INDEX OF DRAWINGS - VOLUME 01 - CIVIL

SHEET NUMBER	SHEET NAME	BID PACKAGE 01 - 07-29-2022	ADDENDUM 01 - 08-17-2022	BID PACKAGE 03 - 10-07-2022	BID PACKAGE 02 - 10-21-2022	ASI 01 - 10-24-2022	ADDENDUM 02 - 11-18-2022	BID PACKAGE 04 - 12-09-2022
GENERAL								
VOL 01	COVER SHEET	■			■		■	■
G0.01	DRAWING INDEX	■		■	■		■	■
CIVIL								
C0-101	GENERAL CIVIL NOTES -BP2				■		■	
C0-102	GENERAL CIVIL NOTES - BP1	■						
C0-103	GENERAL CIVIL NOTES - BP4							■
C0-901	PROJECT MAP							■
C1-101	EXISTING CONDITIONS				■			
C1-102	EXISTING CONDITIONS				■			
C1-103	EXISTING CONDITIONS	■						
C1-104	EXISTING CONDITIONS	■						
C1-105	EXISTING CONDITIONS	■						
C1-106	EXISTING CONDITIONS				■			
C1-107	EXISTING CONDITIONS				■			
C2-101	OVERALL DEMOLITION VIEW				■			
C2-102	A1 DEMOLITION PLAN				■			
C2-103	A2 DEMOLITION PLAN				■			
C2-104	B1 DEMOLITION PLAN				■			
C2-105	B2 DEMOLITION PLAN				■			
C2-106	C1 DEMOLITION PLAN				■			
C2-107	C2 DEMOLITION PLAN				■			
C2-108	NP DEMOLITION OVERVIEW	■						
C2-109	NP 1 DEMOLITION PLAN	■						
C2-110	NP2 DEMOLITION PLAN	■						
C2-111	NP3 DEMOLITION PLAN	■						
C2-112	NP4 DEMOLITION PLAN	■						
C2-113	NP5 DEMOLITION PLAN	■						
C2-701	EROSION CONTROL PLAN - HOSPITAL				■			
C2-702	NP EROSION CONTROL PLAN	■						
C2-750	EROSION CONTROL DETAILS	■						
C2-751	EROSION CONTROL DETAILS				■			
C3-101	OVERALL SITE VIEW							■
C3-102	A1 SITE PLAN							■
C3-103	A2 SITE PLAN							■
C3-104	B1 SITE PLAN							■
C3-105	B2 SITE PLAN							■
C3-106	C1 SITE PLAN							■
C3-107	C2 SITE PLAN							■
C3-108	NP SITE PLAN OVERVIEW	■	■					
C3-109	NP1 SITE PLAN	■	■					
C3-110	NP2 SITE PLAN	■	■					
C3-111	NP3 SITE PLAN	■	■					
C3-112	NP4 SITE PLAN	■	■					
C3-113	NP5 SITE PLAN	■	■					
C3-501	SITE DETAILS	■						
C3-502	SITE DETAILS	■						
C3-503	CONCRETE PAVING JOINT DETAILS	■						
C3-504	SITE DETAILS							■
C3-901	PARKING SPACE ACCOUNTING							■
C4-101	OVERALL UTILITY VIEW				■		■	
C4-102	A1 UTILITIES				■			
C4-103	A2 UTILITIES				■		■	
C4-104	B1 UTILITIES				■			
C4-105	B2 UTILITIES				■			
C4-106	C1 UTILITIES				■			
C4-107	C2 UTILITIES				■			
C4-201	LREC RELOCATION PLAN AND PROFILE					■		
C4-203	PHASE 1 UTILITY PLAN						■	
C4-801	SITE ELECTRIC AND COMMUNICATION CONDUITS				■			
C5-101	OVERALL GRADING VIEW							■
C5-102	A1 GRADING PLAN							■
C5-103	A2 GRADING PLAN							■
C5-104	B1 GRADING PLAN							■
C5-105	B2 GRADING PLAN							■
C5-106	C1 GRADING PLAN							■
C5-107	C2 GRADING PLAN							■
C5-113	NP GRADING OVERVIEW	■						
C5-114	NP1 GRADING PLAN	■						
C5-115	NP2 GRADING PLAN	■						
C5-116	NP3 GRADING PLAN	■						
C5-117	NP4 GRADING PLAN	■						
C5-118	NP5 GRADING PLAN	■						
C5-201	COORDINATES AND ELEVATIONS							■
C5-202	LOOP DRIVE PLAN AND PROFILE							■
C5-203	LOOP DRIVE PLAN AND PROFILE							■
C5-204	LOOP DRIVE PLAN AND PROFILE							■
C5-205	WEST DRIVE PLAN AND PROFILE							■
C5-206	WEST DRIVE PLAN AND PROFILE							■
C5-207	GABION WALL PLAN AND PROFILE							■
C5-208	BLOCK WALL PLAN AND PROFILE							■
C5-210	NP COORDINATES AND ELEVATIONS TABLE	■						
C5-501	GABION WALL DETAILS							■
C5-502	MASONRY BLOCK WALL DETAILS							■
C5-801	EARTWORK ANALYSIS							■
C6-101	STORM DRAIN PLAN				■		■	
C6-102	STORM DRAIN PLAN				■		■	
C6-103	ROOF DRAIN PLAN				■			
C6-104	FOUNDATION DRAIN PLAN				■			
C6-201	NP STORM PLAN & PROFILE	■						
C6-202	NP STORM PLAN & PROFILE	■						
C6-203	STORM DRAIN PROFILES				■		■	
C6-204	STORM DRAIN PROFILES				■		■	
C6-501	STORM DRAIN DETAILS	■						
C6-502	STORM DRAIN DETAILS	■						
C6-503	STORM DRAIN DETAILS	■						
C6-504	STORM DRAIN DETAILS	■						
C6-505	STORM DRAIN DETAILS	■						
C6-506	CICI BOX				■			
C6-507	CICI FRAME				■			
C6-508	CICI GRATE				■			
C6-509	DESIGN 6 AND 7 AREA INLET				■			
C6-510	GRATE AND FRAME FOR AREA INLET				■			
C6-511	STORM DRAIN DETAILS				■			
C6-802	NP EXISTING HYDROLOGY	■						
C6-803	NP DEVELOPMENT HYDROLOGY	■						
C7-101	OVERALL WATER PLAN				■			
C7-201	WATER 1 PLAN AND PROFILE				■			
C7-202	WATER 1 PLAN AND PROFILE				■			
C7-203	WATER 1 PLAN AND PROFILE				■			
C7-204	WATER 2 PLAN AND PROFILE				■			
C7-205	WATER 2 PLAN AND PROFILE				■			
C7-206	WATER 2 PLAN AND PROFILE				■		■	
C7-207	WATER 2 PLAN AND PROFILE				■		■	
C7-208	WATER 3 PLAN AND PROFILE				■			
C7-209	WATER 3 PLAN AND PROFILE				■			
C7-210	WATER 4 PLAN AND PROFILE				■		■	
C7-211	WATER 5 AND 14 PLAN AND PROFILE				■			
C7-212	WATER 6, 9, AND 10 PLAN AND PROFILE				■		■	
C7-213	WATER 7, 8, AND 13 PLAN AND PROFILE				■		■	
C7-214	WATER 11 AND 12 PLAN AND PROFILE				■		■	
C7-501	TPWA WATER DETAILS				■			
C7-502	WATER DETAILS				■			
C8-201	FORCE MAIN PLAN & PROFILE	■						
C8-501	FORCE MAIN DETAILS	■						
C9-201	DOWNING DRIVEWAY PLAN AND PROFILE	■	■					
C9-501	DOWNING DRIVEWAY DETAILS	■						

Grand total: 125

INDEX OF DRAWINGS - VOLUME 02 - LANDSCAPE

SHEET NUMBER	SHEET NAME	BID PACKAGE 01 - 07-29-2022	ADDENDUM 01 - 08-17-2022	BID PACKAGE 03 - 10-07-2022	BID PACKAGE 02 - 10-21-2022	ASI 01 - 10-24-2022	ADDENDUM 02 - 11-18-2022	BID PACKAGE 04 - 12-09-2022
GENERAL								
VOL 02	COVER SHEET							■
G0.01	DRAWING INDEX	■	■	■	■	■	■	■
LANDSCAPE								
HA1.1	SITE HARDSCAPE PLAN							■
HA1.2	ENLARGED HARDSCAPE PLAN - SOUTH							■
HA1.3	ENLARGED HARDSCAPE PLAN - CENTRAL							■
HA1.4	ENLARGED HARDSCAPE PLAN - NORTH							■
HA2.1	HARDSCAPE DETAILS							■

Grand total: 7

INDEX OF DRAWINGS - VOLUME 04 - STRUCTURAL

SHEET NUMBER	SHEET NAME	BID PACKAGE 01 - 07-29-2022	ADDENDUM 01 - 08-17-2022	BID PACKAGE 03 - 10-07-2022	BID PACKAGE 02 - 10-21-2022	ASI 01 - 10-24-2022	ADDENDUM 02 - 11-18-2022	BID PACKAGE 04 - 12-09-2022
GENERAL								
VOL 04	COVER SHEET							■
G0.01	DRAWING INDEX	■	■	■	■	■	■	■
STRUCTURAL								
S0.1	COVER SHEET							■
S0.2	GENERAL NOTES							■
S0.3	GENERAL STRUCTURAL NOTES							■
S0.4	SPECIAL INSPECTIONS TABLE							■
S1.01.1	01 OVERALL FOUNDATION PLAN							■
S1.01.2	01 SLAB PLAN							■
S1.01.3	ELEVATOR PIT PLAN							■
S1.01.4	DRILLED PIER PLAN							■
S2.01.1	FOUNDATION PLAN SECTOR 01							■
S2.01.2	FOUNDATION PLAN SECTOR 02							■
S2.01.3	FOUNDATION PLAN SECTOR 03							■
S2.01.4	FOUNDATION PLAN SECTOR 04							■
S2.01.5	FOUNDATION PLAN SECTOR 05							■
S4.01	CONCRETE TYPICAL DETAILS							■
S4.02	CONCRETE TYPICAL DETAILS							■
S4.03	FOUNDATION DETAILS							■
S4.05	BASE PLATE DETAILS							■
S4.06	FOUNDATION SECTIONS							■
S4.07	FOUNDATION SECTIONS							■
S4.08	FOUNDATION SECTIONS							■
S4.09	FOUNDATION SECTIONS							■
SB101	SKYBRIDGE - ENLARGEMENTS							■
SB102	SKYBRIDGE - SECTIONS							■
SC100	SOUTH CANOPY							■
SC101	SOUTH CANOPY - FOUNDATION PLAN							■
SC103	SOUTH CANOPY - FRAMES							■
SC104	SOUTH CANOPY - DETAILS							■
SE101	EAST CANOPY - STRUCTURAL PLAN							■
SM101	MECHANICAL AREA - OVERALL FOUNDATION PLAN							■
SM111	MECHANICAL AREA (BRIDGE) - ENLARGEMENTS							■
SM112	MECHANICAL AREA (BRIDGE) - SECTIONS							■
SM113	MECHANICAL AREA - FOUNDATION DETAILS							■
SM114	MECHANICAL AREA - ANCHOR RODS							■

Grand total: 35

INDEX OF DRAWINGS - VOLUME 07 - ELECTRICAL

SHEET NUMBER	SHEET NAME	BID PACKAGE 01 - 07-29-2022	ADDENDUM 01 - 08-17-2022	BID PACKAGE 03 - 10-07-2022	BID PACKAGE 02 - 10-21-2022	ASI 01 - 10-24-2022	ADDENDUM 02 - 11-18-2022	BID PACKAGE 04 - 12-09-2022
GENERAL								
VOL 07	COVER SHEET	■						
G0.01	DRAWING INDEX	■	■	■	■	■	■	■
ELECTRICAL								
ENL.00	ELECTRICAL LEGEND	■						
ENL.01	ELECTRICAL SITE PLAN - NORTH LOT	■						

Grand total: 4

INDEX OF DRAWINGS - VOLUME 09 -TECHNOLOGY

SHEET NUMBER	SHEET NAME	BID PACKAGE 01 - 07-29-2022	ADDENDUM 01 - 08-17-2022	BID PACKAGE 03 - 10-07-2022	BID PACKAGE 02 - 10-21-2022	ASI 01 - 10-24-2022	ADDENDUM 02 - 11-18-2022	BID PACKAGE 04 - 12-09-2022
GENERAL								
VOL 09	COVER SHEET	■						
G0.01	DRAWING INDEX	■	■	■	■	■	■	■
TECHNOLOGY								
TN51.02	NETWORK SITE PLAN - NORTH PARKING & ACCESS	■	■					

Grand total: 3

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CONSULTANT LOGO:

CLIENT:

SEAL OF THE CHEROKEE NATION — SEPT. 6, 1839

CHEROKEE NATION REPLACEMENT HOSPITAL

TAHLEQUAH, OKLAHOMA

KEY PLAN:

PROJECT PHASE:

BID PACKAGE 04

(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

#	DATE	DESCRIPTION
1	08-17-22	ADDENDUM 01
2	10-07-22	BID PACKAGE 03
3	10-21-22	BID PACKAGE 02
4	10-24-22	ASI 01
5	11-18-22	ADDENDUM 02
6	12-09-22	BID PACKAGE 04

JOB NUMBER: 21-08.21

DATE: 07-29-2022

SHEET NUMBER: **G0.01**

SHEET TITLE: DRAWING INDEX

1" = 1'-0" 3" = 1'-0" 1 1/2" = 1'-0" 1" = 1'-0" 3/4" = 1'-0" 1/2" = 1'-0" 3/8" = 1'-0" 1/4" = 1'-0" 1/8" = 1'-0"

SITE WORK CONSTRUCTION DOCUMENTS FOR

CHEROKEE NATION - REPLACEMENT HOSPITAL

BID PACKAGE 4 - STRUCTURAL CONCRETE / EARTHWORK

BID PACKAGE 4 GENERAL NOTES

1. REFER TO SHEET C0-102 BID PACKAGE 1 "GENERAL CIVIL NOTES" FOR THE GENERAL CIVIL SITE WORK NOTES THAT APPLY TO ALL CIVIL SITE WORK CONSTRUCTION.
2. REFER TO BID PACKAGE 1 FOR UTILITIES LOCATED IN THE NORTH PARKING LOT, AND FOR GAS SUPPLY FROM NOPFA AND FIBER FROM COX.
3. REFER TO BID PACKAGE 2 FOR UTILITIES.
4. SANITARY SEWER UTILITIES WILL BE ISSUED IN A FUTURE BID PACKAGE.

LINE LEGEND

- BARBED WIRE FENCE
- CHAINLINK FENCE
- PIPERAIL FENCE
- STOCKADE FENCE
- SECURITY FENCE
- ELECTRIC UNDERGROUND
- OVERHEAD ELECTRIC
- GAS LINE
- SANITARY SEWER
- STORM DRAIN PIPE (SURVEYED)
- STORM DRAIN CENTERLINE
- WATER
- NONPOTABLE WATER
- CURB AND GUTTER
- SURFACE DRAINAGE FLOWLINE
- FIBER OPTIC
- TELEPHONE (AERIAL)
- TELEPHONE (BURIED)
- TELEVISION (AERIAL)
- TELEVISION (BURIED)
- CENTERLINE
- EASEMENTS
- PROPERTY LINE
- RIGHT OF WAY
- EXISTING BUILDINGS
- BUILDING SETBACK
- EX 1 FT CONTOUR
- EX 5 FT CONTOUR
- FG 1 FT CONTOUR
- FG 5 FT CONTOUR
- PROJECT BOUNDARY
- DEMOLITION LINE

EXISTING LINE LEGEND

- COMMUNICATIONS FIBER
- ELECTRIC OVERHEAD
- ELECTRIC UNDERGROUND
- NATURAL GAS
- OXYGEN GAS
- SEWER
- STORM SEWER
- WATER
- FOUNDATION DRAIN

BID PACKAGE 2 SHEET LIST

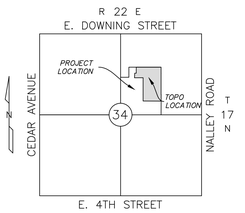
Sheet List Table

Sheet Number	Sheet Title
C0-103	General Civil Notes – BP4
C0-901	Project Map
C3-101	Overall Site View
C3-102	A1 Site Plan
C3-103	A2 Site Plan
C3-104	B1 Site Plan
C3-105	B2 Site Plan
C3-106	C1 Site Plan
C3-107	C2 Site Plan
C3-504	Site Details
C3-901	Parking Space Accounting
C5-101	Overall Grading View
C5-102	A1 Grading Plan
C5-103	A2 Grading Plan
C5-104	B1 Grading Plan
C5-105	B2 Grading Plan
C5-106	C1 Grading Plan
C5-107	C2 Grading Plan
C5-201	Coordinates and Elevations
C5-202	Loop Drive Plan and Profile
C5-203	Loop Drive Plan and Profile
C5-204	Loop Drive Plan and Profile
C5-205	West Drive Plan and Profile
C5-206	West Drive Plan and Profile
C5-207	Gabion Wall Plan and Profile
C5-208	Block Wall Plan and Profile
C5-501	Gabion Wall Details
C5-502	Masonry Block Wall Details
C5-801	Earthwork Analysis

ABBREVIATIONS

@	AT
AFF	ABOVE FINISHED FLOOR
AGRD	ADJACENT GRADE
AHJ	AUTHORITY HAVING JURISDICTION
APPROX	APPROXIMATE
ARCH	ARCHITECTURAL
ASS'Y	ASSEMBLY
BFF	BELOW FINISHED FLOOR
BLDG	BUILDING
CIP	CAST IN PLACE
CL	CENTERLINE
CM	CONSTRUCTION MANAGER
CNTR	CENTER
CONC	CONCRETE
CONST	CONSTRUCT
CONT	CONTINUOUS
CONTR	CONTRACTOR
COORD	COORDINATE
DIA	DIAMETER
DS	DOWN SPOUT
DTL	DETAIL
DWG(S)	DRAWINGS
E	EAST
E	EASTING
EG	EXISTING GRADE
EGCL	EXISTING GRADE CENTER LINE
ELEC	ELECTRICAL
ELEV	ELEVATION
EJ	EXPANSION JOINT
EQ	EQUAL
ETR	EXISTING TO REMAIN
EX	EXISTING
EXHD	EXTRA HEAVY DUTY
FD	FLOOR DRAIN
FF	FINISHED FLOOR
FG	FINISHED GRADE
FGCL	FINISHED GRADE CENTER LINE
FL	FLOWLINE
FOC	FACE OF CONCRETE
FT	FOOT/FEET
FTNG	FOOTING
FV	FIELD VERIFY
GA	GAUGE
GALV	GALVANIZED
GC	GENERAL CONTRACTOR
GU	GUTTER
HD	HEAVY DUTY
HORIZ	HORIZONTAL
HT	HEIGHT
IN	INCH
INFO	INFORMATION
ID	INSIDE DIAMETER
IFGC	INTERNATIONAL FUEL GAS CODE
IPC	INTERNATIONAL PLUMBING CODE
JT	JOINT
LD	LIGHT DUTY
LF	LINEAL FEET
MH	MANHOLE
MAX	MAXIMUM
MECH	MECHANICAL
MIN	MINIMUM
MISC	MISCELLANEOUS
N/A	NOT APPLICABLE
OD	OUTSIDE DIAMETER
MTCH	MATCH
MEP	MECHANICAL / ELECTRICAL / PLUMBING
N	NORTH
N	NORTHING
NO.	NUMBER
OC	ON CENTER
ODEQ	OKLAHOMA DEPARTMENT OF ENVIRONMENTAL QUALITY
ODOT	OKLAHOMA DEPARTMENT OF TRANSPORTATION
OWRB	OKLAHOMA WATER RESOURCES BOARD
OZ	OUNCE
PC	PORTLAND CEMENT
PC	PRE-CAST
PC	POINT OF CURVATURE
PLBG	PLUMBING
POC	POINT OF CONNECTION
PRC	POINT OF REVERSE CURVATURE
PT	POINT OF TANGENCY
PVI	POINT OF VERTICAL INTERSECTION
QTY	QUANTITY
R	RADIUS
RE:	REFERENCE
REINF	REINFORCED
REQ'D	REQUIRED
REV	REVISION
S	SOUTH
SAN	SANITARY
SCHED	SCHEDULE
SECT	SECTION
SF	SQUARE FEET
SHT	SHEET
SPEC	SPECIFICATION
SQ	SQUARE
STRUC	STRUCTURAL
STD	STANDARD
SW	SIDEWALK
SY	SQUARE YARD
TC	TOP OF CURB
TEMP	TEMPORARY
TOC	TOP OF CONCRETE
TOE	TOE OF SLOPE
TOP	TOP OF SLOPE
TOW	TOE OF WALL
TPW	TOP OF WALL
TYP	TYPICAL
UNO	UNLESS NOTED OTHERWISE
VERT	VERTICAL
W	WEST
W/	WITH
WT	WEIGHT

LOCATION MAP



LOCATION MAP

HARDSCAPE PATTERNS

- CONCRETE SIDEWALKS
- LIGHT DUTY CONCRETE PAVING
- MEDIUM DUTY CONCRETE PAVING
- HEAVY DUTY CONCRETE PAVING
- REINFORCED HEAVY DUTY CONCRETE PAVING
- SODDING / SEEDING / VEGETATIVE COVER

SYMBOL LEGEND

- AIR CONDITIONER UNIT
- FIRE HYDRANT
- GAS METER
- GAS VALVE
- GAS VENT
- GROUND TRANSFORMER
- GUARD POST
- LIGHT POLE
- STORM DRAIN MANHOLE
- SANITARY SEWER MANHOLE
- PROPERTY CORNER FOUND
- PROPERTY CORNER SET
- SANITARY SEWER CLEANOUT
- COMMERCIAL SIGN
- SIGN
- SPRINKLER HEAD
- SPRINKLER VALVE
- TELEPHONE RISER
- TRAFFIC/ELECTRIC PULL BOX
- WATER METER
- WATER VALVE BOX

TC=1121.63	TOP OF CURB SPOT ELEVATION
G=1121.13	GUTTER SPOT ELEVATION
X 1123.5	SPOT ELEVATION

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SEPT. 6, 1839

CHEROKEE NATION REPLACEMENT HOSPITAL

TAHLEQUAH, OKLAHOMA

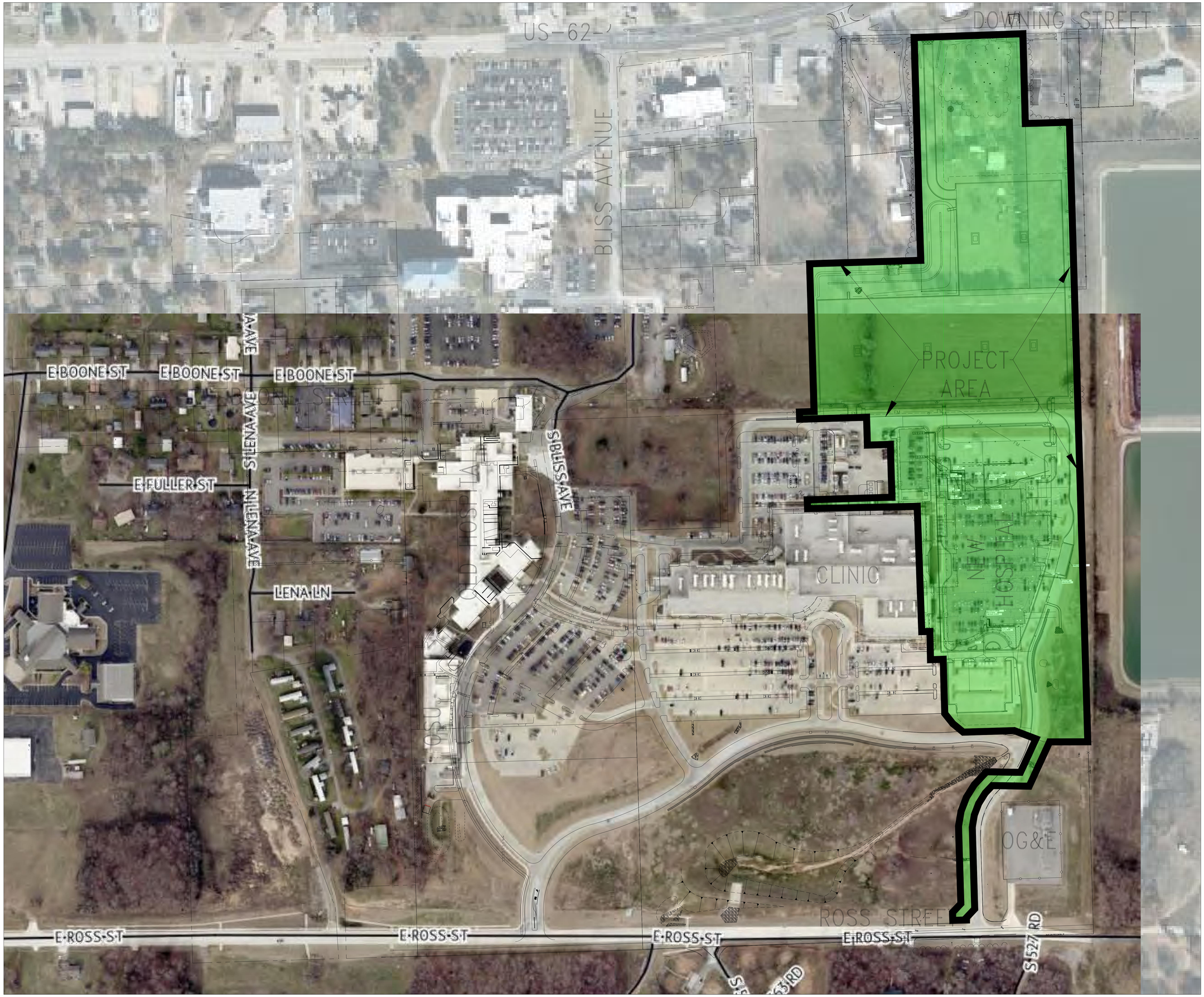
KEY PLAN:

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

JOB NUMBER:	21-08.21
DATE:	12-09-2022
SHEET NUMBER:	**C0-103**
SHEET TITLE:	GENERAL CIVIL NOTES - BP4

12/9/2022 A:\2021\5123.21\03_DSGN\01_DWG\050_CIVIL\C0-TITL-BP4-5123.DWG



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CHEROKEE NATION
REPLACEMENT HOSPITAL
TAHLEQUAH, OKLAHOMA

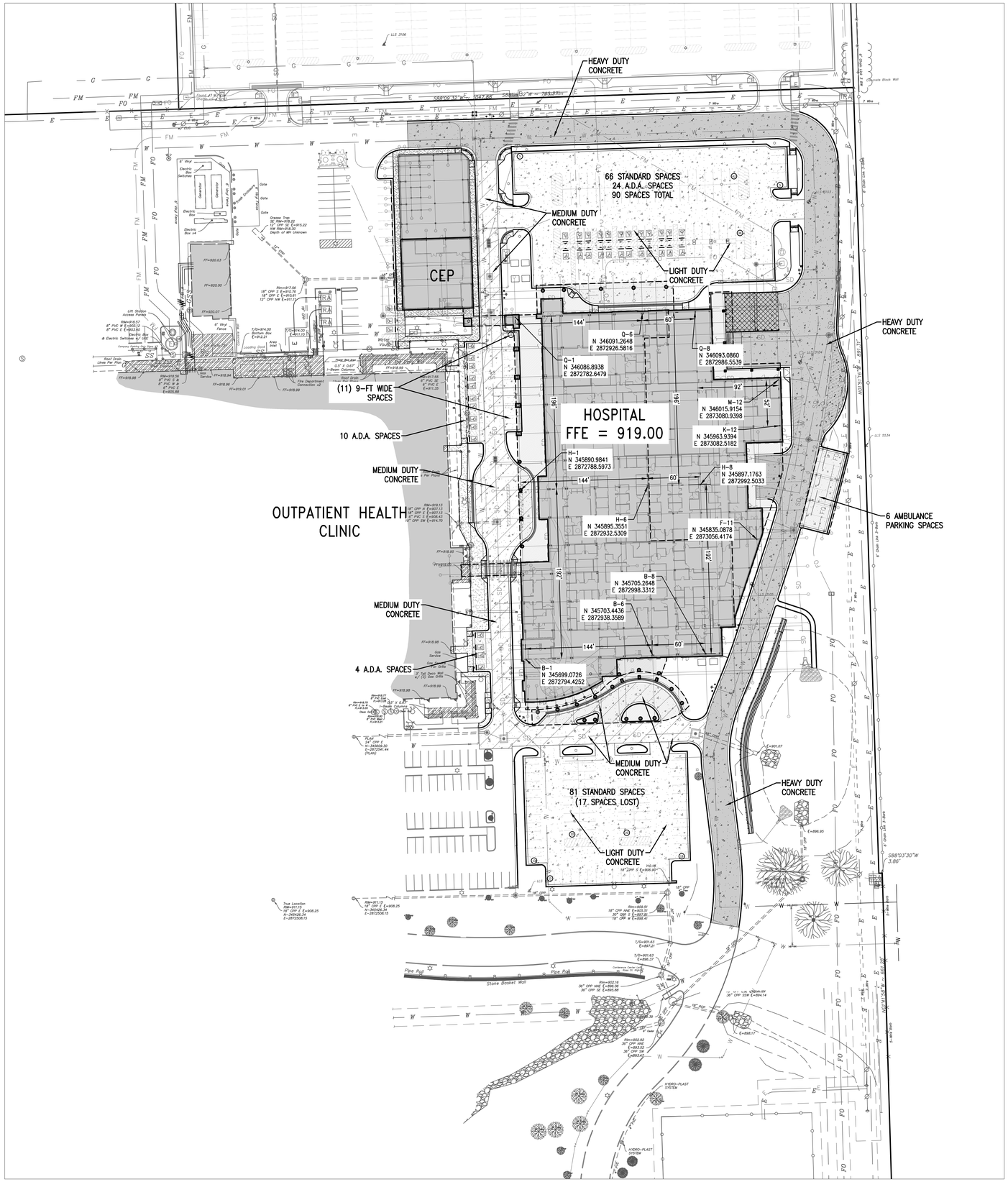
KEY PLAN:

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

JOB NUMBER: 21-08.21
DATE: 12-09-2022
SHEET NUMBER: **C0-901**
SHEET TITLE: PROJECT MAP

12/9/2022 A:\2021\5123\21\03_DSGN\01_DWG\050_CIVIL\C0-MAP-5123.DWG



PARKING TOTALS

ON-SITE SPACES LOST AND RECONSTRUCTED
807 PARKING SPACES
46 A.D.A SPACES
761 STANDARD SPACES

ON-SITE SPACES ADDED IN THE DESIGN
196 PARKING SPACES
38 A.D.A. SPACES
158 STANDARD SPACES

NET LOSS OF ON-SITE SPACES
611 PARKING SPACES LOST
8 A.D.A. SPACES LOST
603 STANDARD SPACES LOST

NORTH SURFACE PARKING
846 STANDARD SPACES

PARKING SPACES DIFFERENCE
8 A.D.A. SPACES LOST
243 STANDARD SPACES ADDED
235 TOTAL ADDITIONAL PARKING SPACES

ON-SITE PAVING ESTIMATES

3,748 SY 4-IN THK SIDEWALK
8,795 SY 5-IN THK CONCRETE
3,291 SY 6-IN THK CONCRETE
5,159 SY 7-IN THK CONCRETE
6,031 LF 6-IN BARRIER CURB
W/2-FT GUTTER / 6-IN THK

NORTH PARKING PAVING ESTIMATES

891 SY 4-IN THK SIDEWALK
436 SY 5-IN THK CONCRETE
36,345 SY 6-IN THK CONCRETE
101 SY 7-IN THK CONCRETE
3,477 LF 6-IN BARRIER CURB
W/2-FT GUTTER / 6-IN THK

0' 50' 100' 200'
SCALE: 1" = 50'

HARDSCAPE PATTERNS

- CONCRETE SIDEWALKS
- LIGHT DUTY CONCRETE PAVING
- MEDIUM DUTY CONCRETE PAVING
- HEAVY DUTY CONCRETE PAVING
- REINFORCED HEAVY DUTY CONCRETE PAVING
- SODDING / SEEDING / VEGETATIVE COVER

UTILITY WARNING:
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12/9/22

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CLIENT:
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SEPT. 6, 1839

CHEROKEE NATION REPLACEMENT HOSPITAL
TAHLEQUAH, OKLAHOMA

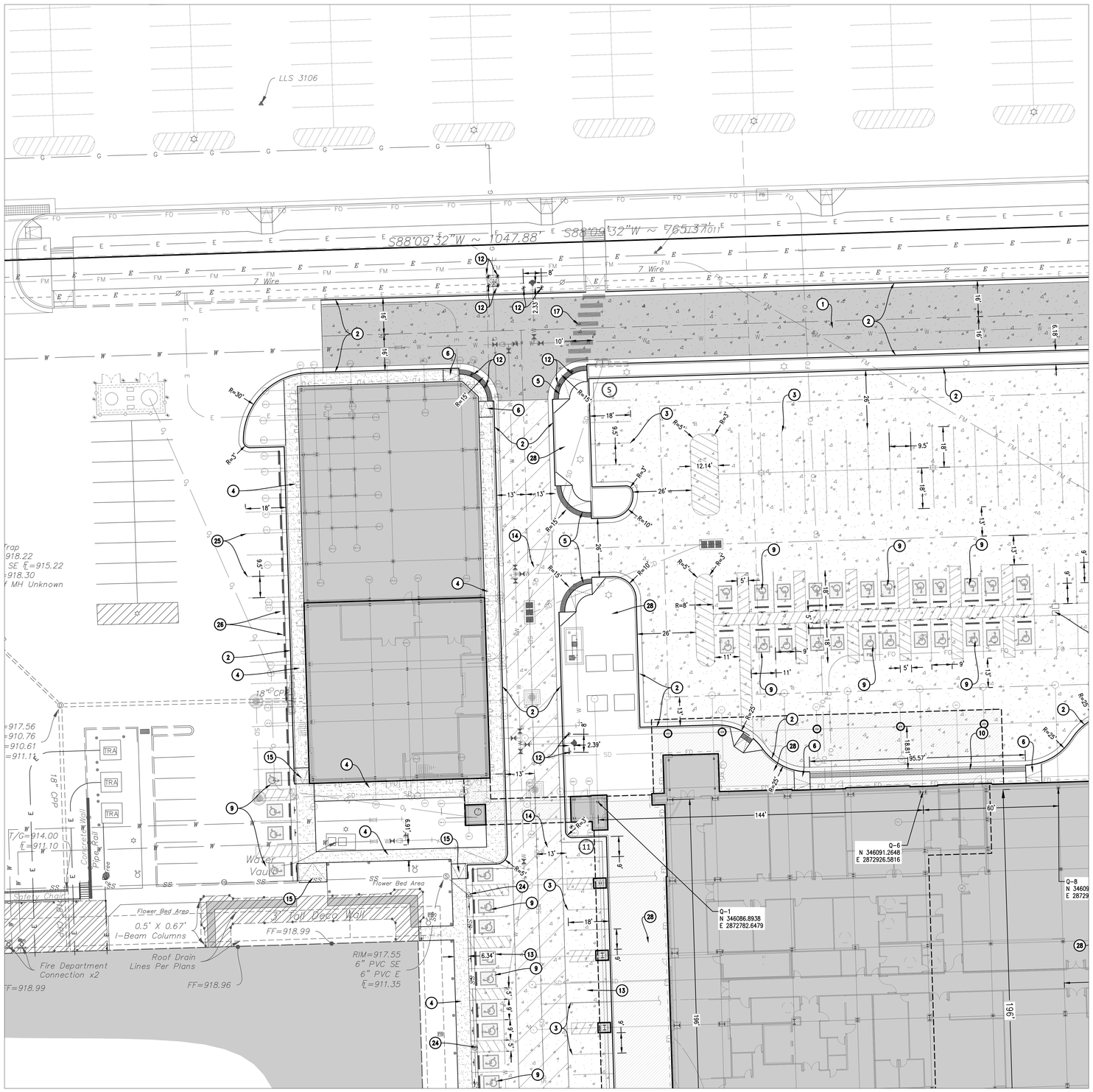
KEY PLAN:
A1 A2
B1 B2
C1 C2

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

JOB NUMBER: 21-08.21
DATE: 12-02-2022
SHEET NUMBER: C3-101
SHEET TITLE: OVERALL SITE VIEW

12/9/2022 A:\2021\5123.21\03_DSGN\01_DWG\050_CIVIL\C3-SITE-5123.DWG



KEYNOTES

1	INSTALL HEAVY DUTY CONCRETE PAVING. SEE DETAIL 4 SHEET C3-504.
2	INSTALL 6-IN TALL BARRIER CURB WITH 24-IN GUTTER. SEE DETAIL 1 SHEET C3-501.
3	INSTALL 4-IN WIDE WHITE TRAFFIC STRIPING FOR PARKING SPACES.
4	CONSTRUCT SIDEWALK. SEE DETAIL 6 SHEET C3-501.
5	INSTALL ADA COMPLIANT CURB TYPE A. REFER TO DETAIL 8 SHEET C3-501.
6	INSTALL ADA COMPLIANT CURB TYPE B. REFER TO DETAIL 9 SHEET C3-501.
7	INSTALL ADA COMPLIANT COMBINATION CURB TYPE D. REFER TO DETAIL 5 SHEET C3-504.
9	CONSTRUCT ADA COMPLIANT ACCESSIBLE PARKING SPACES COMPLETE WITH WHEEL STOPS, PAVEMENT SYMBOL, AISLE STRIPING, AND SIGN.
10	INSTALL 2-FT WIDE ADA TACTILE WARNING STRIP ALONG SIDEWALK WHERE SIDEWALK AND DRIVE ELEVATION ARE EQUAL.
11	CONSTRUCT CONCRETE FLUME. SEE DETAIL 11, SHEET C3-501.
12	INSTALL BOLLARDS, SEE DETAIL 7, SHEET C3-502.
13	INSTALL LIGHT DUTY CONCRETE. SEE DETAIL 2, C3-504.
14	INSTALL MEDIUM DUTY CONCRETE. SEE DETAIL 3, SHEET C3-504.
15	INSTALL ADA COMPLIANT ACCESSIBLE RAMP.
16	INSTALL ELECTRIC VEHICLE CHARGING STATIONS. REFER TO ELECTRIC DRAWINGS.
17	INSTALL 2-FT WIDE X PLAN LENGTH YELLOW CROSSWALK STRIPING WITH 2-FT SPACING BETWEEN STRIPES.
18	INSTALL CONCRETE FLUME. SEE DETAIL 11 SHEET C3-501.
19	INSTALL 4-IN WIDE WHITE NO PARKING STRIPING AT 3-FT SPACING.
20	CONSTRUCT MASONRY BLOCK RETAINING WALL. SEE DETAILS SHEET C5-208.
21	CONSTRUCT GABION BASKET RETAINING WALL. SEE DETAILS SHEET C5-501.
22	INSTALL 2-FT THICK LAYER OF 12-IN STONE RIP-RAP WITH FILTER FABRIC UNDERLAY.
23	INSTALL RIBBON CURB. SEE DETAIL 2 SHEET C3-501.
24	ADJUST EXISTING CLEANOUTS TO BE FLUSH WITH SIDEWALK.
25	REMOVE EXISTING PAVING STRIPES AND RESTRIPE SPACES WITH 4-IN WIDE WHITE STRIPES.
26	INSTALL WHEEL STOPS ALONG PARKING SPACES. SEE DETAIL 6 SHEET C3-504.
27	INSTALL 8-IN WIDE WHITE LANE DIVIDER STRIPING.
28	SEE LANDSCAPE DRAWINGS FOR PEDESTRIAN HARDSCAPE REQUIREMENTS.
29	CONSTRUCT HANDRAIL ALONG TOP OF WALL. SEE DETAIL 8 SHEET C3-504.
30	INSTALL DUMPSTER AREA CONCRETE PAVING. SEE DETAIL 7 SHEET C3-504.

HARDSCAPE PATTERNS

- CONCRETE SIDEWALKS
- LIGHT DUTY CONCRETE PAVING
- MEDIUM DUTY CONCRETE PAVING
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SCALE: 1" = 20'

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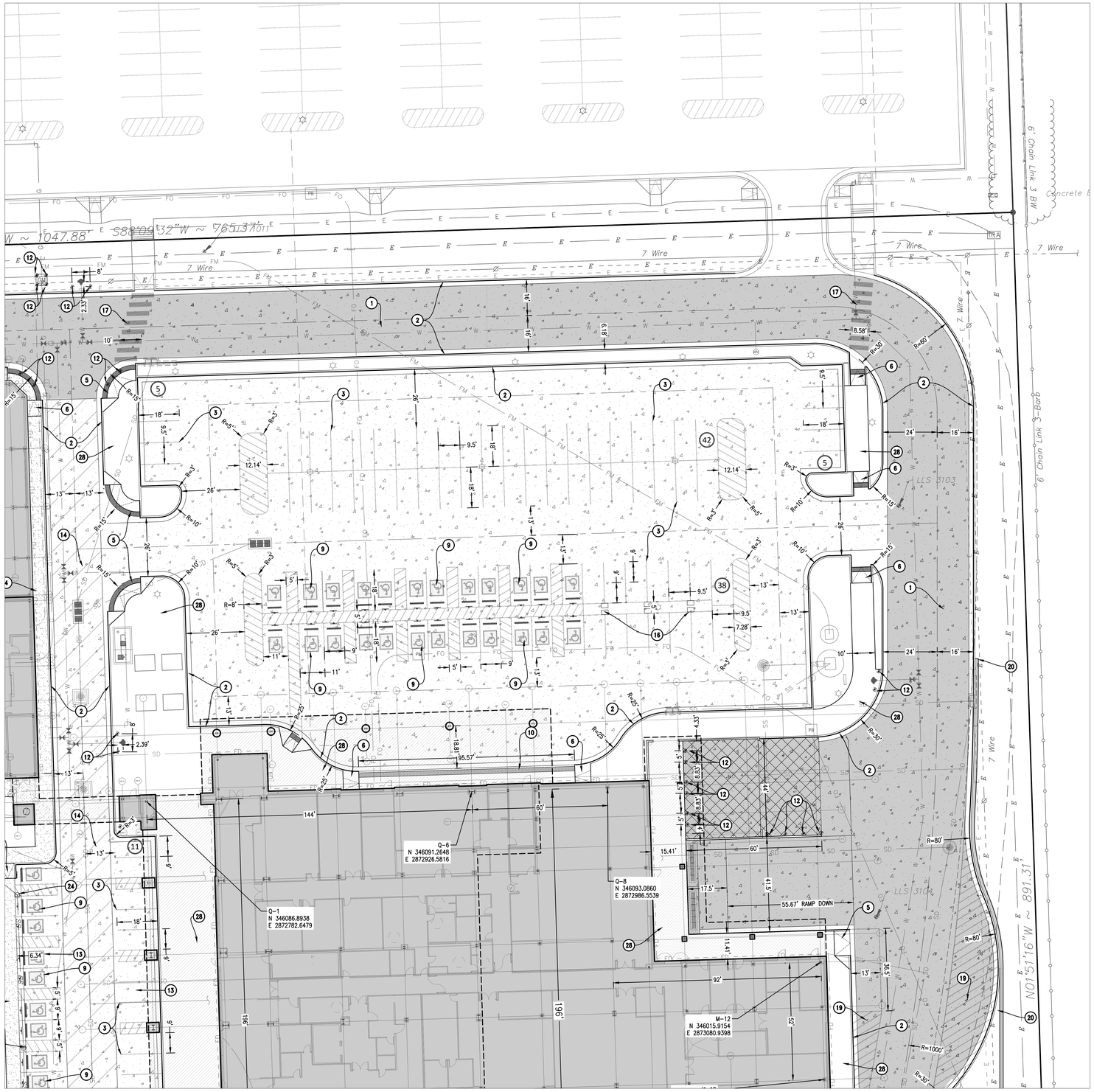
TAHLEQUAH, OKLAHOMA

KEY PLAN:
A1 A2
B1 B2
C1 C2

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

JOB NUMBER: 21-08.21
DATE: 12-02-2022
SHEET NUMBER: **C3-102**
SHEET TITLE: A1 SITE PLAN



KEYNOTES

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2	INSTALL 6-IN TALL BARRIER CURB WITH 24-IN GUTTER. SEE DETAIL 1 SHEET C3-501.
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12	INSTALL BOLLARDS, SEE DETAIL 7, SHEET C3-502.
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23	INSTALL RIBBON CURB. SEE DETAIL 2 SHEET C3-501.
24	ADJUST EXISTING CLEANOUTS TO BE FLUSH WITH SIDEWALK.
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29	CONSTRUCT HANDRAIL ALONG TOP OF WALL. SEE DETAIL 8 SHEET C3-504.
30	INSTALL DUMPSTER AREA CONCRETE PAVING. SEE DETAIL 7 SHEET C3-504.

HARDSCAPE PATTERNS

- CONCRETE SIDEWALKS
- LIGHT DUTY CONCRETE PAVING
- MEDIUM DUTY CONCRETE PAVING
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TAHLEQUAH, OKLAHOMA

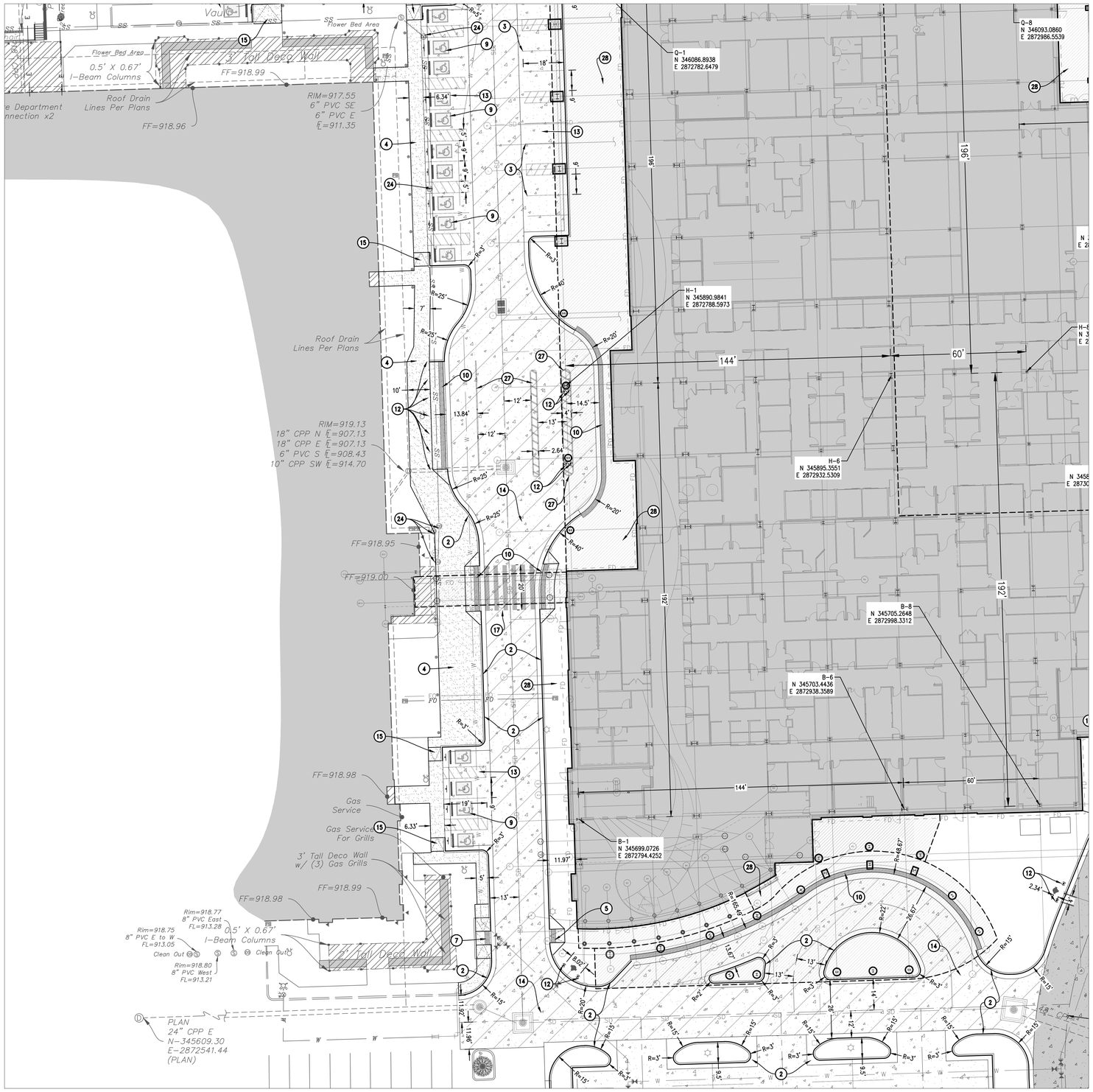
KEY PLAN:
A1 A2
B1 B2
C1 C2

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

JOB NUMBER: 21-08.21
DATE: 12-02-2022
SHEET NUMBER: **C3-103**
SHEET TITLE: A2 SITE PLAN

12/9/2022 A:\2021\5123.21\03_DSGN\01_DWG\050_CIVIL\C3-SITE-5123.DWG



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HARDSCAPE PATTERNS

- CONCRETE SIDEWALKS
- LIGHT DUTY CONCRETE PAVING
- MEDIUM DUTY CONCRETE PAVING
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TAHLEQUAH, OKLAHOMA

KEY PLAN:
A1 A2
B1 B2
C1 C2

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

JOB NUMBER: 21-08.21
DATE: 12-02-2022
SHEET NUMBER: **C3-104**
SHEET TITLE: B1 SITE PLAN

KEYNOTES

1	INSTALL HEAVY DUTY CONCRETE PAVING. SEE DETAIL 4 SHEET C3-504.
2	INSTALL 6-IN TALL BARRIER CURB WITH 24-IN GUTTER. SEE DETAIL 1 SHEET C3-501.
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- LIGHT DUTY CONCRETE PAVING
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KEY PLAN

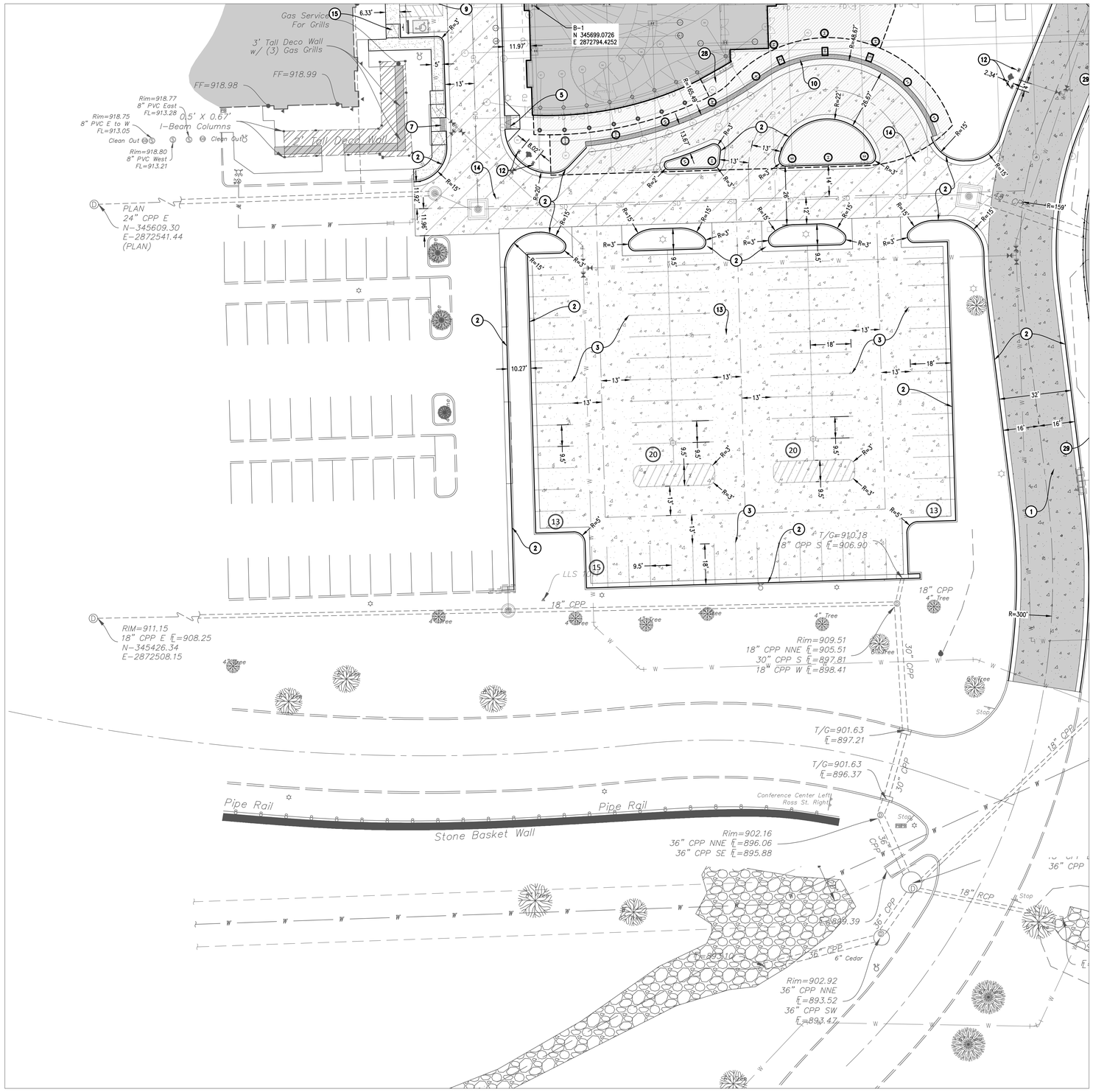
A1	A2
B1	B2
C1	C2

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

JOB NUMBER: 21-08.21
DATE: 12-02-2022
SHEET NUMBER: **C3-105**
SHEET TITLE: B2 SITE PLAN

12/9/2022 A:\2021\5123\21\03_DSGN\01_DWG\050_CIVIL\C3-SITE-5123.DWG



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6	INSTALL ADA COMPLIANT CURB TYPE B. REFER TO DETAIL 9 SHEET C3-501.
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9	CONSTRUCT ADA COMPLIANT ACCESSIBLE PARKING SPACES COMPLETE WITH WHEEL STOPS, PAVEMENT SYMBOL, AISLE STRIPING, AND SIGN.
10	INSTALL 2-FT WIDE ADA TACTILE WARNING STRIP ALONG SIDEWALK WHERE SIDEWALK AND DRIVE ELEVATION ARE EQUAL.
11	CONSTRUCT CONCRETE FLUME. SEE DETAIL 11, SHEET C3-501.
12	INSTALL BOLLARDS, SEE DETAIL 7, SHEET C3-502.
13	INSTALL LIGHT DUTY CONCRETE. SEE DETAIL 2, C3-504.
14	INSTALL MEDIUM DUTY CONCRETE. SEE DETAIL 3, SHEET C3-504.
15	INSTALL ADA COMPLIANT ACCESSIBLE RAMP.
16	INSTALL ELECTRIC VEHICLE CHARGING STATIONS. REFER TO ELECTRIC DRAWINGS.
17	INSTALL 2-FT WIDE X PLAN LENGTH YELLOW CROSSWALK STRIPING WITH 2-FT SPACING BETWEEN STRIPES.
18	INSTALL CONCRETE FLUME. SEE DETAIL 11 SHEET C3-501.
19	INSTALL 4-IN WIDE WHITE NO PARKING STRIPING AT 3-FT SPACING.
20	CONSTRUCT MASONRY BLOCK RETAINING WALL. SEE DETAILS SHEET C5-208.
21	CONSTRUCT GABION BASKET RETAINING WALL. SEE DETAILS SHEET C5-501.
22	INSTALL 2-FT THICK LAYER OF 12-IN STONE RIP-RAP WITH FILTER FABRIC UNDERLAY.
23	INSTALL RIBBON CURB. SEE DETAIL 2 SHEET C3-501.
24	ADJUST EXISTING CLEANOUTS TO BE FLUSH WITH SIDEWALK.
25	REMOVE EXISTING PAVING STRIPES AND RESTRIPE SPACES WITH 4-IN WIDE WHITE STRIPES.
26	INSTALL WHEEL STOPS ALONG PARKING SPACES. SEE DETAIL 6 SHEET C3-504.
27	INSTALL 8-IN WIDE WHITE LANE DIVIDER STRIPING.
28	SEE LANDSCAPE DRAWINGS FOR PEDESTRIAN HARDSCAPE REQUIREMENTS.
29	CONSTRUCT HANDRAIL ALONG TOP OF WALL. SEE DETAIL 8 SHEET C3-504.
30	INSTALL DUMPSTER AREA CONCRETE PAVING. SEE DETAIL 7 SHEET C3-504.

HARDSCAPE PATTERNS

- CONCRETE SIDEWALKS
- LIGHT DUTY CONCRETE PAVING
- MEDIUM DUTY CONCRETE PAVING
- HEAVY DUTY CONCRETE PAVING
- REINFORCED HEAVY DUTY CONCRETE PAVING
- SODDING / SEEDING / VEGETATIVE COVER

UTILITY WARNING:
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SCALE: 1" = 20'

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CLIENT:
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SEPT. 6, 1839

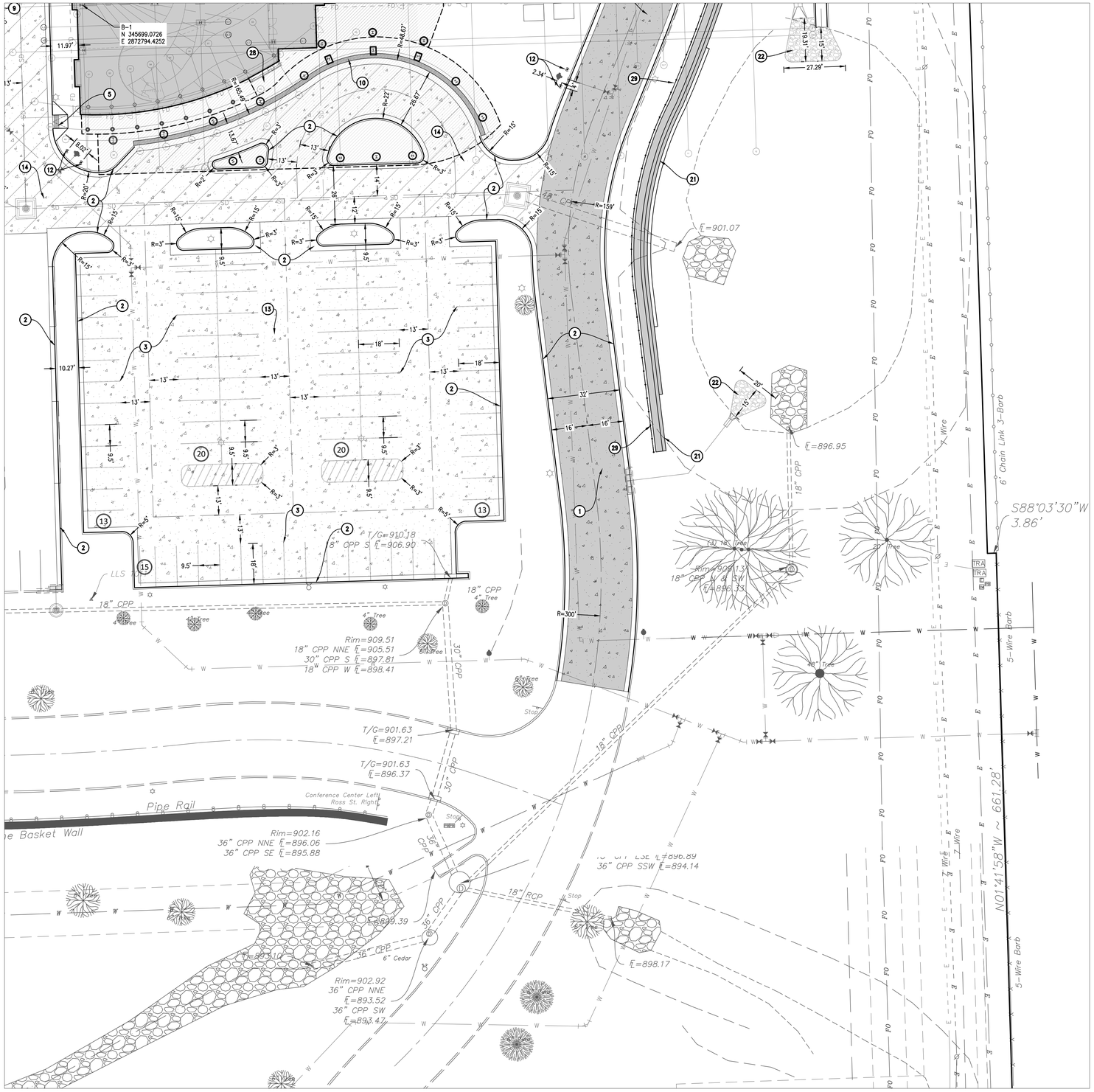
CHEROKEE NATION REPLACEMENT HOSPITAL
TAHLEQUAH, OKLAHOMA

KEY PLAN:
A1 A2
B1 B2
C1 C2

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

JOB NUMBER: 21-08.21
DATE: 12-02-2022
SHEET NUMBER: **C3-106**
SHEET TITLE: C1 SITE PLAN



KEYNOTES

1	INSTALL HEAVY DUTY CONCRETE PAVING. SEE DETAIL 4 SHEET C3-504.
2	INSTALL 6-IN TALL BARRIER CURB WITH 24-IN GUTTER. SEE DETAIL 1 SHEET C3-501.
3	INSTALL 4-IN WIDE WHITE TRAFFIC STRIPING FOR PARKING SPACES.
4	CONSTRUCT SIDEWALK. SEE DETAIL 6 SHEET C3-501.
5	INSTALL ADA COMPLIANT CURB TYPE A. REFER TO DETAIL 8 SHEET C3-501.
6	INSTALL ADA COMPLIANT CURB TYPE B. REFER TO DETAIL 9 SHEET C3-501.
7	INSTALL ADA COMPLIANT COMBINATION CURB TYPE D. REFER TO DETAIL 5 SHEET C3-504.
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- LIGHT DUTY CONCRETE PAVING
- MEDIUM DUTY CONCRETE PAVING
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SEPT. 6, 1839

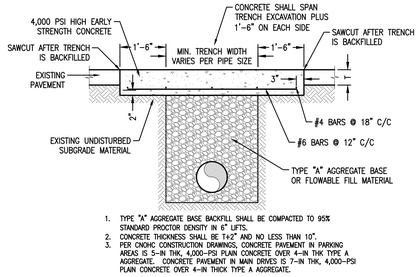
CHEROKEE NATION REPLACEMENT HOSPITAL
TAHLEQUAH, OKLAHOMA

KEY PLAN:
A1 A2
B1 B2
C1 C2

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

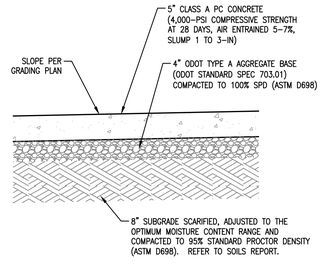
JOB NUMBER: 21-08.21
DATE: 12-02-2022
SHEET NUMBER: **C3-107**
SHEET TITLE: C2 SITE PLAN



1. TYPE "A" AGGREGATE BASE BACKFILL SHALL BE COMPACTED TO 95% STANDARD PROCTOR DENSITY IN 6" LIFTS.
2. CONCRETE THICKNESS SHALL BE T+2" AND NO LESS THAN 10".
3. PER CNOHC CONSTRUCTION DRAWINGS, CONCRETE PAVEMENT IN PARKING AREAS IS 5-IN THK, 4,000-PSI PLAIN CONCRETE OVER 4-IN THK TYPE A AGGREGATE. CONCRETE PAVEMENT IN MAIN DRIVES IS 7-IN THK, 4,000-PSI PLAIN CONCRETE OVER 4-IN THICK TYPE A AGGREGATE.

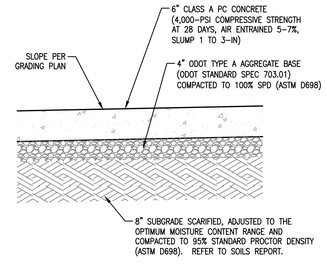
1 PAVEMENT REPAIR FOR UTILITY TRENCH

Scale: NTS



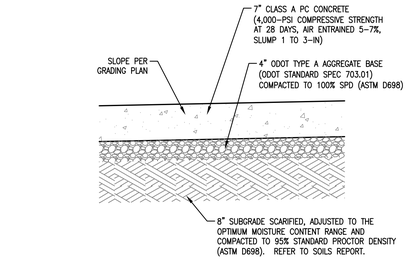
2 LIGHT DUTY PC CONCRETE PAVING(2)

Scale: NTS



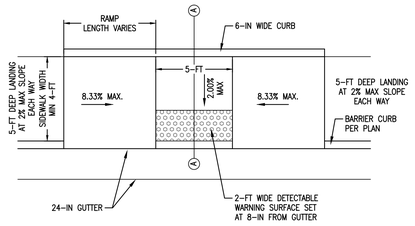
3 MEDIUM DUTY PC CONCRETE PAVING

Scale: NTS



4 HEAVY DUTY PC CONCRETE PAVING

Scale: NTS

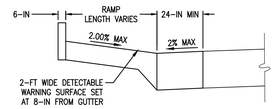


5 CURB RAMP (TYPE D)

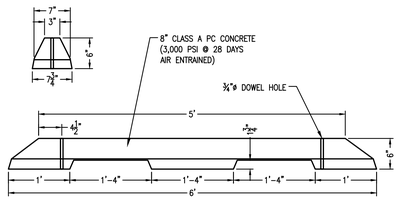
Scale: NTS

DETECTABLE WARNING SURFACE SPECIFICATIONS

1. DOME SIZE: BOTTOM DIAMETER SHALL BE 0.9-IN TO 1.4-IN. TOP DIAMETER SHALL BE 50% TO 65% OF THE BASE DIAMETER. DOME HEIGHT SHALL BE 0.2-IN.
2. DOME SPACING: CENTER-TO-CENTER SPACING SHALL BE 1.6-IN TO 2.4-IN. BASE-TO-BASE SPACING OF 0.65-IN MINIMUM. DISTANCE SHALL BE MEASURED BETWEEN THE MOST ADJACENT DOMES ON A SQUARE GRID.
3. DETECTABLE WARNING SURFACE SHALL CONTRAST VISUALLY WITH ADJACENT SURFACES EITHER LIGHT-ON-DARK OR DARK-ON-LIGHT.
4. DETECTABLE WARNING SURFACE SHALL BE 24-IN WIDE AND EXTEND THE FULL LENGTH OF THE RAMP.



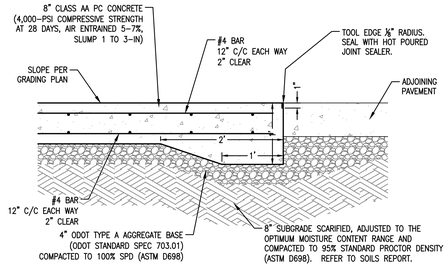
SECTION A-A



24" - #4 BAR SHALL BE USED TO ANCHOR WHEEL STOP TO PAVEMENT. WHEEL STOP SHALL BE INSTALLED 2 ft FROM CENTERLINE OF WHEEL STOP TO FACE OF CURB.

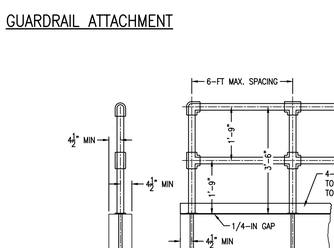
6 PRECAST CONCRETE WHEEL STOP

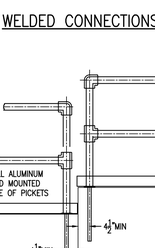
Scale: NTS



7 DUMPSTER AREA CONCRETE PAVING

Scale: NTS





GENERAL NOTES:

1. INSTALLED HANDRAILS SHALL BE CAPABLE OF WITHSTANDING A 200 LB CONCENTRATED LOAD APPLIED IN ANY DIRECTION AND AT ANY POINT ON THE TOP RAIL.
2. TOEBOARDS SHALL BE INSTALLED TO DOCK HANDRAILS, AREAS WHERE WORKERS MAY BE PRESENT BELOW THE ELEVATED SURFACE, AND OTHER OSHA REQUIRED AREAS.
3. TOEBOARDS SHALL CONFORM TO OSHA STANDARDS. TOEBOARDS SHALL BE A MINIMUM 4-IN HIGH AND ATTACHED TO POSTS WITH CLAMPS THAT ALLOW FOR EXPANSION AND CONTRACTION BETWEEN POSTS. TOEBOARDS SHALL BE SET 1/4-IN ABOVE THE WALKING SURFACE.
4. POSTS MAY BE MOUNTED TO SURFACE USING MOUNTING BASE PLATE FOR THE PICKETS. THE INSTALLED MOUNTING BASE PLATE SHALL BE CAPABLE OF PROVIDING THE OSHA POINT FORCE LOAD REQUIREMENTS.
5. WELDED CONNECTIONS MAY BE USED FOR PIPE GUARDRAILS. WELDED CONNECTIONS SHALL BE THOROUGHLY CLEANED OF ALL LOOSE SCALE, GROUND SMOOTH AND SPOT PAINTED WITH TWO COATS OF ALUMINUM PAINT.
6. SHOP DETAILS FOR ALL PIPE HANDRAILS SHALL BE SUBMITTED FOR APPROVAL.

GUARDRAIL ATTACHMENT

WELDED CONNECTIONS

ELEVATION VIEWS

8 OSHA HANDRAILS

Scale: NTS

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Oklahoma CA #4935, Expires 6/30/2023

CLIENT:

CHEROKEE NATION

CHEROKEE NATION REPLACEMENT HOSPITAL

TAHLEQUAH, OKLAHOMA

KEY PLAN:

PROJECT PHASE:

BID PACKAGE 04

(STRUCTURAL CONCRETE / EARTHWORK)

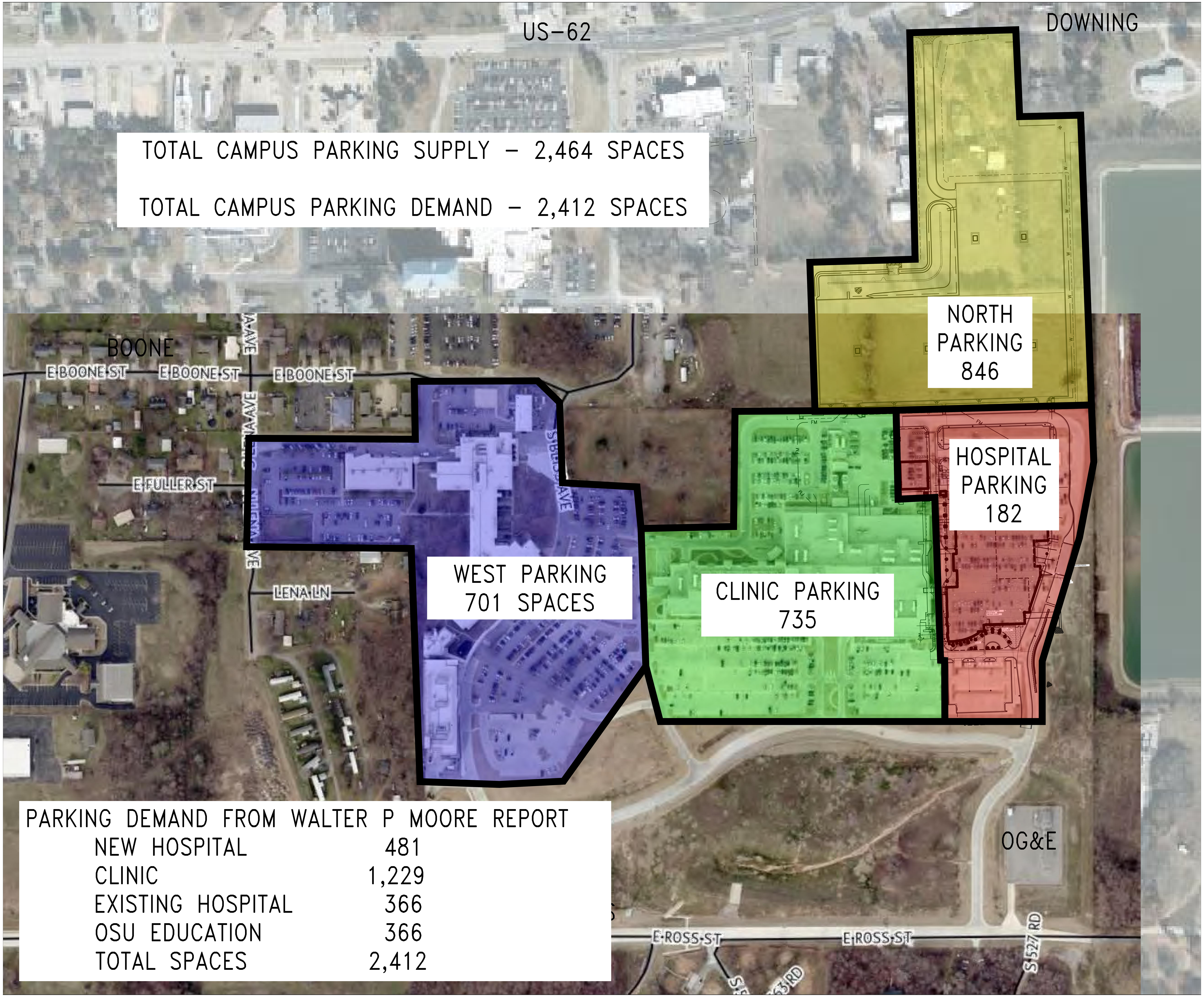
REVISIONS

JOB NUMBER: 21-08.21

DATE: 12-09-2022

SHEET NUMBER: **C3-504**

SHEET TITLE: SITE DETAILS



TOTAL CAMPUS PARKING SUPPLY – 2,464 SPACES

TOTAL CAMPUS PARKING DEMAND – 2,412 SPACES

PARKING DEMAND FROM WALTER P MOORE REPORT

NEW HOSPITAL	481
CLINIC	1,229
EXISTING HOSPITAL	366
OSU EDUCATION	366
TOTAL SPACES	2,412

12/9/2022 A:\2021\5123\21\03_DSGN\01_DWG\050_CIVIL\C0-MAP-5123.DWG

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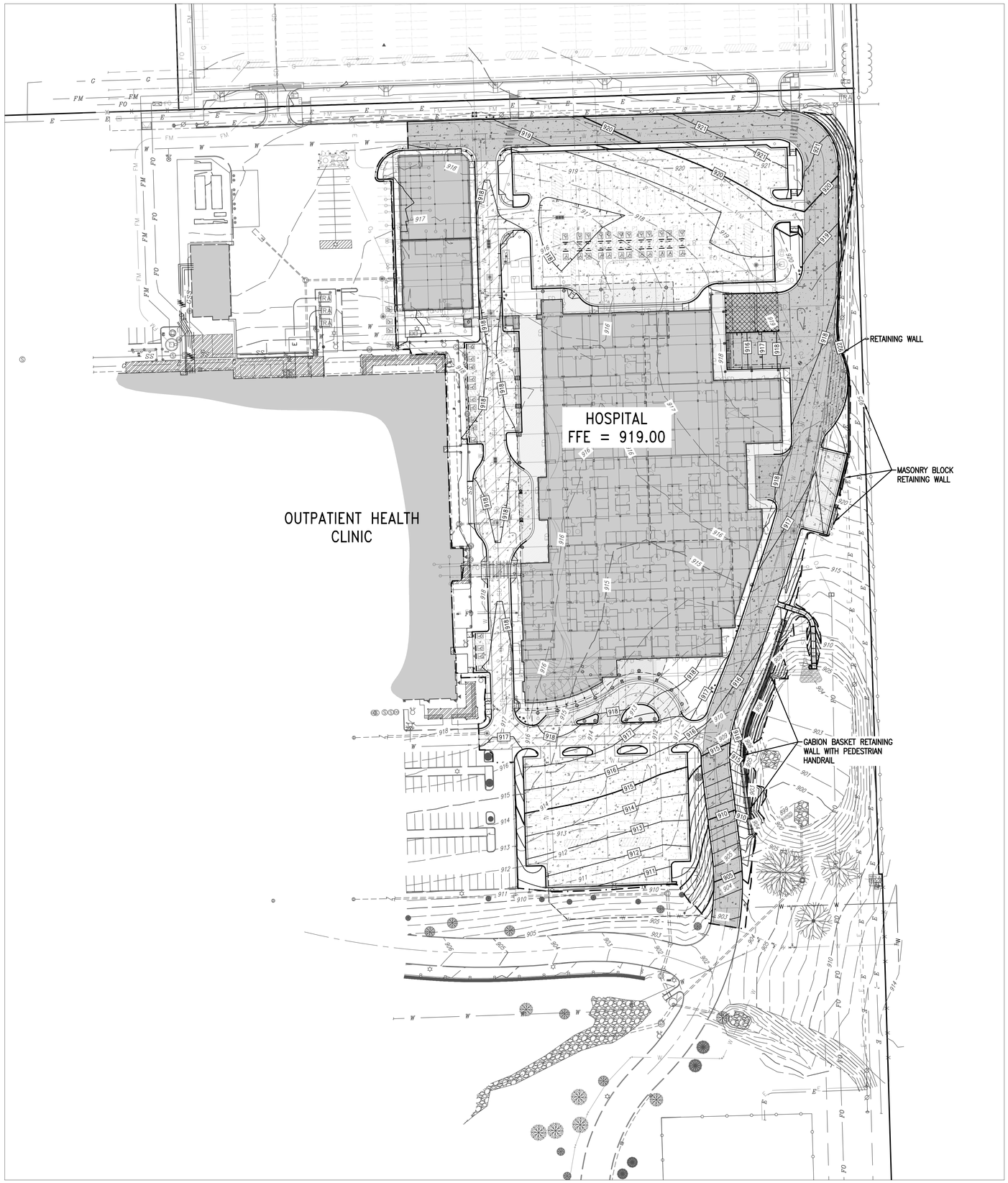
PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

JOB NUMBER: 21-08.21
DATE: 12-09-2022
SHEET NUMBER: C3-901
SHEET TITLE: PARKING SPACE ACCOUNTING



HARDSCAPE PATTERNS

- CONCRETE SIDEWALKS
- LIGHT DUTY CONCRETE PAVING
- MEDIUM DUTY CONCRETE PAVING
- HEAVY DUTY CONCRETE PAVING
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0' 50' 100' 200'

SCALE: 1" = 50'

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CHEROKEE NATION REPLACEMENT HOSPITAL

TAHLEQUAH, OKLAHOMA

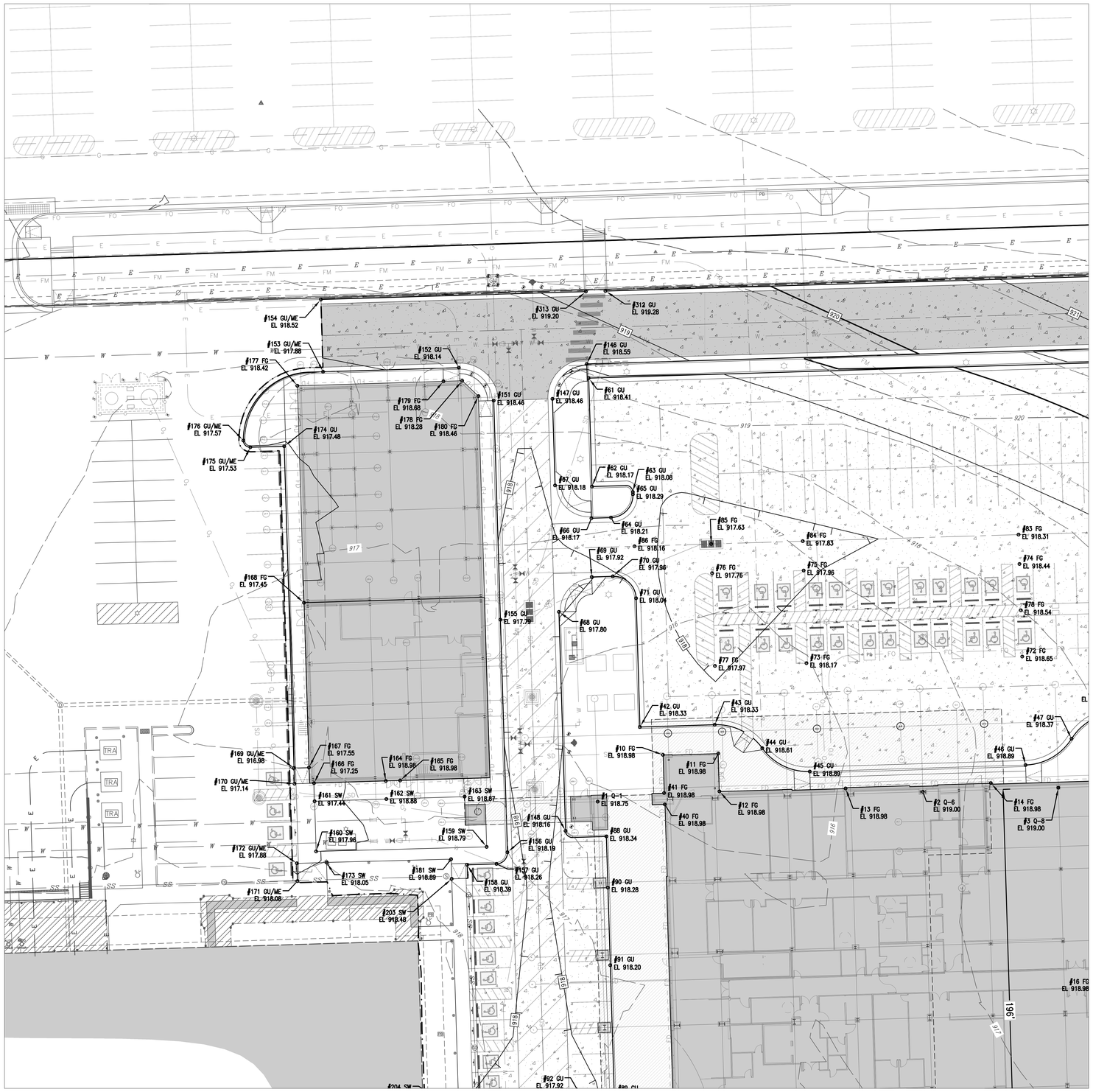
KEY PLAN:
A1 A2
B1 B2
C1 C2

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

JOB NUMBER: 21-08.21
DATE: 12-09-2022
SHEET NUMBER: **C5-101**
SHEET TITLE: OVERALL GRADING VIEW

12/9/2022 A:\2021\5123\21\03_DSGN\01_DWG\050_CIVIL\C5-GRAD-5123.DWG



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- MEDIUM DUTY CONCRETE PAVING
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SCALE: 1" = 20'

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A1

COORDINATES ON THE BUILDING ARE SHOWN FOR GENERAL LOCATION ONLY. CONTRACTOR SHALL REFER TO THE ARCHITECTURAL AND STRUCTURAL PLANS FOR STRUCTURAL FOUNDATION LAYOUT. REFER TO ARCHITECTURAL PLANS AND SPECIFICATIONS FOR CONSTRUCTION INFORMATION AND DETAILS FOR BUILDING ERECTION.

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Oklahoma CA #4935, Expires 6/30/2023

CLIENT:
SEAL OF THE CHEROKEE NATION
SEPT. 6, 1839

CHEROKEE NATION
REPLACEMENT HOSPITAL
TAHLEQUAH, OKLAHOMA

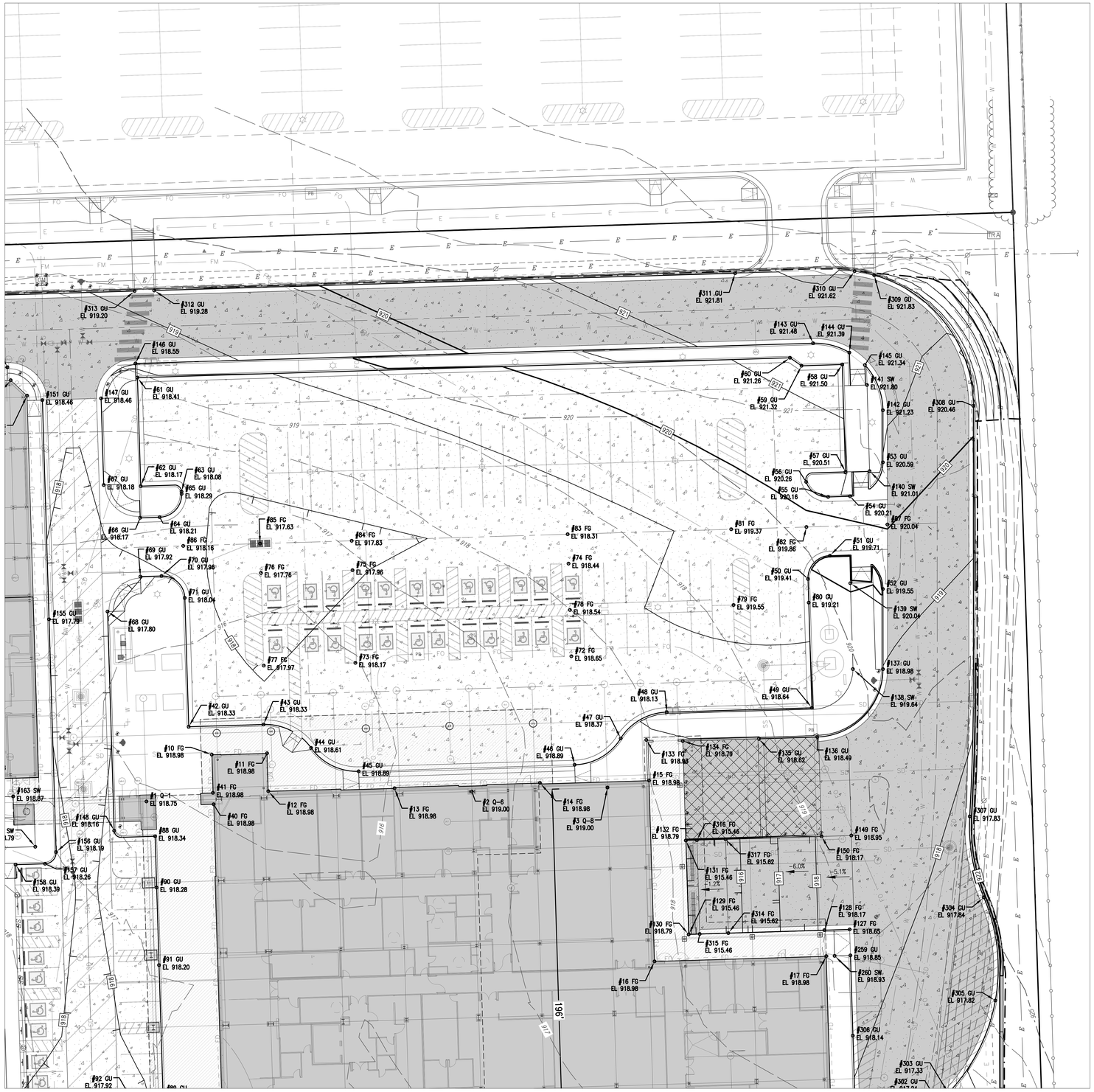
KEY PLAN:
A1 | A2
B1 | B2
C1 | C2

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

JOB NUMBER: 21-08.21
DATE: 12-09-2022
SHEET NUMBER: **C5-102**
SHEET TITLE: A1 GRADING PLAN

12/9/2022 A:\2021\5123.21\03_DSGN\01_DWG\050_CIVIL\C5-GRAD-5123.DWG



HARDSCAPE PATTERNS

- CONCRETE SIDEWALKS
- LIGHT DUTY CONCRETE PAVING
- MEDIUM DUTY CONCRETE PAVING
- HEAVY DUTY CONCRETE PAVING
- REINFORCED HEAVY DUTY CONCRETE PAVING
- SODDING / SEEDING / VEGETATIVE COVER

UTILITY WARNING:
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0' 20' 40' 80'
SCALE: 1" = 20'

BEFORE YOU DIG
CALL OKIE
(800) 522-6543

A2

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James R. Childers Architect, Inc.
45 South 4th Street
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479-783-2480
www.childersarchitect.com

PROFESSIONAL SEAL:
REGISTERED PROFESSIONAL ENGINEER
COREY W. LIPPS
21291
ARKANSAS
12/9/22

CONSULTANT LOGO:
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SEPT. 6, 1839

CHEROKEE NATION REPLACEMENT HOSPITAL
TAHLEQUAH, OKLAHOMA

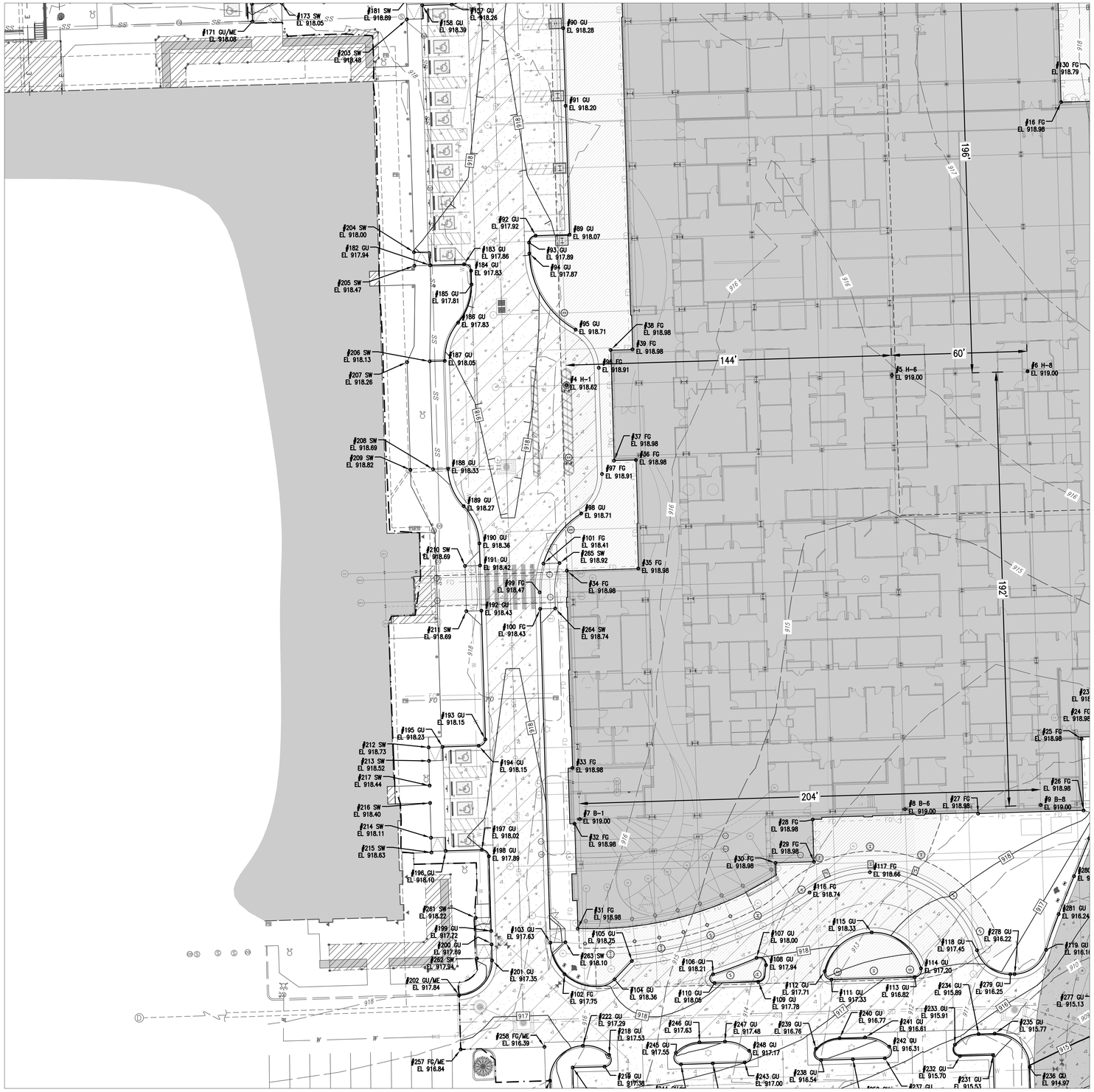
KEY PLAN:
A1 | A2
B1 | B2
C1 | C2

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

JOB NUMBER: 21-08.21
DATE: 12-09-2022
SHEET NUMBER: **C5-103**
SHEET TITLE: A2 GRADING PLAN

12/9/2022 A:\2021\5123.21\03_DSGN\01_DWG\050_CIVIL\C5-GRAD-5123.DWG



HARDSCAPE PATTERNS

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N

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B1

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12/9/2022 A:\2021\5123\21\03_DSGN\01_DWG\050_CIVIL\C5-GRAD-5123.DWG

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SEPT. 6, 1839

CHEROKEE NATION REPLACEMENT HOSPITAL

TAHLEQUAH, OKLAHOMA

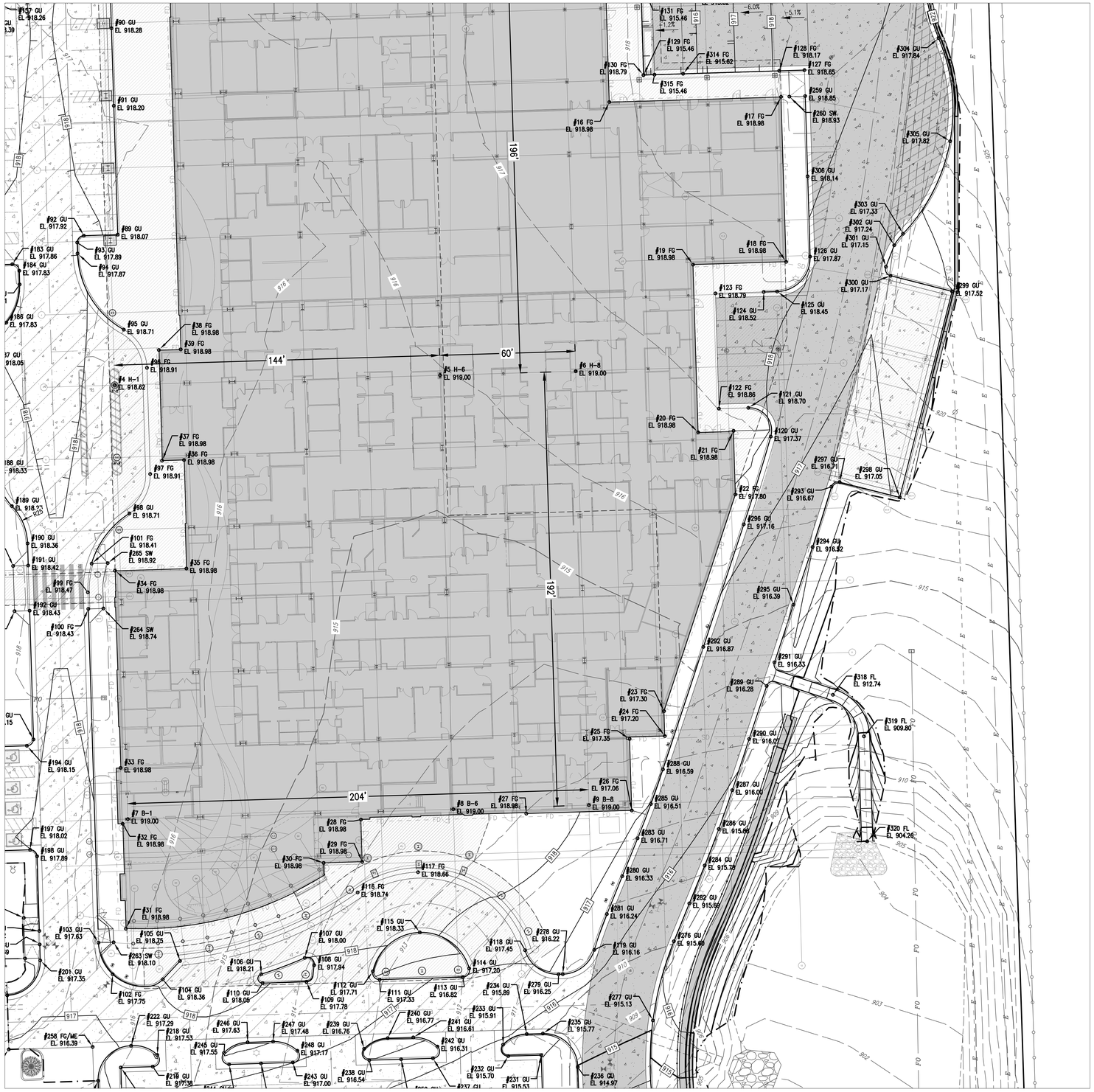
KEY PLAN:

A1	A2
B1	B2
C1	C2

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS:

JOB NUMBER: 21-08.21
DATE: 12-09-2022
SHEET NUMBER: **C5-104**
SHEET TITLE: B1 GRADING PLAN



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SEPT. 6, 1839

CHEROKEE NATION REPLACEMENT HOSPITAL
TAHLEQUAH, OKLAHOMA

KEY PLAN:

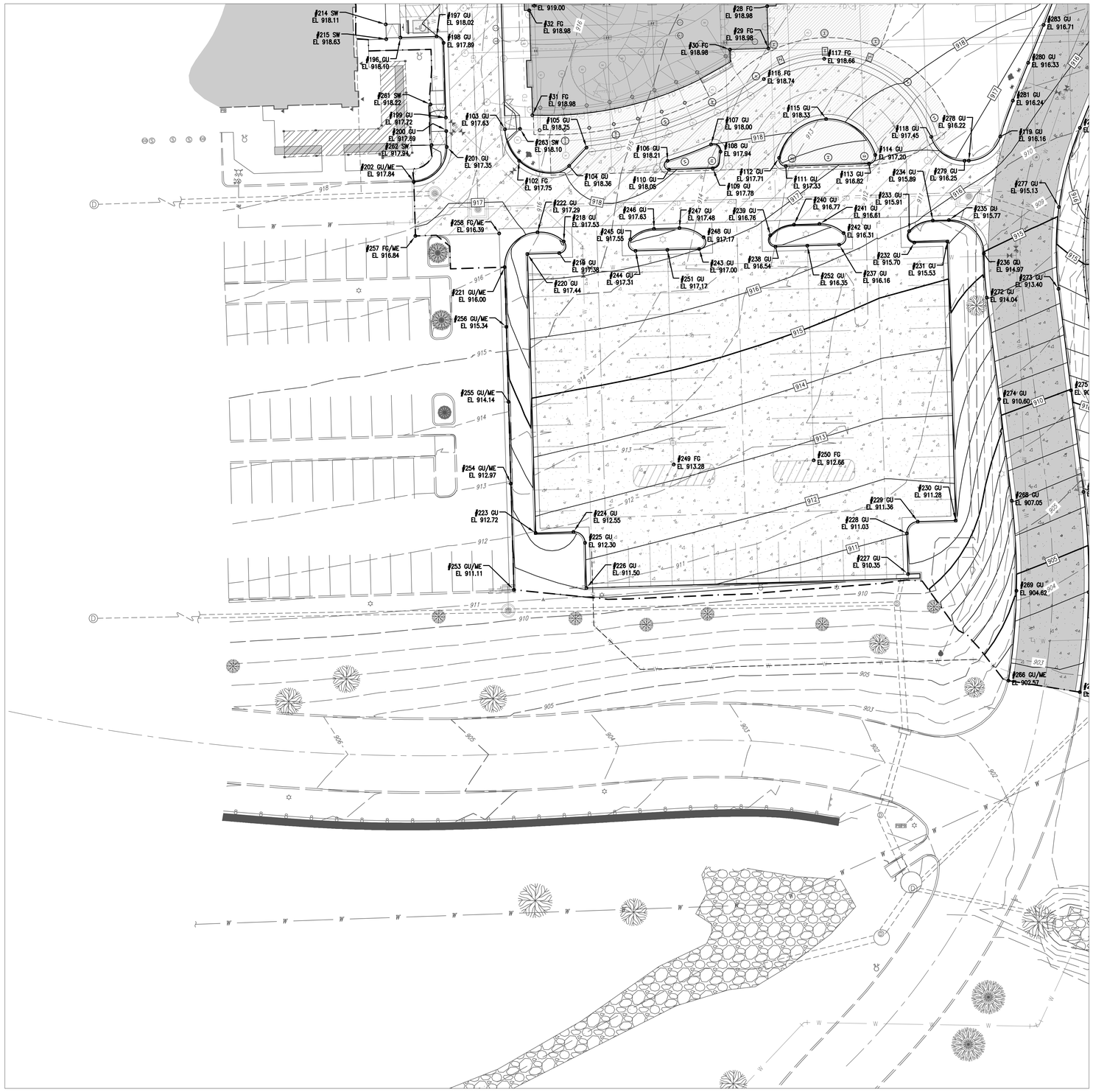
A1	A2
B1	B2
C1	C2

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

JOB NUMBER: 21-08.21
DATE: 12-09-2022
SHEET NUMBER: **C5-105**
SHEET TITLE: B2 GRADING PLAN

12/9/2022 A:\2021\5123\21\03_DSGN\01_DWG\050_CIVIL\C5-GRAD-5123.DWG



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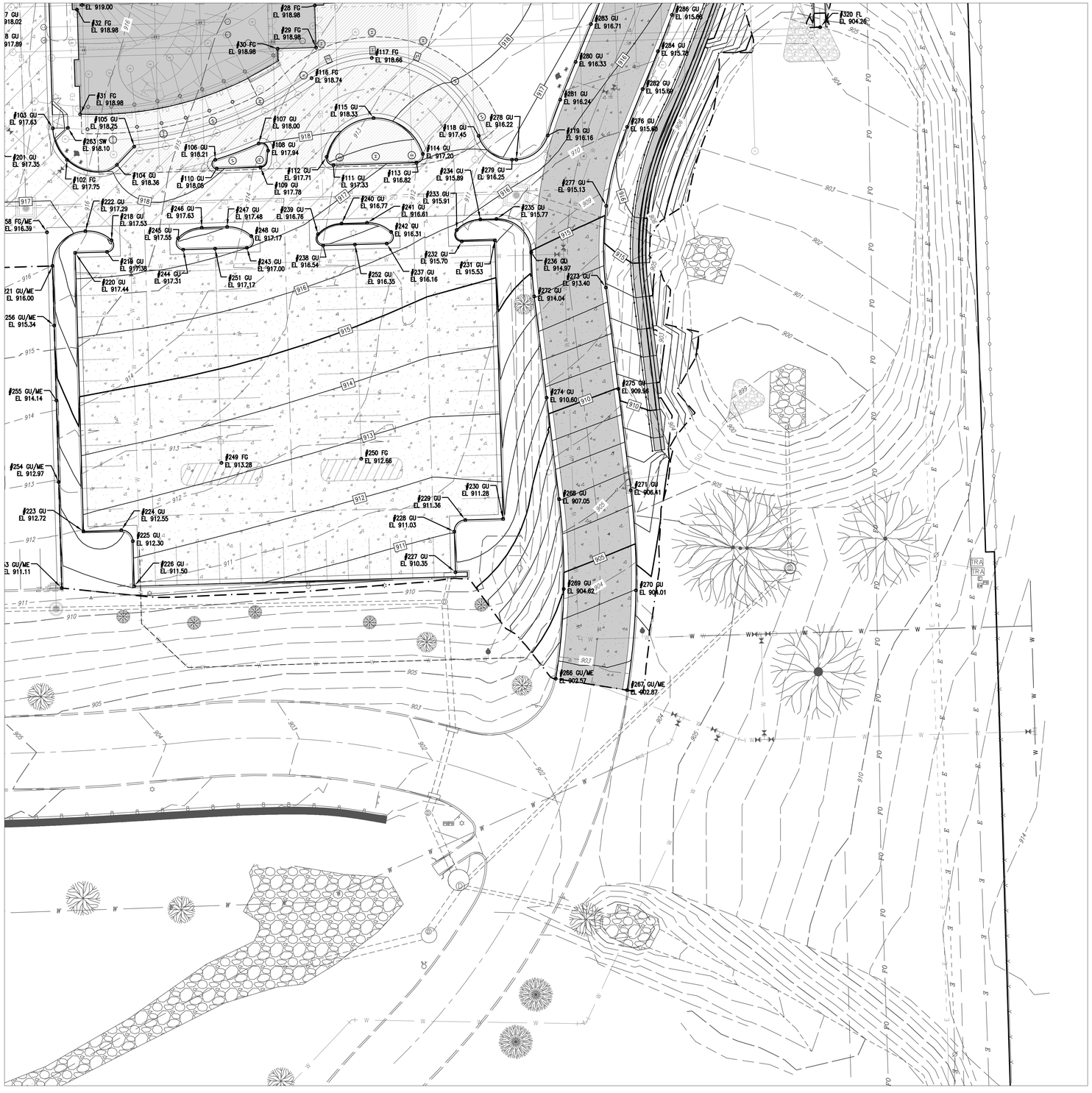
TAHLEQUAH, OKLAHOMA

KEY PLAN:
A1 A2
B1 B2
C1 C2

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

JOB NUMBER: 21-08.21
DATE: 12-09-2022
SHEET NUMBER: **C5-106**
SHEET TITLE: C1 GRADING PLAN



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TAHLEQUAH, OKLAHOMA

KEY PLAN:
A1 A2
B1 B2
C1 C2

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

JOB NUMBER: 21-08.21
DATE: 12-09-2022
SHEET NUMBER: **C5-107**
SHEET TITLE: C2 GRADING PLAN

Point Table				
Point #	Elevation	Northing	Easting	Description
1	918.75	346086.89	2872782.65	Q-1
2	919.00	346091.26	2872926.58	Q-6
3	919.00	346093.09	2872986.55	Q-8
4	918.62	345890.98	2872788.60	H-1
5	919.00	345895.36	2872932.53	H-6
6	919.00	345897.18	2872992.50	H-8
7	919.00	345699.07	2872794.43	B-1
8	919.00	345703.44	2872938.36	B-6
9	919.00	345705.26	2872998.33	B-8
10	918.98	346107.61	2872811.53	FG
11	918.98	346108.19	2872836.02	FG
12	918.98	346091.45	2872836.53	FG
13	918.98	346092.73	2872892.35	FG
14	918.98	346095.10	2872956.69	FG
15	918.98	346096.15	2873004.97	FG
16	918.98	346016.18	2873007.40	FG
17	918.98	346018.49	2873083.36	FG
18	918.98	345945.52	2873085.58	FG
19	918.98	345944.27	2873044.44	FG
20	918.98	345870.12	2873046.69	FG
21	918.98	345870.78	2873062.34	FG
22	917.80	345842.64	2873063.17	FG
23	917.30	345746.84	2873031.58	FG
24	917.20	345735.80	2873031.92	FG
25	917.35	345734.54	2873016.37	FG
26	917.06	345702.92	2873017.33	FG
27	918.98	345701.51	2872970.66	FG
28	918.98	345698.87	2872897.54	FG
29	918.98	345680.21	2872898.11	FG
30	918.98	345679.69	2872881.05	FG
31	918.98	345650.66	2872793.56	FG
32	918.98	345697.00	2872792.15	FG
33	918.98	345721.74	2872791.24	FG
34	918.98	345808.95	2872788.75	FG
35	918.98	345809.91	2872820.32	FG
36	918.98	345857.86	2872819.28	FG
37	918.98	345857.57	2872809.58	FG
38	918.98	345906.51	2872808.13	FG
39	918.98	345906.80	2872817.80	FG
40	918.98	346085.63	2872812.37	FG

Point Table				
Point #	Elevation	Northing	Easting	Description
41	918.98	346090.70	2872812.05	FG
42	918.33	346119.94	2872801.35	GU
43	918.33	346120.94	2872834.45	GU
44	918.61	346110.57	2872855.48	GU
45	918.89	346100.21	2872876.52	GU
46	918.89	346103.11	2872972.05	GU
47	918.37	346114.73	2872992.42	GU
48	918.13	346126.36	2873012.79	GU
49	918.64	346128.31	2873077.00	GU
50	919.41	346185.28	2873075.27	GU
51	919.71	346195.58	2873084.96	GU
52	919.55	346180.83	2873108.40	GU
53	920.59	346236.86	2873108.39	GU
54	920.21	346221.86	2873093.85	GU
55	920.16	346221.57	2873084.17	GU
56	920.26	346228.18	2873074.46	GU
57	920.51	346232.64	2873091.84	GU
58	921.50	346280.12	2873090.40	GU
59	921.32	346279.57	2873072.41	GU
60	921.26	346283.08	2873067.30	GU
61	918.41	346274.32	2872778.65	GU
62	918.17	346226.34	2872780.11	GU
63	918.08	346223.89	2872798.19	GU
64	918.21	346212.59	2872798.53	GU
65	918.29	346222.89	2872798.22	GU
66	918.17	346212.33	2872779.86	GU
67	918.18	346226.84	2872763.75	GU
68	917.80	346170.87	2872765.45	GU
69	917.92	346186.32	2872779.99	GU
70	917.96	346186.60	2872789.32	GU
71	918.04	346176.91	2872799.62	GU
72	918.65	346151.09	2872970.59	FG
73	918.17	346148.19	2872875.06	FG
74	918.44	346192.07	2872969.35	FG
75	917.96	346189.17	2872873.82	FG
76	917.76	346187.94	2872833.30	FG
77	917.97	346146.96	2872834.54	FG
78	918.54	346171.57	2872969.97	FG
79	919.55	346173.78	2873042.32	FG
80	919.21	346174.79	2873075.59	GU

Point Table				
Point #	Elevation	Northing	Easting	Description
81	919.37	346207.26	2873041.31	FG
82	919.86	346208.27	2873074.57	FG
83	918.31	346205.06	2872968.95	FG
84	917.83	346202.16	2872873.42	FG
85	917.63	346200.93	2872832.90	FG
86	918.16	346199.90	2872798.92	FG
87	920.04	346209.36	2873110.40	FG
88	918.34	346071.50	2872786.48	GU
89	918.07	345957.56	2872789.94	GU
90	918.28	346048.81	2872787.17	GU
91	918.20	346014.53	2872788.21	GU
92	917.92	345957.10	2872774.95	GU
93	917.89	345954.01	2872772.04	GU
94	917.87	345949.22	2872772.18	GU
95	918.71	345915.52	2872792.64	GU
96	918.91	345898.67	2872802.87	FG
97	918.91	345851.69	2872804.30	FG
98	918.71	345834.25	2872795.11	GU
99	918.47	345799.38	2872776.73	FG
100	918.43	345792.02	2872776.96	FG
101	918.41	345812.07	2872778.40	FG
102	917.75	345630.47	2872787.72	FG
103	917.63	345644.42	2872781.44	GU
104	918.36	345628.20	2872809.86	GU
105	918.75	345636.33	2872817.52	GU
106	918.21	345630.49	2872853.40	GU
107	918.00	345637.72	2872872.67	GU
108	917.94	345634.50	2872876.85	GU
109	917.78	345627.16	2872873.44	GU
110	918.05	345626.59	2872854.05	GU
111	917.33	345628.12	2872905.80	GU
112	917.71	345631.77	2872902.78	GU
113	916.82	345629.22	2872942.61	GU
114	917.20	345633.04	2872945.40	GU
115	918.33	345648.97	2872923.59	GU
116	918.74	345666.66	2872896.08	FG
117	918.66	345675.63	2872922.78	FG
118	917.45	345641.06	2872970.14	GU
119	916.16	345641.24	2873000.73	GU
120	917.37	345868.24	2873078.78	GU

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Point #	Elevation	Northing	Easting	Description
121	918.70	345880.98	2873068.86	GU
122	918.86	345880.59	2873055.87	FG
123	918.79	345931.57	2873054.33	FG
124	918.52	345932.20	2873075.66	GU
125	918.45	345932.38	2873081.63	GU
126	917.87	345947.83	2873096.18	GU
127	918.65	346030.31	2873093.68	FG
128	918.17	346029.97	2873082.51	FG
129	915.46	346028.15	2873022.54	FG
130	918.79	346028.05	2873022.45	FG
131	915.46	346069.63	2873021.28	FG
132	918.79	346069.63	2873021.19	FG
133	918.93	346114.12	2873003.76	FG
134	918.79	346113.61	2873019.85	FG
135	918.62	346114.63	2873053.60	GU
136	918.49	346115.41	2873079.32	GU
137	918.98	346145.40	2873108.41	GU
138	919.64	346145.44	2873095.14	SW
139	920.04	346183.41	2873094.00	SW
140	921.01	346232.96	2873102.50	SW
141	921.80	346271.05	2873101.34	SW
142	921.23	346259.89	2873108.39	GU
143	921.48	346289.51	2873077.50	GU
144	921.39	346285.40	2873093.57	GU
145	921.34	346279.90	2873100.41	GU
146	918.55	346280.61	2872777.80	GU
147	918.46	346265.14	2872762.59	GU
148	918.16	346073.96	2872768.40	GU
149	918.95	346071.85	2873094.41	FG
150	918.17	346071.45	2873081.25	FG
151	918.46	346264.35	2872736.60	GU
152	918.14	346278.93	2872721.16	GU
153	917.88	346277.14	2872661.05	GU/ME
154	918.52	346309.13	2872660.10	GU/ME
155	917.79	346167.45	2872739.54	GU
156	918.19	346064.50	2872742.67	GU
157	918.26	346059.35	2872737.83	GU
158	918.39	346058.96	2872724.83	GU
159	918.79	346066.87	2872733.48	SW
160	917.96	346064.87	2872657.97	SW

Point Table				
Point #	Elevation	Northing	Easting	Description
161	917.44	346087.09	2872657.30	SW
162	918.88	346088.05	2872689.07	SW
163	918.87	346089.11	2872723.72	SW
164	918.98	346096.05	2872688.83	FG
165	918.98	346096.24	2872695.16	FG
166	917.25	346095.09	2872657.06	FG
167	917.55	346102.02	2872654.80	FG
168	917.45	346174.89	2872652.60	FG
169	916.98	346101.82	2872648.18	GU/ME
170	917.14	346094.82	2872648.39	GU/ME
171	918.08	346051.76	2872649.70	GU/ME
172	917.88	346059.59	2872649.46	GU/ME
173	918.05	346059.99	2872662.59	SW
174	917.48	346244.17	2872643.86	GU
175	917.53	346243.72	2872628.87	GU/ME
176	917.57	346246.69	2872625.78	GU/ME
177	918.42	346270.94	2872649.67	FG
178	918.28	346273.16	2872722.63	FG
179	918.68	346272.91	2872714.34	FG
180	918.46	346266.37	2872729.84	FG
181	918.89	346061.45	2872717.75	SW
182	917.94	345944.01	2872728.32	GU
183	917.86	345944.47	2872743.32	GU
184	917.83	345941.56	2872746.40	GU
185	917.81	345935.53	2872746.59	GU
186	917.83	345918.62	2872740.68	GU
187	918.05	345901.71	2872734.78	GU
188	918.33	345854.04	2872736.22	GU
189	918.27	345837.51	2872743.14	GU
190	918.36	345820.99	2872750.07	GU
191	918.42	345811.22	2872750.36	GU
192	918.43	345791.23	2872750.97	GU
193	918.15	345734.29	2872752.70	GU
194	918.15	345731.50	2872749.78	GU
195	918.23	345731.01	2872733.79	GU
196	918.10	345685.03	2872735.19	GU
197	918.02	345685.52	2872751.18	GU
198	917.89	345682.64	2872754.27	GU
199	917.72	345649.63	2872755.27	GU
200	917.69	345643.63	2872755.45	GU

Point Table				
Point #	Elevation	Northing	Easting	Description
201	917.35	345636.60	2872755.67	GU
202	917.84	345621.15	2872741.08	GU/ME
203	918.48	346052.75	2872718.02	SW
204	918.00	345949.79	2872721.14	SW
205	918.47	345943.80	2872721.33	SW
206	918.13	345901.51	2872728.11	SW
207	918.26	345901.21	2872718.12	SW
208	918.69	345853.83	2872729.56	SW
209	918.82	345853.53	2872719.56	SW
210	918.69	345811.02	2872743.70	SW
211	918.69	345791.03	2872744.31	SW
212	918.73	345730.82	2872727.59	SW
213	918.52	345724.83	2872727.76	SW
214	918.11	345690.86	2872728.68	SW
215	918.63	345684.33	2872728.86	SW
216	918.40	345706.22	2872728.26	SW
217	918.44	345713.90	2872728.05	SW
218	917.53	345594.81	2872807.44	GU
219	917.38	345589.63	2872805.47	GU
220	917.44	345589.21	2872791.36	GU
221	916.00	345583.42	2872781.30	GU/ME
222	917.29	345598.84	2872795.85	GU
223	912.72	345465.76	2872795.02	GU
224	912.55	345466.26	2872811.77	GU
225	912.30	345461.41	2872816.91	GU
226	911.50	345441.42	2872817.51	GU
227	910.35	345447.65	2872959.88	GU
228	911.03	345465.64	2872959.35	GU
229	911.36	345470.79	2872964.20	GU
230	911.28	345471.28	2872980.94	GU
231	915.53	345594.73	2872977.27	GU
232	915.70	345594.30	2872962.67	GU
233	915.91	345599.36	2872960.40	GU
234	915.89	345604.04	2872970.85	GU
235	915.77	345604.26	2872978.00	GU
236	914.97	345589.42	2872993.44	GU
237	916.16	345593.31	2872929.41	GU
238	916.54	345592.47	2872901.20	GU
239	916.76	345597.53	2872898.93	GU
240	916.77	345602.22	2872909.38	GU

Point Table				
Point #	Elevation	Northing	Easting	Description
241	916.61	345602.55	2872920.66	GU
242	916.31	345598.49	2872931.37	GU
243	917.00	345591.47	2872867.43	GU
244	917.31	345590.63	2872839.22	GU
245	917.55	345595.69	2872836.96	GU
246	917.63	345600.38	2872847.41	GU
247	917.48	345600.71	2872858.69	GU
248	917.17	345596.65	2872869.40	GU
249	913.28	345496.09	2872856.15	FG
250	912.66	345497.93	2872918.12	FG
251	917.17	345591.05	2872853.33	GU
252	916.35	345592.89	2872915.30	GU
253	911.11	345440.63	2872785.40	GU/ME
254	912.97	345487.64	2872784.05	GU/ME
255	914.14	345523.81	2872783.01	GU/ME
256	915.34	345556.99	2872782.06	GU/ME
257	916.84	345597.24	2872741.79	FG/ME
258	916.39	345598.34	2872778.85	FG/ME
259	918.85	346018.81	2873094.02	GU
260	918.93	346018.60	2873087.03	SW
261	918.22	345655.43	2872748.42	SW
262	917.94	345637.44	2872748.97	SW
263	918.10	345644.63	2872788.10	SW
264	918.74	345792.22	2872783.62	SW
265	918.92	345812.29	2872785.50	SW
266	902.57	345400.44	2873004.31	GU/ME
267	902.87	345395.36	2873035.90	GU/ME
268	907.05	345480.06	2873005.78	GU
269	904.82	345440.20	2873007.85	GU
270	904.01	345439.61	2873039.84	GU
271	906.41	345483.96	2873037.54	GU
272	914.04	345569.92	2872994.74	GU
273	913.40	345573.82	2873026.50	GU
274	910.60	345524.99	2873000.26	GU
275	909.96	345528.89	2873032.02	GU
276	915.60	345645.05	2873035.94	GU
277	915.13	345610.04	2873026.67	GU
278	916.22	345630.53	2872986.80	GU
279	916.25	345630.47	2872984.93	GU
280	916.33	345673.85	2873013.10	GU

Point Table				
Point #	Elevation	Northing	Easting	Description
281	916.24	345657.09	2873006.29	GU
282	915.69	345661.81	2873042.75	GU
283	916.71	345690.61	2873019.90	GU
284	915.78	345678.57	2873049.55	GU
285	916.51	345705.74	2873025.76	GU
286	915.88	345694.70	2873055.79	GU
287	916.00	345711.87	2873061.78	GU
288	916.59	345721.06	2873031.10	GU
289	916.28	345757.96	2873076.97	GU
290	916.07	345734.50	2873069.23	GU
291	916.33	345768.41	2873080.42	GU
292	916.87	345775.25	2873048.98	GU
293	916.67	345846.06	2873105.63	GU
294	916.52	345819.41	2873097.24	GU
295	916.39	345793.91	2873088.83	GU
296	917.16	345829.43	2873066.86	GU
297	916.71	345848.11	2873109.25	GU
298	917.05	345841.36	2873135.81	GU
299	917.52	345932.46	2873158.96	GU
300	917.17	345939.37	2873131.78	GU
301	917.15	345943.35	2873129.72	GU
302	917.24	345952.86	2873133.38	GU
303	917.33	345957.97	2873137.30	GU
304	917.84	346044.46	2873153.03	GU
305	917.82	345998.86	2873158.11	GU
306	918.14	345983.32	2873095.10	GU
307	917.83	346080.28	2873146.78	GU
308	920.46	346261.83	2873148.39	GU
309	921.83	346319.60	2873104.57	GU
310	921.62	346321.38	2873095.84	GU
311	921.81	346320.50	2873043.02	GU
312	919.28	346312.87	2872786.07	GU
313	919.20	346312.61	2872777.41	GU
314	915.62	346028.68	2873040.03	FG
315	915.46	346028.30	2873027.37	FG
316	915.46	346069.78	2873026.11	FG
317	915.62	346070.16	2873038.77	FG
318	912.74	345754.07	2873106.30	FL
319	909.80	345735.69	2873120.02	FL
320	904.26	345689.26	2873121.43	FL

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Oklahoma CA #6935, Expires 6/30/2023

CLIENT:

CHEROKEE NATION
REPLACEMENT HOSPITAL
TAHLEQUAH, OKLAHOMA

KEY PLAN:

A1	A2
B1	B2
C1	C2

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

JOB NUMBER: 21-08.21
DATE: 12-09-2022
SHEET NUMBER: **C5-201**
SHEET TITLE: COORDINATES AND ELEVATIONS

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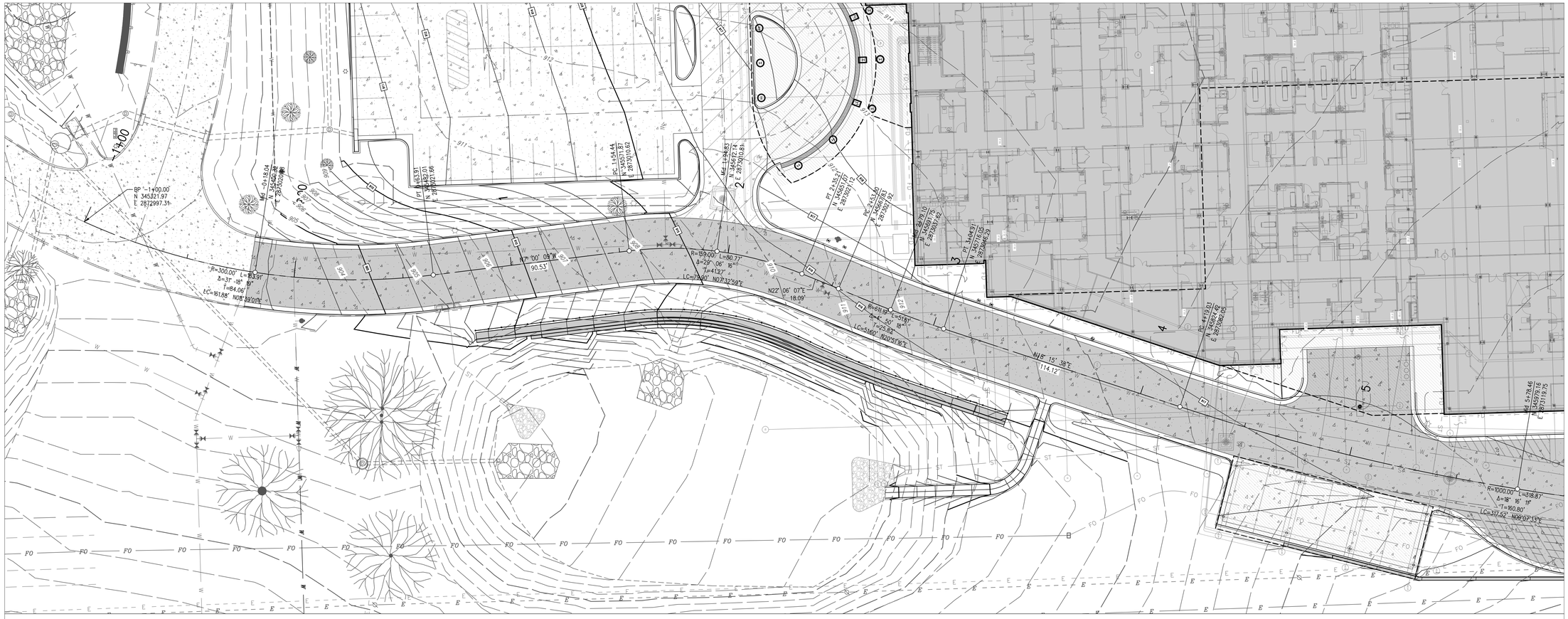

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CHEROKEE NATION REPLACEMENT HOSPITAL

TAHLEQUAH, OKLAHOMA

KEY PLAN:

A1	A2
B1	B2
C1	C2

PROJECT PHASE:

BID PACKAGE 04

(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS:

JOB NUMBER: 21-08.21

DATE: 12-09-2022

SHEET NUMBER: C5-202

SHEET TITLE: LOOP DRIVE PLAN AND PROFILE

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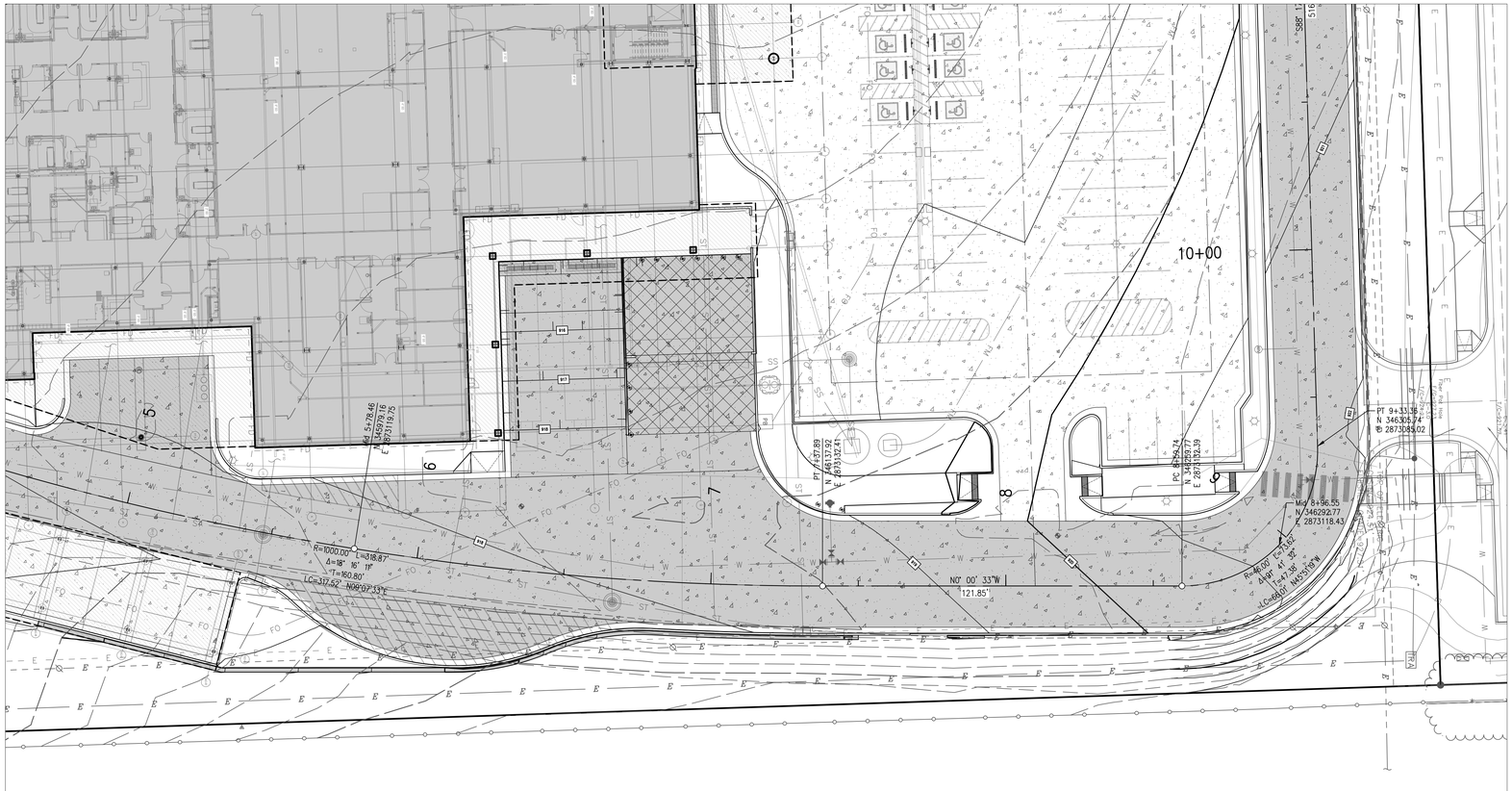

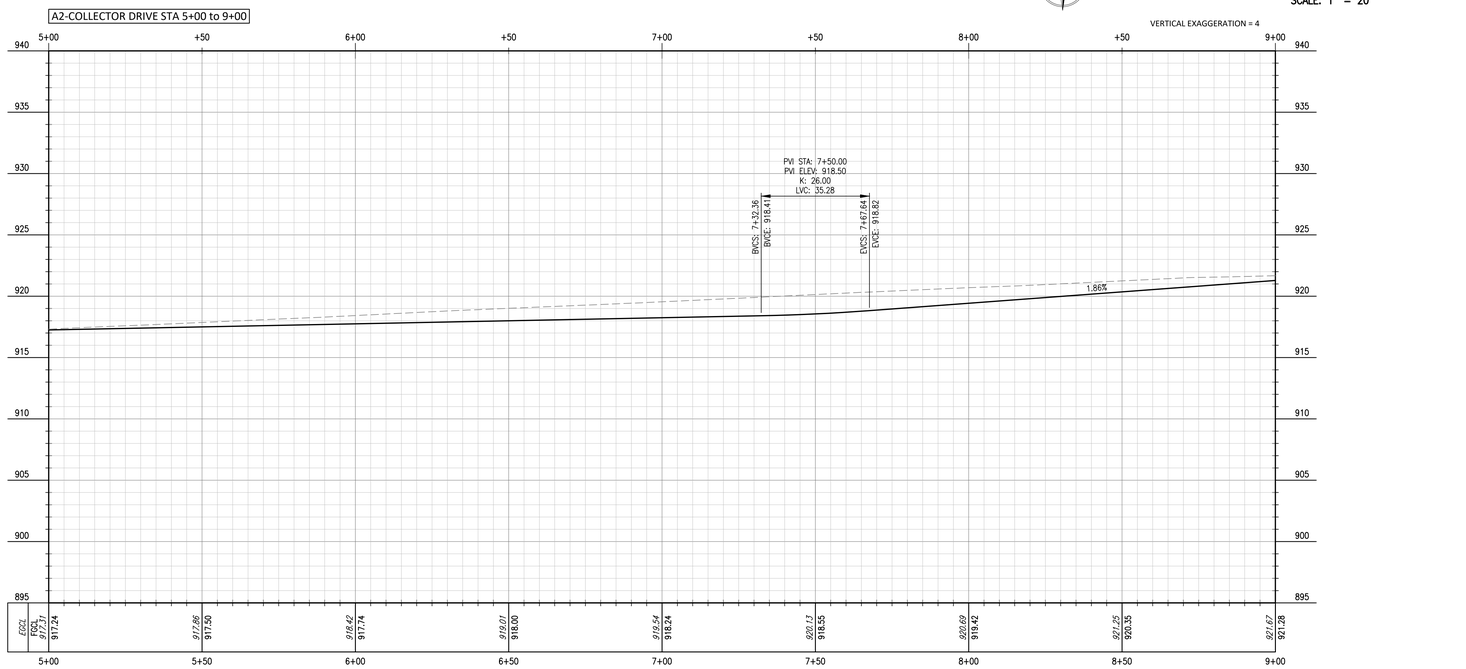

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TAHLEQUAH, OKLAHOMA

KEY PLAN:

A1	A2
B1	B2
C1	C2

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

JOB NUMBER: 21-08.21
DATE: 12-09-2022
SHEET NUMBER: C5-203
SHEET TITLE: LOOP DRIVE PLAN AND PROFILE

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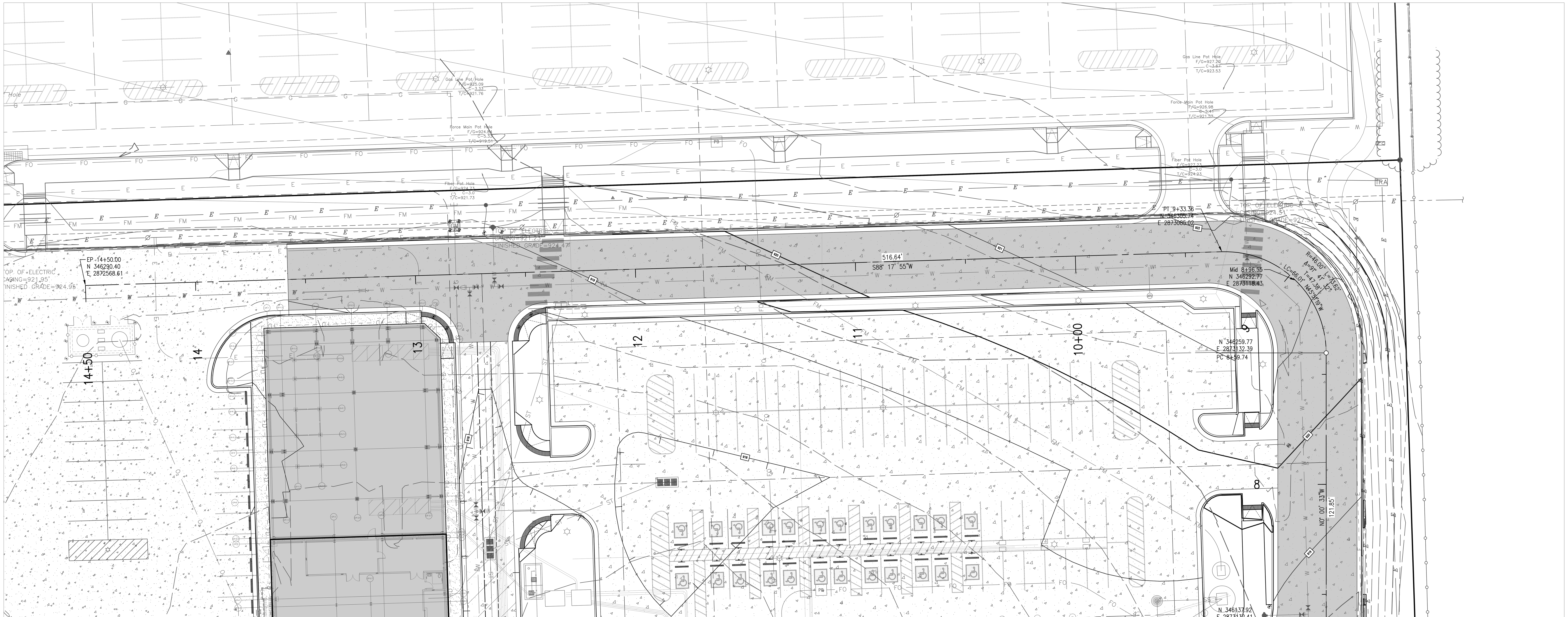
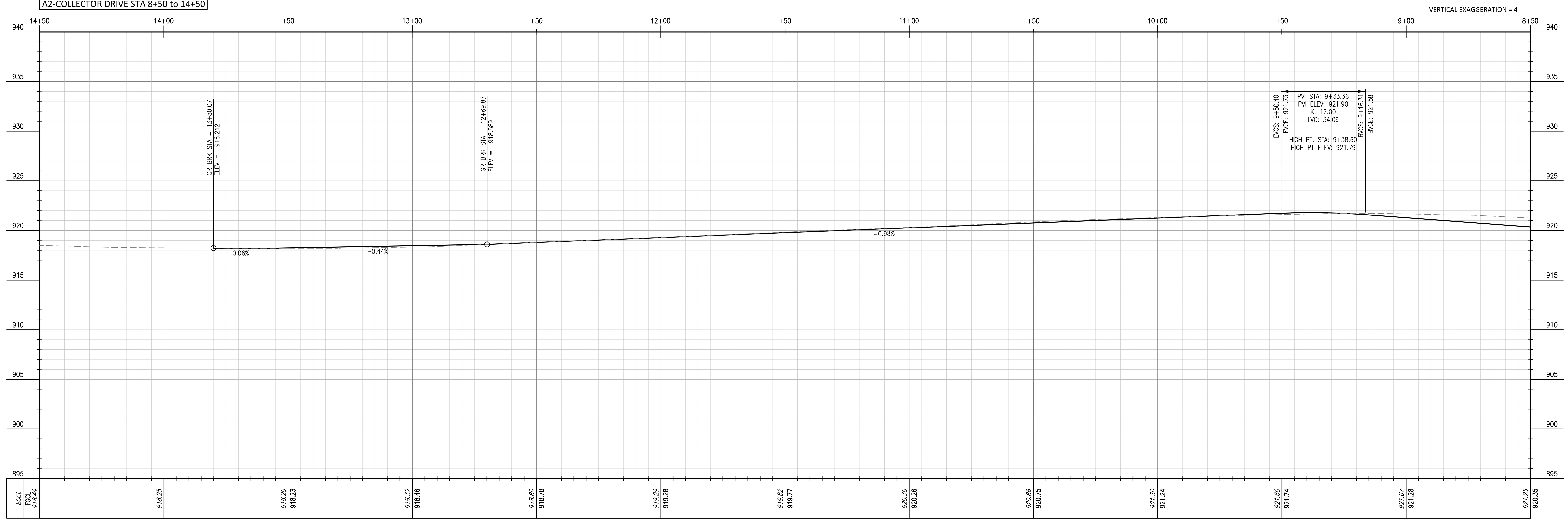


A2-COLLECTOR DRIVE STA 8+50 to 14+50



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SEPT. 6, 1839

CHEROKEE NATION REPLACEMENT HOSPITAL
TAHLEQUAH, OKLAHOMA

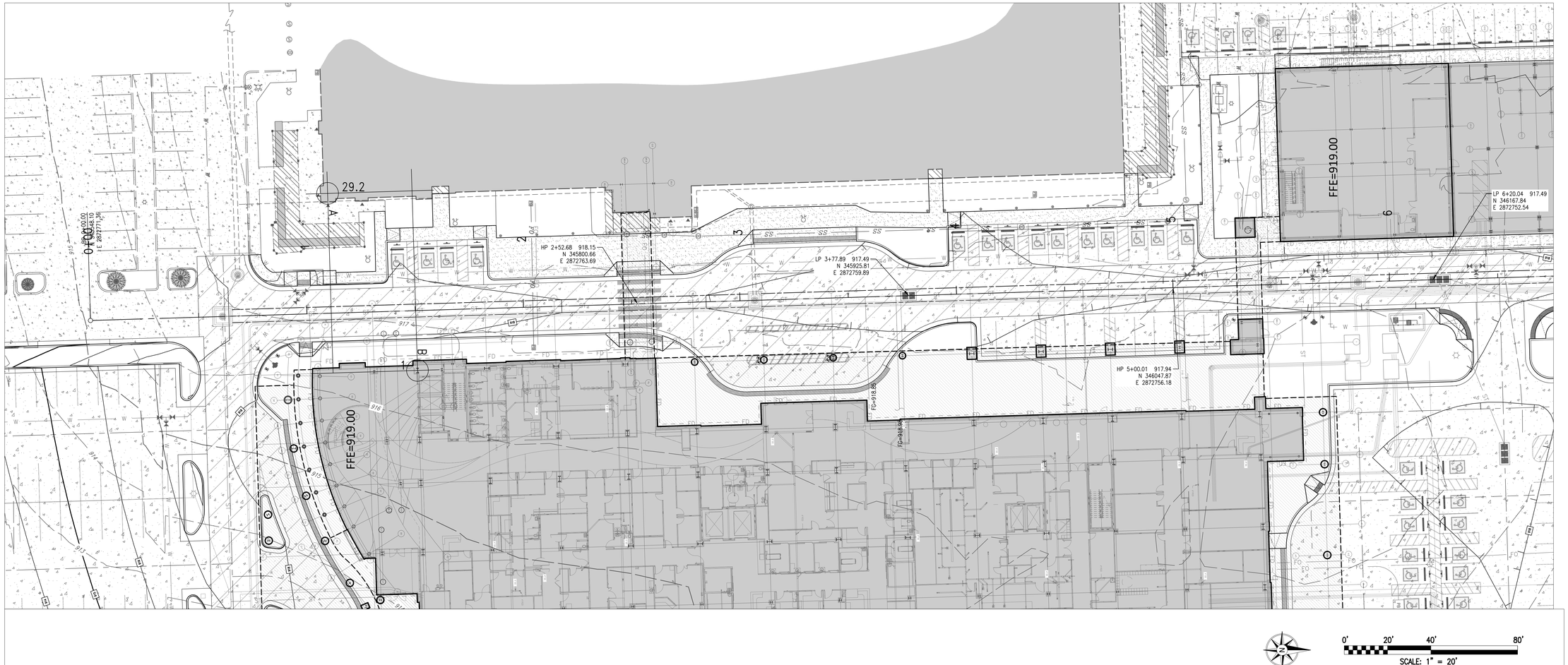
KEY PLAN:

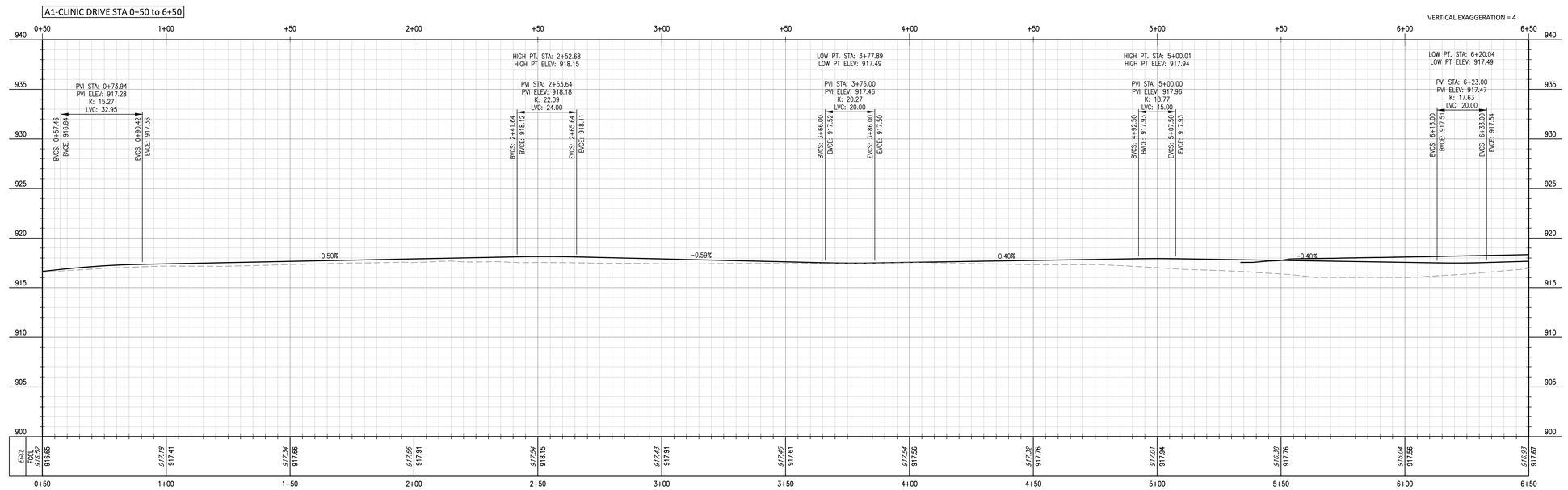
A1	A2
B1	B2
C1	C2

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

JOB NUMBER: 21-08.21
DATE: 12-09-2022
SHEET NUMBER: C5-204
SHEET TITLE: LOOP DRIVE PLAN AND PROFILE





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TAHLEQUAH, OKLAHOMA

KEY PLAN:

A1	A2
B1	B2
C1	C2

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS:

JOB NUMBER: 21-08.21
DATE: 12-09-2022
SHEET NUMBER: C5-205
SHEET TITLE: WEST DRIVE PLAN AND PROFILE

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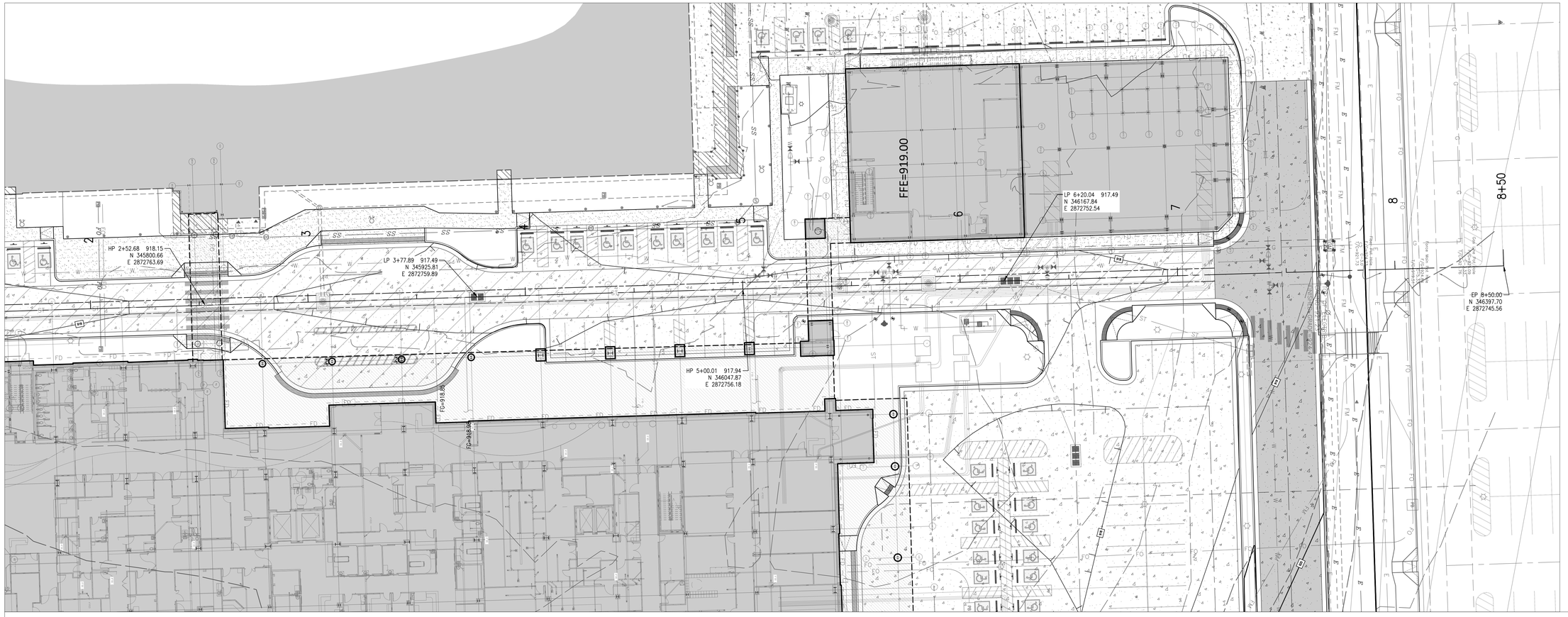
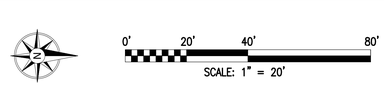
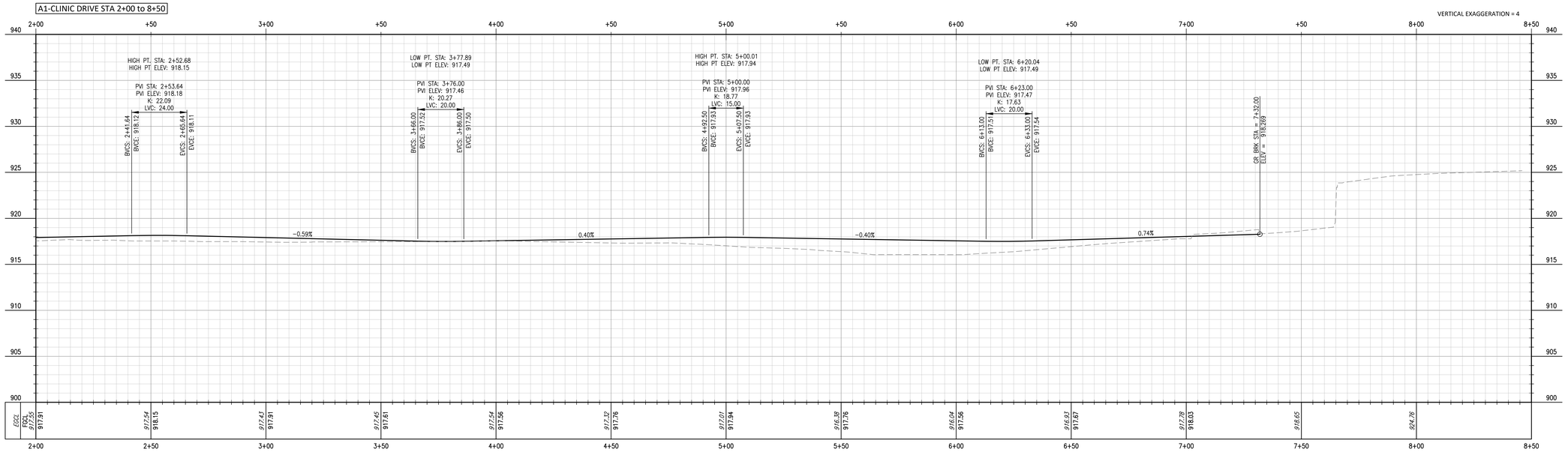




A1-CLINIC DRIVE STA 2+00 to 8+50

VERTICAL EXAGGERATION = 4



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KEY PLAN:

A1	A2
B1	B2
C1	C2

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS:

JOB NUMBER: 21-08.21

DATE: 12-09-2022

SHEET NUMBER: C5-206

SHEET TITLE: WEST DRIVE PLAN AND PROFILE

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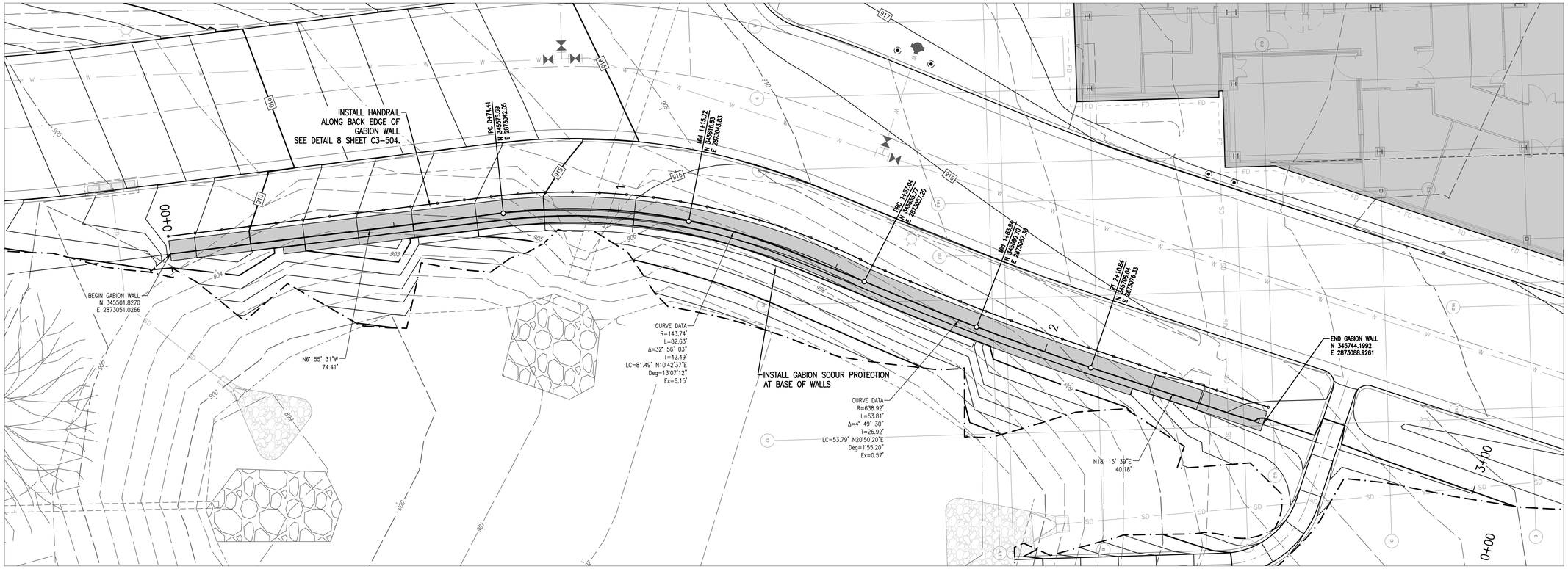

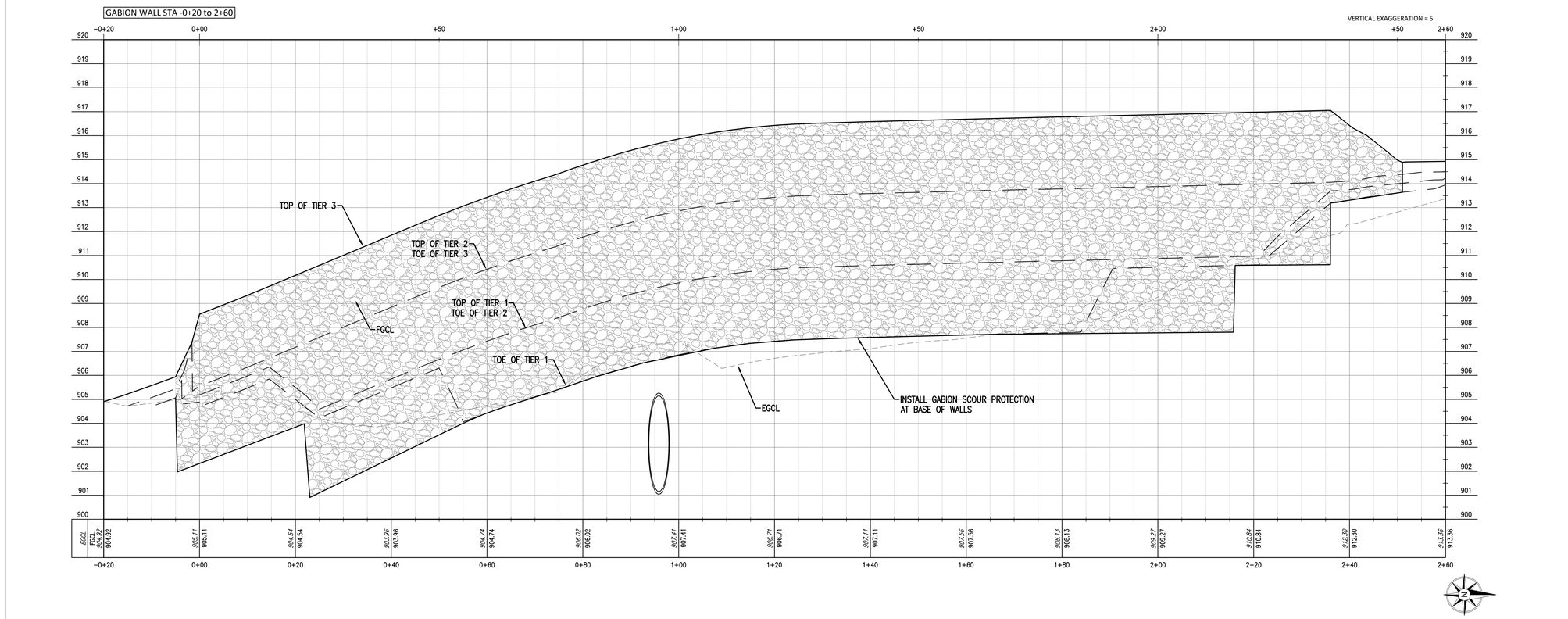

0' 10' 20' 40'

SCALE: 1" = 10'

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REGISTERED PROFESSIONAL ENGINEER
COREY W. LIPPS
21291
OKLAHOMA
12/9/22

CONSULTANT LOGO:

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SEPT. 6, 1839

CHEROKEE NATION
REPLACEMENT HOSPITAL
TAHLEQUAH, OKLAHOMA

KEY PLAN:

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

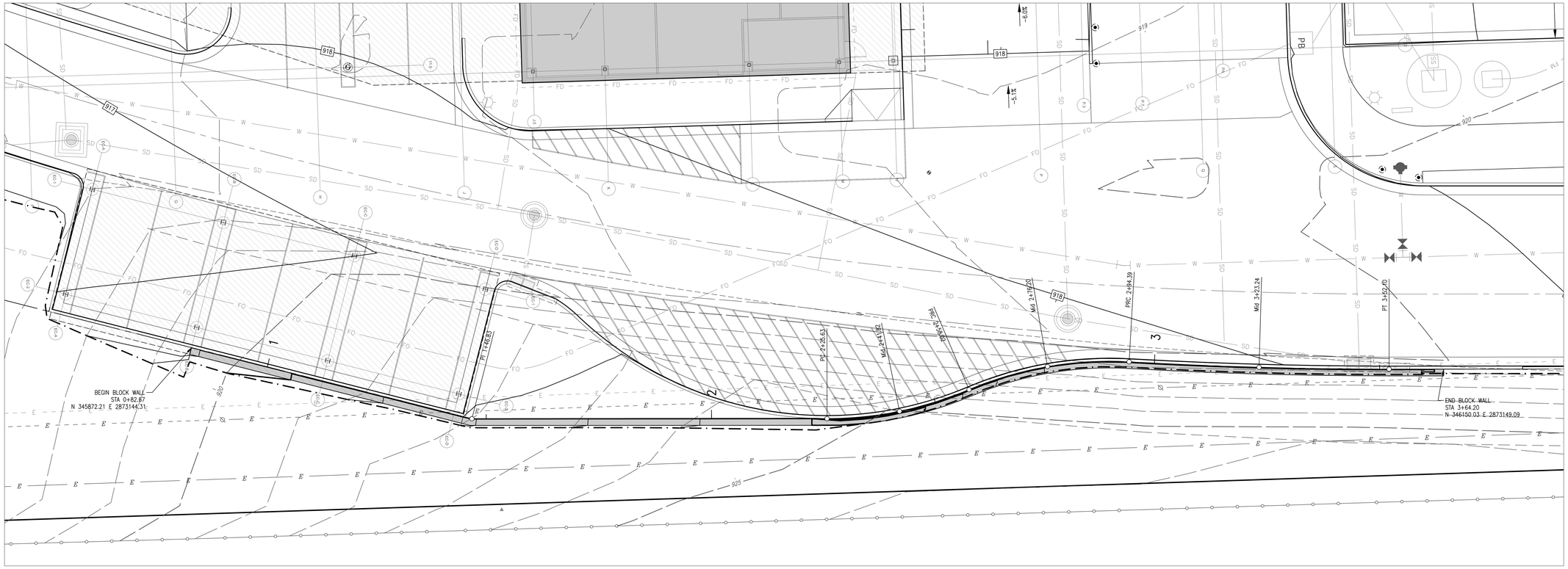
JOB NUMBER: 21-08.21

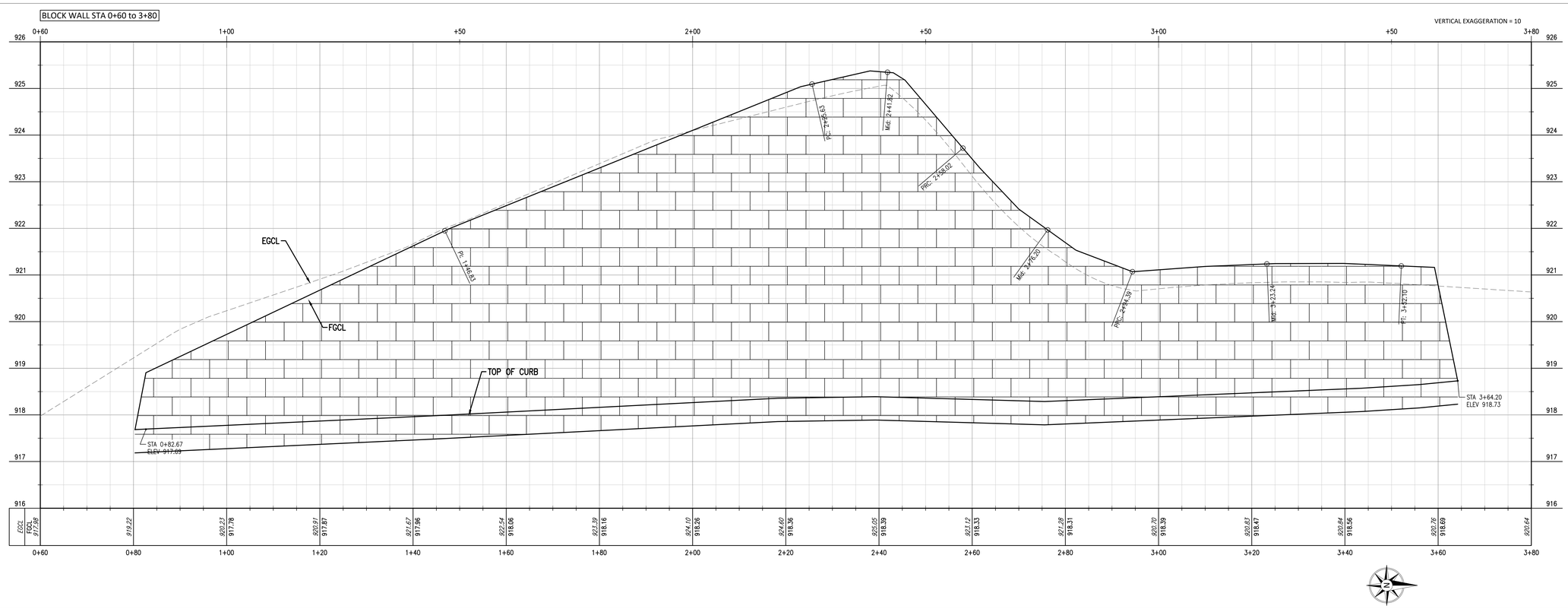
DATE: 12-09-2022

SHEET NUMBER: **C5-207**

SHEET TITLE: GABION WALL PLAN AND PROFILE

12/9/2022 A:\2021\5123.21\03_DSGN\01_DWG\050_CIVIL\C5-WALL-5123.DWG





0' 10' 20' 40'

SCALE: 1" = 10'

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12/9/2022 A:\2021\5123.21\03_DSGN\01_DWG\050_CIVIL\C5-WALL-5123.DWG

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CLIENT:

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SEPT. 6, 1839

CHEROKEE NATION REPLACEMENT HOSPITAL

TAHLEQUAH, OKLAHOMA

KEY PLAN:

A1	A2
B1	B2
C1	C2

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

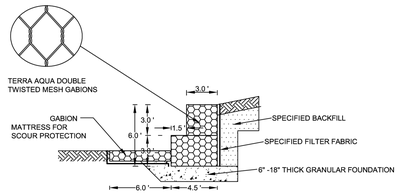
JOB NUMBER: 21-08.21

DATE: 12-09-2022

SHEET NUMBER: **C5-208**

SHEET TITLE: BLOCK WALL PLAN AND PROFILE

REVISIONS				
ZONE	REV	DESCRIPTION	DATE	APPROVED



SECTION

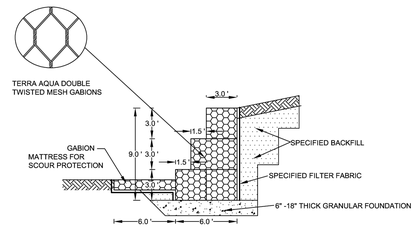
SD # GW 8A - 6' GRAVITY WALL WITH
GRANULAR FOUNDATION

	SD # GW 8A - 6' GRAVITY WALL WITH GRANULAR FOUNDATION
	TERRA AQUA GABIONS 1415 N 32nd, Fort Smith, AR 72904. Phone: 800-736-9089 fax; 479-785-0633
	SIZE: FSCM NO.: DWG NO.: TAGGW8A REV:
	SCALE: DRAWN BY: T.YU CHECKED BY: COLIN GLASS SHEET: 11 of 14

1 6' GRAVITY WALL W/GRANULAR FOUNDATION

Scale: NTS

REVISIONS				
ZONE	REV	DESCRIPTION	DATE	APPROVED



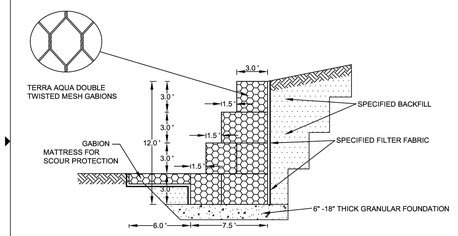
SECTION

	SD # GW 8B - 9' GRAVITY WALL WITH GRANULAR FOUNDATION
	TERRA AQUA GABIONS 1415 N 32nd, Fort Smith, AR 72904. Phone: 800-736-9089 fax; 479-785-0633
	SIZE: FSCM NO.: DWG NO.: TAGGW8B REV:
	SCALE: DRAWN BY: T.YU CHECKED BY: COLIN GLASS SHEET: 12 of 14

2 9' GRAVITY WALL W/GRANULAR FOUNDATION

Scale: NTS

REVISIONS				
ZONE	REV	DESCRIPTION	DATE	APPROVED



SECTION

	SD # GW 8C - 12' GRAVITY WALL WITH GRANULAR FOUNDATION
	TERRA AQUA GABIONS 1415 N 32nd, Fort Smith, AR 72904. Phone: 800-736-9089 fax; 479-785-0633
	SIZE: FSCM NO.: DWG NO.: TAGGW8C REV:
	SCALE: DRAWN BY: T.YU CHECKED BY: COLIN GLASS SHEET: 13 of 14

3 12' GRAVITY WALL W/GRANULAR FOUNDATION

Scale: NTS

1. WALL BACKFILL SHALL BE CLEAN CRUSHED STONE COMPACTED TO 95% STANDARD PROCTOR DENSITY PER ASTM D698.
2. THE UPPER 3-FT OF BACKFILL MAY BE ON-SITE SOILS THAT EXCEED THE REQUIREMENTS FOR FILL MATERIAL.
3. GRANUAL FOUNDATION SHALL BE 12-IN LOW VOLUME CHANGE ENGINEERED FILL COMPACTED TO 95% STANDARD PROCTOR DENSITY PER ASTM D698.
4. FILTER FABRIC SHALL MEET THE REQUIREMENTS OF AASHTO M 288 "PERMANENT EROSION CONTROL GEOTEXTILE REQUIREMENTS."
5. GABION MATTRESS IS REQUIRED.

James R. Childers
Architect, Inc.
45 South 4th Street
Fort Smith, AR 72901
479-783-2480
www.childersarchitect.com

PROFESSIONAL SEAL:

REGISTERED PROFESSIONAL ENGINEER
COREY W. LIPPS
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OKLAHOMA
12/9/22

CONSULTANT LOGO:

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CLIENT:

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SEPT. 6, 1839

CHEROKEE NATION
REPLACEMENT HOSPITAL
TAHLEQUAH, OKLAHOMA

KEY PLAN:

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS		

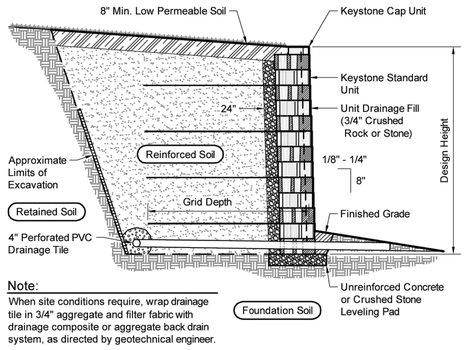
JOB NUMBER: 21-08.21

DATE: 12-09-2022

SHEET NUMBER: **C5-501**

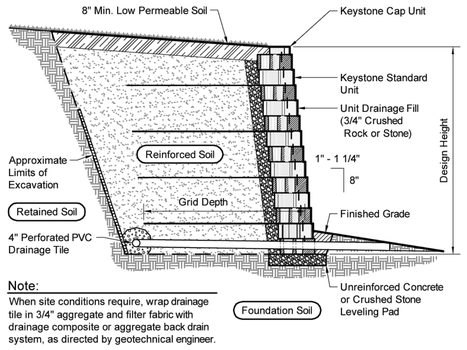
SHEET TITLE: GABION WALL DETAILS

12/9/2022 A:\2021\5123.21\03_DSGN\01_DWG\050_CIVIL\C5-DETL-5123.DWG



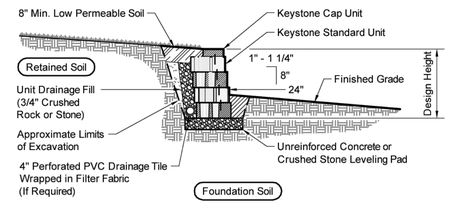
Note:
When site conditions require, wrap drainage tile in 3/4" aggregate and filter fabric with drainage composite or aggregate back drain system, as directed by geotechnical engineer.

Typical Reinforced Wall Section
Standard Unit - Near Vertical Setback



Note:
When site conditions require, wrap drainage tile in 3/4" aggregate and filter fabric with drainage composite or aggregate back drain system, as directed by geotechnical engineer.

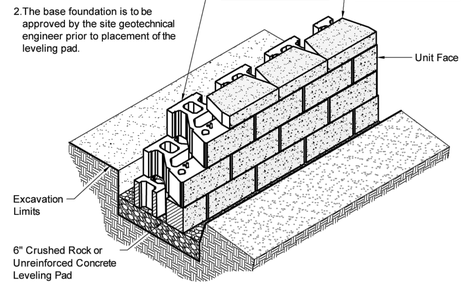
Typical Reinforced Wall Section
Standard Unit - 1" Setback



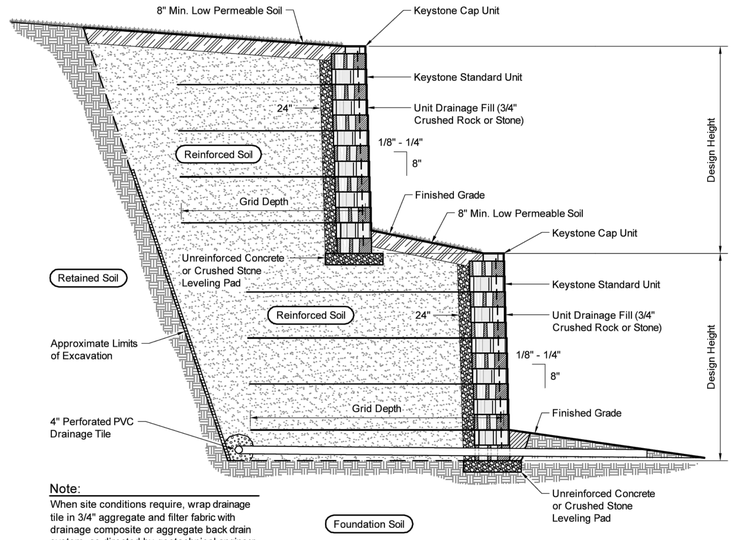
Typical Gravity Wall Section
Standard Unit - 1" Setback

Base Leveling Pad Notes:
1. The leveling pad is to be constructed of crushed stone or 2,000 psi± unreinforced concrete
2. The base foundation is to be approved by the site geotechnical engineer prior to placement of the leveling pad.

Standard Unit		Cap Unit	
Width:	18"	Width:	18"
*Depth:	18"	*Depth:	10 1/2"
Height:	8"	Height:	4"
*Weight:	108 lbs	*Weight:	50 lbs

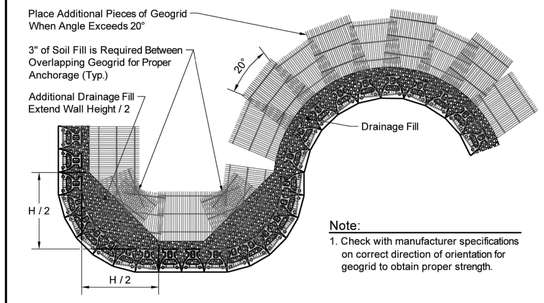


Standard Unit/Base Pad Isometric Section View
* Dimensions & Weight May Vary by Region



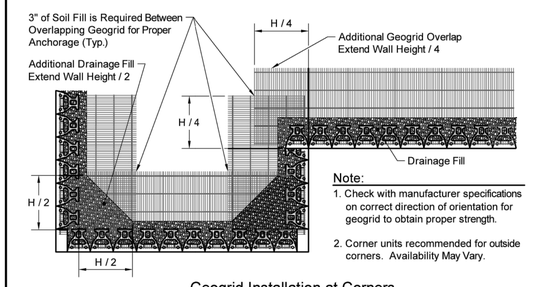
Note:
When site conditions require, wrap drainage tile in 3/4" aggregate and filter fabric with drainage composite or aggregate back drain system, as directed by geotechnical engineer.

Typical Reinforced Tiered Wall Section
Standard Unit - Near Vertical Setback



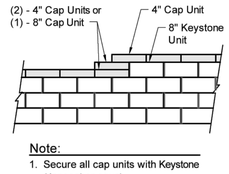
Note:
1. Check with manufacturer specifications on correct direction of orientation for geogrid to obtain proper strength.

Geogrid Installation on Curves



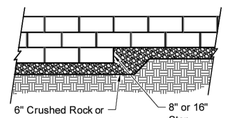
Note:
1. Check with manufacturer specifications on correct direction of orientation for geogrid to obtain proper strength.
2. Corner units recommended for outside corners. Availability May Vary.

Geogrid Installation at Corners



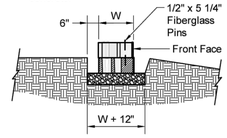
Note:
1. Secure all cap units with Keystone Kapseal or equal.

Top of Wall Steps

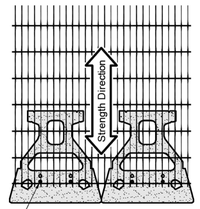


Elevation

Note:
1. The leveling pad is to be constructed of crushed stone or 2000 psi ± unreinforced concrete.

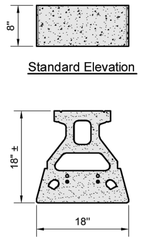


Section
Leveling Pad Detail

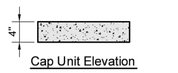


Geogrid is to be Placed on Level Backfill and Extended Over the Fiberglass Pins. Place Next Unit. Pull Grid Taught and Backfill. Stake as required.

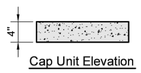
Grid & Pin Connection



Standard Elevation
Standard Plan
Standard Unit
* Dimensions May Vary by Region



Cap Unit Elevation
Cap Unit Plan
Universal Cap Unit Option
* Dimensions & Availability Will Vary by Region



Cap Unit Elevation
Cap Unit Plan
Straight Split Cap Unit Option
* Dimensions & Availability Will Vary by Region

Copyright 2003 Keystone Retaining Wall Systems

Design is for internal stability of the KEYSTONE wall structure only. External stability, including but not limited to foundation and slope stability is the responsibility of the Owner. The design is based on the assumption that the materials within the retained mass, methods of construction, and quality of materials conform to KEYSTONE's specification for this project.

This drawing is being furnished for this specific project only. Any party accepting this document does so in confidence and agrees that it shall not be duplicated in whole or in part, nor disclosed to others without the consent of Keystone Retaining Wall Systems, Inc.

No.	Date	Revision	By

KEYSTONE RETAINING WALL SYSTEMS
A CONTECH COMPANY
4444 W 78th Street
Minneapolis, MN 55435
952-897-1040

Designed By: RKM
Checked By: CDM
Scale: No Scale

Title: Standard Unit 18 - Straight Face Details
Project: Keystone Retaining Wall Systems Typical Wall Details
Date:
Project No:
Drawing No:

1 MASONRY BLOCK WALL DETAILS

Scale: NTS

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CLIENT:
SEAL OF THE CHEROKEE NATION SEPT. 6, 1839

CHEROKEE NATION
REPLACEMENT HOSPITAL
TAHLEQUAH, OKLAHOMA

KEY PLAN:

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

JOB NUMBER: 21-08.21
DATE: 12-09-2022
SHEET NUMBER: C5-502
SHEET TITLE: MASONRY BLOCK WALL DETAILS

12/9/2022 A:\2021\5123.21\03_DSGN\01_DWG\050_CIVIL\C5-DETL-5123.DWG

EARTHWORK CALCULATION NOTES

1. THE EARTHWORK VOLUMES PROVIDED ARE CALCULATED BY COMPARING THE FINISHED GRADE SURFACE MODEL WITH THE EXISTING GRADE SURFACE MODEL.
2. NO ADJUSTMENTS MADE FOR COMPACTION OR EXPANSION OF MATERIAL.
3. NO ADJUSTMENTS ARE MADE FOR BUILDING PAD(S), STRUCTURES, OR PAVEMENTS.
4. NO ADJUSTMENTS ARE MADE FOR UTILITY TRENCH SPOILS.

CUT VOLUME = 1,830 CY
FILL VOLUME = 23,862 CY

NET VOLUME = 22,032 CY CUT / FILL

Cut / Fill Table

Number	Minimum Elevation	Maximum Elevation	Area (sf)	Color
1	−7.13	−6.00	347	
2	−6.00	−3.00	2148	
3	−3.00	0.00	72556	
4	0.00	3.00	205257	
5	3.00	6.00	76010	
6	6.00	9.83	5234	

N

0' 50' 100' 200'

SCALE: 1" = 50'

UTILITY WARNING:
THE UNDERGROUND UTILITIES SHOWN HAVE BEEN LOCATED FROM RECORD DOCUMENTS OR FIELD LOCATIONS BY THE OPERATOR. THE SURVEYOR MAKES NO GUARANTEE THAT THE UNDERGROUND UTILITIES SHOWN COMPRISE ALL SUCH UTILITIES IN THE AREA EITHER IN SERVICE OR ABANDONED. THE SURVEYOR FURTHER DOES NOT WARRANT THAT THE UNDERGROUND UTILITIES SHOWN ARE IN THE EXACT LOCATION INDICATED ALTHOUGH THE SURVEYOR DOES CERTIFY THAT THEY ARE LOCATED AS ACCURATELY AS POSSIBLE FROM THE INFORMATION AVAILABLE. THE SURVEYOR HAS NOT PHYSICALLY LOCATED THE UNDERGROUND UTILITIES.

UTILITY ELEVATIONS AND SIZES MAY HAVE BEEN MEASURED UNDER ADVERSE FIELD CONDITIONS. UPON EXPOSING THE UTILITY, ELEVATIONS AND LINE SIZES SHOULD BE VERIFIED BY THE CONTRACTOR PRIOR TO CONSTRUCTION. CONTRACTOR SHOULD VERIFY CRITICAL ELEVATIONS USING THE BENCHMARK PROVIDED BY THE SURVEYOR OR ENGINEER. ANY DISCREPANCIES SHOULD BE IMMEDIATELY BROUGHT TO THE ENGINEER'S AND SURVEYOR'S ATTENTION.

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SEPT. 6, 1839

CHEROKEE NATION REPLACEMENT HOSPITAL

TAHLEQUAH, OKLAHOMA

KEY PLAN:

A1	A2
B1	B2
C1	C2

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

JOB NUMBER: 21-08.21

DATE: 12-09-2022

SHEET NUMBER: **C5-801**

SHEET TITLE: EARTHWORK ANALYSIS

12/9/2022 A:\2021\5123.21\03_DSGN\01_DWG\050_CIVIL\C5-GRAD-5123.DWG

CHEROKEE NATION
REPLACEMENT HOSPITAL

VOLUME 02 OF 11
(LANDSCAPE)

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CINCINNATI, OH 45233
(877) HELIPAD

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HOWARD-FAIRBAIRN SITE DESIGN
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OKLAHOMA CITY, OK 73134
(405) 752-8018

LANDSCAPE ARCHITECTURE

HKS
999 18th St, #2255N
DENVER, CO 80202
(303) 293-2903

INTERIOR ARCHITECTURE | INTERIOR DESIGN

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(913) 312-0075

STRUCTURAL ENGINEER

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TULSA, OK 74136
(918) 457-6905

OWNERS REPRESENTATIVE

FOREMAN MANHATTAN CONSTRUCTION TEAM
5601 S 122nd E AVE
TULSA, OK 74146
(918) 878-3492

CONSTRUCTION MANAGER

James R. Childers Architect, Inc.
45 South 4th Street
Fort Smith, AR 72901
479-783-2480
www.childersarchitect.com

PROFESSIONAL SEAL:

STATE OF OKLAHOMA
J. BRECK CHILDERS
5120
12/09/2022
REGISTERED ARCHITECT

CONSULTANT LOGO:

CLIENT:

SEAL OF THE CHEROKEE NATION
SEPT. 6, 1839

CHEROKEE NATION REPLACEMENT HOSPITAL
TAHLEQUAH, OKLAHOMA

KEY PLAN:

PROJECT PHASE:

REVISIONS		
#	DATE	DESCRIPTION

JOB NUMBER: 21-08.21

DATE: 12-09-2022

SHEET NUMBER: **VOL 02**

SHEET TITLE: COVER SHEET

INDEX OF DRAWINGS - VOLUME 01 - CIVIL

SHEET NUMBER	SHEET NAME	BID PACKAGE 01 - 07-29-2022	ADDENDUM 01 - 08-17-2022	BID PACKAGE 03 - 10-07-2022	BID PACKAGE 02 - 10-21-2022	ASI 01 - 10-24-2022	ADDENDUM 02 - 11-18-2022	BID PACKAGE 04 - 12-09-2022
GENERAL								
VOL 01	COVER SHEET	■			■		■	■
G0.01	DRAWING INDEX	■		■	■		■	■
CIVIL								
C0-101	GENERAL CIVIL NOTES -BP2				■		■	
C0-102	GENERAL CIVIL NOTES - BP1	■						
C0-103	GENERAL CIVIL NOTES - BP4							■
C0-901	PROJECT MAP							■
C1-101	EXISTING CONDITIONS				■			
C1-102	EXISTING CONDITIONS				■			
C1-103	EXISTING CONDITIONS	■						
C1-104	EXISTING CONDITIONS	■						
C1-105	EXISTING CONDITIONS	■						
C1-106	EXISTING CONDITIONS				■			
C1-107	EXISTING CONDITIONS				■			
C2-101	OVERALL DEMOLITION VIEW				■			
C2-102	A1 DEMOLITION PLAN				■			
C2-103	A2 DEMOLITION PLAN				■			
C2-104	B1 DEMOLITION PLAN				■			
C2-105	B2 DEMOLITION PLAN				■			
C2-106	C1 DEMOLITION PLAN				■			
C2-107	C2 DEMOLITION PLAN				■			
C2-108	NP DEMOLITION OVERVIEW	■						
C2-109	NP 1 DEMOLITION PLAN	■						
C2-110	NP2 DEMOLITION PLAN	■						
C2-111	NP3 DEMOLITION PLAN	■						
C2-112	NP4 DEMOLITION PLAN	■						
C2-113	NP5 DEMOLITION PLAN	■						
C2-701	EROSION CONTROL PLAN - HOSPITAL				■			
C2-702	NP EROSION CONTROL PLAN	■						
C2-750	EROSION CONTROL DETAILS	■						
C2-751	EROSION CONTROL DETAILS				■			
C3-101	OVERALL SITE VIEW							■
C3-102	A1 SITE PLAN							■
C3-103	A2 SITE PLAN							■
C3-104	B1 SITE PLAN							■
C3-105	B2 SITE PLAN							■
C3-106	C1 SITE PLAN							■
C3-107	C2 SITE PLAN							■
C3-108	NP SITE PLAN OVERVIEW	■	■					
C3-109	NP1 SITE PLAN	■	■					
C3-110	NP2 SITE PLAN	■	■					
C3-111	NP3 SITE PLAN	■	■					
C3-112	NP4 SITE PLAN	■	■					
C3-113	NP5 SITE PLAN	■	■					
C3-501	SITE DETAILS	■						
C3-502	SITE DETAILS	■						
C3-503	CONCRETE PAVING JOINT DETAILS	■						
C3-504	SITE DETAILS							■
C3-901	PARKING SPACE ACCOUNTING							■
C4-101	OVERALL UTILITY VIEW				■		■	
C4-102	A1 UTILITIES				■			
C4-103	A2 UTILITIES				■		■	
C4-104	B1 UTILITIES				■			
C4-105	B2 UTILITIES				■			
C4-106	C1 UTILITIES				■			
C4-107	C2 UTILITIES				■			
C4-201	LREC RELOCATION PLAN AND PROFILE					■		
C4-203	PHASE 1 UTILITY PLAN						■	
C4-801	SITE ELECTRIC AND COMMUNICATION CONDUITS				■			
C5-101	OVERALL GRADING VIEW							■
C5-102	A1 GRADING PLAN							■
C5-103	A2 GRADING PLAN							■
C5-104	B1 GRADING PLAN							■
C5-105	B2 GRADING PLAN							■
C5-106	C1 GRADING PLAN							■
C5-107	C2 GRADING PLAN							■
C5-113	NP GRADING OVERVIEW	■						
C5-114	NP1 GRADING PLAN	■						
C5-115	NP2 GRADING PLAN	■						
C5-116	NP3 GRADING PLAN	■						
C5-117	NP4 GRADING PLAN	■						
C5-118	NP5 GRADING PLAN	■						
C5-201	COORDINATES AND ELEVATIONS							■
C5-202	LOOP DRIVE PLAN AND PROFILE							■
C5-203	LOOP DRIVE PLAN AND PROFILE							■
C5-204	LOOP DRIVE PLAN AND PROFILE							■
C5-205	WEST DRIVE PLAN AND PROFILE							■
C5-206	WEST DRIVE PLAN AND PROFILE							■
C5-207	GABION WALL PLAN AND PROFILE							■
C5-208	BLOCK WALL PLAN AND PROFILE							■
C5-210	NP COORDINATES AND ELEVATIONS TABLE	■						
C5-501	GABION WALL DETAILS							■
C5-502	MASONRY BLOCK WALL DETAILS							■
C5-801	EARTWORK ANALYSIS							■
C6-101	STORM DRAIN PLAN				■		■	
C6-102	STORM DRAIN PLAN				■		■	
C6-103	ROOF DRAIN PLAN				■			
C6-104	FOUNDATION DRAIN PLAN				■			
C6-201	NP STORM PLAN & PROFILE	■						
C6-202	NP STORM PLAN & PROFILE	■						
C6-203	STORM DRAIN PROFILES				■		■	
C6-204	STORM DRAIN PROFILES				■		■	
C6-501	STORM DRAIN DETAILS	■						
C6-502	STORM DRAIN DETAILS	■						
C6-503	STORM DRAIN DETAILS	■						
C6-504	STORM DRAIN DETAILS	■						
C6-505	STORM DRAIN DETAILS	■						
C6-506	CICI BOX				■			
C6-507	CICI FRAME				■			
C6-508	CICI GRATE				■			
C6-509	DESIGN 6 AND 7 AREA INLET				■			
C6-510	GRATE AND FRAME FOR AREA INLET				■			
C6-511	STORM DRAIN DETAILS				■			
C6-802	NP EXISTING HYDROLOGY	■						
C6-803	NP DEVELOPMENT HYDROLOGY	■						
C7-101	OVERALL WATER PLAN				■			
C7-201	WATER 1 PLAN AND PROFILE				■			
C7-202	WATER 1 PLAN AND PROFILE				■			
C7-203	WATER 1 PLAN AND PROFILE				■			
C7-204	WATER 2 PLAN AND PROFILE				■			
C7-205	WATER 2 PLAN AND PROFILE				■			
C7-206	WATER 2 PLAN AND PROFILE				■		■	
C7-207	WATER 2 PLAN AND PROFILE				■		■	
C7-208	WATER 3 PLAN AND PROFILE				■			
C7-209	WATER 3 PLAN AND PROFILE				■			
C7-210	WATER 4 PLAN AND PROFILE				■		■	
C7-211	WATER 5 AND 14 PLAN AND PROFILE				■			
C7-212	WATER 6, 9, AND 10 PLAN AND PROFILE				■		■	
C7-213	WATER 7, 8, AND 13 PLAN AND PROFILE				■		■	
C7-214	WATER 11 AND 12 PLAN AND PROFILE				■		■	
C7-501	TPWA WATER DETAILS				■			
C7-502	WATER DETAILS				■			
C8-201	FORCE MAIN PLAN & PROFILE	■						
C8-501	FORCE MAIN DETAILS	■						
C9-201	DOWNING DRIVEWAY PLAN AND PROFILE	■	■					
C9-501	DOWNING DRIVEWAY DETAILS	■						

Grand total: 125

INDEX OF DRAWINGS - VOLUME 02 - LANDSCAPE

SHEET NUMBER	SHEET NAME	BID PACKAGE 01 - 07-29-2022	ADDENDUM 01 - 08-17-2022	BID PACKAGE 03 - 10-07-2022	BID PACKAGE 02 - 10-21-2022	ASI 01 - 10-24-2022	ADDENDUM 02 - 11-18-2022	BID PACKAGE 04 - 12-09-2022
GENERAL								
VOL 02	COVER SHEET							■
G0.01	DRAWING INDEX	■	■	■	■	■	■	■
LANDSCAPE								
HA1.1	SITE HARDSCAPE PLAN							■
HA1.2	ENLARGED HARDSCAPE PLAN - SOUTH							■
HA1.3	ENLARGED HARDSCAPE PLAN - CENTRAL							■
HA1.4	ENLARGED HARDSCAPE PLAN - NORTH							■
HA2.1	HARDSCAPE DETAILS							■

Grand total: 7

INDEX OF DRAWINGS - VOLUME 04 - STRUCTURAL

SHEET NUMBER	SHEET NAME	BID PACKAGE 01 - 07-29-2022	ADDENDUM 01 - 08-17-2022	BID PACKAGE 03 - 10-07-2022	BID PACKAGE 02 - 10-21-2022	ASI 01 - 10-24-2022	ADDENDUM 02 - 11-18-2022	BID PACKAGE 04 - 12-09-2022
GENERAL								
VOL 04	COVER SHEET							■
G0.01	DRAWING INDEX	■	■	■	■	■	■	■
STRUCTURAL								
S0.1	COVER SHEET							■
S0.2	GENERAL NOTES							■
S0.3	GENERAL STRUCTURAL NOTES							■
S0.4	SPECIAL INSPECTIONS TABLE							■
S1.01.1	01 OVERALL FOUNDATION PLAN							■
S1.01.2	01 SLAB PLAN							■
S1.01.3	ELEVATOR PIT PLAN							■
S1.01.4	DRILLED PIER PLAN							■
S2.01.1	FOUNDATION PLAN SECTOR 01							■
S2.01.2	FOUNDATION PLAN SECTOR 02							■
S2.01.3	FOUNDATION PLAN SECTOR 03							■
S2.01.4	FOUNDATION PLAN SECTOR 04							■
S2.01.5	FOUNDATION PLAN SECTOR 05							■
S4.01	CONCRETE TYPICAL DETAILS							■
S4.02	CONCRETE TYPICAL DETAILS							■
S4.03	FOUNDATION DETAILS							■
S4.05	BASE PLATE DETAILS							■
S4.06	FOUNDATION SECTIONS							■
S4.07	FOUNDATION SECTIONS							■
S4.08	FOUNDATION SECTIONS							■
S4.09	FOUNDATION SECTIONS							■
SB101	SKYBRIDGE - ENLARGEMENTS							■
SB102	SKYBRIDGE - SECTIONS							■
SC100	SOUTH CANOPY							■
SC101	SOUTH CANOPY - FOUNDATION PLAN							■
SC103	SOUTH CANOPY - FRAMES							■
SC104	SOUTH CANOPY - DETAILS							■
SE101	EAST CANOPY - STRUCTURAL PLAN							■
SM101	MECHANICAL AREA - OVERALL FOUNDATION PLAN							■
SM111	MECHANICAL AREA (BRIDGE) - ENLARGEMENTS							■
SM112	MECHANICAL AREA (BRIDGE) - SECTIONS							■
SM113	MECHANICAL AREA - FOUNDATION DETAILS							■
SM114	MECHANICAL AREA - ANCHOR RODS							■

Grand total: 35

INDEX OF DRAWINGS - VOLUME 07 - ELECTRICAL

SHEET NUMBER	SHEET NAME	BID PACKAGE 01 - 07-29-2022	ADDENDUM 01 - 08-17-2022	BID PACKAGE 03 - 10-07-2022	BID PACKAGE 02 - 10-21-2022	ASI 01 - 10-24-2022	ADDENDUM 02 - 11-18-2022	BID PACKAGE 04 - 12-09-2022
GENERAL								
VOL 07	COVER SHEET	■						
G0.01	DRAWING INDEX	■	■	■	■	■	■	■
ELECTRICAL								
ENL.00	ELECTRICAL LEGEND	■						
ENL.01	ELECTRICAL SITE PLAN - NORTH LOT	■						

Grand total: 4

INDEX OF DRAWINGS - VOLUME 09 -TECHNOLOGY

SHEET NUMBER	SHEET NAME	BID PACKAGE 01 - 07-29-2022	ADDENDUM 01 - 08-17-2022	BID PACKAGE 03 - 10-07-2022	BID PACKAGE 02 - 10-21-2022	ASI 01 - 10-24-2022	ADDENDUM 02 - 11-18-2022	BID PACKAGE 04 - 12-09-2022
GENERAL								
VOL 09	COVER SHEET	■						
G0.01	DRAWING INDEX	■	■	■	■	■	■	■
TECHNOLOGY								
TN51.02	NETWORK SITE PLAN - NORTH PARKING & ACCESS	■	■					

Grand total: 3

James R. Childers Architect, Inc.
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PROFESSIONAL SEAL:

STATE OF OKLAHOMA — J. BRECK CHILDERS — 8120 — 12/09/2022 — REGISTERED ARCHITECT

CONSULTANT LOGO:

CLIENT:

SEAL OF THE CHEROKEE NATION — SEPT. 6, 1839

CHEROKEE NATION REPLACEMENT HOSPITAL

TAHLEQUAH, OKLAHOMA

KEY PLAN:

PROJECT PHASE:

BID PACKAGE 04

(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

#	DATE	DESCRIPTION
1	08-17-22	ADDENDUM 01
2	10-07-22	BID PACKAGE 03
3	10-21-22	BID PACKAGE 02
4	10-24-22	ASI 01
5	11-18-22	ADDENDUM 02
6	12-09-22	BID PACKAGE 04

JOB NUMBER: 21-08.21

DATE: 07-29-2022

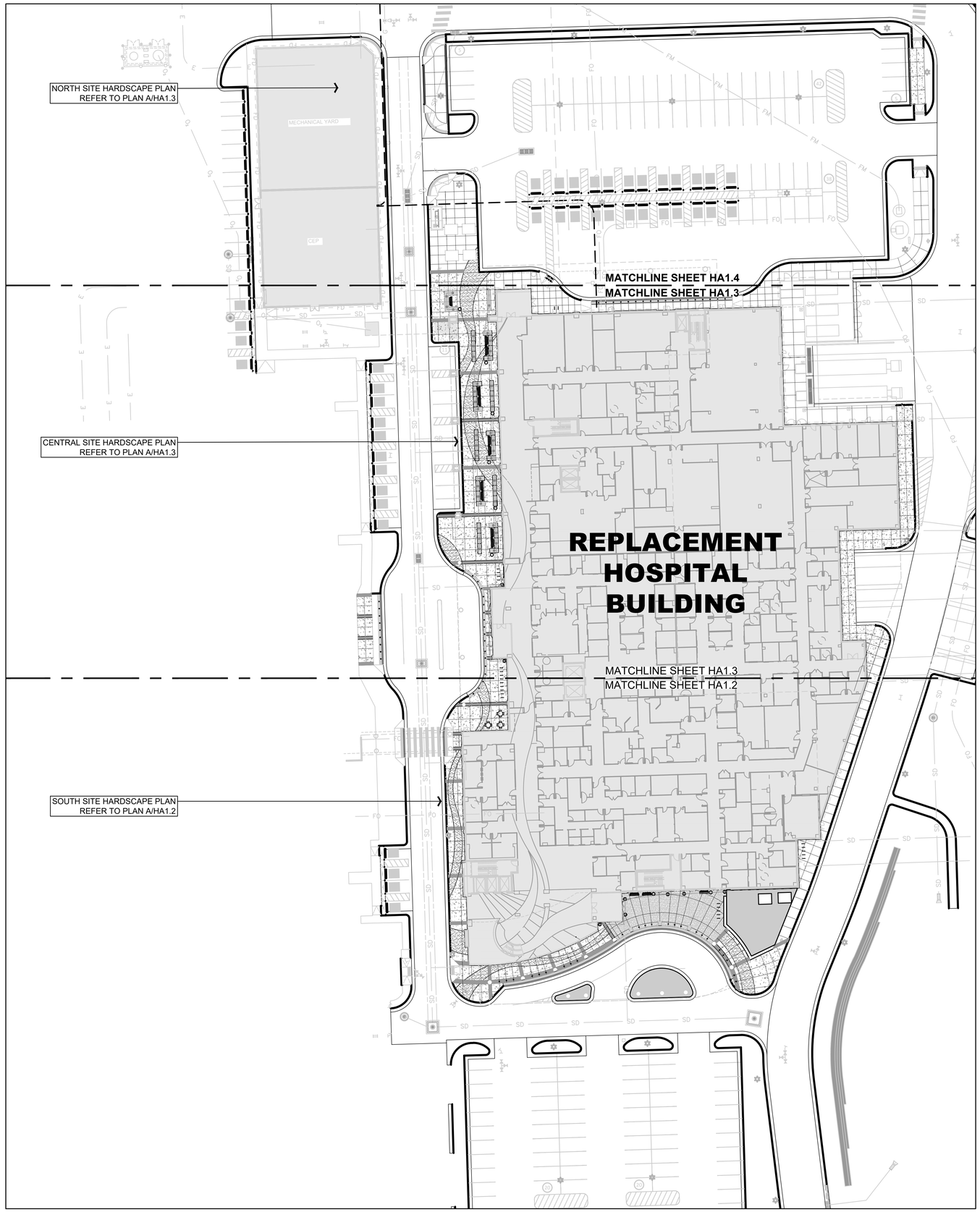
SHEET NUMBER: **G0.01**

SHEET TITLE: DRAWING INDEX

1" = 1'-0" 3" = 1'-0" 1 1/2" = 1'-0" 1" = 1'-0" 3/4" = 1'-0" 1/2" = 1'-0" 3/8" = 1'-0" 1/4" = 1'-0" 1/8" = 1'-0"

LAYOUT NOTES:

1. CONTRACTOR SHALL VERIFY EXISTING CONDITIONS, LAYOUT COORDINATES, AND WORK FROMONGOING CONTRACTS IN THE FIELD. CONTRACTOR SHALL REPORT ANY DISCREPANCIES TO THE OWNER'S REPRESENTATIVE FOR DIRECTIONS IMMEDIATELY BEFORE PROCEEDING WITH THAT PORTION OF THE WORK.
2. CONTRACTOR SHALL REQUEST A FIELD REVIEW BY THE OWNER'S REPRESENTATIVE OF THE LAYOUT OF ALL ELEMENTS, AS SHOWN. CONTRACTOR SHALL STAKE ALL LOCATIONS AND OBTAIN APPROVAL FROM THE OWNER'S REPRESENTATIVE PRIOR TO THE COMMENCEMENT OF WORK..
3. CONTRACTOR SHALL REQUEST A FIELD REVIEW BY THE OWNER'S REPRESENTATIVE OF ALL FORM WORK OR A TYPICAL PORTION OF FORM WORK REPRESENTING SIMILAR WORK. CONTRACTOR SHALL OBTAIN APPROVAL FROM THE OWNER'S REPRESENTATIVE OF ALL FORM WORK FOR FLATWORK AND WALL WORK PRIOR TO CONCRETE POURING.
4. CONTRACTOR SHALL COORDINATE ALL LAYOUT WORK POINTS, GRID LINES, AND CONTROLS, AMONG ALL TRADES; SPECIFICALLY, BUT NOT LIMITED TO, SITE FORMATION, FLATWORK, AND WALL WORK.
5. CONTRACTOR SHALL REFER TO PAVING PLAN(S) FOR ALL FLATWORK AND WALL WORK JOINT LOCATIONS. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS VERIFYING JOINT LAYOUT PRIOR TO INSTALLATION.
6. CONTRACTOR SHALL ERECT AND INSTALL ALL WORK LEVEL, PLUMB, SQUARE, TRUE, STRAIGHT, AND IN PROPER ALIGNMENT.
7. CONTRACTOR SHALL NOT SCALE DRAWINGS; CONTRACTOR SHALL USE DIMENSIONS SHOWN.
8. NO DIMENSIONS ARE ADJUSTABLE WITHOUT THE REVIEW AND APPROVAL OF THE OWNER'S REPRESENTATIVE UNLESS NOTED (+/-) FV (FIELD VERIFY)..
9. DIMENSIONS NOTED CLEAR (CLR) MUST BE STRICTLY MAINTAINED ALLOWING FOR THICKNESS OF ALL FINISHES. CONTRACTOR SHALL FIELD VERIFY (FV) PRIOR TO CONSTRUCTION.
10. TYPICAL (TYP) MEANS IDENTICAL FOR ALL SIMILAR CONDITIONS UNLESS OTHERWISE NOTED.
11. ALL CURVES FOR PAVING, BANDS, PATHS, EDGING, AND HEADER BOARDS SHALL BE ALIGNED IN A SMOOTH AND CONTINUOUS FASHION AND SHALL MEET ADJACENT SURFACES AT 90 DEGREES, UNLESS OTHERWISE INDICATED. ALL WALK RADII AND CURVES SHALL BE SMOOTH AND CONTINUOUS WITHOUT ABRUPT CHANGES OR BENDS UNLESS OTHERWISE SHOWN.



A **SITE HARDSCAPE PLAN**
1" = 30'-0"
0 30' 60' 90'

James R. Childers Architect, Inc.
45 South 4th Street
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PROFESSIONAL SEAL:
STATE OF OKLAHOMA JAMES R. CHILDERS #155 REGISTERED LANDSCAPE ARCHITECT
12/09/22

CONSULTANT LOGO:
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3100 NW 149TH ST.
OKLAHOMA CITY, OKLAHOMA 73134
VOICE: 405.752.8018
FAX: 405.752.8701

CLIENT:
THE SEAL OF THE CHEROKEE NATION SEQUOYAH SEPT. 6, 1839

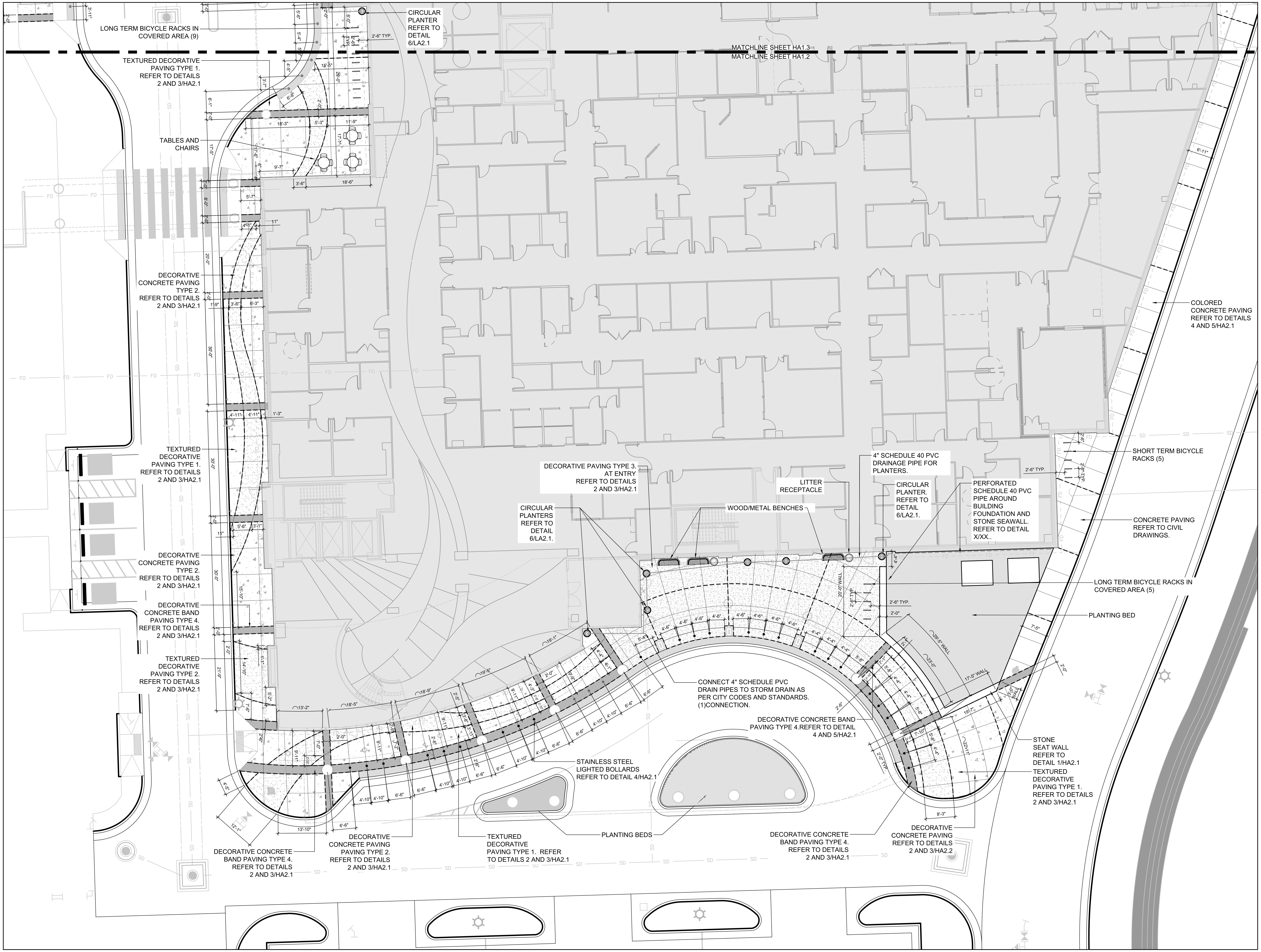
CHEROKEE NATION
REPLACEMENT HOSPITAL
TAHLEQUAH, OKLAHOMA

KEY PLAN:

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS		
#	DATE	DESCRIPTION

JOB NUMBER: 21-08.21
DATE: 12-09-2022
SHEET NUMBER: **HA1.1**
SHEET TITLE: SITE HARDSCAPE PLAN



A **ENLARGED HARDSCAPE PLAN - SOUTH**

1" = 40'-0"

0 10' 20' 30'

PAVING LEGEND

- - - - - - EXPANSION JOINT	——— SAW JOINT

BOMANITE SPECIALTY PAVING
(NO SUBSTITUTIONS)

TYPE 1- SANDSCAPE COLOR BLEND EX-SSR-110-112-14 & EX-SSR-110-112-13	TYPE 3- REVEAL - EX-RV-080211-16
TYPE 2- REVEAL - EX-RV-080211-05	TYPE 4- REVEAL - EX-RV-080211-18

PLAN NORTH
TRUE NORTH

James R. Childers Architect, Inc.
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PROFESSIONAL SEAL:
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SCOTT L HOWARD
REGISTERED LANDSCAPE ARCHITECT
#155
[illegible]
12/09/22

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CLIENT:
SEAL OF THE CHEROKEE NATION
SEPT. 6, 1839

CHEROKEE NATION REPLACEMENT HOSPITAL

TAHLEQUAH, OKLAHOMA

KEY PLAN:

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS		
#	DATE	DESCRIPTION

JOB NUMBER: 21-08.21

DATE: 12-09-2022

SHEET NUMBER: **HA1.2**

SHEET TITLE: ENLARGED HARDSCAPE PLAN - SOUTH

MATCHLINE SHEET HA1.4
MATCHLINE SHEET HA1.3

FIXED STAINLESS STEEL BOLLARD
REFER TO DETAIL 5/HA2.1

FIXED STAINLESS STEEL BOLLARD
REFER TO DETAIL 8/HA2.1

30"- 36" TALL
FIBERGLASS
PLANTERS
REFER TO DETAIL
6/HA2.1

CONNECT 4"
SCHEDULE PVC
DRAIN PIPES
TO STORM DRAIN AS
PER CITY CODES AND
STANDARDS.

LONG TERM
BICYCLE
RACK
IN COVERED
AREA (2)

CONCRETE PAVING
REFER TO CIVIL
DRAWINGS.

WOOD/
METAL BENCHES

DECORATIVE
CONCRETE
BAND PAVING TYPE 4
REFER TO DETAILS
2 AND 3/HA2.1

TEXTURED DECORATIVE
PAVING TYPE 1.
REFER TO DETAILS
2 AND 3/HA2.1

DECORATIVE
CONCRETE PAVING
TYPE 2.
REFER TO DETAILS
2 AND 3/HA2.1

4" SCHEDULE 40 PVC
DRAINAGE PIPE FOR
PLANTERS.

30"-36" TALL
WEATHERED
FIBERGLASS
PLANTERS
REFER TO DETAIL
6/HA2.1

WOOD/
METAL BENCH

WOOD/METAL
BENCHES

30"-36" TALL
WEATHERED
FIBERGLASS
PLANTERS
REFER TO DETAIL
9/HA2.1

LONG TERM BICYCLE
RACKS IN
COVERED AREA (4)

DECORATIVE CONCRETE
BAND
REFER TO DETAILS
2 AND 3/HA2.1

FIXED STAINLESS
STEEL BOLLARD
REFER TO DETAIL
5/HA2.1

DECORATIVE CONCRETE
BAND PAVING TYPE 4.
REFER TO DETAILS
4 AND 5/HA2.1.

TEXTURED DECORATIVE
PAVING TYPE 1.
REFER TO DETAILS
2 AND 3/HA2.1

LONG TERM BICYCLE RACKS IN
COVERED AREA (9)

CIRCULAR
PLANTER.
REFER TO
DETAIL
6/LA2.1.

CIRCULAR
PLANTER
REFER TO
DETAIL
6/LA2.1

MATCHLINE SHEET HA1.3
MATCHLINE SHEET HA1.2

A **ENLARGED HARDSCAPE PLAN - CENTRAL**
1" = 40'-0"
0 10' 20' 30'

PAVING LEGEND

EXPANSION JOINT

SAW JOINT

BOMANITE SPECIALTY PAVING
(NO SUBSTITUTIONS)

TYPE 1- SANDSCAPE COLOR BLEND
EX-SSR-110-112-14 & EX-SSR-110-112-13

TYPE 2- REVEAL - EX-RV-080211-05

TYPE 3- REVEAL - EX-RV-080211-16

TYPE 4- REVEAL - EX-RV-080211-18

PLAN NORTH

TRUE NORTH

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PROFESSIONAL SEAL:

STATE OF OKLAHOMA
REGISTERED LANDSCAPE ARCHITECT
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12/09/22

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CLIENT:

THE CHEROKEE NATION
SEAL OF THE
SEPT. 6, 1839

CHEROKEE NATION REPLACEMENT HOSPITAL

TAHLEQUAH, OKLAHOMA

KEY PLAN:

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

#	DATE	DESCRIPTION

JOB NUMBER: 21-08.21

DATE: 12-09-2022

SHEET NUMBER:
HA1.3

SHEET TITLE:
ENLARGED HARDSCAPE PLAN - CENTRAL

A ENLARGED HARDSCAPE PLAN - NORTH

1" = 40'-0"

0 10' 20' 30'

PAVING LEGEND

EXPANSION JOINT

SAW JOINT

BOMANITE SPECIALTY PAVING (NO SUBSTITUTIONS)	
TYPE 1-	SANDSCAPE COLOR BLEND EX-SSR-110-112-14 & EX-SSR-110-112-13
TYPE 2-	REVEAL - EX-RV-080211-05
TYPE 3-	REVEAL - EX-RV-080211-16
TYPE 4-	REVEAL - EX-RV-080211-18

SITE FURNISHINGS SCHEDULE

SYMBOL	DESCRIPTION	QTY. TOTAL	SIZE/FEATURES	MANUFACTURER	FINISH/COLOR	REMARKS
	CHIPMAN STYLE ROUND TABLE AND CHIPMAN ARMLESS CHAIRS	(3) TOTAL TABLES (12) TOTAL CHAIRS	45" ROUND, 29.25" HEIGHT CHIPMAN TABLE AND 22" X 34" X 33" CHIPMAN CHAIRS. SINGLE EMBEDDED SUPPORT MOUNT.	LANDSCAPE FORMS 800.430.6209 7800 E. MICHIGAN AVE. KALAMAZOO, MI. 49048	POWDERCOAT METAL (METALLIC), COLOR: STONE	SURFACE MOUNT AND ANCHOR IN GROUND. INSTALL AS PER MANUFACTURER'S RECOMMENDATIONS
	MELVILLE BENCH WITH BACK	(14) TOTAL	19" W X 76" L X 30" H WOOD SEATED BENCH WITH BACK	LANDSCAPE FORMS 800.430.6209 7800 E. MICHIGAN AVE. KALAMAZOO, MI. 49048	POWDERCOAT METAL (METALLIC), COLOR: STONE WITH JARRAH WOOD	SURFACE MOUNT AND ANCHOR IN GROUND. INSTALL AS PER MANUFACTURER'S RECOMMENDATIONS
	POE LITTER RECEPTACLE	(8) TOTAL	29" W x 44" H, 34 Gal. SIDE OPENING	LANDSCAPE FORMS 800.430.6209 7800 E. MICHIGAN AVE. KALAMAZOO, MI. 49048	POWDERCOAT METAL (METALLIC) COLOR: STONE	SURFACE MOUNT (8) AND ANCHOR IN GROUND. INSTALL AS PER MANUFACTURER'S RECOMMENDATIONS
	RIDE BICYCLE RACK	(25) TOTAL	RIDE BICYCLE RACK 4" X28" X 26"	LANDSCAPE FORMS 800.430.6209 7800 E. MICHIGAN AVE. KALAMAZOO, MI. 49048	POWDERCOAT METAL (METALLIC) COLOR: STONE	ANCHOR IN GROUND AS AND INSTALL AS PER MANUFACTURER'S RECOMMENDATIONS
	ILLUMINATED FIXED BOLLARD	(27) TOTAL	3' HEIGHT X 8" WIDTH	ATKORE CALPIPE SECURITY BOLLARDS (877) 277-8518 250 CAPITAL DRIVE DR. HEBRON, OH. 43025	STAINLESS STEEL LED GLOBE STYLE 23 MAX 120V	EMBEDDED CONCRETE FOOTING IN GROUND AND INSTALL AS PER MANUFACTURER'S RECOMMENDATIONS
	FIXED BOLLARD	(43) TOTAL	3' HEIGHT X 8" WIDTH	ATKORE CALPIPE SECURITY BOLLARDS (877) 277-8518 250 CAPITAL DRIVE DR. HEBRON, OH. 43025	SSF080 SCHEDULE 40 STAINLESS STEEL	EMBEDDED CONCRETE FOOTING IN GROUND AND INSTALL AS PER MANUFACTURER'S RECOMMENDATIONS
	MODERN ROUND PLANTER	(8) TOTAL	24" DIA.	PLANTERS UNLIMITED 888.320.0626	FIBERGLASS COLOR: CHAPS BROWN: MATTE FINISH	INSTALL AS PER MANUFACTURER'S RECOMMENDATIONS
	MODERN SQUARE PLANTER	(12) TOTAL	72" L 36" W X 36" H	PLANTERS UNLIMITED 888.320.0626	FIBERGLASS COLOR: CHAPS BROWN: MATTE FINISH	INSTALL AS PER MANUFACTURER'S RECOMMENDATIONS
	MODERN SQUARE PLANTER	(29) TOTAL	84" L X 30" W X 30" H	PLANTERS UNLIMITED 888.320.0626	FIBERGLASS COLOR: CHAPS BROWN: MATTE FINISH	INSTALL AS PER MANUFACTURER'S RECOMMENDATIONS

James R. Childers Architect, Inc.
45 South 4th Street
Fort Smith, AR 72901
479-783-2480
www.childersarchitect.com

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CONSULTANT LOGO:

hfsd landscape architecture
3100 NW 149TH ST.
OKLAHOMA CITY, OKLAHOMA 73134
VOICE: 405.752.8018
FAX: 405.752.8701

CLIENT:

CHEROKEE NATION
REPLACEMENT HOSPITAL
TAHLEQUAH, OKLAHOMA

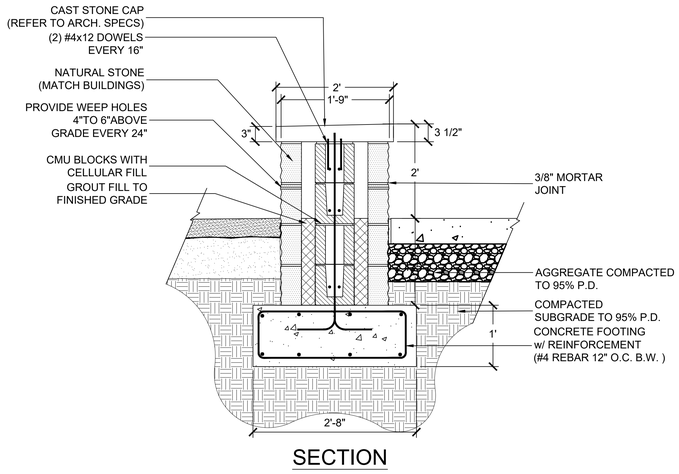
KEY PLAN:

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

#	DATE	DESCRIPTION

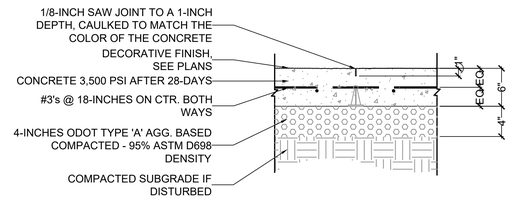
JOB NUMBER: 21-08.21
DATE: 12-09-2022
SHEET NUMBER: **HA1.4**
SHEET TITLE: ENLARGED HARDSCAPE PLAN - NORTH



SECTION

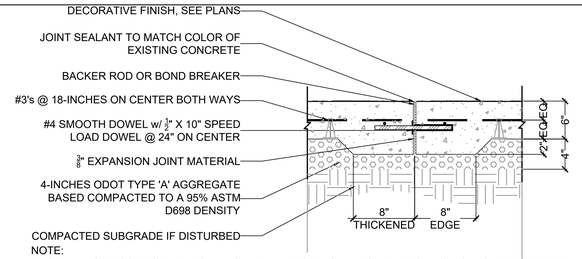
1 STONE SEATWALL AT PLAZA

1" = 1'-0"



2 SAW JOINT - INTEGRAL SANDSCAPE FINISH CONCRETE

1" = 1'-0"



NOTE:
1. SPEED LOAD COMPONENT DOWEL SLEEVE: BY SIKA GREENSTREAK, ST. LOUIS, MO 1.800.325.9504
2. SMOOTH DOWEL AND STEEL REINFORCING: A 615. REINFORCING CLEAR:1-1/2" FROM OTHER CONCRETE EDGE. REINFORCING: 2" CLEAR FROM CONCRETE AND AGGREGATE OR SOIL.

3 CONCRETE EXPANSION JOINT - INTEGRAL SANDSCAPE FINISH CONCRETE

1 1/2" = 1'-0"

NOTES:

BOLLARD MATERIAL: 8" TUBE SIZE STEEL
36" STAINLESS STEEL
WITH 1/2" WALL INSERT

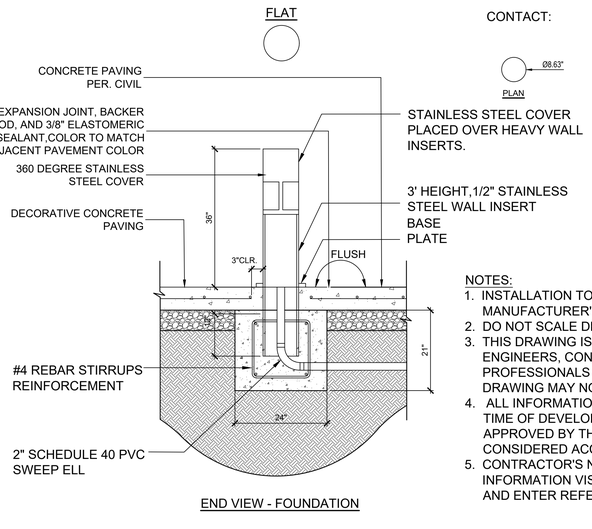
BOLLARD CAP STYLE: STANDARD FLAT

BOLLARD COVER STYLE: 360 DEGREE

GLOBE STYLE: GLASS- LED 23 W MAX (120 VAC ONLY)

TRANSFORMER: 120V

CONTACT: ATKORE CALPIPE
SECURITY BOLLARDS
250 CAPITAL DRIVE DR.
HEBRON, OH. 43025
TOLL FREE: 1-877-283-8518
PHONE: (562) 803-4388
FAX: (562) 803-9883
www. calpipebollards.com

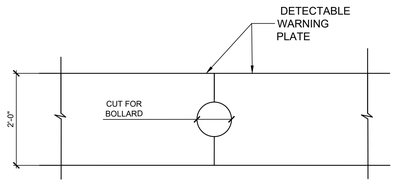


END VIEW - FOUNDATION

NOTES:
1. INSTALLATION TO BE COMPLETED IN ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS.
2. DO NOT SCALE DRAWING.
3. THIS DRAWING IS INTENDED FOR USE BY ARCHITECTS, ENGINEERS, CONTRACTORS, CONSULTANTS AND DESIGN PROFESSIONALS FOR PLANNING PURPOSES ONLY. THIS DRAWING MAY NOT BE USED FOR CONSTRUCTION.
4. ALL INFORMATION CONTAINED HEREIN WAS CURRENT AT THE TIME OF DEVELOPMENT BUT MUST BE REVIEWED AND APPROVED BY THE PRODUCT MANUFACTURER TO BE CONSIDERED ACCURATE.
5. CONTRACTOR'S NOTE: FOR PRODUCT AND COMPANY INFORMATION VISIT www.CADdetails.com/info AND ENTER REFERENCE NUMBER Habase_Hardscape Sheets.

4 LIGHTED BOLLARD AT ENTRY DRIVE

3/4" = 1'-0"



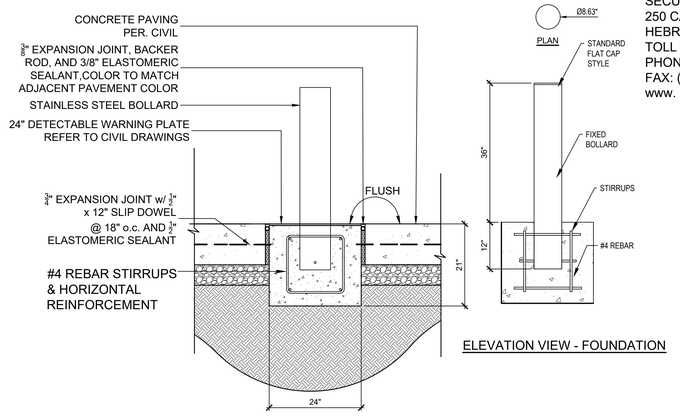
NOTES:

BOLLARD MATERIAL: STEEL

FINISH: SSF08040 SCH 40
STAINLESS STEEL

CAP STYLE: STANDARD FLAT

CONTACT: ATKORE CALPIPE
SECURITY BOLLARDS
250 CAPITAL DRIVE DR.
HEBRON, OH. 43025
TOLL FREE: 1-877-283-8518
PHONE: (562) 803-4388
FAX: (562) 803-9883
www. calpipebollards.com



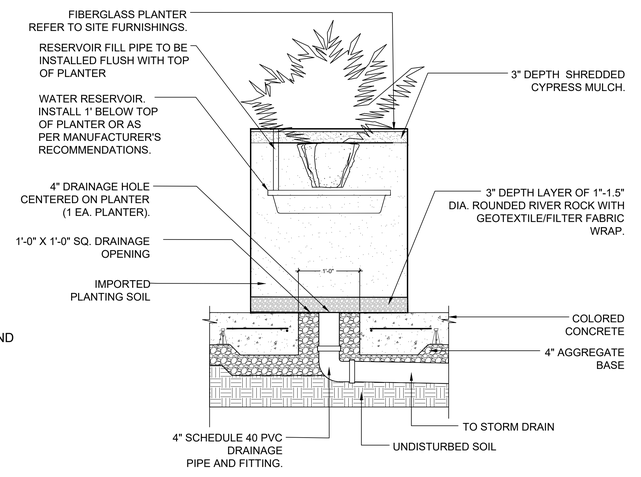
END VIEW - FOUNDATION

ELEVATION VIEW - FOUNDATION

NOTES:
1. INSTALLATION TO BE COMPLETED IN ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS.
2. DO NOT SCALE DRAWING.
3. THIS DRAWING IS INTENDED FOR USE BY ARCHITECTS, ENGINEERS, CONTRACTORS, CONSULTANTS AND DESIGN PROFESSIONALS FOR PLANNING PURPOSES ONLY. THIS DRAWING MAY NOT BE USED FOR CONSTRUCTION.
4. ALL INFORMATION CONTAINED HEREIN WAS CURRENT AT THE TIME OF DEVELOPMENT BUT MUST BE REVIEWED AND APPROVED BY THE PRODUCT MANUFACTURER TO BE CONSIDERED ACCURATE.
5. CONTRACTOR'S NOTE: FOR PRODUCT AND COMPANY INFORMATION VISIT www.CADdetails.com/info AND ENTER REFERENCE NUMBER Habase_Hardscape Sheets.

5 FIXED BOLLARD AT TACTILE SURFACE

3/4" = 1'-0"



6 FIBERGLASS PLANTER AT PLAZA

1" = 1'-0"

James R. Childers Architect, Inc.
45 South 4th Street
Fort Smith, AR 72901
479-783-2480
www.childersarchitect.com

PROFESSIONAL SEAL:

CONSULTANT LOGO:

hfsd landscape architecture
3100 NW 149TH ST.
OKLAHOMA CITY, OKLAHOMA 73134
VOICE: 405.752.8018
FAX: 405.752.8701

CLIENT:

CHEROKEE NATION
REPLACEMENT HOSPITAL
TAHLEQUAH, OKLAHOMA

KEY PLAN:

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

#	DATE	DESCRIPTION

JOB NUMBER: 21-08.21

DATE: 12-09-2022

SHEET NUMBER: **HA2.1**

SHEET TITLE: HARDSCAPE DETAILS

CHEROKEE NATION
REPLACEMENT HOSPITAL

VOLUME 04 OF 11
(STRUCTURAL)

PARKHILL
4101 WIRELESS WAY, SUITE 350
OKLAHOMA CITY, OK 73134
(405) 832-9900
CIVIL ENGINEERING

PALMERTON & PARRISH, INC.
4168 W. KEARNY ST.
SPRINGFIELD, MO 65803
(417) 864-6000
GEOTECHNICAL ENGINEERING

FEC HELIPORTS
5228 RIVER ROAD
CINCINNATI, OH 45233
(877) HELIPAD
HELIPAD DESIGN

HOWARD-FAIRBAIRN SITE DESIGN
3100 NW 149TH ST
OKLAHOMA CITY, OK 73134
(405) 752-8018
LANDSCAPE ARCHITECTURE

HKS
999 18th St, #2255N
DENVER, CO 80202
(303) 293-2903
INTERIOR ARCHITECTURE | INTERIOR DESIGN

FOY CONSULTING & ENGINEERING, LLC
6900 COLLEGE BLVD #600
OVERLAND, KS 66211
(913) 312-0075
STRUCTURAL ENGINEER

HP ENGINEERING, INC.
5504 PINNACLE POINTE DRIVE, SUITE 200
ROGERS, AR 72758
(479) 899-6370
MECHANICAL | ELECTRICAL | PLUMBING

INTERIOR LOGISTICS
1316 E 35TH PLACE, SUITE 100
TULSA, OK 74105
(918) 382-9120
EQUIPMENT | FURNITURE PLANING

MOBLEY FIRE PROTECTION
6242 LLANO AVE.
DALLAS, TX 75214
(817) 614-2361
FIRE PROTECTION | LIFE SAFETY

FDP
2655 VILLA CREEK DR. #233
DALLAS, TX 75234
(972) 245-5300
FOOD SERVICE

ECOWORKS STUDIO
659 AUBURN AVE. NE, SUITE 135
ATLANTA, GA 30312
(404) 480-4499
LEED CONSULTING

CRUX TECHNOLOGY + SECURITY SOLUTIONS
401 SOUTH BOSTON AVE, SUITE 500-15
TULSA, OK 74103
(800) 685-6440
LOW-VOLTAGE | IT | SECURITY

DIRECT 2 COMPLETION
7134 S. YALE AVE #400
TULSA, OK 74136
(918) 457-6905
OWNERS REPRESENTATIVE

FOREMAN MANHATTAN CONSTRUCTION TEAM
5601 S 122nd E AVE
TULSA, OK 74146
(918) 878-3492
CONSTRUCTION MANAGER

James R. Childers
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45 South 4th Street
Fort Smith, AR 72901
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www.childersarchitect.com

PROFESSIONAL SEAL:

STATE OF OKLAHOMA
J. BRECK CHILDERS
9120
12/09/2022
REGISTERED ARCHITECT

CONSULTANT LOGO:

CLIENT:

CHEROKEE NATION
REPLACEMENT HOSPITAL
TAHLEQUAH, OKLAHOMA

KEY PLAN:

PROJECT PHASE:

REVISIONS		
#	DATE	DESCRIPTION

JOB NUMBER: 21-08.21
DATE: 12-09-2022
SHEET NUMBER: **VOL 04**
SHEET TITLE: COVER SHEET

INDEX OF DRAWINGS - VOLUME 01 - CIVIL

SHEET NUMBER	SHEET NAME	BID PACKAGE 01 - 07-29-2022	ADDENDUM 01 - 08-17-2022	BID PACKAGE 03 - 10-07-2022	BID PACKAGE 02 - 10-21-2022	ASI 01 - 10-24-2022	ADDENDUM 02 - 11-18-2022	BID PACKAGE 04 - 12-09-2022
GENERAL								
VOL 01	COVER SHEET	■			■		■	■
G0.01	DRAWING INDEX	■		■	■		■	■
CIVIL								
C0-101	GENERAL CIVIL NOTES -BP2				■		■	
C0-102	GENERAL CIVIL NOTES - BP1	■						
C0-103	GENERAL CIVIL NOTES - BP4							■
C0-901	PROJECT MAP							■
C1-101	EXISTING CONDITIONS				■			
C1-102	EXISTING CONDITIONS				■			
C1-103	EXISTING CONDITIONS	■						
C1-104	EXISTING CONDITIONS	■						
C1-105	EXISTING CONDITIONS	■						
C1-106	EXISTING CONDITIONS				■			
C1-107	EXISTING CONDITIONS				■			
C2-101	OVERALL DEMOLITION VIEW				■			
C2-102	A1 DEMOLITION PLAN				■			
C2-103	A2 DEMOLITION PLAN				■			
C2-104	B1 DEMOLITION PLAN				■			
C2-105	B2 DEMOLITION PLAN				■			
C2-106	C1 DEMOLITION PLAN				■			
C2-107	C2 DEMOLITION PLAN				■			
C2-108	NP DEMOLITION OVERVIEW	■						
C2-109	NP 1 DEMOLITION PLAN	■						
C2-110	NP2 DEMOLITION PLAN	■						
C2-111	NP3 DEMOLITION PLAN	■						
C2-112	NP4 DEMOLITION PLAN	■						
C2-113	NP5 DEMOLITION PLAN	■						
C2-701	EROSION CONTROL PLAN - HOSPITAL				■			
C2-702	NP EROSION CONTROL PLAN	■						
C2-750	EROSION CONTROL DETAILS	■						
C2-751	EROSION CONTROL DETAILS				■			
C3-101	OVERALL SITE VIEW							■
C3-102	A1 SITE PLAN							■
C3-103	A2 SITE PLAN							■
C3-104	B1 SITE PLAN							■
C3-105	B2 SITE PLAN							■
C3-106	C1 SITE PLAN							■
C3-107	C2 SITE PLAN							■
C3-108	NP SITE PLAN OVERVIEW	■	■					
C3-109	NP1 SITE PLAN	■	■					
C3-110	NP2 SITE PLAN	■	■					
C3-111	NP3 SITE PLAN	■	■					
C3-112	NP4 SITE PLAN	■	■					
C3-113	NP5 SITE PLAN	■	■					
C3-501	SITE DETAILS	■						
C3-502	SITE DETAILS	■						
C3-503	CONCRETE PAVING JOINT DETAILS	■						
C3-504	SITE DETAILS							■
C3-901	PARKING SPACE ACCOUNTING							■
C4-101	OVERALL UTILITY VIEW				■		■	
C4-102	A1 UTILITIES				■			
C4-103	A2 UTILITIES				■		■	
C4-104	B1 UTILITIES				■			
C4-105	B2 UTILITIES				■			
C4-106	C1 UTILITIES				■			
C4-107	C2 UTILITIES				■			
C4-201	LREC RELOCATION PLAN AND PROFILE					■		
C4-203	PHASE 1 UTILITY PLAN						■	
C4-801	SITE ELECTRIC AND COMMUNICATION CONDUITS				■			
C5-101	OVERALL GRADING VIEW							■
C5-102	A1 GRADING PLAN							■
C5-103	A2 GRADING PLAN							■
C5-104	B1 GRADING PLAN							■
C5-105	B2 GRADING PLAN							■
C5-106	C1 GRADING PLAN							■
C5-107	C2 GRADING PLAN							■
C5-113	NP GRADING OVERVIEW	■						
C5-114	NP1 GRADING PLAN	■						
C5-115	NP2 GRADING PLAN	■						
C5-116	NP3 GRADING PLAN	■						
C5-117	NP4 GRADING PLAN	■						
C5-118	NP5 GRADING PLAN	■						
C5-201	COORDINATES AND ELEVATIONS							■
C5-202	LOOP DRIVE PLAN AND PROFILE							■
C5-203	LOOP DRIVE PLAN AND PROFILE							■
C5-204	LOOP DRIVE PLAN AND PROFILE							■
C5-205	WEST DRIVE PLAN AND PROFILE							■
C5-206	WEST DRIVE PLAN AND PROFILE							■
C5-207	GABION WALL PLAN AND PROFILE							■
C5-208	BLOCK WALL PLAN AND PROFILE							■
C5-210	NP COORDINATES AND ELEVATIONS TABLE	■						
C5-501	GABION WALL DETAILS							■
C5-502	MASONRY BLOCK WALL DETAILS							■
C5-801	EARTWORK ANALYSIS							■
C6-101	STORM DRAIN PLAN				■		■	
C6-102	STORM DRAIN PLAN				■		■	
C6-103	ROOF DRAIN PLAN				■			
C6-104	FOUNDATION DRAIN PLAN				■			
C6-201	NP STORM PLAN & PROFILE	■						
C6-202	NP STORM PLAN & PROFILE	■						
C6-203	STORM DRAIN PROFILES				■		■	
C6-204	STORM DRAIN PROFILES				■		■	
C6-501	STORM DRAIN DETAILS	■						
C6-502	STORM DRAIN DETAILS	■						
C6-503	STORM DRAIN DETAILS	■						
C6-504	STORM DRAIN DETAILS	■						
C6-505	STORM DRAIN DETAILS	■						
C6-506	CICI BOX				■			
C6-507	CICI FRAME				■			
C6-508	CICI GRATE				■			
C6-509	DESIGN 6 AND 7 AREA INLET				■			
C6-510	GRATE AND FRAME FOR AREA INLET				■			
C6-511	STORM DRAIN DETAILS				■			
C6-802	NP EXISTING HYDROLOGY	■						
C6-803	NP DEVELOPMENT HYDROLOGY	■						
C7-101	OVERALL WATER PLAN				■			
C7-201	WATER 1 PLAN AND PROFILE				■			
C7-202	WATER 1 PLAN AND PROFILE				■			
C7-203	WATER 1 PLAN AND PROFILE				■			
C7-204	WATER 2 PLAN AND PROFILE				■			
C7-205	WATER 2 PLAN AND PROFILE				■			
C7-206	WATER 2 PLAN AND PROFILE				■		■	
C7-207	WATER 2 PLAN AND PROFILE				■		■	
C7-208	WATER 3 PLAN AND PROFILE				■			
C7-209	WATER 3 PLAN AND PROFILE				■			
C7-210	WATER 4 PLAN AND PROFILE				■		■	
C7-211	WATER 5 AND 14 PLAN AND PROFILE				■			
C7-212	WATER 6, 9, AND 10 PLAN AND PROFILE				■		■	
C7-213	WATER 7, 8, AND 13 PLAN AND PROFILE				■		■	
C7-214	WATER 11 AND 12 PLAN AND PROFILE				■		■	
C7-501	TPWA WATER DETAILS				■			
C7-502	WATER DETAILS				■			
C8-201	FORCE MAIN PLAN & PROFILE	■						
C8-501	FORCE MAIN DETAILS	■						
C9-201	DOWNING DRIVEWAY PLAN AND PROFILE	■	■					
C9-501	DOWNING DRIVEWAY DETAILS	■						

Grand total: 125

INDEX OF DRAWINGS - VOLUME 02 - LANDSCAPE

SHEET NUMBER	SHEET NAME	BID PACKAGE 01 - 07-29-2022	ADDENDUM 01 - 08-17-2022	BID PACKAGE 03 - 10-07-2022	BID PACKAGE 02 - 10-21-2022	ASI 01 - 10-24-2022	ADDENDUM 02 - 11-18-2022	BID PACKAGE 04 - 12-09-2022
GENERAL								
VOL 02	COVER SHEET							■
G0.01	DRAWING INDEX	■	■	■	■	■	■	■
LANDSCAPE								
HA1.1	SITE HARDSCAPE PLAN							■
HA1.2	ENLARGED HARDSCAPE PLAN - SOUTH							■
HA1.3	ENLARGED HARDSCAPE PLAN - CENTRAL							■
HA1.4	ENLARGED HARDSCAPE PLAN - NORTH							■
HA2.1	HARDSCAPE DETAILS							■

Grand total: 7

INDEX OF DRAWINGS - VOLUME 04 - STRUCTURAL

SHEET NUMBER	SHEET NAME	BID PACKAGE 01 - 07-29-2022	ADDENDUM 01 - 08-17-2022	BID PACKAGE 03 - 10-07-2022	BID PACKAGE 02 - 10-21-2022	ASI 01 - 10-24-2022	ADDENDUM 02 - 11-18-2022	BID PACKAGE 04 - 12-09-2022
GENERAL								
VOL 04	COVER SHEET							■
G0.01	DRAWING INDEX	■	■	■	■	■	■	■
STRUCTURAL								
S0.1	COVER SHEET							■
S0.2	GENERAL NOTES							■
S0.3	GENERAL STRUCTURAL NOTES							■
S0.4	SPECIAL INSPECTIONS TABLE							■
S1.01.1	01 OVERALL FOUNDATION PLAN							■
S1.01.2	01 SLAB PLAN							■
S1.01.3	ELEVATOR PIT PLAN							■
S1.01.4	DRILLED PIER PLAN							■
S2.01.1	FOUNDATION PLAN SECTOR 01							■
S2.01.2	FOUNDATION PLAN SECTOR 02							■
S2.01.3	FOUNDATION PLAN SECTOR 03							■
S2.01.4	FOUNDATION PLAN SECTOR 04							■
S2.01.5	FOUNDATION PLAN SECTOR 05							■
S4.01	CONCRETE TYPICAL DETAILS							■
S4.02	CONCRETE TYPICAL DETAILS							■
S4.03	FOUNDATION DETAILS							■
S4.05	BASE PLATE DETAILS							■
S4.06	FOUNDATION SECTIONS							■
S4.07	FOUNDATION SECTIONS							■
S4.08	FOUNDATION SECTIONS							■
S4.09	FOUNDATION SECTIONS							■
SB101	SKYBRIDGE - ENLARGEMENTS							■
SB102	SKYBRIDGE - SECTIONS							■
SC100	SOUTH CANOPY							■
SC101	SOUTH CANOPY - FOUNDATION PLAN							■
SC103	SOUTH CANOPY - FRAMES							■
SC104	SOUTH CANOPY - DETAILS							■
SE101	EAST CANOPY - STRUCTURAL PLAN							■
SM101	MECHANICAL AREA - OVERALL FOUNDATION PLAN							■
SM111	MECHANICAL AREA (BRIDGE) - ENLARGEMENTS							■
SM112	MECHANICAL AREA (BRIDGE) - SECTIONS							■
SM113	MECHANICAL AREA - FOUNDATION DETAILS							■
SM114	MECHANICAL AREA - ANCHOR RODS							■

Grand total: 35

INDEX OF DRAWINGS - VOLUME 07 - ELECTRICAL

SHEET NUMBER	SHEET NAME	BID PACKAGE 01 - 07-29-2022	ADDENDUM 01 - 08-17-2022	BID PACKAGE 03 - 10-07-2022	BID PACKAGE 02 - 10-21-2022	ASI 01 - 10-24-2022	ADDENDUM 02 - 11-18-2022	BID PACKAGE 04 - 12-09-2022
GENERAL								
VOL 07	COVER SHEET	■						
G0.01	DRAWING INDEX	■	■	■	■	■	■	■
ELECTRICAL								
ENL.00	ELECTRICAL LEGEND	■						
ENL.01	ELECTRICAL SITE PLAN - NORTH LOT	■						

Grand total: 4

INDEX OF DRAWINGS - VOLUME 09 -TECHNOLOGY

SHEET NUMBER	SHEET NAME	BID PACKAGE 01 - 07-29-2022	ADDENDUM 01 - 08-17-2022	BID PACKAGE 03 - 10-07-2022	BID PACKAGE 02 - 10-21-2022	ASI 01 - 10-24-2022	ADDENDUM 02 - 11-18-2022	BID PACKAGE 04 - 12-09-2022
GENERAL								
VOL 09	COVER SHEET	■						
G0.01	DRAWING INDEX	■	■	■	■	■	■	■
TECHNOLOGY								
TN51.02	NETWORK SITE PLAN - NORTH PARKING & ACCESS	■	■					

Grand total: 3

James R. Childers Architect, Inc.
45 South 4th Street
Fort Smith, AR 72901
479-783-2480
www.childersarchitect.com

PROFESSIONAL SEAL:

STATE OF OKLAHOMA — J. BRECK CHILDERS — 8120 — 12/09/2022 — REGISTERED ARCHITECT

CONSULTANT LOGO:

CLIENT:

SEAL OF THE CHEROKEE NATION — SEPT. 6, 1839

CHEROKEE NATION REPLACEMENT HOSPITAL

TAHLEQUAH, OKLAHOMA

KEY PLAN:

PROJECT PHASE:

BID PACKAGE 04

(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

#	DATE	DESCRIPTION
1	08-17-22	ADDENDUM 01
2	10-07-22	BID PACKAGE 03
3	10-21-22	BID PACKAGE 02
4	10-24-22	ASI 01
5	11-18-22	ADDENDUM 02
6	12-09-22	BID PACKAGE 04

JOB NUMBER: 21-08.21

DATE: 07-29-2022

SHEET NUMBER: **G0.01**

SHEET TITLE: DRAWING INDEX

1" = 1'-0" 3" = 1'-0" 1 1/2" = 1'-0" 1" = 1'-0" 3/4" = 1'-0" 1/2" = 1'-0" 3/8" = 1'-0" 1/4" = 1'-0" 1/8" = 1'-0"

DRAWING	DESCRIPTION
S0.1	COVER SHEET
S0.2	GENERAL NOTES
S0.3	GENERAL STRUCTURAL NOTES
S0.4	SPECIAL INSPECTIONS TABLE

DRAWING	DESCRIPTION
S1.00.1	WIND ON CLADDINGS AND SNOW DRIFT DIAGRAMS
S1.00.2	OVERALL GRID PLAN
S1.01.1	01 OVERALL FOUNDATION PLAN
S1.01.2	01 SLAB PLAN
S1.01.3	ELEVATOR PIT PLAN
S1.01.4	DRILLED PIER PLAN
S1.02.1	02 OVERALL FRAMING PLAN
S1.02.2	02 SLAB PLAN
S1.02.3	02 LOADING PLAN
S1.03.1	03 OVERALL FRAMING PLAN
S1.03.2	03 SLAB PLAN
S1.03.3	03 LOADING PLAN
S1.04.1	04 OVERALL FRAMING PLAN
S1.04.2	04 SLAB PLAN
S1.04.3	04 LOADING PLAN
S1.05.1	05 OVERALL FRAMING PLAN
S1.05.2	05 SLAB PLAN
S1.05.3	05 LOADING PLAN
S1.06.1	06 & ROOF OVERALL FRAMING PLAN
S1.06.2	06 & ROOF SLAB PLAN
S1.06.3	06 & ROOF LOADING PLANS

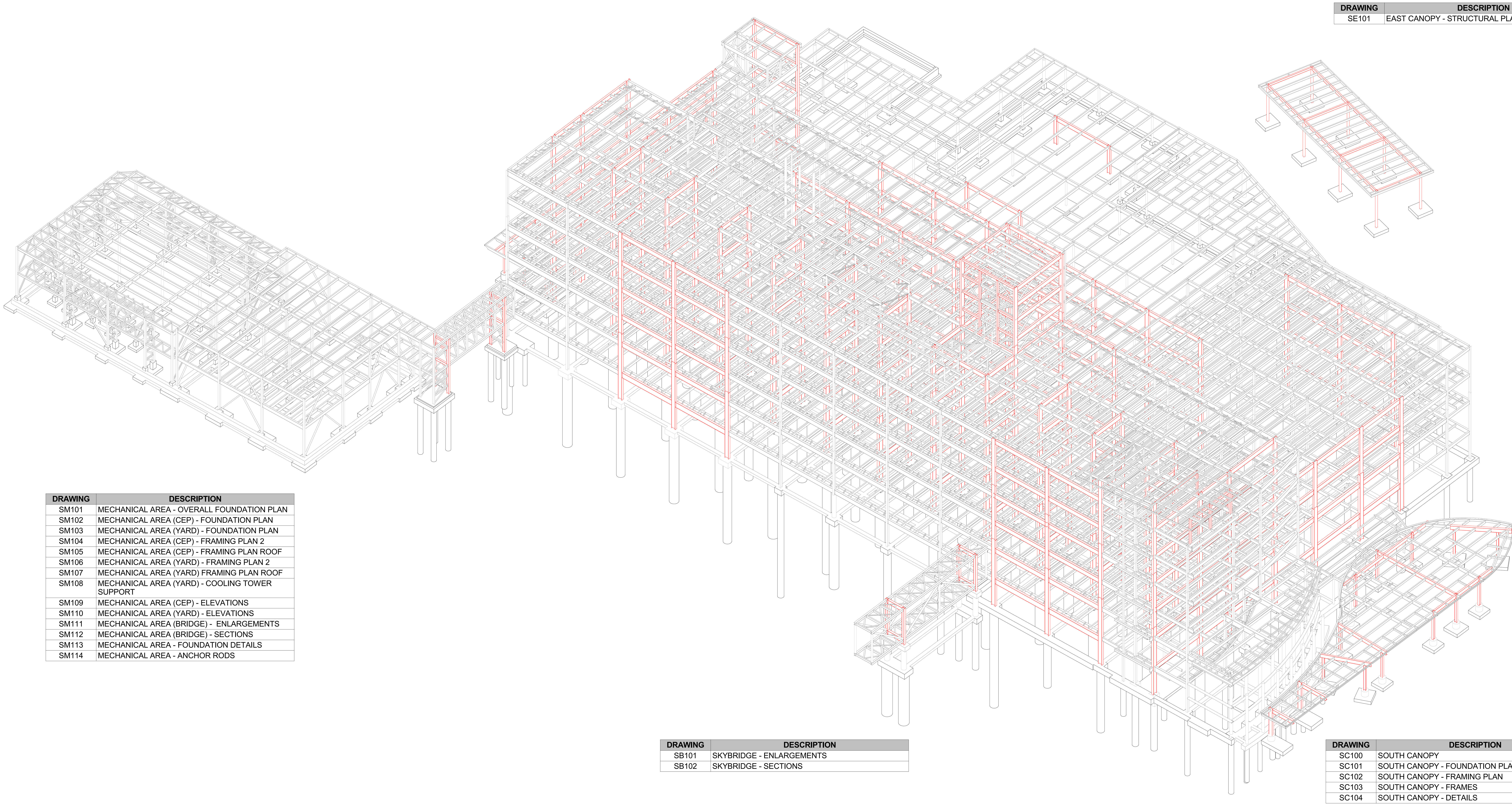
DRAWING	DESCRIPTION
S2.01.1	FOUNDATION PLAN SECTOR 01
S2.01.2	FOUNDATION PLAN SECTOR 02
S2.01.3	FOUNDATION PLAN SECTOR 03
S2.01.4	FOUNDATION PLAN SECTOR 04
S2.01.5	FOUNDATION PLAN SECTOR 05
S2.02.1	02 FRAMING PLAN SECTOR 01
S2.02.2	02 FRAMING PLAN SECTOR 02
S2.02.3	02 FRAMING PLAN SECTOR 03
S2.02.4	02 FRAMING PLAN SECTOR 04
S2.02.5	02 FRAMING PLAN SECTOR 05
S2.03.1	03 FRAMING PLAN SECTOR 01
S2.03.2	03 FRAMING PLAN SECTOR 02
S2.03.3	03 FRAMING PLAN SECTORS 03 & 04
S2.04.1	04 FRAMING PLAN SECTOR 01
S2.04.2	04 FRAMING PLAN SECTOR 02
S2.04.3	04 FRAMING PLAN SECTORS 03 & 04
S2.05.1	05 FRAMING PLAN SECTOR 01
S2.05.2	05 FRAMING PLAN SECTOR 02
S2.05.3	05 FRAMING PLAN SECTORS 03 & 04
S2.06.1	06 FRAMING PLAN SECTOR 01
S2.06.2	06 FRAMING PLAN - SECTORS 02 & 03
S2.07.1	ROOF FRAMING PLAN - SECTOR 01
S2.07.2	ROOF FRAMING PLAN - SECTORS 02 & 03
S2.08.1	STORAGE PENTHOUSE
S2.08.2	HELIPAD
S2.09	ELEVATOR HOUSE
S2.10.1	FRAMING ENLARGEMENTS - ATRIUM

DRAWING	DESCRIPTION
S3.01	RIGID FRAMES 1 & 2
S3.02	RIGID FRAMES 3 & 4
S3.03	RIGID FRAMES 6, 7, 8, & 11
S3.04	RIGID FRAMES B, C, D & F
S3.05	RIGID FRAMES E, G & H
S3.06	RIGID FRAMES J, K & L
S3.07	RIGID FRAMES N, P, Q & S
S3.08	FRAMES 5, 9, 12, R & A

DRAWING	DESCRIPTION
S4.01	CONCRETE TYPICAL DETAILS
S4.02	CONCRETE TYPICAL DETAILS
S4.03	FOUNDATION DETAILS
S4.04	BASE PLATE DETAILS
S4.05	BASE PLATE DETAILS
S4.06	FOUNDATION SECTIONS
S4.07	FOUNDATION SECTIONS
S4.08	FOUNDATION SECTIONS
S4.09	FOUNDATION SECTIONS

DRAWING	DESCRIPTION
S5.01	STEEL TYPICAL DETAILS
S5.02	STEEL TYPICAL DETAILS
S5.03	MASONRY TYPICAL DETAILS
S5.04	TYPICAL FRAMING DETAILS
S5.05	FRAMING DETAILS

DRAWING	DESCRIPTION
SE101	EAST CANOPY - STRUCTURAL PLAN



DRAWING	DESCRIPTION
SM101	MECHANICAL AREA - OVERALL FOUNDATION PLAN
SM102	MECHANICAL AREA (CEP) - FOUNDATION PLAN
SM103	MECHANICAL AREA (YARD) - FOUNDATION PLAN
SM104	MECHANICAL AREA (CEP) - FRAMING PLAN 2
SM105	MECHANICAL AREA (CEP) - FRAMING PLAN ROOF
SM106	MECHANICAL AREA (YARD) - FRAMING PLAN 2
SM107	MECHANICAL AREA (YARD) FRAMING PLAN ROOF
SM108	MECHANICAL AREA (YARD) - COOLING TOWER SUPPORT
SM109	MECHANICAL AREA (CEP) - ELEVATIONS
SM110	MECHANICAL AREA (YARD) - ELEVATIONS
SM111	MECHANICAL AREA (BRIDGE) - ENLARGEMENTS
SM112	MECHANICAL AREA (BRIDGE) - SECTIONS
SM113	MECHANICAL AREA - FOUNDATION DETAILS
SM114	MECHANICAL AREA - ANCHOR RODS

DRAWING	DESCRIPTION
SB101	SKYBRIDGE - ENLARGEMENTS
SB102	SKYBRIDGE - SECTIONS

DRAWING	DESCRIPTION
SC100	SOUTH CANOPY
SC101	SOUTH CANOPY - FOUNDATION PLAN
SC102	SOUTH CANOPY - FRAMING PLAN
SC103	SOUTH CANOPY - FRAMES
SC104	SOUTH CANOPY - DETAILS

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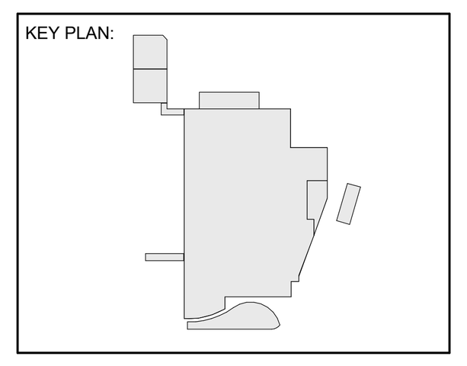
KEY PLAN:



PROJECT PHASE:

BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS		
#	DATE	DESCRIPTION

JOB NUMBER: 21-335-1

DATE: 12-09-2022

SHEET NUMBER: **S0.1**

SHEET TITLE: COVER SHEET

APPLICABLE CODES AND STANDARDS:

1.	International Building Code and all other local and state agencies having jurisdiction over this project	IBC 2018
2.	Minimum Design Loads for Buildings and Other Structures	ASCE/SEI 7-16
3.	American Institute of Steel Construction "Specifications for Structural Steel Buildings"	AISC 360-10
4.	American Institute of Steel Construction "Seismic Provisions for Structural Steel Buildings"	AISC 341-10 (R > 3.0)
5.	American Welding Society	AWS D1.1, D1.3, D1.4
6.	Steel Deck Institute Specifications and Load Tables	SDI
7.	Steel Joist Institute, Standard Specifications, Load Tables and Weight Tables for Steel Joists and Joist Girders	SJI
8.	Specifications for Design of Cold Formed Steel Structural Members (AISI)	AISI S100-12
9.	ASTM Material Standards as Noted	
10.	Building Code Requirements for Reinforced Concrete	ACI 318-14
11.	Building Code Requirements and Specifications for Masonry Structures	ACI 530.1-13

DESIGN INFORMATION

The below design criteria pertains to all building structures designed within these structural design documents, unless noted on thieir individual framing / loading plans or preceeded with a diamond symbol (♦) below.

1	**Risk Category:**	IV
	♦ *Pedestrian Sky Bridge*	*III*
	♦ *Detached Canopies*	*III*
2	**Floor Loads:**	per S1-Series loading identified on sheet S0.1 or as noted on framing plans
3	**Roof Loads:**	per S1-Series loading identified on sheet S0.1 or noted on framing plans
4	**Snow Loads:**	
	Importance Factor, Is	1.2
	♦ *Pedestrian Sky Bridge*	*1.1*
	Ground Snow Load, Pg	10 psf
	Exposure Factor, Ce	1.0
	Thermal Factor, Ct	1.0
	Flat Roof Snow Load, Pf	8.4 psf
	Minimum Roof Snow Load, Pm	12 psf
	Rain-on-Snow Surcharge Load	--- psf
	Snow Drift Load	See Sheet S1.00.1
5	**Wind Design Data:**	
	Importance Factor, I_W	1.0
	Basic Wind Speed, V	119 mph
	♦ *Central Eneergy Plant (inc. bridge)*	*135 mph*
	♦ *Mechanical Yard*	*135 mph*
	♦ *Pedestrian Sky Bridge*	*114 mph*
	♦ *Detached Canopies*	*114 mph*
	Wind Directionality Factor, Kd	0.85
	Exposure Category	C
	Topographic Factor, Kzt	1.0
	Ground Elevation Factor, Ke	1.0
	Gust Effect Factor, G	0.85
	Enclosure Classification	Enclosed
	♦ *Mechanical Yard*	*Partially Open*
	♦ *Detached Canopies*	*Open*
	Internal Pressure Coefficient, GCpi	±0.18
	♦ *Detached Canopies*	*0.00*
	Components & Cladding	Per building code & specifications and/or noted on sheet S1.00.1 or framing plans.
6	**Earthquake Design Data:**	
	Seismic Importance Factor, Ie	1.5
	♦ *Pedestrian Sky Bridge*	*1.25*
	Site Class (per geotechnical report)	C
	Long Transition Period, TL	12.00 sec
	Mapped Spectral Response Coefficients:	
	Short Period, Ss	0.139 g
	1-Sec Period, S1	0.078 g
	Site Coefficients:	
	Short Period, Fa	1.3
	1-Sec Period, Fv	1.5
	Design Spectral Response Coefficients:	
	Short Period, S_{DS}	0.120 g
	1-Sec Period, S_{D1}	0.078 g
	Seismic Design Category, SDC	C
	Seismic Force-Resisting System:	Structural Steel System Not Specifically Detailed for Seismic Resistance
	Response Modification Coefficient, R	3.0
	System Overstrength Factor, Ωo	3.0
	Deflection Amplification Factor, Cd	3.0
	Seismic Response Coefficient, $Cs=S_{DS}\cdot I_E$ / R	0.1095
	Design Base Shear, $V = C_S \cdot W_{EFF}$	0.1095·W
	Analysis Procedure used:	Equivalent lateral force
7	**Structural Integrity**	
	♦ This section only applies to the Hospital Structure	
	Structure is defined as a High-rise category Category IV. Sec 1616.2.2 Structural Steel and Composite Steel, IBC Sec 1616.2.2.1 Columns (ref AISC 360-16 sec. B3.9). See additional structural requirements on Structrual Steel Notes.	

GENERAL REQUIREMENTS:

1. The structural construction documents represent the finished structure. The contractor shall be responsible for adequate design and construction of all forms, shoring and temporary bracing. The contractor shall provide all measures necessary to protect the structure and safety of workmen during construction.
2. Do not place construction materials or other construction loads on the structure such that the loads placed exceed the capacity of the structure.
3. Construction material shall be spread out if placed on framed structural elements. The weight of these materials shall not exceed the design live load per square foot.
4. Take into consideration that full structural capacity of many structural members is not realized until structural assembly is complete; that is, until slabs, decks, diagonal bracing and/or moment frames are installed.
5. Provide temporary bracing and guying to provide stability and resist all loads to which the partially completed structure may be subjected including erection equipment and its operation. Adequacy of temporary bracing and guying for this purpose is the sole responsibility of the contractor.
6. The contractor shall hire a licensed professional engineer in the state of Oklahoma to design the bracing and to provide bracing details.
7. The structural engineer shall not be responsible for the contractor's means, methods, techniques, sequences, or procedures of construction, or the safety precautions and the programs incident thereto (nor shall observation visits to the site include inspection of these items).
8. The contractor shall provide and be responsible for the protection and repair of adjacent existing surfaces and areas which may be damaged by new work.
9. The contractor shall visit the site and familiarize himself with the existing conditions. The contractor shall verify existing dimensions and take additional measurements as needed prior to starting construction. The structural engineer shall be notified of any discrepancies or inconsistencies before proceeding with construction. The structural engineer is not responsible for the consequences of construction that do not comply with the requirements specified or the reasonable intent conveyed on these drawings or approved revisions thereof.
10. Do not scale drawings.
11. The design is valid only for the dimensions shown. The design may not be valid if actual constructed dimensions vary substantially from those shown on these plans.
12. Modifications of details of construction shall not be made without written approval of the structural engineer.
13. All column and foundations, unless noted otherwise, shall be centered on gridlines in each direction. Unless noted in the design documents
14. Typical details shall apply in general construction unless specifically detailed. Where no details are specified, construction shall be as shown for similar work. For condition that are not similar a written letter shall be sent to the SEOR for direction.
15. All Drawings and specifications are considered to be part of the contract documents. Structural drawings shall be used in conjunction with the civil, architectural, mechanical, electrical, and plumbing drawings for location and size of openings, blockouts, floor depressions, curbs, dimensions, etc. not indicated on the structural drawings. The location and size of mechanical and electrical openings in the slabs, wall, and/or decks shall be coordinated by the contractor. Provide all additional framing or reinforcing to accommodate openings as required by the applicable standard details shown on the structural drawings or provided by the structural engineer. No holes, notches, blockouts, etc are allowed in structural members unless detailed on the structural drawings or approved by the structural engineer.
16. Where the dimensions are provided for the openings, blockouts, floor depressions, curbs, etc but may be affected by the equipment purchased, the contractor shall verify the information provided prior to construction.
17. Provide concrete equipment pads and intertial bases for mechanical and electrical installations. Construct pads and bases in accordance with typical details. Re: Mechanical and Electrical drawings for limits and locations.
18. Gravel for placement under the slab-on-grade shall consist of well-graded crushed store ¾" maximum particle size and less than 5% passing thru a No. 4 sieve.
19. Unless noted otherwise, all concrete slab-on-grade shall be poured upon 4" gravel, U.N.O.. Prior to concrete placement the gravel shall be compacted with at least 4 passes of a vibratory plate compactor or vibratory drum roller. However, If the soils report provides a more stringent compaction requirement, this will govern.
20. All structural steel 3D models shall use grids B/1 as 0,0,0 for their "orgin point" . Levle 1 is +100-0".

SUBMITTALS:

1. The structural engineer shall not be accountable for acts or omissions of the contractor or any party / person, affiliated or not, performing the work or failing to carry out the work in accordance with the contract documents.

   Shop Drawings and related material (if any) required are indicated below. Should Foy Consulting & Engineering, LLC, (Foy) project SEOR or designated representative, required more than ten (10) working days to perform the review, The GC will be notified by Foy.
   1. Concrete mix designs and material certifications including Admixtures and compounds applied to the concrete mix after placement.
   2. Reinforcing steel shop drawings including erection drawings and bending details. Bar list will not be reviewed for correct quantities.
   3. Structural steel shop drawings including erection drawings, and piece details. Include joists, decking, and connector submittals, include miscellaneous framing specified on the structural drawings, but do not submit framing specified on non-structural drawings for Foy to review. Large steel submittals can result in the SEOR's reviews exceeding 10 working days.
2. The structural engineer reviews submittals to ensure the general conformance with the intent conveyed in contract documents. Quantities and dimensions are not checked. Checking of any submittal by the structural engineer does not relieve the contractor's responsibility of contract deviations or from the submittals errors and/or omissions.
3. Contractor shall comply with the Division 1 Specifications for "Submittals".
4. Submittals must be checked and stamped by the contractor prior to submission. The contractor's stamp of approval will constitute certification that he has verified all field measurements, construction criteria, materials and similar data and has checked each drawing for completeness, coordination, and compliance with the contract documents.
5. Submittals shall be transmitted in advance of related construction activities to avoid unnecessary delay. The structural engineer of record reserves the right to reject any submittal or withhold action on a submittal requiring coordination with other submittals until all related submittals are received. All submittals shall be electronic format. Any submittal that are scanned and of poor quality will be rejected without review. The definition of "poor quality" is subjective and, as finally determined by the SEOR's perspective. All drawings submitted for review shall be the original drawing's unscaled size which it was created and shall not be reduced from printing, scanning or other digital manipulation.
6. Reproduction of any portion of the structural contract drawings for submittal as shop drawings is prohibited.
7. The structural engineer's cursory review of submissions by specialty engineers may be limited to verifying the contact document's design intent was understood and drawings have been signed and sealed by said specialty engineer. The specialty engineer is solely responsible for their design and compliance with codes and standards. The SEOR maintains the right to review and ensure all calculations are accurate and meets professional standards for engineering analysis. Design analysis are required for all connections supplied by steel fabricator. Specific calculations may be required for areas of particular concern.
8. DEFERRED Speciality submittals :
   1. Deferred submittal: Structural steel connections, including vertical & horizontal bracing and rigid frame connections. Including moment connections on gravity structure that are not part of the lateral frame system.
   2. Deferred Submittal: Metal Pan Stairs, Railings and Guardrails.
   3. Deferred Submittal: Metal Stair Framing. All forces from the stair shall be identified and transmitted to the SEOR for review and approval before going to fabrication.
   4. Deferred Submittal: Exterior cold-formed metal framing (light gage).
   5. Deferred Submittal: Exterior & Interior Curtain Wall framing and their attachment includng glazing and mullion designs.
   6. Miscellaneous anchors shown on the structural drawings.
   7. Any Pre-Engineered Modular Buildings.
   8. Any Pre-Engineered canopies supprted off of the structrure. All forces from the pre-engineered canopies, by others, shall be identified and transmitted to the SEOR for review and approval for coordination of attachment to the structure.

SITE PREPARATION:

1. Site preparation shall follow the geotechnical report recommendations.
2. Shallow foundations shall be supported on approved stiff or dense natural overburden soils or controlled fill approved by the geotechnical engineer.
3. The contractor shall be entirely responsible for safely excavating into the ground and constructing stable soil slopes.
4. Positive drainage shall be provided during construction and maintained throughout the life of the project.
5. The contractor shall provide dewatering of excavations from either surface water or seepage. The moisture content in soils prior to excavation should not be allowed to change relevantly after the excavation is made. Concrete for foundations shall not be placed on ground softened from excess water.
6. The base of the excavation shall be free of water and loose soil prior to placement of reinforcing or concrete. Ideally, foundation concrete shall be placed the same day when the excavation is made.

FOUNDATIONS:

1. Foundation design is based upon Report prepared by Palmerton & Parrish, Inc.
   a. Geotechnical Engineering Report W.W. Hastings Replacement Hospital & Parking Garage Project No: 277340
      Issued - February 9, 2022
      Addendum No. 1 - August 18, 2022
      Addendum No. 2 - November 9, 2022
   b. Pier Pre-Drill Results Summary Letter
      Issued - November 4, 2022
   c. The soils report will be made available upon request and all recommendations contained in the soils report shall be considered as a requirement for this project unless noted otherwise.
   d. The Geotechnical Engineer is the sole judge of stability of underlying material to support foundations and shall approve bearing material before foundation installation.
2. Minimum design frost & variation in soil moisture depth 24 inches
3. Foundations have been designed for the following:
   a. Mechanical Yard & Central Energy Plant
      1. Shallow foundations
      2. Net allowable pressure: 5,000 psf
   b. Mechanical Pipe Bridge
      1. Drilled piers.
      2. Per the Pre-Drill Results summary
   c. Pedestrian Sky Bridge
      1. Drilled piers.
      2. Per the Pre-Drill Results summary
   d. Hospital – one (1) story zone, & canopies.
      1. Shallow foundations.
      2. Net allowable pressure = 3,500 spread footing and 3,000 psf continuous
   e. Hospital – Multi levels.
      1. Drilled piers.
      2. Per the Pre-Drill Results summary
   f. Minimum Footing Depth and Widths:
      1. Exterior footing below lowest adjacent grade 2'-0"
      2. Isolated footing width 2'-6"
      3. Continuous footing width 1'-6"
4. Drilled Piers additional requirements during construction:
   a. Drilled piers have been proportioned in designed with skin friction, skin friction and end bearing or just end bearing with +50 KSF capacity.
   b. If a drilling requires a steel casing to resist water or soil stability / caving in the drilled shaft, steel casing shall be pulled from the drilled pier to prevent the loss of the skin friction capacity
5. All earthwork and site preparation shall be performed in strict accordance with the specifications and the Geotechnical Report.
6. If the contractor discovers poor soils conditions upon excavation, he shall notify the geotechnical engineer and structural engineer in writing.
7. The geotechnical engineer shall inspect and approve the soil excavation after the footings are excavated and before the concrete for the footings are poured.
8. The contractor shall notify the geotechnical engineer at least 48 hours prior to the inspection.
9. All footings shall be placed on either undisturbed previously compacted controlled fill or undisturbed native soils.
10. Any existing fills or unsuitable soils as determined by the geotechnical engineer shall be excavated and replaced with properly compacted fill.
11. Remove all debris from the excavation before the concrete is poured.
12. All over excavation shall be filled with concrete, engineering fill, or flowable fill.
13. All forms and organic debris shall be removed prior to backfilling.
14. Do not excavate below the bearing excavation of any completed footing nor any closer to the footing than a slope of 2 horizontal (measured from the edge of tooting to nearest point in the excavation) to 1 vertical.
15. Horizontal construction joints in column footings, slabs on grade and matt foundations are not permitted.

CAST-IN-PLACE CONCRETE:

1. All concrete work shall be in accordance with the "Building Code Requirements for Reinforced Concrete" (ACI 318).
2. All concrete shall develop a minimum ultimate compressive strength in 28 days as noted below, with not less than 550 pounds of Type I/II Portland cement per cubic yard of concrete, regardless of the strengths obtained, maximum w/c ratio of 0.45, with course aggregate size not larger than ¾" diameter, and a maximum of 4" slump:

   a. F'c = 4,000 psi (normal weight, air entrained) all exposed concrete flatwork and retaining walls.
   b. F'c = 4,000 psi (normal weight, air entrained) all foundation concrete (footings, tie beams, stem walls, grade beams and interior concrete walls.)
   c. F'c = 4,000 psi (normal weight) all interior slabs on grade.
   d. F'c = 4,000 psi (normal weight) concrete over steel deck.
   e. F'c = 4,000 psi (normal weight) all other concrete.
   f. F'c = 8,000 psi at 28 days for non-shrink grout for placement under column base plates. Grout to comply with ASTM C 1107.
3. Concrete may have up to 15% of Portland cement weight replaced with an equivalent weight of an approved Class C or Class F fly ash.
4. All admixtures shall be submitted and noted in mix design for approval by the structural engineer before use.
5. Air entrained exterior exposed concrete and concrete flatwork shall have 6% ± 1% air.
6. No aluminum items shall be embedded in any concrete.
7. All concrete shall be vibrated during placement.
8. Provide ¾" chamfer on all exposed concrete corners.
9. All concrete is reinforced unless specifically called out as unreinforced. Reinforce all concrete not otherwise shown with same steel as in similar sections or areas. Any sections not shown shall be detailed per ACI 315, "Details and Detailing of Concrete Reinforcement", current edition.
10. Concrete Cover Requirements: (Non-Prestressed Construction)
    a. Concrete placed against and permanently exposed to earth: 3"
    b. Concrete exposed to earth or weather:
       #6 bars or larger 2"
       #5 bars or smaller 1 1/2"
    c. Concrete not exposed to weather or in contact with ground:
       Slabs, Walls, Joists 3/4"
       Beams and Columns 1 1/2"
11. All Concrete Slab-on-Grade shall have construction joints located to form approximate square panels of not more than 250 square feet or as shown on the drawings. See typical details for construction or control joint information. Joints shall generally be on column centerlines. The ratio of the longer dimension to the shorter dimension on any concrete slab panel shall not exceed 1.5 unless noted otherwise. Lap reinforcement bars including corner bars and dowels, in accordance with the Reinforcement Bar Tension Lap Slice Schedule.
12. Cut saw joints in slab-on-grade concrete maximum 8 hours after concrete pour.
13. No conduit or piping larger than 1" I.D. shall be located in concrete members including slabs, unless shown on the structural drawings or approved by the structural engineer. The spacing of the conduit or piping in slabs shall be a minimum of 3 diameters center to center and multiple conduits over a 15" width shall be approved by the structural engineer. No conduit or piping is allowed in concrete over composite deck without the approval of the structural engineer.

CONCRETE REINFORCING:

1. Bar reinforcing shall conform to ASTM A615 grade 60 deformed reinforcing steel.
2. Plain welded wire fabric shall conform to ASTM A185.
3. All reinforcement shall be detailed, fabricated and placed in accordance with ACI 315.
4. Continuous reinforcement in walls and footings may be spliced as required, provided that bars are the longest practical length and all splices are shown on the reinforcement shop drawings. Splices are to be staggered when possible. Provide lap slices and development lengths in accordance with the Reinforcement Bar Tension Lap Slice Schedule.
5. Provide dowels of same size and number from adjacent pour both vertically and horizontally to match typical reinforcing shown unless noted otherwise. Lap bars in accordance with the Reinforcement Bar Tension Lap Splice Schedule.
6. Field welding or bending of reinforcing is not permitted except as indicated on the drawings or as approved by the structural engineer.
7. Provide corner bars at all intersections of continuous footings, grade beams and walls to match typical horizontal reinforcing in size and spacing. Extend all bars to far side of intersecting footing, grade beam, or wall unless noted otherwise. Lap reinforcing bars, including corner bars and dowels per the Reinforcement Bar Tension Splice Schedule.
8. The contractor shall submit for approval reinforcement shop drawings including elevations, section cuts, dimension, and schedules showing size and placement of all reinforcement in sufficient detail to be placed without reference to contract documents.

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Oklahoma Certificate of Authorization" No. 4570

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CHEROKEE NATION REPLACEMENT HOSPITAL

TAHLEQUAH, OKLAHOMA

KEY PLAN:

PROJECT PHASE:

BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

#	DATE	DESCRIPTION

JOB NUMBER: 21-335-1

DATE: 12-09-2022

SHEET NUMBER:

S0.2

SHEET TITLE:

GENERAL NOTES

MASONRY:

1. All concrete masonry units (CMU) shall be normal weight per ASTM C-90 with a minimum net area compressive strength of the concrete masonry units of 1900 psi.
2. All CMU shall be assembled using Type "S" mortar per ASTM C 270 with a minimum net area compressive strength f'm = 1500 psi.
3. All CMU shall have grout per ASTM C-476 using aggregates that do not exceed 3/8" diameter. Configure proportions of ingredients as required to achieve 3000 psi breaking strength in 28 days per ASTM C-1019.
4. All CMU shall have galvanized horizontal joint reinforcement. The joint reinforcement shall be 9 gage deformed wire, ladder-type, set continuously on CMU bed joints not exceeding 16" vertical spacing the full height of wall unless noted otherwise. Provide in first bed joint above and below openings and extend 24" beyond openings.
5. All CMU shall be constructed in running bond. Stack bond is not permitted.
6. All CMU shall be two-cell type units except lintels which shall be u-shaped units. Bond Beam units may be u-shaped or two-cell type.
7. All CMU bond beams and lintels shall be completely grouted. Bond beams at intersecting CMU walls shall meet at the same elevation and the reinforcing shall be lapped as required.
8. Any hollow masonry, brick or CMU, in contact with earth shall be grouted solid.
9. Add vertical reinforcing bars to all CMU walls at corners, cells adjacent to corners, anchored intersections, each side of openings, each side of control joints and in the last 2 cells at the end of walls. Vertical reinforcement size shall match typical wall reinforcement size unless noted otherwise.
10. All CMU vertical voids and horizontal bond beams to be grouted shall be free of debris and mortar droppings prior to grouting. Mortar projections into the grout shall not exceed 1/2" beyond inside face of masonry.
11. Reinforcing shall be placed prior to grouting and secured adequately to maintain rigid positioning during grouting. All cells containing reinforcing shall be grouted. Within the grout space, all voids shall be filled solid.
12. All reinforcing shall have a minimum coverage of one bar diameter (1/2" min.) of grout. Centered bars shall be securely placed in the center of a cell. Each face bars shall be placed 1" clear of the face shell. Where two layers are required in 8" or smaller block, use one bar in each of two adjacent cells.
13. Grout pours in excess of 5'-0" shall have access holes (clean outs) cut out of the bottom face of the void to facilitate debris removal and the check that reinforcing has been properly aligned. All clean-outs shall be inspected prior to plugging and shall be plugged prior to grouting.
14. All grout pours shall be constructed in grout lifts not exceeding 5'-0". For pours comprised of several lifts, place the next lift a minimum of 30 minutes after 2nd-vibration of the previous lift.
15. Vertical control joints in masonry shall not be located within 2'-0" of openings, unless noted otherwise.
16. Maximum spacing of control joints shall be as follows unless noted otherwise. Exterior walls = 30 ft; Interior walls = 40 ft.
17. All reinforcing including the horizontal reinforcement shall be discontinuous at control joints, except for bond beam reinforcing occurring at top of wall.
18. Continuous reinforcing in walls may be spliced as required. Provide bars of the longest practical length and all splices shall be shown on the reinforcing shop drawings. Wherever possible, splices shall be staggered.
19. Lap all reinforcing bars in masonry in accordance with the masonry lap bar schedule. Horizontal CMU reinforcing shall be continuous around all corners and intersections.
20. All vertical reinforcing shall be continuous for full height of wall and doweled into the footing or slab on grade below and extended into the bond beams above. Continuity may be established with lapped splices meeting all indicated requirements.
21. Cells containing bolts shall be grouted solid with at least ½" grout coverage between the bolt and the CMU at the block face.
22. The contractor shall submit for approval CMU reinforcement shop drawings including elevations, section cuts, and schedules showing size and placement of all reinforcement in sufficient detail to be placed without reference to contract documents.

POST INSTALLED ANCHORS:

1. Install anchors in strict accordance with the current ICC-ES report for the bolt and installation instructions from the manufacturer.
2. Install anchors perpendicular to the face of the concrete. Deviation from perpendicular greater than 10 degrees will not be acceptable.
3. Contractor shall create a template at each anchor connection location and locate existing rebar prior to fabricating holes in connection plates. Contractor shall notify the structural engineer if there is a conflict with rebar prior to fabrication of holes in connection plates.
4. Drill holes to depth and diameter recommended by manufacturer or as indicated in the construction documents. Do not enlarge or redirect holes along the length of the bolt.
5. Drill holes in continuous operation. Blow dust from the hole using compressed air or per manufacturer's recommendations.
6. Fill abandoned holes with epoxy grout.
7. Provide holes in connection plates no more than 1/16" larger than the bolt diameter. If larger holes are needed for erection purposes, provide plate washers welded to the connection plate to transfer the bolt load.
8. Contractor shall submit product data for all post installed or adhesive anchors to the structural engineer for approval. The contractor shall indicate the location and type of use in relation to the contract documents for each post installed anchor.

METAL PAN STAIRS:

1. The contractor shall submit complete stair shop drawings and structural calculations to the architect/engineer. Calculations for the stairs, stair landings, stair members, stair supports, rails, and stair connection design and shop drawings shall be signed and sealed by an engineer registered in the state of which the project is located. The sealed shop drawings and calculations shall be by the same engineer and be submitted together.
2. All require embedded angles and plates in concrete for stairs shall be part of the stair design and detailing.
3. The concrete strength, thickness, and reinforcement shall be indicated and called out on the shop drawings for all landings and pans designed by the stair manufacturer. The call outs shall include the phrase "By Others" if not supplied by the stair manufacturer.
4. Stairs shall be designed for the superimposed dead load, self-weight and live load as indicated below:
   a. Uniform Load: 100 PSF
   b. Concentrated Load: 300 lbs applied on an area of 4 square inches.
   c. Uniform and concentrated loads need not be assumed to act concurrently.
   d. Stair Framing: Capable of withstanding stresses resulting from railing loads in addition to loads specified above.
   e. Limit deflection of treads, platforms, and framing members to L/360 or 1/4 inch, whichever is less.
   f. If members are supported off of primary framing, super imposed deflection of the primary member shall be taken into account.
5. Stair handrails and top rails of guards shall be designed for the self-weight and live load as indicated below:
   a. Uniform Load: 50 PLF applied in any direction.
   b. Concentrated Load: 200 lbs applied in any direction.
   c. Uniform and concentrated loads need not be assumed to act concurrently.
6. Stair infill of guards shall be designed for the self-weight and live load as indicated below:
   a. Concentrated Load: 200 lbs applied horizontally on an area of 1 square foot.
   b. Infill load and other loads need not be assumed to act concurrently.
7. Seismic Performance of Stairs: Metal stairs shall withstand the effects of earthquake motions determined according to ASCE 7.
   a. Component Importance Factor: 1.5
8. Stairs and related items shall comply with the building code.
9. Refer to the architectural drawings for stairway dimensions, details, and other requirements.
10. Stairs may be supported by the primary structure provided stair framing does not impose eccentric or torsional loading upon the primary framing. Stair reactions shall only impart vertical reaction to building steel support members. Any torsion developed by the stair manufacture shall be resolved with additional steel supplied by the stair manufacturer and documents provided to the SEOR for review prior to fabrication.
11. All stair/ rail steel detailing shall be provided in a 3D model in SDS2 version 2021, Steel Detailing Software. The SDS2 model shall be submitted to the SEOR for our design intent and submission review. Additionally, an IFC or NWC model provided to the GC for BIM coordination. All stair shop models shall have a Level of Development (LOD) 400-Fabrication-ready Geometry

STRUCTURAL STEEL:

1. The design, fabrication and erection shall be according to the AISC "Specifications for Structural Steel Buildings."
2. Structural steel shall meet the following minimum yield strengths and specifications:

DESCRIPTION	YIELD	ASTM
Headed Anchor Stud	50 ksi	A108 (Grades 1015 to 1020)
Steel W or WT U.N.O.	50 ksi	A992
Steel Channels and Angles	36 ksi	A36
Structural Bars and Plates	36 ksi	A36
Structural Steel HSS	46 ksi MIN	A500 Grade B or C
Structural Steel Pipe	35 ksi	A53 Grade B
Anchor Rods	36 ksi	F1554 Grade 36
Anchor Rods (Rigid Frames)	55 ksi	F1554 Grade 55 (Weldable)

3. Bolts for steel beam and column connections shall be 3/4" diameter ASTM A325N high-strength bolts, unless noted otherwise.
4. Bolt spacing in steel columns and beams flanges shall be per the AISC Manual, Table 1-1, "Workable Gage". Beams/Columns with flange widths of 4" shall have workable gages of 2 ¼" with a maximum of 5/8" diameter bolts.
5. All bolts shall be tightened to a "Snug Tight" condition and inspected accordingly, unless noted otherwise on plans or connection details.
6. All composite beam connections noted with camber shall only use A325N bolts.
   a. F1852 Tension Control (TC) bolts will not be allowed, UNO.
7. At composite beam connections indicated on the contract documents where the bolts are indicated as fully tensioned, the dead load due to the placement of the concrete shall be applied prior to fully tensioning the bolts.
8. Welding shall meet ANSI/AWS D1.1 structural welding codes. Electrodes shall be 70 ksi low hydrogen, unless noted otherwise. Any weld sizes shown on the design drawings are considered effective weld sizes and shall be increased in accordance with AWS as required by gaps or skews between components.
9. The contractor shall submit structural steel shop drawings and structural steel connection calculations to the architect/engineer. Connections not specifically detailed on the design document shall be designed under the guidance of a licensed professional engineer registered in the state of which the project is located. The structural steel connection calculations shall be clear, concise, and well organized with page numbers, summary and conclusions submitted to the structural engineer of record for approval. The shop drawings and calculations shall be signed and sealed by the same engineer and be submitted together.
10. Any deviations from the contract documents requirements shall be approved in writing through the proper procedures prior to the submittal of shop drawings and calculations.
11. Composite beams shall be designed for 50% of its web shear capacity and Non-Composite beams shall be designed for 40% of its web shear capacity based upon the AISC Table 3-6, "Maximum Total Uniform Load Table", unless noted otherwise (UNO). Both shall adhere to the minimum number of bolts identified as follows, UNO.

Section Size/Nominal Beam Depth	Min No. of Bolts
W8, W10	2
W12, W14, W16	3
W18	4
W21, W24	5
W27	6
W30, W33, W36	7

12. Additional requirements for moment connections are specified in the design documents. These shall be identified in the delegated specialty engineer's (DSEOR) calculation submittal for structural steel connections. Including web panel-zone review w/ required continuity and web-double plates or welds. Unbalanced moment conditions are provided in the contract documents are shall be reviewed . All frame moment connections are "FIELD WELDED" connections and shall be designed with the design moments "±M" and beam shear reactions "±V" forces specified on the building elevations and rigid frame elevations. In addition these "FIELD WELDED" moments shall adhear to the additional specified design configuration requirements and design force combinations shown on drawing S-520. The minimum design load for any connection shall be 6 kips (ASD) and 10 kips (LRFD) regardless of the beam reaction (s) shown on the contract documents. Field bolted moment connections are allowed where approved .
13. Provide double nuts and double washers for steel column anchor rods to allow for adjustment in base plate elevation.
14. Place non-shrink grout under all column base plates before placing elevated slabs unless noted otherwise. Place flowable grout under all lateral frames with shear lugs or any base plate with dimensions over 24". Provide the following non-shrink and/or flowable grout under base plates after erection, UNO:

Anchor Rod Diameter	Non-Shrink Grout Thickness*
Up to 1"	1 1/2"
1" to 1 1/2"	2"
1 3/4" to 2"	2 1/2"

* If the steel erector does not require leveling nuts under the base plates, then 1 1/2" grout may be used at all locations.

15. Non-shrink and/or flowable grout shall be non-metallic with a minimum compressive strength of 8000 psi at 28 days.
16. All anchor rods shall be placed accurately with setting templates and securely held in position while placing concrete. Use 1/8" steel templates at all base plates using 1 1/4" and 1 1/2" diameter anchor rods. Anchor rod tolerances shall be +/-1/8" horizontally and +/- ¼" vertically.
17. Tolerance requirements - structural drawings indicate miscellaneous steel elements such as lintels, support members for curtain walls and edge angles for openings and perimeter conditions which are intended to support or be coordinated with materials furnished with other trades. It is the intent of these drawings, that these elements be field attached by field welding or bolting to meet the tolerances required by other trades, which may be more stringent than AISC tolerances for structural steel. Contractor shall coordinate trades and field install miscellaneous steel elements and the structural steel frame to comply with the tolerance criteria for proper installation of materials by other trades.
18. Contractor shall protect any unprimed structural steel from detrimental effects of corrosion, as required, until the steel is enclosed and protected by the new construction.
19. All steel, directly exposed to the weather or corrosive environments, shall be galvanized or coated with approved products to prevent corrosion, unless noted otherwise.
   a. All exposed interior and exterior steel in "butterfly" clerestories shall minimally recieve SSPC-SP6 Commercial Blast Cleaning surface preparation and coated with high performance primer per architectural paint specification.
   b. All exposed steel "butterfly" canopies shall minimally recieve SSPC-SP6 Commercial Blast Cleaning surface preparation and coated with high performance primer per architectural paint specification.
20. Prime paint all steel unless noted. Do not paint elements that are galvanized, receiving fireproofing, or noted otherwise.
21. All structural steel embedded in concrete and not permanently exposed to weather shall be unprimed. All structural steel embedded in concrete and permanently exposed to weather shall be hot-dip galvanized in accordance with ASTM A-123.
22. Contractor shall coordinate structural steel fireproofing requirements. All interior structural steel scheduled or indicated to receive fireproofing shall be delivered to the project site unprimed.
23. Where the work of other trades requires cuts or openings to be made in structural steel members, approval shall be obtained from the structural engineer. Such openings shall be made in the shop and clearly indicated on the shop drawings.
24. Beams shall be fabricated with the camber indicated on the plans. Beams without specified camber shall be erected with the standard mill tolerance camber up.
25. Do not weld bottom flange braces until all roof dead loads are in place.
26. In the event that beam CAMBER for a beam on the job site does NOT meet specified camber on the design documents, contact the SEOR for recommended resolution. Cambers shall be verified prior to erection.
27. All perimeter roof beams with type "B" roof deck shall have a connection with a minimum axial capacity of 3k (UNO). Single sided clips may not be used in these perimeter conditions without written permission from the SEOR.
28. All structural steel exposed to view both interior and exterior shall be considered Architecturally Exposed Structural Steel (AESS).
29. All welds on visually exposed or AESS steel will be ground smooth.
30. All moment connection welds that are visible shall have all backer bars removed.
31. Hangers for DUCKWORK, piping, electrical conduits, etc shall be hung directly from structural; steel or anchors embedded into concrete (excluding concrete composite floors). All extra support steel is the responsibility of the vendors. For single point loads or accumulative loads exceeding 500# per beam or #2000 per girder, submit official hanging load layout for review.
32. FABRICATION Qualification: Fabricator must participate AISC's Quality Certification program and designated an AISC-Certified plant, category STD and an "Approved Fabricator" in accordance with IBC Section 1704.2, "Special Inspection and Tests, Contractors Responsibility and Structural Observations". AISC CERTIFICATION shall be shown clearly on the shop drawings to avoid shop drawings being rejected. Steel fabricator may not sub-out steel fabrication to non- AISC Steel fabrication plants without explicit written approval from the Owners representative.
33. All Steel detailing shall be provided in a 3D model in SDS2 version 2022, Steel Detailing Software. A copy of the SDS2 model provided to the SEOR for our use and an IFC or NWC model provided to the GC for BIM coordination & shop drawing / calculation review. The 3D model shall minimally include the following items modeled in place: columns, beams, braces, connections (shear tabs, clips, end plates brace gussets), bolts and bolt holes, dge angles, frame braces, bottom flange braces/kickers, lintels, framed floor & roof openings, gates, stairs, rails, steel jambs & headers, and all cast in pace embeds & anchor rods. Items not explicitly required for 3D models: welds, LG material, floor deck and miscellaneous filler steel, concrete reinforcement not connected to structural steel.
   Note:The original existing structure is detailed in SDS2 and will be required to be for the new expansion.
34. Allow for 10,000# of structural steel to be used as directed in the field for special conditions or as required by the steel fabricator to complete design connections. 25% shall be bid as AESS steel and 25% should be bid as galvanized angles and plates.

STRUCTURAL STEEL (CONTINUED):

35. IBC Sec 1616.2.2.1 **Columns**. Each column splice shall have the minimum design strength in tension to transfer the design dead and live load tributary to the column between the splice and the splice or base immediately below. (All loads in the design document are Factored Loads, UNO)
36. IBC Sec 1616.2.2.2 **Beams**. End connections of all beams and girders shall have a minimum nominal axial tensile strength equal to the required vertical shear strength for *allowable stress design* (ASD) or two-thirds (2/3) of the required shear strength for load and resistance factor design (LRFD) but not less than 10 kips. For the purpose of this section, the shear force and axial tensile force need not be considered to act simultaneously. (All loads in the design document are Factored Loads, UNO)
37. Allow a typical two (2) weeks for review of shop drawings and delegated connection engineering services for each 200 tons of structural steel submitted. If larger submittals are anticipated, let the SEOR know in advance so scheduling can be expected, and time allowed.

METAL DECK:

1. Roof deck:
   a. Roof deck shall be galvanized. Provide deck type and gage thicknesses as shown on the roof framing plans.
   b. Roof deck shall be required to act as a diaphragm. Connections shall be in accordance with Steel Deck Institute specifications. Refer to the roof diaphragm connection detail for attachments.
   c. Decking to be continuous over a minimum of (3) supports, unless noted otherwise.
   d. Do not suspend loads from the roof deck.
2. Composite deck:
   a. Composite deck shall be galvanized. Provide deck type and gage thicknesses as shown on the floor framing plans.
   b. Composite deck design is for unshored construction and shall be a minimum of a 2 span condition.
   c. Provide headed stud placement per the "Headed Stud Shear Connectors Typical Detail" shown on the construction documents.
   d. Do not paint surfaces which receive welded studs.
   e. Loads exceeding 50 lbs shall not be permitted to be hung from metal decking. Hangers for ductwork, piping, electrical conduits, etc. shall be hung directly from Structural steel or anchors embedded in concrete. Submit hanging load layout for review.
   f. Loads suspended from composite floor exceeding 50 lbs shall not hung from metal decking. Hangers shall be 24" from adjacent deck hanger, except loads < 25 lbs may be minimally spaced 12" from adjacent deck hangers.

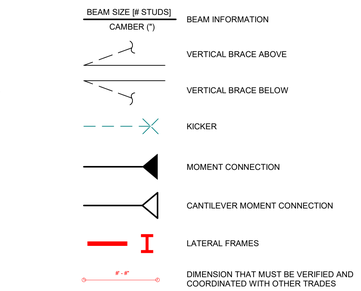
COLD FORMED METAL FRAMING:

1. All properties, fabrication and erection shall be according to the AISI, "Specification for the Design of Cold-Formed Structural Members".
2. All load bearing and exterior cold formed metal studs shall be at least 20 gage with a minimum 33 ksi yield.
3. All framing members shown on the Architectural/Structural drawings shall be considered minimum sizes. The contractor is responsible for coordination of actual cold formed metal framing sizes with other trades such as structural steel.
4. Fabrication of cold formed metal framing shall not begin until shop drawings and calculations have been reviewed, approved, and returned.
5. The cold formed metal framing suppliers shop drawings shall include as a minimum the following:
   a. All member sizes
   b. The size, location and fastening of members.
   c. The fastening of the top and bottom tracks to the structure.
   d. The size and type of vertical slide clips.
   e. The size and type of vertical deflection top tracks.
   f. The size and fastening of headers at all openings.
   g. All screwed and welded connections.
6. All cold formed metal framing members shall be designed for the indicated gravity and lateral loads on the contract documents. The cold metal framing supplier shall reference architectural drawings for all openings and material associated with the cold metal framing, unless noted otherwise.
7. All cold formed metal framing shall be galvanized and conform to ASTM A653. Galvanized finish shall conform to ASTM A653 with a minimum coating class of G60.
8. Cold formed metal framing lateral deflection shall take into account architectural finishes and provide a minimum of the following:
   a. L/600 of span where exterior finish material is stone, brick, or masonry.
   b. L/360 of span otherwise.
9. All framing members shall be cut as required to fit squarely against abutting members. Splicing of axially-loaded members is not permitted. Members shall be held in place until properly fastened. Attachments shall be made by welds, screws or bolts; wire tying is not permitted.
10. All vertically-oriented metal studs shall be framed between continuous top and bottom "C" shaped tracks. Top tracks framed between floors shall have a "Slip" track that allows for vertical deflection of the member the track is attaching too.
11. The contractor shall submit, to the structural engineer for review, cold formed metal framing shop drawings to the architect/engineer with design loads indicated and openings shown. Calculations for the member design and connection design and shop drawing shall be signed and sealed by an engineer registered in the state of which the project is located. The sealed shop drawings and calculations shall be by the same engineer and be submitted together for review. The engineer shall have a minimum of 3 years' experience and provide evidence of previous work experience in cold formed metal framing of projects equal is size and complexity.

ABBREVIATIONS

-A.B.C.	AGGREGATE BASE COURSE	I.F.W.	INSIDE FACE OF WALL
A/C	AIR CONDITIONER	HORIZ.	HORIZONTAL
A.F.F.	ABOVE FINISHED FLOOR	K (KIP)	1000 POUNDS
ALT.	ALTERNATE	L.L.	LIVE LOAD
A.B.	ANCHOR BOLT	LBS. (#)	POUNDS
@	AT (MEASUREMENT)	L.L.H.	LONG LEG HORIZONTAL
BM.	BEAM	L.L.V.	LONG LEG VERTICAL
B.F.F.	BELOW FINISHED FLOOR	L.D.H.	LONG DIMENSION HORIZONTAL
B.O.B.	BOTTOM OF BEAM	L.D.V.	LONG DIMENSION VERTICAL
B.O.D.	BOTTOM OF DECK	MFR(S)	MANUFACTURER(S)
B.O.F.	BOTTOM OF FOOTING	MAS. C.J.	MASONRY CONTROL JOINT
BRG.	BEARING	MECH'L.	MECHANICAL
C.I.P.	CAST IN PLACE	N/A	NOT APPLICABLE
CL.	CENTERLINE	N.T.S.	NOT TO SCALE
C.L.B.	CENTERLINE OF BEAM	O.C.	ON CENTER
C.L.C.	CENTERLINE OF COLUMN	O.F.W.	OUTSIDE FACE OF WALL
C.L.F.	CENTERLINE OF FOOTING	OPP.	OPPOSITE
C.L.W.	CENTERLINE OF WALL	P.C.	PRECAST CONCRETE
CLR.	CLEAR	PLF.	POUNDS PER LINEAR FOOT
CONC.	CONCRETE	PREFAB.	PREFABRICATED
CONC. C.J.	CONCRETE CONTROL JOINT	PSF.	POUNDS PER SQUARE FOOT
CONC. S.J.	CONCRETE SAWCUT JOINT	PSI.	POUNDS PER SQUARE INCH
C.M.U.	CONCRETE MASONRY UNIT	RE:	REFERENCE TO
CONN.	CONNECTION	REINF.	REINFORCING
CONT.	CONTINUOUS	S.L.H.	SHORT LEG HORIZONTAL
D.L.	DEAD LOAD	S.L.V.	SHORT LEG VERTICAL
DIA.	DIAMETER	SIM.	SIMILAR
DN	DOWN	SQ.	SQUARE
DWG(S)	DRAWING(S)	STD.	STANDARD
E.O.S.	EDGE OF SLAB	T.L.	TOTAL LOAD
ELEV.	ELEVATION	T.O.B.	TOP OF BEAM
EQ.	EQUAL	T.O.D.	TOP OF DECK
EQUIP.	EQUIPMENT	T.O.F.	TOP OF FOOTING
EXP. BOLT	EXPANSION BOLT	T.O.L.	TOP OF LEDGER
EXP. JT. (E.J.)	EXPANSION JOINT	T.O.M.	TOP OF MASONRY
E.F.	EACH FACE	T.O.P.	TOP OF PLATE
E.W.	EACH WAY	T.O.S.	TOP OF STEEL
F.F.	FINISHED FLOOR	T.O.W.	TOP OF WALL
F.O.M.	FACE OF MEMBER	T. & B.	TOP AND BOTTOM
F.O.S.	FACE OF STEEL	TYP.	TYPICAL
F.O.W.	FACE OF WALL	U.N.O.	UNLESS NOTED OTHERWISE
GA.	GAGE	VERT.	VERTICAL
GALV.	GALVANIZED	W.W.R.	WELDED WIRE REINFORCING
G.S.N.	GENERAL STRUCTURAL NOTES	W/	WITH
GLB (GLULAM)	GLUED-LAMINATED BEAM	W/O	WITHOUT

SYMBOLS LEGEND



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Oklahoma Certificate of Authorization" No. 4570

CLIENT:

CHEROKEE NATION REPLACEMENT HOSPITAL

TAHLEQUAH, OKLAHOMA

KEY PLAN:

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

#	DATE	DESCRIPTION

JOB NUMBER: 21-335-1
DATE: 12-09-2022
SHEET NUMBER: **S0.3**
SHEET TITLE: GENERAL STRUCTURAL NOTES

Special Inspections and Tests

The general contractor shall maintain a quality control program separate from the special inspection program provided by the owner. The owner or the owner's authorized agent, other than the contractor shall employ one or more approved agencies to provide special inspections and tests during construction. The contracor/subcontractor(s) shall designate a quality control representative for each area listed under special inspections. The general contractor shall be responsible for all subcontractor(s) providing quality control and shall be responsible for the entire quality control program.

1. General qualifications of inspectors and testing agencies:

a. The testing agencies are required to submit evidence of qualifications to perform the specified tests.

b. The inspectors must show evidence of qualifed professional staff employed to make inspections.

c. Laboratory testing facilities must be accredited by a nationally recognized agency such as The American Association for Laboratory Accreditation (AALA), The National Institute of Standards and Technology (NIST), The National Voluntary Laboratory Accreditation Program (NAVLAP), or The Washington Area Council Engineering Laboratories (WACEL).

2. The special inspector(s) shall review the project plans, specifications, and construction schedule to become familiar with the scope of the inspection and testing services required.

3. The special inspector(s) shall keep records of their inspections.

4. The special inspector(s) shall furnish inspection reports to The Authority Having Jurisdiction and the Registered Design Professional in Responsible Charge. Reports shall indicate that the work inspected was done in conformance to the approved construction documents.

5. Inspection of contractor's quality control shall include: Verification of dimensions, dimensional tolerance, location and number of items supplied; Verificaiton of proper care of construction materials during periods of deleterious weather conditions; and Verification of proper construction in terms of materials, location, dimensional tolerance, and constuction details.

6. The special inspector(s) shall bring discrepancies to the immediate attention of the contractor for correction. If discrepancies are not corrected, the special inspector(s) shall bring discrepancies to the attention of The Authority Having Jurisdiction and The Registered Design Professional in Reponsible Charge prior to completion of that phase of work.

7. The special inspector(s) shall periodically submit a report of inspections documenting required inspections and method of correction action of all discrepancies noted in the inspections at a frequency determined by The Authority Having Jurisdiction but no longer than once per month.

8. The special inspector(s) to verify that each fabricator maintains detailed fabrication and quality control proceedures that provide a basis for inspection, control of workmanship and the fabricator's ability to conform to the approved construction documents and referenced standards. The special inspector(s) shall review the proceedures for completeness and adequacy relative to the code requirements for the fabricator's scope of work. Special inspections are not required where the fabricator is apppoved as described below.

9. The special inspection of fabricators are not required where the work is done on the premises of a fabricator registered and approved to perform such work without special inspection per IBC Section 1704.2.5.1. Approval shall be based upon review of the fabrictor's written procedural and quality control manuals and periodic auditing of fabrication practices by an approved special inspection agency. At completion of fabrication, the approved fabricator shall submit a certificate of completion with the approved construction documents.

10. The constractor shall provide access to the work being inspected to the special inspector and shall provide a minimum of one business day notice of the intention of any work required to have special inspections. Regardless of the notice provided to the special inspector, all work performed without required special inspection will be subject to removal and replacement at the discretion of the Architect, Design Engineer, or Authority Having Jurisdiction at no cost to the owner.

11. Any rework required due to non-conformance with the construction doucments shall be performed by the contractor at no additional cost to the owner. Any rework required by the contractor due to non-conformance with the construction documents shall be noted and time kept separate the the special inspector(s). Any further inspection(s) and testing that are required due to non-conforming work shall be paid by the contractor.

12. Special inspector(s) and representative of the testing agencies are not authorized to alter any requirements of the contract documents nor approve or accept any portion of the work.

13. Structural work requiring special inspection and structural testings shall be as listed under "Special Inspection Tables and Structural Tests" as a minimum.

14. The items checked with an "X" shall be inspected in accordance with IBC Chapter 17 by a certified special inspector from an established testing agency. The testing agency shall send copies of all structural testing and inspection reports directly to the Architect, Structural Engineer, Contractor, and The Authority Having Jurisdication. Any material that fails to meet the project specifications shall immediately be brought to the attention of the Architect, Contractor, and Structural Engineer. Special inspection testing requirments apply equally to all bidder designed components.

15. Continuous Special Inspection means that the special inspector is on the site at all times observing the work requiring special inspection (IBC Chapter 2). Periodic Special Inspection means that the special inspector is on site at time intervals necssary to confirm that all work requiring special inspection is in compliance.

16. Perform these task for each welded joint, member, element, or bolted connection

17. The fabricator of erector, as applicable, shall maintain a system by which a welder who has welded a joint or member can be identified. Stamps, if used, shall be the low-stress type.

18. Inspection for prefabricated construction shall be the same as if the material used in the contruction took place on site. Continuous inspection will not be required during prefactrication if the approved agency certifies the construction and furnishes evidence of compliance.

19. The Geothechical Report and construction documents shall be used to determine compliance. The Geotechnical Engineer should also verify compliance.

SPECIAL INSPECTIONS TABLES AND STRUCTURAL TESTS (Note 14)

REQUIRED VERIFICATION OR INSPECTION	CONTINUOUS (Note 16)	PERIODIC (Note 16)	REFERENCED STANDARD	IBC REFERENCE	COMMENTS
A. Concrete Construction					
1. Inspect reinforcement and verify placement.		X	ACI 318: 3.5.7.1-7.7	1910.4	
2. Inspection of reinforcing steel welding and coupling in accordance with Table 1705.2.2, item 2b	X	X	AWS D1.4; ACI 318: 3.5.2		
3. Inspection of anchors cast in concrete prior to and during placement of concrete .		X	ACI 318; 8.1.3, 21.2.8	1908.5, 1909.1	
4. Inspection of anchors post-installed installed in hardened concrete		X	ACI 318: 3.8.6,8.1.3,21.2.8	1909.1	
5. Verify use of required design mix.		X	ACI 318: CH. 4, 5.2 - 5.4	1904.2.2, 1910.2, 1910.3	
6. At the time fresh concrete is sampled to fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete	X		ASTM C172 ASTM C31 ACI 318: 5.6, 5.8	1910.10	
7. Inspection of concrete placement for proper application techniques.	X		ACI 318: 5.9, 5.10	1910.6, 1910.7, 1910.8	
8. Inspection for maintenance of specified curing temperature and techniques.		X	ACI 318: 5.11-5.13	1910.9	
9. Erection of precast concrete members.		X	ACI 318: Ch. 16		
10. Inspect formwork for shape, location and dimension of the concrete member being formed.		X	ACI 318: 6.1.1		
B. Structural Steel Construction					
Inspections Tasks Prior to Welding					
1. Welding procedure specifications (WPS's) available	X		AISC 360-10 Table N5.4-1		Note 17
2. Manufacturer certification for welding consumables available	X				Note 17
3. Material identificaiton (type/grade)		X			
4. Welder identification system		X			Note 18
5. Fit-up groove weld (including joint geometry): a. Joint preparation b. Dimensions (alignment, root opening, root face, bevel) c. Cleanliness (condition of steel surfaces) d. Tacking (tack weld quality and location) e. Backing type and fit (if applicable)		X X X X X			
6. Configuration and finish of access holes		X			
7. Fit-up of fillet welds: a. Dimensions (alignment, gaps at root) b. Cleanliness (condition of steel surfaces) c. Tacking (tack weld qualify and location)		X X X			
Inspections Tasks During Welding					
1. Use of qualified welders		X	AISC 360-10 Table N5.4-2		
2. Control and handling of welding consumables a. Packaging b. Exposure control		X X			
3. No welding over cracked tack welds		X			
4. Environmental conditions a. Wind speed within limits b. Precipitation and temperature		X X			
5. Welding procedure specificaitons (WPS) followed: a. Settings and welding equipment b. Travel Speed c. Selected welding materials d. Shielding gas type/flow rate e. Preheat applied f. Interpass temperature maintained (min/max) g. Proper position (F,V,H, OH)		X X X X X X X			
6. Welding Techniques a. Interpass and final cleaning b. Each pass within profile limitations c. Each pass meets quality requirements		X X X			
Inspection Tasks After Welding.					
1. Welds cleaned		X	AISC 360-10 Table N5.4-3		
2. Size, lengh and location of welds	X				Note 17
3. Welds meet visual acceptance criteria a. Crack prohibition b. Weld/base-metal fusion c. Crater cross section d. Weld profiles e. Weld size f. Undercut g. Porosity	X X X X X X X				Note 17
4. Arc strikes	X				Note 17
5. k-area (When welding of doubler plates, continuity plates or stiffeners has been performed in the k-area, visually inspect the web k-area for cracks within 3 in. of the weld.	X				Note 17
6. Backing removed and weld tabs removed (if required)	X				Note 17
7. Repair activities	X				Note 17
8. Document acceptance or rejection of welded joint or member	X				Note 17
Frequency of Inspections for welding:					
1. Structural steel and cold-formed steel deck:					
a. Complete and partial joint penetration groove welds.	X		AWS D1.1 - 100% Field Welded shall be Ultrasonic Testing, or approved equal		Note 9
b. Multipass fillet welds	X		AWS D1.1 - 15% Liquid Dye Penetration - Remaining Visual Inspection		Note 9
c. Single-pass fillet welds > 5/16"	X				Note 9
d. Plug and slot welds.	X				Note 9
e. Single-pass fillet Welds <= 5/16"		X			Note 9
f. Floor and roof deck welds		X	AWS D1.3 - Visual Inspection		
Inspection Tasks Prior to Bolting:					
1. Manufacturer's certifications available for fastener materials.	X		AISC 360-10 Table N5.6-1		Note 17
2. Fasteners marked in accordance with ASTM requirements.		X			
3. Proper fsteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)		X			
4. Proper bolting procedure selected for joint detail.		X			
5. Connecting elements, including the appropprate faying surface condition and hole preparation, if specified, meet applicable requirements.		X			
6. Pre-installation verification testing by installation personnel observed and documented for fastender assemblies and methods used.		X			
7. Proper storage provided for bolts, nuts, washers and other fastener components.		X			
Inspection Tasks During Bolting:					
1. Fastener assemblies, of suitable condition, placed in all holes and washers (if required) are positioned as required.		X	AISC 360-10 Table N5.6-2		
2. Joints brought to the snug-tight condition prior to the pretensioning operation.		X			
3. Fastener component not turned by the wrench prevented from rotating.		X			
4. Fasteners are pretensioned in accordance with the RCSC Specifications, progressing systematically from the most rigid point toward the free edges.		X			
Inspection Tasks After Bolting					
1. Document acceptance or rejection of bolted connections.	X		AISC 360-10 Table N5.6-3		Note 17
Other Inspection Tasks					
1. Placement of anchor rods and other embedments supporting structural steel for compliance with the construction documents: a. Diameter b. Grade c. Type d. Embedment Length		X X X X			
2. Inspection of the fabricated steel or erected steel frame for compliance with the details shown on the construction documents: a. Braces b. Stiffeners c. Member locations d. Proper application of joint details at each connection		X X X X			
3. All shear keys under base plates shall be inspected for debris removal, water removal, and clear of any obstructions prior to placing flowable grout.		X			
Inspection of Steel Elements of Composite Construction Prior to Concrete Placement					
1. Placement of installation of steel deck:			AISC 360-10 Table N6.1		
a. For Welding of Steel Deck (Visual Inspection per AWS D1.3) i) Welding consumables ii) Welding proceedure iii) Specifications iv) Qualificaitons of welding personnel prior to start of work.	X X X X				Note 17
b. For Attachment other then Welding of Steel Deck i) Fasteners to be used prior to start of work ii) Observation of the progress to confirm installation in conformance with manufactuer's recommendations iii) Visual Inspection of the completed installation	X X X				Note 17
2. Placement and installation of steel headed stud anchors.	X				Note 17 and See Stud Placement Quality Control on Headed Stud Shear Shear Connector Typical Detail.
3. Document acceptance or rejection of steel elements.	X				Note 17
Required Verification and Inspection of steel construction other than structural steel					
1. Material verification of cold-formed steel deck:					
a. Identification marking to conform to ASTM standards specified in the approved construction documents.		X	Applicable ASTM material standards		
b. Manufactuer's certified test reports.		X			
2. Inspection of welding:					
a. Cold-formed steel deck: 1) Floor and roof deck welds.		x	AWS D1.3		
b. Reinforcing steel:					
1) Verification of weldability of reinforcing steel other than ASTM A 706		X	AWS D1.4 ACI 318: Section 3.5.2		
2) Shear reinforcement	X				
3) Other reinforcing steel.		X			
C. Soils					
1. Verify materials below shallow foundation are adequate to achieve the design bearing capacity		X			Note 20
2. Verify excavations are extended to proper depth and have reached proper material.		X			
3. Perform classification and testing of compacted fill materials.		X			
4. Verify use of proper materials, densities and lift thicknesses during placement and compaction of compacted fill.	X				
5. Prior to placement of compacted fill, observe subgrade and verify that site has been prepared properly.		X			
D. Sprayed Fire-Resistant Materials:					
1. Verify condition of substrates. a. Structural member surface conditions. b. Minimum ambient temperature c. Proper venting of area during and after installation.		X X X		1705.13.2; 1705.13.3	
2. Verify thickness of application.		X		1705.13.4	
3. Verify bond strength adhesion/cohesion.		X		1705.13.6	
4. Verify and record the condition of finished application.		X			
5. Verify mastic and intumescent fire-resistant coatings applied to structural elements and decks shall be in accordance with AWCI 12-B.		X		1705.14	
E. Masonry Construction (Level B Quality Assurance)					
1. Verification of Slump flow and Visual Stability Index (VSI) as delivered to the project site in accordance with ACI 530.1 Specification Article 1.5 B.1.b.3 for self-consolidating grout.		X	TMS 602/ACI 530.1/ASCE 6, Art 1.5 B.1.b.3		
2. Verification of f'm in accordance with ACI 530.1 Specification Article 1.4B prior to construction except where specifically except by ACI 530.1.		X	TMS 602/ACI 530.1/ASCE 6, Art 1.4B		
3. Verify compliance with approved submittals a. Grout mix design b. Mortar mix design c. Material Certificates -Reinforcement -Anchor Ties -Masonry Units e. Constuction proceedures -Cold weather construction proceedures -Hot weather construction proceedures	 X X X X		TMS 602/ACI 530.1/ASCE 6, Art 1.5		
4. As masonry construction begins, verify that the following are in compliance:					
a . Proportions of site-prepared mortar		X	TMS 602/ACI 530.1/ASCE 6, Art 2.1, 2.6A		
b. Construction of mortar joints		X	TMS 602/ACI 530.1/ASCE 6, Art 3.3B		
c. Location of reinforcement and connectors.		X	TMS 602/ACI 530.1/ASCE 6, Art 3.4, 3.6A		
5. Prior to grouting, verify the following are in compliance:					
a. Grout space		X	TMS 602/ACI 530.1/ASCE 6, Art 3.2D, 3.2F		
b. Grade, type, and size of reinforcement and anchor bolts		X	TMS 602/ACI 530.1/ASCE 6, Art 2.4, 3.4		TMS 402/ACI 530/ ASCE 5, Sec. 1.16
c. Placement of reinforcement and connectors		X	TMS 602/ACI 530.1/ASCE 6, Art 3.2E, 3.4, 3.6A		TMS 402/ACI 530/ ASCE 5, Sec. 1.16
d. Proportions of site-prepared grout		X	TMS 602/ACI 530.1/ASCE 6, Art 2.6B, 2.4 G.1.b		
e. Construction of mortar joints		X	TMS 602/ACI 530.1/ASCE 6, Art 3.3B		
6. Verify during construction:					
a. Size and location of structural elements		X	TMS 602/ACI 530.1/ASCE 6, Art 3.3F		
b. Type, size, and location of anchors, including other details of anchorage of masonry to structural members, frames or other construction		X	TMS 402/ACI 530/ ASCE 5, Sec. 1.16.4.3, 1.17.1		
c. Welding of reinforcing	X		TMS 402/ACI 530/ ASCE 5, Sec. 2.1.7.7.2, 3.3.3.4c, 8.3.3.4b		
d. Preparation, construction and protection of masonry during cold weather (temperature below 40 deg F (4.4 deg C)) and hot weather (temperatures above 90 deg. F(32.2 deg C)).		X	TMS 602/ACI 530.1/ASCE 6, Art 1.8C, 1.8D		
e. Placement of grout	X		TMS 602/ACI 530.1/ASCE 6, Art 3.5, 3.6C		
7. Observe preparation of grout specimens, mortar specimens, and/or prisms		X	TMS 602/ACI 530.1/ASCE 6, Art. 1.4 B.2.a.3, 1.4 B.2.b.3; 1.4 B.2.c.3, 1.4 B.3, 1.4 B.4		
F. Prefabricated Construction					Note 19

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Oklahoma Certificate of Authorization" No. 4570

CLIENT:

CHEROKEE NATION
REPLACEMENT HOSPITAL
TAHLEQUAH, OKLAHOMA

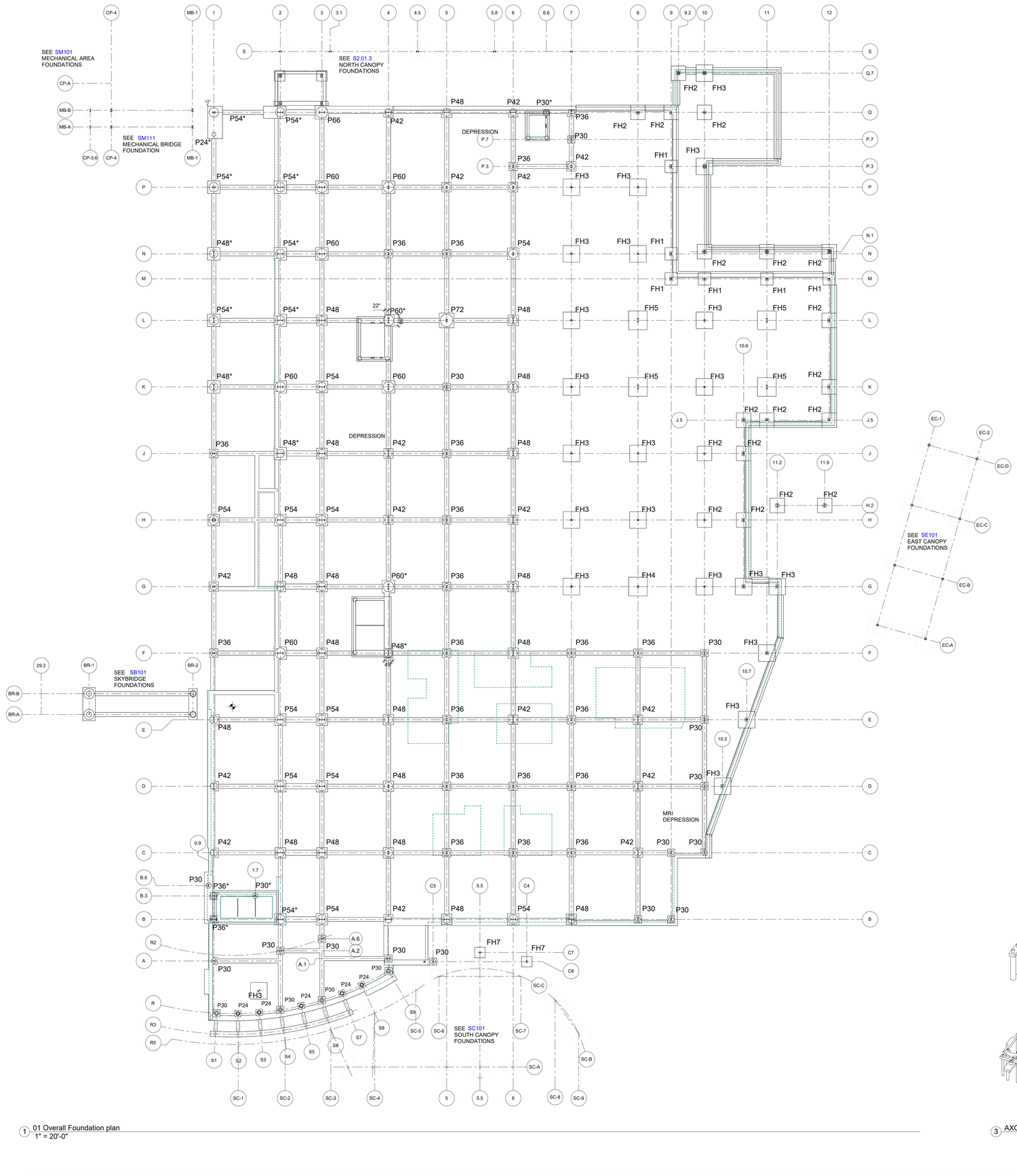
KEY PLAN:

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

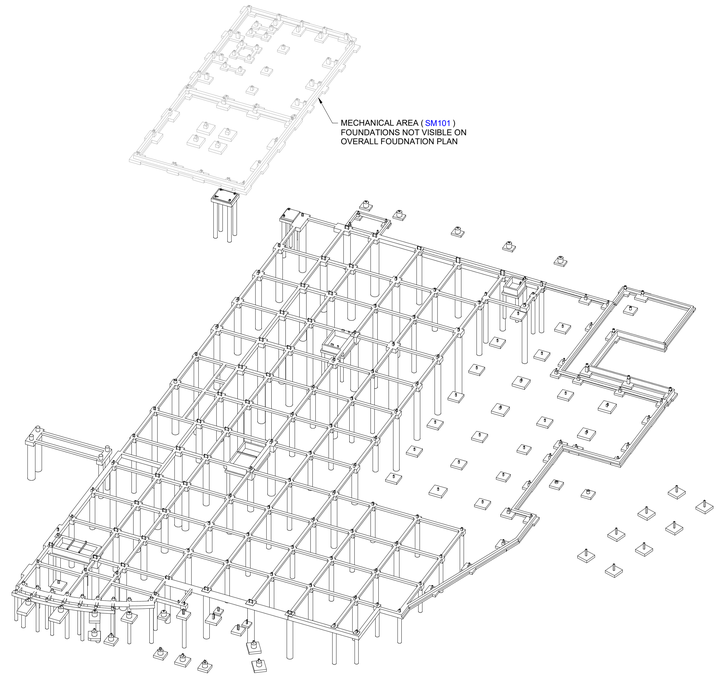
REVISIONS

#	DATE	DESCRIPTION

JOB NUMBER: 21-335-1
DATE: 12-09-2022
SHEET NUMBER: **S0.4**
SHEET TITLE: SPECIAL INSPECTIONS TABLE



1 01 Overall Foundation plan
1" = 20'-0"



3 AXONOMETRIC - FOUNDATION

HOSPITAL FOUNDATION OVERALL

1. TOP OF CONCRETE = 99" U.N.O.
2. PILES TAGGED WITH ASTERISK (*) DO NOT INCLUDE A PIER CAP PER SCHEDULE.
3. SEE SECTOR FOUNDATION PLANS FOR GRADE BEAM INFORMATION.
4. CENTER ALL FOUNDATION ELEMENTS UNDER COLUMNS U.N.O.
5. S1.01.2 - SEE SLAB PLAN FOR SLAB ON GRADE DIMENSIONS.
6. S1.01.3 - SEE ELEVATOR PIT PLAN FOR ELEVATOR SUBSTRUCTURE COMPONENTS.
7. S1.01.4 - SEE PILE PLAN FOR DRILLED PIER ELEVATIONS & LENGTHS.
8. S4.03 - SEE FOR DRILLED PIER & SPREAD FOOTING REINFOCEMENT, U.N.O.
9. S1.00.2 - SEE OVERALL GRID PLAN FOR ORIGIN, WORK-POINTS, RADII & DIMENSIONS.

NOMENCLATURE
- PILASTER / PIER CAP DIMENSIONS SHOWN ON PLAN - LENGTH (in) x WIDTH (in) x DEPTH (ft).
- FOOTING DIMENSIONS SHOWN ON PLAN - LENGTH (ft-in) x WIDTH (ft-in) x DEPTH (ft-in).

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REVISIONS		
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JOB NUMBER: 21-335-1

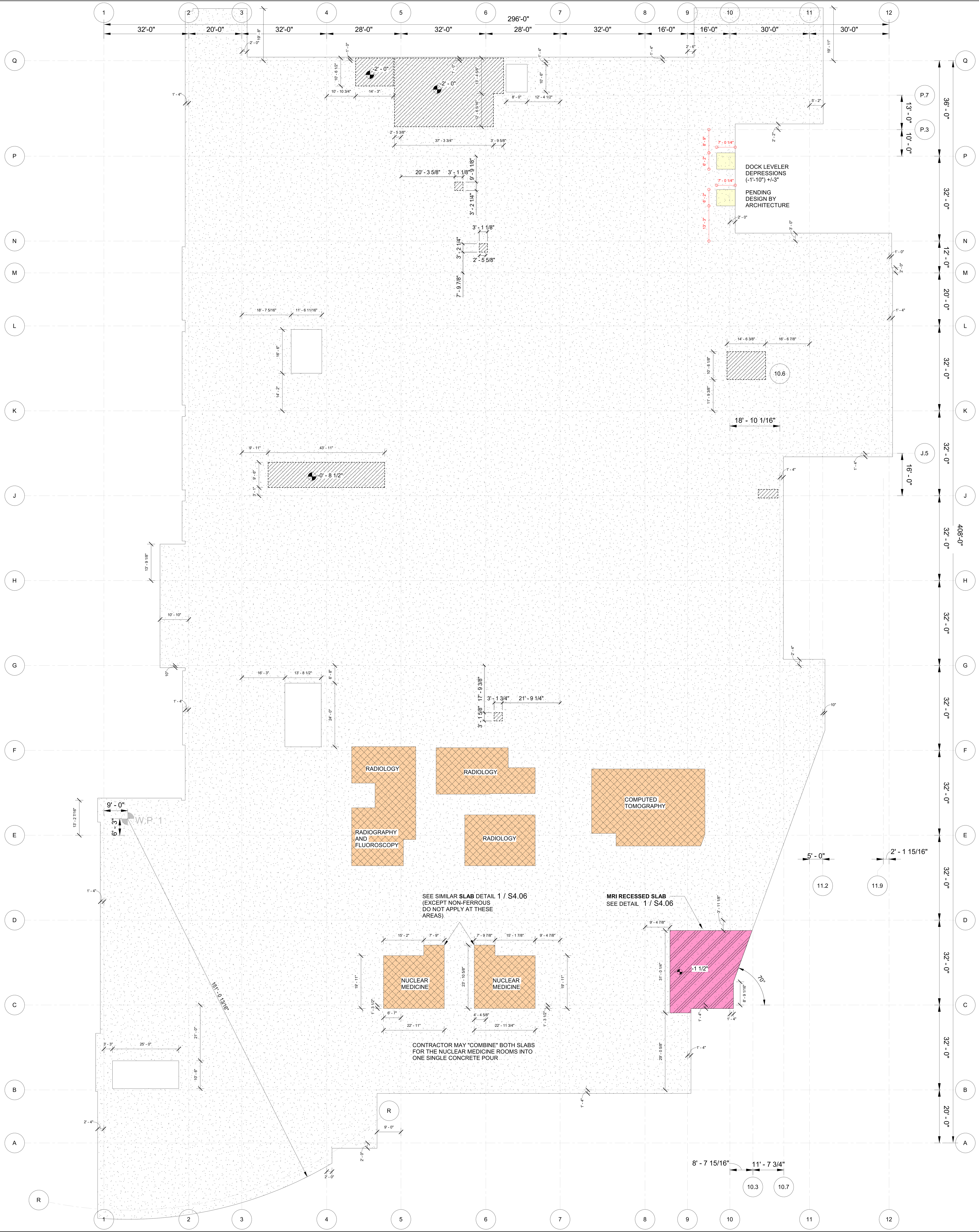
DATE: 12-09-2022

SHEET NUMBER:

S1.01.1

SHEET TITLE:

01 OVERALL FOUNDATION PLAN



1 01 Slab plan
1/16" = 1'-0"

SLAB ON GRADE NOTES:

1. FINISHED FLOOR EL: +100'-0" = DATUM EL: 919.00'
2. SLAB OPENING / DEPRESSION DIMENSIONS AND LOCATIONS.
   A. TRADES SHALL COORDIANTE WITH THESE DRAWINGS FOR FINAL DEPRESSION LOCATIONS AND DIMENSIONS. DIMENSIOSN SHOWN ARE REFERENCED TO LINKED ARCHITECTURAL MODEL.
   B. TRADES REQUIRING MINIMUM OPENINGS OR RESTRICTRED LOCATIONS SHOULE VERIFY SLAB INFORMATION WITH THESE DRAWINGS.
3. GC SHALL REVIEW ALL FLOOR OPENING & DEPRESSION REQUIREMENT WITH FINAL MECHANICAL EQUIPMENT .

SLAB ON GRADE SYSTEM

▨ 100 PSF

TYPICAL SLAB ON GRADE (UNO)
A. 5" NORMAL WEIGHT CONCRETE (4000 PSI)
B. 15ML POLY VAPORT BARRIOR
C. 4" CRUSHED 3/4" STONE
D. SLAB REINFORCEMENT: #4 @ 18" E.W. AND CENTERED IN SLAB, U.N.O.

▨ T.B.D.

MRI SLAB IS RECESSED 1 1/2" B.F.F.
A. 8" NORMAL WEIGHT CONCRETE (4000 PSI)
B. 15ML POLY VAPORT BARRIOR
C. 8" CRUSHED 3/4" STONE
D. SLAB REINFORCEMENT: #4 @ 16" E.W. CENTER IN SLAB
   • NON-METTALIC (FIBERGLAS)
E. ISOLATED PERIMETER W/ NON-METTALIC 3/4" DOWELS
F. CONTRACTOR TO REVIEW ADDITIONAL SPECIAL REQUIREMENTS BY MRI VENDOR

▨ PER PLAN

TYPICAL RECESSED SLAB (UNO)
A. MATCH SURROUNDING FLOOR REQUIREMETNS
B. ELEVATION PER PLAN (-3" UNO)

▨

ROOMS WITH SPECIAL MEDICAL EQUIPMENT
A. CONTRACTOR TO REVIEW SPECIAL REQUIREMENTS BY MEDICAL EQUIPMENT VENDORS

EQUIPMENT NOTES:
Below are equipment types identified in Vol 3, on drawing A1.10B and their GE product model numbers being designed for. In all situations the GC shall be familiar with the equipment's vibration specification, equipment's structural notes and all other environment considerations for each item. In all situations the GC may need to contract a vibration consultant to attenuate vibration to meet GE's design specification and implement appropriate solution(s). The project structural documents are limited to slab thickness, suggested isolated floor slabs and the use of non-ferrous slab reinforcement (i.e., ref S4.06 for MRI slab). Below are some items that the GC shall be aware of. i.e. floor flatness, concrete strength minimum and thickness minimum and sub floor mounting plates or slab suppression minimum.

MRI Slab Notes (Based on GE Healthcare Signa Artist / Optima MR450W)
1. The Owner may hire a qualified acoustics and vibration engineer to further attenuate the transmitted noise and vibration, if required.
2. Contractor shall supply patient table dock anchors. Anchors shall be electrically conductive and non-magnetic. See Patient Table Dock Anchor Mounting Requirements for additional information.

NM (Nuclear Medicine) Slab Notes (Based on GE Healthcare NM830)
1. Concrete strength, f'c, in floor shall be 4,500 psi minimum.
2. Floor surface shall be placed in a single pour.
3. Slope: Within 3mm (0.125 in) over 3048mm (120 in.)
4. Floor levelness must be smooth, with deviations of no more than 5mm (0.195 in.) between depressions and high spots in any 1500mm (59 in.) throughout the room or system area.
5. General contractor may need to contract a vibration consulting. See product Environment (vibration) specifications.
6. Refer to unit specification for gantry anchors (Hilti-HSL-3). The selected anchoring method must have a pulling tensile force of 19.7kN on each of the anchors bolting the NM gantry to the floor.

CT (COMPUTED TOMOGRAPHY) Slab Notes (Based in GE Healthcare Revolution Apex)
1. Floor slabs on which equipment is to be installed must be level to ¼" in 10'-0".
2. Refer to unit specifications for gantry anchors installation requirements.

RF (RADIOGRAPH AND FLUOROSCOPY) Slab Notes (Based on GE Healthcare Discovery RF180)
1. Contractor shall install a grout pad to meet ground a maximum tolerance for leveling of 0.2" per 10 feet. The maximum grout pad thickness is 0.25 inches.
2. Refer to unit specification for underfloor plate anchors.

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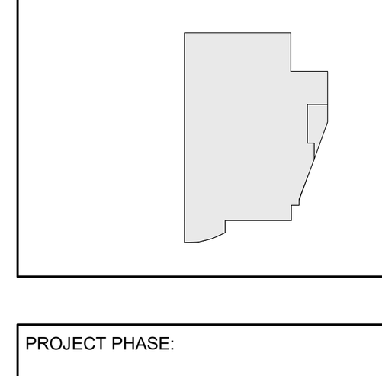
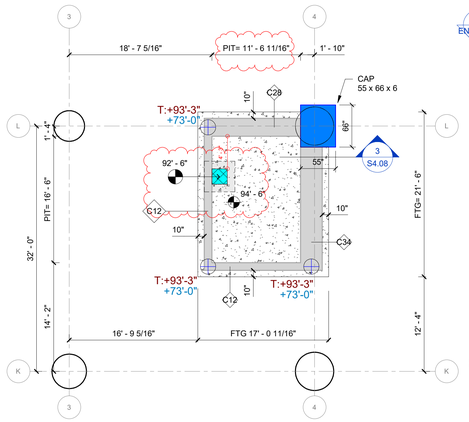
KEY PLAN:


PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

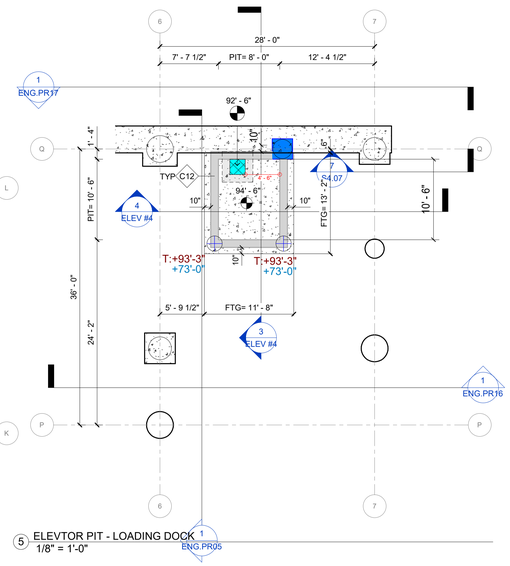
REVISIONS

#	DATE	DESCRIPTION

JOB NUMBER: 21-335-1
DATE: 12-09-2022
SHEET NUMBER: **S1.01.2**
SHEET TITLE: 01 SLAB PLAN



6 ELEVTOR PIT - STAFF
1/8" = 1'-0"



5 ELEVTOR PIT - LOADING DOCK
1/8" = 1'-0"

8 ELEVTOR PIT - PUBLIC
1/8" = 1'-0"

7 ELEVTOR PIT - PATIENT TRANSFER
1/8" = 1'-0"

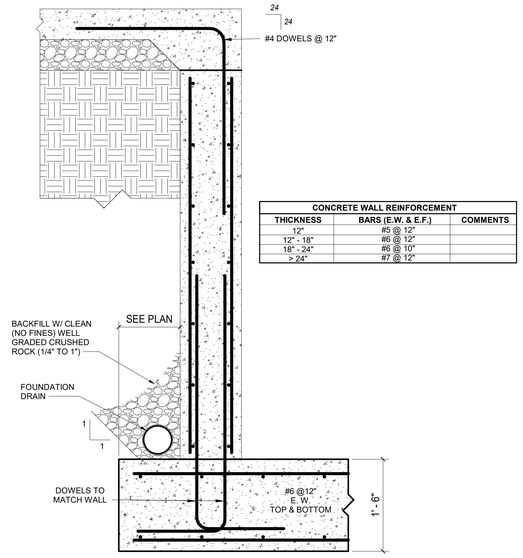
NOTES

DRILLED PIERS UNDER ELEVATOR PIT SLABS ARE P0 (24") AND CENTERED BASED ON THE WALLS ABOVE THEM U.N.O.
*BOTTOM OF PIER ELEVATION SHOWN ON PLAN VIEW *# ft - # in*

*TOP OF SUMP PIT AND PIT SLAB SHOWN ON PLAN VIEW

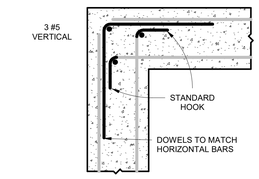
*TOP OF ELEVATOR PIT CONCRETE WALLS ELEVATION: 99' - 0" U.N.O.
*THICKNESS OF CONCRETE WALLS SHOWN ON PLAN VIEW

*ELEVATOR DIMENSIONS ARE BASED ON ELEVATOR VENDOR PRELIMINAR INFORMATION, SUBJECT TO CHANGE AFTER FINAL EQUIPMENT IS PURCHASED,

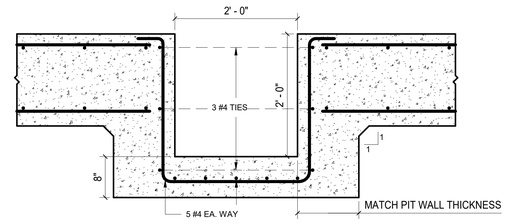


CONCRETE WALL REINFORCEMENT		
THICKNESS	BARS (E.W. & E.F.)	COMMENTS
12"	#5 @ 12"	
12" - 18"	#6 @ 12"	
18" - 24"	#6 @ 10"	
> 24"	#7 @ 12"	

3 TYPICAL ELEVATOR PIT SLAB AND WALLS

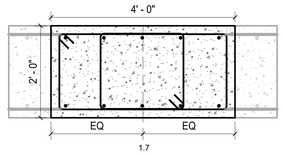


4 TYPICAL WALL CONCRETE CORNER

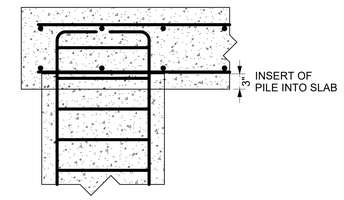


2 TYPICAL SUMP PIT DETAIL

COLUMN 48 x 24



9 CONCRETE COLUMN IN ELEVATOR CONCRETE WALL
3/4" = 1'-0"



1 TYPICAL PILE INTO ELEVATOR PIT SLAB DETAIL

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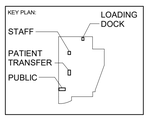
TAHLEQUAH, OKLAHOMA

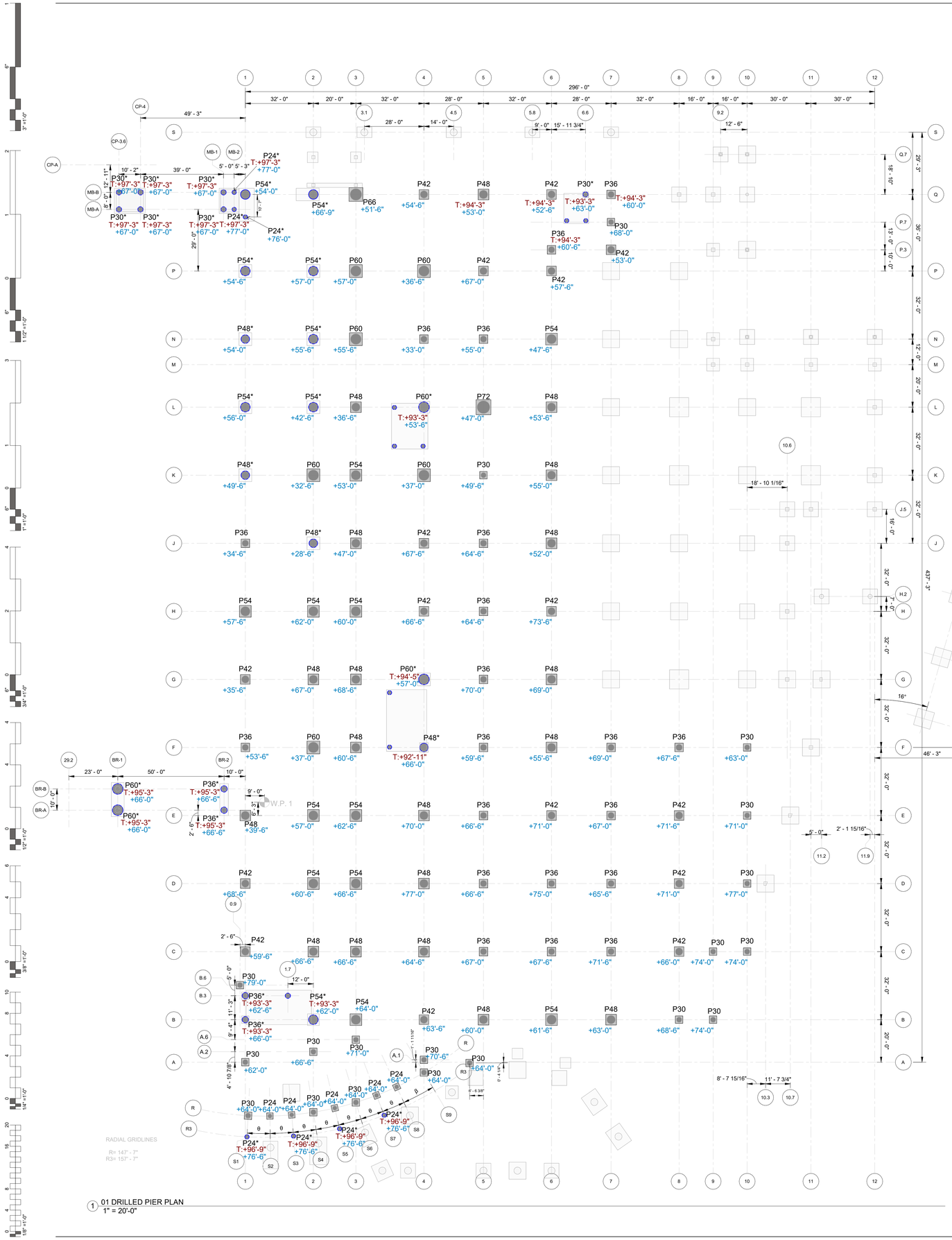
KEY PLAN:



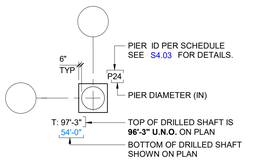
PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS		
#	DATE	DESCRIPTION
1	Date 1	Revision 1

JOB NUMBER: 21-335-1
DATE: 12-09-2022
SHEET NUMBER: **S1.01.3**
SHEET TITLE: ELEVATOR PIT PLAN

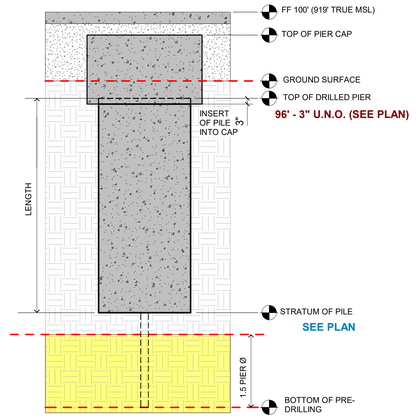


1 01 DRILLED PIER PLAN
1" = 20'-0"



1. F.F. EL: +100'-0" REFERENCES CIVIL DATUM 919.00'
2. PIER ID CONTAINING AN ASTRIX (*) DO NOT HAVE A PILE CAP BUT STILL EXTENDS 3" INTO THE FOUNDATION OR CONCRETE BEAM ABOVE

PIER KEY
3/32" = 1'-0"



DRILLED PIER NOTES

1. PIERS TAGGED WITH AN ASTERISK (*) IS NOT TYPICAL
   A. SEE OVERALL AND SECTOR FOUNDATION PLANS OR FOUNDATION DETAILS FOR NON-TYPICAL PIER CAP DIMENSIONS.
2. SEE ELEVATOR PIT PLAN FOR ADDITIONAL PIERS NOT INCLUDED ON THIS DRAWING.

PILE SCHEDULE

LOCATION	TYPE MARK	STRATUM (TRUE MSL)	STRATUM (PROJECT)	TOP OF DRILLED PIER (PROJECT)	LENGTH
0.9-B.6	P30	898' - 0"	79' - 0"	96' - 3"	17' - 3"
1-A	P30	881' - 0"	62' - 0"	96' - 3"	34' - 3"
1-B	P36*	885' - 0"	66' - 0"	93' - 3"	27' - 3"
1-B.3	P36*	881' - 6"	62' - 6"	93' - 3"	30' - 9"
1-C	P42	878' - 6"	59' - 6"	96' - 3"	36' - 9"
1-D	P42	887' - 6"	68' - 6"	96' - 3"	27' - 9"
1-E	P48	858' - 6"	39' - 6"	96' - 3"	56' - 9"
1-F	P36	872' - 6"	53' - 6"	96' - 3"	42' - 9"
1-G	P42	854' - 6"	35' - 6"	96' - 3"	60' - 9"
1-H	P54	876' - 6"	57' - 6"	96' - 3"	38' - 9"
1-J	P36	853' - 6"	34' - 6"	96' - 3"	61' - 9"
1-K	P48*	868' - 6"	49' - 6"	96' - 3"	46' - 9"
1-L	P54*	875' - 0"	56' - 0"	96' - 3"	40' - 3"
1-N	P48*	873' - 0"	54' - 0"	96' - 3"	42' - 3"
1-P	P54*	873' - 6"	54' - 6"	96' - 3"	41' - 9"
1-Q	P54*	873' - 0"	54' - 0"	96' - 3"	42' - 3"
1-Q (-10' - 7")	P24*	895' - 0"	76' - 0"	96' - 3"	20' - 3"
2-A.2	P30	885' - 6"	66' - 6"	96' - 3"	29' - 9"
2-B	P54*	881' - 0"	62' - 0"	93' - 3"	31' - 3"
2-C	P48	885' - 6"	66' - 6"	96' - 3"	29' - 9"
2-D	P54	879' - 6"	60' - 6"	96' - 3"	35' - 9"
2-E	P54	876' - 0"	57' - 0"	96' - 3"	39' - 3"
2-F	P60	856' - 0"	37' - 0"	96' - 3"	59' - 3"
2-G	P48	886' - 0"	67' - 0"	96' - 3"	29' - 3"
2-H	P54	881' - 0"	62' - 0"	96' - 3"	34' - 3"
2-J	P48*	847' - 6"	28' - 6"	96' - 3"	67' - 9"
2-K	P60	851' - 6"	32' - 6"	96' - 3"	63' - 9"
2-L	P54*	861' - 6"	42' - 6"	96' - 3"	53' - 9"
2-N	P54*	874' - 6"	55' - 6"	96' - 3"	40' - 9"
2-P	P54*	876' - 0"	57' - 0"	96' - 3"	39' - 3"
2-Q	P54*	885' - 9"	66' - 9"	96' - 3"	29' - 6"
3-A.6	P30	890' - 0"	71' - 0"	96' - 3"	25' - 3"
3-B	P54	883' - 0"	64' - 0"	96' - 3"	32' - 3"
3-C	P48	885' - 6"	66' - 6"	96' - 3"	29' - 9"
3-D	P54	885' - 6"	66' - 6"	96' - 3"	29' - 9"
3-E	P54	881' - 6"	62' - 6"	96' - 3"	33' - 9"
3-F	P48	879' - 6"	60' - 6"	96' - 3"	35' - 9"
3-G	P48	887' - 6"	68' - 6"	96' - 3"	27' - 9"
3-H	P54	879' - 0"	60' - 0"	96' - 3"	36' - 3"
3-J	P48	866' - 0"	47' - 0"	96' - 3"	49' - 3"
3-K	P54	872' - 0"	53' - 0"	96' - 3"	43' - 3"
3-L	P48	855' - 6"	36' - 6"	96' - 3"	59' - 9"
3-N	P60	874' - 6"	55' - 6"	96' - 3"	40' - 9"
3-P	P60	876' - 0"	57' - 0"	96' - 3"	39' - 3"
3-Q	P66	870' - 6"	51' - 6"	96' - 3"	44' - 9"
4-A.1	P30	889' - 6"	70' - 6"	96' - 3"	25' - 9"
4-B	P42	882' - 6"	63' - 6"	96' - 3"	32' - 9"
4-C	P48	883' - 6"	64' - 6"	96' - 3"	31' - 9"
4-D	P48	896' - 0"	77' - 0"	96' - 3"	19' - 3"
4-E	P48	889' - 0"	70' - 0"	96' - 3"	26' - 3"
4-F	P48*	885' - 0"	66' - 0"	92' - 11"	26' - 11"
4-G	P60*	876' - 0"	57' - 0"	94' - 5"	37' - 5"
4-H	P42	885' - 6"	66' - 6"	96' - 3"	29' - 9"
4-J	P42	886' - 6"	67' - 6"	96' - 3"	28' - 9"
4-K	P60	856' - 0"	37' - 0"	96' - 3"	59' - 3"
4-L	P60*	872' - 6"	53' - 6"	93' - 3"	39' - 9"
4-N	P36	852' - 0"	33' - 0"	96' - 3"	63' - 3"
4-P	P60	855' - 6"	36' - 6"	96' - 3"	59' - 9"
4-Q	P42	873' - 6"	54' - 6"	96' - 3"	41' - 9"
5(-6'-6 3/8")-A(-4 1/4")	P30	883' - 0"	64' - 0"	96' - 3"	32' - 3"
5-B	P48	879' - 0"	60' - 0"	96' - 3"	36' - 3"
5-C	P36	886' - 0"	67' - 0"	96' - 3"	29' - 3"
5-D	P36	885' - 6"	66' - 6"	96' - 3"	29' - 9"
5-E	P36	885' - 6"	66' - 6"	96' - 3"	29' - 9"
5-F	P36	878' - 6"	59' - 6"	96' - 3"	36' - 9"
5-G	P36	889' - 0"	70' - 0"	96' - 3"	26' - 3"
5-H	P36	883' - 6"	64' - 6"	96' - 3"	31' - 9"
5-J	P36	883' - 6"	64' - 6"	96' - 3"	31' - 9"
5-K	P30	868' - 6"	49' - 6"	96' - 3"	46' - 9"
5-L	P72	866' - 0"	47' - 0"	96' - 3"	49' - 3"
5-N	P36	874' - 0"	55' - 0"	96' - 3"	41' - 3"
5-P	P42	886' - 0"	67' - 0"	96' - 3"	29' - 3"
5-Q	P48	872' - 0"	53' - 0"	94' - 3"	41' - 3"
6-B	P54	880' - 6"	61' - 6"	96' - 3"	34' - 9"
6-C	P36	886' - 6"	67' - 6"	96' - 3"	28' - 9"
6-D	P36	894' - 0"	75' - 0"	96' - 3"	21' - 3"
6-E	P42	890' - 0"	71' - 0"	96' - 3"	25' - 3"
6-F	P48	874' - 6"	55' - 6"	96' - 3"	40' - 9"
6-G	P48	888' - 0"	69' - 0"	96' - 3"	27' - 3"
6-H	P42	892' - 6"	73' - 6"	96' - 3"	22' - 9"
6-J	P48	871' - 0"	52' - 0"	96' - 3"	44' - 3"
6-K	P48	874' - 0"	55' - 0"	96' - 3"	41' - 3"
6-L	P48	872' - 6"	53' - 6"	96' - 3"	42' - 9"
6-N	P54	866' - 6"	47' - 6"	96' - 3"	48' - 9"
6-P	P42	876' - 6"	57' - 6"	96' - 3"	38' - 9"
6-P.3	P36	879' - 6"	60' - 6"	94' - 3"	33' - 9"
6-Q	P42	871' - 6"	52' - 6"	94' - 3"	41' - 9"
6.6-Q	P30*	882' - 0"	63' - 0"	93' - 3"	30' - 3"
7-B	P48	882' - 0"	63' - 0"	96' - 3"	33' - 3"
7-C	P36	890' - 6"	71' - 6"	96' - 3"	24' - 9"
7-D	P36	884' - 6"	65' - 6"	96' - 3"	30' - 9"
7-E	P36	886' - 0"	67' - 0"	96' - 3"	29' - 3"
7-F	P36	888' - 0"	69' - 0"	96' - 3"	27' - 3"
7-P.3	P42	872' - 0"	53' - 0"	96' - 3"	43' - 3"
7-P.7	P30	887' - 0"	68' - 0"	96' - 3"	28' - 3"
7-Q	P36	879' - 0"	60' - 0"	94' - 3"	34' - 3"
8-B	P30	887' - 6"	68' - 6"	96' - 3"	27' - 9"
8-C	P42	885' - 0"	66' - 0"	96' - 3"	30' - 3"
8-D	P42	890' - 0"	71' - 0"	96' - 3"	25' - 3"
8-E	P42	890' - 6"	71' - 6"	96' - 3"	24' - 9"
8-F	P36	886' - 6"	67' - 6"	96' - 3"	28' - 9"
9-B	P30	893' - 0"	74' - 0"	96' - 3"	22' - 3"
9-C	P30	893' - 0"	74' - 0"	96' - 3"	22' - 3"
10-C	P30	893' - 0"	74' - 0"	96' - 3"	22' - 3"
10-D	P30	896' - 0"	77' - 0"	96' - 3"	19' - 3"
10-E	P30	890' - 0"	71' - 0"	96' - 3"	25' - 3"
10-F	P30	882' - 0"	63' - 0"	96' - 3"	33' - 3"
BR-1-BR-A	P60*	885' - 0"	66' - 0"	95' - 3"	29' - 3"
BR-1-BR-B	P60*	885' - 0"	66' - 0"	95' - 3"	29' - 3"
BR-2-BR-A	P36*	885' - 6"	66' - 6"	95' - 3"	28' - 9"
BR-2-BR-B	P36*	885' - 6"	66' - 6"	95' - 3"	28' - 9"
CP-3.6-MB-A	P30*	886' - 0"	67' - 0"	97' - 3"	30' - 3"
CP-3.6-MB-B	P30*	886' - 0"	67' - 0"	97' - 3"	30' - 3"
CP-4-MB-A	P30*	886' - 0"	67' - 0"	97' - 3"	30' - 3"
CP-4-MB-B	P30*	886' - 0"	67' - 0"	97' - 3"	30' - 3"
MB-1-MB-A	P30*	886' - 0"	67' - 0"	97' - 3"	30' - 3"
MB-1-MB-B	P30*	886' - 0"	67' - 0"	97' - 3"	30' - 3"
MB-2-MB-A	P24*	896' - 0"	77' - 0"	97' - 3"	20' - 3"
MB-2-MB-B	P24*	896' - 0"	77' - 0"	97' - 3"	20' - 3"
S1-R	P30	883' - 0"	64' - 0"	96' - 3"	32' - 3"
S1-R3	P24*	895' - 6"	76' - 6"	96' - 9"	20' - 3"
S2-R	P24	883' - 0"	64' - 0"	96' - 3"	32' - 3"
S3-R	P24	883' - 0"	64' - 0"	96' - 3"	32' - 3"
S3-R3	P24*	895' - 6"	76' - 6"	96' - 9"	20' - 3"
S4-R	P30	883' - 0"	64' - 0"	96' - 3"	32' - 3"
S5-R	P24	883' - 0"	64' - 0"	96' - 3"	32' - 3"
S5-R3	P24*	895' - 6"	76' - 6"	96' - 9"	20' - 3"
S6-R	P30	883' - 0"	64' - 0"	96' - 3"	32' - 3"
S7-R	P24	883' - 0"	64' - 0"	96' - 3"	32' - 3"
S7-R3	P24*	895' - 6"	76' - 6"	96' - 9"	20' - 3"
S8-R	P24	883' - 0"	64' - 0"	96' - 3"	32' - 3"
S9-R	P30	883' - 0"	64' - 0"	96' - 3"	32' - 3"

Grand total: 132

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Oklahoma Certificate of Authorization" No. 4570

CLIENT:

CHEROKEE NATION
REPLACEMENT HOSPITAL
TAHLEQUAH, OKLAHOMA

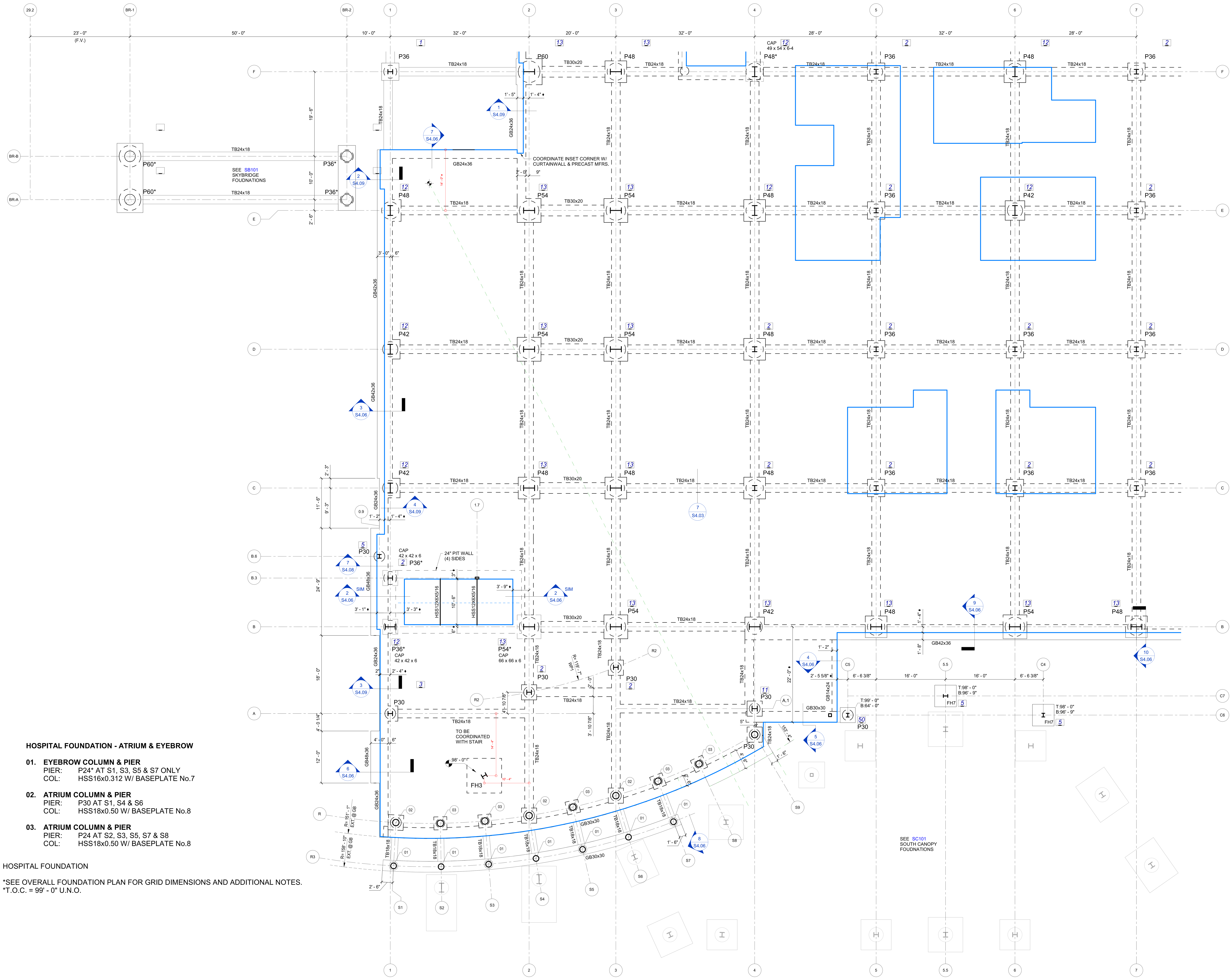
KEY PLAN:

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

#	DATE	DESCRIPTION

JOB NUMBER: 21-335-1
DATE: 12-09-2022
SHEET NUMBER: **S1.01.4**
SHEET TITLE: DRILLED PIER PLAN



HOSPITAL FOUNDATION - ATRIUM & EYEBROW

01. EYEBROW COLUMN & PIER
PIER: P24* AT S1, S3, S5 & S7 ONLY
COL: HSS16x0.312 W/ BASEPLATE No.7

02. ATRIUM COLUMN & PIER
PIER: P30 AT S1, S4 & S6
COL: HSS18x0.50 W/ BASEPLATE No.8

03. ATRIUM COLUMN & PIER
PIER: P24 AT S2, S3, S5, S7 & S8
COL: HSS18x0.50 W/ BASEPLATE No.8

HOSPITAL FOUNDATION

*SEE OVERALL FOUNDATION PLAN FOR GRID DIMENSIONS AND ADDITIONAL NOTES.
*T.O.C. = 99' - 0" U.N.O.

1 01 FOUNDATION PLAN - SECTOR 01
1/8" = 1'-0"

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12/9/22

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Oklahoma Certificate of Authorization" No. 4570

CLIENT:

SEAL OF THE CHEROKEE NATION
SEPT. 6, 1839

CHEROKEE NATION REPLACEMENT HOSPITAL

TAHLEQUAH, OKLAHOMA

KEY PLAN:

03
02
05
04
01

PROJECT PHASE:

BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

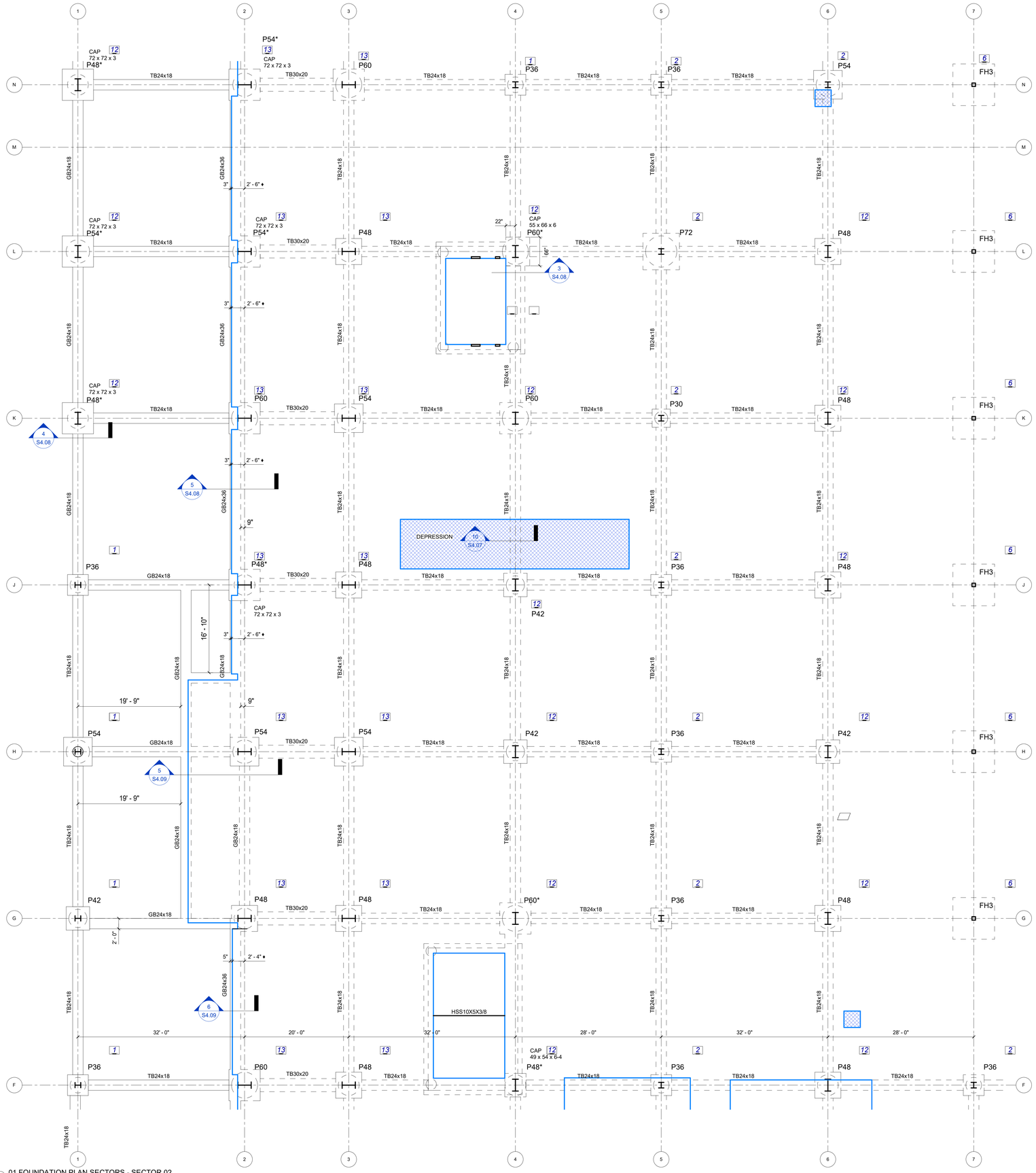
REVISIONS		
#	DATE	DESCRIPTION
1	Date 1	Revision 1

JOB NUMBER: 21-335-1

DATE: 12-09-2022

SHEET NUMBER: **S2.01.1**

SHEET TITLE: FOUNDATION PLAN SECTOR 01



1 01 FOUNDATION PLAN SECTORS - SECTOR 02
1/8" = 1'-0"

HOSPITAL FOUNDATION

*SEE OVERALL FOUNDATION PLAN FOR GRID DIMENSIONS AND ADDITIONAL NOTES.
*T.O.C. = 99' - 0" U.N.O.

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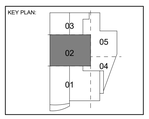
SEAL OF THE CHEROKEE NATION
SEPT. 6, 1839

CHEROKEE NATION REPLACEMENT HOSPITAL

TAHLEQUAH, OKLAHOMA

KEY PLAN:



PROJECT PHASE:

BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

#	DATE	DESCRIPTION

JOB NUMBER: 21-335-1

DATE: 12-09-2022

SHEET NUMBER:

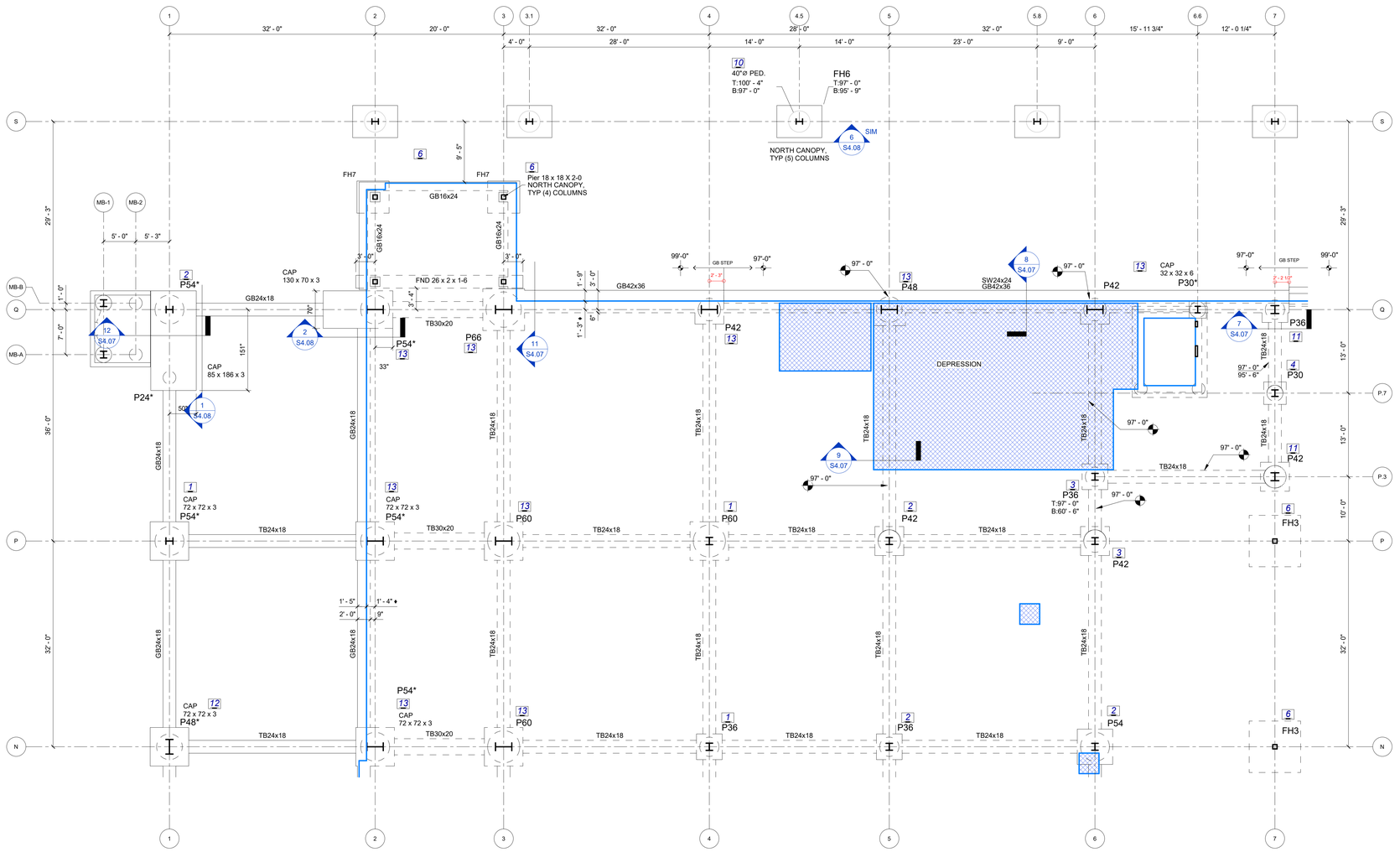
S2.01.2

SHEET TITLE:

FOUNDATION PLAN SECTOR 02

HOSPITAL FOUNDATION

*SEE OVERALL FOUNDATION PLAN FOR GRID DIMENSIONS AND ADDITIONAL NOTES.
*T.O.C. = 99' - 0" U.N.O.



1 01 FOUNDATION PLAN SECTORS - SECTOR 03
1/8" = 1'-0"

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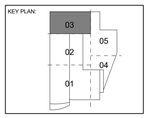
CLIENT:

THE SEAL OF THE CHEROKEE NATION SEPT. 6, 1839

CHEROKEE NATION REPLACEMENT HOSPITAL

TAHLEQUAH, OKLAHOMA

KEY PLAN:



PROJECT PHASE:

BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS		
#	DATE	DESCRIPTION

JOB NUMBER: 21-335-1

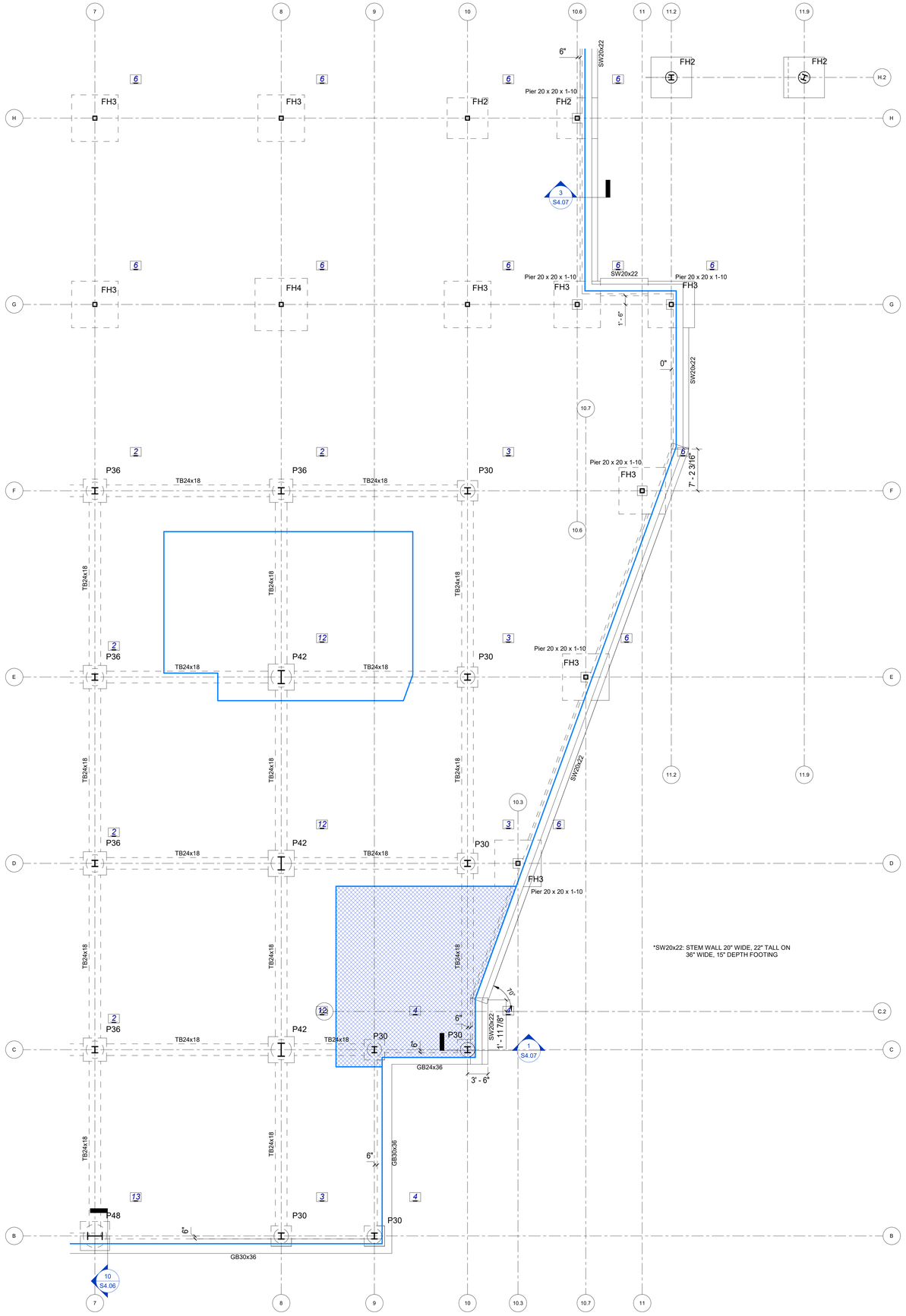
DATE: 12-09-2022

SHEET NUMBER:

S2.01.3

SHEET TITLE:

FOUNDATION PLAN SECTOR 03



1 01 FOUNDATION PLAN SECTORS - SECTOR 04
1/8" = 1'-0"

HOSPITAL FOUNDATION

*SEE OVERALL FOUNDATION PLAN FOR GRID DIMENSIONS AND ADDITIONAL NOTES.
*T.O.C. = 99' - 0" U.N.O.

*SW20x22: STEM WALL 20" WIDE, 22" TALL ON 36" WIDE, 15" DEPTH FOOTING

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CLIENT:

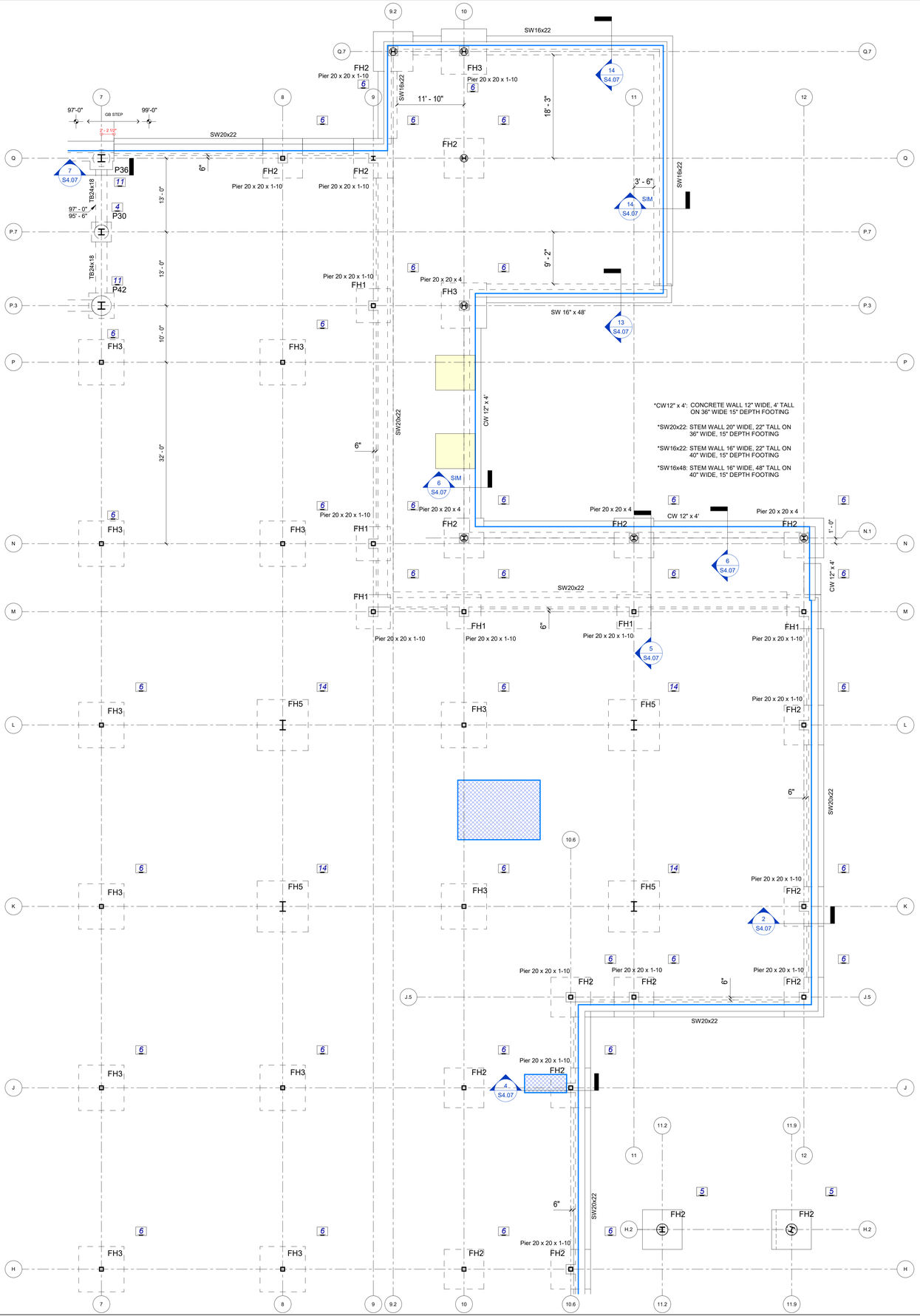
CHEROKEE NATION
REPLACEMENT HOSPITAL
TAHLEQUAH, OKLAHOMA

KEY PLAN:

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS		
#	DATE	DESCRIPTION

JOB NUMBER: 21-335-1
DATE: 12-09-2022
SHEET NUMBER: **S2.01.4**
SHEET TITLE: FOUNDATION PLAN SECTOR 04



1 01 FOUNDATION PLAN SECTORS - SECTOR 05
1/8" = 1'-0"

HOSPITAL FOUNDATION

*SEE OVERALL FOUNDATION PLAN FOR GRID DIMENSIONS AND ADDITIONAL NOTES.
*T.O.C. = 99' - 0" U.N.O.

*CW12" x 4": CONCRETE WALL 12" WIDE, 4' TALL ON 36" WIDE 15" DEPTH FOOTING

*SW20x22: STEM WALL 20" WIDE, 22" TALL ON 36" WIDE, 15" DEPTH FOOTING

*SW16x22: STEM WALL 16" WIDE, 22" TALL ON 40" WIDE, 15" DEPTH FOOTING

*SW16x48: STEM WALL 16" WIDE, 48" TALL ON 40" WIDE, 15" DEPTH FOOTING

James R. Childers Architect, Inc.
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Fort Smith, AR 72901
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PROFESSIONAL SEAL:

CONSULTANT LOGO:

Foy
Consulting & Engineering, LLC

Structural Engineering Services
6900 College Blvd. Suite 600
Overland Park, KS 66211
Ph. (913) 814-0404

Oklahoma Certificate of Authorization" No. 4570

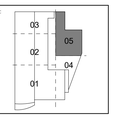
CLIENT:

THE SEAL OF THE CHEROKEE NATION SEPT. 6, 1839

CHEROKEE NATION REPLACEMENT HOSPITAL

TAHLEQUAH, OKLAHOMA

KEY PLAN:



PROJECT PHASE:

BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

#	DATE	DESCRIPTION

JOB NUMBER: 21-335-1

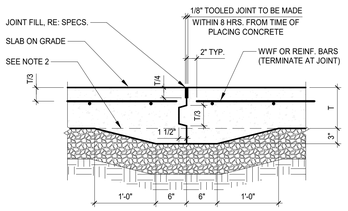
DATE: 12-09-2022

SHEET NUMBER:

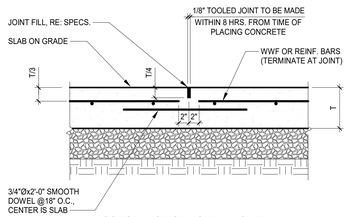
S2.01.5

SHEET TITLE:

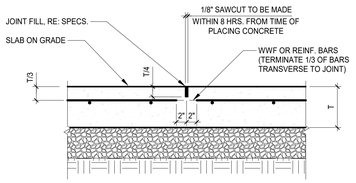
FOUNDATION PLAN SECTOR 05



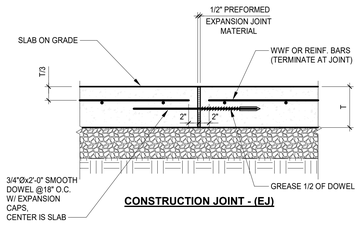
CONSTRUCTION JOINT - KEYED



CONSTRUCTION JOINT - DOWELED



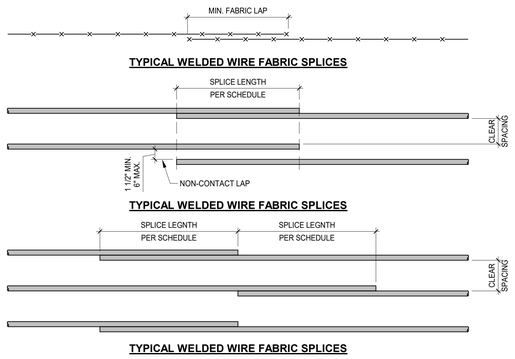
CONTROL JOINT - (CJ)



CONSTRUCTION JOINT - (EJ)

NOTES:

1. PROJECTING DOWEL TO BE GREASED PRIOR TO PLACEMENT OF ADJACENT SLAB. DOWELS TO BE SET STRAIGHT, PERPENDICULAR TO FORMWORK, AND PARALLEL TO EACH OTHER. PAINT WITH BOND BREAKER PRIOR TO PLACING ADJACENT SLAB. DO NOT LOCATE DOWELS CLOSER THAN 12" FROM EDGE OF SLAB).
2. CONTRACTOR HAS OPTION OF USING EITHER DOWELED OR KEYED CONSTRUCTION JOINT.
3. OMIT THICKENED PORTION OF SLAB WHERE SLAB THICKNESS IS 8" OR GREATER.
4. JOINTS NOTED ON PLAN BY (C.J.) MAY BE EITHER CONSTRUCTION OR CONTROL JOINTS AT CONTRACTOR'S OPTION. JOINTS TERMINATING A POUR ARE TO BE CONSTRUCTION JOINTS EXCEPT AT WALLS AND COLUMNS. THESE ARE TO BE ISOLATION JOINTS.
5. SEAL JOINTS IN EXPOSED FLOORS WITH JOINT SEALANT, RE: SPEC.
6. SEE PLANS FOR TYPICAL SLAB REINFORCING.
7. SAWCUT DEPTH TO BE "T/4" UNLESS EXCEEDING 1" DEPTH. IF "T/4 EXCEEDS 1", USE 1" MAX. DEPTH.
8. CONTRACTOR SHALL SUBMITT DRAWINGS SHOWING CJ & EJ FOR REVIEW BY THE ARCHITECT AND DESIGN TEAMS: STRUCTURAL, PLUMBING, MECHANICAL & CIVIL

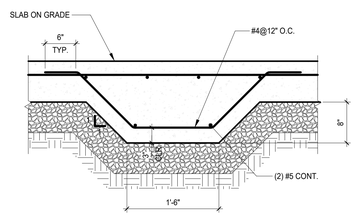


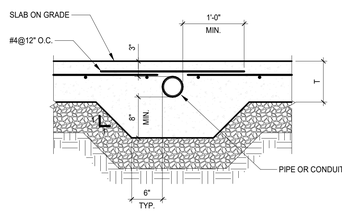
REINFORCING BAR TENSION LAP SCHEDULE (CLASS B)				
	f'_c = 3500 psi		f'_c = 4000 psi	
	NORMAL WEIGHT		NORMAL WEIGHT	
BAR SIZE	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS
#3	2'-2"	1'-8"	2'-1"	1'-7"
#4	2'-10"	2'-3"	2'-9"	2'-1"
#5	3'-7"	2'-9"	3'-4"	2'-7"
#6	4'-4"	3'-4"	4'-0"	3'-1"
#7	6'-10"	5'-4"	6'-6"	5'-0"
#8	8'-8"	6'-8"	8'-0"	6'-2"
#9	10'-5"	8'-0"	9'-9"	7'-6"
#10	12'-7"	9'-8"	11'-8"	9'-0"
#11	14'-9"	11'-4"	13'-9"	10'-7"

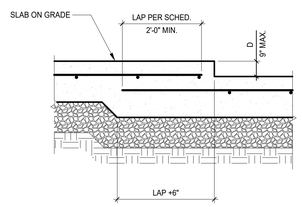
NOTES:

1. LAP BASED UPON CLEAR CONCRETE COVER AND THE CENTER TO CENTER SPACING OF THE BARS.
2. TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12 INCHES OF CONCRETE CAST BELOW THE BARS.
3. OTHER BARS INCLUDE VERTICAL BARS AND HORIZONTAL BARS WITH LESS THAN 12" OF CONCRETE CAST BELOW THE HORIZONTAL BARS.
4. BAR SPLICES NOT COVERED BY THIS SCHEDULE ARE SPECIFICALLY DETAILED AND DIMENSIONED ON PLANS.
5. MULTIPLY VALUES IN TABLE BY 1.3 FOR LIGHT WEIGHT CONCRETE.
6. MULTIPLY VALUES IN TABLE BY 1.2 FOR EPOXY COATED REBAR.

REINFORCING TENSION LAP SCHEDULE
2 TYPICAL DETAIL
SCALE: NO SCALE

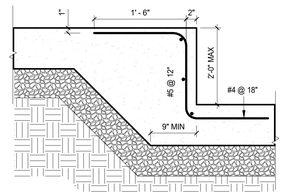






NOTES:

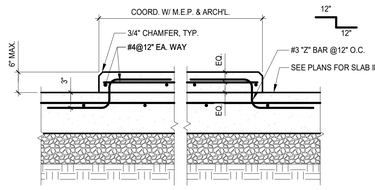
1. FOR SLAB DEPRESSIONS NOT GREATER THAN 9" MAX.



NOTES:

1. FOR SLAB DEPRESSIONS GREATER THAN 9", MAXIMUM 2'

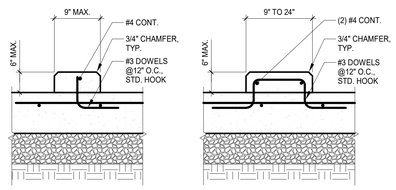
SLAB ON GRADE JOINTS
1 TYPICAL DETAIL
SCALE: NO SCALE



NOTES:

1. HOUSEKEEPING PADS ARE NOT TO BE CAST UNTIL SLAB ON GRADE HAS REACHED DESIGNED COMPRESSIVE STRENGTH. CONTRACTOR TO COORD. ANCHORAGE REQUIREMENTS FOR PARTICULAR EQUIPMENT SUPPLIED.

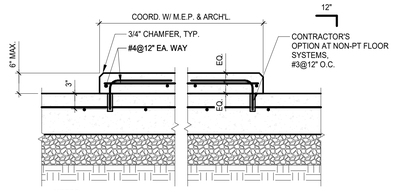
THICKENED SLAB
3 TYPICAL DETAIL
SCALE: NO SCALE



NOTES:

1. HOUSEKEEPING PADS ARE NOT TO BE CAST UNTIL SLAB ON GRADE HAS REACHED DESIGNED COMPRESSIVE STRENGTH. CONTRACTOR TO COORD. ANCHORAGE REQUIREMENTS FOR PARTICULAR EQUIPMENT SUPPLIED.

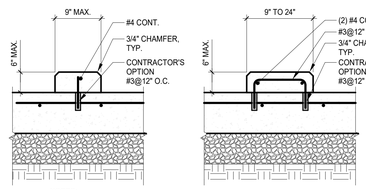
SLEEVE FOR PIPE IN SLAB
4 TYPICAL DETAIL
SCALE: NO SCALE



NOTES:

1. HOUSEKEEPING PADS ARE NOT TO BE CAST UNTIL SLAB ON GRADE HAS REACHED DESIGNED COMPRESSIVE STRENGTH. CONTRACTOR TO COORD. ANCHORAGE REQUIREMENTS FOR PARTICULAR EQUIPMENT SUPPLIED.
2. DRILL AND EPOXY DOWEL OPTION SHALL NOT BE USED AT PT SLABS OR BEAMS.
3. DRILL AND EPOXY 3" MINIMUM INTO SLAB W/ HY 200 SD ADHESIVE OR APPROVED EQUAL.

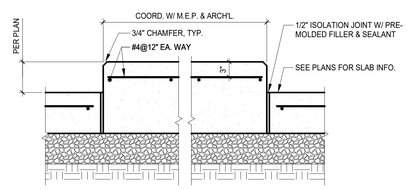
STEP IN SLAB
5 TYPICAL DETAIL
SCALE: NO SCALE



NOTES:

1. HOUSEKEEPING PADS ARE NOT TO BE CAST UNTIL SLAB ON GRADE HAS REACHED DESIGNED COMPRESSIVE STRENGTH. CONTRACTOR TO COORD. ANCHORAGE REQUIREMENTS FOR PARTICULAR EQUIPMENT SUPPLIED.
2. DRILL AND EPOXY DOWEL OPTION SHALL NOT BE USED AT PT SLABS OR BEAMS.
3. DRILL AND EPOXY 3" MINIMUM INTO SLAB W/ HY 200 SD ADHESIVE OR APPROVED EQUAL.

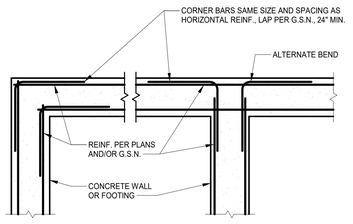
STEP IN SLAB 9" - 2'
6 TYPICAL DETAIL
SCALE: NO SCALE



NOTES:

1. HOUSEKEEPING PADS ARE NOT TO BE CAST UNTIL SLAB ON GRADE HAS REACHED DESIGNED COMPRESSIVE STRENGTH. CONTRACTOR TO COORD. ANCHORAGE REQUIREMENTS FOR PARTICULAR EQUIPMENT SUPPLIED.

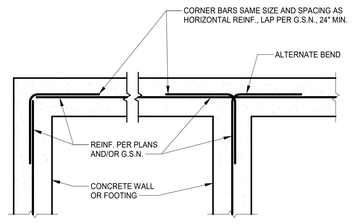
HOUSEKEEPING PAD - CAST IN PLACE DOWELS
7 TYPICAL DETAIL
SCALE: NO SCALE



CORNER **INTERSECTION**

NOTE:
1. VERTICAL REINFORCEMENT NOT SHOWN FOR CLARITY.

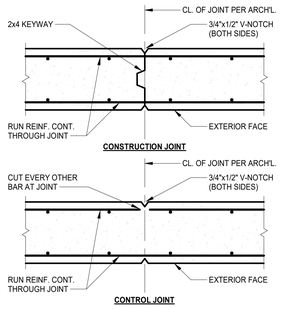
CONC. CURB - CAST IN PLACE DOWELS
8 TYPICAL DETAIL
SCALE: NO SCALE



CORNER **INTERSECTION**

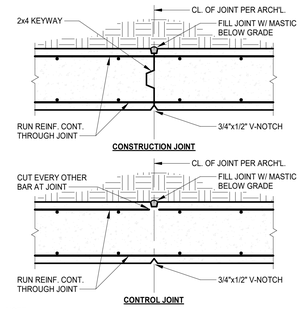
NOTE:
1. VERTICAL REINFORCEMENT NOT SHOWN FOR CLARITY.

HOUSEKEEPING PAD - DRILL AND EPOXY DOWELS
9 TYPICAL DETAIL
SCALE: NO SCALE



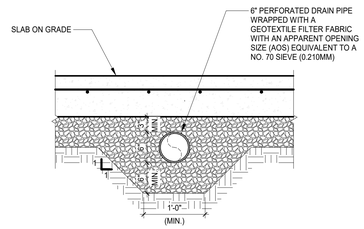
NOTE:
1. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL JOINT REQUIREMENTS.
2. PLACE AT 30'-0" O.C., MAX.

CONC. CURB - DRILL AND EPOXY DOWELS
10 TYPICAL DETAIL
SCALE: NO SCALE



NOTE:
1. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL JOINT REQUIREMENTS.
2. PLACE AT 30'-0" O.C., MAX.

ISOLATION PAD
11 TYPICAL DETAIL
SCALE: NO SCALE



NOTES:

1. RE: PLUMBING FOR LOCATION OF PERFORATED PIPE.

FOOTING OR CONC. WALL DOUBLE CORNER REINF.
12 TYPICAL DETAIL
SCALE: NO SCALE

FOOTING OR CONCRETE WALL CORNER REINF.
13 TYPICAL DETAIL
SCALE: NO SCALE

VERT. OR HORIZ. WALL JOINT
14 TYPICAL DETAIL
SCALE: NO SCALE

15 RETAINING WALL JOINT
SCALE: 1" = 1'-0"

16 UNDERSLAB DRAINAGE
SCALE: 1" = 1'-0"

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Oklahoma Certificate of Authorization" No. 4570

CLIENT:

THE SEAL OF THE CHEROKEE NATION — SEPT. 6, 1839

CHEROKEE NATION
REPLACEMENT HOSPITAL
TAHLEQUAH, OKLAHOMA

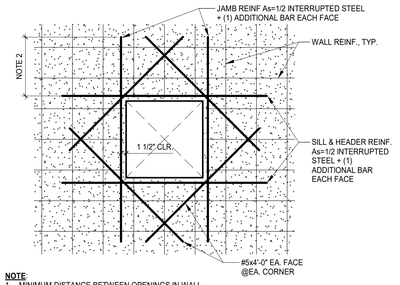
KEY PLAN:

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

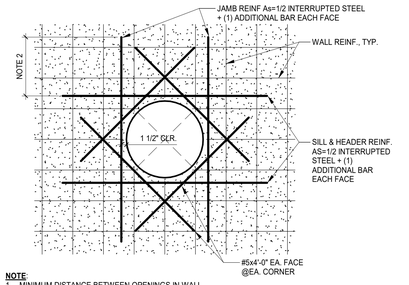
#	DATE	DESCRIPTION

JOB NUMBER: 21-335-1
DATE: 12-09-2022
SHEET NUMBER: **S4.01**
SHEET TITLE: CONCRETE TYPICAL DETAILS



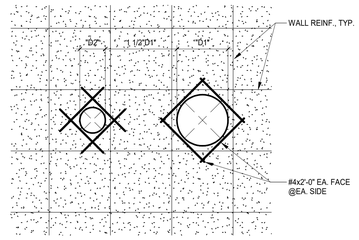
NOTE:
1. MINIMUM DISTANCE BETWEEN OPENINGS IN WALL TO BE 2'-0".
2. CLASS B SPLICE OR 2'-6" MIN. WHICHEVER IS GREATER.
3. AT OPENINGS 18" OR LESS IN DIAMETER OR ON ANY SIDE, NO EXTRA REBAR ARE REQUIRED UNLESS SHOWN OTHERWISE. SCHEDULED REBAR SHALL BE SPREAD (NOT CUT) TO ALLOW OPENING TO BE MADE. SUCH OPENINGS SHALL BE PROVIDED WITH A MIN. OF (1) #5 EA. FACE ALL SIDES TOP PLACED ALONG EDGES. SUCH BARS SHALL EXTEND AT LEAST 2'-0" BEYOND THE CORNERS OF THE OPENINGS.

CONCRETE WALL SQUARE PENETRATION
1 TYPICAL DETAIL
SCALE: NO SCALE



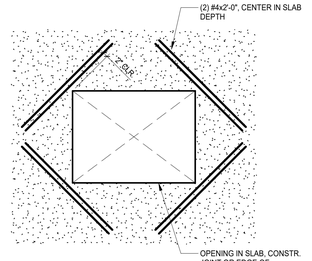
NOTE:
1. MINIMUM DISTANCE BETWEEN OPENINGS IN WALL TO BE 2'-0".
2. CLASS B SPLICE OR 2'-6" MIN. WHICHEVER IS GREATER.
3. AT OPENINGS 18" OR LESS IN DIAMETER OR ON ANY SIDE, NO EXTRA REBAR ARE REQUIRED UNLESS SHOWN OTHERWISE. SCHEDULED REBAR SHALL BE SPREAD (NOT CUT) TO ALLOW OPENING TO BE MADE. SUCH OPENINGS SHALL BE PROVIDED WITH A MIN. OF (1) #5 EA. FACE ALL SIDES TOP PLACED ALONG EDGES. SUCH BARS SHALL EXTEND AT LEAST 2'-0" BEYOND THE CORNERS OF THE OPENINGS.

CONCRETE WALL ROUND PENETRATION
2 TYPICAL DETAIL
SCALE: NO SCALE

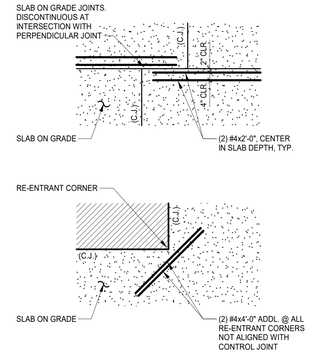


NOTE:
1. FOR SLEEVES > 10" REFERENCE TYPICAL WALL PENETRATION DETAIL.

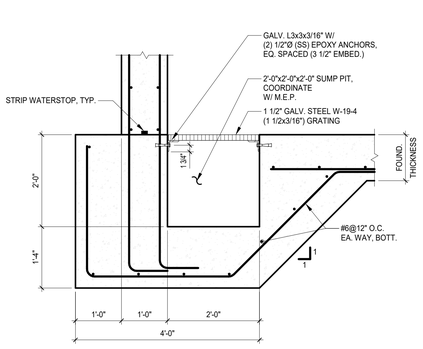
CONCRETE WALL PIPE PENETRATION
3 TYPICAL DETAIL
SCALE: NO SCALE



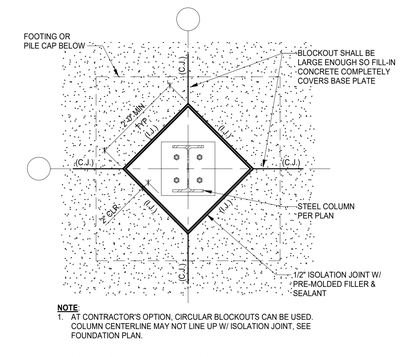
SLAB CORNER BAR
4 TYPICAL DETAIL
SCALE: NO SCALE



SLAB RE-ENTRANT BAR
5 TYPICAL DETAIL
SCALE: NO SCALE

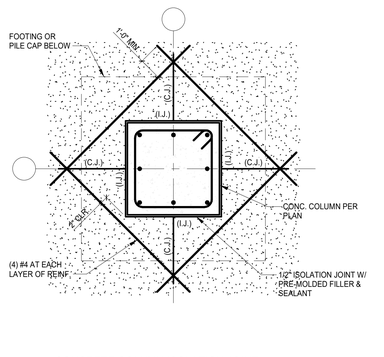


6 ELEVATOR SUMP PIT
SCALE: 3/4" = 1'-0"

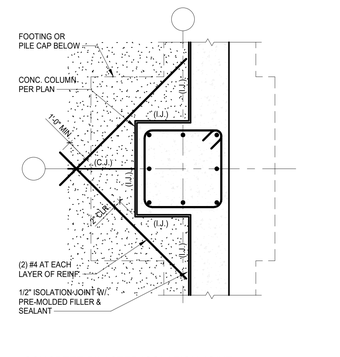


NOTE:
1. AT CONTRACTOR'S OPTION, CIRCULAR BLOCKOUTS CAN BE USED. COLUMN CENTERLINE MAY NOT LINE UP W/ ISOLATION JOINT, SEE FOUNDATION PLAN.

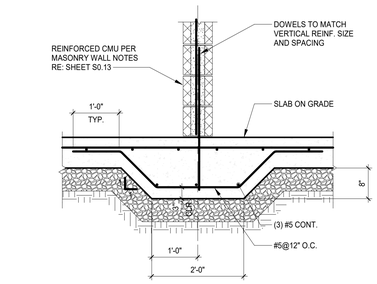
STEEL COLUMN ISOLATION JOINT
7 TYPICAL DETAIL
SCALE: NO SCALE



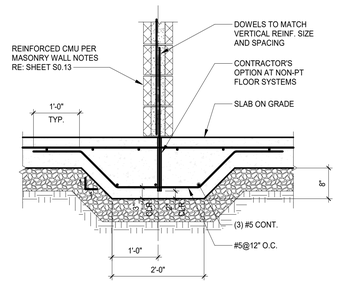
CONCRETE COLUMN ISOLATION JOINT
8 TYPICAL DETAIL
SCALE: NO SCALE



CONCRETE PILASTER ISOLATION JOINT
9 TYPICAL DETAIL
SCALE: NO SCALE

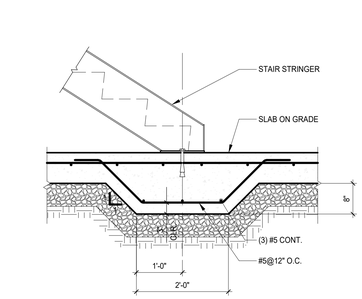


10 THICKENED SLAB AT MAS. WALL-CAST IN PLACE
SCALE: NO SCALE

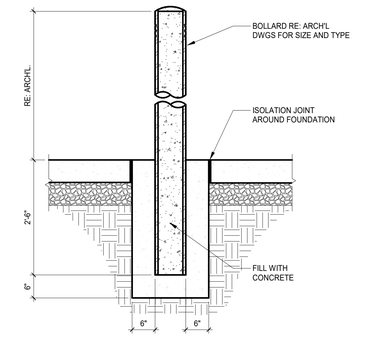


NOTES:
1. DRILL AND EPOXY DOWEL OPTION SHALL NOT BE USED AT PT SLABS OR BEAMS.
2. DRILL AND EPOXY INTO SLAB W/ HY 200 SD ADHESIVE OR APPROVED EQUAL.

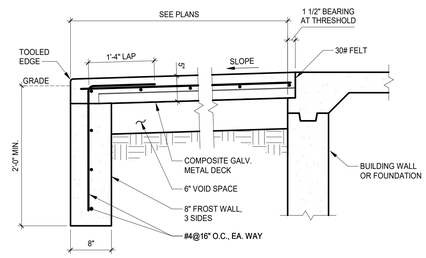
THICKENED SLAB AT MAS. WALL-DRILL AND EPOXY
11 TYPICAL DETAIL
SCALE: NO SCALE



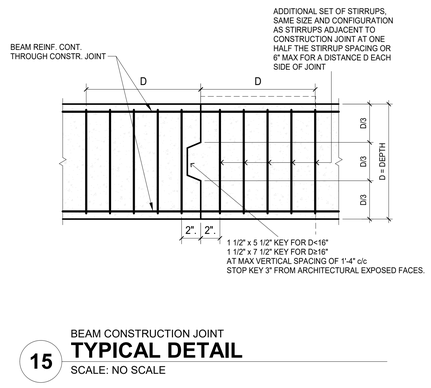
THICKENED SLAB AT STAIR STRINGER
12 TYPICAL DETAIL
SCALE: NO SCALE



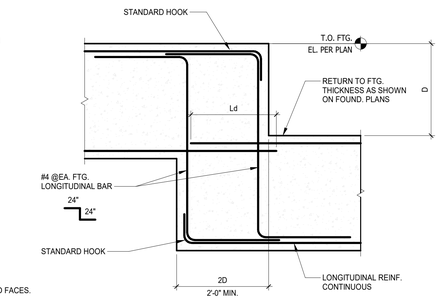
STEEL BOLLARD
13 TYPICAL DETAIL
SCALE: NO SCALE



STOOP DETAIL
14 TYPICAL DETAIL
SCALE: NO SCALE

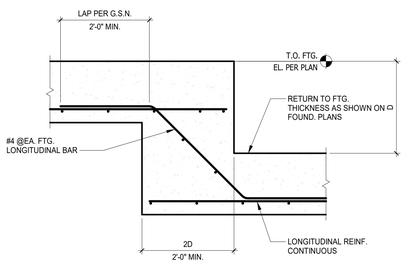


BEAM CONSTRUCTION JOINT
15 TYPICAL DETAIL
SCALE: NO SCALE

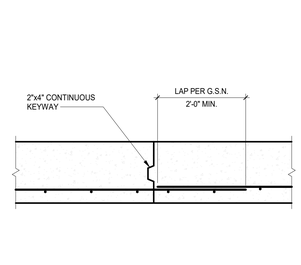


NOTE:
1. TRANSVERSE REINF. NOT SHOWN FOR CLARITY.

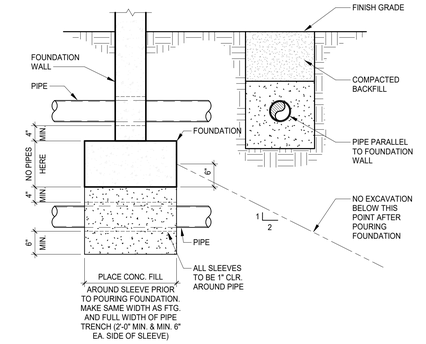
GRADE BEAM STEP
16 TYPICAL DETAIL
SCALE: NO SCALE



SPREAD FOOTING STEP
17 TYPICAL DETAIL
SCALE: NO SCALE



CONT. SPREAD FOOTING CONSTRUCTION JOINT
18 TYPICAL DETAIL
SCALE: NO SCALE



PIPE TRENCHING AND SLEEVES AT FOUNDATIONS
19 TYPICAL DETAIL
SCALE: NO SCALE

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SEPT. 6, 1839

CHEROKEE NATION
REPLACEMENT HOSPITAL
TAHLEQUAH, OKLAHOMA

KEY PLAN:

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

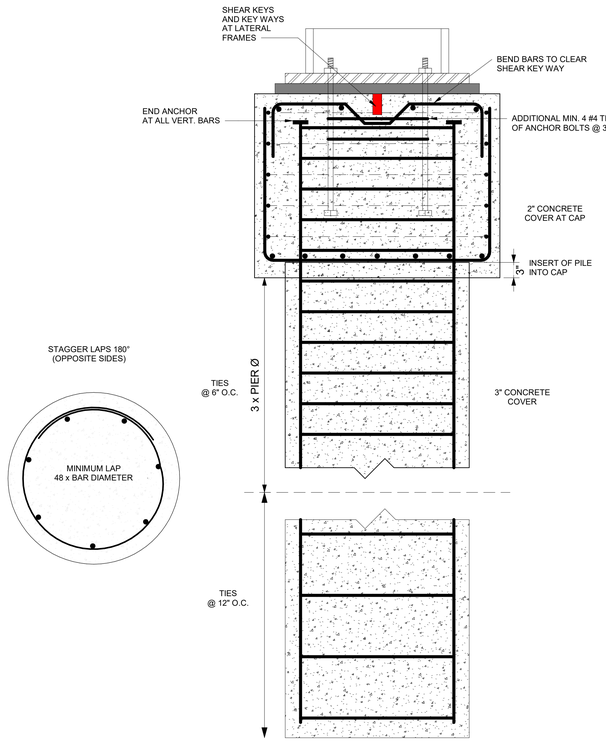
#	DATE	DESCRIPTION

JOB NUMBER: 21-335-1

DATE: 12-09-2022

SHEET NUMBER: **S4.02**

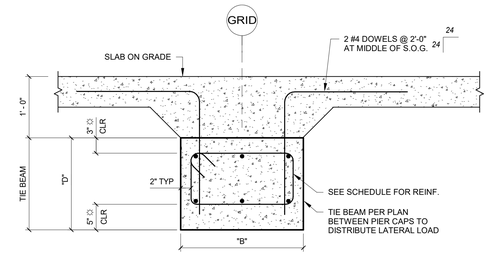
SHEET TITLE: CONCRETE TYPICAL DETAILS



1 **PILE REINFORCEMENT**

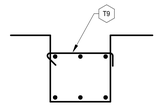
DRILLED PIER & CAP										
01 . PILES MARKED WITH AN ASTRIX (*) HAVE CAPS DEFINED ON PLAN OR SECTIONS, NOT DEFINED IN SCHEDULE.										
ID	FRILLED SHAFT			PIER CAP						Type Comments
	DIA	REINFORCEMENT		DIMENSIONS			REINFORCEMENT			
		VERTS	TIES	W	L	D	TIES SPA @ 6"	BOT (E.W.)	TOP (E.W.)	
P24	P24	7 #6	#4	36"	36"	36"	#4		-	
P30	P30	8 #7	#4	42"	42"	36"	#4	5 #5	-	
P36	P36	8 #8	#4	48"	48"	36"	#4	6 #5	-	
P42	P42	10 #8	#4	54"	54"	36"	#5	7 #6	7 #5	
P48	P48	12 #8	#5	60"	60"	36"	#5	8 #6	8 #5	
P54	P54	10 #10	#5	66"	66"	36"	#5	9 #6	9 #5	
P60	P60	12 # 10	#5	72"	72"	36"	#5	10 #6	10 #6	
P66	P66			78"	78"	36"				
P72	P72	17 # 10	#5	84"	84"	36"	#5	12 #6	12 #6	

2 **PILE REINFORCEMENT SCHEDULE**



1. ☼ ALTERNATE TOP & BOTTOM CLEAR COVERS AS REQUIRED TO ACCOMMODATE REBAR PLACEMENT IF REQUIRED AT PIER CAPS OR SPLICES.
2. LONGITUDINAL T&B BARS SHALL BE CONTINUOUS THROUGH ALL PIER CAPS
3. LONGITUDINAL BARS SPLICED WITH CLASS-B TENSION LAP SPLICES OR MECHANICAL
   A. DO NOT SPLICE AT RO THROUGHT PIER CAP

7 **HOSPITAL - TYPICAL TIE BEAM**



ALTERNATE BAR PATTERN AT CONTRACTORS OPTION

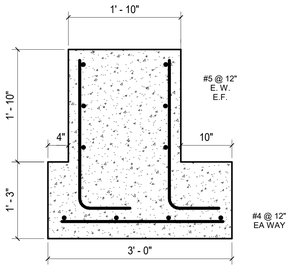
TIE BEAM REINFORCING (B x D)			
BxD	T&B BARS	TIES	COMMENTS
18x18	3 #7	#4 @ 12"	
24x18	3 #7	#4 @ 12"	
30x20	4 #7	#4 @ 12"	
42x30	6 #8	#4 @ 12"	

GRADE BEAM REINFORCING (B x D)			
B x D	T&B BARS	TIES	COMMENTS
14x24	2 #8	#4 @ 12"	
16x24	3 #7	#4 @ 12"	
20x30	4 #7	#4 @ 12"	
21x15	4 #7	#4 @ 12"	
24x18	4 #7	#4 @ 12"	
24x36	3 #9	#4 @ 12"	
30x30	5 #7	#4 @ 12"	
30x36	4 #9	#4 @ 12"	
42x36	6 #8	#4 @ 12"	
44x18	7 #7	#4 @ 12"	
48x36	6 #9	#4 @ 12"	

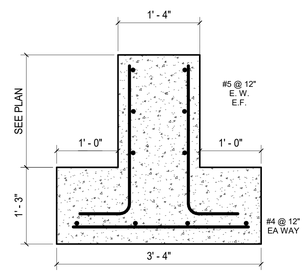
1. LONGITUDINAL T&B BARS SHALL BE CONTINUOUS THROUGH ALL PIER CAPS
2. DO NOT SPLICE AT PIER CAP
   A. CLASS-B TENSION LAP SPLICES ONLY

HOSPITAL SPREAD FOOTING								
TYPE MARK	GEOMETRY			BOTTOM REINF.		TOP REINF.		COMMENTS
	T	W	L	SHORT	LONG	SHORT	LONG	
FH1	1' - 6"	6' - 0"	6' - 0"	8#6	E.W.	-	-	
FH2	1' - 6"	7' - 0"	7' - 0"	6#6	E.W.	-	-	
FH3	1' - 6"	8' - 0"	8' - 0"	8#6	E.W.	-	-	
FH4	1' - 9"	9' - 0"	9' - 0"	12#6	E.W.	-		
FH5	1' - 9"	9' - 0"	9' - 0"	12#6	E.W.	12#6	E.W.	LATERAL
FH7	1' - 3"	5' - 0"	5' - 0"	7#5	E.W.	-	-	

5 **FOOTING SCHEDULE HOSPITAL**

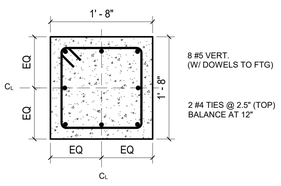


3 **TYPICAL STEM WALL - PRECAST - HOSPITAL**



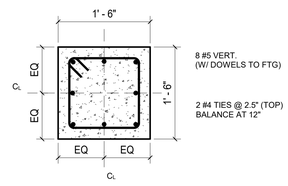
10 **TYPICAL STEM WALL - CMU - HOSPITAL**

*PENDING TO ASSIGN TYPE MARK
PIER 20 x 20

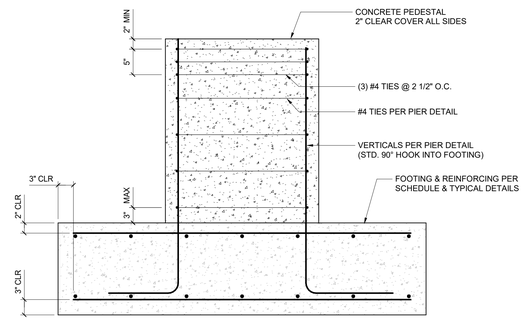


4 **PEDESTAL HOSPITAL 1**

*PENDING TO ASSIGN TYPE MARK
PIER 18 x 18



6 **PEDESTAL HOSPITAL 2**



9 **TYPICAL PEDESTAL**

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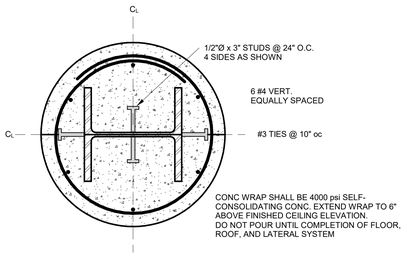
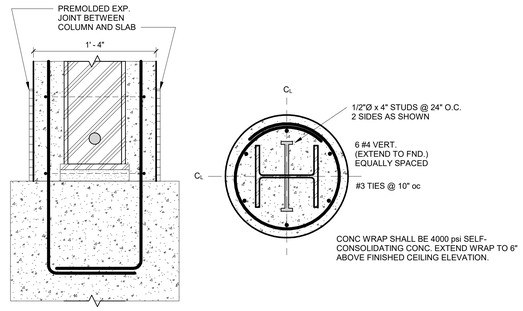
PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS		
#	DATE	DESCRIPTION

JOB NUMBER: 21-335-1
DATE: 12-09-2022
SHEET NUMBER: **S4.03**
SHEET TITLE: FOUNDATION DETAILS

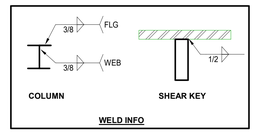


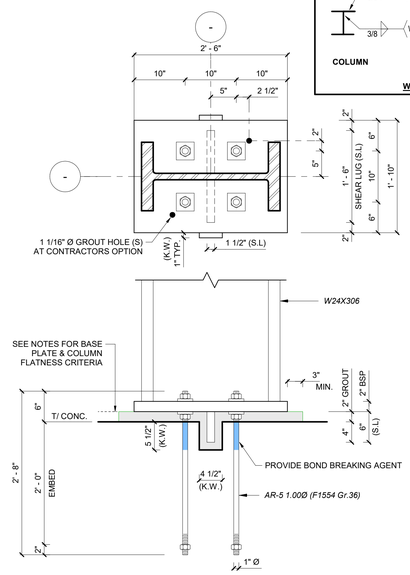
1 24"Ø CONCRETE ENCASED STEEL COLUMN
1 1/2" = 1'-0"



3 16"Ø CONCRETE ENCASED STEEL COLUMN
1 1/2" = 1'-0"

ANCHOR RODS							
Type Mark	Type	DIA	E	P	S	L	Grout
AR-1	AR-1 0.750Ø (F1554 Gr.36)	0.750"	14.000"	6.000"	2.000"	1' - 10"	1.50"
AR-2	AR-2 1.00Ø (F1554 Gr.36)	1.000"	18.000"	6.000"	2.000"	2' - 2"	2.00"
AR-3	AR-3 1.125Ø (F1554 Gr.55-S1)	1.125"	18.000"	7.000"	2.000"	2' - 3"	1.50"
AR-4	AR-4 1.25Ø (F1554 Gr.36)	1.250"	24.000"	8.000"	2.000"	2' - 10"	2.00"
AR-5	AR-5 1.00Ø (F1554 Gr.36)	1.000"	24.000"	6.000"	2.000"	2' - 8"	2.00"
AR-6	AR-6 1.250Ø (F1554 Gr.55-S1)	1.250"	22.000"	6.000"	2.000"	2' - 6"	2.00"
AR-8	AR-8 0.750Ø (F1554 Gr.36)	0.750"	9.000"	6.000"	2.000"	1' - 5"	2.00"
AR-9	AR-9 1.00Ø (F1554 Gr.36) E-15"	1.000"	15.000"	6.000"	2.000"	1' - 11"	1.50"
AR-HDG-1	HDG-AR-1 0.750Ø (F1554 Gr.36)	0.750"	12.000"	6.000"	2.000"	1' - 8"	1.50"
AR-HDG-2	HDG-AR-2 0.750Ø (F1554 Gr.36)	0.750"	18.000"	6.000"	2.000"	2' - 2"	1.50"
AR-HDG-3	HDG-AR-3 1.1250Ø (F1554 Gr.55-S1)	1.125"	30.000"	7.000"	2.000"	3' - 3"	1.50"



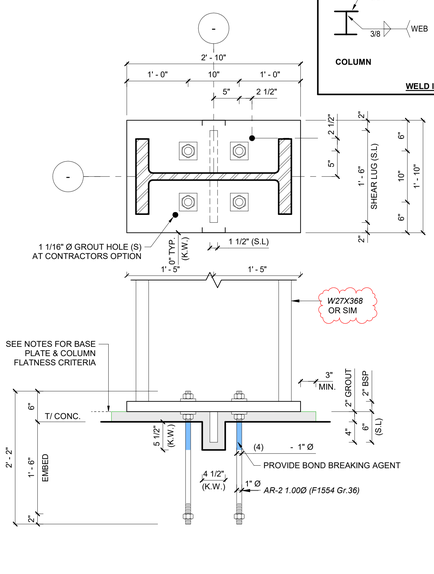


DETAILING REFERENCE: #12

HOSPITAL BASE PLATE NOTES:

1. KEYWAY (K.W.) SHALL HAVE MIN. 1 1/2" GROUT AROUND SHEAR KEY
   A. EXTEND AT LEAST 1" BEYOND EDGE OF BASE PLATE
   B. SEE TYPICAL KEYWAY (K.W.) DETAIL
   C. INDEPENDENT INSPECTION REQUIRED TO VERIFY SHEAR KEY WAY IS CLEAN OF DEBRIS PRIOR TO PLACING FLOAWABLE NON-SHRINK GROUT
2. TACK WELD OR DAMAGE ANCHOR ROD'S EMBEDED NUT PRIOR TO SHIPPING
3. BASE PLATES ARE 50 KSI, U.N.O.
4. ANCHOR ROD HOLES AND WASHERS ARE PER AISC 360 (U.N.O.):
   T14-2 RECOMMENDED MAXIMUM SIZES FOR ANCHOR-ROD HOLES IN BASE PLATES.
5. NON-SHRINK FLOWABLE GROUT (MIN. 8,000 PSI) SHALL EXTEND AT LEAST 2" BEYOND EDGE OF BASE PLATE.
6. A.R. BOND BREAKER SHALL EXTEND TO BOTTOM OF KEYWAY (K.W.)
7. SHIMS OR HALF NUTS ALLOWED BENEATH BASE PLATES
8. FLATNESS CRITERIA AT COLUMN / BASE PLATE INTERFACE:
   A. SQUARE CUT COLUMN:
      a. 85% OF COLUMN SHALL COMPLETLY BEAR ON BASE PLATE.
      b. INSPECTION PRIOR TO WELDING IS REQUIRED
   B. MILL BASE PLATE ONLY IF FLUSH BEARING CONDITION CAN NOT BE MET OR OBTAINED
   C. ALTERNATE OPTION:
      a. PJP WELD THE COLUMN FLANGES
         - PREPARE FLAN W/ GE SINGLE BEVEL GROOVE ON OUTSIDE FACE
         - EFFECTIVE WIDTH 5/16" LESS THAN THE COLUMN FLANGE WIDTH
         - ADD 5/16" REINFORCING FILLET TO INSIDE FACE
      b. ADD FILLET WELDS NS/FS OF COLUMN WEB
         - TOTAL WELD SHALL BE 75% OF THE WEB THICKNESS (t_w), U.N.O.
         - MINIMUM FILLET SIZE SHALL BE 37.5% OF (t_w).

4 BASEPLATE #12
1" = 1'-0"

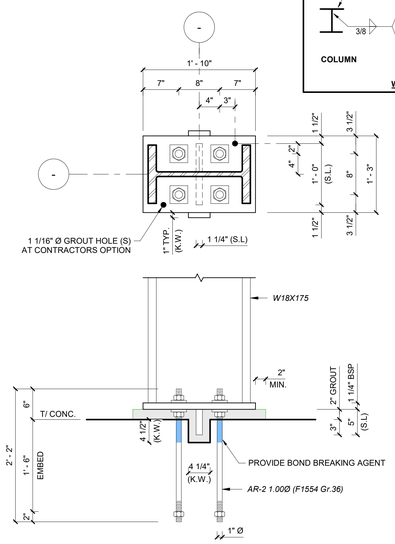


DETAILING REFERENCE: #13

HOSPITAL BASE PLATE NOTES:

1. KEYWAY (K.W.) SHALL HAVE MIN. 1 1/2" GROUT AROUND SHEAR KEY
   A. EXTEND AT LEAST 1" BEYOND EDGE OF BASE PLATE
   B. SEE TYPICAL KEYWAY (K.W.) DETAIL
   C. INDEPENDENT INSPECTION REQUIRED TO VERIFY SHEAR KEY WAY IS CLEAN OF DEBRIS PRIOR TO PLACING FLOAWABLE NON-SHRINK GROUT
2. TACK WELD OR DAMAGE ANCHOR ROD'S EMBEDED NUT PRIOR TO SHIPPING
3. BASE PLATES ARE 50 KSI, U.N.O.
4. ANCHOR ROD HOLES AND WASHERS ARE PER AISC 360 (U.N.O.):
   T14-2 RECOMMENDED MAXIMUM SIZES FOR ANCHOR-ROD HOLES IN BASE PLATES.
5. NON-SHRINK FLOWABLE GROUT (MIN. 8,000 PSI) SHALL EXTEND AT LEAST 2" BEYOND EDGE OF BASE PLATE.
6. A.R. BOND BREAKER SHALL EXTEND TO BOTTOM OF KEYWAY (K.W.)
7. SHIMS OR HALF NUTS ALLOWED BENEATH BASE PLATES
8. FLATNESS CRITERIA AT COLUMN / BASE PLATE INTERFACE:
   A. SQUARE CUT COLUMN:
      a. 85% OF COLUMN SHALL COMPLETLY BEAR ON BASE PLATE.
      b. INSPECTION PRIOR TO WELDING IS REQUIRED
   B. MILL BASE PLATE ONLY IF FLUSH BEARING CONDITION CAN NOT BE MET OR OBTAINED
   C. ALTERNATE OPTION:
      a. PJP WELD THE COLUMN FLANGES
         - PREPARE FLAN W/ GE SINGLE BEVEL GROOVE ON OUTSIDE FACE
         - EFFECTIVE WIDTH 5/16" LESS THAN THE COLUMN FLANGE WIDTH
         - ADD 5/16" REINFORCING FILLET TO INSIDE FACE
      b. ADD FILLET WELDS NS/FS OF COLUMN WEB
         - TOTAL WELD SHALL BE 75% OF THE WEB THICKNESS (t_w), U.N.O.
         - MINIMUM FILLET SIZE SHALL BE 37.5% OF (t_w).

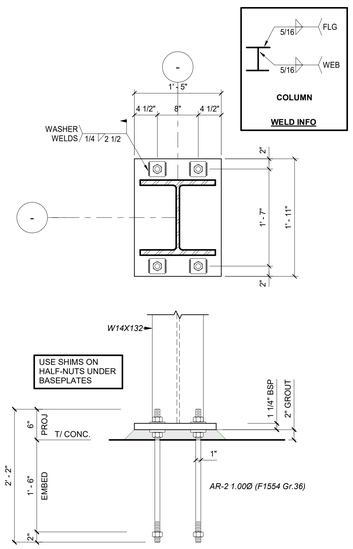
5 BASEPLATE #13
1" = 1'-0"



DETAILING REFERENCE: #14

NOTES:
1. KEYWAY (K.W.) SHALL HAVE MIN. 1 1/2" GROUT AROUND SHEAR KEY
   A. EXTEND AT LEAST 1" BEYOND EDGE OF BASE PLATE
   B. SEE TYPICAL KEYWAY (K.W.) DETAIL
   C. INDEPENDENT INSPECTION REQUIRED TO VERIFY SHEAR KEY WAY IS CLEAN OF DEBRIS PRIOR TO PLACING FLOAWABLE NON-SHRINK GROUT
2. TACK WELD OR DAMAGE ANCHOR ROD'S EMBEDED NUT PRIOR TO SHIPPING
3. BASE PLATES ARE 50 KSI, U.N.O.
4. ANCHOR ROD HOLES AND WASHERS ARE PER AISC 360 (U.N.O.):
   T14-2 RECOMMENDED MAXIMUM SIZES FOR ANCHOR-ROD HOLES IN BASE PLATES.
5. NON-SHRINK FLOWABLE GROUT (MIN. 8,000 PSI) SHALL EXTEND AT LEAST 2" BEYOND EDGE OF BASE PLATE.
6. A.R. BOND BREAKER SHALL EXTEND TO BOTTOM OF KEYWAY (K.W.)
7. SHIMS OR HALF NUTS ALLOWED BENEATH BASE PLATES

6 BASEPLATE #14
1" = 1'-0"



2 BASEPLATE #11
1" = 1'-0"

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TAHLEQUAH, OKLAHOMA

KEY PLAN:

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

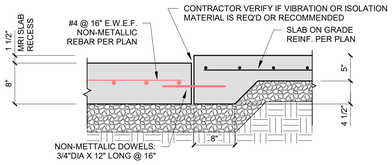
REVISIONS		
#	DATE	DESCRIPTION
1	Date 1	Revision 1

JOB NUMBER: 21-335-1

DATE: 12-09-2022

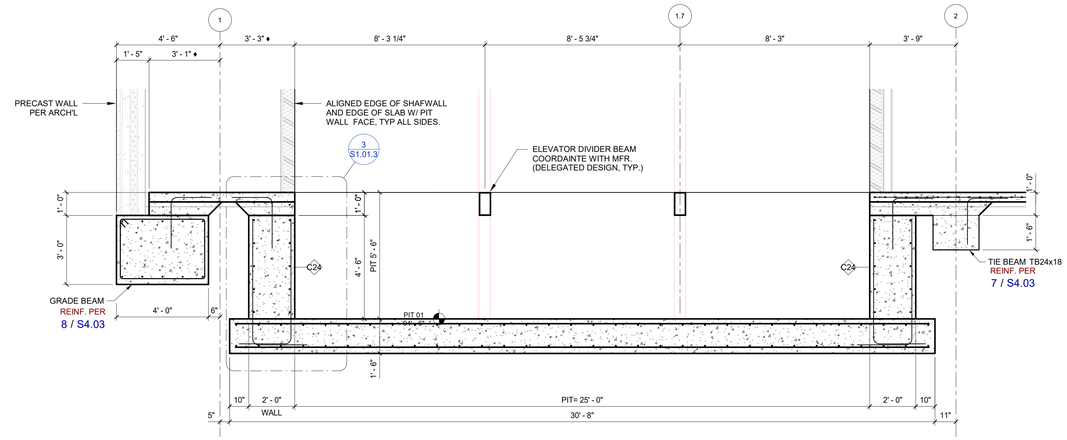
SHEET NUMBER:
S4.05

SHEET TITLE:
BASE PLATE DETAILS

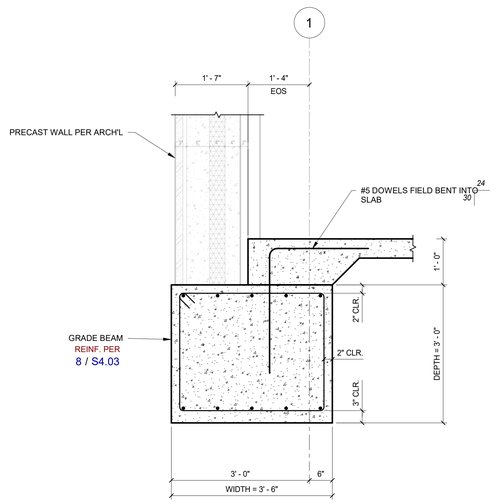


NOTES:
1. OWENS CORNING FIBERGLAS REBAR AND DOWELS OR EQUIVELANT.
2. DOWEL MAY BE CAST-IN-PLACE OR POST-INSTALL W/ HILTI HIT-HY200 EPOXY
3. FLOOR LEVELNESS MUST BE 3 mm BETWEEN HIGH AND LOW SPOTS (SEE MAGNET ROOM FLOOR SPECIFICATIONS PROVIDED BY MRI VENDOR).

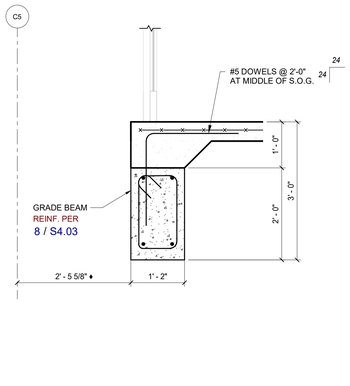
1 FDN - MRI RECESSED SLAB
1" = 1'-0"



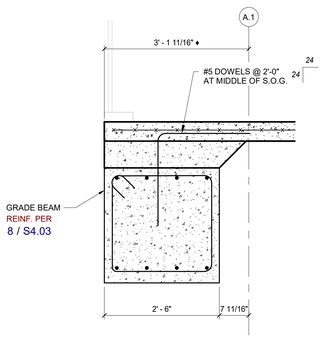
2 FDN - HOPSPITAL - ELEVATOR PIT (PUBLIC)
3/8" = 1'-0"



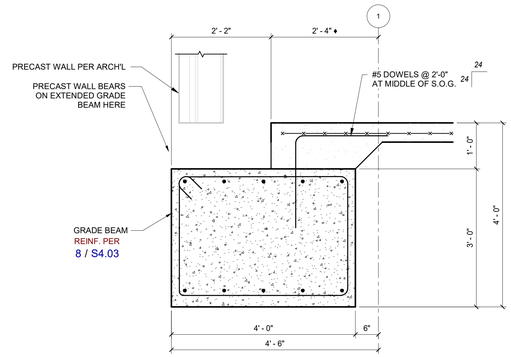
3 FDN - HOPSPITAL - PERIMETER LINE 01
3/4" = 1'-0"



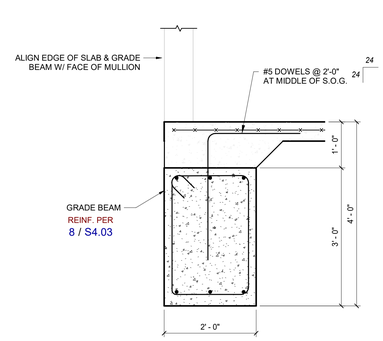
4 FDN - HOPSPITAL - PERIMETER LINE C5
3/4" = 1'-0"



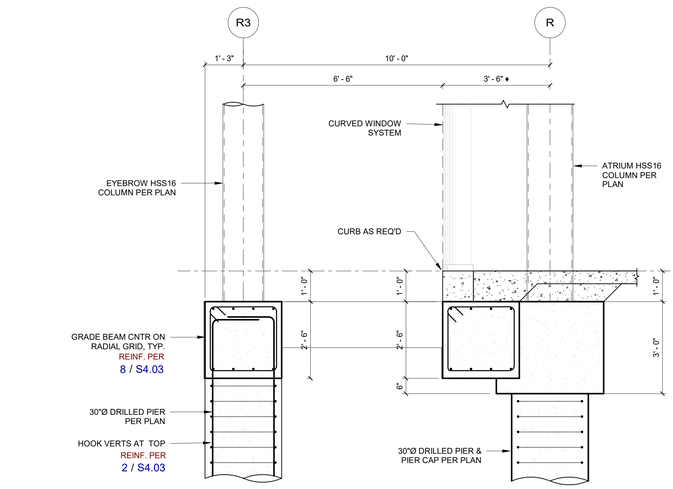
5 FDN - HOPSPITAL - PERIMETER LINE A.1
3/4" = 1'-0"



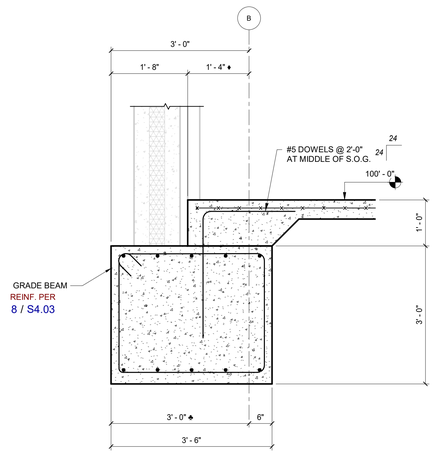
6 FDN - HOPSPITAL - PERIMETER LINE 01 AT BUMPOUT
3/4" = 1'-0"



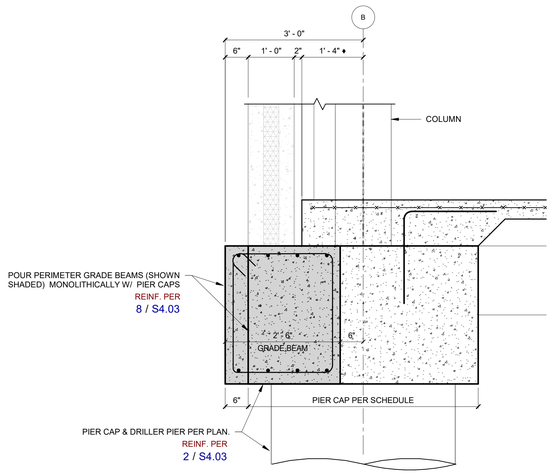
7 FDN - HOPSPITAL - PERIMETER LINE NEAR SKYBRIDGE
3/4" = 1'-0"



8 FDN - HOPSPITAL - PERIMETER RADIAL LINE R & R3
1/2" = 1'-0"



9 FDN - HOPSPITAL - PERIMETER LINE B
3/4" = 1'-0"



10 FDN - HOPSPITAL - PIER CAP LINE B
3/4" = 1'-0"

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CLIENT:

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CHEROKEE NATION
REPLACEMENT HOSPITAL
TAHLEQUAH, OKLAHOMA

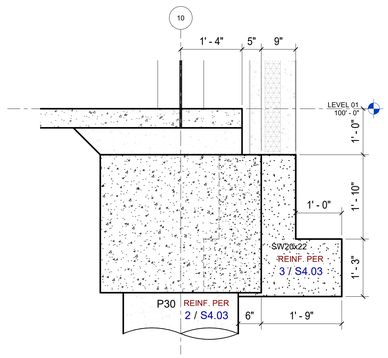
KEY PLAN:

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

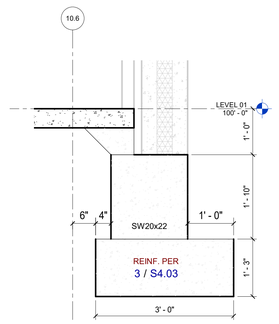
REVISIONS

#	DATE	DESCRIPTION

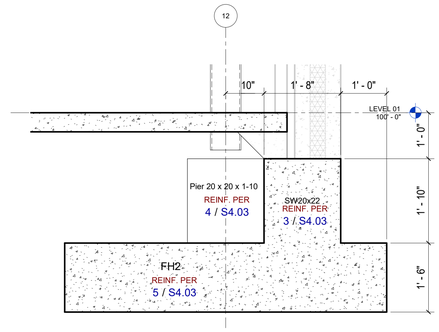
JOB NUMBER: 21-335-1
DATE: 12-09-2022
SHEET NUMBER: **S4.06**
SHEET TITLE: FOUNDATION SECTIONS



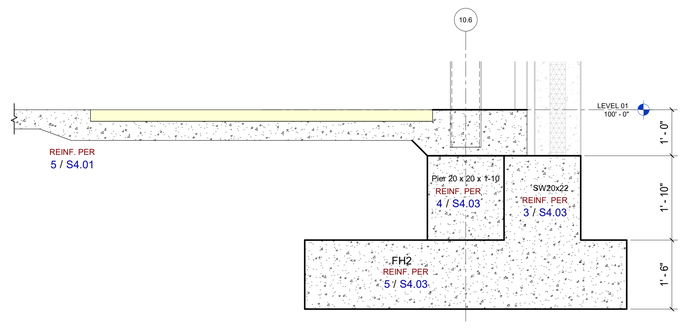
1 PIER CAP - STEM WALL INTERFACE GRID 10
3/4" = 1'-0"



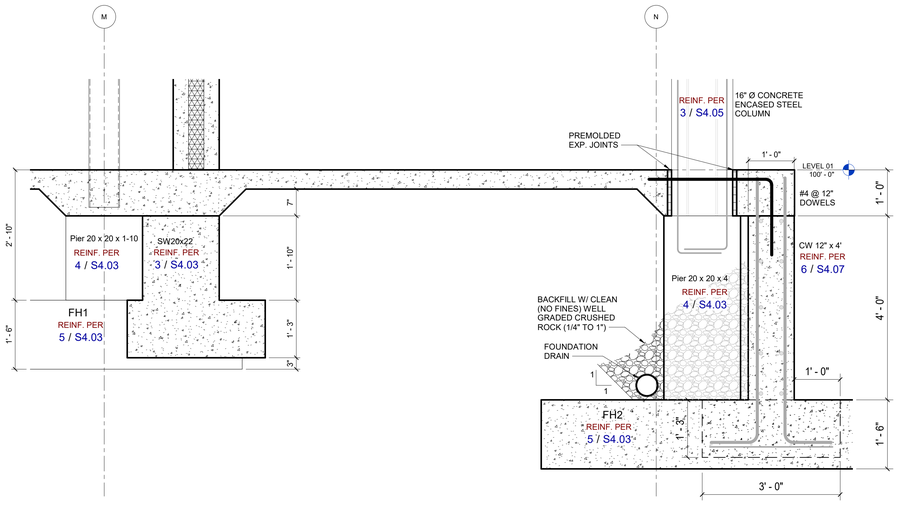
3 TYPICAL STEM WALL ON FOOTING
3/4" = 1'-0"



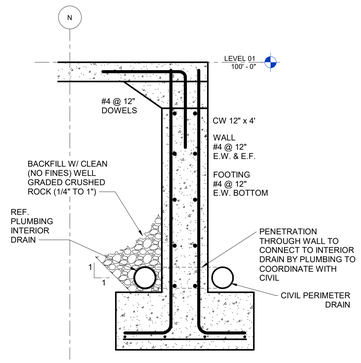
2 TYPICAL COLUMN ON PILASTER AND STEM WALL ON FOOTING
3/4" = 1'-0"



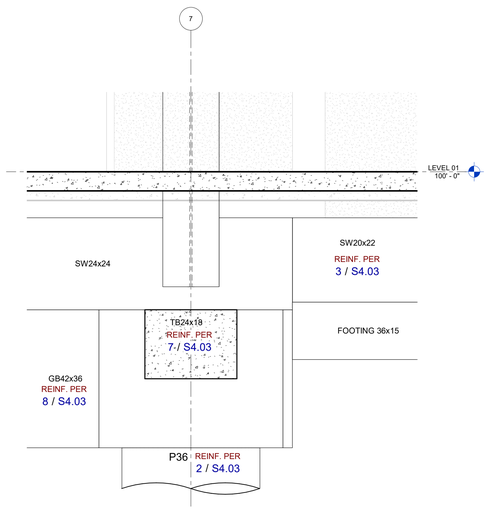
4 COLUMN ON PILASTER AND STEM WALL ON FOOTING - DEPRESSED SLAB
3/4" = 1'-0"



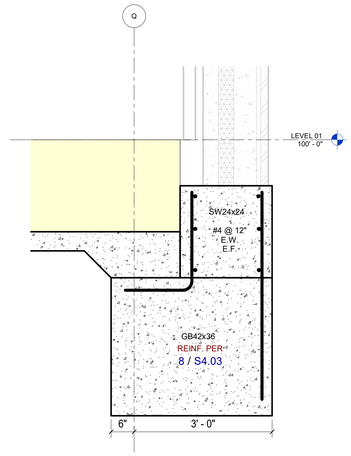
5 STEP AT DOCK
3/4" = 1'-0"



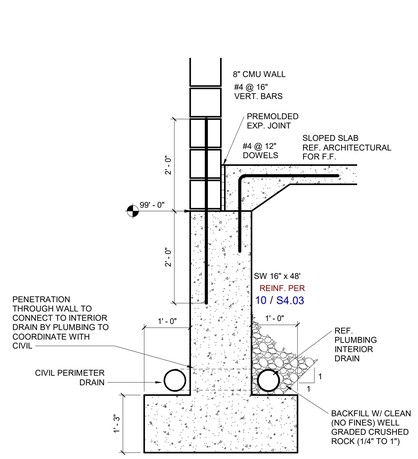
6 DOCK CONCRETE WALL INTERIOR / EXTERIOR DRAINAGE
3/4" = 1'-0"



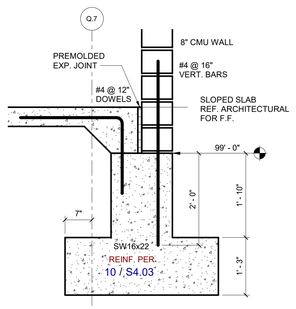
7 GRADE BEAM - STEM WALL TRANSITION GRID Q
3/4" = 1'-0"



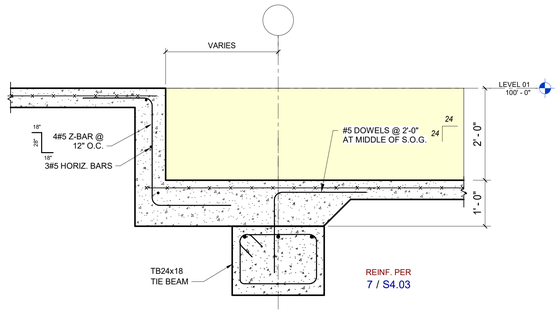
8 EXTERIOR GRADE BEAM UNDER 2' SLAB DEPRESSION
3/4" = 1'-0"



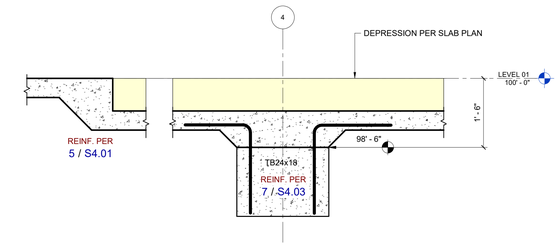
13 DOCK CONCRETE WALL UNDER 8" CMU WALL
3/4" = 1'-0"



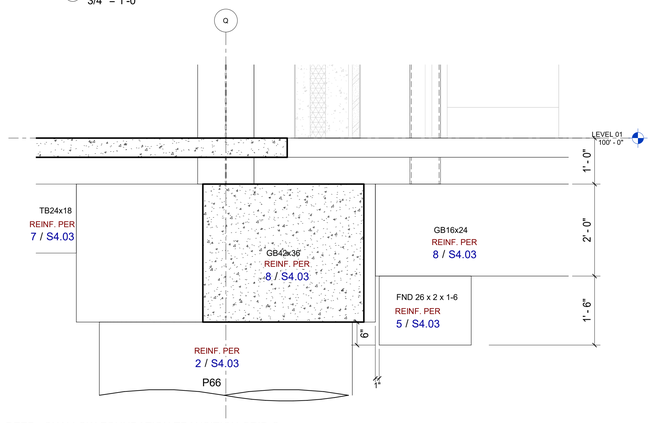
14 STEM WALL UNDER 8" CMU WALL
3/4" = 1'-0"



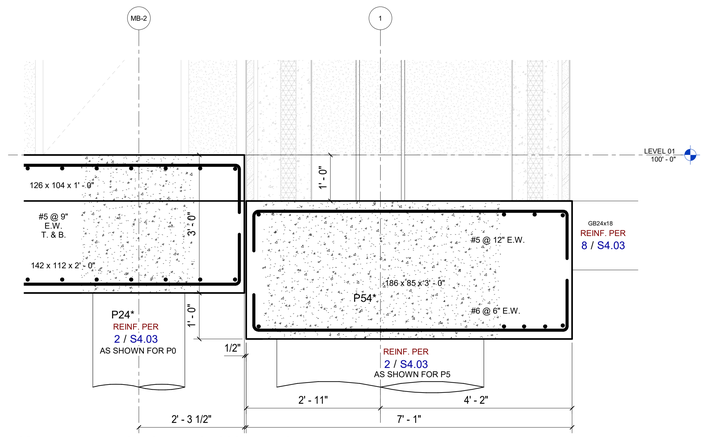
9 INTERIOR TIE BEAM UNDER 2' SLAB DEPRESSION
3/4" = 1'-0"



10 INTERIOR TIE BEAM UNDER 8 1/2" SLAB DEPRESSION
3/4" = 1'-0"



11 DEEP - SHALLOW FOUNDATION TRANSITION GRID Q
3/4" = 1'-0"



12 HOSPITAL - PIPE BRIDGE FOUNDATION COORDINATION
3/4" = 1'-0"

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PROJECT PHASE:
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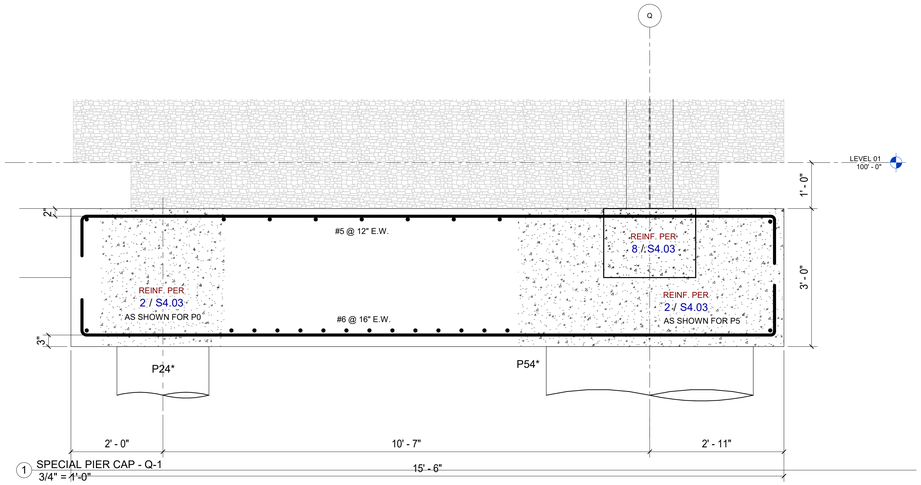
REVISIONS		
#	DATE	DESCRIPTION

JOB NUMBER: 21-335-1

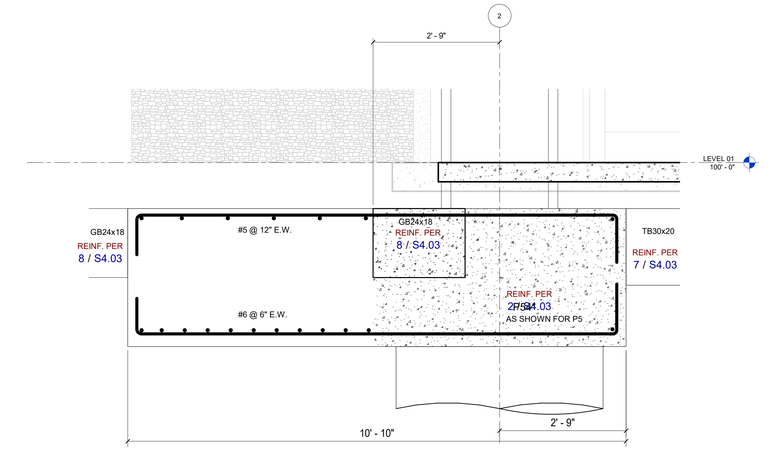
DATE: 12-09-2022

SHEET NUMBER: **S4.07**

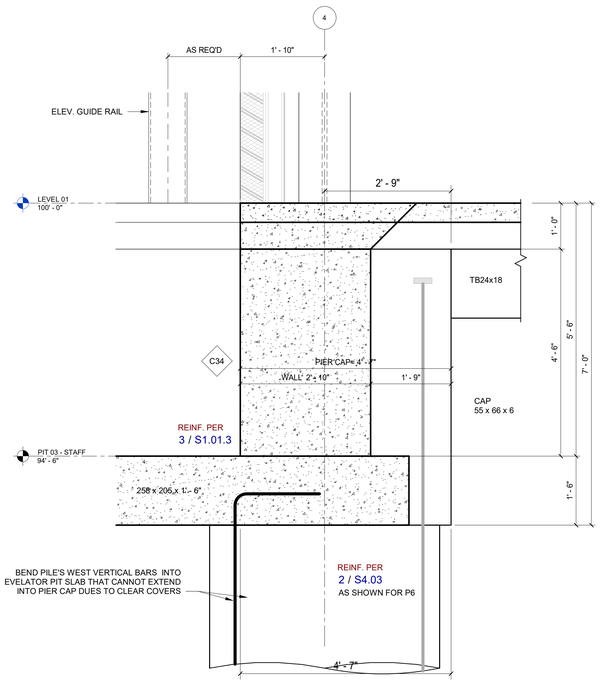
SHEET TITLE: FOUNDATION SECTIONS



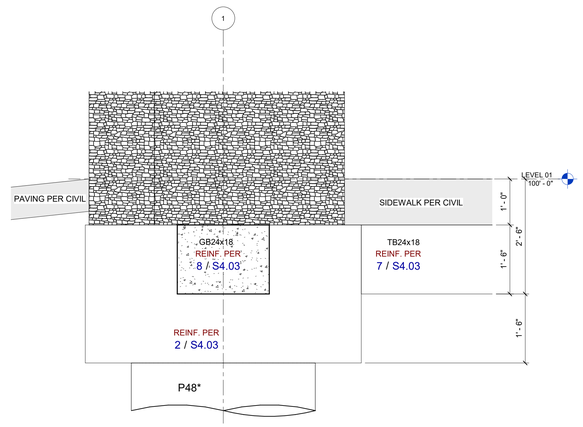
1 SPECIAL PIER CAP - Q-1
3/4" = 1'-0"



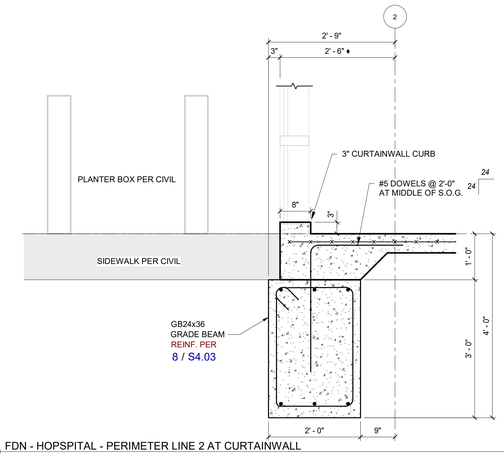
2 SPECIAL PIER CAP -Q-2
3/4" = 1'-0"



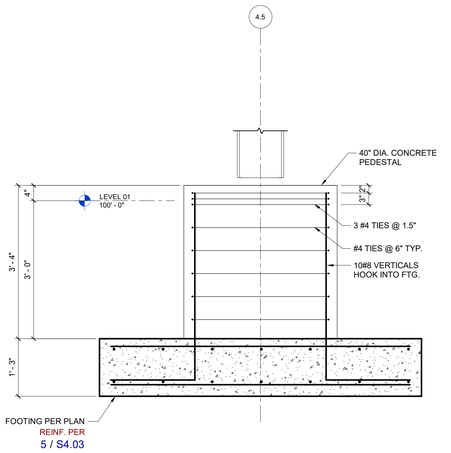
3 SPECIAL PIER CAP - (OR COLUMN - PIER BOND) - ELEVATOR PITS
3/4" = 1'-0"



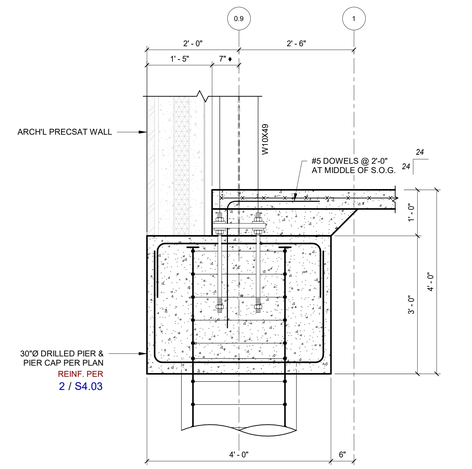
4 PIER CAP SUPPORTING PRECAST PANELS - GRID 1
3/4" = 1'-0"



5 FDN - HOPSPITAL - PERIMETER LINE 2 AT CURTAINWALL
3/4" = 1'-0"



6 FDN - NORTH CANOPY
3/4" = 1'-0"



7 FDN - HOPSPITAL - PERIMETER LINE 01 AT BUMPOUT NEAR ELEVATOR
3/4" = 1'-0"

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TAHLEQUAH, OKLAHOMA

KEY PLAN:

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BID PACKAGE 04
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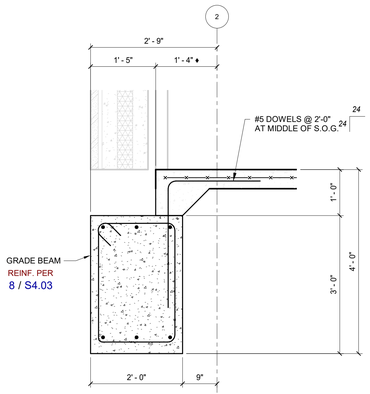
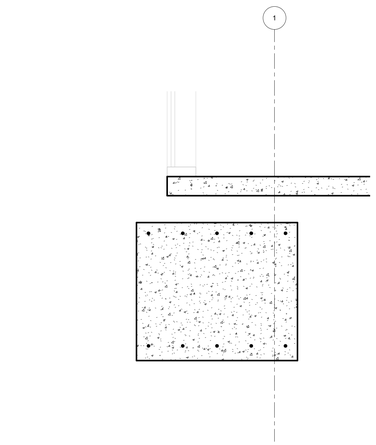
REVISIONS		
#	DATE	DESCRIPTION

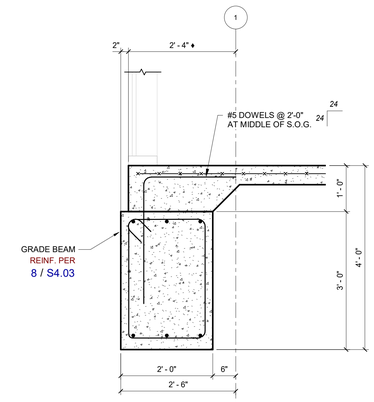
JOB NUMBER: 21-335-1
DATE: 12-09-2022
SHEET NUMBER: **S4.08**
SHEET TITLE: FOUNDATION SECTIONS



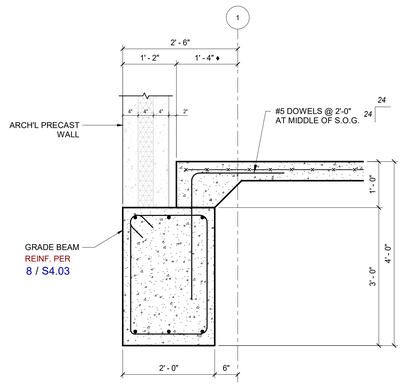
1 FDN - HOPSPITAL - PERIMETER LINE 2
3/4" = 1'-0"



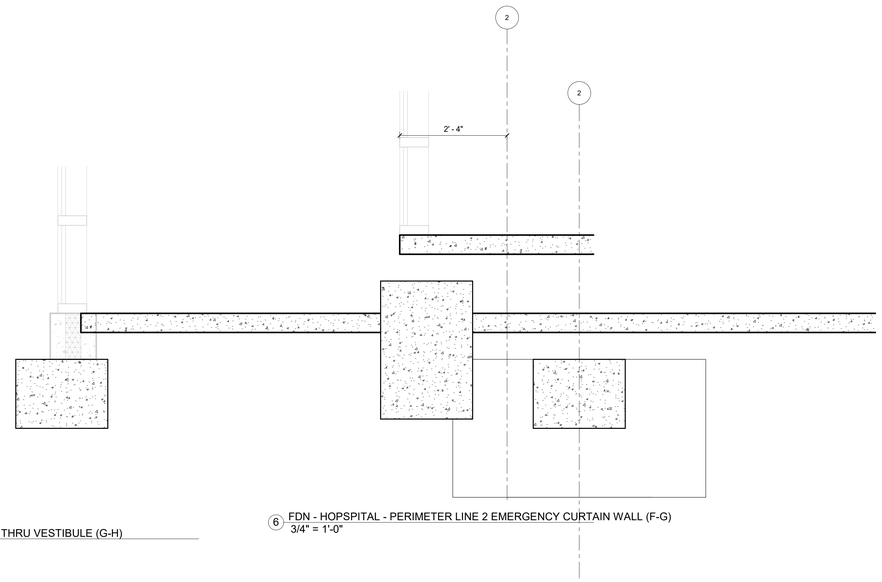
2 FDN - HOPSPITAL - PERIMETER LINE 1
3/4" = 1'-0"



3 FDN - HOPSPITAL - PERIMETER LINE 01 B
3/4" = 1'-0"



4 FDN - HOPSPITAL - PERIMETER LINE 01 C
3/4" = 1'-0"



5 EMERGENCY DRIVE THRU VESTIBULE (G-H)
3/4" = 1'-0"

6 FDN - HOPSPITAL - PERIMETER LINE 2 EMERGENCY CURTAIN WALL (F-G)
3/4" = 1'-0"

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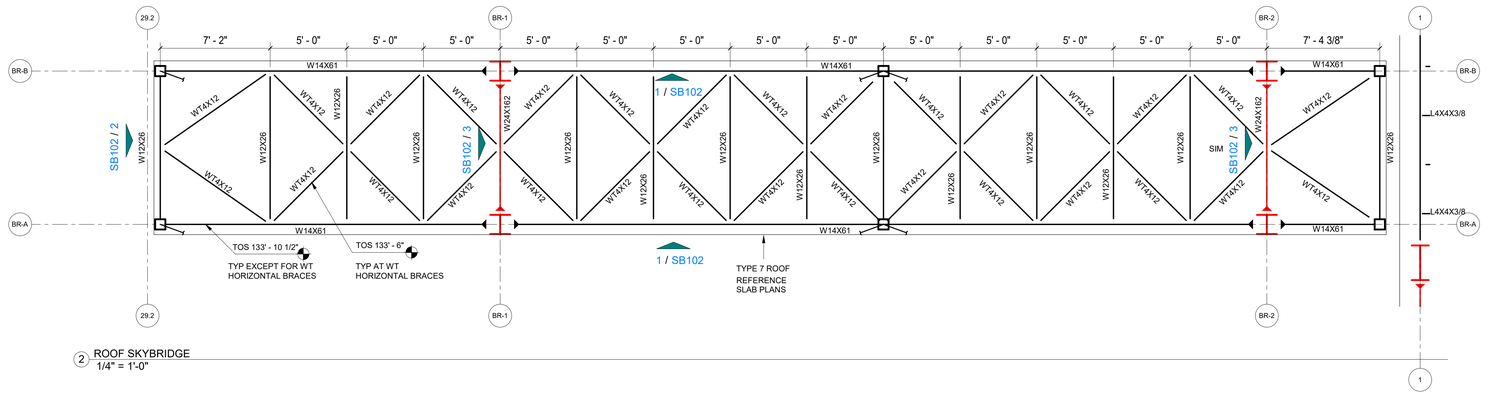
#	DATE	DESCRIPTION

JOB NUMBER: 21-335-1

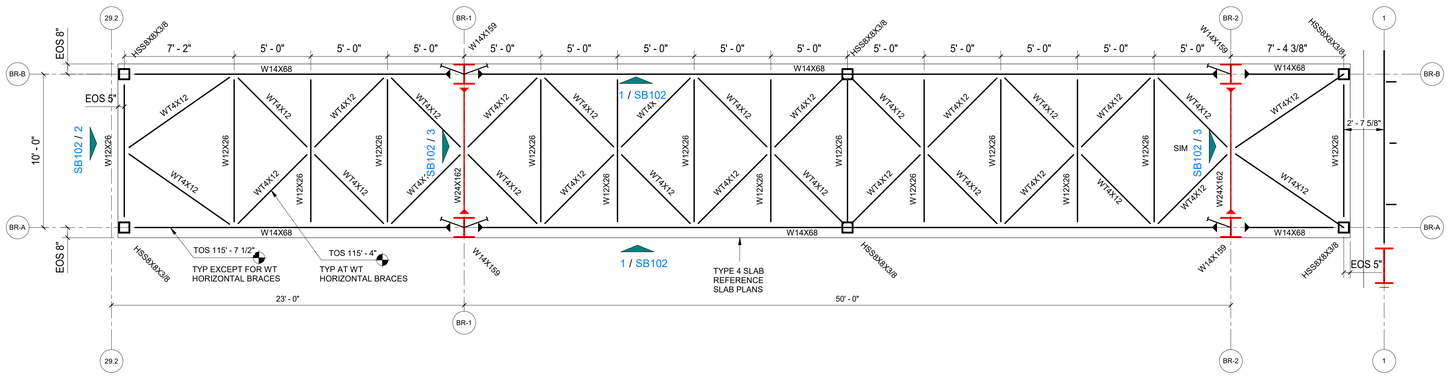
DATE: 12-09-2022

SHEET NUMBER:
S4.09

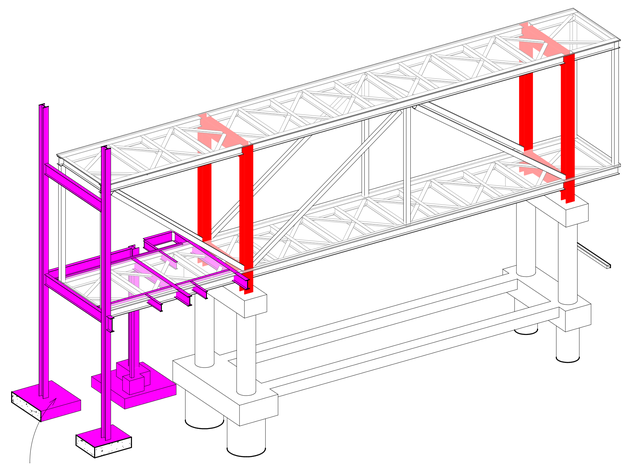
SHEET TITLE:
FOUNDATION SECTIONS



2 ROOF SKYBRIDGE
1/4" = 1'-0"

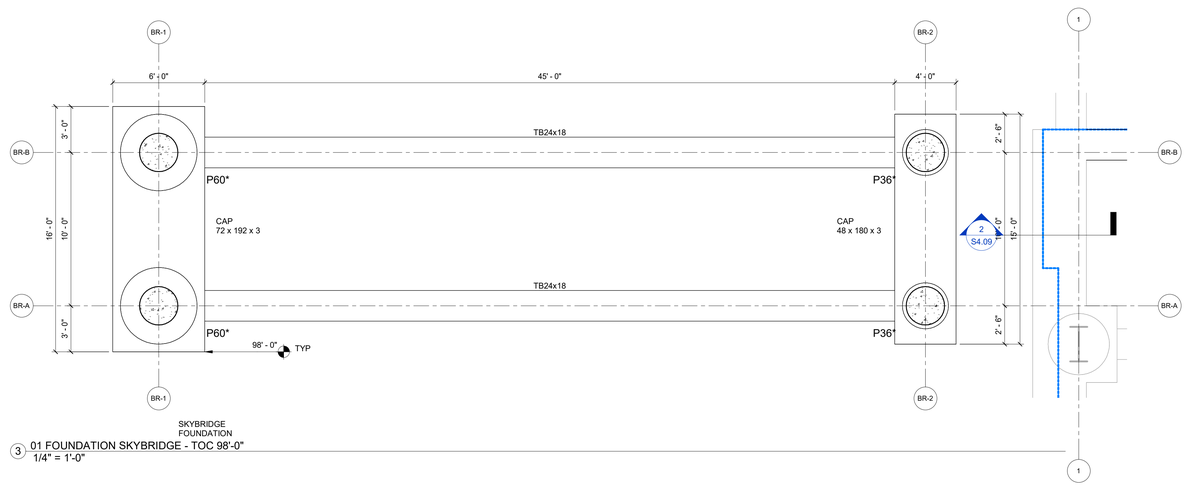


1 LEVEL 02 SKYBRIDGE
1/4" = 1'-0"



EXISTING

4 3D- SKYBRIDGE (CNOHC + WWH)



3 01 FOUNDATION SKYBRIDGE - TOC 98'-0"
1/4" = 1'-0"

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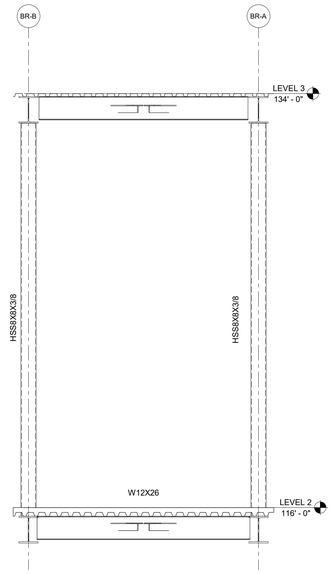
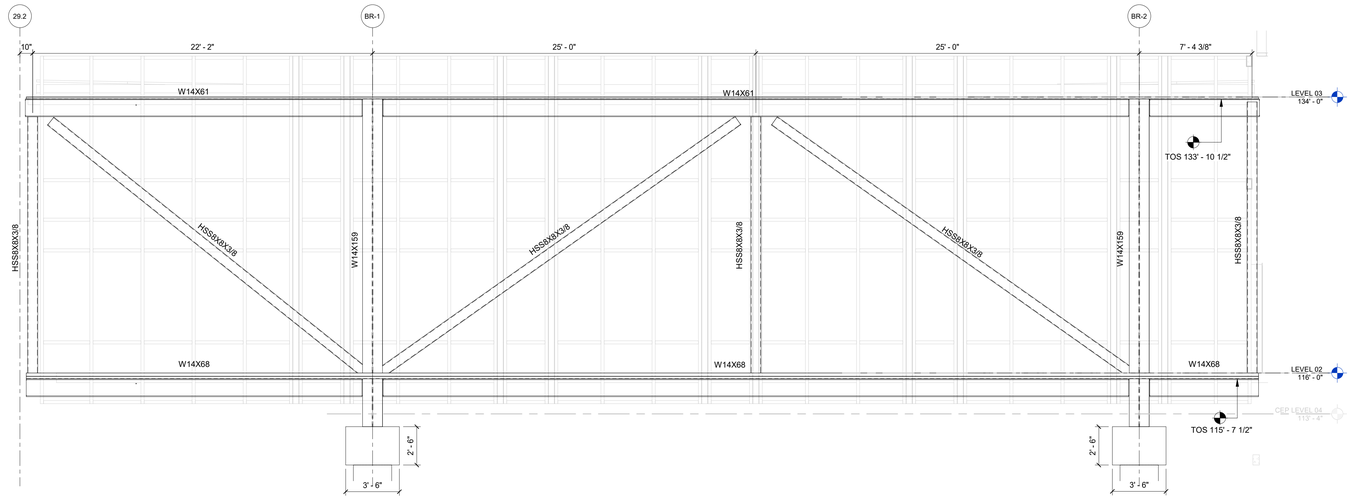
REVISIONS

#	DATE	DESCRIPTION

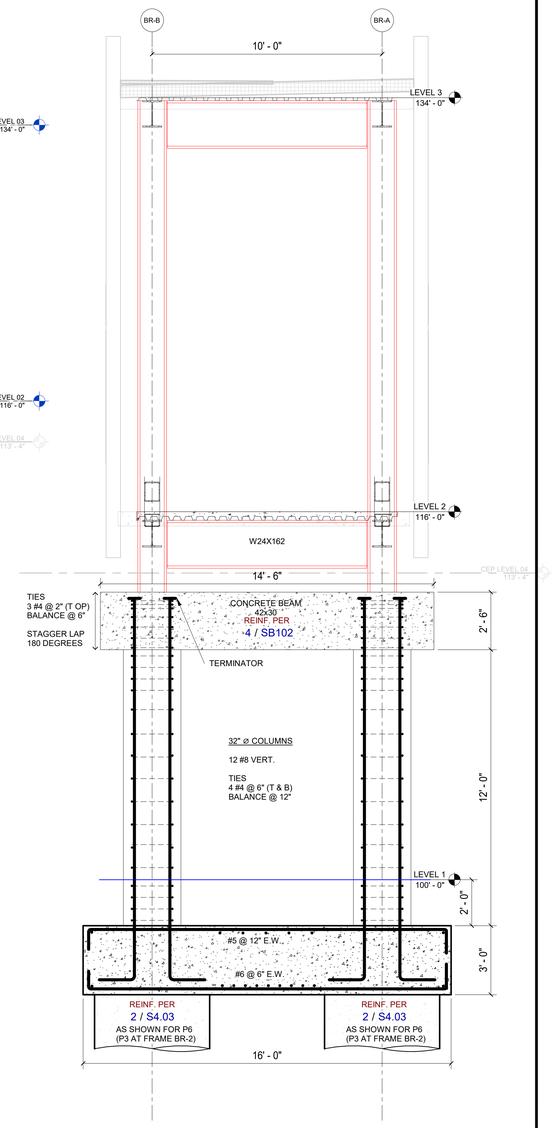
JOB NUMBER: 21-335-1
DATE: 12-09-2022
SHEET NUMBER: **SB101**
SHEET TITLE: SKYBRIDGE - ENLARGEMENTS



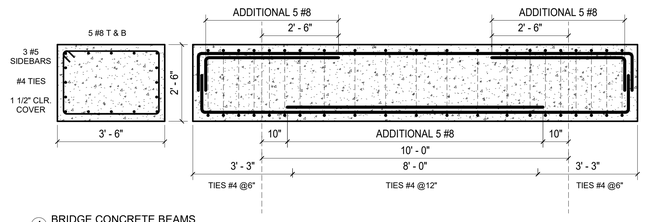
2 SKYBRIDGE FRAME
3/8" = 1'-0"



1 SKYBRIDGE NORTH ELEVATION
1/4" = 1'-0"



3 SKYBRIDGE RIGID FRAME BR-1
3/8" = 1'-0"



4 BRIDGE CONCRETE BEAMS
1/2" = 1'-0"

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REVISIONS

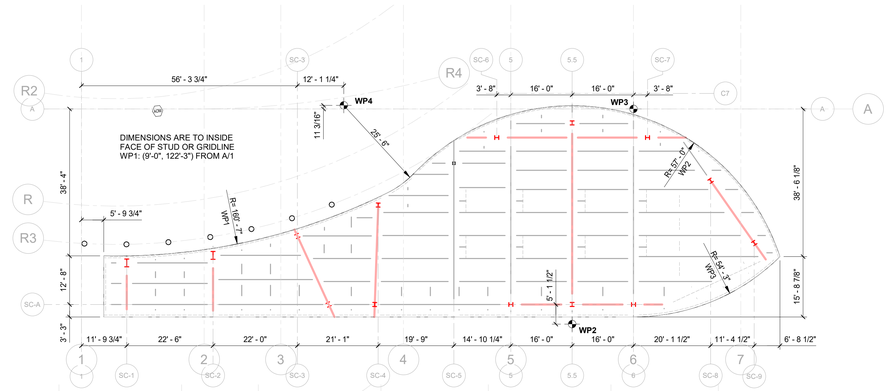
#	DATE	DESCRIPTION

JOB NUMBER: 21-335-1

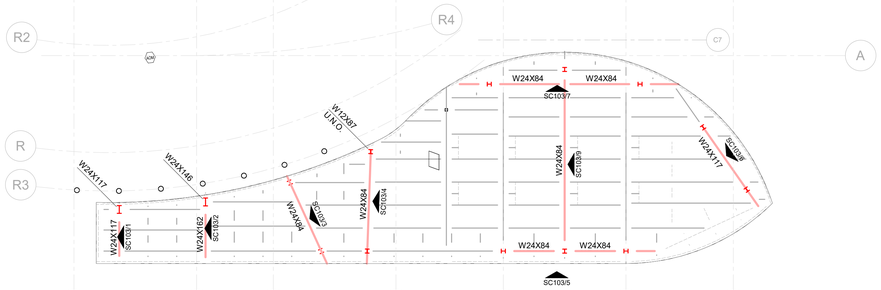
DATE: 12-09-2022

SHEET NUMBER: **SB102**

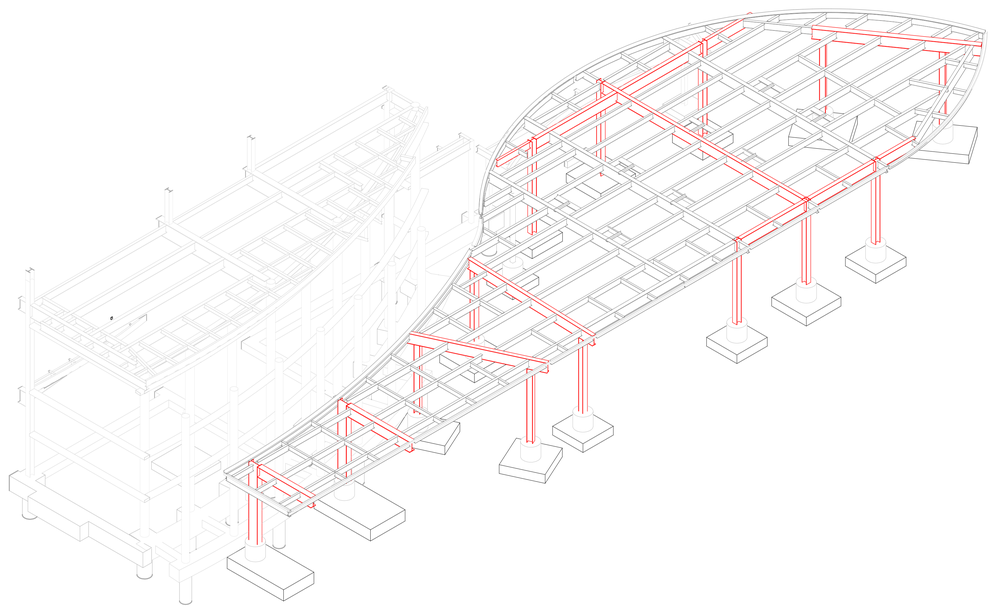
SHEET TITLE: SKYBRIDGE - SECTIONS



2 FRAMING LAYOUT
1/16" = 1'-0"



3 LATERAL FRAME PLAN
1/16" = 1'-0"



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Oklahoma Certificate of Authorization" No. 4570

CLIENT:

SEAL OF THE CHEROKEE NATION
SEPT. 6, 1839

CHEROKEE NATION REPLACEMENT HOSPITAL

TAHLEQUAH, OKLAHOMA

KEY PLAN:

PROJECT PHASE:

BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

#	DATE	DESCRIPTION

JOB NUMBER: 21-335-1

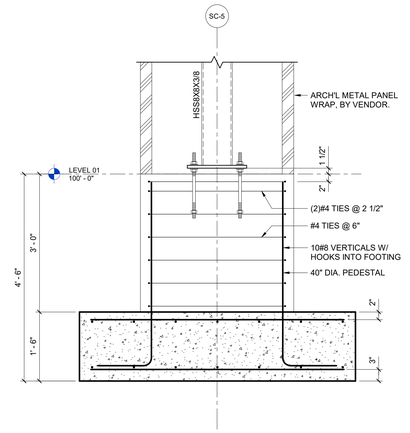
DATE: 12-09-2022

SHEET NUMBER: **SC100**

SHEET TITLE: SOUTH CANOPY

ID	LENGTH	WIDTH	THICKNESS	REINF. (T&B U.N.O.)		STRIP	
				LONG	SHORT	LENGTH	
FC1	13'	7'	2' - 3"	PER SC103		7' FROM END	12 OF 19
FC2	13'	8'	2' - 3"	PER SC103		7' FROM END	12 OF 19
FC3	9'	7'	1' - 9"	PER SC103		7' FROM END	8 OF 11
FC4	8'	8'	1' - 9"	9 #6	9 #6		
FC5	7'	7'	1' - 9"	8 #6	8 #6		
FC6	6'	6'	1' - 6"	6 #6 E.W. (BOT. ONLY)			
FC7	10'	8'	1' - 9"	9 #6	12 #6		

2 FOOTING SCHEDULE SOUTH CANOPY



3 FOUNDATION AT HSS8x8
3/4" = 1'-0"

SOUTH CANOPY FOUNDATION

1. SEE OVERALL FOUNDATION PLAN FOR GRID DIMENSIONS AND ADDITIONAL NOTES.
2. SEE OVERALL FRAMING PLAN FOR WP1 LOCATION
   A. 6'-3" NORTH OF LINE-E
   B. 9'-0" EAST OF LINE-1
3. COLUMNS ARE W12x87 U.N.O.
4. ROUND CONCRETE PEDESTALS SUPPORT ARCH'L METAL PANEL
   A. 40" DIAMETER
   B. EXTEND AT LEAST 4" ABOVE FINISHED GRADE.
   C. SEE FRAME ELEVATIONS FOR T/ PIER
5. SPREAD FOOTING ELEVATIONS PER PLAN
6. ANGULAR DIMENSIONS WITH MORE THAN 2 DECIAML PLACES ARE ROUNDED AND MAY NOT BE EXACT.

KEY NOTES:

01 - FIELD MAY TRIM FORM AND REBAR UP TO 6" AT FOUNDATION CORNER NEAR RADIAL GRAD BEAM FO CONVENINCE

1 01 FOUNDATION PLAN SC
3/16" = 1'-0"

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TAHLEQUAH, OKLAHOMA

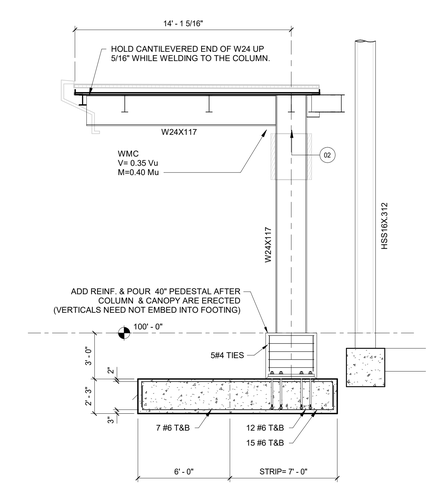
KEY PLAN:

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

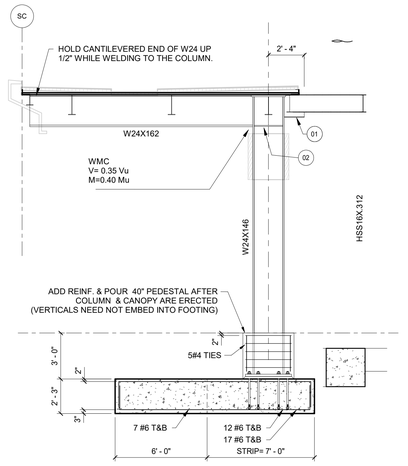
REVISIONS

#	DATE	DESCRIPTION

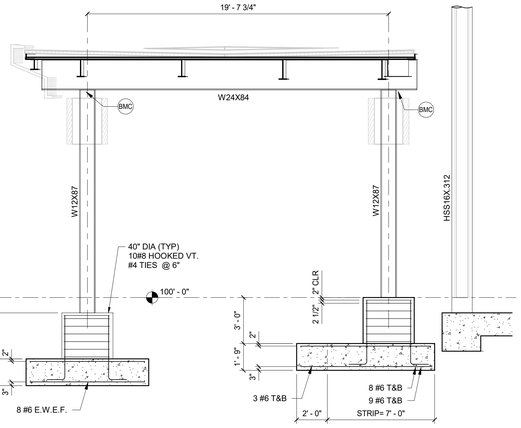
JOB NUMBER: 21-335-1
DATE: 12-09-2022
SHEET NUMBER: **SC101**
SHEET TITLE: SOUTH CANOPY - FOUNDATION PLAN



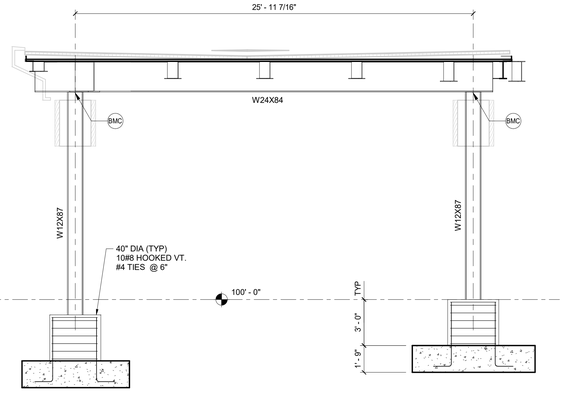
1 FRAME @ SC-1'
1/4" = 1'-0"



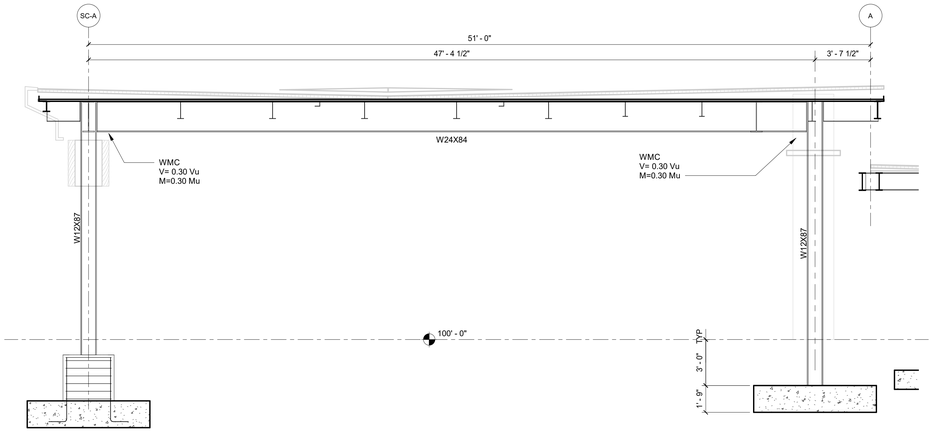
2 FRAME @ SC-2'
1/4" = 1'-0"



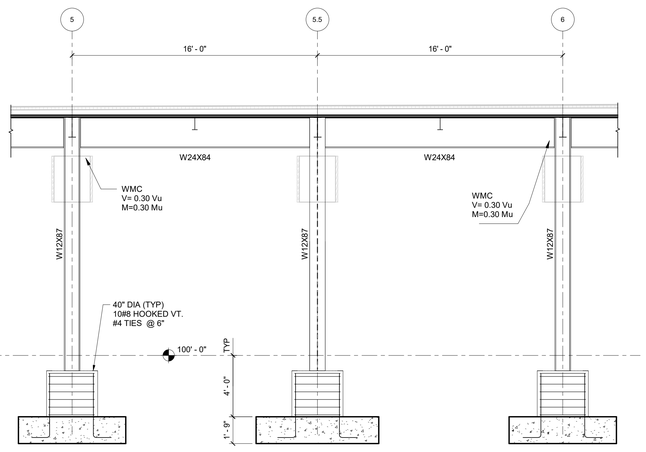
3 FRAME @ SC-3'
1/4" = 1'-0"



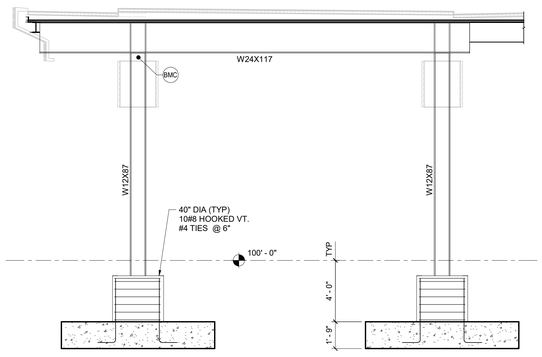
4 FRAME @ SC-4'
1/4" = 1'-0"



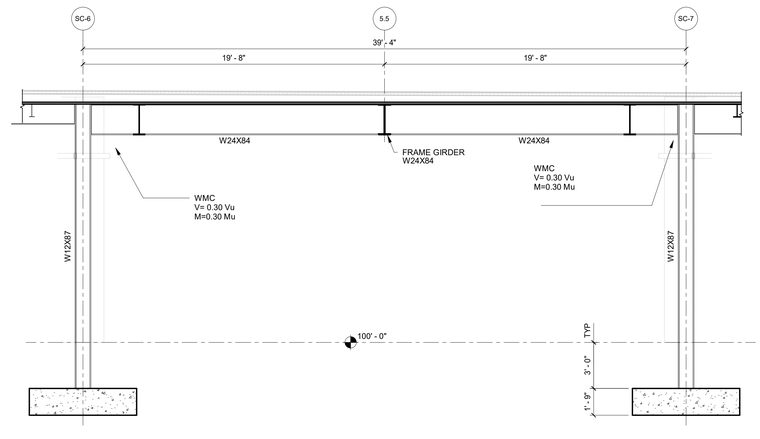
9 FRAME @ SC-5.5'
1/4" = 1'-0"



5 FRAME @ SC-A
1/4" = 1'-0"



6 FRAME @ SC-B'
1/4" = 1'-0"



7 FRAME @ SC-C'
1/4" = 1'-0"

SOUTH CANOPY FRAMES

1. SEE OVERALL FOUNDATION PLAN FOR FOOTING INFORMATION NOT SHOWN
2. SEE SOUTH CANOPY FOUNDATION PLAN FOR RADIAL WP LOCATIONS
3. DECK BEARING ELEVATION = 115' - 6 12" U.N.O.
4. ALL ANGULAR DIMENSIONS WITH MORE THAN 2 DECIAML PLACES ARE ROUNDED AND MAY NOT BE EXACT.
5. WMC = WELDED MOMENT CONNECTION
6. BMC = BOLTED MOMOENT CONNECTION. AT CONDITION WHERE THE BEAM SPANS OVER THE COLUMN. MINIMUM 3/8" STIFFENER PLATES SHALL MATCH THE COLUMN FLANGES. ADDITIONAL ALL COLUMN CAP PLATES SHALL BE DESINGED WIHT A 25K NET UP LIFT FORCE IN ADDITION TO ALL SHEAR AND MOMENT FORCES.
7. AT ALL W12 LATERAL FRAME COLUMN'S MOMENT CONNECTIONA AT THE TOP SHALL BE DESIGNE FOR : $M_u = \emptyset_b M_p$ AND $V_u = \emptyset_v V_n$ (REFERENCE AISC 14TH ED TABLE 3.6)

KEY NOTES:

01 - W18 STUB SHOP ATTACHED TO COLUMN.

02 - AT W24 COLUMNS. THE WELDED CAP PLATE SHALL BE A MINIMUM THICKNESS AND SAME GRADE AS THE BEAM FLANGE THICKNESS. THE COLUMN STIFFENER PLATES FOR THE BOTTOM FLANGE SHALL BE A MINIMUM OF 75% (+/- 5%) OF THE BEAM FLANGE THICKNESS. COLUMN FLANGE STINNER PLATES MAY BE HALF COLUMN DEPTH IS ACCEPTABLE IF DESIGN FORCES ARE MEET. TOTAL COMBINED FILLET WELDS ON THE COLUMN STIFFENER, OPPOSING THE W24 BEAM FLANGE SHALL BE NO LESS THAN 75% OF THE OF TEH COLUMN STIFFENER PLATE THICKNESS. a CJP WELD IS AN ALTERNATE OPTION.

$LET\ M_u = \emptyset_b M_p$
$LET\ V_u = \emptyset_v V_n$

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SEPT. 6, 1839

CHEROKEE NATION REPLACEMENT HOSPITAL
TAHLEQUAH, OKLAHOMA

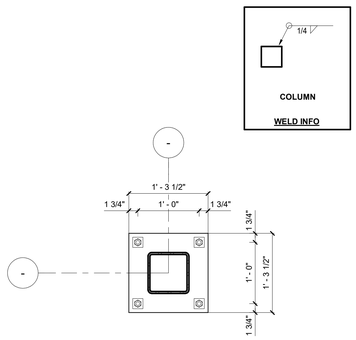
KEY PLAN:

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

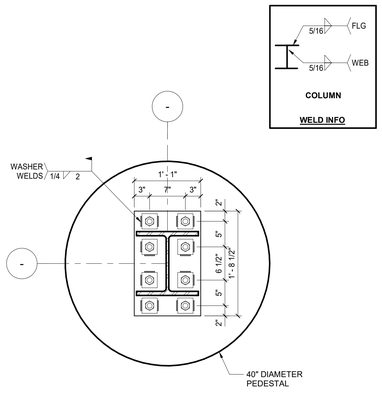
REVISIONS

#	DATE	DESCRIPTION

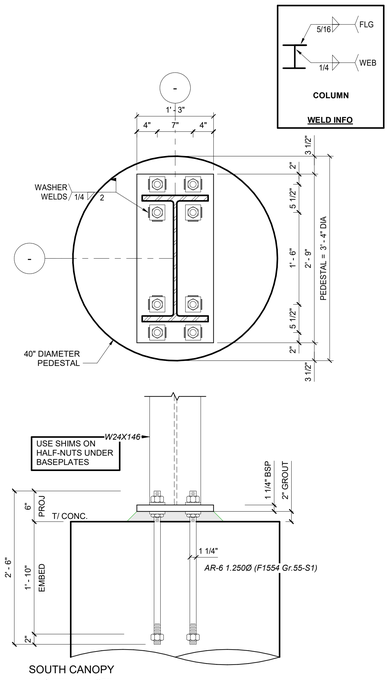
JOB NUMBER: 21-335-1
DATE: 12-09-2022
SHEET NUMBER: **SC103**
SHEET TITLE: SOUTH CANOPY - FRAMES



SOUTH CANOPY
1 BASEPLATE #50
1" = 1'-0"



SOUTH CANOPY
2 BASEPLATE #51
1" = 1'-0"



SOUTH CANOPY
3 BASEPLATE #52
1" = 1'-0"

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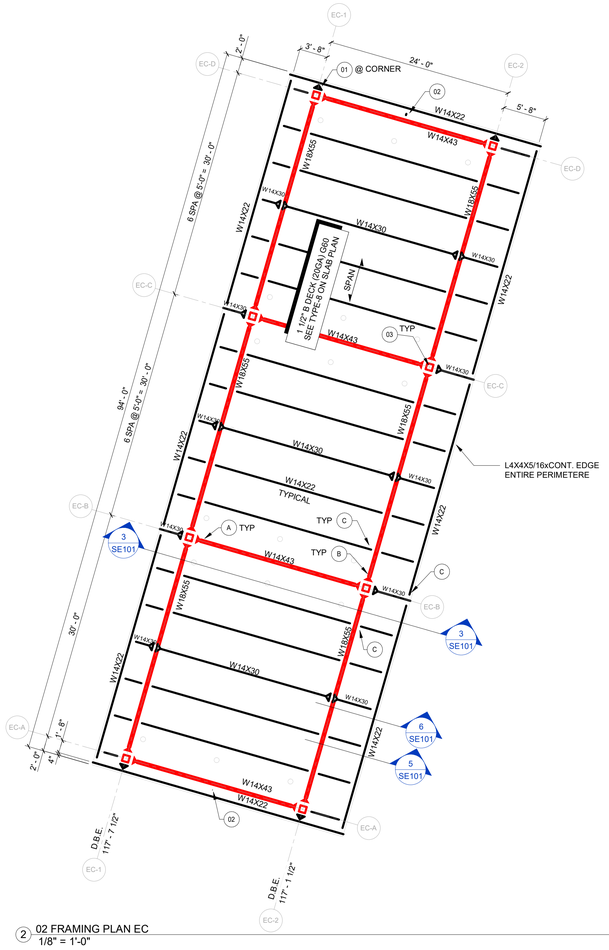
REVISIONS

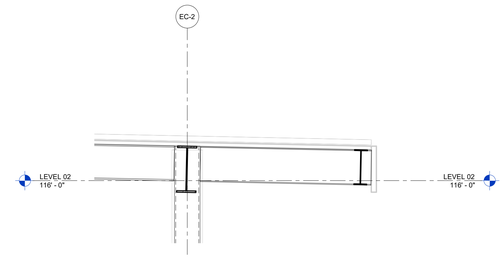
#	DATE	DESCRIPTION

JOB NUMBER: 21-335-1
DATE: 12-09-2022
SHEET NUMBER: **SC104**
SHEET TITLE: SOUTH CANOPY - DETAILS

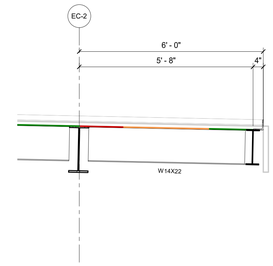
1 01 FOUNDATION PLAN EC
1/8" = 1'-0"



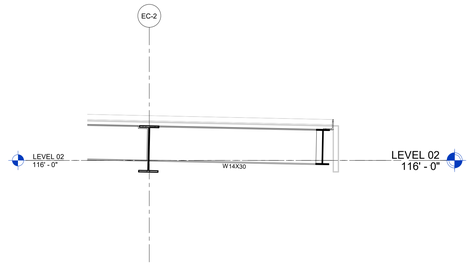
2 02 FRAMING PLAN EC
1/8" = 1'-0"



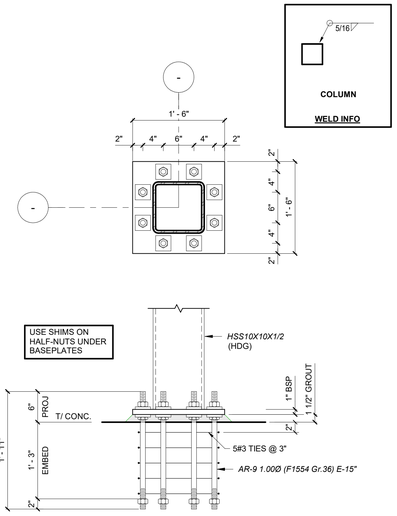
7 East canopy4
1/2" = 1'-0"



5 East canopy2
1/2" = 1'-0"



6 East canopy3
1/2" = 1'-0"

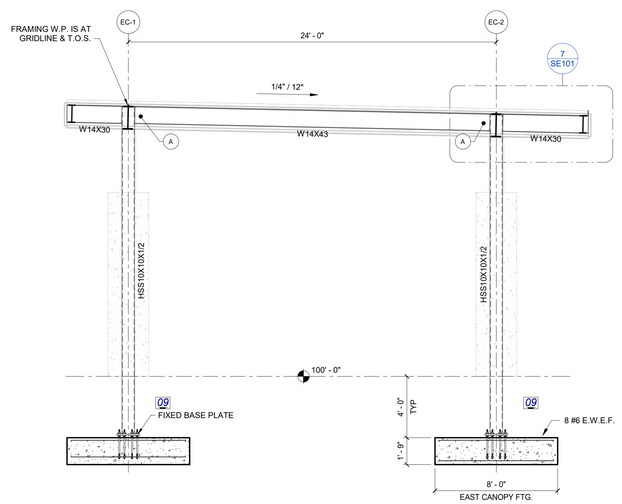


DETAILING REFERENCE: #09

EAST CANOPY BASEPLATE NOTES:

1. TACK WELD OR DAMAGE ANCHOR ROD'S EMBEDED NUT PRIOR TO SHIPPING
2. BASE PLATES ARE 50 KSI, U.N.O.
3. ANCHOR ROD HOLES AND WASHERS ARE PER AISC 360 (U.N.O.):
   T14-2 RECOMMENDED MAXIMUM SIZES FOR ANCHOR-ROD HOLES IN BASE PLATES.
4. NON-SHRINK GROUT (MIN. 5,000 PSI) PER GENERAL NOTES.

4 BASEPLATE #09 (FIXED)
1" = 1'-0"



3 East canopy
1/4" = 1'-0"

EAST CANOPY FOUNDATION

1. SEE OVERALL FOUNDATION PLAN FOR GRID DIMENSIONS AND ADDITIONAL NOTES.
2. SEE OVERALL FRAMING PLAN FOR WP1 LOCATION
   A. 6'-3" NORTH OF LINE-E
   B. 9'-0" EAST OF LINE-1
3. TOP OF SPREAD FOOTING = XX' - 0" U.N.O.
4. ANGULAR DIMENSIONS WITH MORE THAN 2 DECIAML PLACES ARE ROUNDED AND MAY NOT BE EXACT.

EAST CANOPY FRAMES

1. ALL FRAMING MEMBERS ARE SLOPED AND ROTATED TO BE NORMAL TO DECK BEARING
2. BEAM SHEAR AND MOMENT CONNECTIONS SHALL BE DESIGNED FOR THE FOLLOWING PERCENTAGE OF ITS MAXIMUM UNIFORM LOAD CAPACITY PER AISC 360-10 TABLE 3-6.
   A. W14 MOMENT FRAME BEAM
      - Vu=25%, Mu=30%
   B. W18 MOMENT FRAME GIRDER
      - Vu=25%, Mu=25%
   C. W14 BEAMS
      - Vu=25%
   D. ALL MOMNET CONNECTIONS ARE CJP WELDS TO THE COLUMN

KEY NOTES:

01 - W14x30 STUB, SHOP WELDED TO EACH CORNER COLUMN

02 - W14X22 @ MID SPAN

03 - COLUMN CAP PLATES ARE 1" THICK, 50 KSI AND SLOPED TO MATCH THE ROOF SLOPE. ATTACHMENT OF THE CAP PLATE TO THE COLUMN SHABE BE A MINIMU OF 5/16" FILLETS WELDS OR 5/16" PJP WELDS. (BUTT WELDS ARE NOT ACCEPTABLE).

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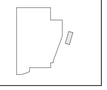
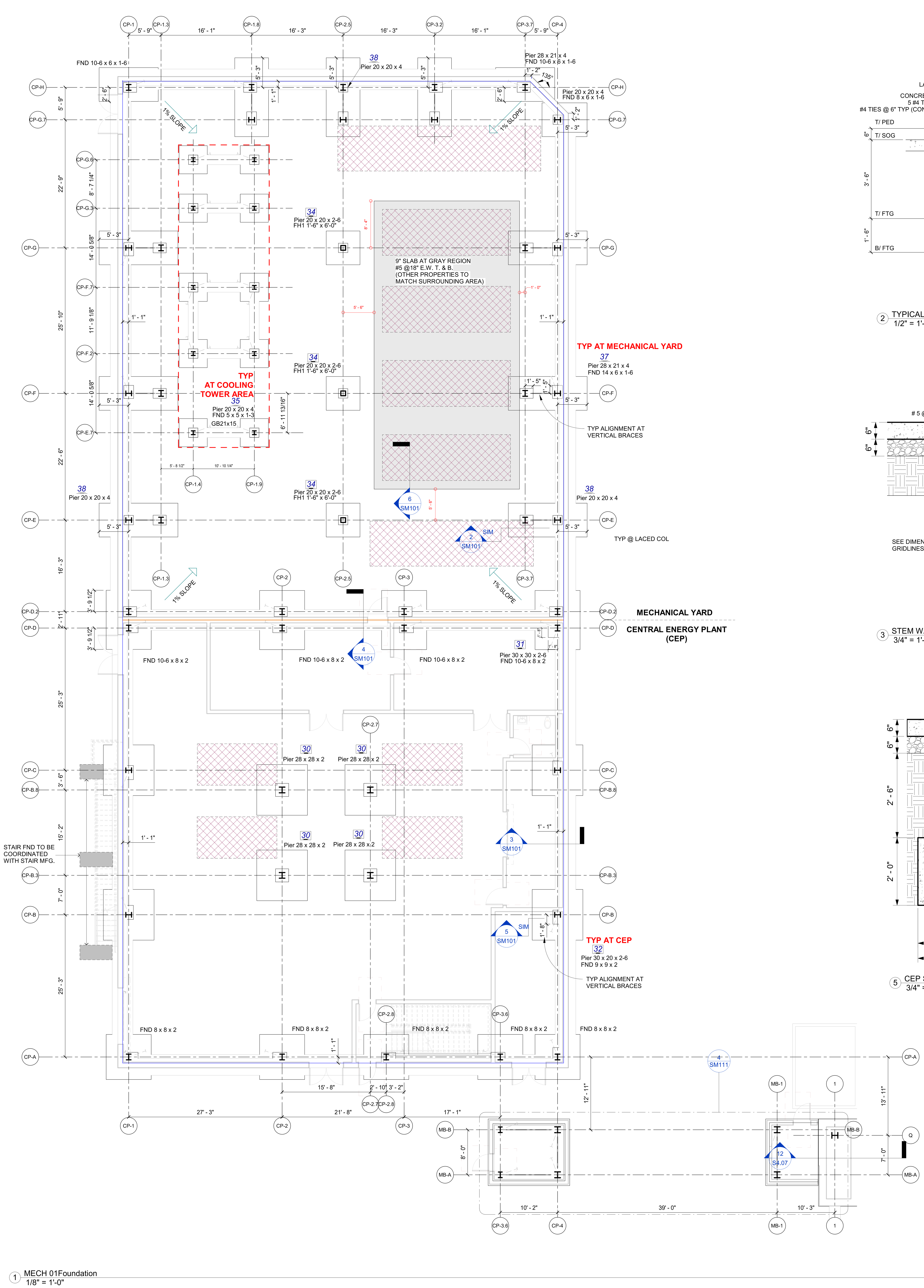


PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

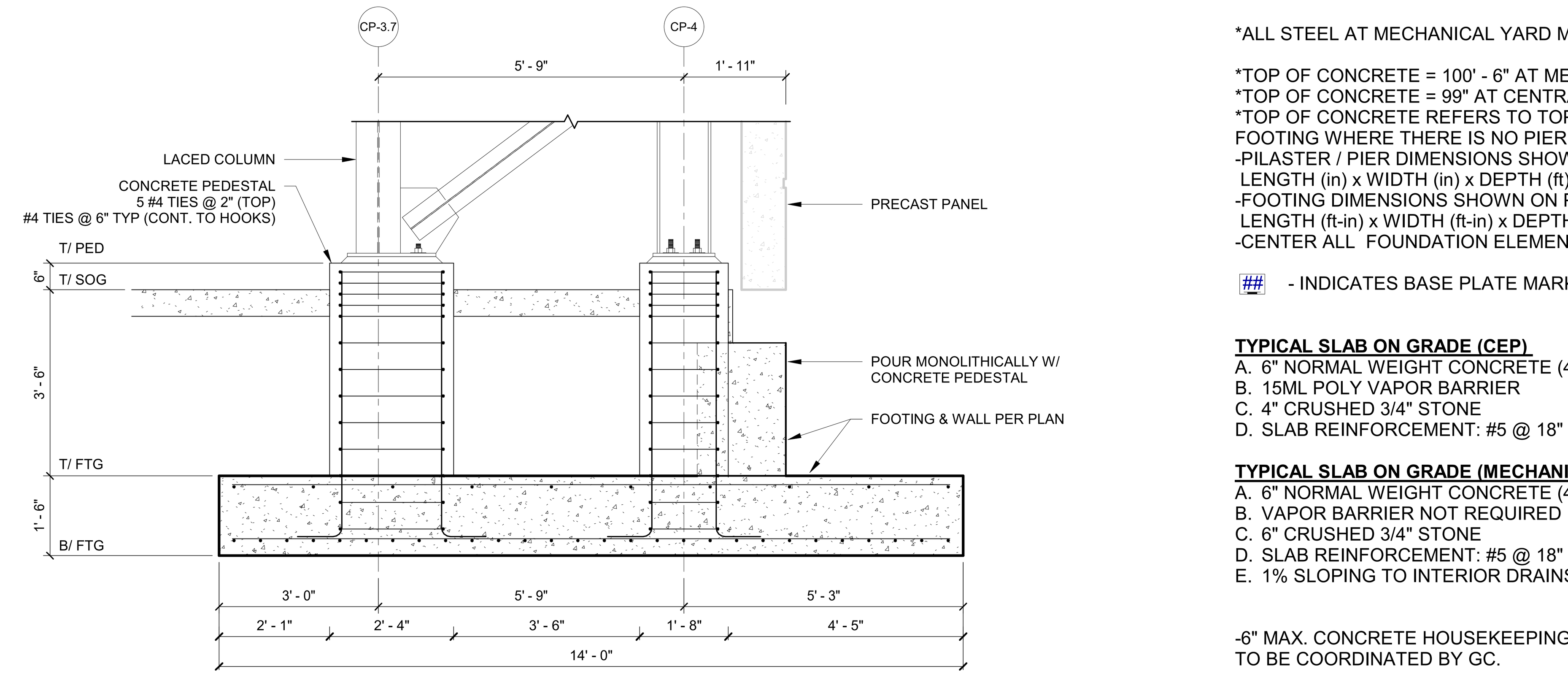
REVISIONS

#	DATE	DESCRIPTION

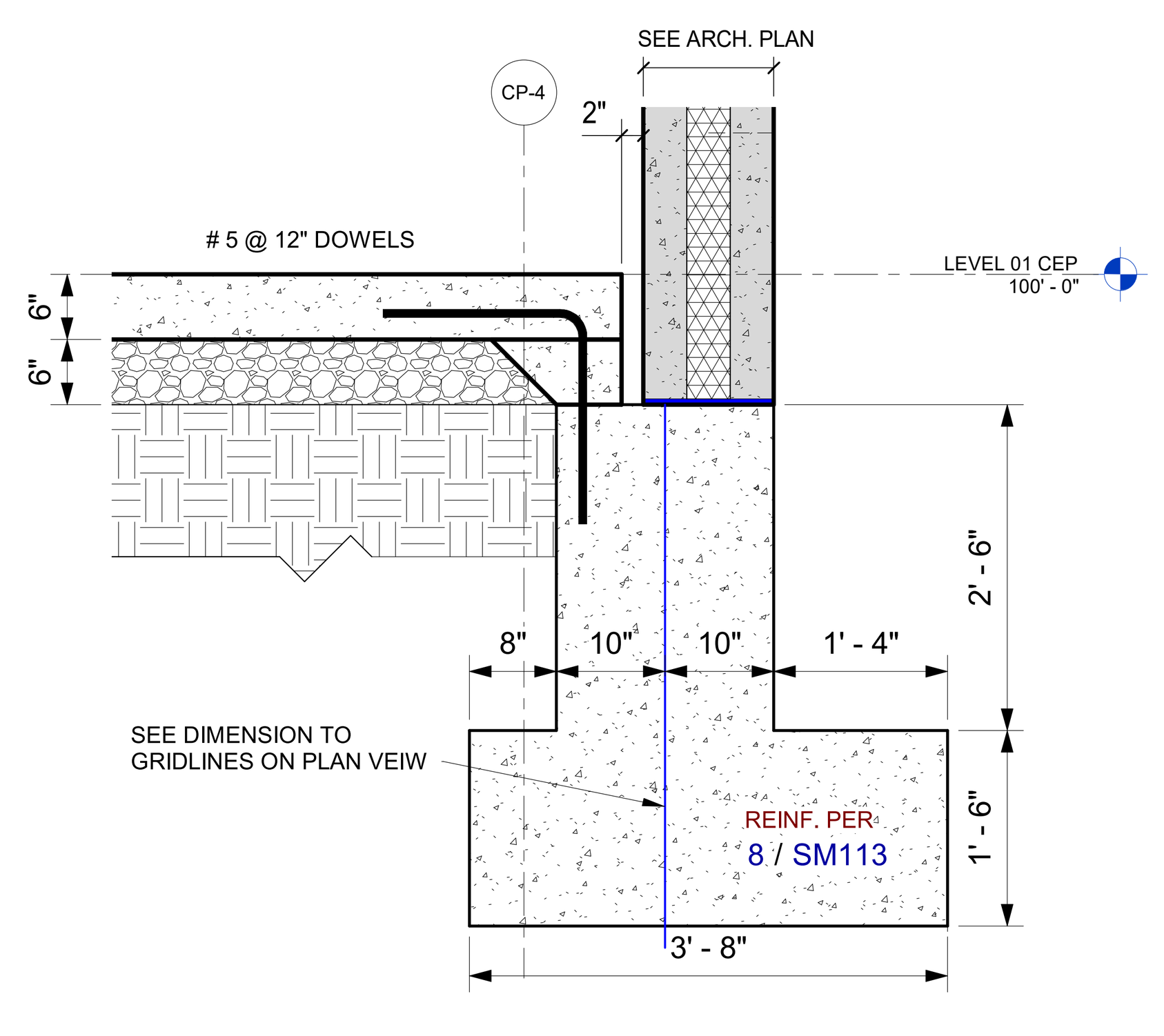
JOB NUMBER: 21-335-1
DATE: 12-09-2022
SHEET NUMBER: **SE101**
SHEET TITLE: EAST CANOPY - STRUCTURAL PLAN



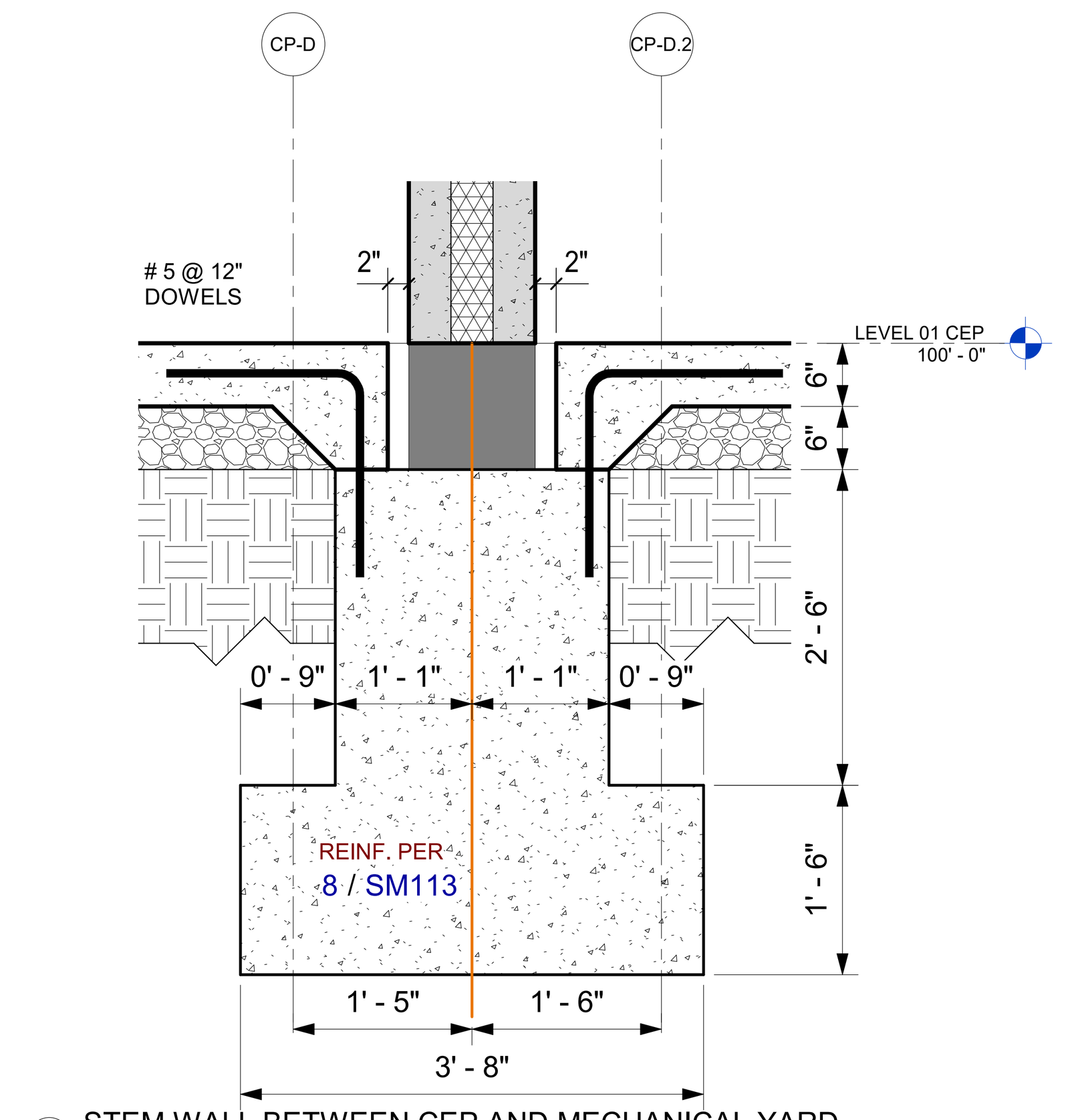
1 MECH 01Foundation
1/8" = 1'-0"



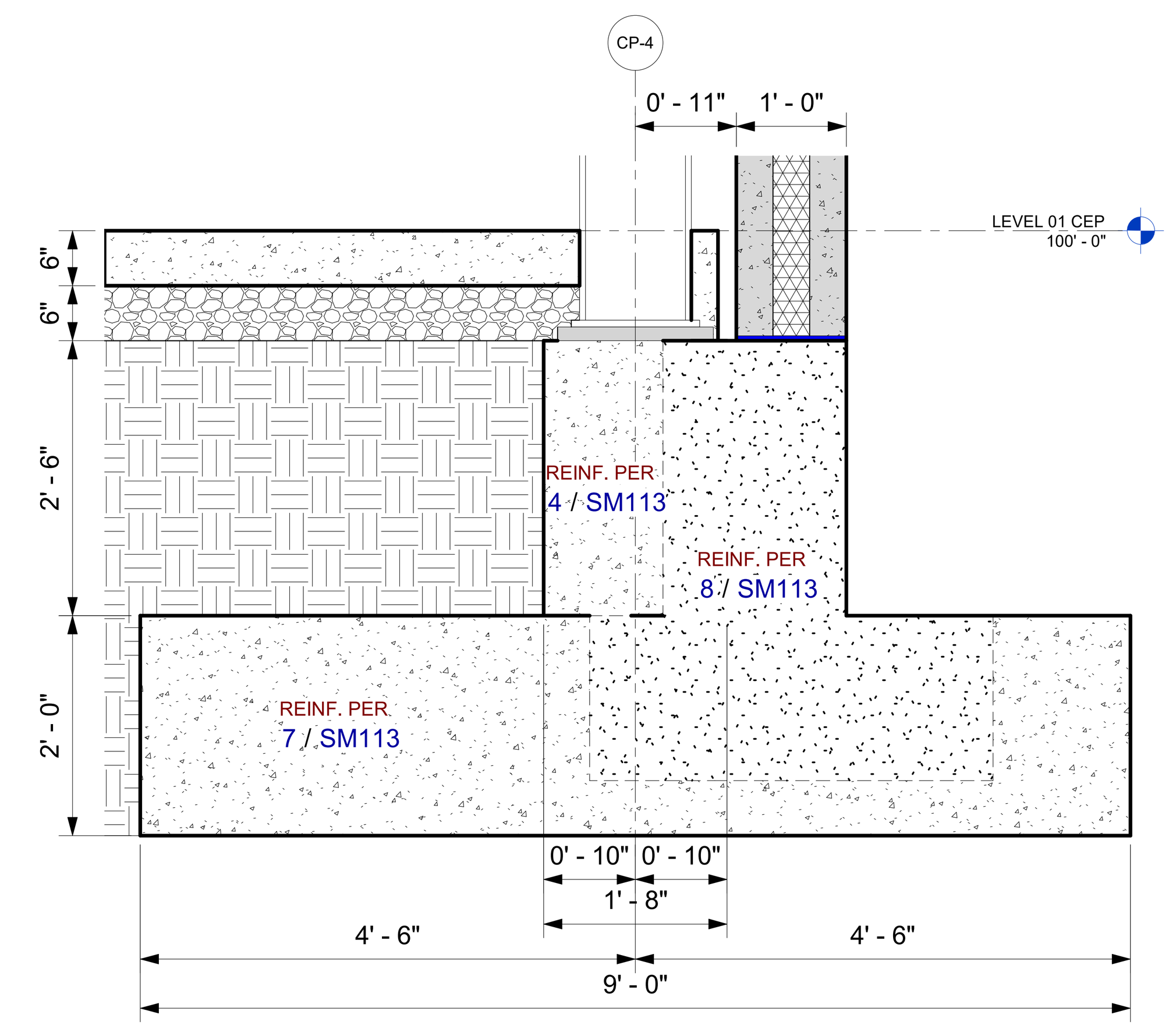
2 TYPICAL AT LACED COLUMN
1/2" = 1'-0"



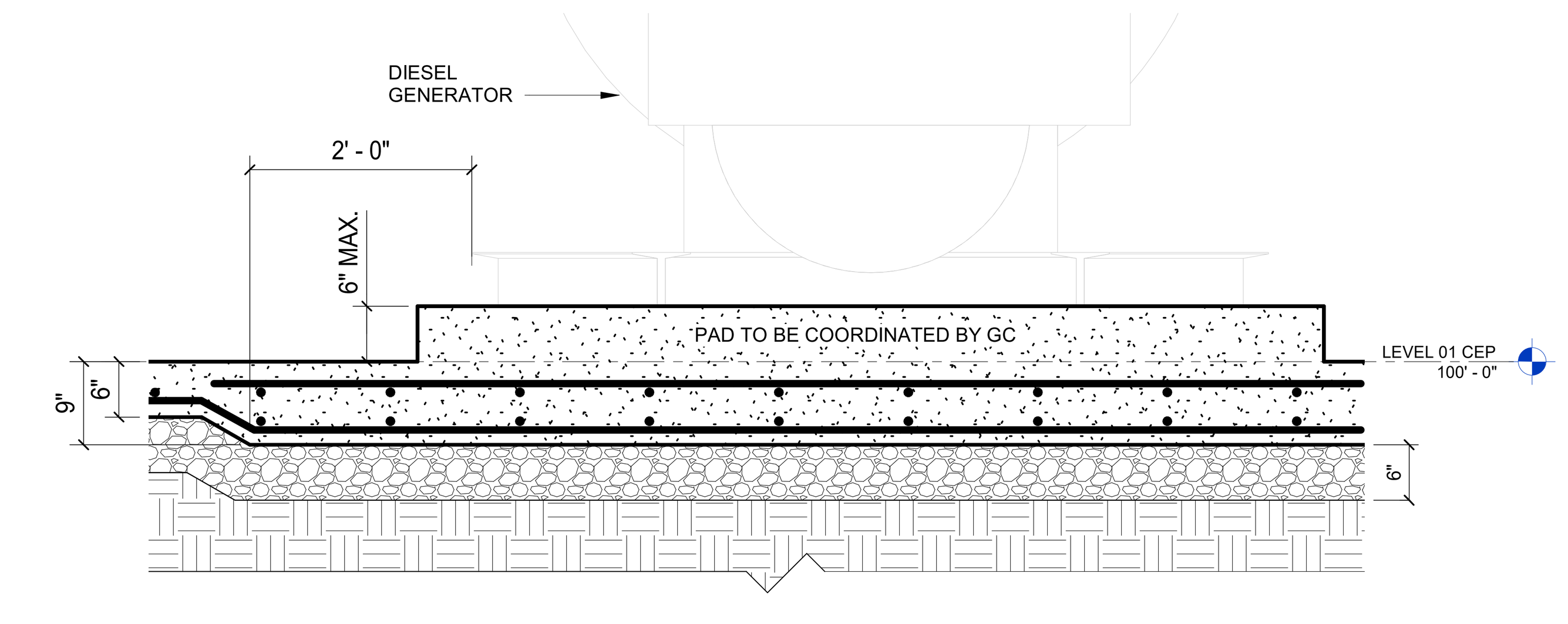
3 STEM WALL AROUND CEP AND MECHANICAL YARD PERIMETER
3/4" = 1'-0"



4 STEM WALL BETWEEN CEP AND MECHANICAL YARD
3/4" = 1'-0"



5 CEP STEM WALL FOUNDATION1
3/4" = 1'-0"



6 THICKENED SLAB AT MECHANICAL YARD
3/4" = 1'-0"

MECHANICAL NOTES

*ALL STEEL AT MECHANICAL YARD MUST BE GALVANIZED

*TOP OF CONCRETE = 100' - 6" AT MECHANICAL YARD U.N.O.
*TOP OF CONCRETE = 99" AT CENTRAL ENERGY PLANT U.N.O.
*TOP OF CONCRETE REFERS TO TOP OF PIER, OR TOP OF FOOTING WHERE THERE IS NO PIER REQUIRED
-PILASTER / PIER DIMENSIONS SHOWN ON PLAN:
LENGTH (in) x WIDTH (in) x DEPTH (ft).
-FOOTING DIMENSIONS SHOWN ON PLAN:
LENGTH (ft-in) x WIDTH (ft-in) x DEPTH (ft-in).
-CENTER ALL FOUNDATION ELEMENTS UNDER COLUMNS U.N.O.

- INDICATES BASE PLATE MARK

TYPICAL SLAB ON GRADE (CEP)
A. 6" NORMAL WEIGHT CONCRETE (4000 PSI)
B. 15ML POLY VAPOR BARRIER
C. 4" CRUSHED 3/4" STONE
D. SLAB REINFORCEMENT: #5 @ 18" E.W. U.N.O.

TYPICAL SLAB ON GRADE (MECHANICAL YARD)
A. 6" NORMAL WEIGHT CONCRETE (4000 PSI) AIR ENTRAINED
B. VAPOR BARRIER NOT REQUIRED
C. 6" CRUSHED 3/4" STONE
D. SLAB REINFORCEMENT: #5 @ 18" E.W. U.N.O.
E. 1% SLOPING TO INTERIOR DRAINS

-6" MAX. CONCRETE HOUSEKEEPING PAD TO BE COORDINATED BY GC.

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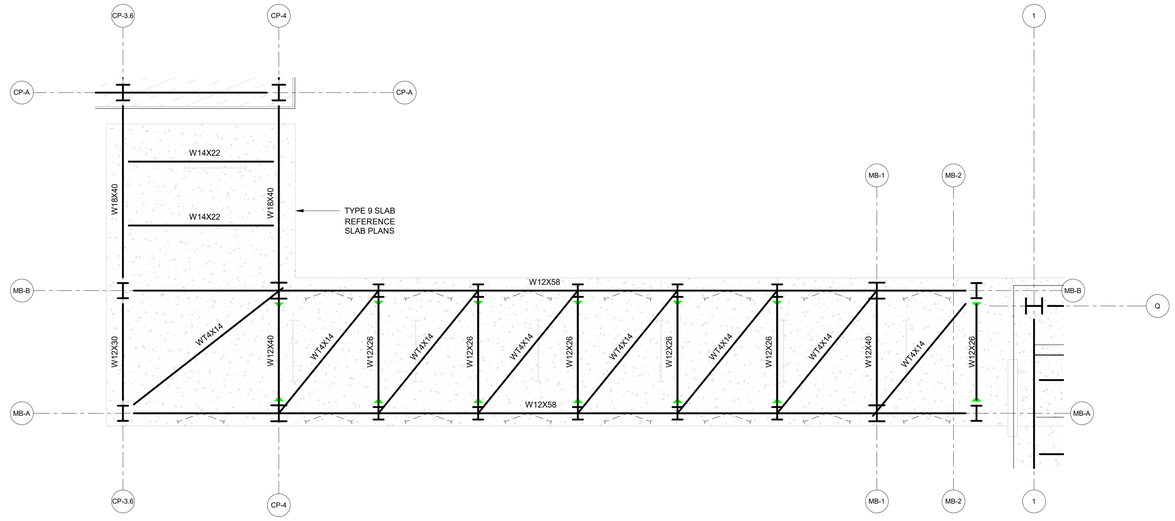
KEY PLAN

PROJECT PHASE:
BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

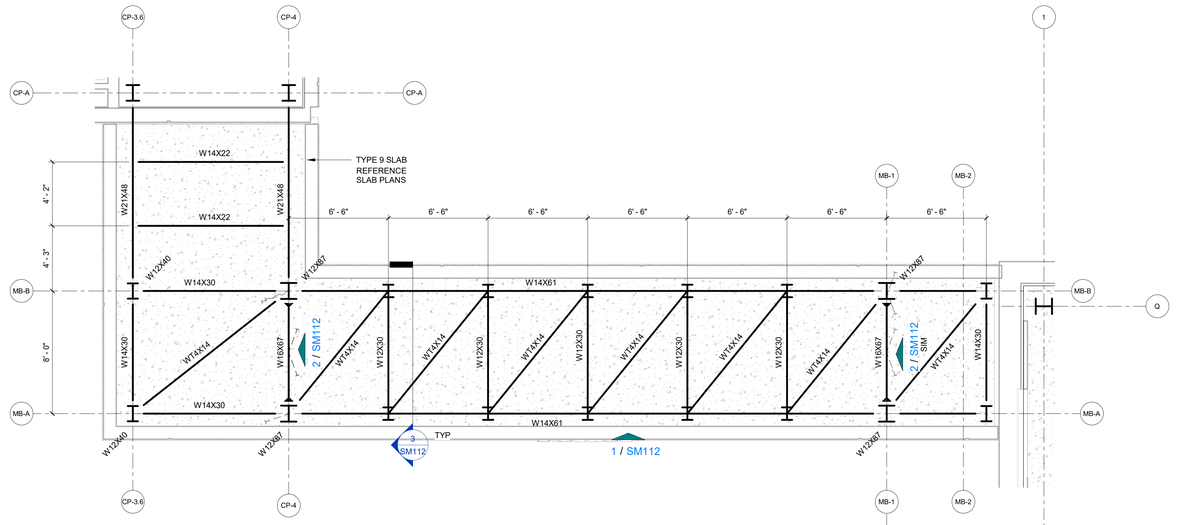
REVISIONS

#	DATE	DESCRIPTION

JOB NUMBER: 21-335-1
DATE: 12-09-2022
SHEET NUMBER: SM101
SHEET TITLE: MECHANICAL AREA - OVERALL FOUNDATION PLAN



1 MECH CEP Roof - BRIDGE ENLARGEMENT
1/4" = 1'-0"



2 MECH CEP 02 - BRIDGE ENLARGEMENT
1/4" = 1'-0"

4 MECH 01Foundation - BRIDGE
1/4" = 1'-0"

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Structural Engineering Services
6900 College Blvd. Suite 600
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Ph. (913) 814-0404

Oklahoma Certificate of
Authorization" No. 4570

CLIENT:

SEAL OF THE CHEROKEE NATION
SEPT. 6, 1839

CHEROKEE NATION REPLACEMENT HOSPITAL

TAHLEQUAH, OKLAHOMA

KEY PLAN:

PROJECT PHASE:

BID PACKAGE 04
(STRUCTURAL CONCRETE / EARTHWORK)

REVISIONS

#	DATE	DESCRIPTION
1	Date 1	Revision 1

JOB NUMBER: 21-335-1

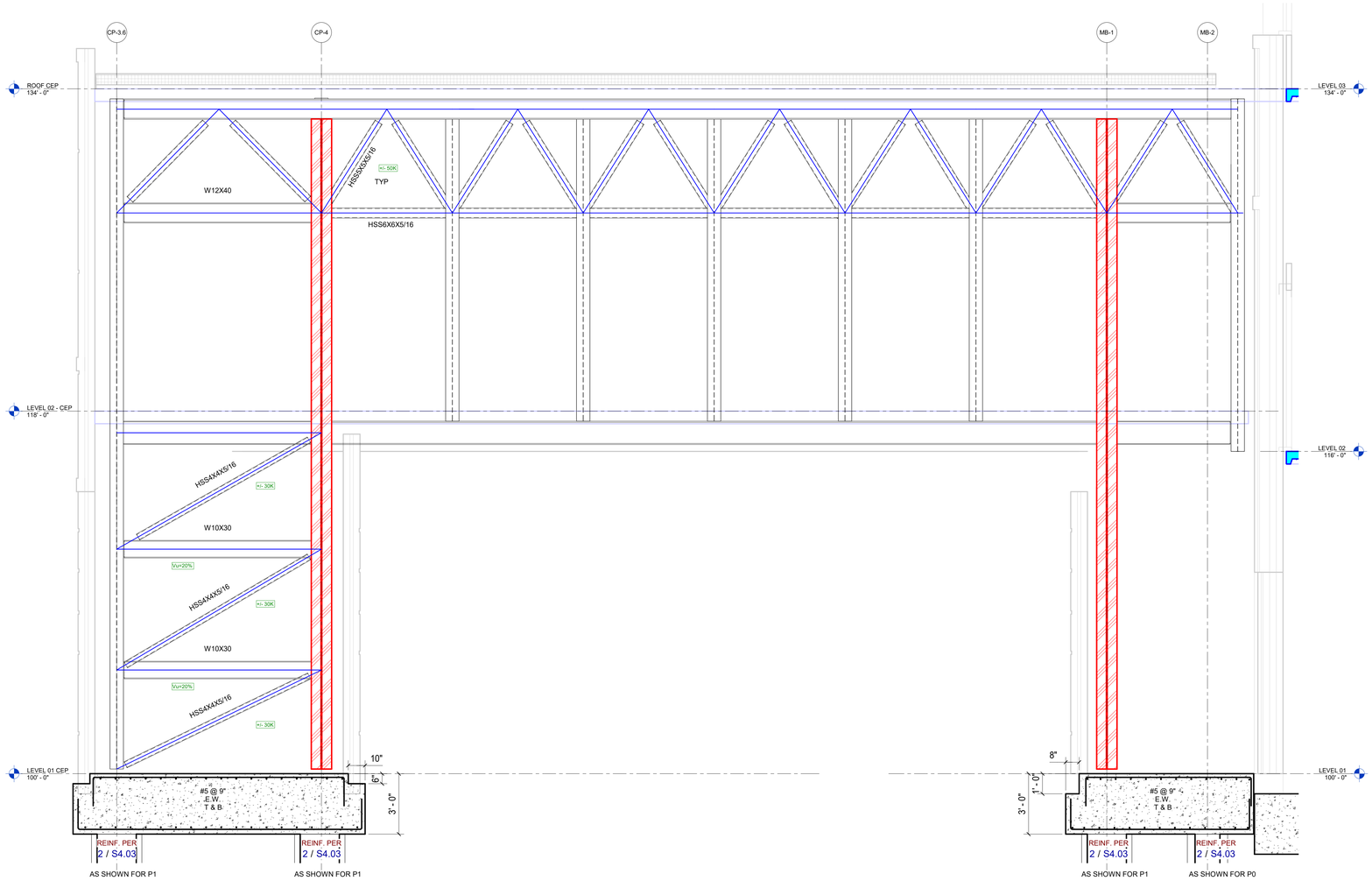
DATE: 12-09-2022

SHEET NUMBER:

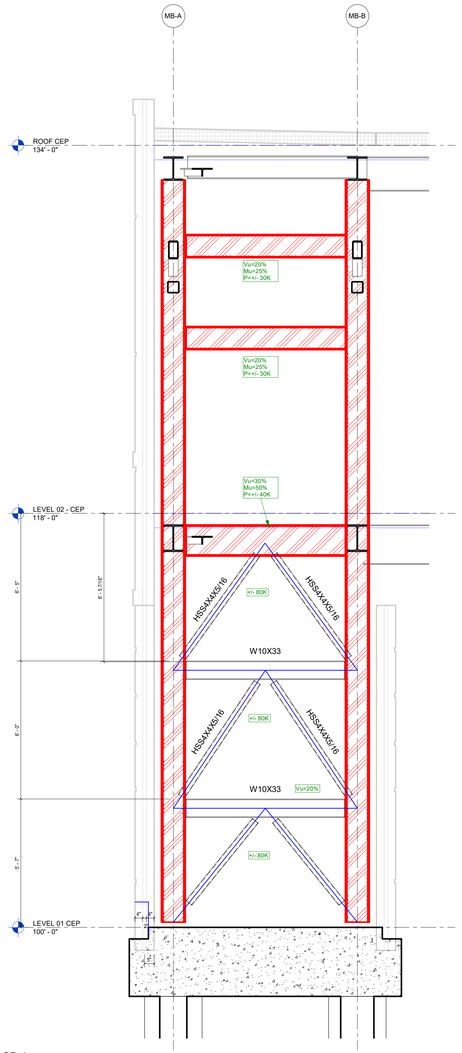
SM111

SHEET TITLE:

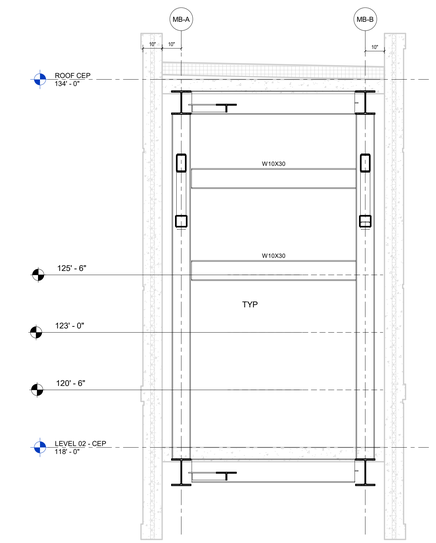
MECHANICAL AREA (BRIDGE) - ENLARGEMENTS



1 CEP BRIDGE MB-A
3/8" = 1'-0"



2 CEP BRIDGE CP-4
3/8" = 1'-0"



3 CEP- BRIDGE TYPICAL SUPPORT FOR PIPES AT FRAMES
3/8" = 1'-0"

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479-783-2480
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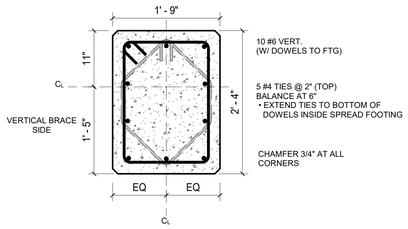
SM112

SHEET TITLE:
MECHANICAL AREA (BRIDGE) - SECTIONS

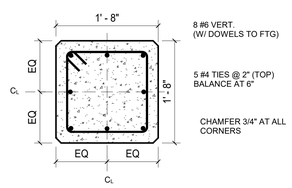
*2" CONCRETE CLEAR COVER U.N.O.

PIER 28 x 21



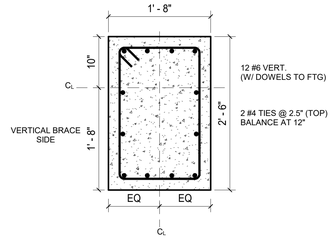
1 LATERAL PEDESTAL - 28x21

PIER 20 x 20



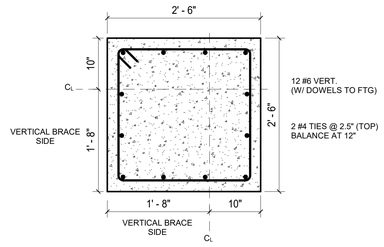
6 PILASTER Mechanical yard 2

PIER 30 x 20



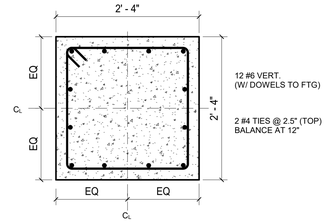
2 PILASTER CEP 1

PIER 30 x 30



4 PILASTER CEP 2

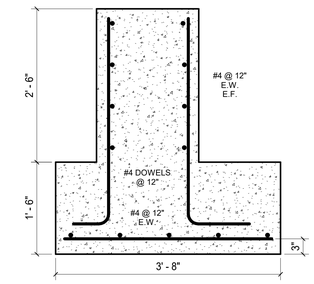
PIER 28 x 28



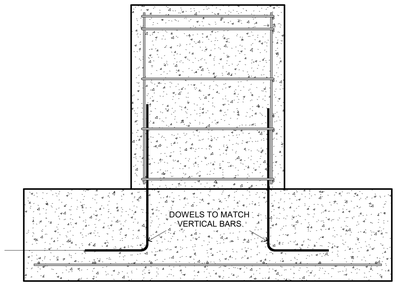
3 PILASTER CEP 3

TYPE	LENGTH	WIDTH	THICKNESS	REINFORCEMENT (E.W.)		COMMENTS
				BOTTOM	TOP	
	8'	8'	2'	#6 @ 6"	#6 @ 1' - 6"	CEP
	9'	9'	2'			
	10' - 6"	8'	2'			CEP - MECH. YARD
	8'	6'	1' - 6"			MECH YARD
	10' - 6"	6'	1' - 6"			
	14'	6'	1' - 6"			
	6'	6'	1' - 6"	#5 @ 6"	N.A.	
	5'	5'	1' - 3"			COOLING TOWER

7 FOOTING SCHEDULE - CEP AND MECHANICAL YARD



8 TYPICAL STEM WALL - CEP AND MECHANICAL YARD



5 TYPICAL PILASTER - FOOTING BOND

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REVISIONS

#	DATE	DESCRIPTION

JOB NUMBER: 21-335-1

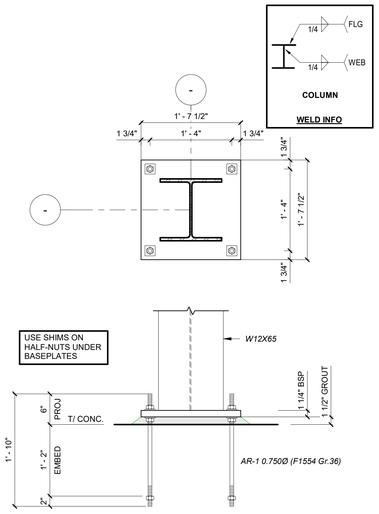
DATE: 12-09-2022

SHEET NUMBER:

SM113

SHEET TITLE:

MECHANICAL AREA - FOUNDATION DETAILS

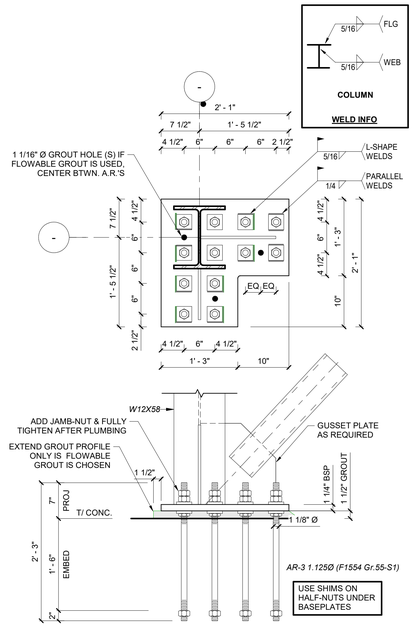


DETAILING REFERENCE: #30

C.E.P. BASEPLATE NOTES:

1. TACK WELD OR DAMAGE ANCHOR ROD'S EMBEDED NUT PRIOR TO SHIPPING
2. BASE PLATES ARE 50 KSI, U.N.O.
3. ANCHOR ROD HOLES AND WASHERS ARE PER AISC 360 (U.N.O.):
   T14-2 RECOMMENDED MAXIMUM SIZES FOR ANCHOR-ROD HOLES IN BASE PLATES.
4. NON-SHRINK GROUT (MIN. 5,000 PSI) PER GENERAL NOTES.

1 BASEPLATE #30
1" = 1'-0"

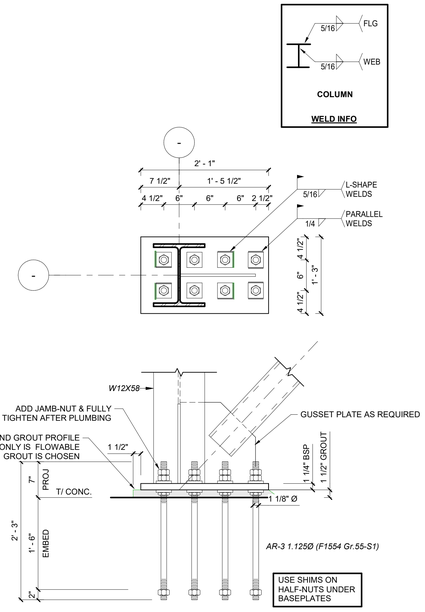


DETAILING REFERENCE: #31

C.E.P. BASEPLATE NOTES:

1. TACK WELD OR DAMAGE ANCHOR ROD'S EMBEDED NUT PRIOR TO SHIPPING
2. BASE PLATES ARE 50 KSI, U.N.O.
3. ANCHOR ROD HOLES AND WASHERS ARE PER AISC 360 (U.N.O.):
   T14-2 RECOMMENDED MAXIMUM SIZES FOR ANCHOR-ROD HOLES IN BASE PLATES.
4. NON-SHRINK GROUT (MIN. 5,000 PSI) PER GENERAL NOTES.

2 BASEPLATE #31
1" = 1'-0"



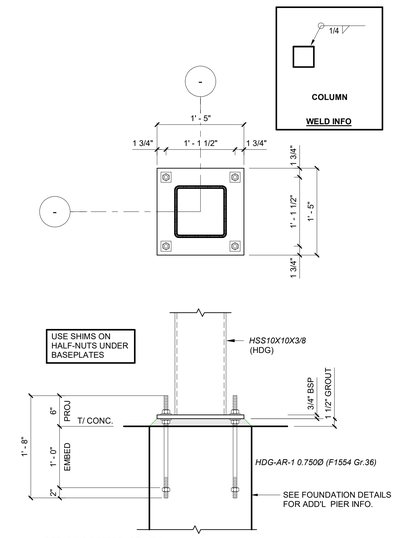
DETAILING REFERENCE: #32

C.E.P. BASEPLATE NOTES:

1. TACK WELD OR DAMAGE ANCHOR ROD'S EMBEDED NUT PRIOR TO SHIPPING
2. BASE PLATES ARE 50 KSI, U.N.O.
3. ANCHOR ROD HOLES AND WASHERS ARE PER AISC 360 (U.N.O.):
   T14-2 RECOMMENDED MAXIMUM SIZES FOR ANCHOR-ROD HOLES IN BASE PLATES.
4. NON-SHRINK GROUT (MIN. 5,000 PSI) PER GENERAL NOTES.

3 BASEPLATE #32
1" = 1'-0"

ANCHOR RODS							
Type Mark	Type	DIA	E	P	S	L	Grout
AR-1	AR-1 0.750Ø (F1554 Gr.36)	0.750"	14.000"	6.000"	2.000"	1' - 10"	1.50"
AR-2	AR-2 1.00Ø (F1554 Gr.36)	1.000"	18.000"	6.000"	2.000"	2' - 2"	2.00"
AR-3	AR-3 1.125Ø (F1554 Gr.55-S1)	1.125"	18.000"	7.000"	2.000"	2' - 3"	1.50"
AR-4	AR-4 1.25Ø (F1554 Gr.36)	1.250"	24.000"	8.000"	2.000"	2' - 10"	2.00"
AR-5	AR-5 1.00Ø (F1554 Gr.36)	1.000"	24.000"	6.000"	2.000"	2' - 8"	2.00"
AR-6	AR-6 1.250Ø (F1554 Gr.55-S1)	1.250"	22.000"	6.000"	2.000"	2' - 6"	2.00"
AR-8	AR-8 0.750Ø (F1554 Gr.36)	0.750"	9.000"	6.000"	2.000"	1' - 5"	2.00"
AR-9	AR-9 1.00Ø (F1554 Gr.36) E-15"	1.000"	15.000"	6.000"	2.000"	1' - 11"	1.50"
AR-HDG-1	HDG-AR-1 0.750Ø (F1554 Gr.36)	0.750"	12.000"	6.000"	2.000"	1' - 8"	1.50"
AR-HDG-2	HDG-AR-2 0.750Ø (F1554 Gr.36)	0.750"	18.000"	6.000"	2.000"	2' - 2"	1.50"
AR-HDG-3	HDG-AR-3 1.1250Ø (F1554 Gr.55-S1)	1.125"	30.000"	7.000"	2.000"	3' - 3"	1.50"



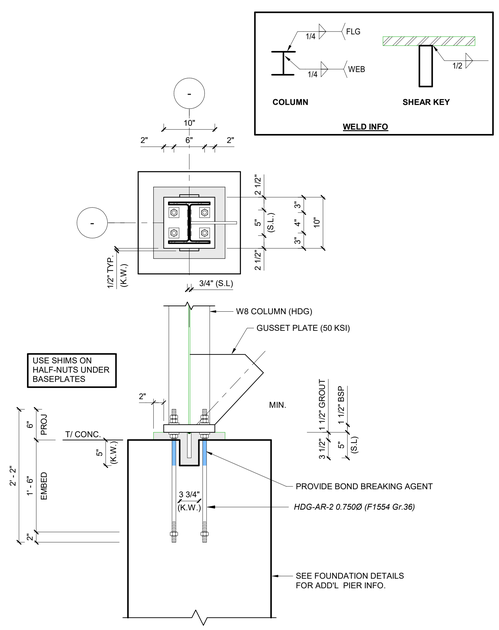
DETAILING REFERENCE: #34

MECHANICAL YARD BASEPLATE NOTES:

ALL STEEL, BASEPLATES AND ANCHOR RODS ARE EXPOSED TO WEATHER AND SHALL BE HOT DIP GALVANIZED (HDG) PER ASTM A153 CLASS C.

1. TACK WELD OR DAMAGE ANCHOR ROD'S EMBEDED NUT PRIOR TO SHIPPING
2. BASE PLATES ARE 50 KSI, U.N.O.
3. ANCHOR ROD HOLES AND WASHERS ARE PER AISC 360 (U.N.O.):
   T14-2 RECOMMENDED MAXIMUM SIZES FOR ANCHOR-ROD HOLES IN BASE PLATES.
4. NON-SHRINK GROUT (MIN. 5,000 PSI) PER GENERAL NOTES.

4 BASEPLATE #34
1" = 1'-0"



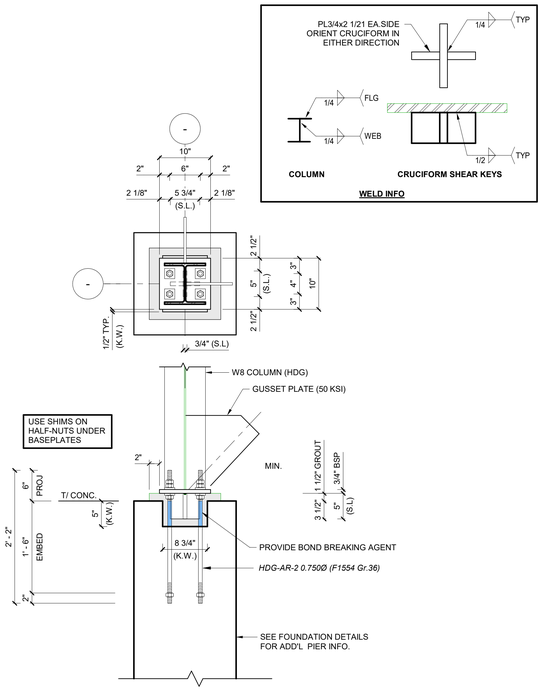
DETAILING REFERENCE: #35

MECHANICAL YARD BASEPLATE (AT COLUMNS) NOTES:

ALL STEEL, BASEPLATES AND ANCHOR RODS ARE EXPOSED TO WEATHER AND SHALL BE HOT DIP GALVANIZED (HDG) PER ASTM A153 CLASS C.

1. KEYWAY (K.W.) SHALL HAVE MIN. 1 1/2" GROUT AROUND SHEAR KEY
   A. EXTEND AT LEAST 1" BEYOND EDGE OF BASE PLATE
   B. SEE TYPICAL KEYWAY (K.W.) DETAIL
   C. INDEPENDENT INSPECTION REQUIRED TO VERIFY SHEAR KEY WAY IS CLEAN OF DEBRIS PRIOR TO PLACING FLOAWABLE NON-SHRINK GROUT
2. TACK WELD OR DAMAGE ANCHOR ROD'S EMBEDED NUT PRIOR TO SHIPPING
3. BASE PLATES ARE 50 KSI, U.N.O.
4. ANCHOR ROD HOLES AND WASHERS ARE PER AISC 360 (U.N.O.):
   T14-2 RECOMMENDED MAXIMUM SIZES FOR ANCHOR-ROD HOLES IN BASE PLATES.
5. NON-SHRINK FLOWABLE GROUT (MIN. 6,000 PSI) SHALL EXTEND AT LEAST 2" BEYOND EDGE OF BASE PLATE.
6. REFER TO ANCHOR ROD SCHEDULE FOR ADDITIONAL INFORMATION.

5 BASEPLATE #35
1" = 1'-0"



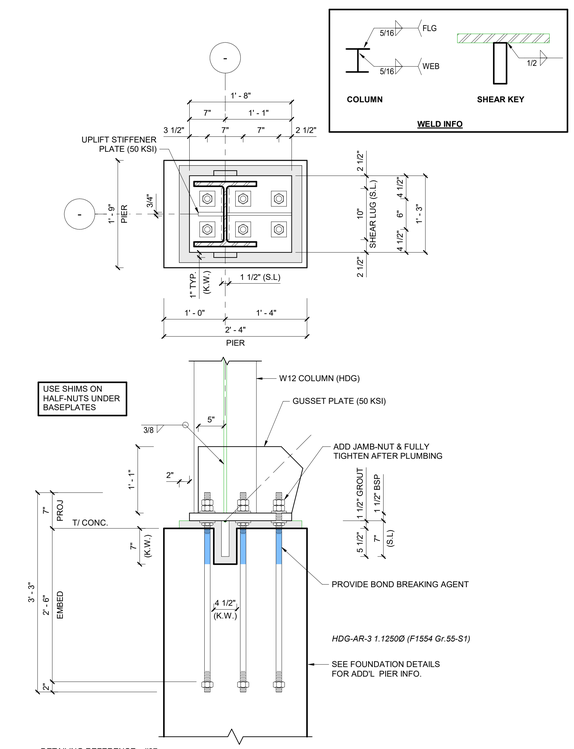
DETAILING REFERENCE: #35A

MECHANICAL YARD BASEPLATE (AT COLUMNS) NOTES:

ALL STEEL, BASEPLATES AND ANCHOR RODS ARE EXPOSED TO WEATHER AND SHALL BE HOT DIP GALVANIZED (HDG) PER ASTM A153 CLASS C.

1. KEYWAY (K.W.) SHALL HAVE MIN. 1 1/2" GROUT AROUND SHEAR KEY
   A. EXTEND AT LEAST 1" BEYOND EDGE OF BASE PLATE
   B. SEE TYPICAL KEYWAY (K.W.) DETAIL
   C. INDEPENDENT INSPECTION REQUIRED TO VERIFY SHEAR KEY WAY IS CLEAN OF DEBRIS PRIOR TO PLACING FLOAWABLE NON-SHRINK GROUT
2. TACK WELD OR DAMAGE ANCHOR ROD'S EMBEDED NUT PRIOR TO SHIPPING
3. BASE PLATES ARE 50 KSI, U.N.O.
4. ANCHOR ROD HOLES AND WASHERS ARE PER AISC 360 (U.N.O.):
   T14-2 RECOMMENDED MAXIMUM SIZES FOR ANCHOR-ROD HOLES IN BASE PLATES.
5. NON-SHRINK FLOWABLE GROUT (MIN. 6,000 PSI) SHALL EXTEND AT LEAST 2" BEYOND EDGE OF BASE PLATE.
6. REFER TO ANCHOR ROD SCHEDULE FOR ADDITIONAL INFORMATION.

7 BASEPLATE #35A
1" = 1'-0"



DETAILING REFERENCE: #37

MECHANICAL YARD BASEPLATE (AT COLUMNS) NOTES:

ALL STEEL, BASEPLATES AND ANCHOR RODS ARE EXPOSED TO WEATHER AND SHALL BE HOT DIP GALVANIZED (HDG) PER ASTM A153 CLASS C.

1. KEYWAY (K.W.) SHALL HAVE MIN. 1 1/2" GROUT AROUND SHEAR KEY
   A. EXTEND AT LEAST 1" BEYOND EDGE OF BASE PLATE
   B. SEE TYPICAL KEYWAY (K.W.) DETAIL
   C. INDEPENDENT INSPECTION REQUIRED TO VERIFY SHEAR KEY WAY IS CLEAN OF DEBRIS PRIOR TO PLACING FLOAWABLE NON-SHRINK GROUT
2. TACK WELD OR DAMAGE ANCHOR ROD'S EMBEDED NUT PRIOR TO SHIPPING
3. BASE PLATES ARE 50 KSI, U.N.O.
4. ANCHOR ROD HOLES AND WASHERS ARE PER AISC 360 (U.N.O.):
   T14-2 RECOMMENDED MAXIMUM SIZES FOR ANCHOR-ROD HOLES IN BASE PLATES.
5. NON-SHRINK FLOWABLE GROUT (MIN. 6,000 PSI) SHALL EXTEND AT LEAST 2" BEYOND EDGE OF BASE PLATE.
6. REFER TO ANCHOR ROD SCHEDULE FOR ADDITIONAL INFORMATION.

6 BASEPLATE #37
1" = 1'-0"

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MECHANICAL AREA - ANCHOR RODS