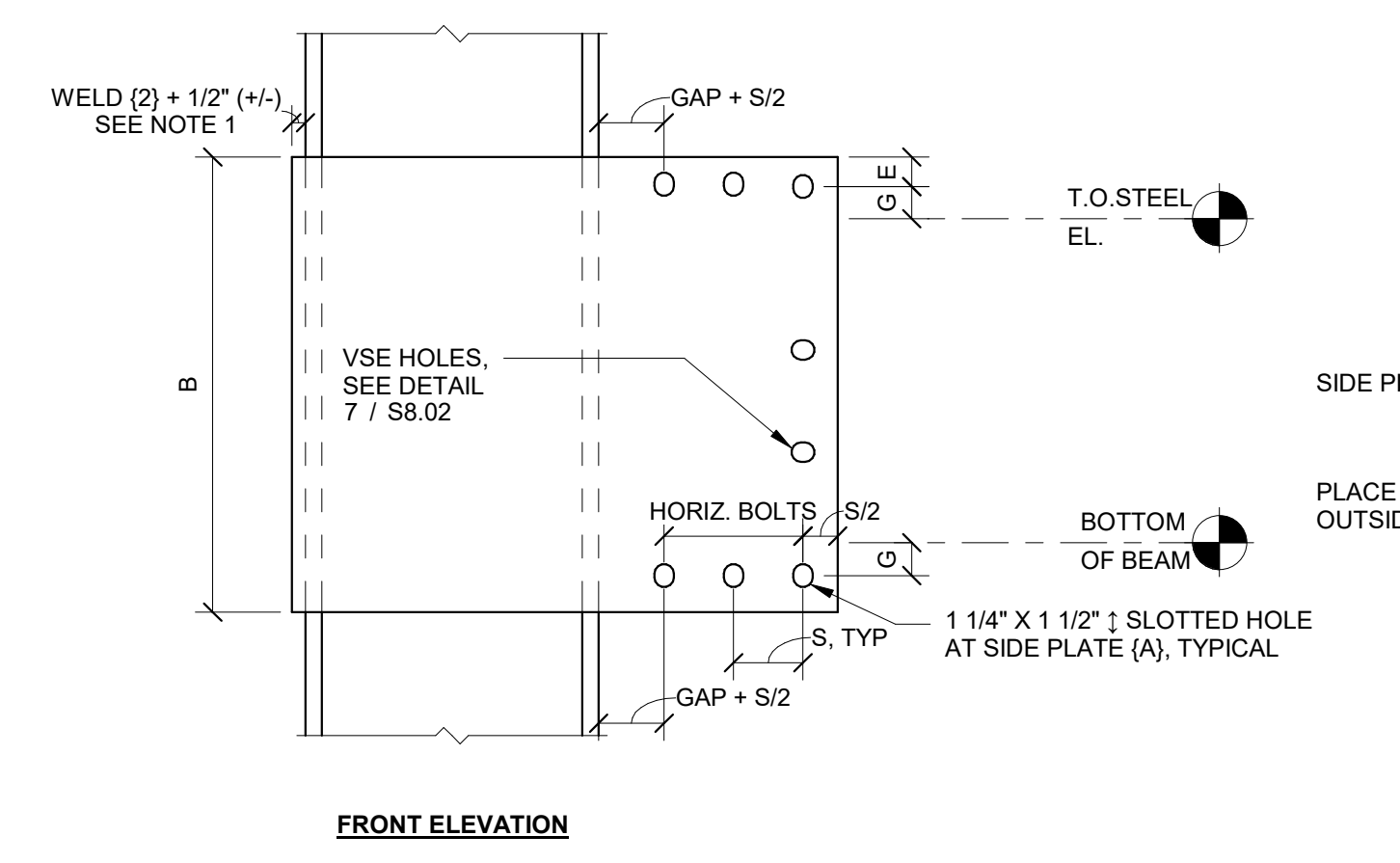
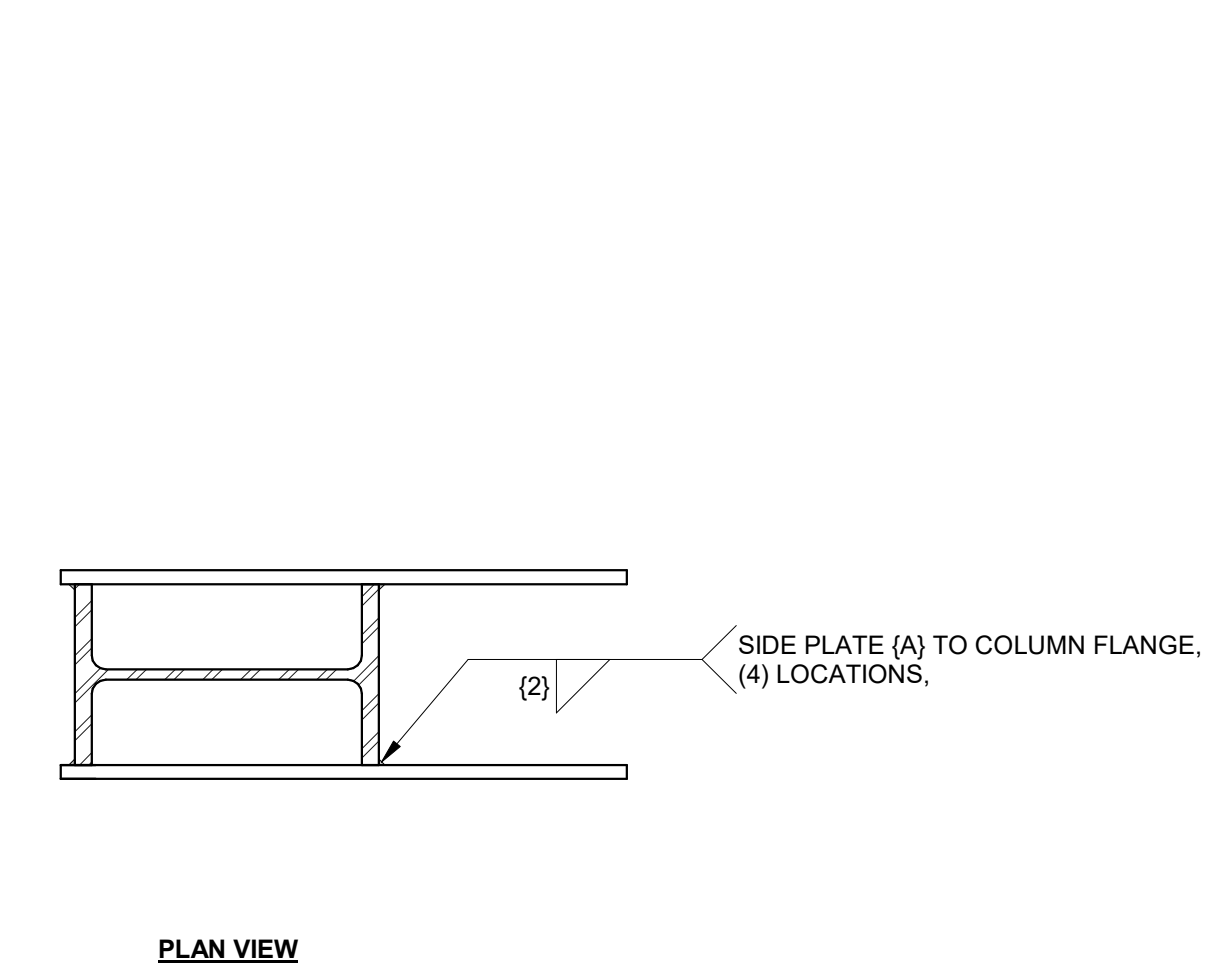
7 SIDE PLATE (A) VSE BOLT HOLE DETAIL
 N.T.S.

ID	COLUMN PANEL ZONE DESIGN (INCHES)				SIDE PLATE (A) EXTENSION DESIGN (INCHES)								
	SERIES	WELD (2)	BEAM SHAPE	GAP	PLATE (A)			BOLT					
					THICKNESS	B	E	Y	DIAMETER	HORIZONTAL #	VERTICAL #	G	S
A15	W14x	3/8	W24X68	2	5/8	31 3/4	1 3/8	2 1/2	1 1/8	4	2	2 1/8	4 1/2
A25	W14x	3/8	W24X94	2	5/8	32 1/4	1 3/8	3 5/8	1 1/8	5	2	2 1/8	4 1/2
A45	W14x	3/8	W36X160	2	5/8	44	1 3/8	5	1 1/8	6	3	2 1/8	4 1/2

6 A TYPE NARROW COLUMN CONNECTION SCHEDULE
 N.T.S.

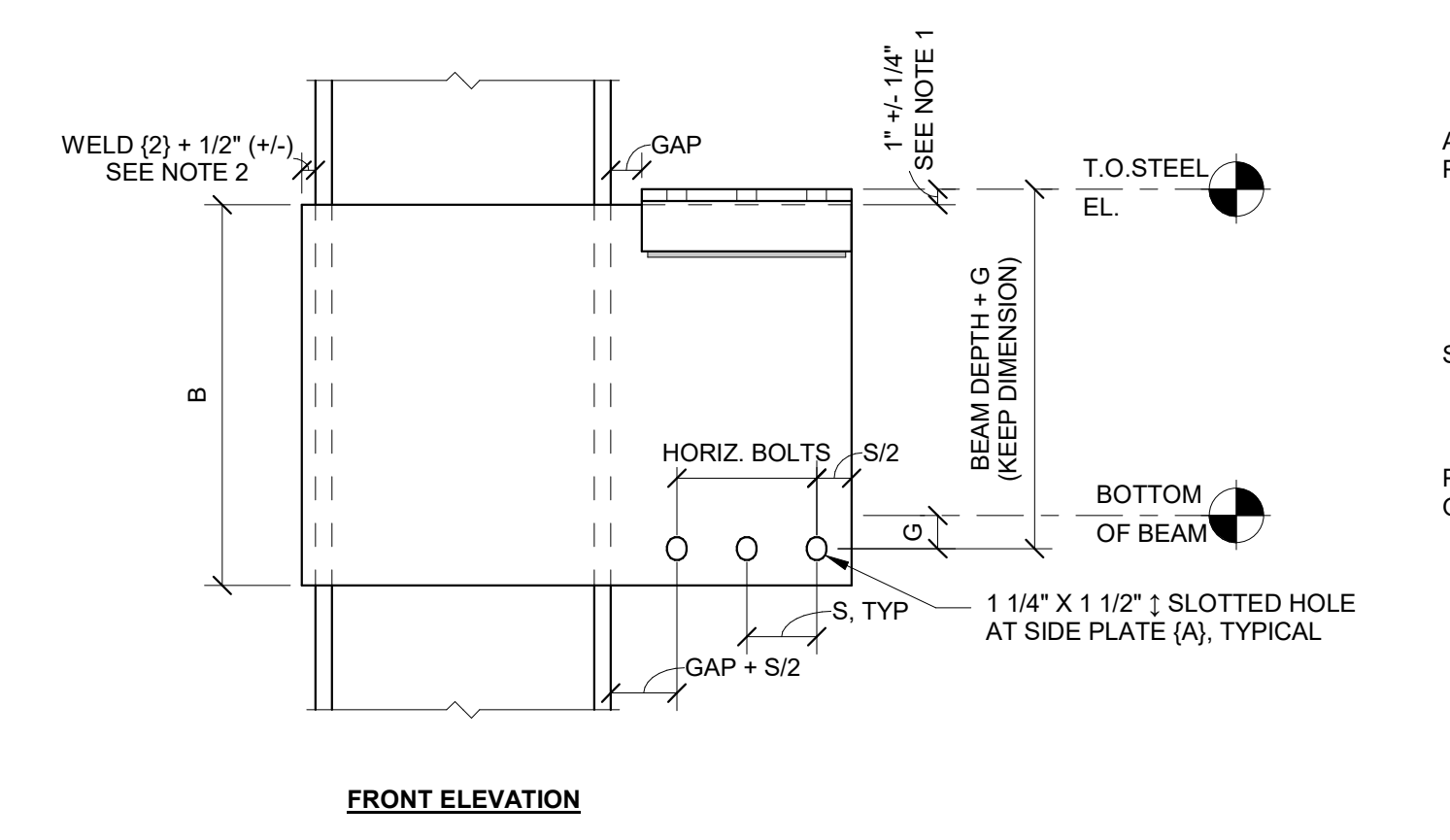
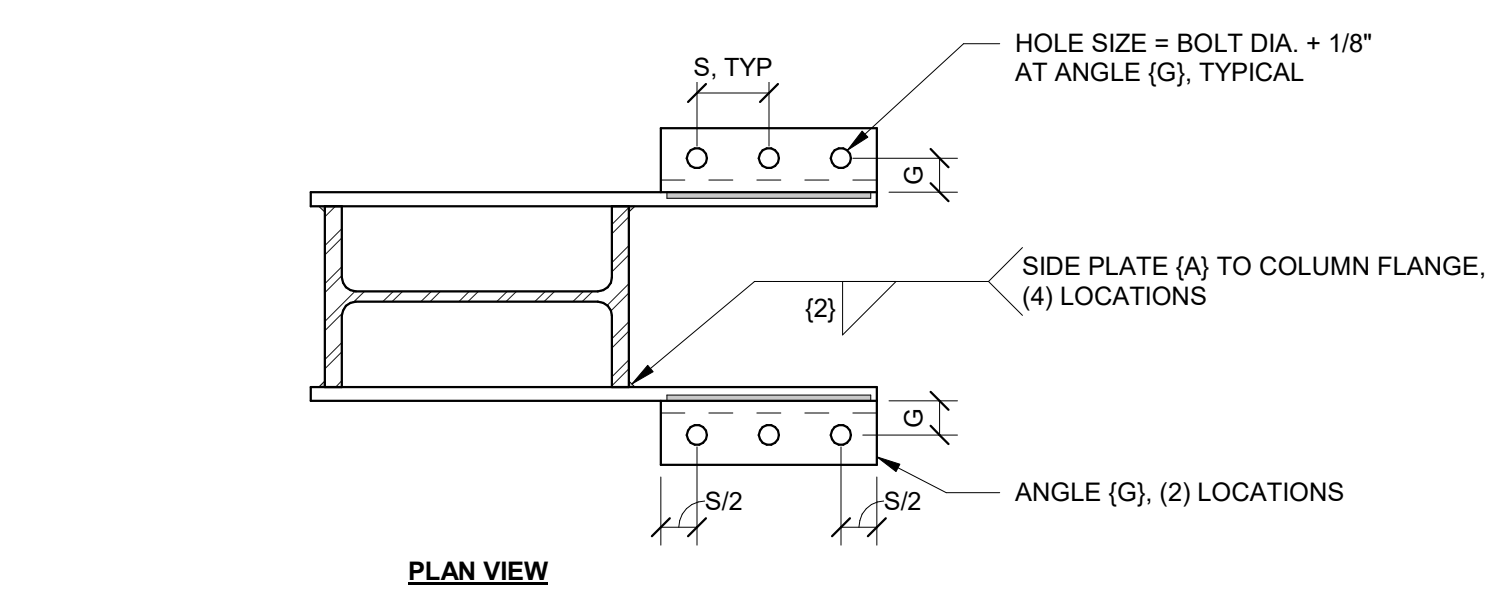
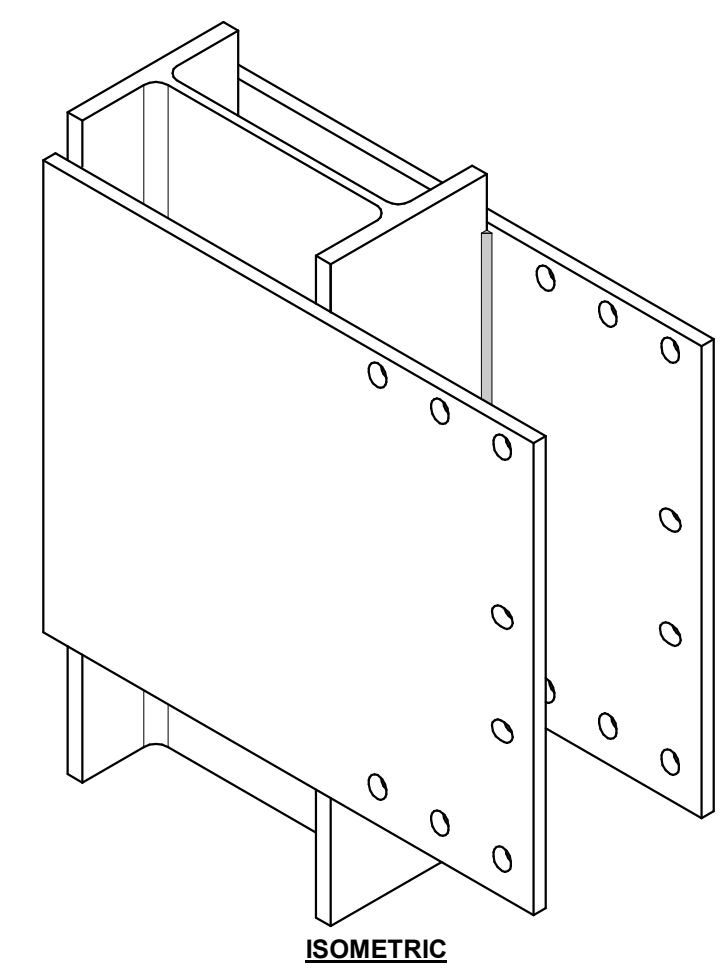
ID	COLUMN PANEL ZONE DESIGN (INCHES)				SIDE PLATE (A) EXTENSION DESIGN (INCHES)										
	SERIES	WELD (2)	BEAM SHAPE	GAP	PLATE (A)			ANGLE (G)				BOLT			
					THICKNESS	B	Y	SUGGESTED SIZE	HORIZONTAL LEG	VERTICAL LEG	SIZE	DIAMETER	HORIZONTAL #	G	S
A10, A11, A19	W14x	3/8	W24X68	2	5/8	27 1/4	2 1/2	L5X3-1/2X5/8	3-1/2 to 6	4 to 6	5/16	1 1/8	4	2 1/8	4 1/2
A12	W14x	7/16	W24X68	2 1/4	1	27 1/4	1 7/8	L5X3-1/2X5/8	3-1/2 to 6	4 to 6	5/16	1 1/8	4	2 1/8	4 1/2
A13	W14x	3/8	W24X68	2	3/4	27 1/4	2 1/8	L5X3-1/2X5/8	3-1/2 to 6	4 to 6	5/16	1 1/8	4	2 1/8	4 1/2
A20	W14x	3/8	W24X94	2	7/8	27 3/4	2 7/8	L5X3-1/2X5/8	3-1/2 to 6	4 to 6	5/16	1 1/8	5	2 1/8	4 1/2
A30	W14x	3/8	W36X150	2	5/8	39 3/8	5	L5X3-1/2X5/8	3-1/2 to 6	4 to 6	5/16	1 1/8	6	2 1/8	4 1/2

2 A TYPE COLUMN CONNECTION SCHEDULE
 N.T.S.



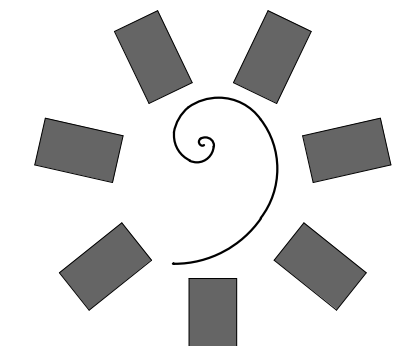
NOTE(S):
 1. THE 1/2 INCH OVERHANG ON THE SIDE PLATE (A) IS TO ENSURE SUFFICIENT ROOM FOR WELD (2). THE +/- TOLERANCE IS APPLIED SO THAT IF DESIRED, THE DETAILER CAN MAKE THE SIDE PLATES (A) THE SAME LENGTH WITH SLIGHTLY VARYING COLUMN DEPTHS WITHIN A GROUP OF THE SAME CONNECTION ID'S.

5 A TYPE NARROW BOLTED CONNECTION
 N.T.S.

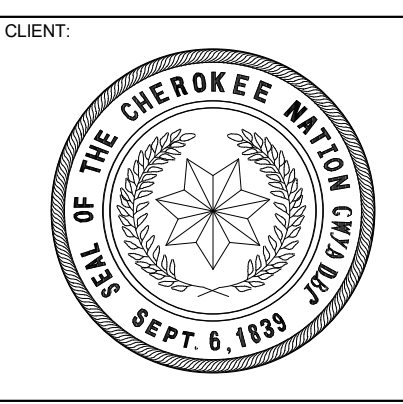
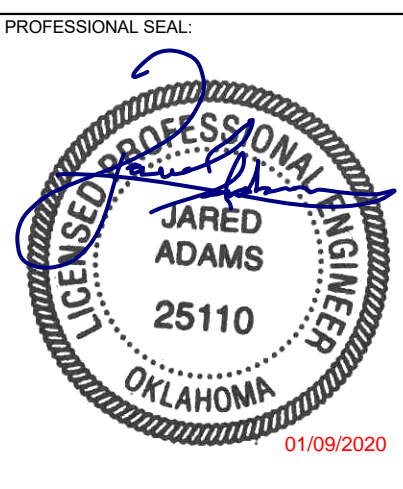


NOTE(S):
 1. THE +/- 1/4 INCH TOLERANCE FOR PLACEMENT OF ANGLES (G) IS TO ENSURE CORRECT TOP OF STEEL PLACEMENT RELATIVE TO THE CENTERLINE OF THE BOTTOM HORIZONTAL ROW OF BOLT HOLES. THE PLACEMENT OF ANGLES (G) SHALL NEVER BE MEASURED FROM THE BOTTOM EDGE OF SIDE PLATE (A) TO ESTABLISH THE CORRECT TOP OF STEEL.
 2. THE 1/2 INCH OVERHANG ON THE SIDE PLATE (A) IS TO ENSURE SUFFICIENT ROOM FOR WELD (2). THE +/- TOLERANCE IS APPLIED SO THAT IF DESIRED, THE DETAILER CAN MAKE THE SIDE PLATES (A) THE SAME LENGTH WITH SLIGHTLY VARYING COLUMN DEPTHS WITHIN A GROUP OF THE SAME CONNECTION ID'S.

4 A TYPE BOLTED CONNECTION
 N.T.S.



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**WILMA P. MANKILLER HEALTH CENTER
 EXPANSION
 STILWELL, OKLAHOMA**

KEY PLAN:

PROJECT PHASE:
STEEL DETAILS

#	DATE	REVISIONS
1	01/19/20	BID PACKAGE 01 - ASI 01

DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER: **S8.02**

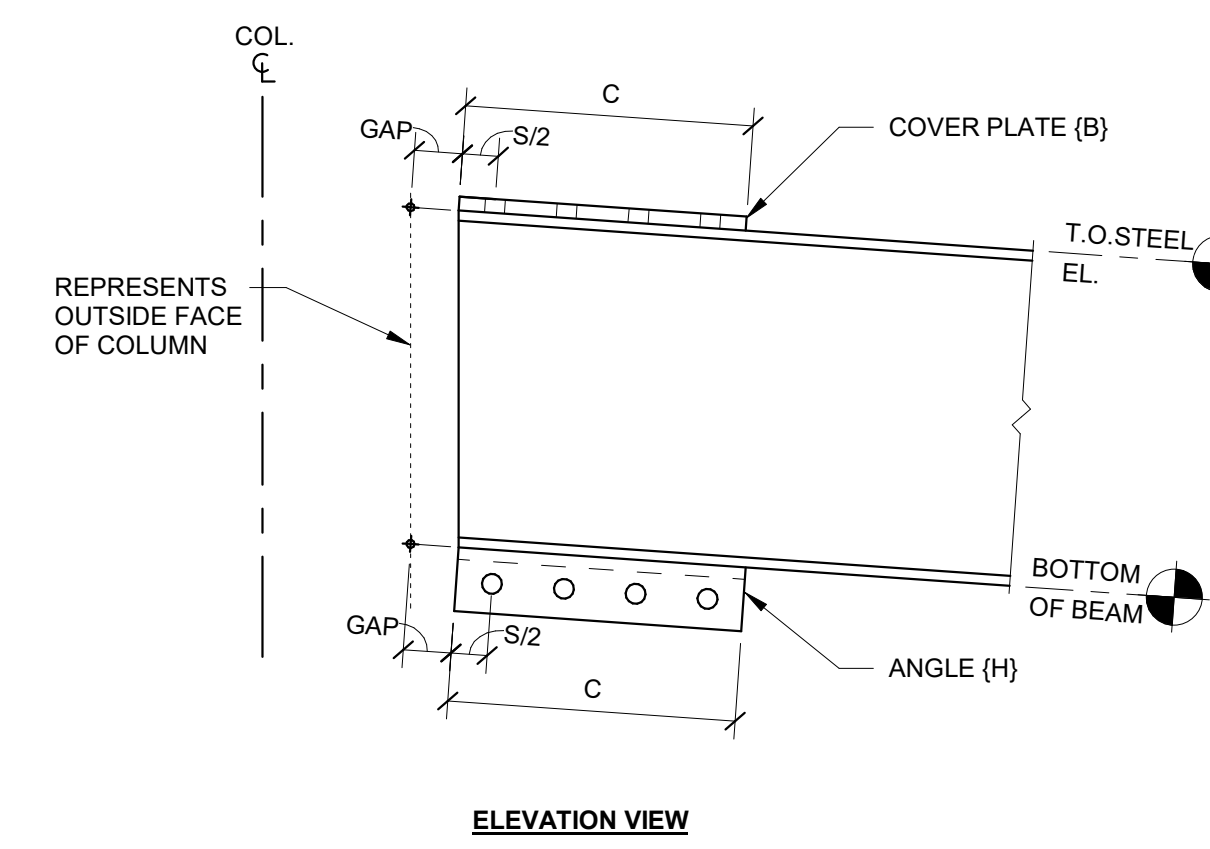
SIDEPLATE COLUMN DETAILS, A TYPE

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 The SIDEPLATE® steel frame connection system is covered by one or more of U.S. Pat. Nos. 6,138,427; 6,516,583; 6,591,573; 7,178,296; 8,122,671; 8,122,672; 8,140,322; 8,176,706; 8,205,408; and 9,091,065 and foreign counterparts. Other U.S. and foreign applications pending.

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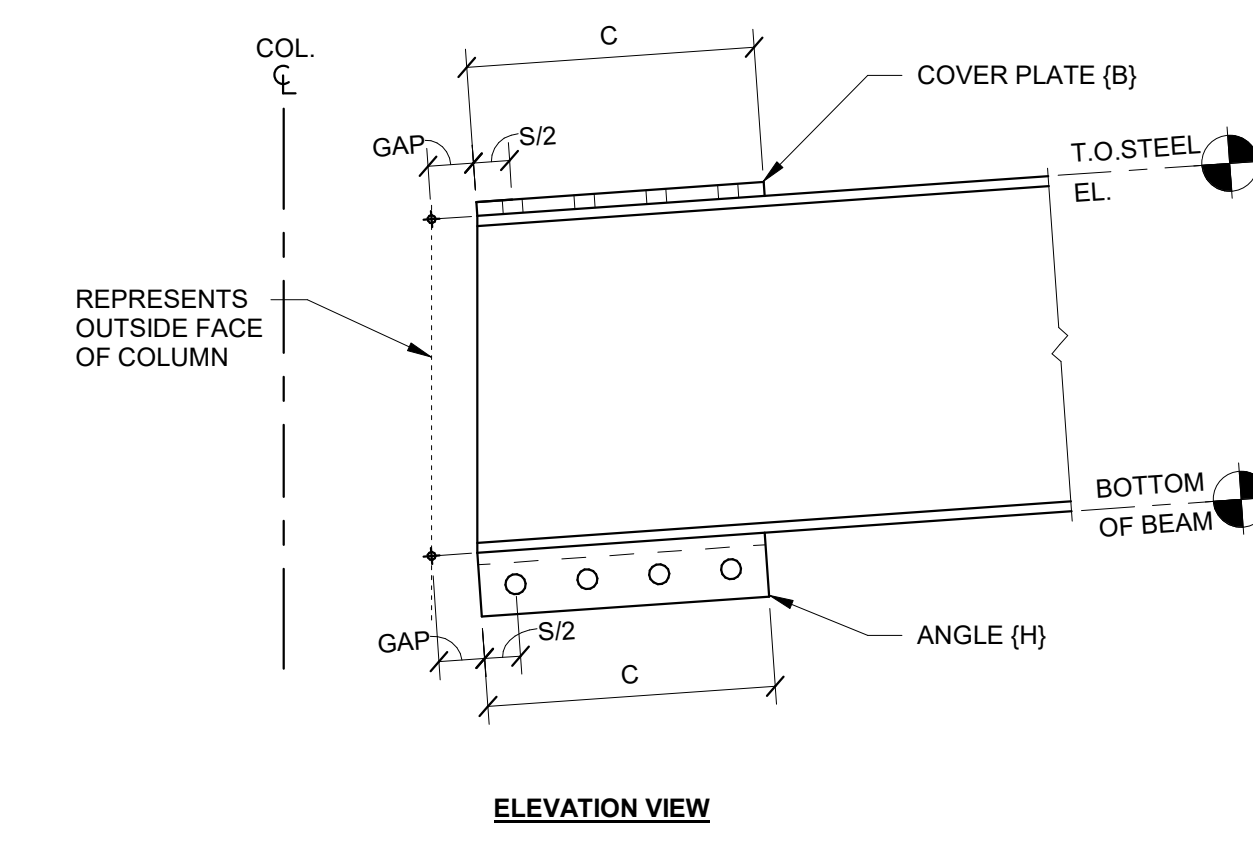
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vis.06.02 3/20/19/PTD



NOTE(S):
 1. FOR BEAM SLOPES > 1" PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC.

4 SLOPED DOWN BEAM END (AS APPLICABLE)
 N.T.S.

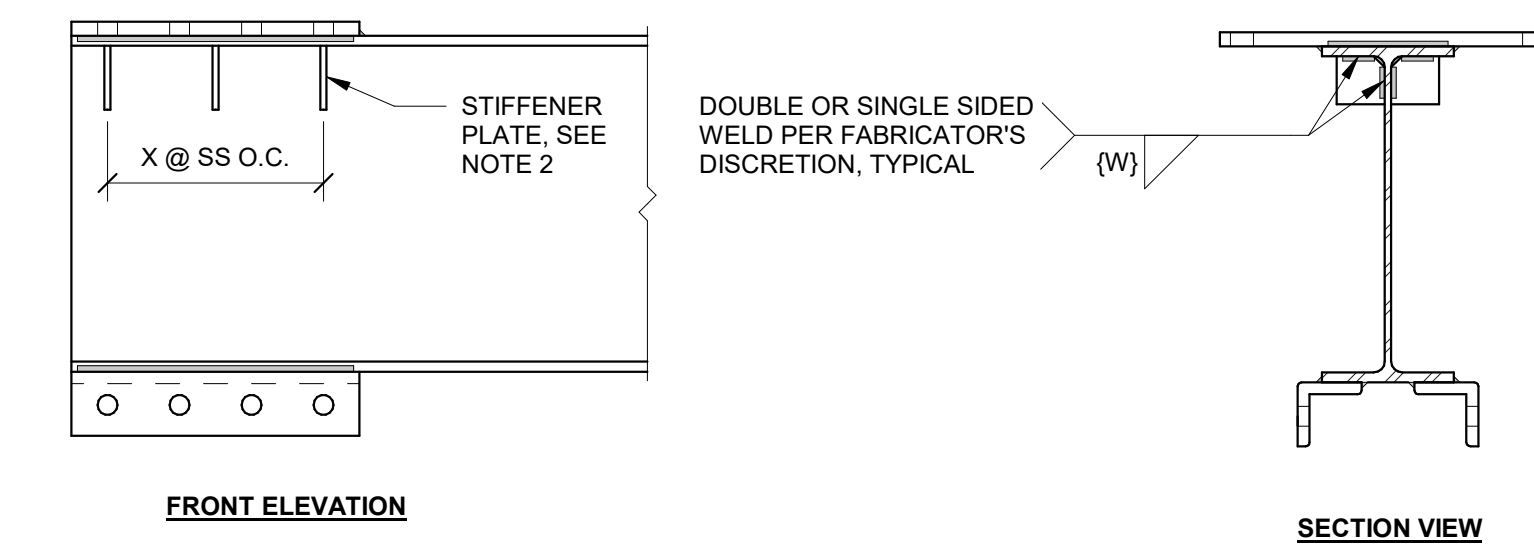


NOTE(S):
 1. FOR BEAM SLOPES > 1" PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC.

3 SLOPED UP BEAM END (AS APPLICABLE)
 N.T.S.

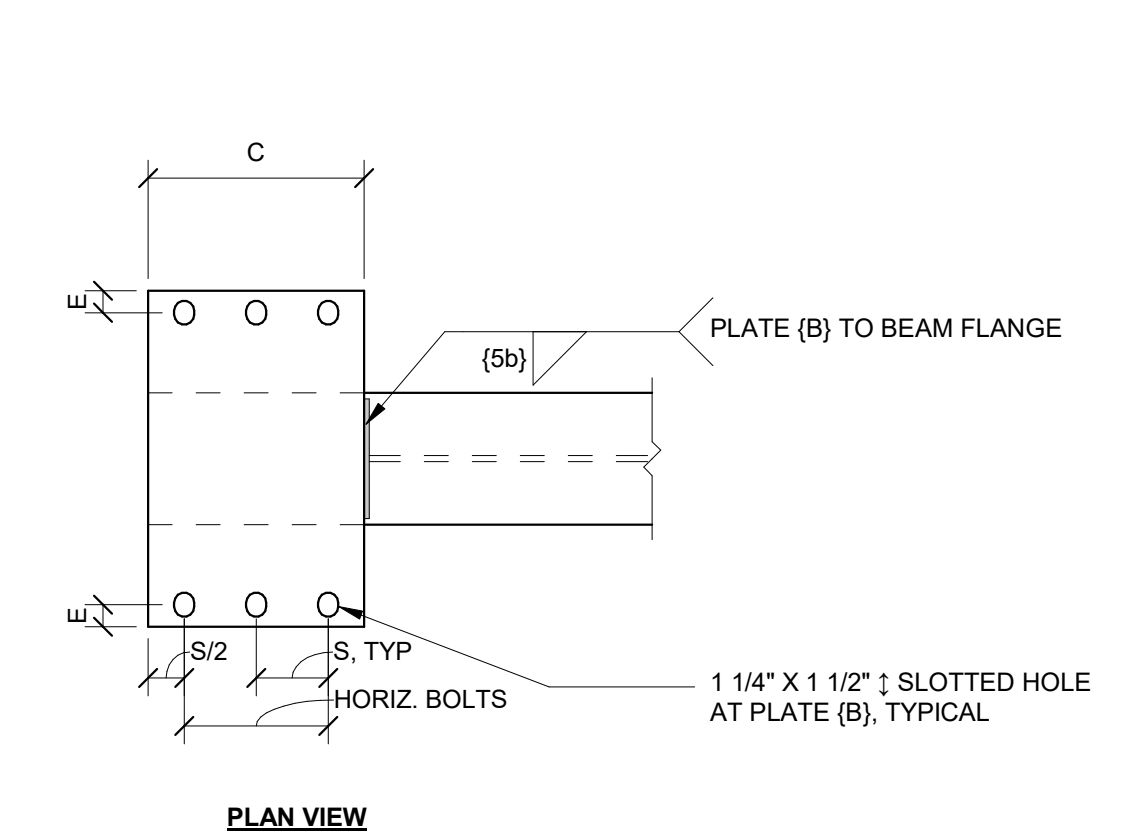
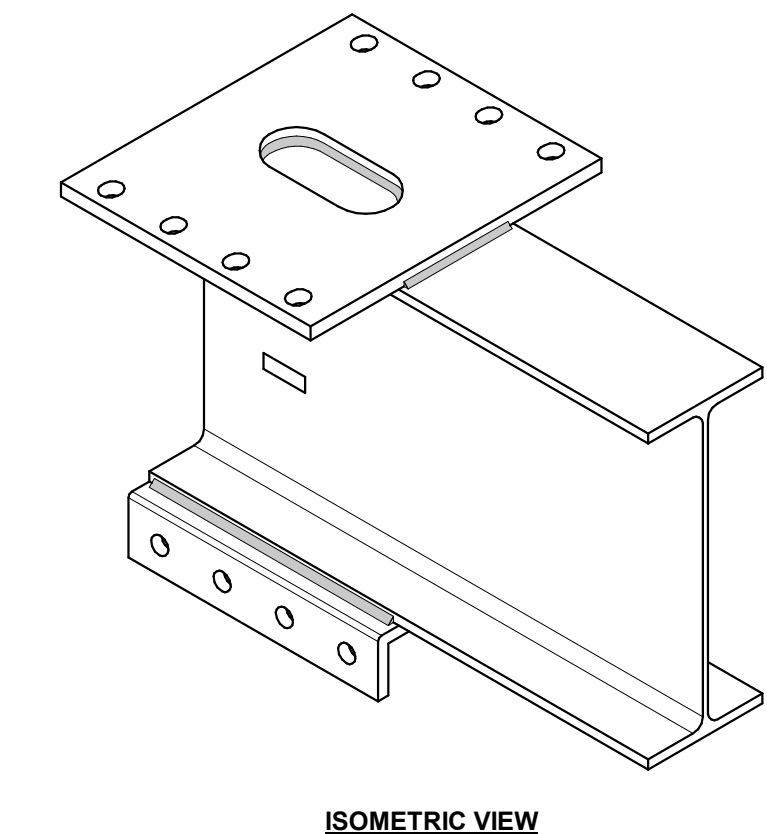
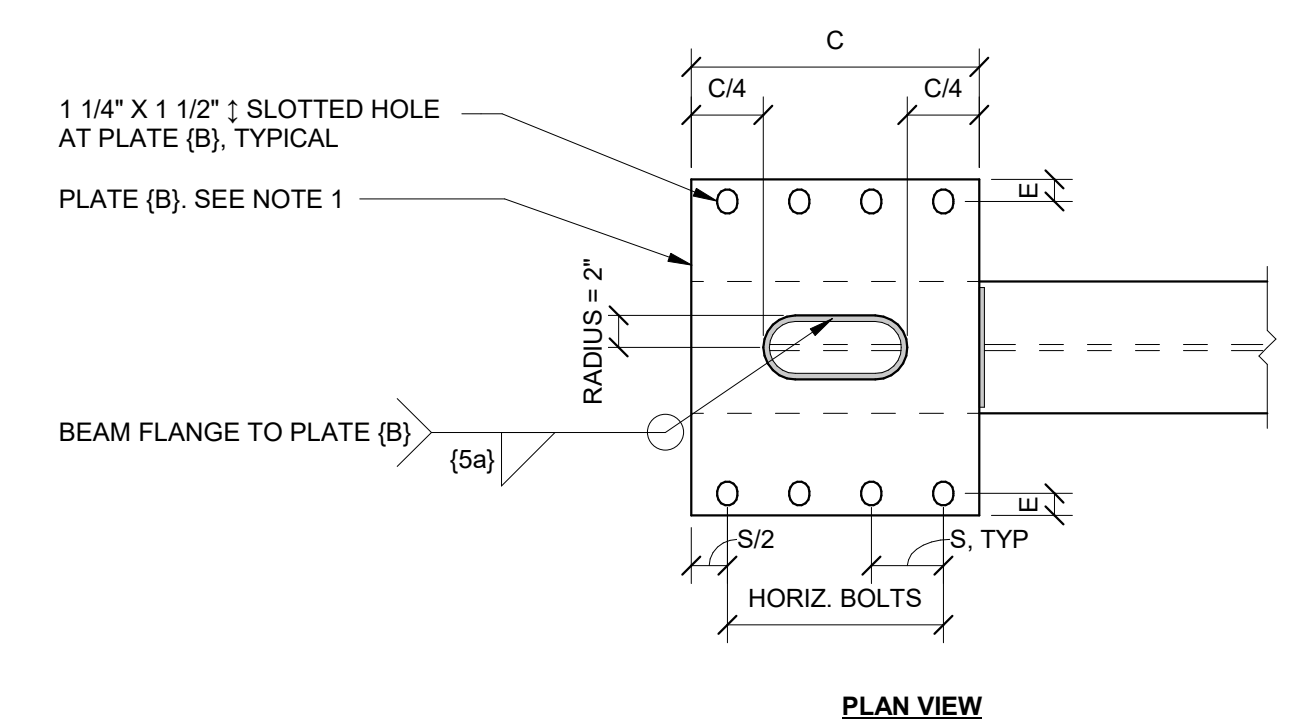
ID	BEAM DESIGN (INCHES)																						
	BEAM		PLATE						ANGLE				WELD				BOLT						
	SHAPE	GAP	(B)		STIFFENER (SEE DETAIL 6 THIS SHEET)				(H)		(5)	(5a)	(5b)	(W) (SEE DETAIL 6 THIS SHEET)		DIAMETER	HORIZONTAL #	G	S				
A10	W24X68	2	Slotted	1 1/8	1 3/8	8 1/4	-	-	-	-	L6X4X5/8	18	6	4	5/16	5/16	5/16	-	-	1 1/8	4	2 1/8	4 1/2
A11, B11	W24X68	2	Slotted	3/4	1 3/8	8 1/4	-	-	-	-	L7X4X5/8	18	7	4	5/16	5/16	5/16	-	-	1 1/8	4	2 1/8	4 1/2
A12, B12	W24X68	2 1/4	Slotted	3/4	1 3/8	9	-	-	-	-	L7X4X5/8	18	7	4	5/16	5/16	5/16	-	-	1 1/8	4	2 1/8	4 1/2
A13	W24X68	2	Slotted	7/8	1 3/8	8 1/2	-	-	-	-	L7X4X5/8	18	7	4	5/16	5/16	5/16	-	-	1 1/8	4	2 1/8	4 1/2
A19, B19	W24X68	2	Slotted	1	1 3/8	8 1/4	1/4	4 X 4	3	6 3/4	L6X4X5/8	18	6	4	5/16	5/16	5/16	1/4	1/8	1 1/8	4	2 1/8	4 1/2
A20, B20	W24X94	2	Slotted	1 1/4	1 3/8	8 3/4	-	-	-	-	L6X4X5/8	22 1/2	6	4	5/16	5/16	5/16	-	-	1 1/8	5	2 1/8	4 1/2
A30, B30	W36X150	2	Slotted	1 1/4	1 3/8	8 1/4	-	-	-	-	L5X3-1/2X5/8	27	5	3 1/2	5/16	5/16	5/16	-	-	1 1/8	6	2 1/8	4 1/2

2 BEAM END SCHEDULE
 N.T.S.



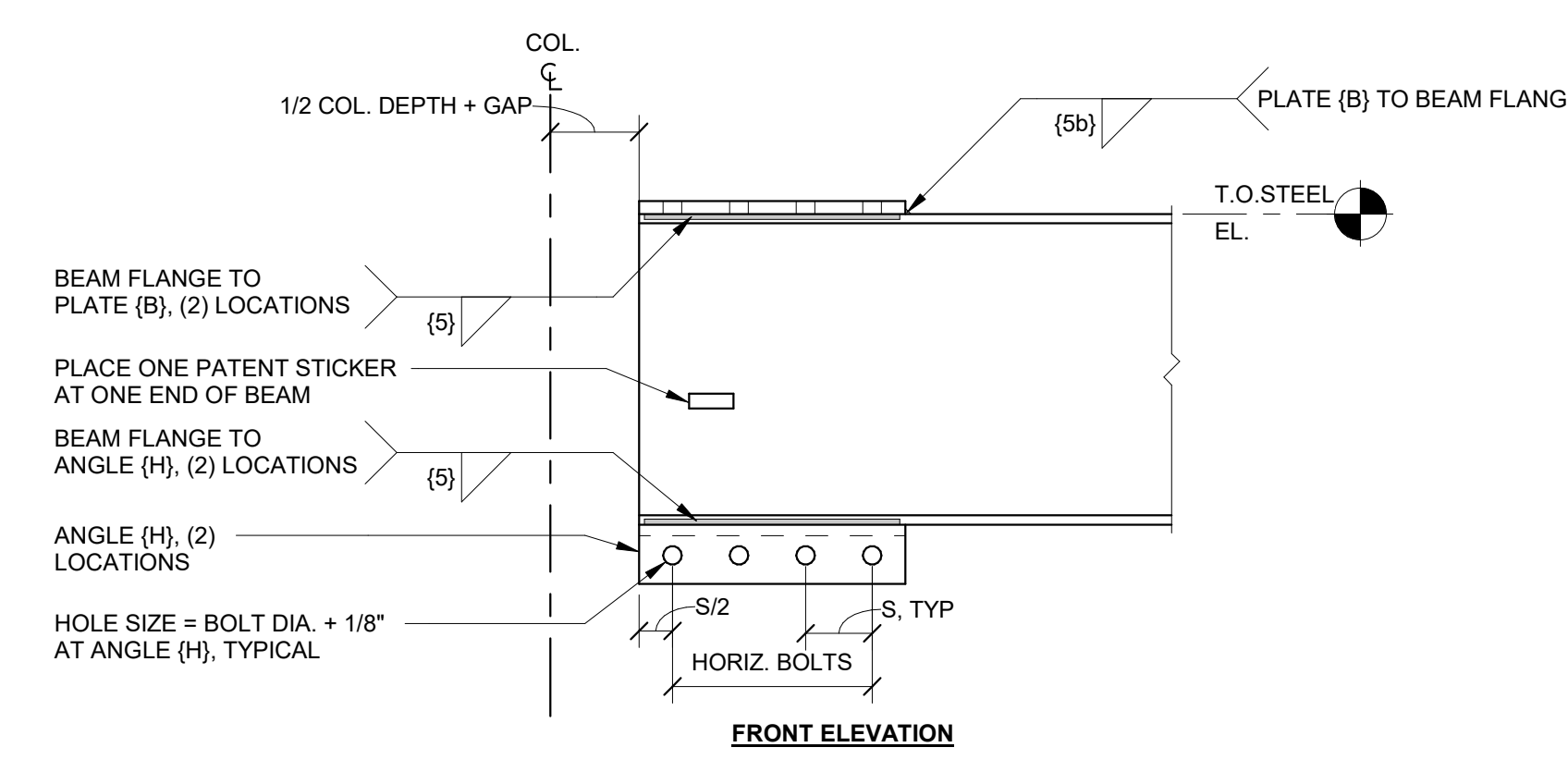
NOTE(S):
 1. SEE BEAM END SCHEDULE FOR QUANTITY, SPACING, AND WELDING OF STIFFENER PLATES.
 2. STIFFENER PLATES SHALL BE MADE OF GRADE 50 MATERIAL.
 3. STIFFENER PLATES AND WELDS ARE NOT CREATED BY SIDEPLATE CUSTOM COMPONENT TOOL.

6 STIFFENER PLATES
 N.T.S.



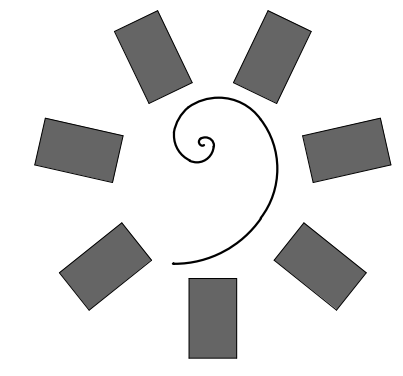
NOTE(S):
 1. FOR ITEMS NOT NOTED, SEE DETAIL 1 / S8.04

5 RECTANGULAR COVER PLATE (B)
 N.T.S.

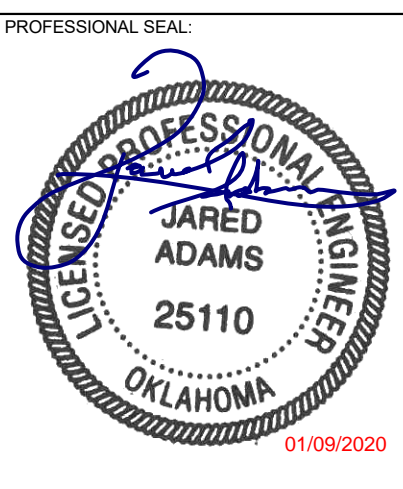


NOTE(S):
 1. USE SLOTTED OR RECTANGULAR COVER PLATE (B) PER SCHEDULE. FOR RECTANGULAR COVER PLATE, SEE DETAIL 5 / S8.04

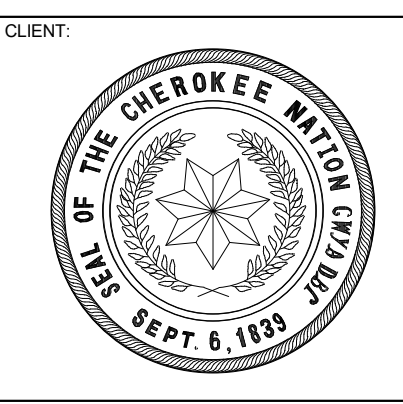
1 BEAM END DETAIL
 N.T.S.



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**WILMA P. MANKILLER HEALTH CENTER
 EXPANSION**
 STILWELL, OKLAHOMA

KEY PLAN

PROJECT PHASE
 STEEL DETAILS

#	DATE	REVISIONS DESCRIPTION
1	01/10/20	BID PACKAGE 01-ASB 01

DATE: 11-01-19 JOB NUMBER: 18-01.01

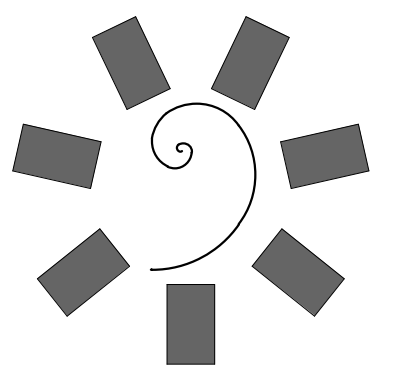
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SIDEPLATE BEAM DETAILS

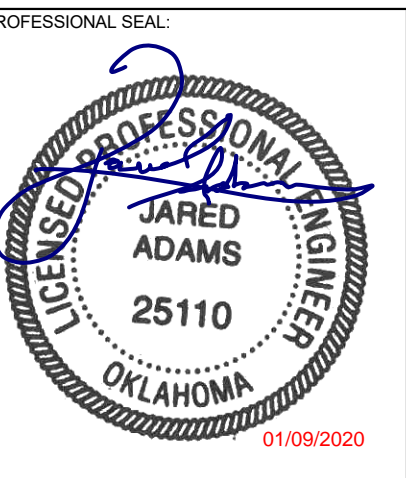
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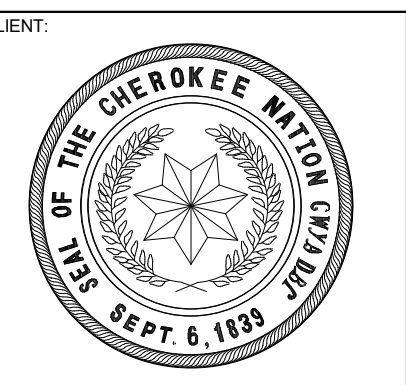
DATE PLOTTED: 11/08/20



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**WILMA P. MANKILLER HEALTH CENTER
 EXPANSION**
 STILWELL, OKLAHOMA

KEY PLAN:

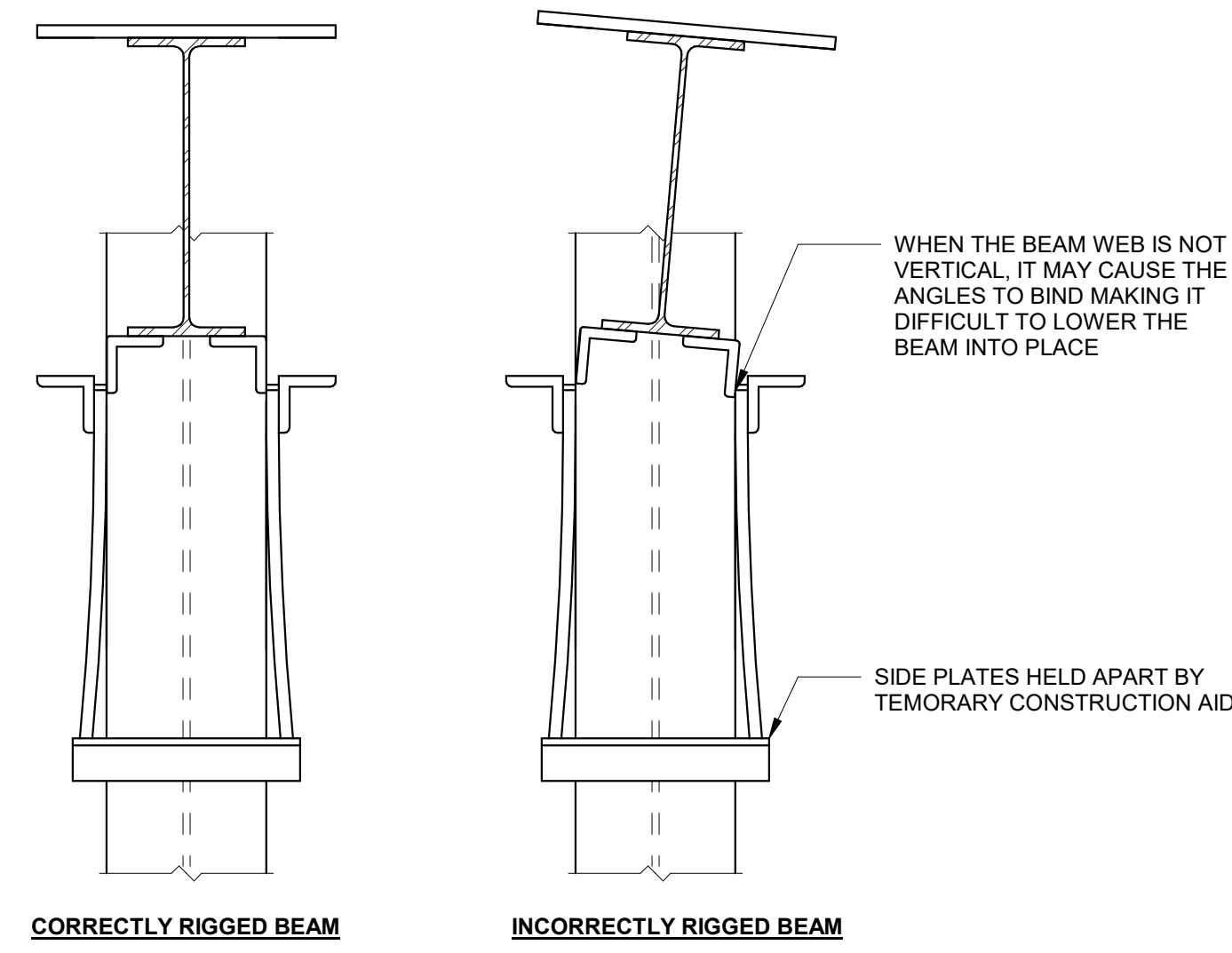
PROJECT PHASE:
 STEEL DETAILS

#	DATE	REVISIONS DESCRIPTION
1	01/10/20	BID PACKAGE 01-AS1 01

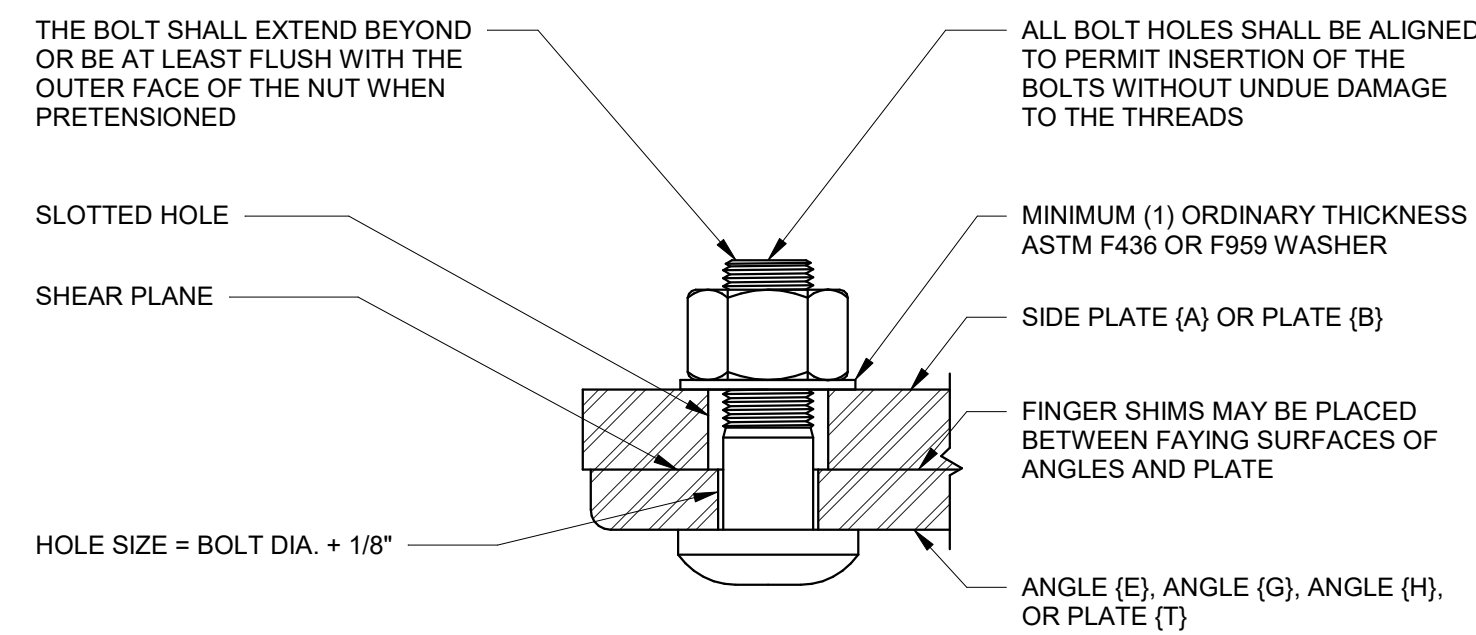
DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER: S8.06

SIDEPLATE FIELD ERECTION DETAILS



4 BEAM INSTALLATION DETAIL
 N.T.S.



- NOTES:
 1. BOLTS SHALL BE INSTALLED AS SHOWN TO KEEP THREADS OUTSIDE OF SHEAR PLANE.
 2. BOLTS SHALL BE SYSTEMATICALLY INSTALLED AS OUTLINED IN THE BOLTING SPECIFICATIONS. FIRST TO A SNUG TIGHT CONDITION, AND THEN PRETENSIONED.
 3. THE USE OF FINGER SHIMS ARE ALLOWED FOR GAPS GREATER THAN 1/8 INCH UP TO 1/4 INCH. CONTACT SIDEPLATE SYSTEMS, INC. IF GAPS ARE GREATER THAN 1/4 INCH.
 4. NUT SHALL BE ASTM A563.
 5. THE BOLT/FASTENER ASSEMBLY SHALL BE COVERED IN A LIGHT PROTECTIVE OIL.
 6. FOLLOW QUALITY CONTROL SECTION FOR EXPOSURE LIMITATION ON BOLTS/FASTENERS.

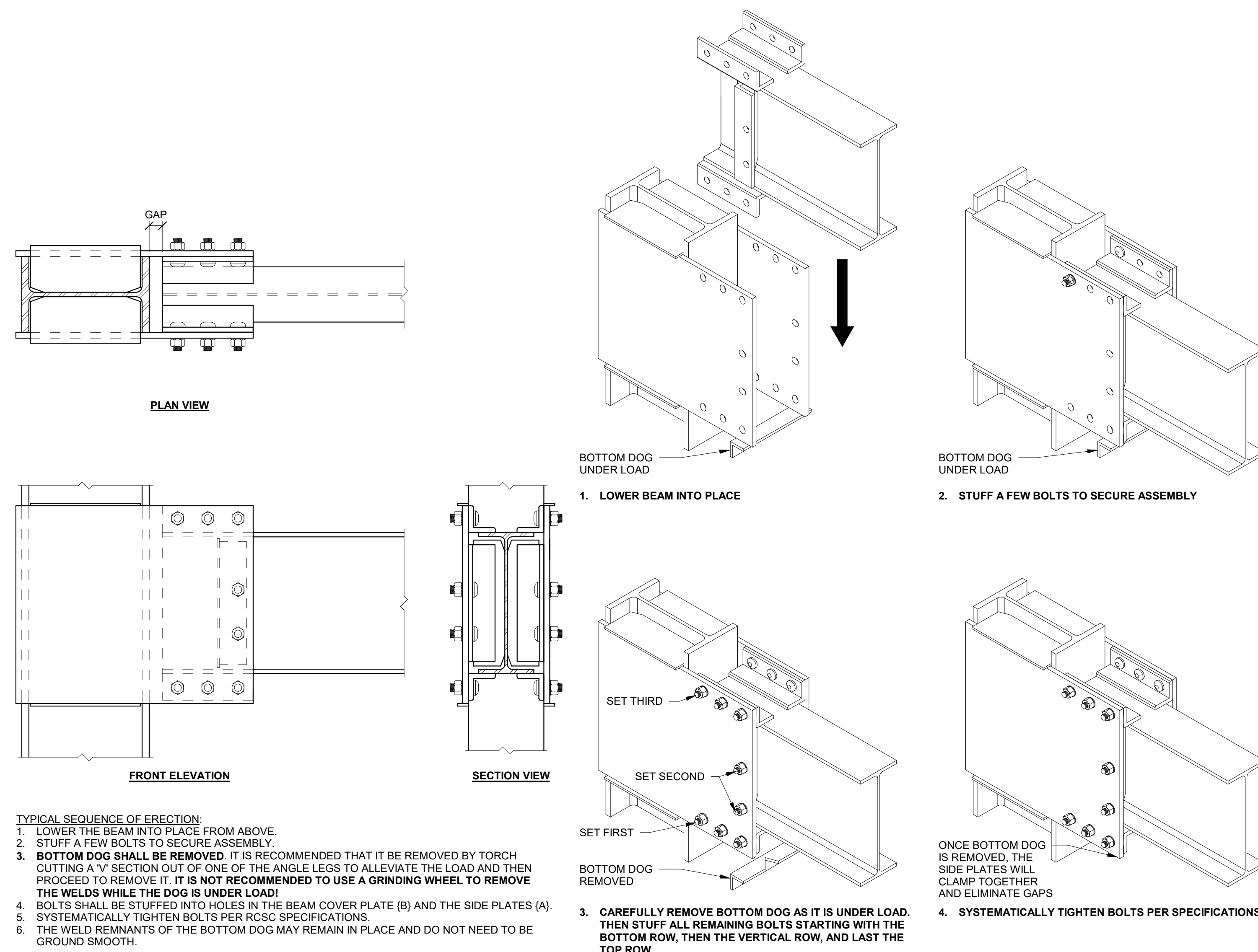
ID	ERECTION DESIGN (INCHES)			
	BEAM		BOLT	
	SHAPE	DIAMETER	HORIZONTAL #	TOTAL # PER BEAM END
A10, A11, A12, A13, A19, B11, B12, B19	W24X88	1 1/8	4	16
A20, B20	W24X94	1 1/8	5	20
A30, B30	W36X150	1 1/8	6	24

2 BEAM ERECTION SCHEDULE
 N.T.S.

ID	ERECTION DESIGN (INCHES)				
	BEAM		BOLT		
	SHAPE	DIAMETER	HORIZONTAL #	VERTICAL #	TOTAL # PER BEAM END
A15, B15	W24X88	1 1/8	4	2	20
A25, B25	W24X94	1 1/8	5	2	24
A45, B45	W36X160	1 1/8	6	3	30

6 NARROW BEAM ERECTION SCHEDULE
 N.T.S.

3 FIELD BOLTING DETAIL
 N.T.S.



- TYPICAL SEQUENCE OF ERECTION:
 1. LOWER THE BEAM INTO PLACE FROM ABOVE.
 2. STUFF A FEW BOLTS TO SECURE ASSEMBLY.
 3. **BOTTOM DOG SHALL BE REMOVED.** IT IS RECOMMENDED THAT IT BE REMOVED BY TORCH CUTTING A V SECTION OUT OF ONE OF THE ANGLE LEGS TO ALLEVIATE THE LOAD AND THEN PROCEED TO REMOVE IT. IT IS NOT RECOMMENDED TO USE A GRINDING WHEEL TO REMOVE THE WELDS WHILE THE DOG IS UNDER LOAD.
 4. BOLTS SHALL BE STUFFED INTO HOLES IN THE BEAM COVER PLATE (B) AND THE SIDE PLATES (A). SYSTEMATICALLY TIGHTEN BOLTS PER RCSC SPECIFICATIONS.
 5. THE WELD REMNANTS OF THE BOTTOM DOG MAY REMAIN IN PLACE AND DO NOT NEED TO BE GROUND SMOOTH.

5 NARROW BEAM ERECTION DETAIL
 N.T.S.

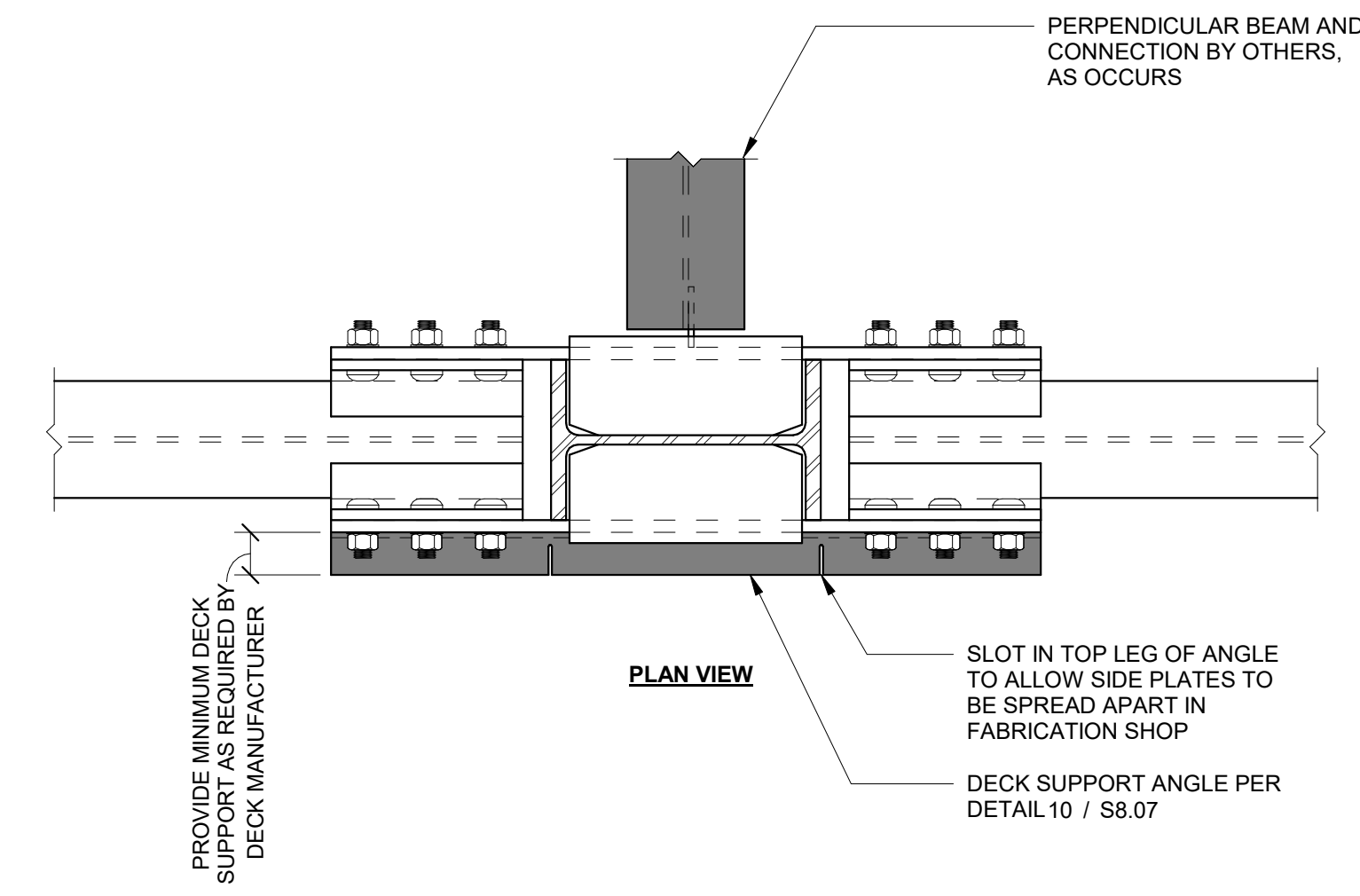
1 BEAM ERECTION DETAIL
 N.T.S.

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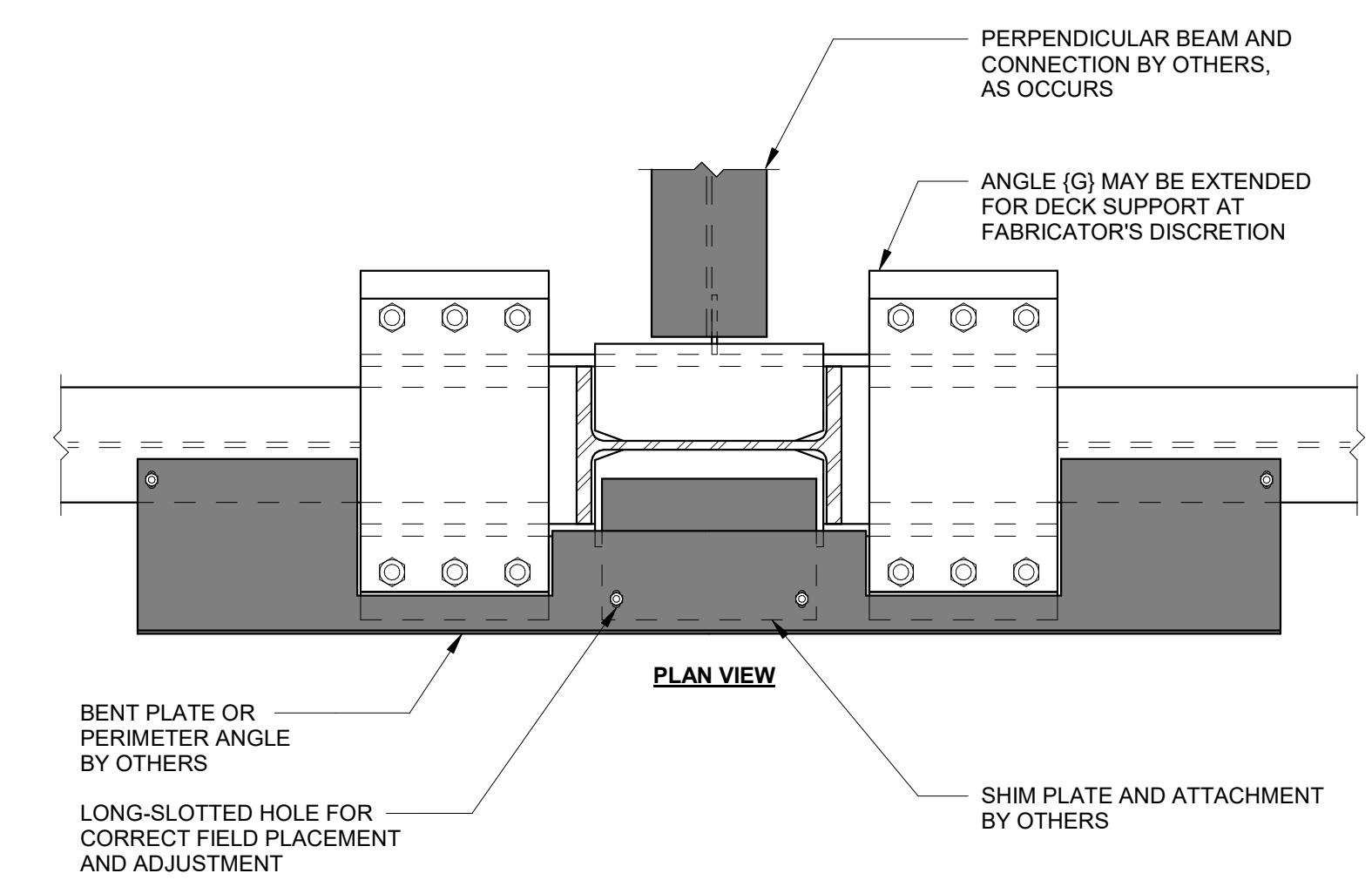
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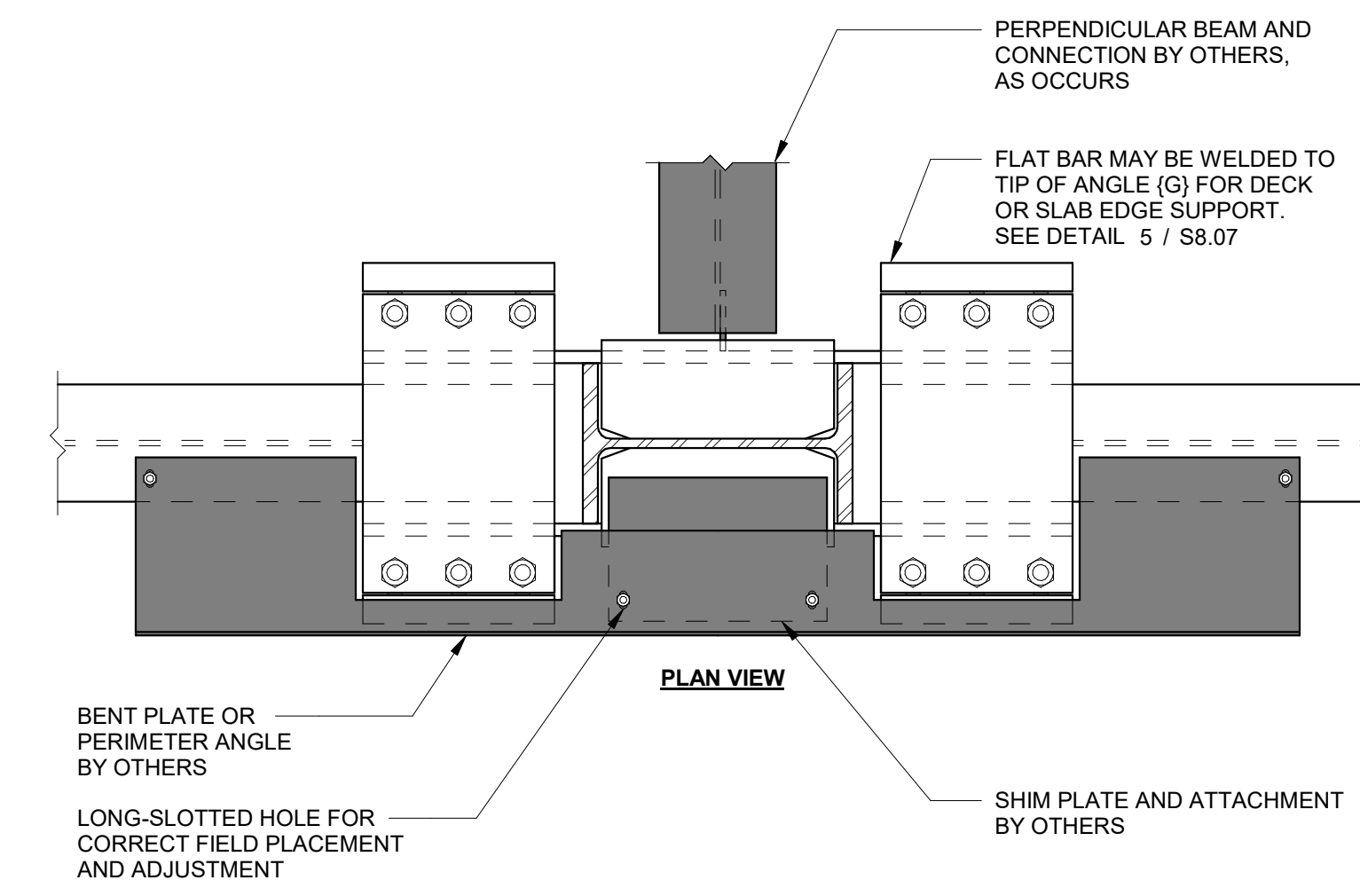
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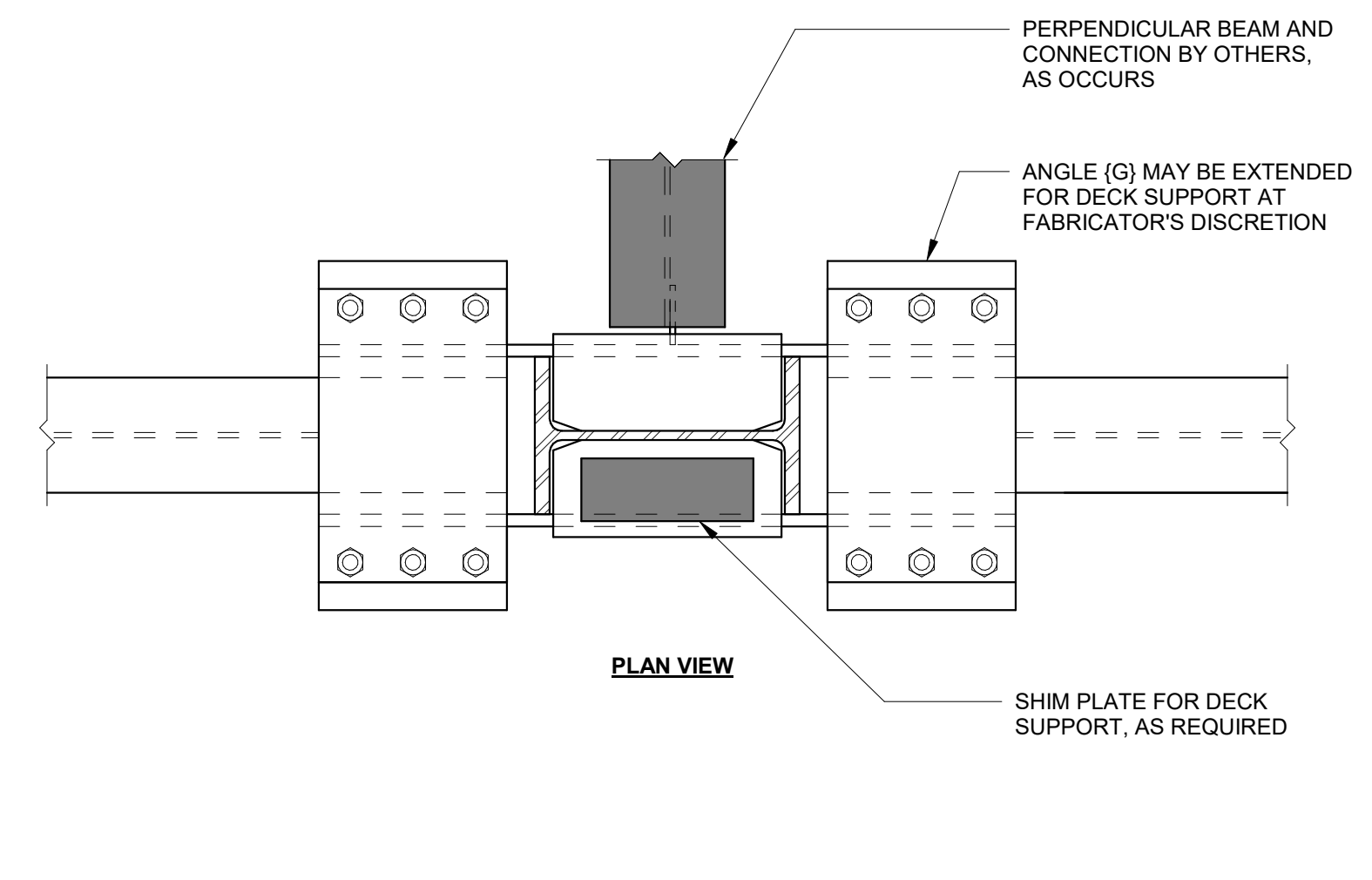
8 NARROW CONFIGURATION DECK SUPPORT DETAIL
N.T.S.



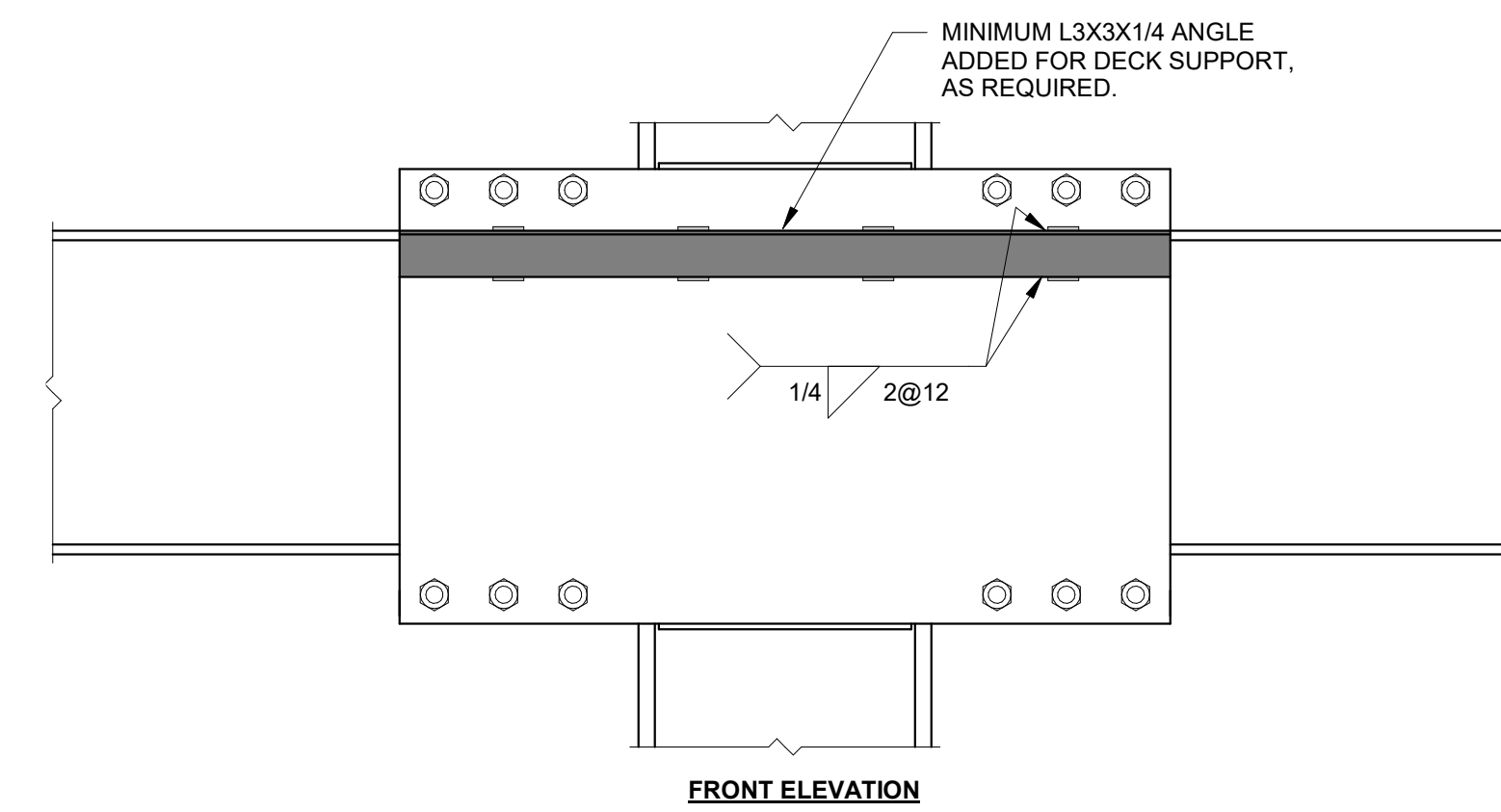
4 SLAB EDGE DETAIL
N.T.S.



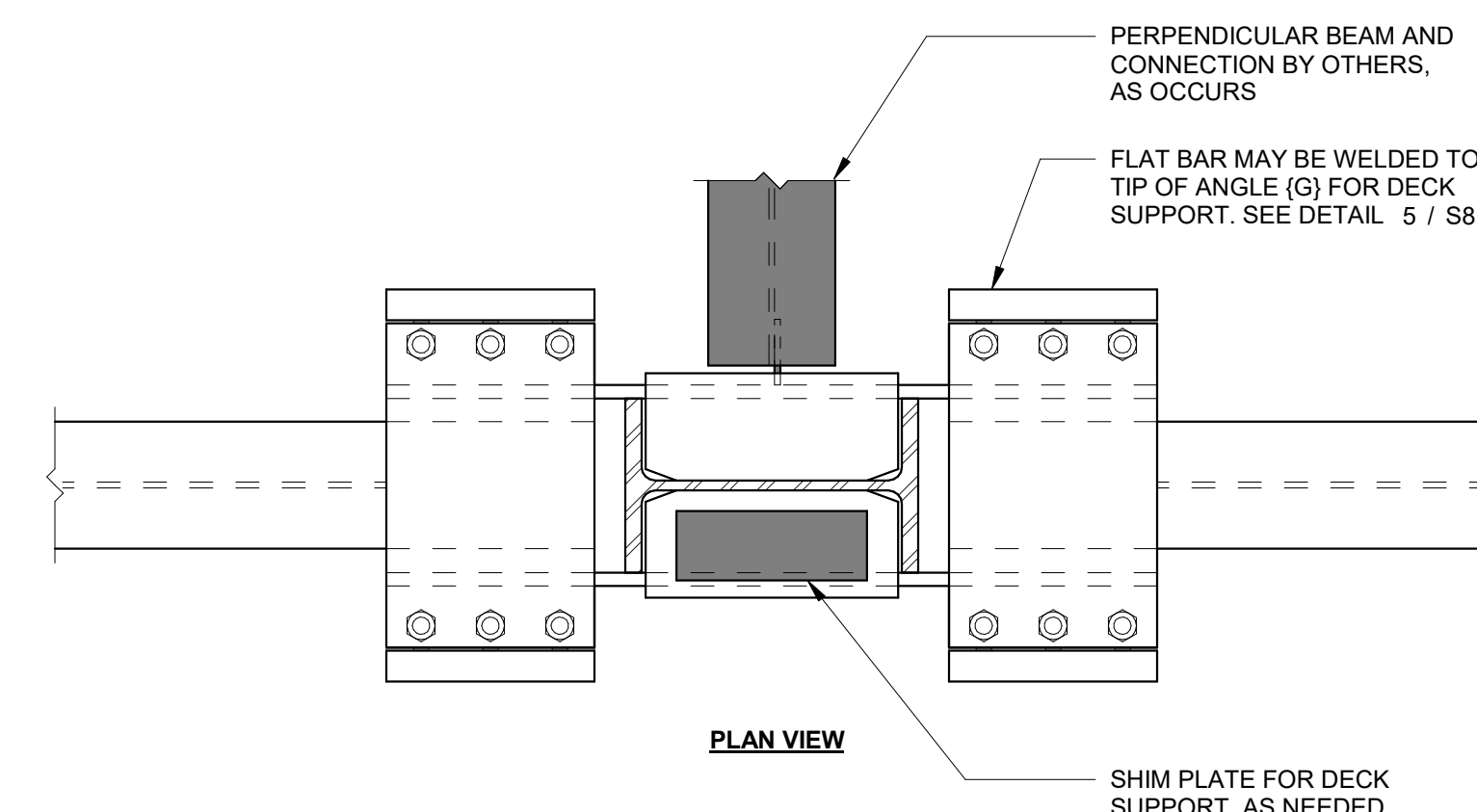
7 WELDED FLAT BAR FOR SLAB EDGE SUPPORT DETAIL
N.T.S.



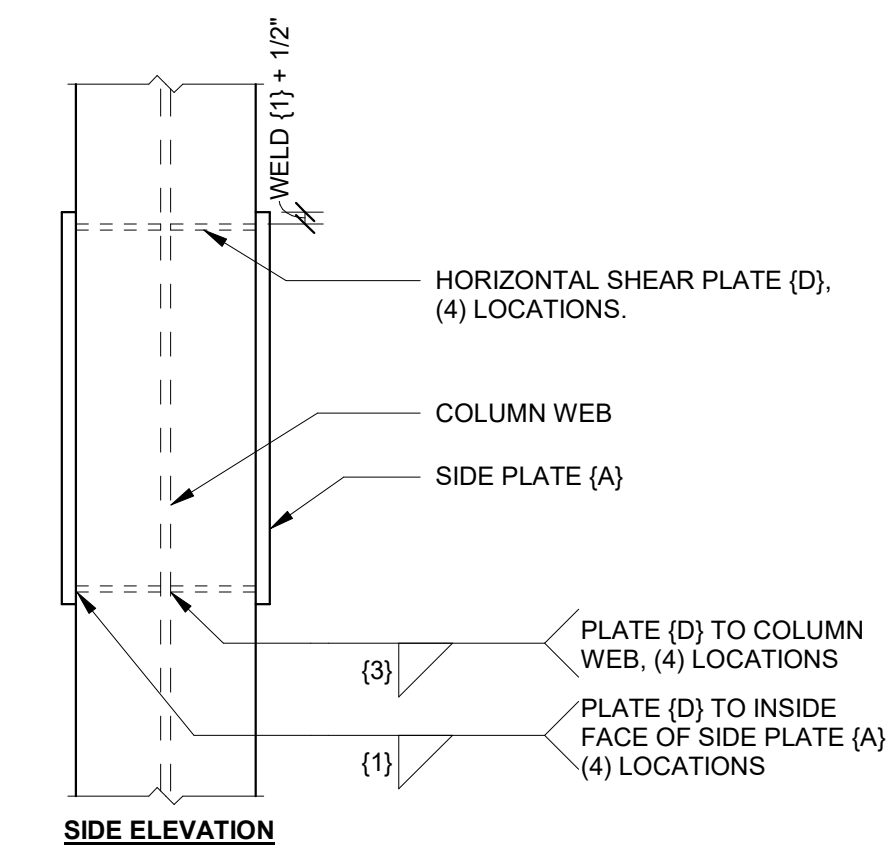
3 DECK SUPPORT DETAIL
N.T.S.



10 DECK SUPPORT ANGLE DETAIL
N.T.S.

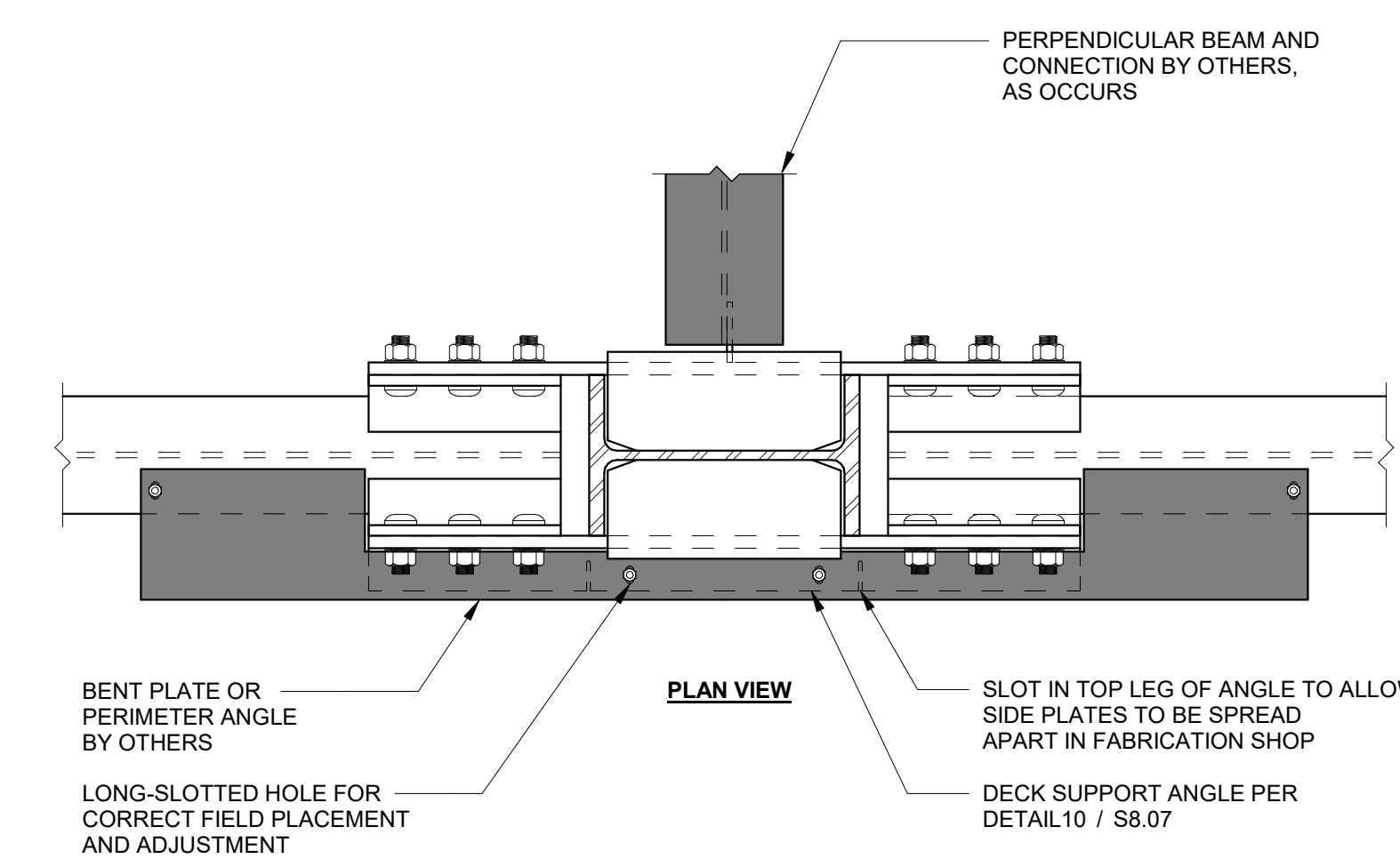


6 WELDED FLAT BAR DECK SUPPORT DETAIL
N.T.S.

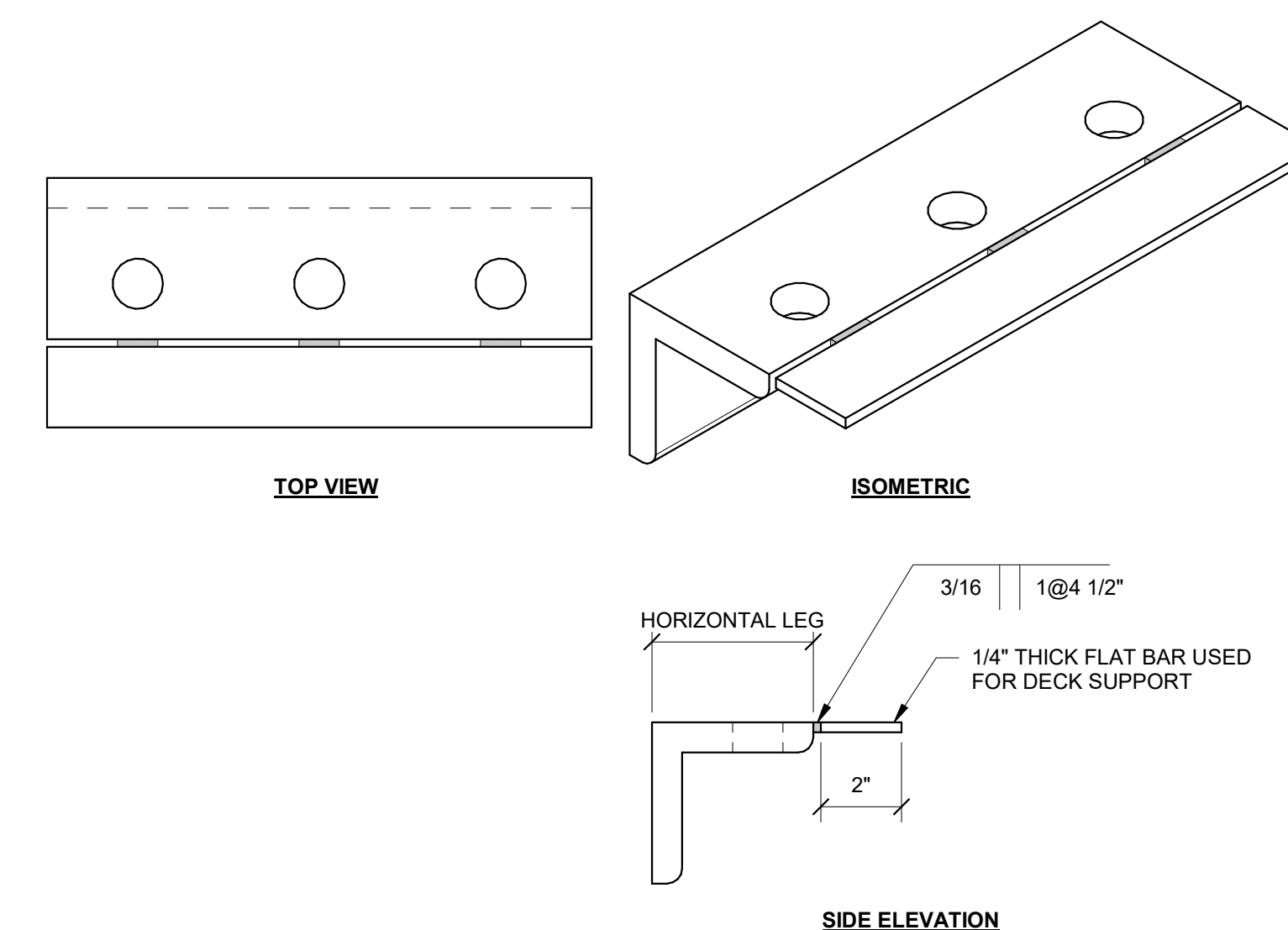


NOTE(S):
1. LONGITUDINAL ANGLES (G) NOT SHOWN FOR CLARITY.

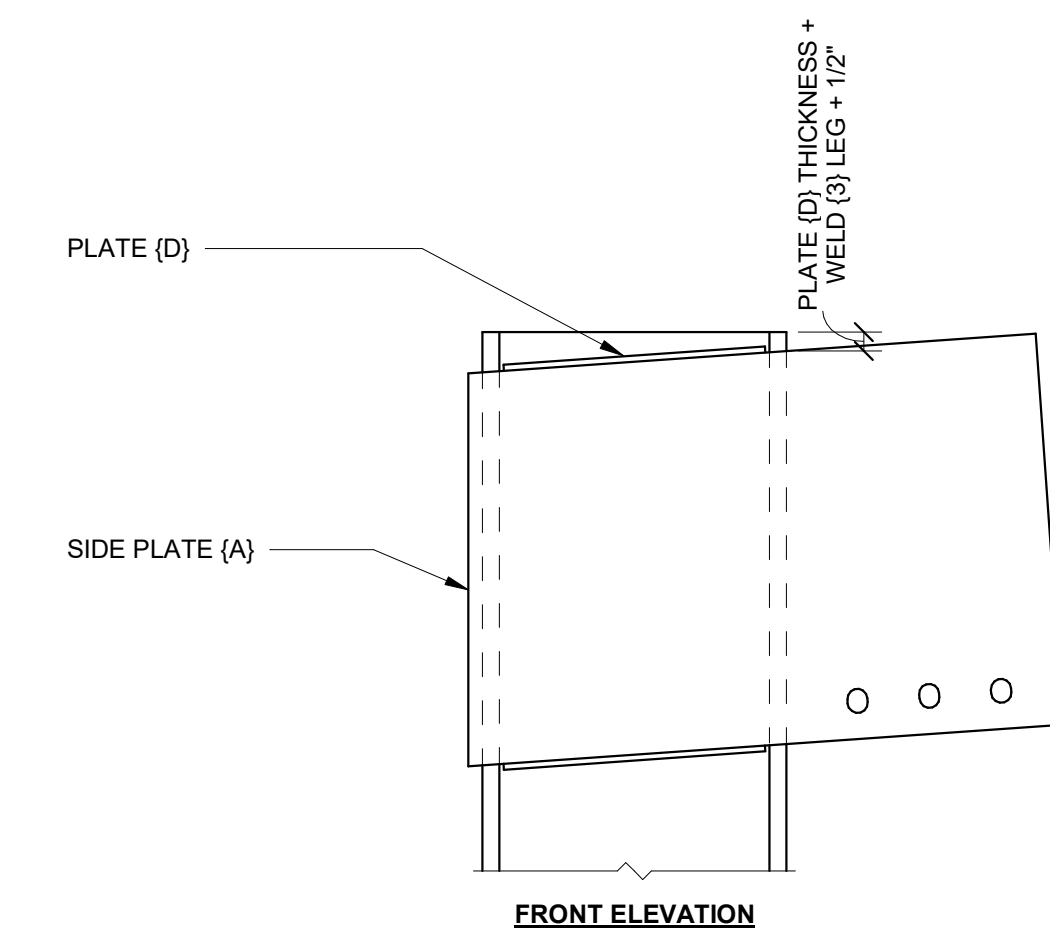
2 PLATE (D) ALTERNATE DETAIL
N.T.S.



9 NARROW CONFIGURATION SLAB EDGE DETAIL
N.T.S.

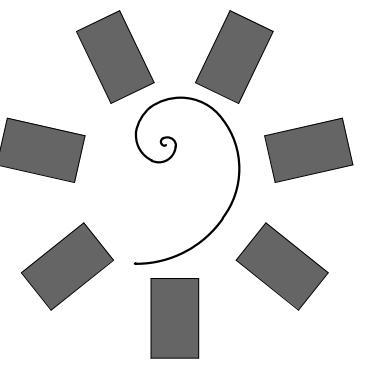


5 WELDED FLAT BAR TO ANGLE (G) FOR DECK SUPPORT
N.T.S.



NOTE(S):
1. SLOPED CONDITION SHOWN, CONNECTION MAY BE FLAT.
2. LONGITUDINAL ANGLES (G) NOT SHOWN FOR CLARITY.

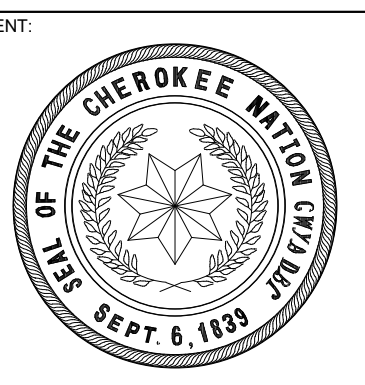
1 DISCONTINUOUS COLUMN DETAIL
N.T.S.



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**WILMA P. MANKILLER HEALTH CENTER
EXPANSION**
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KEY PLAN:

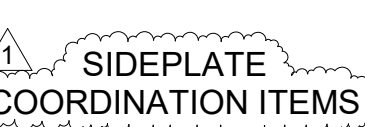
PROJECT PHASE:
STEEL DETAILS

#	DATE	REVISIONS DESCRIPTION
1	01/19/20	BID PACKAGE 01 - ASI 01

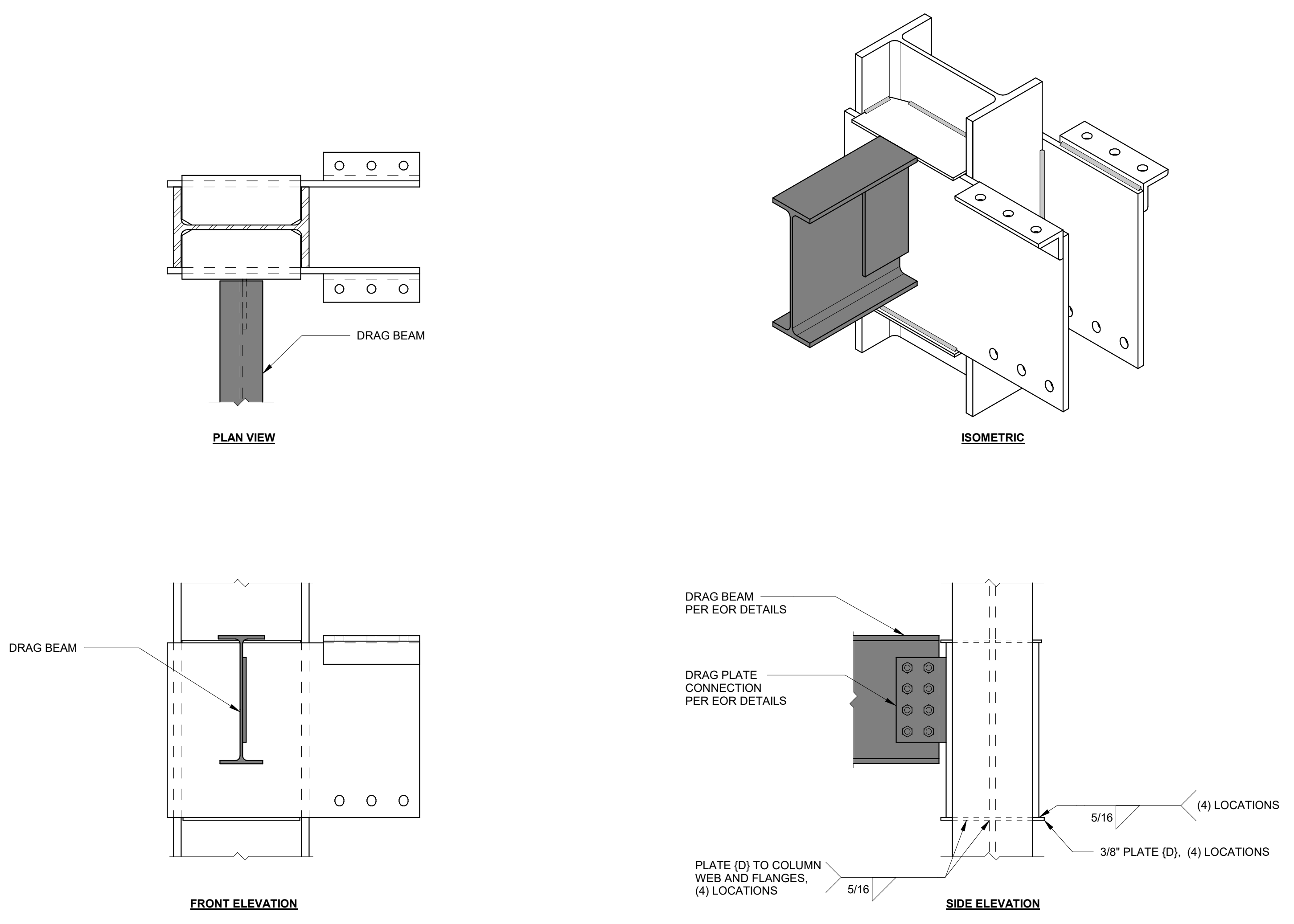
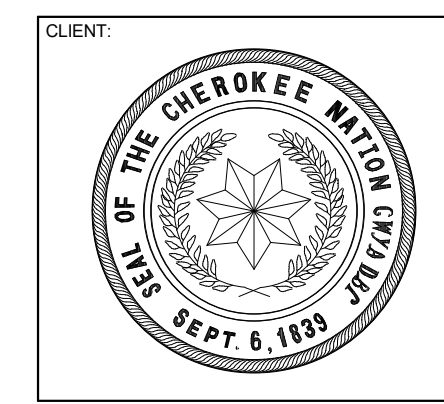
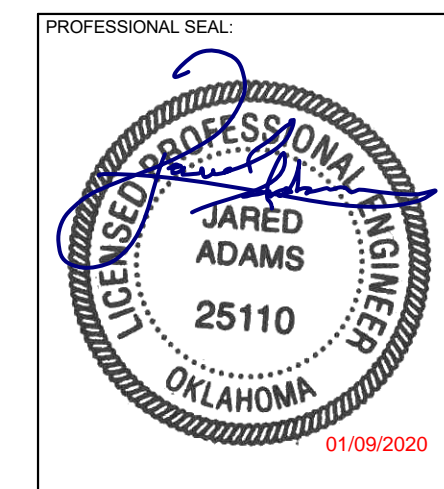
DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER:

S8.07

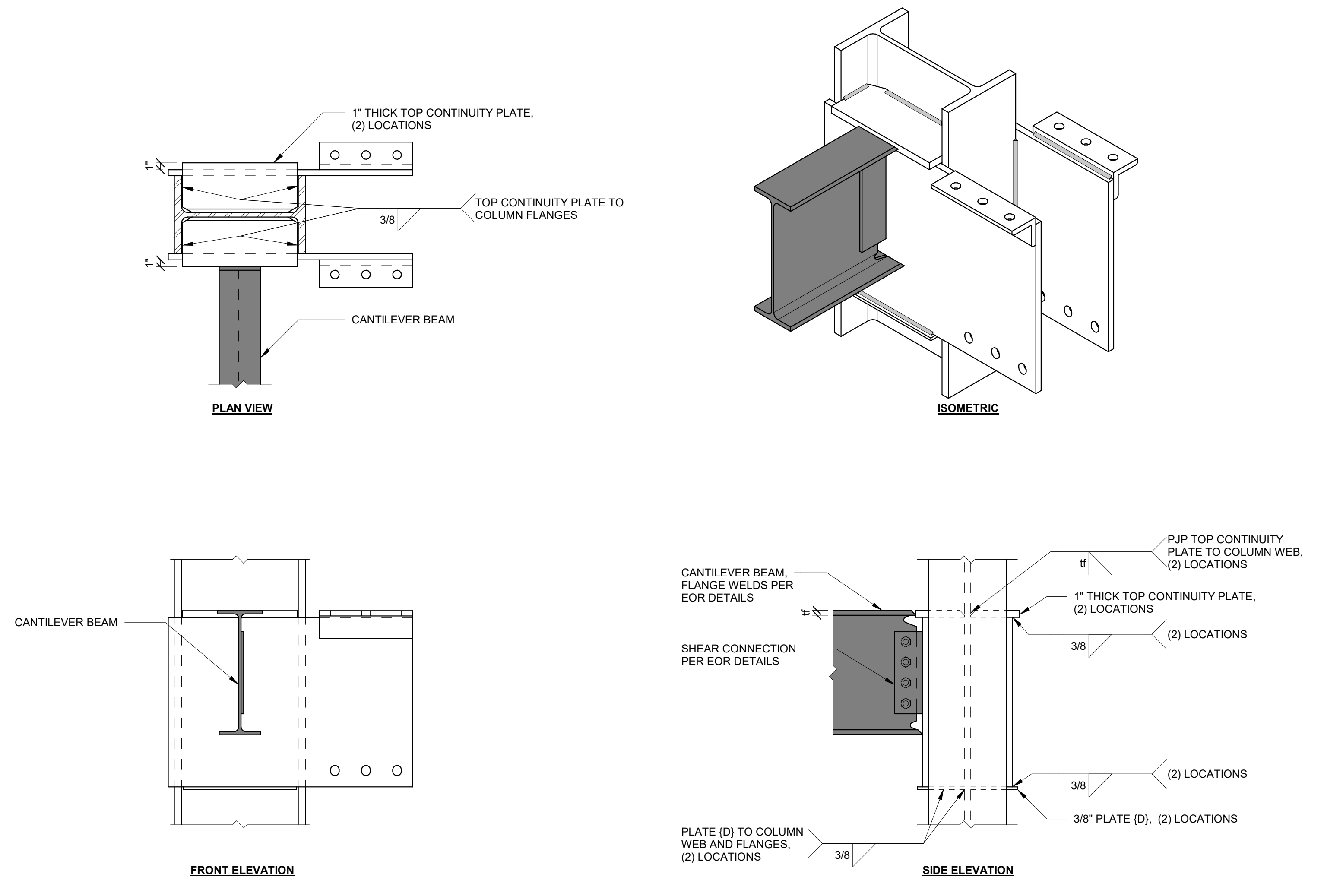


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 The SIDEPLATE® steel frame connection system is covered by one or more of U.S. Pat. Nos. 6,138,427; 6,516,583; 6,591,573; 7,178,296; 8,122,671; 8,122,672; 8,146,322; 8,176,706; 8,205,408; and 9,091,065 and foreign counterparts.
 Other U.S. and foreign applications pending.
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 v15.08.02



NOTE(S):
 1. ATTACHMENT SHOWN ON ONE SIDE OF SIDEPLATE CONNECTION FOR ILLUSTRATION. ATTACHMENT CAN OCCUR ON LEFT SIDE, RIGHT SIDE, OR BOTH SIDES OF CONNECTION AS APPLICABLE.

M2 SHEAR PLATE DRAG BEAM TO SIDEPLATE CONNECTION
 N.T.S.



NOTE(S):
 1. ATTACHMENT SHOWN ON ONE SIDE OF SIDEPLATE CONNECTION FOR ILLUSTRATION. ATTACHMENT CAN OCCUR ON LEFT SIDE, RIGHT SIDE, OR BOTH SIDES OF CONNECTION AS APPLICABLE.

M1 CANTILEVER TO SIDEPLATE CONNECTION
 N.T.S.

1 MISCELLANEOUS DETAILS SCHEDULE
 N.T.S.

Misc ID	Coordinate with Detail
M1	M1/S8.08
M2	M2/S8.08

**WILMA P. MANKILLER HEALTH CENTER
 EXPANSION**
 STILWELL, OKLAHOMA

KEY PLAN:

PROJECT PHASE:
 STEEL DETAILS

#	DATE	REVISIONS DESCRIPTION

DATE: 01-10-20 JOB NUMBER: 18-01.01

SHEET NUMBER:
S8.08

SIDEPLATE MISCELLANEOUS DETAILS

 **AIA** Document G710™ – 2017

Architect's Supplemental Instructions

PROJECT: *(name and address)*
Wilma P. Mankiller Health Center
Expansion
Stilwell, OK

CONTRACT INFORMATION:
Contract For: CMAR

Date:

ASI INFORMATION:
ASI Number: Bid Package 01 - ASI 002

Date: 02/05/20

OWNER: *(name and address)*
Cherokee Nation Property Management,
LLC

ARCHITECT: *(name and address)*
James R Childers Architect Inc.

CONTRACTOR: *(name and address)*
M Ross Inc

The Contractor shall carry out the Work in accordance with the following supplemental instructions without change in Contract Sum or Contract Time. Proceeding with the Work in accordance with these instructions indicates your acknowledgment that there will be no change in the Contract Sum or Contract Time.

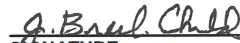
(Insert a detailed description of the Architect's supplemental instructions and, if applicable, attach or reference specific exhibits.)

See attached narrative from Chavez Grieves

ISSUED BY THE ARCHITECT:

James R Childers Architect Inc.

ARCHITECT *(Firm name)*



SIGNATURE

Breck Childers

PRINTED NAME AND TITLE

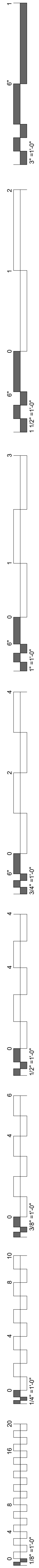
02/05/20

DATE

Bid Package 01- ASI 02 – Wilma P. Mankiller Health Center Expansion

Chavez-Grieves would like to incorporate the following revisions into the drawings for the above referenced project.

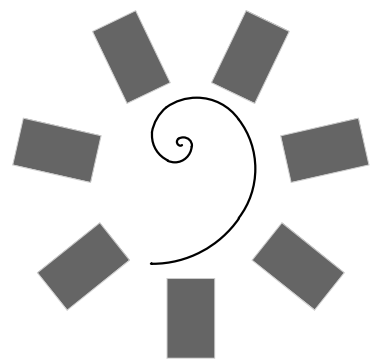
<u>Sheet</u>	<u>Description</u>
S1.13	Sheet keynote 12 removed from Grid 10.
S1.13	Section C1/S3.31 cut along Grid 10.
S1.13	Framing and deck edge near Grid G/10 modified.
S1.21	Beam sizes updated along Grid 10, between Grids G and D.
S1.21	Section C1/S3.31 cut along Grid 10.
S3.31	Section C1 added.



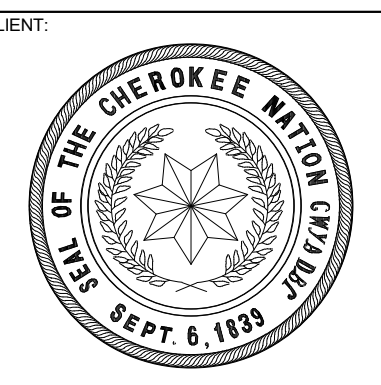
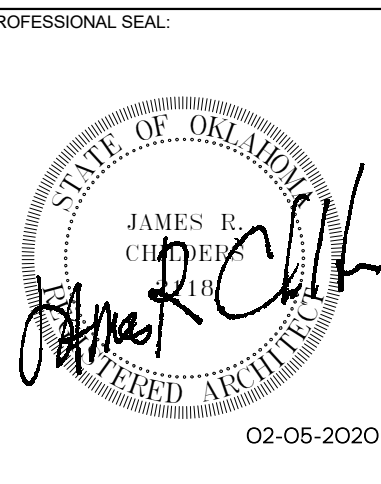
WILMA P. MANKILLER HEALTH CENTER EXPANSION

BID PACKAGE 01 (DEMOLITION / STEEL / FOUNDATIONS)

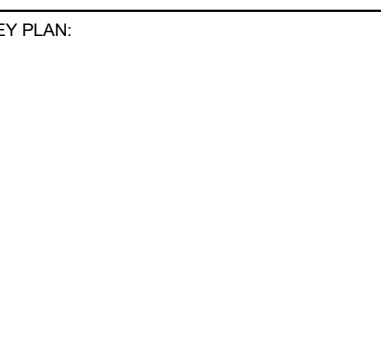
INDEX OF DRAWINGS - BID PACKAGE 01		11-01-19 - BID PACKAGE 01	11-22-19 - BID PACKAGE 01 - ADDENDUM 01	12-10-19 - BID PACKAGE 01 - ADDENDUM 02	01-10-20 - BID PACKAGE 01 - ASB 01	02-05-20 - BID PACKAGE 01 - ASB 02
SHEET NUMBER	SHEET NAME					
GENERAL						
G0.01	COVER / INDEX					
CIVIL						
C002	GENERAL NOTES					
CS100	EXISTING SITE PLAN					
CS101	DEMOLITION PLAN					
CS102	DEMOLITION PLAN					
CE100	EROSION CONTROL SITE PLAN					
CE500	EROSION CONTROL DETAILS					
ARCHITECTURAL						
A0.01	OVERALL BUILDING DEMOLITION PLAN					
STRUCTURAL						
S0.01	ABBREVIATIONS AND LEGENDS					
S0.02	GENERAL STRUCTURAL NOTES					
S0.03	GENERAL STRUCTURAL NOTES AND SPECIAL INSPECTIONS					
SD0.01	DEMOLITION GENERAL STRUCTURAL NOTES					
SD1.01	DEMOLITION PLANS - SECTOR 1					
SD2.01	DEMOLITION SECTIONS					
S1.00	OVERALL PLAN - FOUNDATION					
S1.01	FOUNDATION PLAN SECTOR 1					
S1.02	FOUNDATION PLAN SECTOR 2					
S1.10	OVERALL PLAN - FLOOR FRAMING					
S1.11	FLOOR FRAMING PLAN - SECTOR 1					
S1.12	FLOOR FRAMING PLAN - SECTOR 2					
S1.13	LOW ROOF FRAMING PLAN					
S1.20	OVERALL PLAN - ROOF FRAMING					
S1.21	ROOF FRAMING PLAN - SECTOR 1					
S1.22	ROOF FRAMING PLAN - SECTOR 2					
S2.01	MOMENT FRAME ELEVATIONS					
S2.02	MOMENT FRAME AND BRACED FRAME ELEVATIONS					
S3.01	WALL SECTIONS					
S3.02	WALL SECTIONS					
S3.03	WALL SECTIONS					
S3.04	WALL SECTIONS					
S3.11	FOUNDATION SECTIONS					
S3.12	FOUNDATION SECTIONS					
S3.21	FLOOR FRAMING SECTIONS					
S3.31	ROOF FRAMING SECTIONS					
S4.01	ENLARGED PLANS					
S5.21	MASONRY FRAMING SECTIONS AND DETAILS					
S6.41	VERTICAL CIRCULATION DETAILS					
S5.51	STEEL DETAILS					
S5.52	STEEL DETAILS					
S5.53	STEEL DETAILS					
S5.54	STEEL DETAILS					
S6.01	SCHEDULES					
S7.11	TYPICAL CONCRETE DETAILS					
S7.21	TYPICAL MASONRY DETAILS					
S7.31	TYPICAL COLD-FORMED DETAILS					
S7.41	TYPICAL STEEL DETAILS					
S7.42	TYPICAL STEEL DETAILS					
S8.01	SIDEPLATE GENERAL NOTES AND CONSTRUCTION GUIDELINES					
S8.02	SIDEPLATE COLUMN DETAILS, A TYPE					
S8.03	SIDEPLATE COLUMN DETAILS, B TYPE					
S8.04	SIDEPLATE BEAM DETAILS					
S8.05	SIDEPLATE BEAM DETAILS, NARROW					
S8.06	SIDEPLATE FIELD ERECTION DETAILS					
S8.07	SIDEPLATE COORDINATION ITEMS					
S8.08	SIDEPLATE MISCELLANEOUS DETAILS					
ELECTRICAL						
E0.01	ELECTRICAL DEMOLITION PLAN					
Grand total: 66						



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**WILMA P. MANKILLER HEALTH CENTER
EXPANSION**
 STILWELL, OKLAHOMA



PROJECT PHASE:
BID PACKAGE 01

#	DATE	REVISIONS	DESCRIPTION
1	11/22/19	BID PACKAGE 01 - ADD 01	
2	12/10/19	BID PACKAGE 01 - ADD 02	
3	1/10/20	BID PACKAGE 01 - ASB 01	
4	2/5/2020	BID PACKAGE 01 - ASB 02	

DATE: 11-01-19 JOB NUMBER: 18-01.01

SHEET NUMBER:
G0.01

COVER / INDEX



1836 SOUTH BALTIMORE AVE.
TULSA, OK 74119
(539) 664-4618

MECHANICAL / ELECTRICAL / PLUMBING ENGINEER



3902 UNIVERSITY BOULEVARD
DURANT, OK 74701
(580) 931-9045

CIVIL ENGINEER



4700 LINCOLN ROAD NE, SUITE 102
ALBUQUERQUE, NM 87109
(505) 344-4080

STRUCTURAL ENGINEER



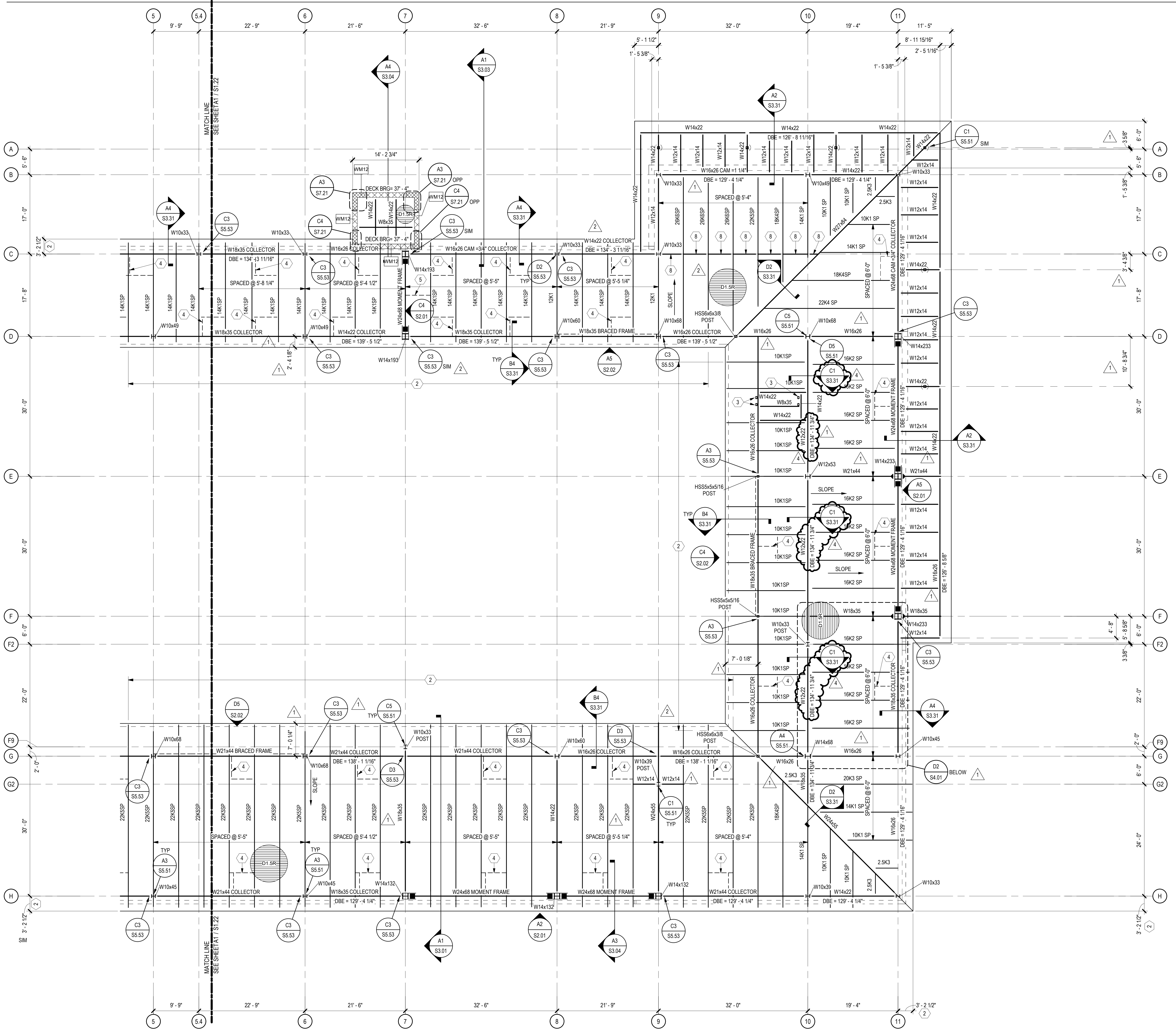
808 TRAVIS STREET, SUITE 200
HOUSTON, TX 77002
(281) 589-5900

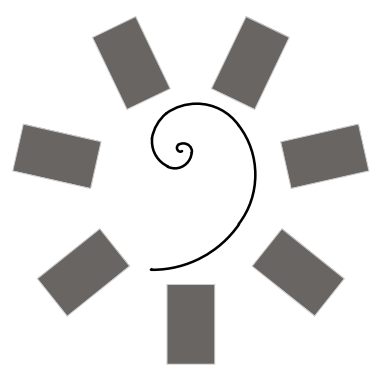
FIRE PROTECTION / LIFE SAFETY



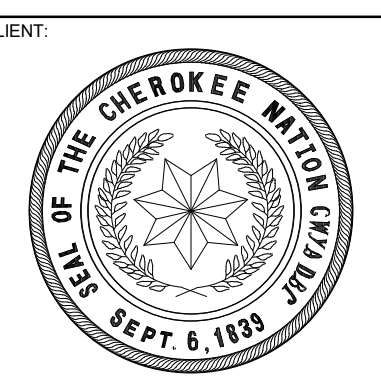
1316 E 35TH PLACE, SUITE 100
TULSA, OK 74105
(918) 382-9120

EQUIPMENT PLANNER





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WILMA P. MANKILLER HEALTH CENTER
EXPANSION
STILWELL, OKLAHOMA

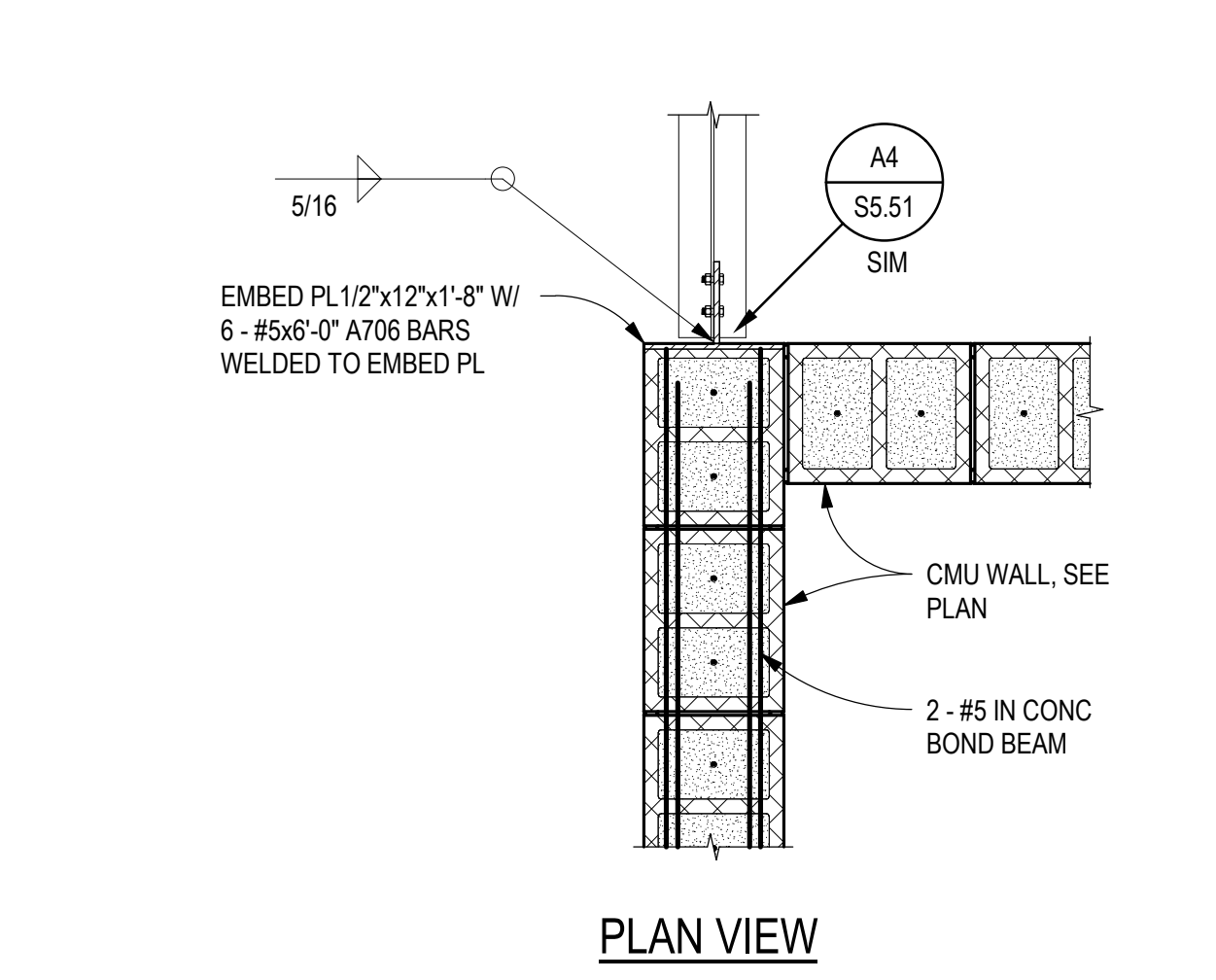
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4	02/05/20	BID PACKAGE 01 - ASH 02	

PROJECT PHASE:
BID PACKAGE 01

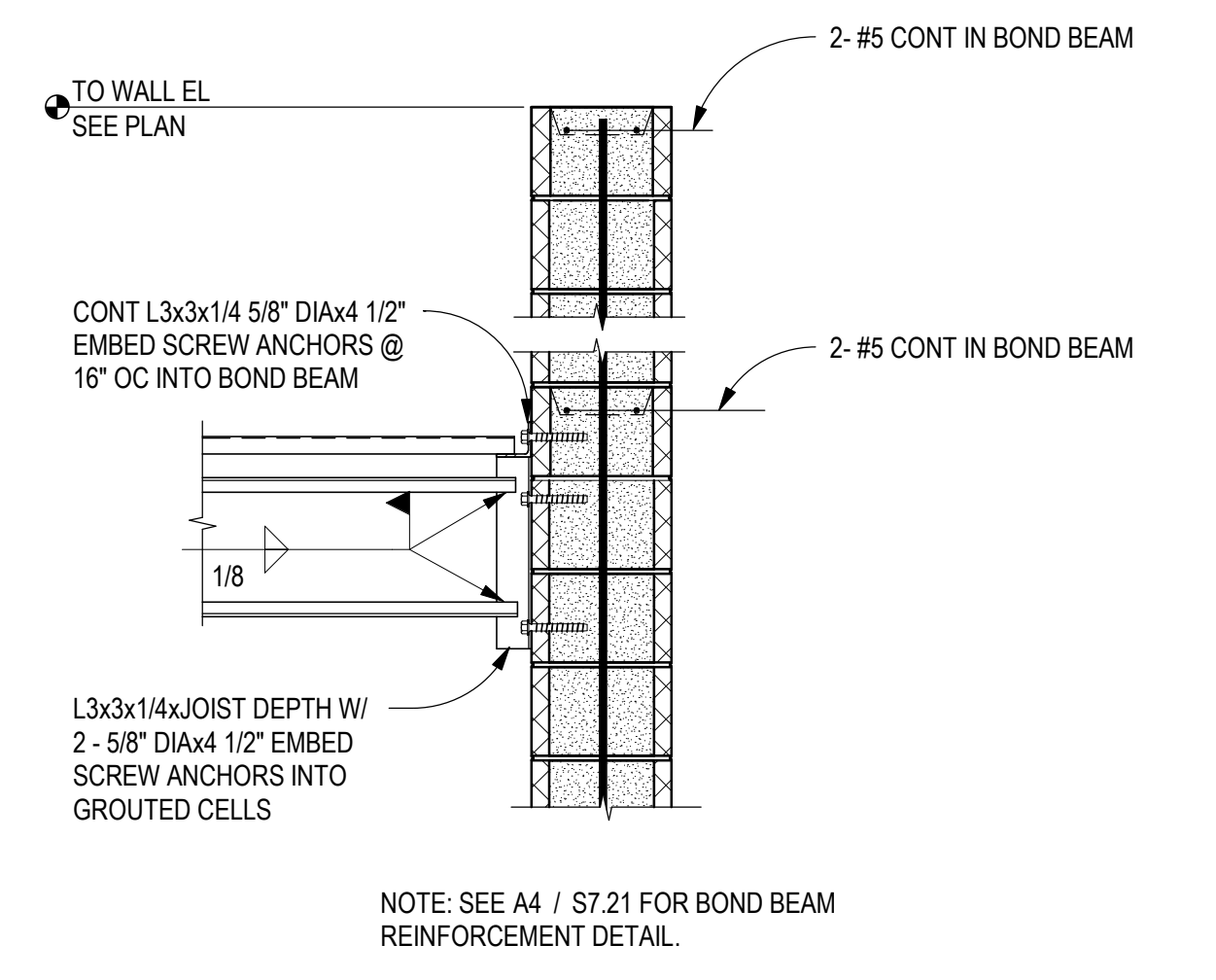
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11-01-19	18-01.01

SHEET NUMBER:
S3.31

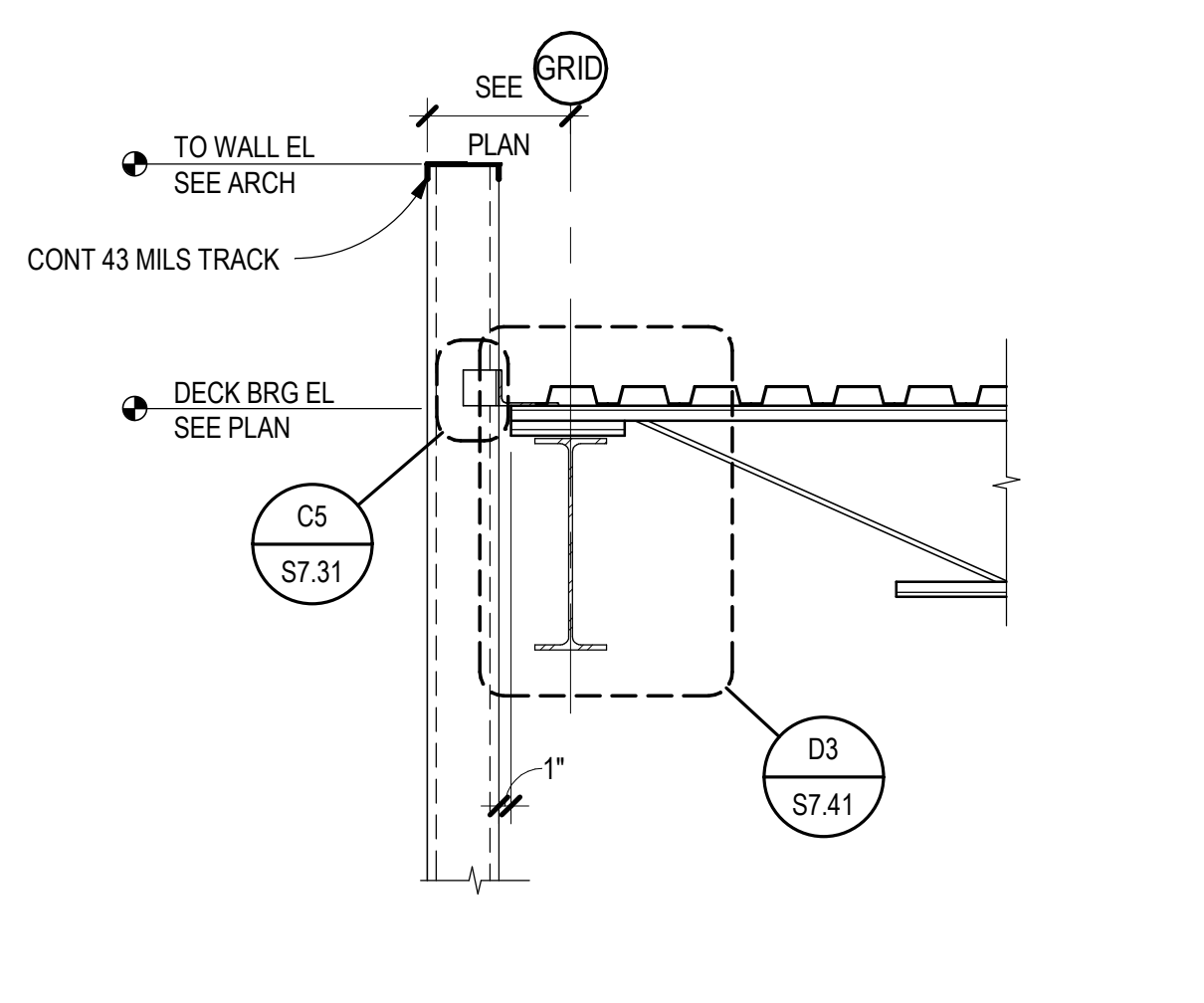
ROOF FRAMING SECTIONS



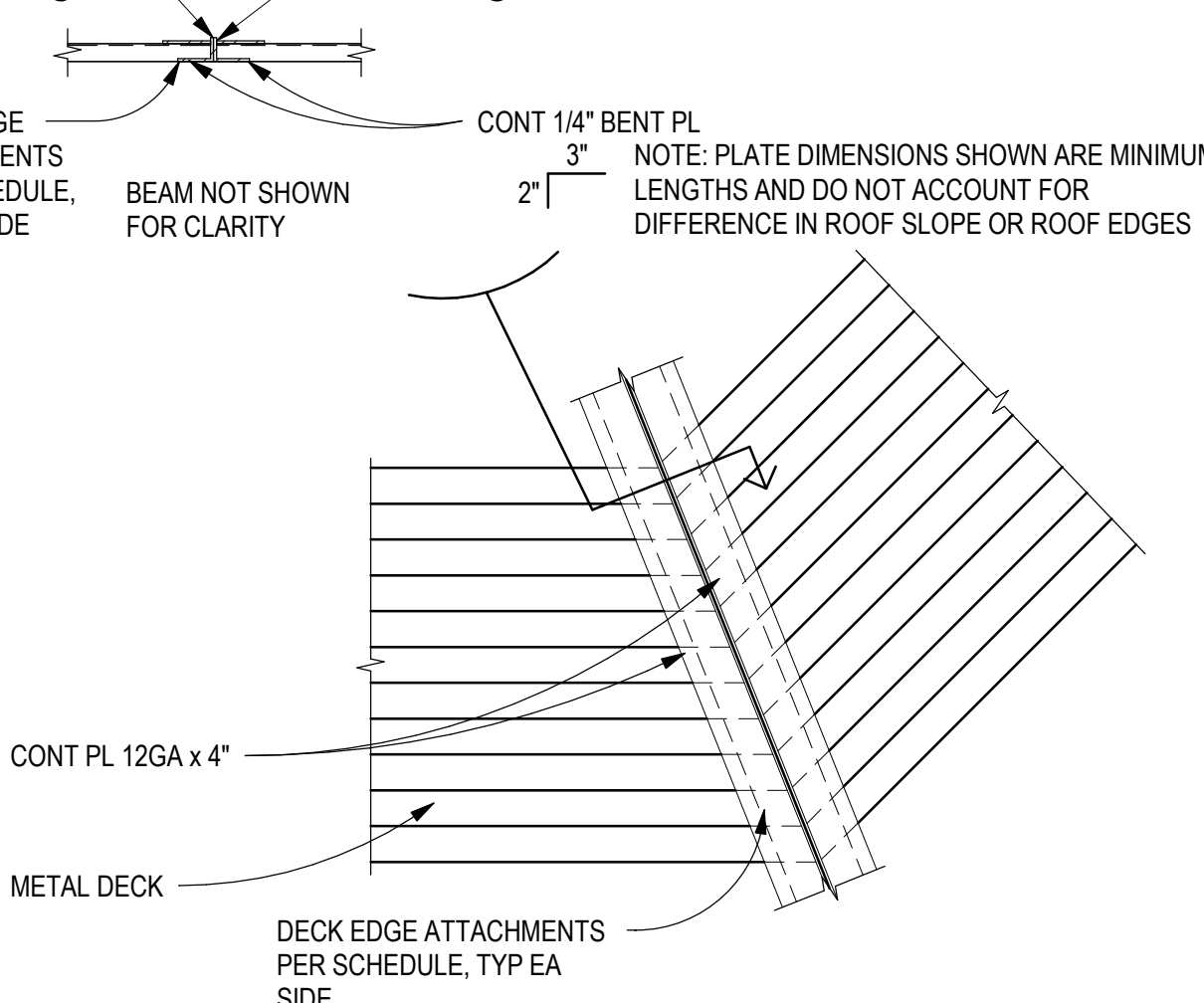
D5 WF COLLECTOR BEAM TO CMU
SCALE: 3/4" = 1'-0"



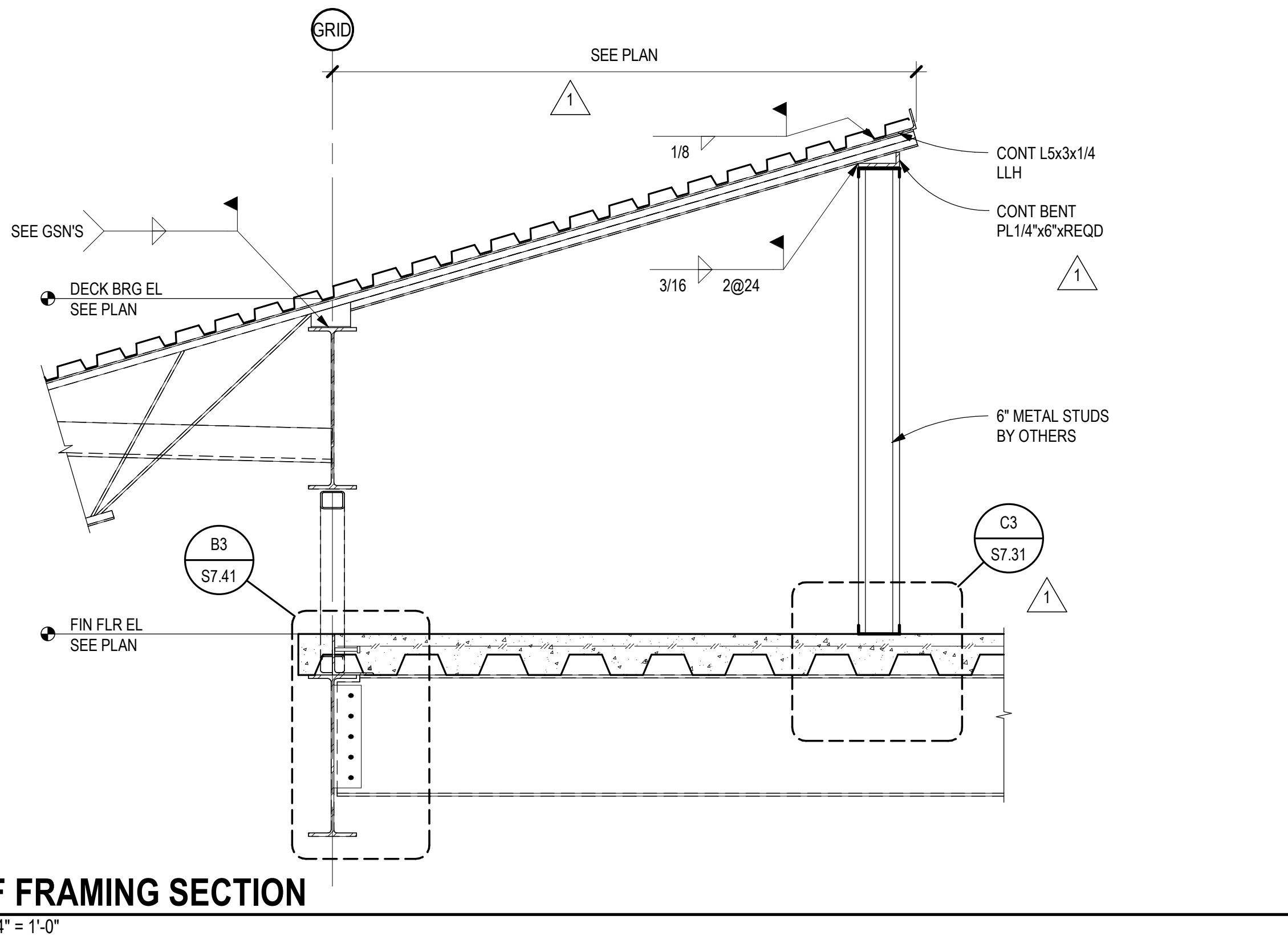
D4 ROOF NON-BEARING AT CMU WALL
SCALE: 3/4" = 1'-0"



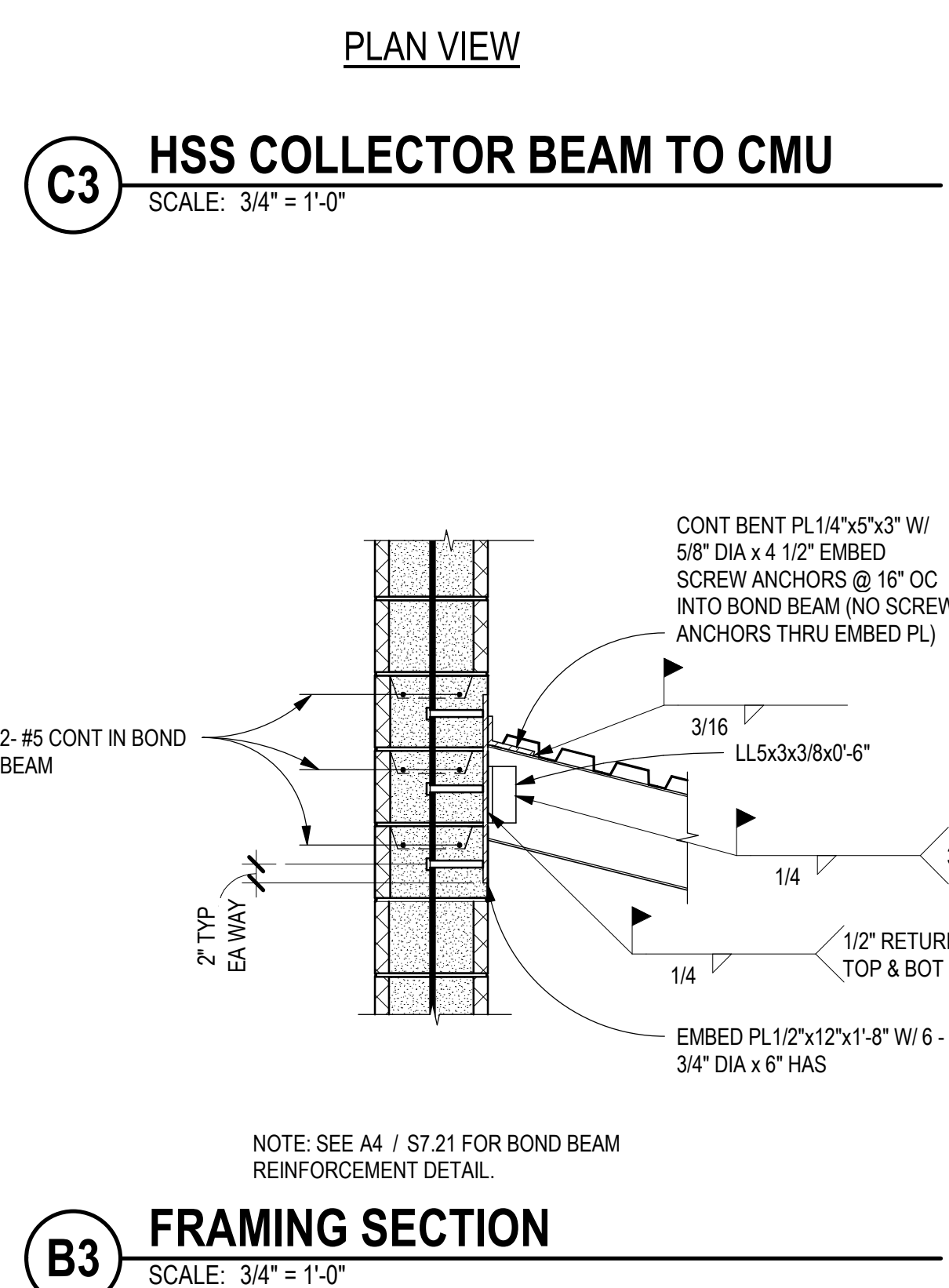
D3 ROOF FRAMING SECTION
SCALE: 3/4" = 1'-0"



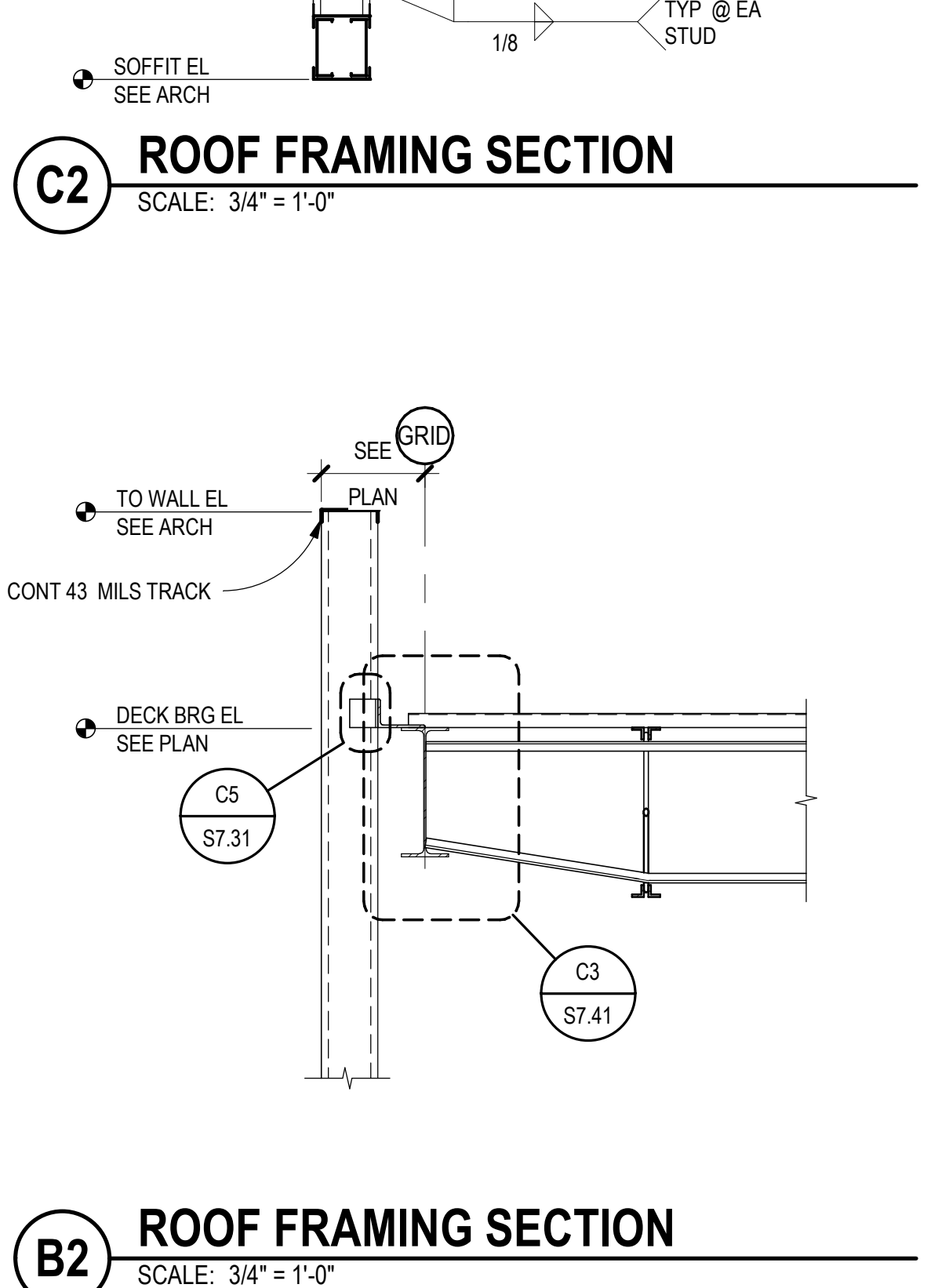
D2 ROOF DECK TRANSITION
SCALE: 3/4" = 1'-0"



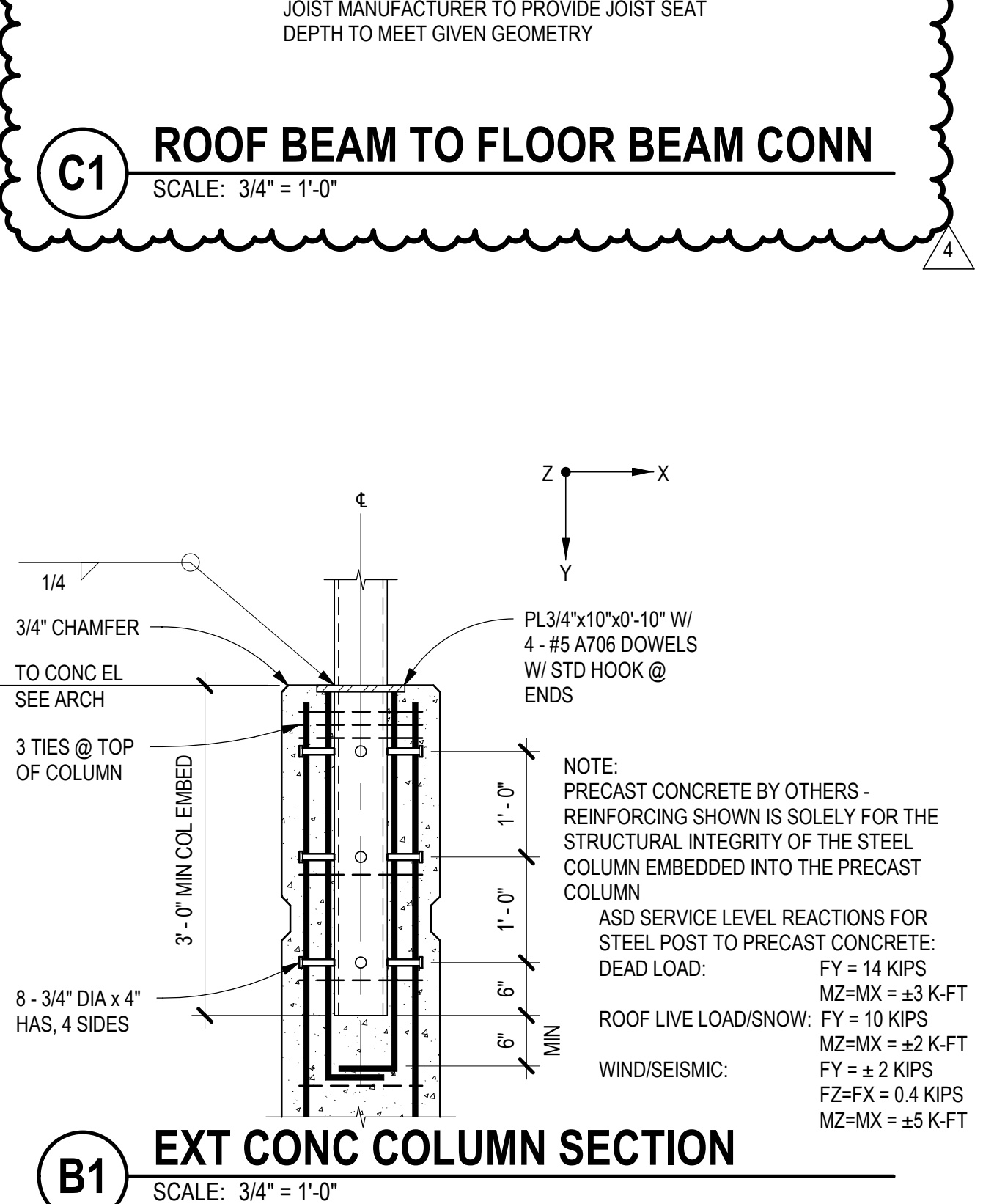
B4 ROOF FRAMING SECTION
SCALE: 3/4" = 1'-0"



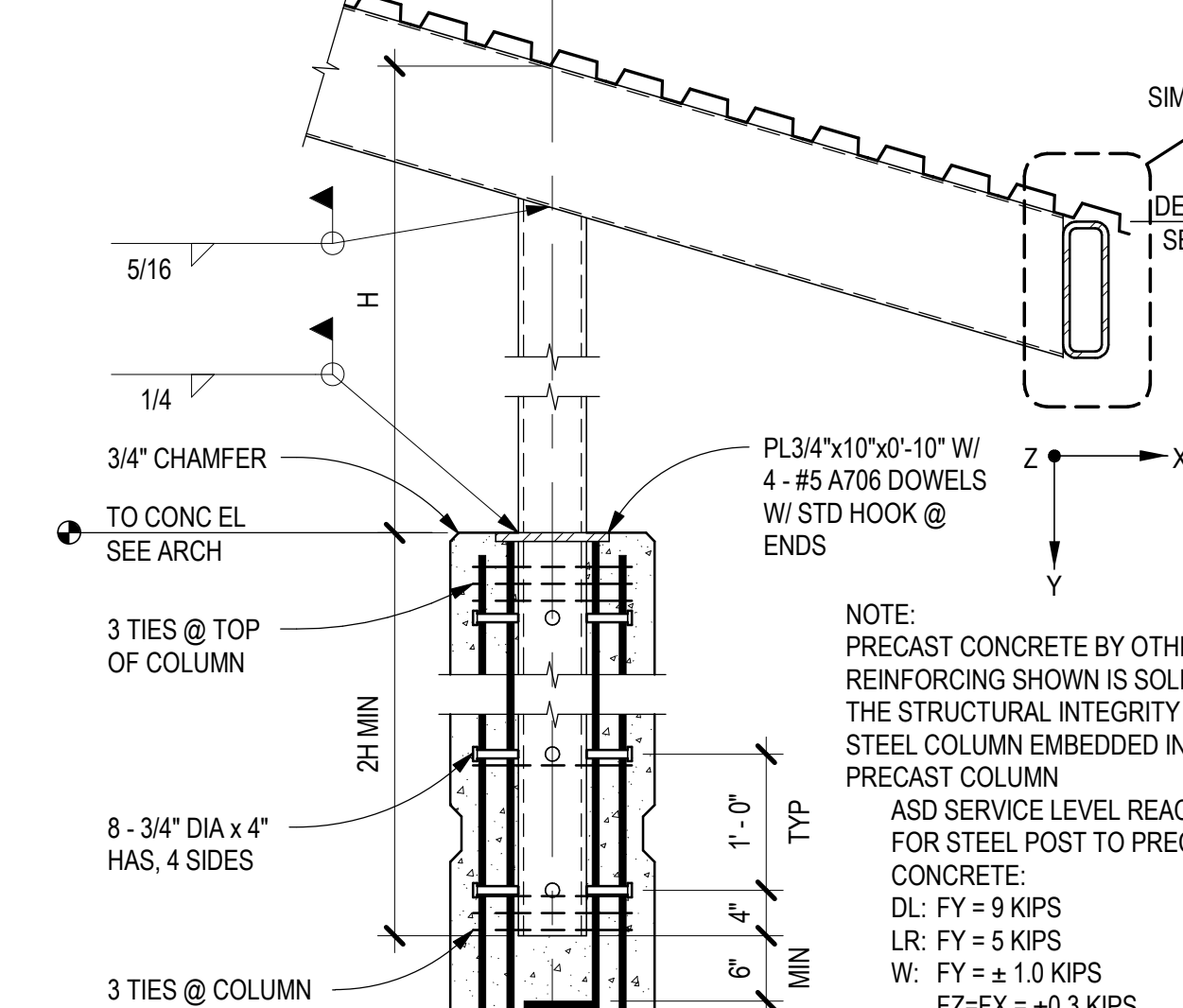
B3 FRAMING SECTION
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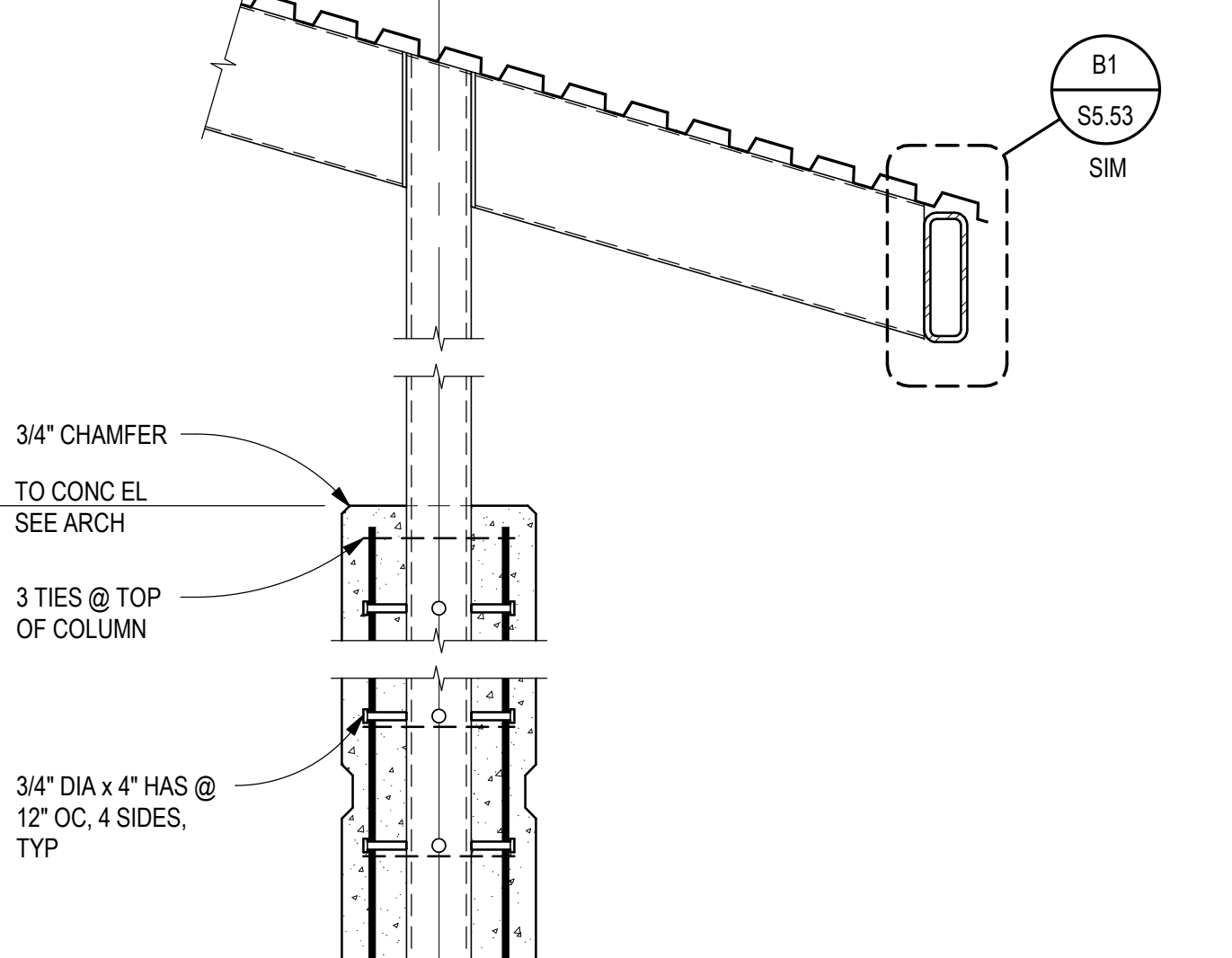
B2 ROOF FRAMING SECTION
SCALE: 3/4" = 1'-0"



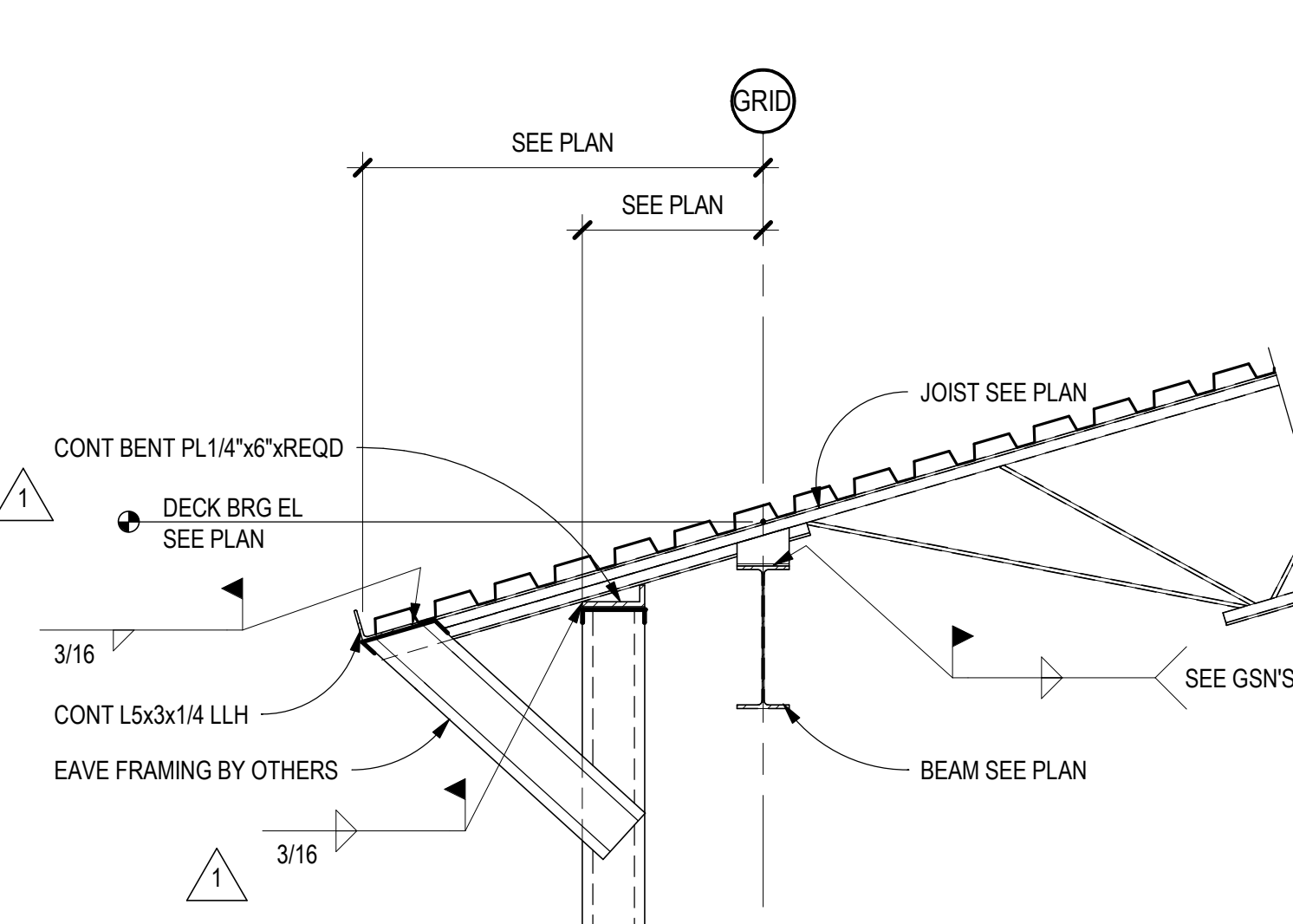
B1 EXT CONC COLUMN SECTION
SCALE: 3/4" = 1'-0"



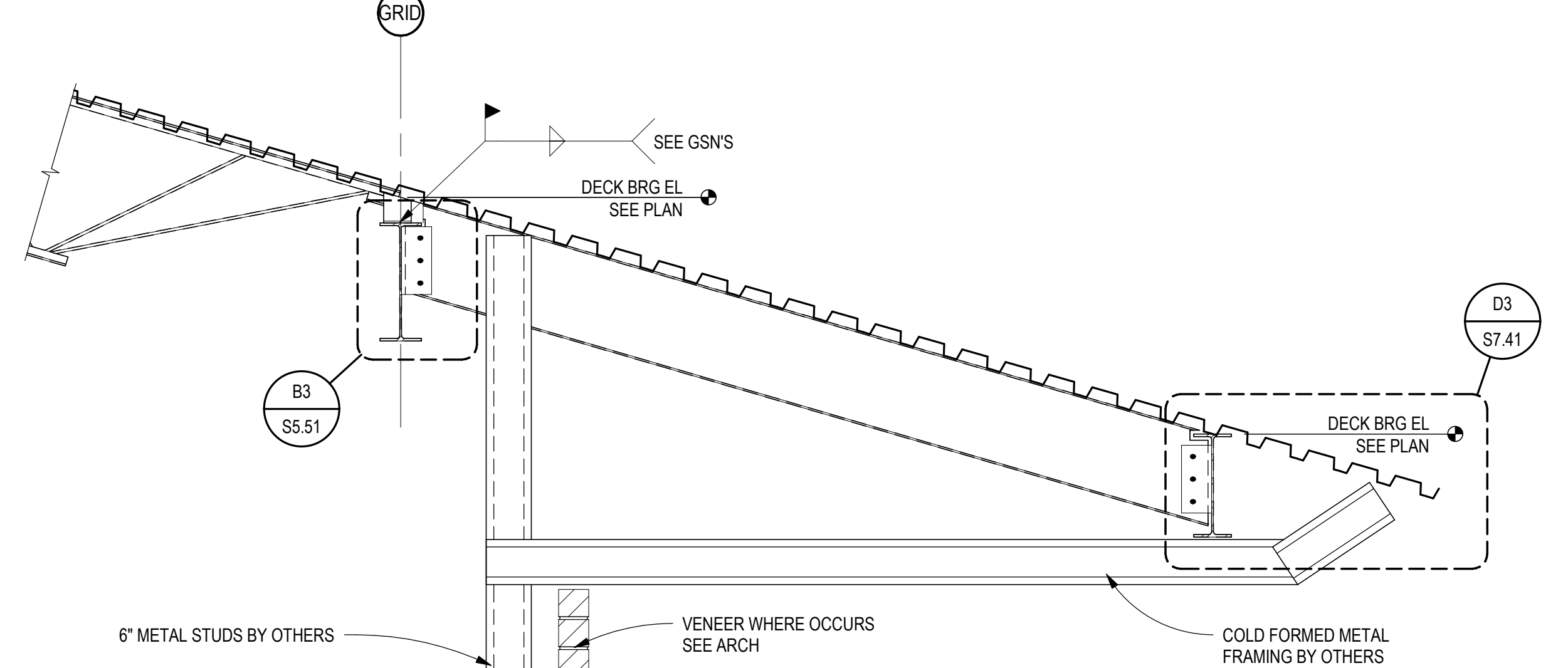
A5 EXT CONC COLUMN SECTION
SCALE: 3/4" = 1'-0"



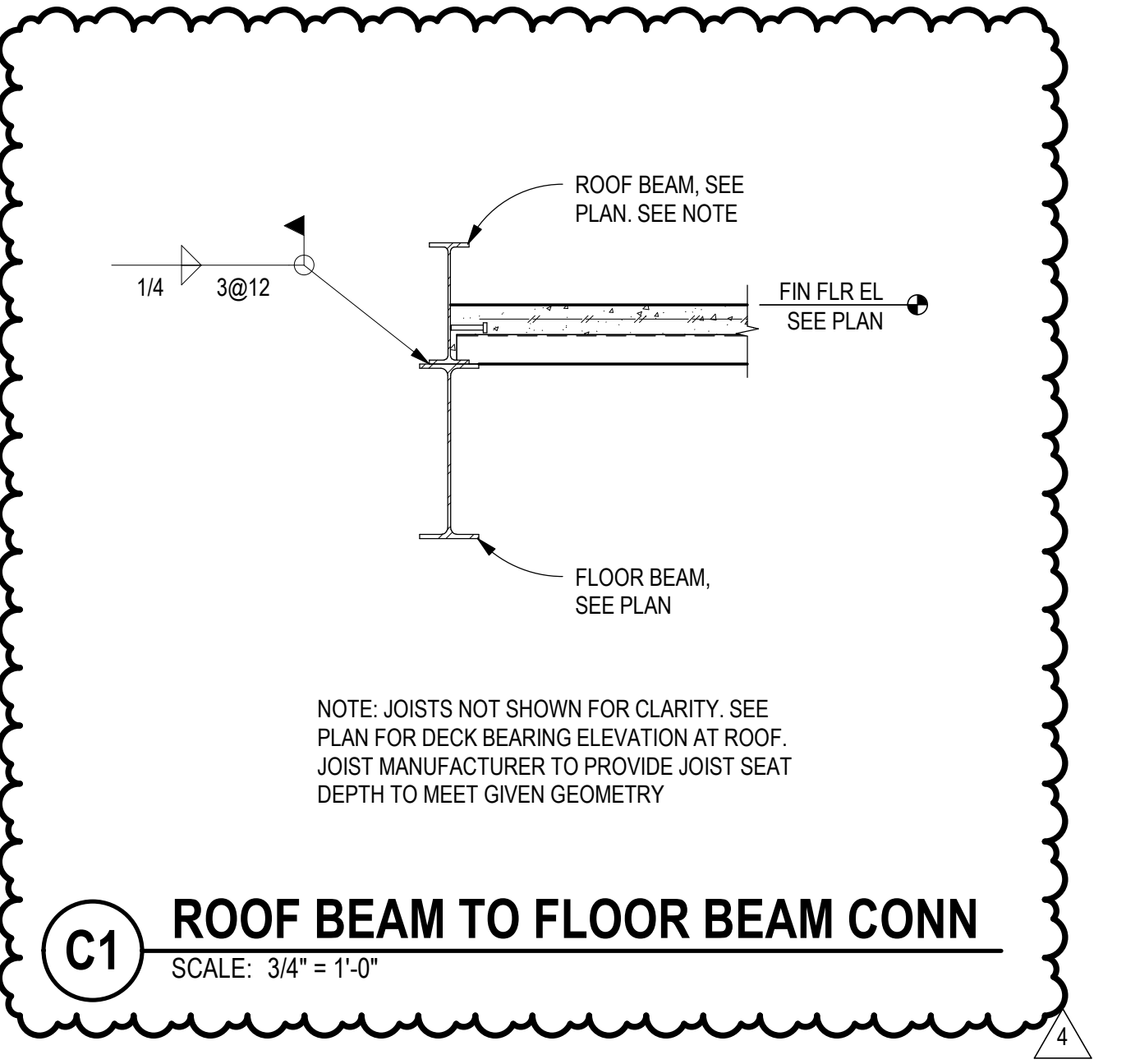
A1 EXT CONC COLUMN SECTION
SCALE: 3/4" = 1'-0"



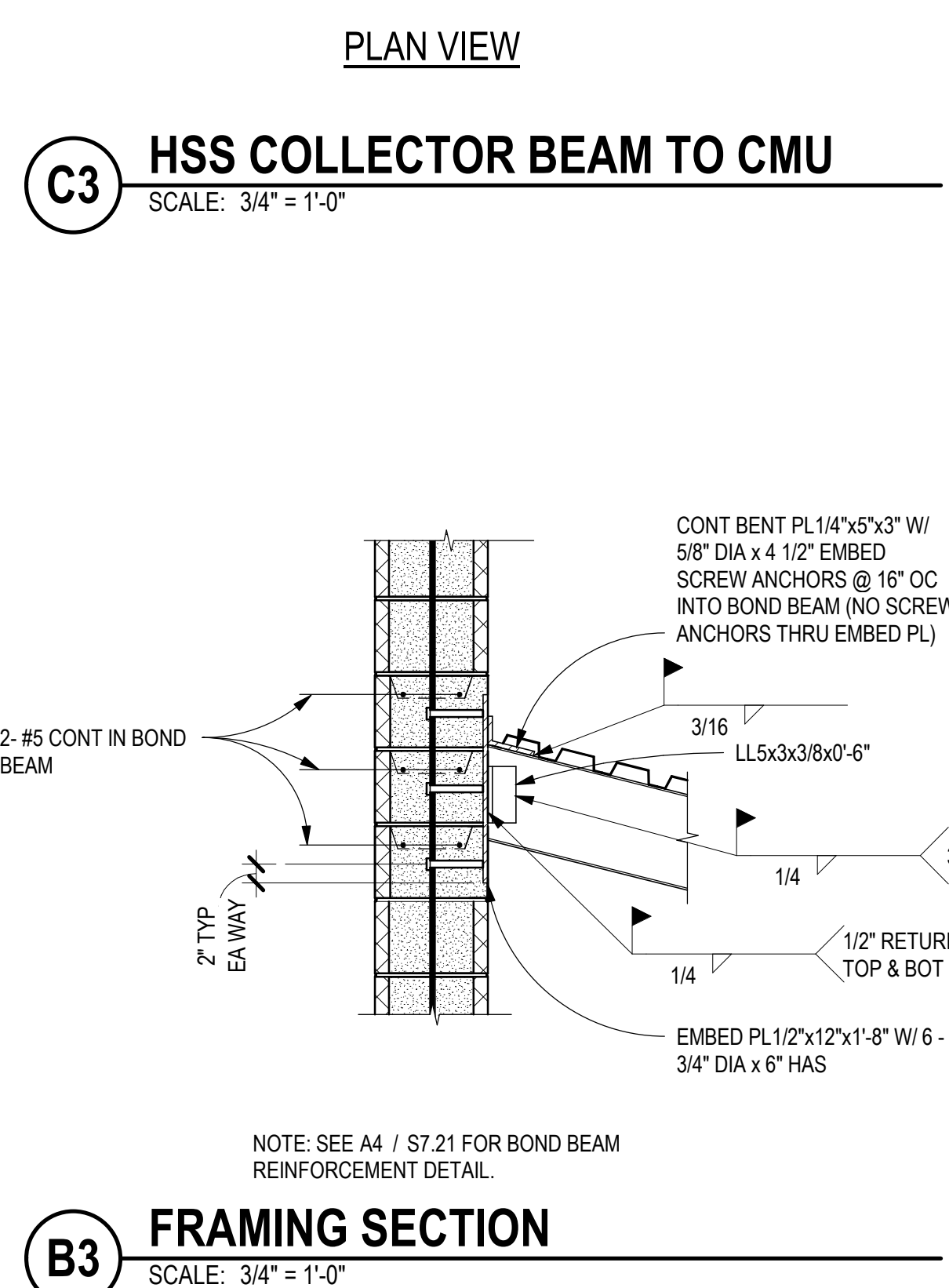
A4 TYPICAL ROOF FRAMING SECTION
SCALE: 3/4" = 1'-0"



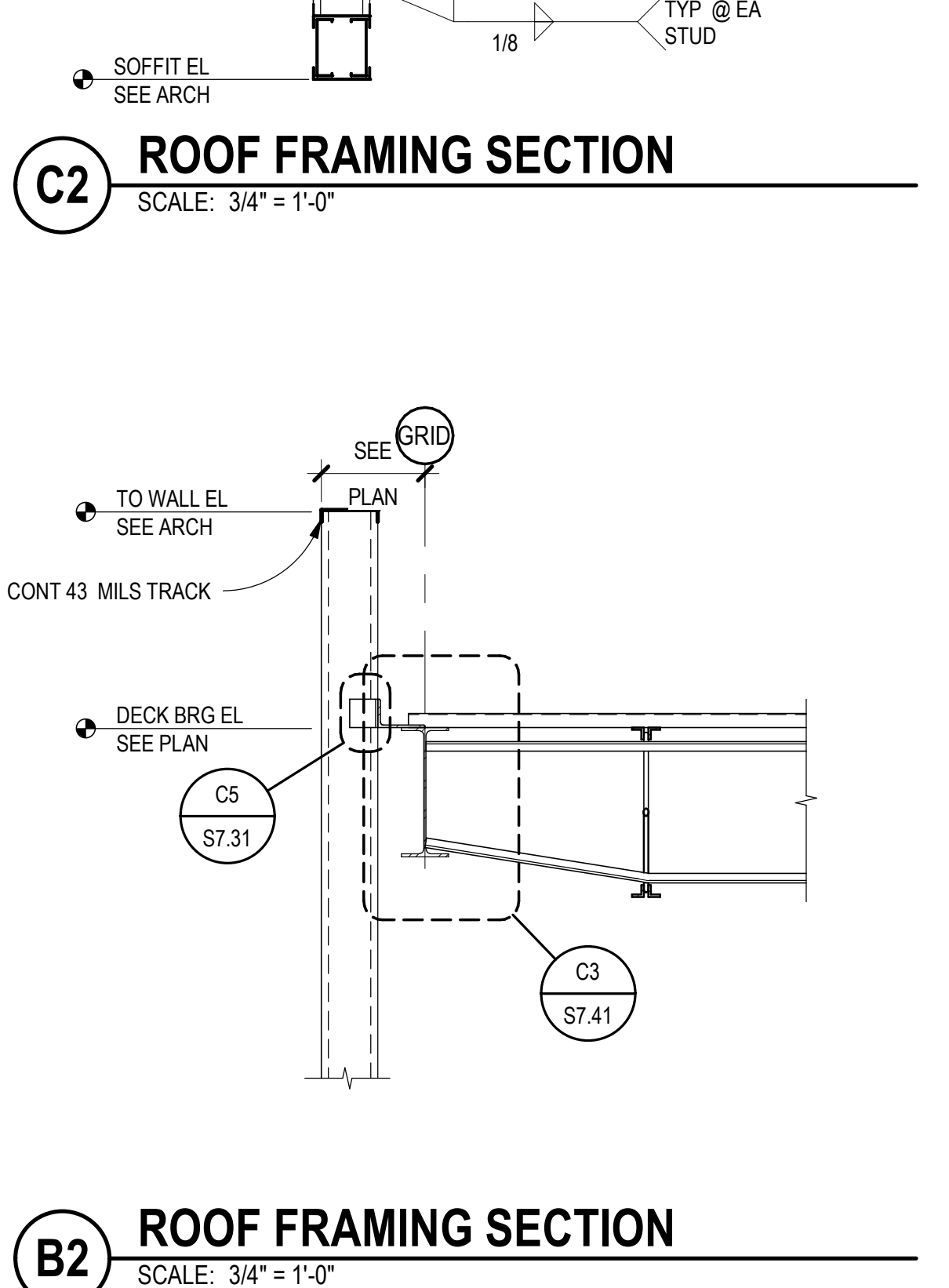
A2 ROOF FRAMING SECTION
SCALE: 3/4" = 1'-0"



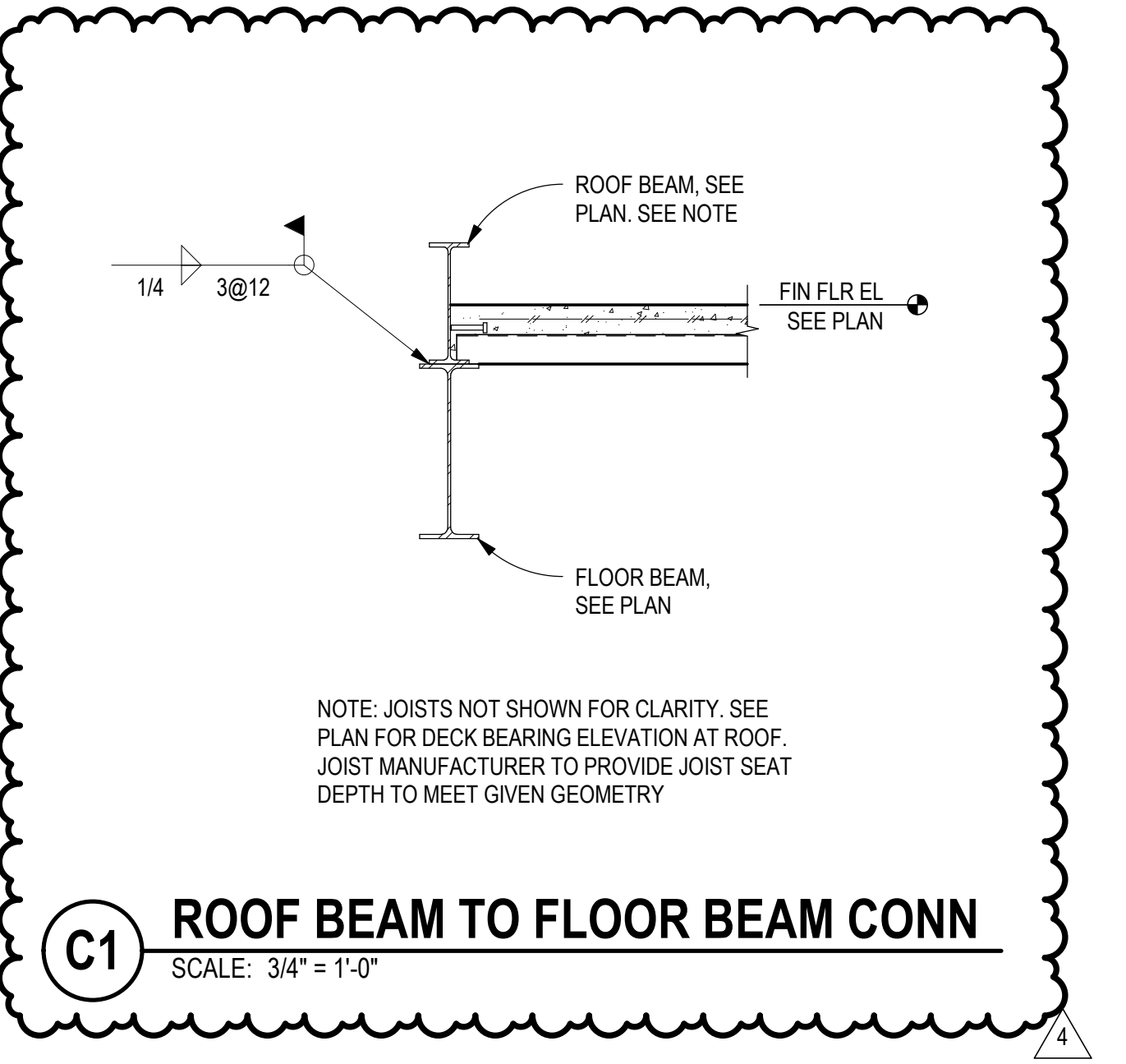
C1 ROOF BEAM TO FLOOR BEAM CONN
SCALE: 3/4" = 1'-0"



C3 HSS COLLECTOR BEAM TO CMU
SCALE: 3/4" = 1'-0"



C2 ROOF FRAMING SECTION
SCALE: 3/4" = 1'-0"



C1 ROOF BEAM TO FLOOR BEAM CONN
SCALE: 3/4" = 1'-0"

