Report of Subsurface Exploration
and Geotechnical Evaluation
Cherokee Nation Housing, Hancock-Muskogee
Muskogee, Oklahoma

Wallace Project Number: 2240526
Building & Earth Project No.: TU230050

Prepared For:
Wallace Design Collective

April 24, 2023

BUILDING & EARTH
Geotechnical, Environmental, and Materials Engineers

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April 24, 2023

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Attention: Mr. Lance Woolsey, P.E., RA

Subject: Report of Subsurface Exploration and Geotechnical Evaluation
Cherokee Nation Housing, Hancock-Muskogee
Muskogee, Oklahoma
Wallace Project No.: 2240526
Building & Earth Project No: TU230050

Dear Mr. Woolsey:

Building & Earth Sciences, Inc. has completed the authorized subsurface exploration and geotechnical engineering evaluation for the above referenced project in Muskogee, Oklahoma.

The purpose of this exploration and evaluation was to determine general subsurface conditions at the site and to address applicable geotechnical aspects of the proposed construction and site development. The recommendations in this report are based on a physical reconnaissance of the site and observation and classification of samples obtained from eighteen (18) test borings conducted at the site. Confirmation of the anticipated subsurface conditions during construction is an essential part of geotechnical services.

We appreciate the opportunity to provide consultation services for the proposed project. If you have any questions regarding the information in this report or need any additional information, please call us.

Respectfully Submitted,
BUILDING & EARTH SCIENCES, INC.
Certificate of Authorization #3975, Expires 6/30/2024

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Table of Contents

1.0 PROJECT & SITE DESCRIPTION

The project site is located on the north side of East Hancock Street, approximately 0.34 miles east from the intersection with South York Street in Muskogee, Oklahoma. General information relative to the proposed site and the proposed development is listed in Table 1 below. Google Earth satellite imagery of the site and photographs depicting the current site conditions are presented on the following pages.

Development Item	Detail	Description
General Site	Size (Ac.)	Approx. 12.1
	Existing Development	The site is currently an undeveloped tract of land that appears to be a farmland
	Vegetation	Most of the site was covered with grass, and trees located sporadically along the property boundaries and scattered across the site
	Slopes	The planned development area slopes down gradually from the northwest corner to southeast corner, with a grade differential of roughly 8 feet
	Drainage	Standing water was noted across most of the site along with very soft to soft soils extending to depths of about 2 to 2.5 feet. A pond was noted at the northeast corner of the project site. The project site is very poorly drained.
	Proposed Cuts & Fills	See table 5 in this report
Proposed Buildings	No. of structures	Thirty-six (36) residential housing lots Two (2) Detention/retention ponds
	Square Ft.	Lots ranging between 5,800 and 10,600 sq ft Detention Ponds – 4,900 and 13,100 sq ft
	Stories	All units are single-story (assumed)
	Construction	Wood framed residential dwellings (assumed)
	Column Loads	<30 kips (assumed)
	Wall Loads	1 to 2 kips per linear foot (assumed)
	Preferred Foundation	Post-tensioned slab foundation (assumed)
	Preferred Slab	Post-tension reinforced slab-on-grade (assumed)
Pavements	Traffic	Not provided, assumed to be minor residential street

Table 1: Project and Site Description

References:

- Google Earth Aerial with Project Extents, provided by Wallace Design Collective, undated
- Bore Location Map, provided by Wallace Design Collective, undated
- Preliminary Construction Documents, prepared by Wallace Design Collective, dated 3/24/2023

Table 1 Notes:

- If actual loading conditions exceed our assumed loads, Building & Earth should be allowed to review the proposed structural design and its effects on our recommendations for foundation design.
- If changes are made to the provided preliminary grading plan, Building & Earth should be allowed to review the updated plan and its effects on our recommendations.

At the time of our subsurface exploration and site reconnaissance, most of the project site was covered with grass and topsoil. Standing water was noted in several areas throughout the site and a pond was noted near boring B-03, located at the northeast corner of the project site. Historic aerial imagery of the site indicated the presence of a residential dwelling in the south-central area of the project site, which was demolished sometime between 1995 and 2003.

Figure 1: Google Earth aerial image, dated October 2021

Figure 2: Google Earth aerial image, dated March 1995

Figure 3: Southeast corner of the project site, looking northwest

Figure 4: Northwest corner of the project site, looking southeast

Figure 5: Near boring B-03, looking northeast at the pond

2.0 SCOPE OF SERVICES

The authorized subsurface exploration was performed on March 27 and 28, 2023, in conformance with our proposal TU25072, dated March 8, 2023. Notice to proceed was provided by signing and returning our proposal on the same day.

The purpose of the geotechnical exploration was to determine general subsurface conditions at specific boring locations and to gather data on which to base a geotechnical evaluation with respect to the proposed construction. The subsurface exploration for this project consisted of eighteen (18) test borings. The site was drilled using a track mounted Diedrich D-50 drill rig equipped with solid flight augers and an automatic hammer for performing Standard Penetration Tests (SPT) to help evaluate the relative soil strength.

Boring locations were determined in the field by a representative of our staff using a handheld GPS device. As such, the boring locations shown on the Boring Location Plan attached to this report should be considered approximate.

The soil/rock samples recovered during our site investigation were visually classified and specific samples were selected by the project engineer for laboratory analysis. The laboratory analysis consisted of:

Test	ASTM	No. of Tests
Natural Moisture Content	D2216	78
Atterberg Limits	D4318	14
Material Finer Than No. 200 Sieve by Washing	D1140	5
Loss on Ignition (LOI)	D2974	2

Table 2: Scope of Laboratory Tests

The results of the laboratory analysis are presented on the enclosed Boring Logs and in tabular form in the Appendix of this report. Descriptions of the laboratory tests that were performed are also included in the Appendix.

The information gathered from the exploration was evaluated to determine a suitable foundation type for the proposed structures. The information was also evaluated to help determine if any special subgrade preparation procedures will be required during the earthwork phase of the project.

The results of the work are presented within this report that addresses:

- General site geology.
- Summary of existing surface conditions.
- A description of the subsurface conditions encountered at the boring locations.
- A description of the groundwater conditions observed in the boreholes during drilling. Long-term monitoring was not included in our scope of work.
- Presentation of laboratory test results.
- Site preparation considerations including material types to be expected at the site, treatment of any encountered unsuitable soils, excavation considerations, and surface drainage.
- Presentation of expected total and differential settlements.
- Recommendations to be used for design of slabs-on-grade, including modulus of subgrade reaction. Post-tension slab design recommendations are included following the latest PTI slab design methodology.
- Compaction requirements and recommended criteria to establish suitable material for structural backfill.
- Recommended typical minimum flexible and rigid pavement sections for residential streets based on assumed traffic loading conditions.

3.0 GEOTECHNICAL SITE CHARACTERIZATION

The following discussion is intended to create a general understanding of the site from a geotechnical engineering perspective. It is not intended to be a discussion of every potential geotechnical issue that may arise, nor to provide every possible interpretation of the conditions identified. The following conditions and subsequent recommendations assume that significant changes in subsurface conditions do not occur between boreholes. However, anomalous conditions can occur due to variations in existing fill that may be present at the site, or the geologic conditions at the site, and it will be necessary to evaluate the assumed conditions during site grading and foundation installation.

3.1 GENERAL SITE GEOLOGY

According to the Oklahoma State Geologic Map published by the United States Geological Survey (USGS), the subject site is underlain by the Savanna and McAlester Formations noted to consist of middle Pennsylvanian aged shale and minor sandstone. The conditions encountered at the project site generally correlate with the published geologic references.

3.2 EXISTING SURFACE CONDITIONS

At the time of our subsurface exploration, most of the project site was covered with grass and topsoil that had a thickness of about 3 to 6 inches. The topsoil conditions reported apply only to the specific boring locations.

It should be noted that topsoil thicknesses likely vary at unexplored locations of the project site, especially in heavily wooded areas. No testing has been performed to verify that soils meet the requirements of "topsoil". For this report, topsoil is defined as the soil horizon which contains the root mat of the noted vegetation.

During our site investigation, ponding of water across the surface and very soft to soft, wet near surface silty clays and lean clays were noted throughout the site.

3.3 SUBSURFACE CONDITIONS

A generalized stratification summary has been prepared using data from the test borings and is presented in the table below. The stratification depicts the general soil and rock conditions and stratum types encountered during our field investigation.

Stratum No.	Typical Thickness	Description	Consistency/Rock Hardness	Lab Test Data [4]
1 [1]	2′	**Possible Fill:** Silt (ML), with roots Dark brown in color	Soft	Atterberg Limits: LL = 47 PI = 15 Moisture Content: 46%
2	1.1 to 2.2′	**Near Surface, Low Consistency Soils:** Lean Clays (CL) and some Silty Clays (CL-ML) with decomposed organics Various shades and combinations of brown, red, yellow, and gray	Generally, very soft to soft	Atterberg Limits: LL = 25 to 28 PI = 4 to 11 Moisture Contents: 19 to 38% (generally wet) Organic Content: 2.5 and 3.4%
3 [2]	5.5 to 11.5′	**Residuum:** Lean to Fat Clays (CL-CH), Fat Clays (CH), Lean Clays (CL), and Sandy Lean Clays (CL) with roots, sandstone fragments, ferrous nodules, and staining Various shades and combinations of brown, red, yellow, and gray	Generally stiff to very stiff Medium stiff clays in borings B-09 and P-05 at depths of about 2 and 1.5 feet, respectively	Atterberg Limits: LL = 37 to 58 PI = 21 to 41 Moisture Contents: 15 to 27% Passing #200 Sieve: 86 to 92%
4 [3]	Termination Layer	**Weathered Rock:** Weathered Shale with ferrous staining and sandstone lenses Various shades and combinations of brown, yellow, and gray	Soft rock formation	Atterberg Limits: LL = 42 PI = 19 *Moisture contents:* *7 to 15%*

Table 3: Stratification Summary

Notes:

(1) Possible fill materials were only encountered in boring P-06.

(2) All pavement borings were terminated within this stratum and therefore not used to determine thickness.

(3) Encountered only in borings B-01 through B-12. All building borings were terminated within this stratum at depths of about 14.5 to 15 feet below existing grades.

(4) For Atterberg limits, LL = Liquid Limit, and PI = Plasticity Index

Subsurface profiles were prepared based on the data obtained at the specific boring locations and are presented in the Appendix. For specific details on the information obtained from individual borings, refer to the Boring Logs included in the Appendix. The ground surface elevations at the boring locations indicated in this report were estimated from the contour lines shown on the provided grading plan and should be considered approximate.

3.3.1 GROUNDWATER

At the time of drilling, groundwater was encountered in boring B-09 at a depth of about 5 feet below existing grades. Free water was measured in most of the building borings at depths of about 1 to 12 feet, prior to backfilling the boreholes. The following table shows the water levels encountered in the borings during drilling and free water levels measured prior to backfilling on March 28, 2023.

Boring No.	Water Depth During Drilling (ft)	Water Depth Prior to Backfilling on 3/28/23 (ft)
B-01 [1]	Dry	5.5
B-02 [1]	Dry	8
B-03 [1]	Dry	5.5
B-04 [1]	Dry	6
B-05 [1]	Dry	9
B-06 [1]	Dry	9
B-07 [2]	Dry	12
B-08 [2]	Dry	1.5
B-09 [1]	5	4.5
B-10 [1]	Dry	1

Table 4: Groundwater levels during drilling

Notes:

1. Borings drilled on 3/27/23 and backfilled on 3/28/23
2. Borings drilled on 3/28/23 and backfilled on same day

Groundwater was not encountered in the other borings during drilling, and each was dry upon completion and prior to backfilling of the borehole.

Fluctuations in the water level can occur due to seasonal rainfall. Water levels as observed during drilling are accurate for only the time and date that the boring was drilled. Short term groundwater level readings may not accurately reflect the actual groundwater levels at the borings.

4.0 SITE DEVELOPMENT CONSIDERATIONS

Preliminary construction documents, prepared by Wallace Design Collective, dated 3/24/2023 were provided for our review and to aid with development of geotechnical recommendations. The following table summarizes the existing ground surface and design grade elevations at each building and pavement boring location, along with estimated amounts of cut or fill.

Boring No.	Existing Grade Elevation (ft.)	Planned Finished Floor and/or Finished Grade Elevations (ft.)	Anticipated Cut (-) or Fill (+) (ft.)
B-01	554.5	556	+1.5
B-02	553	554.25	+1.25
B-03	550	553	+3.0
B-04	552	555.5	+3.5
B-05	553	554.4	+1.4
B-06	551.5	553.5	+2.0
B-07	549	552	+3.0
B-08	552	552	0
B-09	550	549	-1
B-10	551	552.75	+1.75
B-11	551	551.5	+0.5
B-12	547.5	542	-5.5
P-01	554	554	0
P-02	550.5	551	+0.5
P-03	552	552	0
P-04	550	549.5	-0.5
P-05	549.5	550	+0.5
P-06	548	547	-1

Table 5: Anticipated cut or fill depths at each boring location

Based on our evaluation of the subsurface conditions, and the planned residential housing units, it appears that construction with a post-tensioned slab foundation can be used for the planned development. The site development recommendations outlined below are intended for development of the site to support construction with a post-tensioned slab foundation.

***If a different type of foundation system is preferred, Building & Earth should be allowed to review the site development recommendations to verify that they are appropriate for the preferred foundation system*.**

The primary geotechnical considerations for this project are:

- The presence of a residential structure within the south-central area of the project site prior to 2003.
- A pond is located at the northeast corner of the property. The pond will be backfilled as part of mass grading for planned development.
- Possible fill materials were encountered in boring P-06 to a depth of about 2.5 feet.
- The near surface, low plasticity lean clays and silty clays extending to depths of about 2 to 2.5 feet exhibited very soft to soft consistencies across the entire proposed development. As evidenced from the borings, the near-surface soils lose strength and stability when they become saturated.
- Higher plasticity residual clay soils with a moderate to high shrink-swell potential were encountered below the near-surface, low plasticity clay soils.
- Perched water develops at the contact of the near surface, low plasticity clay soils and the underlying higher plasticity, stiff to very stiff clay soils.
- A weathered shale unit was encountered in all building borings at depths of approximately 8 to 13.5 feet below current grades.
- At the time of drilling, groundwater was encountered at a depth of about 5 feet below existing grades in boring B-09. Free water was measured in most of the building borings at depths of about 1 to 12 feet, prior to backfilling the boreholes.

Recommendations addressing the site conditions are presented in the following sections.

4.1 INITIAL SITE PREPARATION

Because of past use of the site, buried structures could be encountered such as foundations, utility lines, septic tanks, etc. If encountered, they should be removed and backfilled in accordance with requirements outlined in the *Structural Fill* section of this report.

All trees, vegetation, roots, topsoil, and any other deleterious materials, should be removed from the proposed construction areas. Approximately 3 to 6 inches of topsoil was observed in the borings; however, topsoil could extend to greater depths in unexplored areas of the site.

The geotechnical engineer or their designated representative should observe demolition, grubbing, and stripping operations to evaluate that all unsuitable materials are removed from locations for proposed construction.

Materials disturbed during clearing operations should be stabilized in place or, if necessary, undercut to undisturbed materials and backfilled with properly compacted, approved structural fill.

During site preparation activities, the contractor should identify borrow source materials that will be used as structural fill and provide samples to the testing laboratory so that conformance to the structural fill requirements outlined below and appropriate moisture-density relationship curves can be determined.

4.2 EXISTING POND – BACKFILL CONSIDERATIONS

A pond is located at the northeast corner of the property. We understand that residential lots will be developed over the existing pond. The primary geotechnical considerations for when constructing a building over the pond include:

- Conventional dewatering techniques such as sump pumping should be used to remove the water from the existing pond.
- Very moist and soft soils (muck) are anticipated to be present at the bottom of the pond area. The extent and depth of any muck was not assessed at the time of preparing this report. We recommend that muck encountered in the pond and surrounding area be removed full-depth to firm and stable material.
- Following dewatering and removal of muck, we recommend that a geotechnical engineer assess the condition of exposed soils. When exposed soils are firm and stable and free of groundwater seepage, proceed with fill placement starting at the bottom of the dewatered and demucked pond. When groundwater seepage is apparent, then alternate recommendations will need to be developed depending on the conditions encountered in the field during construction.
- We recommend that the slopes within the pond steeper than 5 Horizontal to 1 Vertical, 5(H): 1(V), be benched prior to fill placement. Benching of the slopes provides interlocking between the new fill and onsite soils and facilitates compaction of the fill. Benches should be cut as the fill placement progresses and should have a maximum bench height of 2 to 3 feet.
- Any fill placed within the pond area should be compacted to at least 98% of the standard Proctor maximum dry density and moisture conditioned within a range of -2% to +2% of the optimum moisture content.

4.3 PERMANENT GROUNDWATER INTERCEPTION

Considering the presence of wet and very soft to soft clays across the proposed development, and the presence of shallow perched water and the groundwater seepage in the building borings, we recommend the permanent subsurface drainage measures be implemented as part of the planned construction to intercept and divert perched water and groundwater away from proposed pavements and building areas.

According to the Utility Mains Plan C600, new sanitary sewer lines are planned along the west, north, and east property lines. Building & Earth did not review profiles of the planned sewer line to assess its depth along the alignment. Depending on the flow line of the sewer line, the trench backfill could serve as a French drain to aid with interception perched water that may originate offsite at higher elevation and divert it away from the planned development. When relying on the sanitary sewer trench backfill to serve as a French drain, the backfill should comprise of free draining stone that is wrapped into a filter fabric. The trench should be sloped such as to promote gravity flow toward a suitable outfall to be specified by the civil design engineer. Building & Earth can provide further detailed recommendations to assist with the design of a permanent subsurface drainage system upon further request.

It is recommended that the sewer line be installed prior to the start of initial site preparation and mass grading to help control water seepage. If utility lines are installed after start of mass grading, the contractor should anticipate the need for temporary ditches to control perched water seepage in excavations.

4.4 NEAR-SURFACE, LOW CONSISTENCY CLAY SOILS

Near-surface low plasticity, very soft to soft and wet lean clays and silty clays were encountered throughout the subject site. These soils extended to depths of about 2 to 2.5 feet below original grade in the test borings.

The near-surface, low consistency soils pose a concern for low bearing capacity, high risk for foundation settlement, and they do not provide a stable platform for start of fill placement and support of pavements.

Following initial site preparation and prior to any fill placement and site development, we recommend the soft, wet and unstable soils be undercut full depth across the planned development to expose a stable, suitable subgrade.

For construction budget estimate purposes, an average undercut depth of 2.5 feet below existing grades is to be anticipated across the proposed development. Actual undercut depths will be dependent on the soil conditions during construction, and they could extend to depths greater than 2.5 feet within parts of the site.

The placement procedure, compaction, and composition of the structural fill should meet the requirements of the *Structural Fill* section of this report. The undercutting should be conducted under the observation of the geotechnical engineer or their designated representative. Once the undercut is complete, the areas planned for construction should be proofrolled to identify any remaining soft soils requiring further removal.

4.5 SUBGRADE EVALUATION AND PREPARATION

Following full depth removal of soft, wet, and unstable soils and prior to fill placement, the exposed subgrade should be scarified to a depth of at least 12 inches, moisture conditioned to within range of 0 to 3 percent above the optimum moisture content, and recompacted to at least 95 percent of the standard Proctor maximum dry density (ASTM D 698).

We recommend the project geotechnical engineer, or their qualified representative evaluate the subgrade after the site is prepared. All pavement and building lot areas should be carefully proofrolled with a heavy (20- to 25-ton), tandem-axle dump truck at the following times.

- After an area has been stripped and undercut as recommended, prior to the placement of any fill.
- After grading an area to the finished subgrade elevation.
- After areas have been exposed to any precipitation, and/or have been exposed for more than 48 hours.

Some instability may exist during construction, depending on climatic and other factors immediately preceding and during construction. If any soft or otherwise unsuitable soils are identified during the proofrolling process, they should be undercut or moisture conditioned and recompacted prior to fill placement, and pavement or floor slab construction. Soft and unstable materials identified during the construction should be removed and replaced in accordance with the *Structural Fill* section of this report.

4.6 STRUCTURAL FILL

Requirements for structural fill on this project are as follows:

Soil Type	USCS Classification	Property Requirements	Placement Location
Imported Lean Clay, Clayey Sand, or Shale	CL, SC	LL<40, PI<18, γ_d>100 pcf, P200>30%, Maximum 3" particle size in any dimension	**Low Plasticity Structural Fill** to be used for construction of building pad and below pavements
Onsite Near-Surface Soils Lean Clays and Silty Clays	CL, CL-ML	Same as above	***Likely Suitable*** for placement as low plasticity structural fill (see notes 1 and 2)
Onsite Residual Lean Clays, Lean to Fat Clays, and Fat Clays	CL (PI≥18), CL-CH, CH	Not Applicable	***Likely suitable*** for placement as structural fill in building lots provided that all buildings will be supported on a post-tensioned slab foundation ***Not suitable*** for placement in pavement areas due to higher plasticity characteristics

Table 6: Structural Fill Requirements

Notes:

1. All structural fill should be free of vegetation, topsoil, and any other deleterious materials. The near-surface site soils are prone to losing stability with slight increases in soil moisture levels. The contractor should use care during and after fill placement to prevent over wetting of the soils and to limit repeat construction traffic, as this will increase the risk of these soils losing stability.
2. The near-surface soils were wet at the time of drilling. When considering the use of onsite soils, the contractor needs to be prepared for double handing and drying of the soils using wind rowing techniques. Drying of soils could be expedited when mixing with chemical additives such Fly Ash or Cement Kiln Dust. Further recommendations can be provided upon request during construction based on the conditions encountered at that time.
3. LL indicates the soil Liquid Limit; PI indicates the soil Plasticity Index; γ_d indicates the maximum dry density as defined by the density standard outlined in the table below.
4. Laboratory testing of the soils proposed for fill should be performed to verify their conformance with the above recommendations. Any fill to be placed at the site should be reviewed by the geotechnical engineer.

Placement requirements for structural fill are as follows:

Specification	Requirement
Lift Thickness	Maximum loose lift thickness of 8 to 12 inches, depending on type of compaction equipment used.
Density	At least 95% of the standard Proctor (ASTM D698) maximum dry density
Moisture	**Imported Low Plasticity Structural Fill and Onsite Near-Surface Silty Clays and Lean Clays:** ±2% of the optimum moisture content as determined by ASTM D698 **Onsite Lean to Fat Clays and Fat Clays:** 0 to 3% above the optimum moisture content as determined by ASTM D698
Density Testing Frequency	**Building and foundation areas**: One test per 2,500 square feet (SF) per lift with a minimum of three tests performed per lift **Pavement areas and utility trenches**: One test per 150 linear feet per lift with a minimum of three tests performed per lift The testing frequency can be increased or decreased by the Geotechnical Engineer of Record in the field based on uniformity of material being placed and compactive effort used.

Table 7: Structural Fill Placement Requirements

4.7 EXCAVATION CONSIDERATIONS

All excavations performed at the site should follow OSHA guidelines for temporary excavations. Excavated soils should be stockpiled according to OSHA regulations to limit the potential cave-in of soils.

4.7.1 DIFFICULT EXCAVATION

We anticipate the residuum can be excavated using a backhoe in good working condition. A weathered shale unit was encountered in all building borings at depths of approximately 8 to 13.5 feet below current grades. The weathered rock likely can be excavated with a large track hoe equipped with rock teeth.

The depth that weathered rock and competent rock can be excavated is a function of the material, the equipment used, the skill of the operator, the desired rate of removal and other factors. The contractor should review the boring logs and should use his own method to evaluate excavation difficulty.

4.7.2 PERCHED WATER OR GROUNDWATER

At the time of drilling, groundwater was encountered at a depth of about 5 feet below existing grades in boring B-09. Free water was measured in most of the building borings at depths of about 1 to 12 feet, prior to backfilling the boreholes.

It should be noted that fluctuations in the water level could occur due to seasonal variations in rainfall. The contractor should be prepared to remove groundwater seepage from excavations if encountered during construction. Excavations extending below groundwater levels will require dewatering systems (such as sump pumps or trench drains). The contractor should evaluate the most economical and practical dewatering method based on the conditions encountered at the time of construction.

4.8 UTILITY TRENCH BACKFILL

All utility trenches should be backfilled and compacted in the manner specified above for structural fill. It may be necessary to reduce the lift thickness to 4 to 6 inches to achieve compaction using hand-operated equipment.

At the perimeter wall crossings, we recommend that clay soils or a flowable fill be used to backfill utility trenches. The clay or flowable fill will act as a relatively impermeable plug reducing the risk of water migration from the outside into the interior of the building. The plug should be at least 36 inches wide and should extend below the perimeter walls to provide for a proper seal.

4.9 LANDSCAPING AND DRAINAGE CONSIDERATION

The potential for soil moisture fluctuations within structure areas and pavement subgrades should be reduced to lessen the potential of subgrade movement. Site grading should include positive drainage away from buildings and pavements. Excessive irrigation of landscaping poses a risk of saturating and softening soils below shallow footings and pavements, which could result in settlement of footings and premature failure of pavements. In addition, ponding of water or irrigation of landscaped areas can result in swelling of higher plasticity clay soils and subsequent heave of grade supported slabs and premature failure of pavements.

4.10 WET WEATHER CONSTRUCTION

Excessive movement of construction equipment across the site during wet weather may result in ruts, which will collect rainwater, prolonging the time required to dry the subgrade soils.

During rainy periods, additional effort will be required to properly prepare the site and establish/maintain an acceptable subgrade. The difficulty will increase in areas where clay or silty soils are exposed at the subgrade elevation.

Likewise, rainwater may become perched on the higher plasticity clay soils encountered across the site, which could require additional dewatering efforts not needed during dry conditions.

A perched-water condition occurs when water seeping downward is slowed by a low permeability soil layer, such as the underlying clays and rock units. The perched-water level can be any number of feet above the true groundwater level. Due to the prevalence of clay soils and shallow rock encountered across the project site, the successful contractor should expect to encounter perched water during wet weather construction.

Grading contractors typically postpone grading operations during wet weather to wait for conditions that are more favorable. Contractors can typically disk or aerate the upper soils to promote drying during intermittent periods of favorable weather. When deadlines restrict postponement of grading operations, additional measures such as undercutting and replacing saturated soils or stabilization can be utilized to facilitate placement of additional fill material.

5.0 FOUNDATION RECOMMENDATIONS

Specific structural loading conditions were not known at the time of this report. For this report, we have assumed the individual column loads will be less than 30 kips and wall loads will be between 1 and 2 kips per linear foot. ***When final structural loading information is available, our office should be contacted, such that our recommendations can be reviewed and revised if needed.***

5.1 POST-TENSIONED SLAB FOUNDATION

The planned houses can be supported on a post tensioned slab foundation with turndown edges or perimeter footings extending at least 2 feet below the finished exterior grade.

Perimeter footings, edge turndowns and stiffening beams of post-tensioned slab foundations are anticipated to be founded in undisturbed stiff residual clay soils, new structural fill, or a combination of the materials. Footings bearing in the anticipated materials can be dimensioned using a maximum net allowable bearing pressure of 2,500 pounds per square foot (psf).

Post-tensioned foundation systems may be designed using the procedures detailed in "Design of Post-Tensioned Slabs-on-Ground", Post Tensioning Institute publication PTI DC10.1-08 (3rd edition with 2008 Supplement), using the design parameter values presented in the following table.

Design Parameter	Parameter Value
Thornthwaite Moisture Index (Muskogee, OK)	+23
Moisture Active Zone Depth	8 feet
Equilibrium Soil Suction	3.39 pF
Wettest Soil Suction	3.0 pF
Driest Soil Suction	4.5 pF
Edge Moisture Variation Distance (e_m), Center Lift	9.0 feet
Edge Moisture Variation Distance (e_m), Edge Lift	5.5 feet
Differential Soil Movement (y_m), Center Lift	-0.5 inches
Differential Soil Movement (y_m), Edge Lift	1.3 inches

Table 8: Post-tensioned Slab-on-Ground Design Parameter Values

The estimated y_m and e_m values provided above are based on soil moisture conditions that are controlled by climate alone. Differential swell can be influenced by other non-climatic conditions that are unpredictable, such as pre-construction and post-construction vegetation cover, drainage conditions, local water sources (downspouts, irrigation, plumbing leaks, etc.) The PT slab designer should provide additional comments relative to the influence of non-climatic moisture conditions on PT slab performance.

5.2 SHEAR RESISTANCE

Passive earth pressures of materials adjacent to the footings as well as bearing material friction at the base may be used to resist shear.

The following table presents recommended friction coefficient and passive earth pressure values for new structural fill or stiff residual clays. The structural engineer should use a factor of safety of at least 1.5 when sizing the foundations to resist shear loads using the below ultimate soil parameter values.

Material	Friction Coefficient	Equivalent Fluid Unit Weight for Passive Condition Lateral Earth Pressures (pcf)
New Structural Fill or Stiff Residual Clays	0.30	250

Table 9: Soil Parameter Values Resisting Shear

5.3 GENERAL CONSIDERATIONS

The following items should be considered during the preparation of construction documents and foundation installation:

- The geotechnical engineer of record should observe the exposed foundation bearing surfaces prior to concrete placement to verify that the conditions anticipated during the subsurface exploration are encountered.
- All bearing surfaces must be free of soft or loose soil and debris prior to placing concrete.
- The bottom surface of all footings should be level.
- Water should not be allowed to pond in foundation excavations prior to concrete placement or above the concrete after the foundation is completed.
- Concrete should be placed the same day the excavations are completed and bearing materials verified by the engineer. If the excavations are left open for an extended period, or if the bearing surfaces are disturbed after the initial observation, then the bearing surfaces should be re-evaluated prior to concrete placement.
- Wherever possible, the foundation concrete should be placed “neat”, using the sides of the excavations as forms. Where this is not possible, the excavations created by forming the foundations must be backfilled with suitable structural fill and properly compacted.
- Grades around the building pad should be sloped to drain away from the building foundations.
- Roof drains should be routed away from the foundation soils.

6.0 FLOOR SLABS

Site development recommendations presented in this report should be followed to provide for subgrade conditions suitable for support of grade supported slabs.

We recommend floor slabs for the proposed structures be supported on a minimum four-inch layer of ½-inch up to 1½-inch, free-draining, gap-graded gravel, such as No. 57 stone, with no more than 5 percent passing the ASTM No. 200 sieve. The purpose of this layer is to help distribute concentrated loads and act as a capillary break for moisture migration through the subgrade soil.

The open graded stone should be consolidated in-place with vibratory equipment. The surface of these bases should be choked off with finer material. A clean fine-graded material with a least 10 to 30 percent of particles passing a No. 100 sieve but not contaminated with clay, silt or organic material is recommended.

The open graded stone should be consolidated in-place with vibratory equipment. The surface of these bases should be choked off with finer material. A clean fine-graded material with a least 10 to 30 percent of particles passing a No. 100 sieve but not contaminated with clay, silt or organic material is recommended.

We recommend a minimum 10-mil thick vapor retarder meeting ASTM E 1745, Class C requirements be placed directly below the slab-on-grade floors. A higher quality vapor retarder (Class A or B) may be used if desired to further inhibit the migration of moisture through the slab-on-grade and should be evaluated based on the floor covering and use. The vapor retarder should extend to the edge of the slab-on-grade floors and should be sealed at all seams and penetrations.

An effective modulus of subgrade of 125 pci can be used for slabs supported on the recommended base stone. The slab should be appropriately reinforced (if required) to support anticipated floor loads.

7.0 PAVEMENT CONSIDERATIONS

Specific traffic information was not provided. We assume the proposed roadway classifies as a minor, low frequency residential street subjected to frequent passenger vehicles and light box trucks, and occasional garbage collection trucks. The street needs to be accessible to an 80,000-pound fire truck. Building & Earth can provide alternate pavement sections when given specific traffic information for the planned development.

If the pavement were a typical roadway, according to the "AASHTO Guide for Design of Pavement Structures, 1993", the pavement section recommended for this project will have the following Equivalent 18-kip Single Axle Load (ESAL):

Type	Assumed Equivalent Single Axle Loads (ESAL)
Minor, low frequency residential street	360,000

Table 10: Assumed ESAL Capacity

In addition, we have assumed the following design parameters:

Design Criteria	Value
Design life (Years)	20
Terminal Serviceability	2.0
Reliability	85%
Initial Serviceability	4.2 (Flexible) 4.5 (Rigid)
Standard Deviation	0.45 (Flexible) 0.35 (Rigid)

Table 11: Assumed Design Parameters

All subgrade, base and pavement construction operations should meet minimum requirements of the Oklahoma Department of Transportation (ODOT), Standard Specifications for Highway Construction, dated 2019. The applicable sections of the specifications are identified as follows:

Material	Specification Section
Portland Cement Concrete Pavement	414 & 701
Bituminous Asphalt Wearing Layer	411 & 708
Bituminous Asphalt Binder Layer	411 & 708
Mineral Aggregate Base Materials	303 & 703

Table 12: ODOT Specification Sections

7.1 FLEXIBLE PAVEMENT

The asphalt pavement sections described herein were designed using the "AASHTO Guide for Design of Pavement Structures, 1993". Alternative pavement sections were designed by establishing the structural numbers used for the AASHTO design system and substituting materials based upon structural equivalency as follows:

Material	Structural No.
Asphalt Concrete	0.44
Crushed Stone Base	0.14

Table 13: Structural Equivalent Coefficient

Based on the materials encountered at the boring locations and after our recommendations for site preparation are implemented, flexible pavements at the subject site may be designed based on an estimated California Bearing Ratio (CBR) of 2.5. The following flexible pavement sections are based on the design parameters presented above:

Minimum Recommended Thickness (in)	Material
2.0	HMAC Surface Course (Superpave "S4")
3.5	HMAC Binder Course (Superpave "S3")
8.0	Crushed Aggregate Base (ODOT Type "A")

Table 14: Asphalt Pavement Recommendations

In accordance with the ODOT specifications, asphaltic concrete should be compacted within 92 to 97 percent of the theoretical maximum specific gravity of the asphaltic concrete mix. The underlying aggregate base course should be compacted to at least 98 percent of the material's standard Proctor maximum dry density with a moisture content range of ± 2 percent of the optimum moisture content at the time of placement.

7.2 RIGID PAVEMENT

The following rigid pavement section is based on the design parameters presented above. We assume a modulus of subgrade reaction (k) of 75 pci. We have assumed concrete elastic modulus (E_c) of 3.1 X 10^6 psi, and a concrete modulus of rupture (S'_c) of 600 psi.

Minimum Recommended Thickness (in)	Material
6.0	Portland Cement Concrete, f'_c=3,500 psi
6.0	Crushed Aggregate Base (ODOT Type "A")

Table 15: Reinforced Rigid Pavement Recommendations

For entrance approaches that are frequently subject to high traffic loads with frequent braking and turning of wheels, consideration should be given to using a reinforced rigid pavement section comprised of seven (7) inches of Portland cement concrete and 6 inches ODOT Type "A" crushed aggregate base course.

The recommended aggregate base course will serve as a leveling course, improve the subgrade support properties, and reduce the risk of pumping of fine-grained subgrade soils through the joints.

The concrete should be protected against moisture loss, rapid temperature fluctuations, and construction traffic for several days after placement. All pavements should be sloped for positive drainage. We suggest that a curing compound be applied after the concrete has been finished.

Although not referenced in the ODOT specifications, based on our experience with project sites in this region and anticipated traffic loads, we recommend Portland cement concrete should have a minimum 28-day compressive strength of 3,500 psi, maximum lump of 4 inches, and air content of 5 to 7 percent.

For rigid pavements, we recommend a jointing plan be developed to control cracking and help preclude surficial migration of water into the base course and subgrade. If a jointing plan includes a widely spaced pattern (spacing typically greater than 30 times the slab thickness), consideration should be given to include steel reinforcement in rigid pavements, per Section 3.4 of the American Association of State Highway and Transportation Officials (AASHTO) Guide for Design of Pavement Structures 1993, and Section 3.8 of the American Concrete Institute (ACI) Guide for the Design and Construction of Concrete Parking Lots. Additionally, we recommend the joints be sealed to further preclude surficial moisture migration into the underlying supporting soils.

7.3 GENERAL PAVEMENT DESIGN CONSIDERATIONS

With the use of aggregate base course, the aggregate should have uniform thickness and the subgrade graded such as to provide positive drainage from the granular base. The aggregate base section should grade toward a storm sewer or drainage ditch to provide drainage from the aggregate base.

Due to the moisture sensitive nature of the near-surface clay soils across the site, it is recommended to place a non-woven separator fabric between the subgrade and aggregate base course to prevent fines from the soils intermixing with the aggregate.

Pavements should be sloped, approximately ¼ inch per foot, to provide rapid surface drainage. Water allowed to pond on or adjacent to the pavement could saturate the subgrade and cause premature deterioration of the pavements due to loss of strength and stability.

Periodic maintenance of the pavement should be anticipated. This should include sealing of cracks and joints and maintaining proper surface drainage to avoid ponding of water on or near the pavement areas.

8.0 SUBGRADE REHABILITATION

The subgrade soils often become disturbed during the period between initial site grading and construction of surface improvements. The amount and depth of disturbance will vary with soil type, weather conditions, construction traffic, and drainage.

The engineer should evaluate the subgrade soil during final grading to verify that the subgrade is suitable to receive pavement and/or concrete slab base materials. The final evaluation may include proofrolling or density tests.

Subgrade rehabilitation can become a point of controversy when different contractors are responsible for site grading and building construction. The construction documents should specifically state which contractor will be responsible for maintaining and rehabilitating the subgrade. Rehabilitation may include moisture conditioning and re-compacting soils. When deadlines or weather restrict grading operations, additional measures such as undercutting and replacing saturated soils or chemical stabilization can often be utilized.

9.0 CONSTRUCTION MONITORING

Field verification of site conditions is an essential part of the services provided by the geotechnical consultant. To confirm our recommendations, it will be necessary for Building & Earth personnel to make periodic visits to the site during site grading. Typical construction monitoring services are listed below.

- Periodic observations and consultations by a member of our engineering staff during site grading
- Field density tests during structural fill placement on a continuous basis
- Observation and verification of the bearing surfaces exposed after foundation excavation
- Reinforcing steel inspections
- Post-tension reinforcement inspections, including elongation of tendons.
- Molding and testing of concrete cylinders
- Continuous monitoring and testing during pavement installation

10.0 CLOSING AND LIMITATIONS

This report was prepared for Wallace Design Collective for specific application to the subject project located in Muskogee, Oklahoma. The information in this report is not transferable. This report should not be used for a different development on the same property without first being evaluated by the engineer.

The recommendations in this report were based on the information obtained from our field exploration and laboratory analysis. The data collected is representative of the locations tested. Variations are likely to occur at other locations throughout the site. Engineering judgment was applied in regard to conditions between borings. It will be necessary to confirm the anticipated subsurface conditions during construction.

This report has been prepared in accordance with generally accepted standards of geotechnical engineering practice. No other warranty is expressed or implied. In the event that changes are made, or anticipated to be made, to the nature, design, or location of the project as outlined in this report, Building & Earth must be informed of the changes and given the opportunity to either verify or modify the conclusions of this report in writing, or the recommendations of this report will no longer be valid.

The scope of services for this project did not include any environmental assessment of the site or identification of pollutants or hazardous materials or conditions. If the owner is concerned about environmental issues Building & Earth would be happy to provide an additional scope of services to address those concerns.

This report is intended for use during design and preparation of specifications and may not address all conditions at the site during construction. Contractors reviewing this information should acknowledge that this document is for design information only.

An article published by the Geoprofessional Business Association (GBA), titled *Important Information About Your Geotechnical Report*, has been included in the Appendix. We encourage all individuals to become familiar with the article to help manage risk.

Appendix Table of Contents

GEOTECHNICAL INVESTIGATION METHODOLOGIES

The subsurface exploration, which is the basis of the recommendations of this report, has been performed in accordance with industry standards. Detailed methodologies employed in the investigation are presented in the following sections.

DRILLING PROCEDURES – STANDARD PENETRATION TEST (ASTM D1586)

At each boring location, soil samples were obtained at standard sampling intervals with a split-spoon sampler. The borehole was first advanced to the sample depth by augering and the sampling tools were placed in the open hole. The sampler was then driven 18 inches into the ground with a 140-pound automatic hammer free-falling 30 inches. The number of blows required to drive the sampler each 6-inch increment was recorded. The initial increment is considered the “seating” blows, where the sampler penetrates loose or disturbed soil in the bottom of the borehole.

The blows required to penetrate the final two (2) increments are added together and are referred to as the Standard Penetration Test (SPT) N-value. The N-value, when properly evaluated, gives an indication of the soil’s strength and ability to support structural loads. Many factors can affect the SPT N-value, so this result cannot be used exclusively to evaluate soil conditions.

The SPT testing was performed using a drill rig equipped with an automatic hammer. Automatic hammers mechanically control the height of the hammer drop, and doing so, deliver higher energy efficiency (90 to 99 % efficiency) than manual hammers (60 % efficiency) which are dropped using a manually operated rope and cathead system. Because historic data correlations were developed based on use of a manual hammer, it is necessary to adjust the N-values obtained using an automatic hammer to make these correlations valid. Therefore, an energy correction factor of 1.3 was applied to the recorded field N-values from the automatic hammer for the purpose of our evaluation. The N-values discussed or mentioned in this report and shown on the boring logs are recorded field values.

Samples retrieved from the boring locations were labeled and stored in plastic bags at the jobsite before being transported to our laboratory for analysis. The project engineer prepared Boring Logs summarizing the subsurface conditions at the boring locations.

BORING LOG DESCRIPTION

Building & Earth Sciences, Inc. used the gINT software program to prepare the attached boring logs. The gINT program provides the flexibility to custom design the boring logs to include the pertinent information from the subsurface exploration and results of our laboratory analysis. The soil and laboratory information included on our logs is summarized below:

DEPTH AND ELEVATION

The depth below the ground surface and the corresponding elevation are shown in the first two columns.

SAMPLE TYPE

The method used to collect the sample is shown. The typical sampling methods include Split Spoon Sampling, Shelby Tube Sampling, Grab Samples, and Rock Core. A key is provided at the bottom of the log showing the graphic symbol for each sample type.

SAMPLE NUMBER

Each sample collected is numbered sequentially.

BLOWS PER INCREMENT, REC%, RQD%

When Standard Split Spoon sampling is used, the blows required to drive the sampler each 6-inch increment are recorded and shown in column 5. When rock core is obtained the recovery ration (REC%) and Rock Quality Designation (RQD%) is recorded.

SOIL DATA

Column 6 is a graphic representation of four different soil parameters. Each of the parameters use the same graph, however, the values of the graph subdivisions vary with each parameter. Each parameter presented on column 6 is summarized below:

- N-value- The Standard Penetration Test N-value, obtained by adding the number of blows required to drive the sampler the final 12 inches, is recorded . The graph labels range from 0 to 50.
- Qu – Unconfined Compressive Strength estimate from the Pocket Penetrometer test in tons per square foot (tsf). The graph labels range from 0 to 5 tsf.
- Atterberg Limits – The Atterberg Limits are plotted with the plastic limit to the left, and liquid limit to the right, connected by a horizontal line. The difference in the plastic and liquid limits is referred to as the Plasticity Index. The Atterberg Limits test results are also included in the Remarks column on the far right of the boring log. The Atterberg Limits graph labels range from 0 to 100%.
- Moisture – The Natural Moisture Content of the soil sample as determined in our laboratory.

SOIL DESCRIPTION

The soil description prepared in accordance with ASTM D2488, Visual Description of Soil Samples. The Munsel Color chart is used to determine the soil color. Strata changes are indicated by a solid line, with the depth of the change indicated on the left side of the line and the elevation of the change indicated on the right side of the line. If subtle changes within a soil type occur, a broken line is used. The Boring Termination or Auger Refusal depth is shown as a solid line at the bottom of the boring.

GRAPHIC

The graphic representation of the soil type is shown. The graphic used for each soil type is related to the Unified Soil Classification chart. A chart showing the graphic associated with each soil classification is included.

REMARKS

Remarks regarding borehole observations, and additional information regarding the laboratory results and groundwater observations.

Major Divisions			*Symbols*		*Group Name & Typical Description*
			Lithology	*Group*	
Coarse Grained Soils *More than 50% of material is larger than No. 200 sieve size*	***Gravel and Gravelly Soils*** *More than 50% of coarse fraction is larger than No. 4 sieve*	***Clean Gravels*** *(Less than 5% fines)*	▨	***GW***	*Well-graded gravels, gravel – sand mixtures, little or no fines*
			▨	***GP***	*Poorly-graded gravels, gravel – sand mixtures, little or no fines*
		Gravels with Fines *(More than 12% fines)*	▨	***GM***	*Silty gravels, gravel – sand – silt mixtures*
			▨	***GC***	*Clayey gravels, gravel – sand – clay mixtures*
	Sand and Sandy Soils *More than 50% of coarse fraction is smaller than No. 4 sieve*	***Clean Sands*** *(Less than 5% fines)*	▨	***SW***	*Well-graded sands, gravelly sands, little or no fines*
			▨	***SP***	*Poorly-graded sands, gravelly sands, little or no fines*
		Sands with Fines *(More than 12% fines)*	▨	***SM***	*Silty sands, sand – silt mixtures*
			▨	***SC***	*Clayey sands, sand – clay mixtures*
Fine Grained Soils *More than 50% of material is smaller than No. 200 sieve size*	***Silts and Clays*** *Liquid Limit less than 50*	***Inorganic***	▨	***ML***	*Inorganic silts and very find sands, rock flour, silty or clayey fine sands or clayey silt with slight plasticity*
			▨	***CL***	*Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays*
		Organic	▨	***OL***	*Organic silts and organic silty clays of low plasticity*
	Silts and Clays *Liquid Limit greater than 50*	***Inorganic***	▨	***MH***	*Inorganic silts, micaceous or diatomaceous fine sand, or silty soils*
			▨	***CH***	*Inorganic clays of high plasticity*
		Organic	▨	***OH***	*Organic clays of medium to high plasticity, organic silts*
Highly Organic Soils			▨	***PT***	*Peat, humus, swamp soils with high organic contents*

Table 1: Soil Classification Chart (based on ASTM D2487)

SOIL CLASSIFICATION METHODOLOGY

Building & Earth Sciences classifies soil in general accordance with the Unified Soil Classification System (USCS) presented in ASTM D2487. Table 1 and Figure 1 exemplify the general guidance of the USCS. Soil consistencies and relative densities are presented in general accordance with Terzaghi, Peck, & Mesri's (1996) method, as shown on Table 2, when quantitative field and/or laboratory data is available. Table 2 includes Consistency and Relative Density correlations with N-values obtained using either a manual hammer (60 percent efficiency) or automatic hammer (90 percent efficiency). The *Blows Per Increment* and *SPT N-values* displayed on the boring logs are the unaltered values measured in the field. When field and/or laboratory data is not available, we may classify soil in general accordance with the Visual Manual Procedure presented in ASTM D2488.

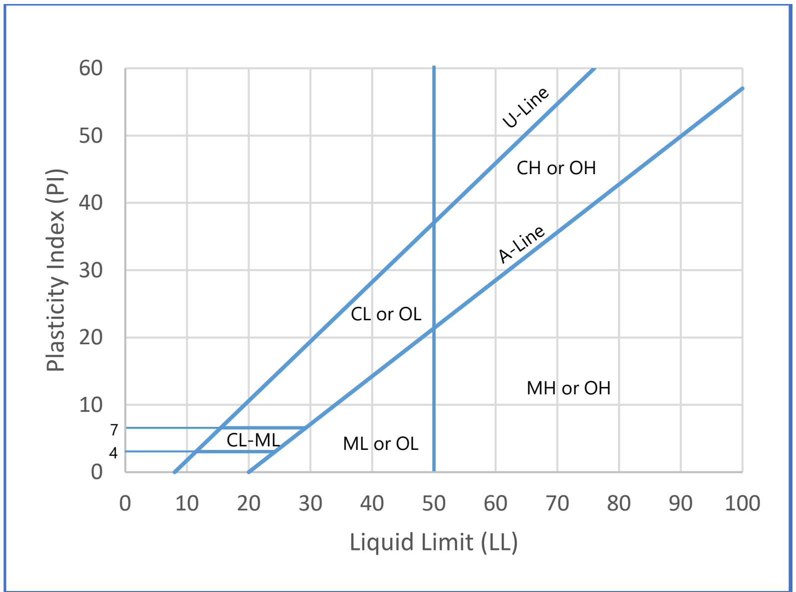


Figure 1: Plasticity Chart (based on ASTM D2487)

Non-cohesive: Coarse-Grained Soil			**Cohesive: Fine-Grained Soil**			
SPT Penetration (blows/foot)		**Relative Density**	**SPT Penetration (blows/foot)**		**Consistency**	**Estimated Range of Unconfined Compressive Strength (tsf)**
			Automatic Hammer*	Manual Hammer		
Automatic Hammer*	Manual Hammer		< 2	< 2	Very Soft	< 0.25
0 - 3	0 - 4	Very Loose	2 - 3	2 - 4	Soft	0.25 – 0.50
3 - 8	4 - 10	Loose	3 - 6	4 - 8	Medium Stiff	0.50 – 1.00
8 - 23	10 - 30	Medium Dense	6 - 12	8 - 15	Stiff	1.00 – 2.00
23 - 38	30 - 50	Dense	12 - 23	15 - 30	Very Stiff	2.00 – 4.00
> 38	> 50	Very Dense	> 23	> 30	Hard	> 4.00

Table 2: Soil Consistency and Relative Density (based on Terzaghi, Peck & Mesri, 1996)

* - Modified based on 80% hammer efficiency

KEY TO LOGS

☒	Standard Penetration Test ASTM D1586 or AASHTO T-206	▼	Dynamic Cone Penetrometer (Sower DCP) ASTM STP-399
■	Shelby Tube Sampler ASTM D1587	○	No Sample Recovery
▥	Rock Core Sample ASTM D2113	▽	Groundwater at Time of Drilling
▮	Auger Cuttings	▼	Groundwater as Indicated

Table 1: Symbol Legend

Soil	Particle Size	U.S. Standard
Boulders	Larger than 300 mm	N.A.
Cobbles	300 mm to 75 mm	N.A.
Gravel	75 mm to 4.75 mm	3-inch to #4 sieve
Coarse	75 mm to 19 mm	3-inch to ¾-inch sieve
Fine	19 mm to 4.75 mm	¾-inch to #4 sieve
Sand	4.75 mm to 0.075 mm	#4 to #200 Sieve
Coarse	4.75 mm to 2 mm	#4 to #10 Sieve
Medium	2 mm to 0.425 mm	#10 to #40 Sieve
Fine	0.425 mm to 0.075 mm	#40 to #200 Sieve
Fines	Less than 0.075 mm	Passing #200 Sieve
Silt	Less than 5 µm	N.A.
Clay	Less than 2 µm	N.A.

Table 2: Standard Sieve Sizes

N-Value ☐	Standard Penetration Test Resistance calculated using ASTM D1586 or AASHTO T-206. Calculated as sum of original, field recorded values.	Atterberg Limits ├─┤ PL LL	A measure of a soil's plasticity characteristics in general accordance with ASTM D4318. The soil Plasticity Index (PI) is representative of this characteristic and is bracketed by the Liquid Limit (LL) and the Plastic Limit (PL).
Qu ▲	Unconfined compressive strength, typically estimated from a pocket penetrometer. Results are presented in tons per square foot (tsf).	% Moisture ●	Percent natural moisture content in general accordance with ASTM D2216.

Table 3: Soil Data

Hollow Stem Auger	Flights on the outside of the shaft advance soil cuttings to the surface. The hollow stem allows sampling through the middle of the auger flights.
Mud Rotary / Wash Bore	A cutting head advances the boring and discharges a drilling fluid to support the borehole and circulate cuttings to the surface.
Solid Flight Auger	Flights on the outside bring soil cuttings to the surface. Solid stem requires removal from borehole during sampling.
Hand Auger	Cylindrical bucket (typically 3-inch diameter and 8 inches long) attached to a metal rod and turned by human force.

Table 4: Soil Drilling Methods

Descriptor	Meaning
Trace	Likely less than 5%
Few	5 to 10%
Little	15 to 25%
Some	30 to 45%
Mostly	50 to 100%

Table 5: Descriptors

Manual Hammer	The operator tightens and loosens the rope around a rotating drum assembly to lift and drop a sliding, 140-pound hammer falling 30 inches.
Automatic Trip Hammer	An automatic mechanism is used to lift and drop a sliding, 140-pound hammer falling 30 inches.
Dynamic Cone Penetrometer (Sower DCP) ASTM STP-399	Uses a 15-pound steel mass falling 20 inches to strike an anvil and cause penetration of a 1.5-inch diameter cone seated in the bottom of a hand augered borehole. The blows required to drive the embedded cone a depth of 1-3/4 inches have been correlated by others to N-values derived from the Standard Penetration Test (SPT).

Table 6: Sampling Methods

Non-plastic	A 1/8-inch thread cannot be rolled at any water content.
Low	The thread can barely be rolled and the lump cannot be formed when drier than the plastic limit.
Medium	The thread is easy to roll and not much time is required to reach the plastic limit. The thread cannot be re-rolled after reaching the plastic limit. The lump crumbles when drier than the plastic limit.
High	It takes considerable time rolling and kneading to reach the plastic limit. The thread can be re-rolled several times after reaching the plastic limit. The lump can be formed without crumbling when drier than the plastic limit.

Table 7: Plasticity

Dry	Absence of moisture, dusty, dry to the touch.
Moist	Damp but no visible water.
Wet	Visible free water, usually soil is below water table.

Table 8: Moisture Condition

Stratified	Alternating layers of varying material or color with layers at least ½ inch thick.
Laminated	Alternating layers of varying material or color with layers less than ¼ inch thick.
Fissured	Breaks along definite planes of fracture with little resistance to fracturing.
Slickensides	Fracture planes appear polished or glossy, sometimes striated.
Blocky	Cohesive soil that can be broken down into small angular lumps which resist further breakdown.
Lensed	Inclusion of small pockets of different soils, such as small lenses of sand scattered through a mass of clay.
Homogeneous	Same color and appearance throughout.

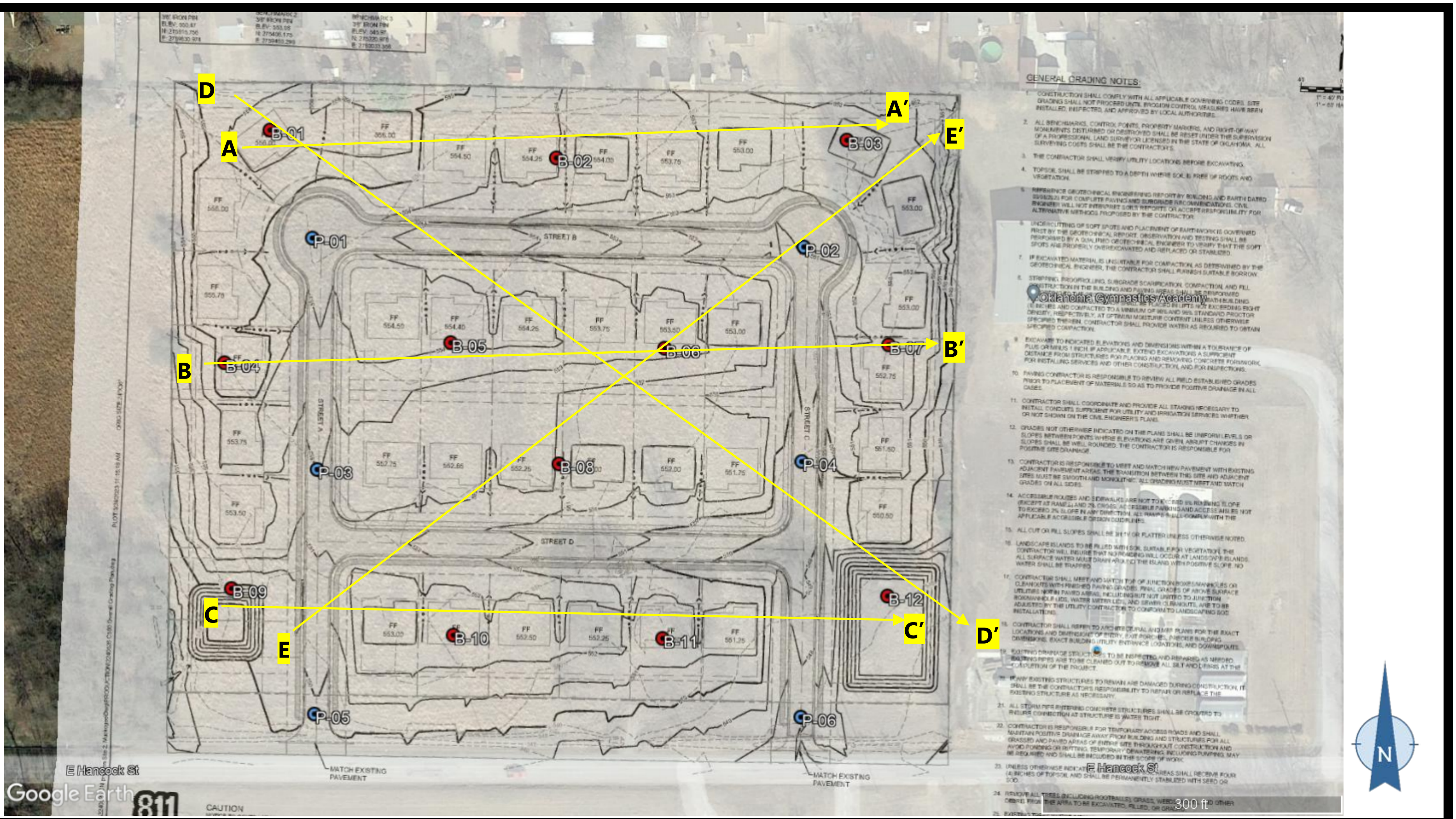
Table 9: Structure

KEY TO HATCHES

Hatch	Description	Hatch	Description	Hatch	Description
▨	***GW*** *- Well-graded gravels, gravel – sand mixtures, little or no fines*	■	*Asphalt*	▨	*Clay with Gravel*
▨	***GP*** *- Poorly-graded gravels, gravel – sand mixtures, little or no fines*	▨	*Aggregate Base*	▨	*Sand with Gravel*
▨	***GM*** *- Silty gravels, gravel – sand – silt mixtures*	▨	*Topsoil*	▨	*Silt with Gravel*
▨	***GC*** *- Clayey gravels, gravel – sand – clay mixtures*	▨	*Concrete*	▨	*Gravel with Sand*
▨	***SW*** *- Well-graded sands, gravelly sands, little or no fines*	▨	*Coal*	▨	*Gravel with Clay*
▨	***SP*** *- Poorly-graded sands, gravelly sands, little or no fines*	▨	***CL-ML*** *- Silty Clay*	▨	*Gravel with Silt*
▨	***SM*** *- Silty sands, sand – silt mixtures*	▨	*Sandy Clay*	▨	*Limestone*
▨	***SC*** *- Clayey sands, sand – clay mixtures*	▨	*Clayey Chert*	▨	*Chalk*
▨	***ML*** *- Inorganic silts and very find sands, rock flour, silty or clayey fine sands or clayey silt with slight plasticity*	▨	*Low and High Plasticity Clay*	▨	*Siltstone*
▨	***CL*** *- Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays*	▨	*Low Plasticity Silt and Clay*	▨	*Till*
▨	***OL*** *- Organic silts and organic silty clays of low plasticity*	▨	*High Plasticity Silt and Clay*	▨	*Sandy Clay with Cobbles and Boulders*
▨	***MH*** *- Inorganic silts, micaceous or diatomaceous fine sand, or silty soils*	▨	*Fill*	▨	*Sandstone with Shale*
▨	***CH*** *- Inorganic clays of high plasticity*	▨	*Weathered Rock*	▨	*Coral*
▨	***OH*** *- Organic clays of medium to high plasticity, organic silts*	▨	*Sandstone*	▨	*Boulders and Cobbles*
▨	***PT*** *- Peat, humus, swamp soils with high organic contents*	▨	*Shale*	▨	*Soil and Weathered Rock*

Table 1: Key to Hatches Used for Boring Logs and Soil Profiles

BORING LOCATION PLAN



REFERENCE USED TO PRODUCE THIS DRAWING:

Google Earth Satellite Imagery dated January 2021, with Topographic Site Plan overlay provided by Wallace Design Collective, undated

BORING LOCATION PLAN

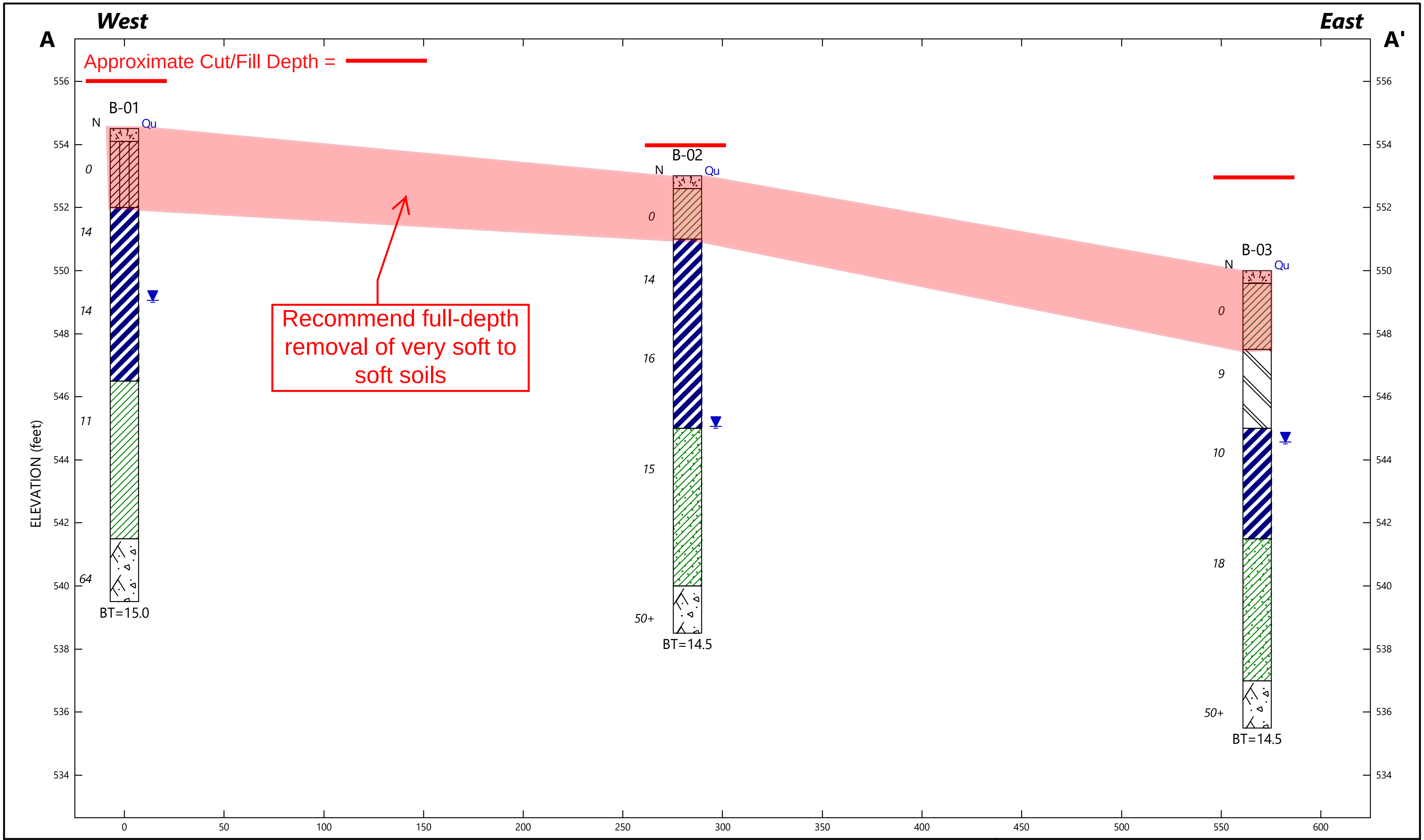
DATE: 03/28/2023

PROJECT NO.	**PROJECT NAME / LOCATION:**	**SCALE:**
TU230050	Cherokee Nation Housing Hancock-Muskogee Muskogee, Oklahoma	As Shown

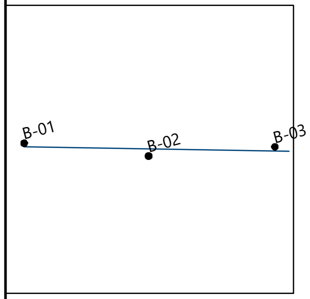
BUILDING & EARTH

Geotechnical, Environmental, and Materials Engineers

SUBSURFACE SOIL PROFILES



Recommend full-depth removal of very soft to soft soils



Site Map Scale 1 inch equals 435 feet

Key to Hatches

- ▨ Topsoil
- ▥ USCS Silty Clay
- ▨ USCS High Plasticity Clay
- ▨ USCS Low Plasticity Clay
- ▨ Weathered Rock
- ▨ USCS Low Plasticity Sandy Clay
- ▧ USCS Low to High Plasticity Clay

Legend

BT=Boring Termination, TPT=Test Pit Terminated
AR=Auger Refusal, ER=Excavation Refusal
N=Standard Penetration Test N-Value
Qu=Unconfined compressive strength estimate from pocket penetrometer test (tsf)
▽ Water Level Reading at time of drilling.
▼ Water Level Reading after drilling.

0 71

Horizontal Scale (feet)
Vertical Exaggeration: 16x

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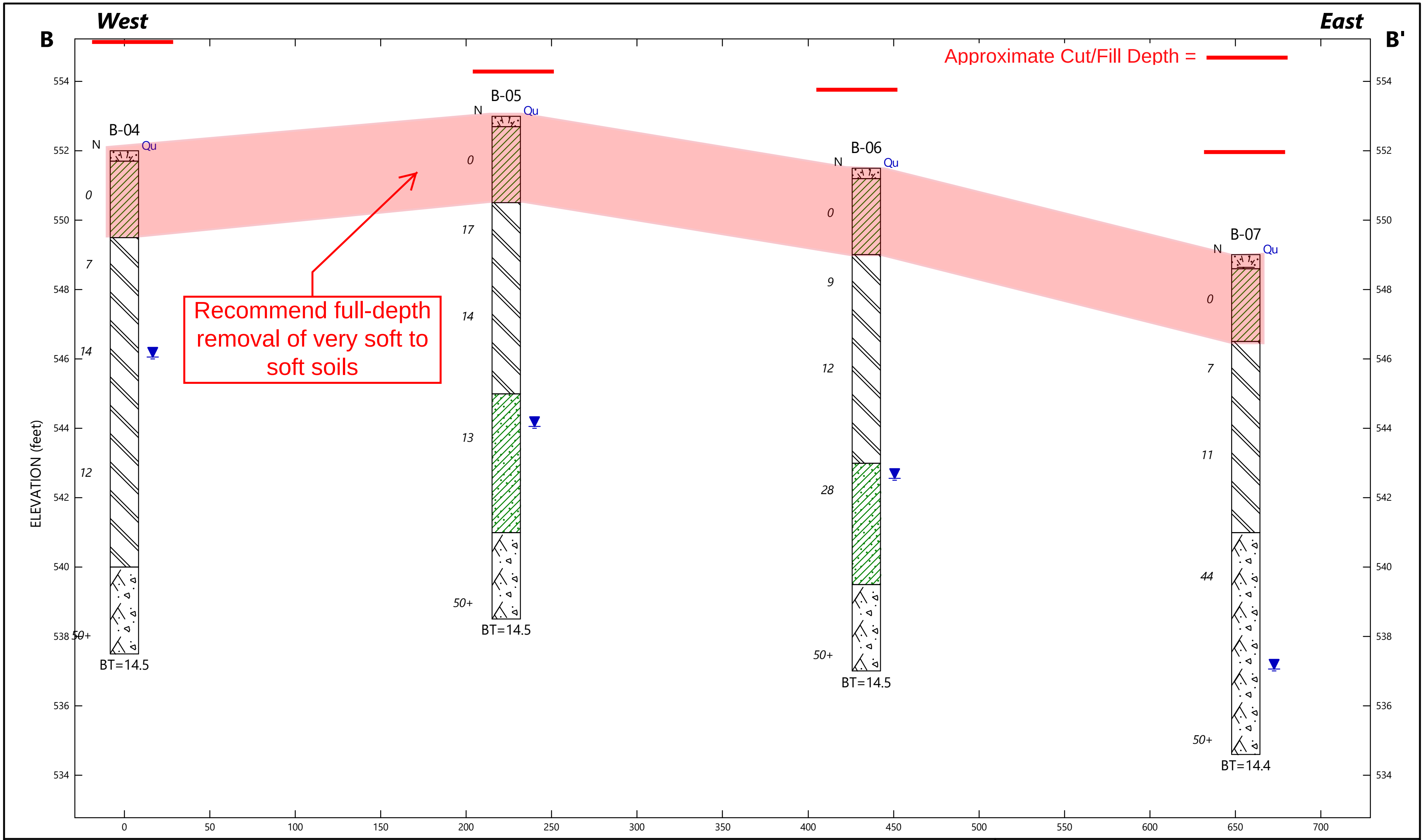
Cherokee Nation Housing, Hancock-Muskogee
Muskogee, Oklahoma

A-A': Subsurface Profile

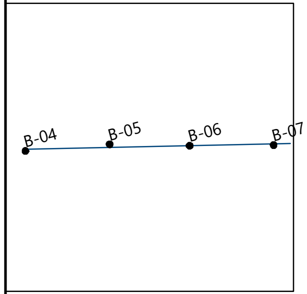
PROJECT NO: TU230050	PLATE NO: A-1	DATE: 4/12/23

BUILDING & EARTH
Geotechnical, Environmental, and Materials Engineers



Approximate Cut/Fill Depth =

Recommend full-depth removal of very soft to soft soils



Site Map Scale 1 inch equals 505 feet

Key to Hatches

▨	Topsoil
▨	Weathered Rock
▨	USCS Low Plasticity Clay
▨	USCS Low Plasticity Sandy Clay
▨	USCS Low to High Plasticity Clay

Legend

BT=Boring Termination, TPT=Test Pit Terminated
AR=Auger Refusal, ER=Excavation Refusal
N=Standard Penetration Test N-Value
Qu=Unconfined compressive strength estimate from pocket penetrometer test (tsf)

▽ Water Level Reading at time of drilling.
▼ Water Level Reading after drilling.

0 83
Horizontal Scale (feet)
Vertical Exaggeration: 20.5x

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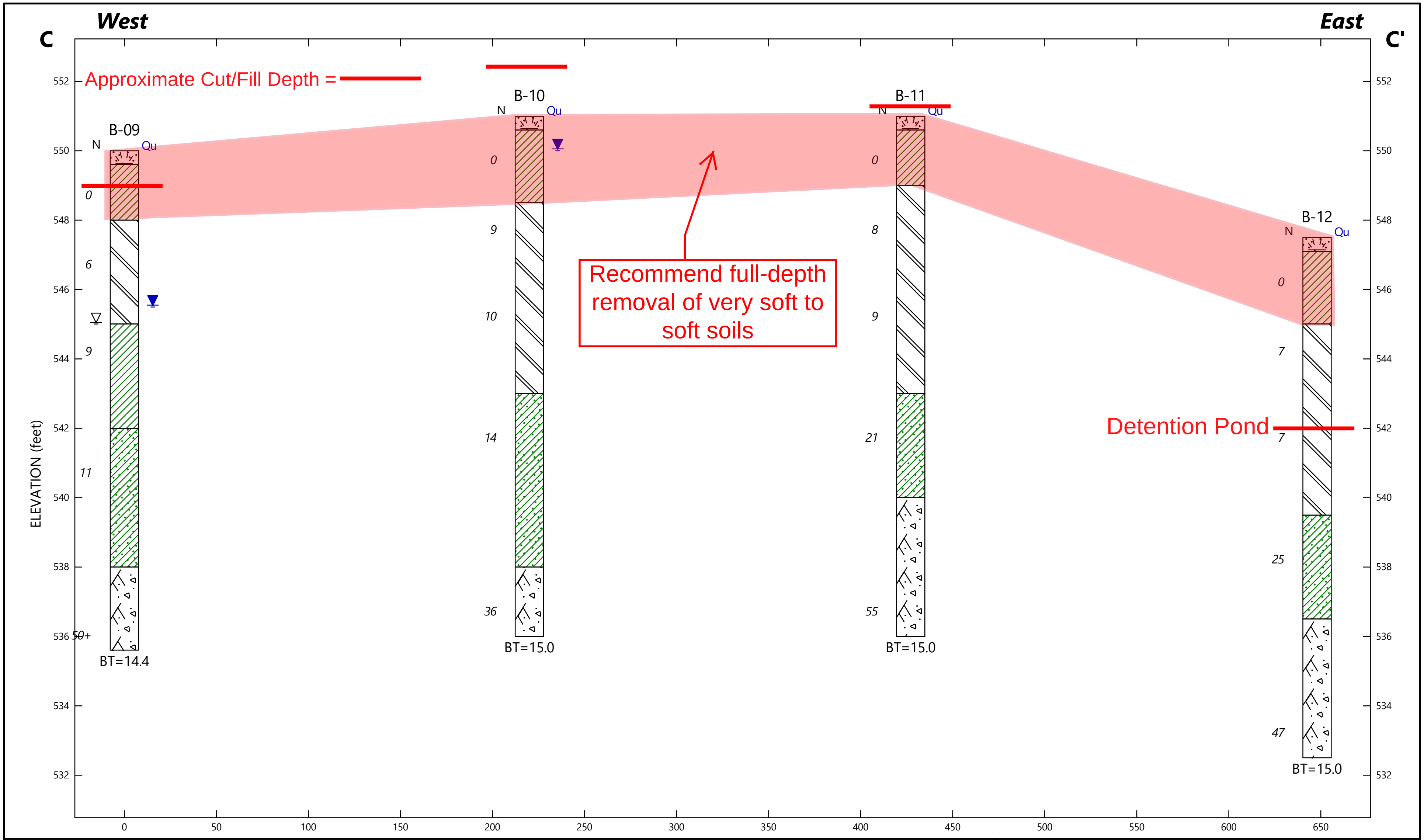
Cherokee Nation Housing, Hancock-Muskogee
Muskogee, Oklahoma

B-B': Subsurface Profile

PROJECT NO: TU230050	PLATE NO: B-1	DATE: 4/24/23

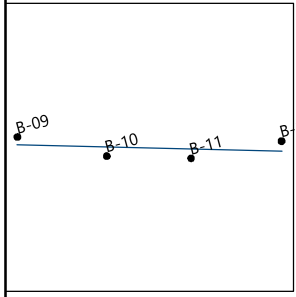
BUILDING & EARTH
Geotechnical, Environmental, and Materials Engineers



Approximate Cut/Fill Depth =

Recommend full-depth removal of very soft to soft soils

Detention Pond



Site Map Scale 1 inch equals 470 feet

Key to Hatches

▨	Topsoil
🟩	USCS Low Plasticity Clay
▨	USCS Low to High Plasticity Clay
🟩	USCS Low Plasticity Sandy Clay
▨	Weathered Rock

Legend

BT=Boring Termination, TPT=Test Pit Terminated
AR=Auger Refusal, ER=Excavation Refusal
N=Standard Penetration Test N-Value
Qu=Unconfined compressive strength estimate from pocket penetrometer test (tsf)
∇ Water Level Reading at time of drilling.
▼ Water Level Reading after drilling.

0 77

Horizontal Scale (feet)
Vertical Exaggeration: 19x

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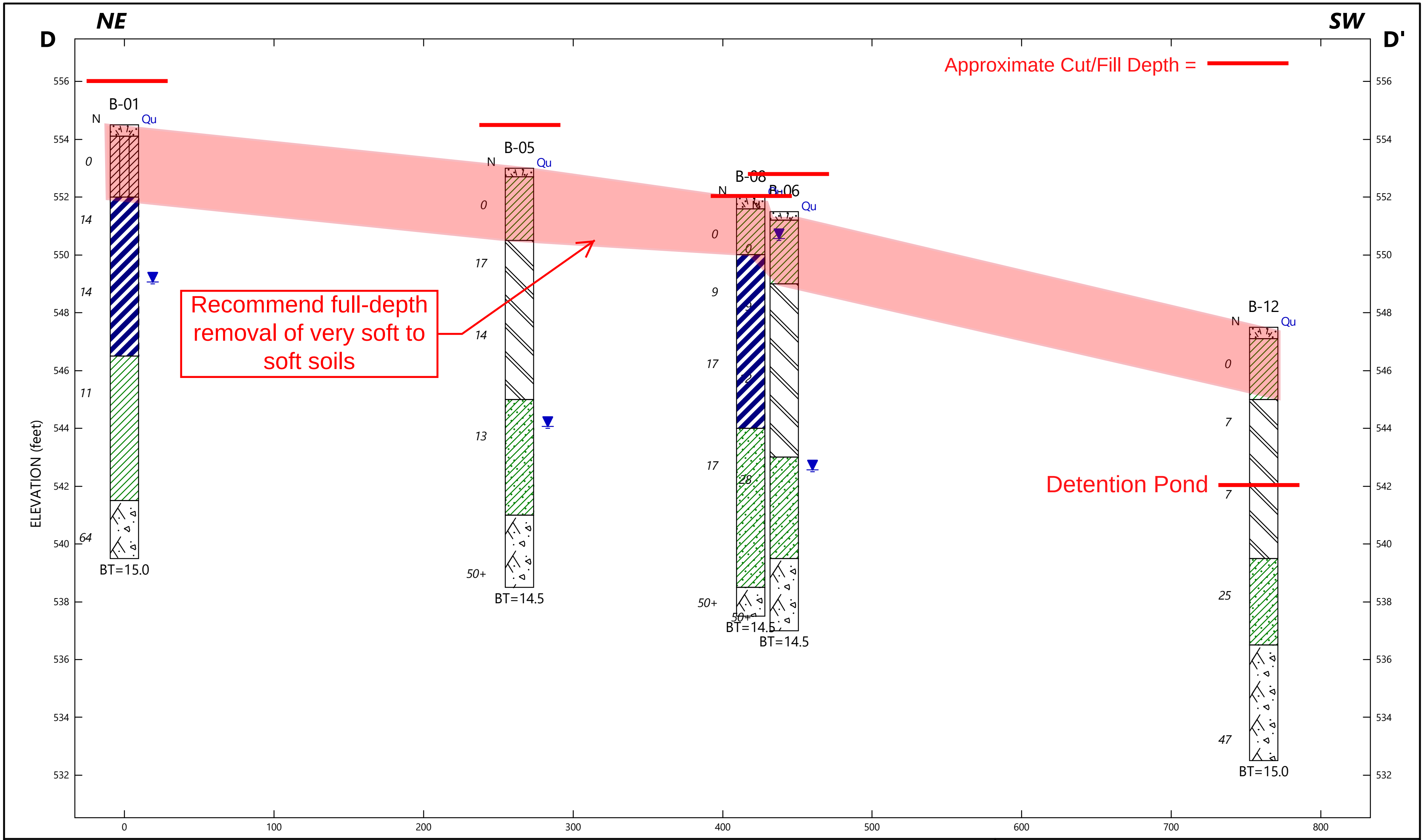
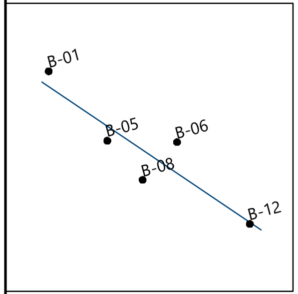
Cherokee Nation Housing, Hancock-Muskogee
Muskogee, Oklahoma

C-C': Subsurface Profile

PROJECT NO: TU230050	PLATE NO: C-1	DATE: 4/12/23

BUILDING & EARTH
Geotechnical, Environmental, and Materials Engineers





Site Map Scale 1 inch equals 580 feet

Key to Hatches

- ▨ Topsoil
- ▨ USCS Silty Clay
- ▨ USCS High Plasticity Clay
- ▨ USCS Low Plasticity Clay
- ▨ Weathered Rock
- ▨ USCS Low to High Plasticity Clay
- ▨ USCS Low Plasticity Sandy Clay

Legend

BT=Boring Termination, TPT=Test Pit Terminated
AR=Auger Refusal, ER=Excavation Refusal
N=Standard Penetration Test N-Value
Qu=Unconfined compressive strength estimate from pocket penetrometer test (tsf)
▽ Water Level Reading at time of drilling.
▼ Water Level Reading after drilling.

0 95

Horizontal Scale (feet)
Vertical Exaggeration: 19.5x

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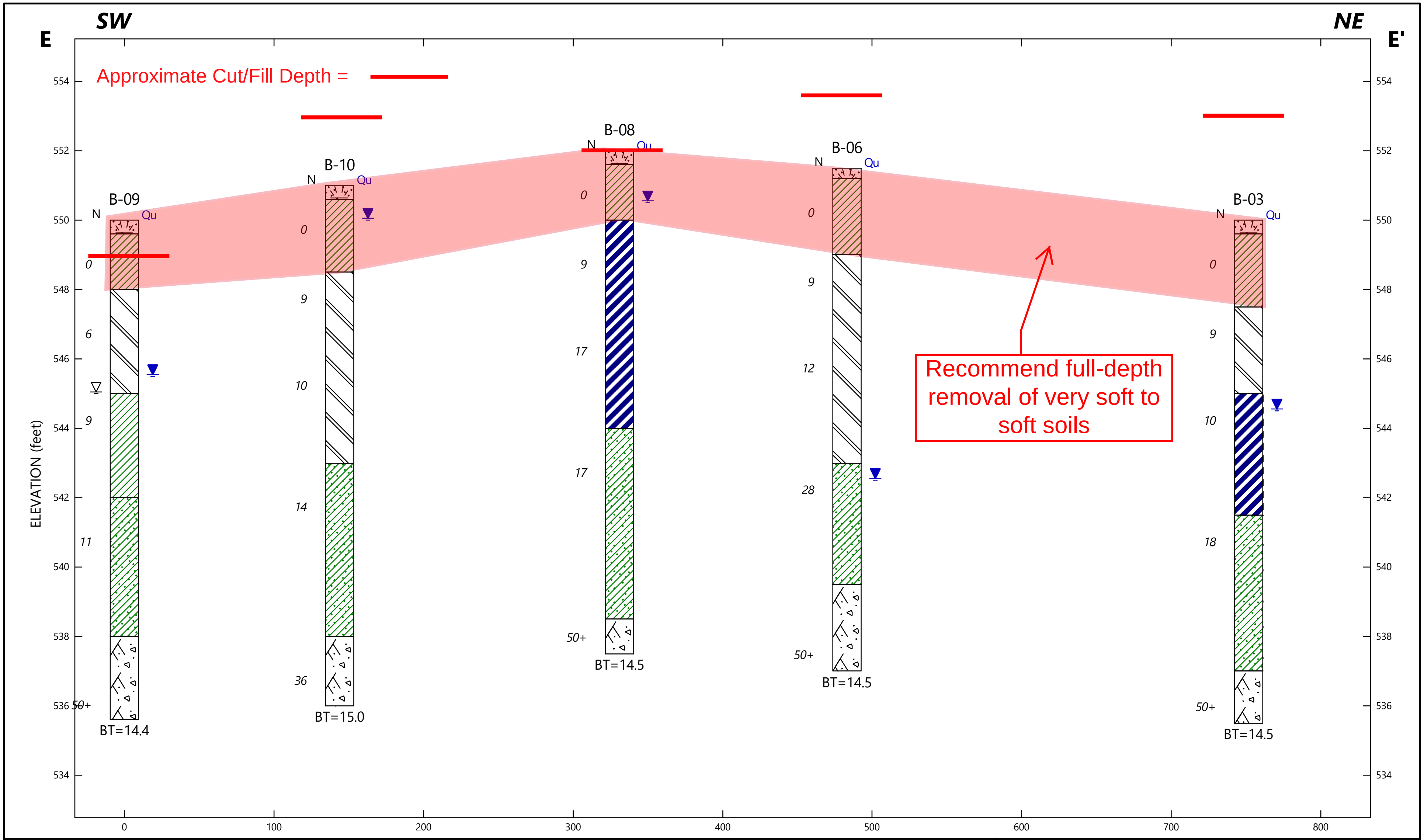
Cherokee Nation Housing, Hancock-Muskogee
Muskogee, Oklahoma

D-D': Subsurface Profile

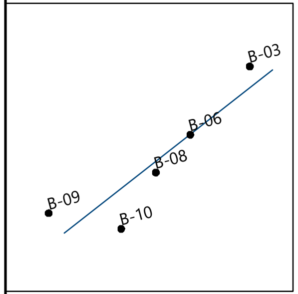
PROJECT NO: TU230050	PLATE NO: D-1	DATE: 4/12/23

BUILDING & EARTH
Geotechnical, Environmental, and Materials Engineers



Approximate Cut/Fill Depth =

Recommend full-depth removal of very soft to soft soils



Site Map Scale 1 inch equals 580 feet

Key to Hatches

- ▨ Topsoil
- ▨ USCS High Plasticity Clay
- ▨ USCS Low Plasticity Clay
- ▨ USCS Low Plasticity Sandy Clay
- ▨ USCS Low to High Plasticity Clay
- ▨ Weathered Rock

Legend

BT=Boring Termination, TPT=Test Pit Terminated
AR=Auger Refusal, ER=Excavation Refusal
N=Standard Penetration Test N-Value
Qu=Unconfined compressive strength estimate from pocket penetrometer test (tsf)
▽ Water Level Reading at time of drilling.
▼ Water Level Reading after drilling.

0 95

Horizontal Scale (feet)
Vertical Exaggeration: 23x

Building & Earth Sciences, Inc.

1403 South 70th East Avenue, Tulsa, OK 74112

Cherokee Nation Housing, Hancock-Muskogee
Muskogee, Oklahoma

E-E': Subsurface Profile

PROJECT NO: TU230050	PLATE NO: E-1	DATE: 4/12/23

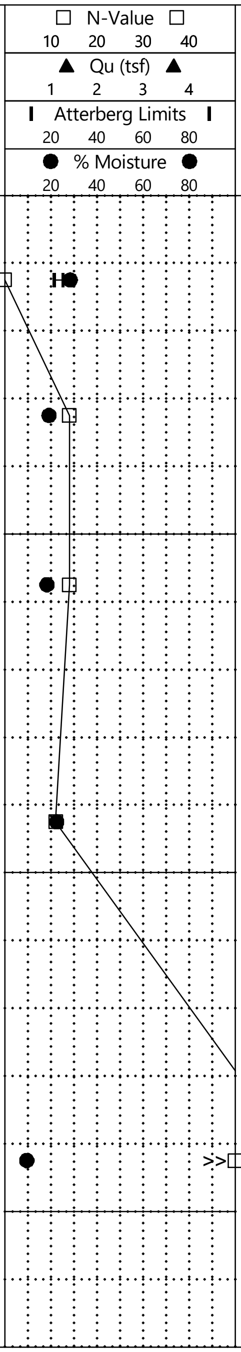
BUILDING & EARTH
Geotechnical, Environmental, and Materials Engineers

BORING LOGS

LOG OF BORING
Designation: B-01
Sheet 1 of 1

1403 South 70th East Avenue
Tulsa, OK 74112
Office: (918) 439-9005

PROJECT NAME: Cherokee Nation Housing, Hancock-Muskogee
PROJECT NUMBER: TU230050
DRILLING METHOD: Solid Flight Auger
EQUIPMENT USED: Diedrich D-50 ATV
HAMMER TYPE: Automatic
BORING LOCATION: 35.727825, -95.335270
LOCATION: Muskogee, Oklahoma
DATE DRILLED: 3/27/23
WEATHER: Clear
ELEVATION: 554.5
DRILL CREW: Building & Earth
LOGGED BY: J. Swyden



DEPTH (ft)	ELEVATION (ft)	SAMPLE TYPE	SAMPLE NO.	BLOWS PER INCREMENT	LAB DATA	SOIL DESCRIPTION	GRAPHIC	REMARKS
0.4	554.1					TOPSOIL: 4"		
		☒	1	0 0 0	Sample 1 LL: 25 PL: 21 PI: 4 M: 28.4%	SILTY CLAY (CL-ML): very soft, brown, low plasticity, wet, (RESIDUAL)		**Finished Grade @ 556**
2.5	552.0							
		☒	2	4 5 9	Sample 2 M: 19.1%	FAT CLAY (CH): very stiff, light brown, brownish yellow, high plasticity, moist, (RESIDUAL)		
5	550							
		☒	3	4 5 9	Sample 3 M: 18.3%	gray, yellow, with ferrous nodules ▼		Free water was measured at 5.5 feet, prior to backfilling the borehole
8.0	546.5							
		☒	4	4 5 6	Sample 4 M: 22.5%	LEAN CLAY (CL): stiff, light gray, yellow, medium plasticity, moist, with ferrous staining, (RESIDUAL)		
10	545							
13.0	541.5							
		☒	5	25 29 35	Sample 5 M: 9.5%	WEATHERED SHALE: soft, yellowish brown, (WEATHERED ROCK)		
15.0	539.5							
						Boring Terminated at 15 feet.		Borehole backfilled on 3/28/2023. Consistency/Relative Density based on correction factor for Automatic hammer.

SAMPLE TYPE ☒ Split Spoon

N-VALUE STANDARD PENETRATION RESISTANCE (AASHTO T-206)
% MOISTURE PERCENT NATURAL MOISTURE CONTENT
∇ GROUNDWATER LEVEL IN THE BOREHOLE AT TIME OF DRILLING
▼ STABILIZED GROUNDWATER LEVEL
REC RECOVERY
RQD ROCK QUALITY DESIGNATION
UD UNDISTURBED
Qu POCKET PENETROMETER UNCONFINED COMPRESSIVE STRENGTH
LL: LIQUID LIMIT
PL: PLASTIC LIMIT
PI: PLASTICITY INDEX
M: NATURAL MOISTURE CONTENT
F: PERCENT PASSING NO. 200 SIEVE

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Tulsa, OK 74112
Office: (918) 439-9005

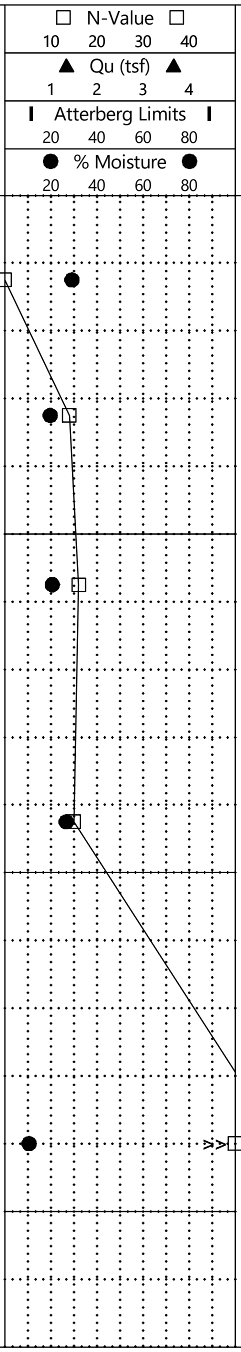
LOG OF BORING

Designation: B-02

Sheet 1 of 1

PROJECT NAME: Cherokee Nation Housing, Hancock-Muskogee
PROJECT NUMBER: TU230050
DRILLING METHOD: Solid Flight Auger
EQUIPMENT USED: Diedrich D-50 ATV
HAMMER TYPE: Automatic
BORING LOCATION: 35.727750, -95.334318

LOCATION: Muskogee, Oklahoma
DATE DRILLED: 3/27/23
WEATHER: Clear
ELEVATION: 553
DRILL CREW: Building & Earth
LOGGED BY: J. Swyden



DEPTH (ft)	ELEVATION (ft)	SAMPLE TYPE	SAMPLE NO.	BLOWS PER INCREMENT	LAB DATA	SOIL DESCRIPTION	REMARKS
0.4	552.6					TOPSOIL: 4"	
		☒	1	0 0 0	Sample 1 M: 29.1%	LEAN CLAY (CL): very soft, brown, low plasticity, wet, (RESIDUAL)	**Finished Grade @ 554.25** **Organic Content = 2.5%**
2.0	551.0	☒	2	3 5 9	Sample 2 M: 19.7%	FAT CLAY (CH): very stiff, yellowish brown, light brownish gray, high plasticity, moist, with ferrous staining, (RESIDUAL)	
5		☒	3	5 7 9	Sample 3 M: 20.6%	gray	
8.0	▼ 545.0	☒	4	5 7 8	Sample 4 M: 26.6%	SANDY LEAN CLAY (CL): very stiff, brownish yellow, medium plasticity, moist, with sandstone fragments and ferrous staining, (RESIDUAL)	Free water was measured at 8 feet, prior to backfilling the borehole
13.0	540.0	☒	5	31 50/5.75"	Sample 5 M: 10.5%	WEATHERED SHALE: soft, yellowish brown, with ferrous staining, (WEATHERED ROCK)	
14.5	538.5					Boring Terminated at 14.5 feet.	Borehole backfilled on 3/28/2023. Consistency/Relative Density based on correction factor for Automatic hammer.

SAMPLE TYPE ☒ Split Spoon

N-VALUE STANDARD PENETRATION RESISTANCE (AASHTO T-206)
% MOISTURE PERCENT NATURAL MOISTURE CONTENT
▽ GROUNDWATER LEVEL IN THE BOREHOLE AT TIME OF DRILLING
▼ STABILIZED GROUNDWATER LEVEL

REC RECOVERY
RQD ROCK QUALITY DESIGNATION
UD UNDISTURBED
Qu POCKET PENETROMETER UNCONFINED COMPRESSIVE STRENGTH

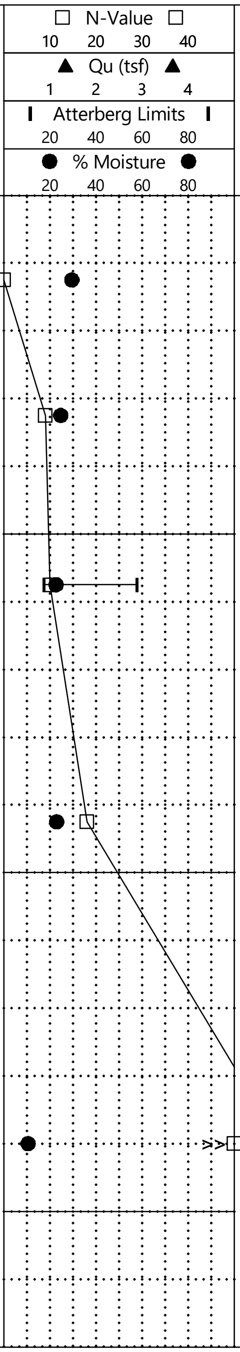
LL: LIQUID LIMIT
PL: PLASTIC LIMIT
PI: PLASTICITY INDEX
M: NATURAL MOISTURE CONTENT
F: PERCENT PASSING NO. 200 SIEVE

BUILDING & EARTH
Geotechnical, Environmental, and Materials Engineers

1403 South 70th East Avenue
Tulsa, OK 74112
Office: (918) 439-9005

LOG OF BORING
Designation: B-03
Sheet 1 of 1

PROJECT NAME: Cherokee Nation Housing, Hancock-Muskogee
PROJECT NUMBER: TU230050
DRILLING METHOD: Solid Flight Auger
EQUIPMENT USED: Diedrich D-50 ATV
HAMMER TYPE: Automatic
BORING LOCATION: 35.727797, -95.333353
LOCATION: Muskogee, Oklahoma
DATE DRILLED: 3/27/23
WEATHER: Clear
ELEVATION: 550
DRILL CREW: Building & Earth
LOGGED BY: J. Swyden



DEPTH (ft)	ELEVATION (ft)	SAMPLE TYPE	SAMPLE NO.	BLOWS PER INCREMENT	LAB DATA	SOIL DESCRIPTION	GRAPHIC	REMARKS
	550							
0.4	549.6					TOPSOIL: 5"		
		☒	1	0 0 0	Sample 1 M: 29.5%	LEAN CLAY (CL): very soft, brown, grayish brown, low plasticity, wet, (RESIDUAL)		**Finished Grade @ 553**
2.5	547.5	☒	2	2 4 5	Sample 2 M: 24.7%	LEAN TO FAT CLAY (CL-CH): stiff, yellowish brown, brown, medium to high plasticity, moist, (RESIDUAL)		
5.0 5	545.0 545	☒	3	3 4 6	Sample 3 LL: 58 PL: 17 PI: 41 M: 22.6% F: 86.1%	FAT CLAY (CH): stiff, brownish yellow, grayish brown, high plasticity, moist, (RESIDUAL) ▼		Free water was measured at 5.5 feet, prior to backfilling the borehole
8.5	541.5	☒	4	4 7 11	Sample 4 M: 22.9%	SANDY LEAN CLAY (CL): very stiff, reddish yellow, medium plasticity, moist, with sandstone fragments and ferrous staining, (RESIDUAL)		
10	540							
13.0	537.0	☒	5	33 50/6"	Sample 5 M: 10.5%	WEATHERED SHALE: soft, light brown, (WEATHERED ROCK)		
14.5	535.5					Boring Terminated at 14.5 feet.		Borehole backfilled on 3/28/2023 Consistency/Relative Density based on correction factor for Automatic hammer.
15	535							

SAMPLE TYPE ☒ Split Spoon

N-VALUE STANDARD PENETRATION RESISTANCE (AASHTO T-206)
% MOISTURE PERCENT NATURAL MOISTURE CONTENT
▽ GROUNDWATER LEVEL IN THE BOREHOLE AT TIME OF DRILLING
▼ STABILIZED GROUNDWATER LEVEL
REC RECOVERY
RQD ROCK QUALITY DESIGNATION
UD UNDISTURBED
Qu POCKET PENETROMETER UNCONFINED COMPRESSIVE STRENGTH
LL: LIQUID LIMIT
PL: PLASTIC LIMIT
PI: PLASTICITY INDEX
M: NATURAL MOISTURE CONTENT
F: PERCENT PASSING NO. 200 SIEVE

Geotechnical, Environmental, and Materials Engineers

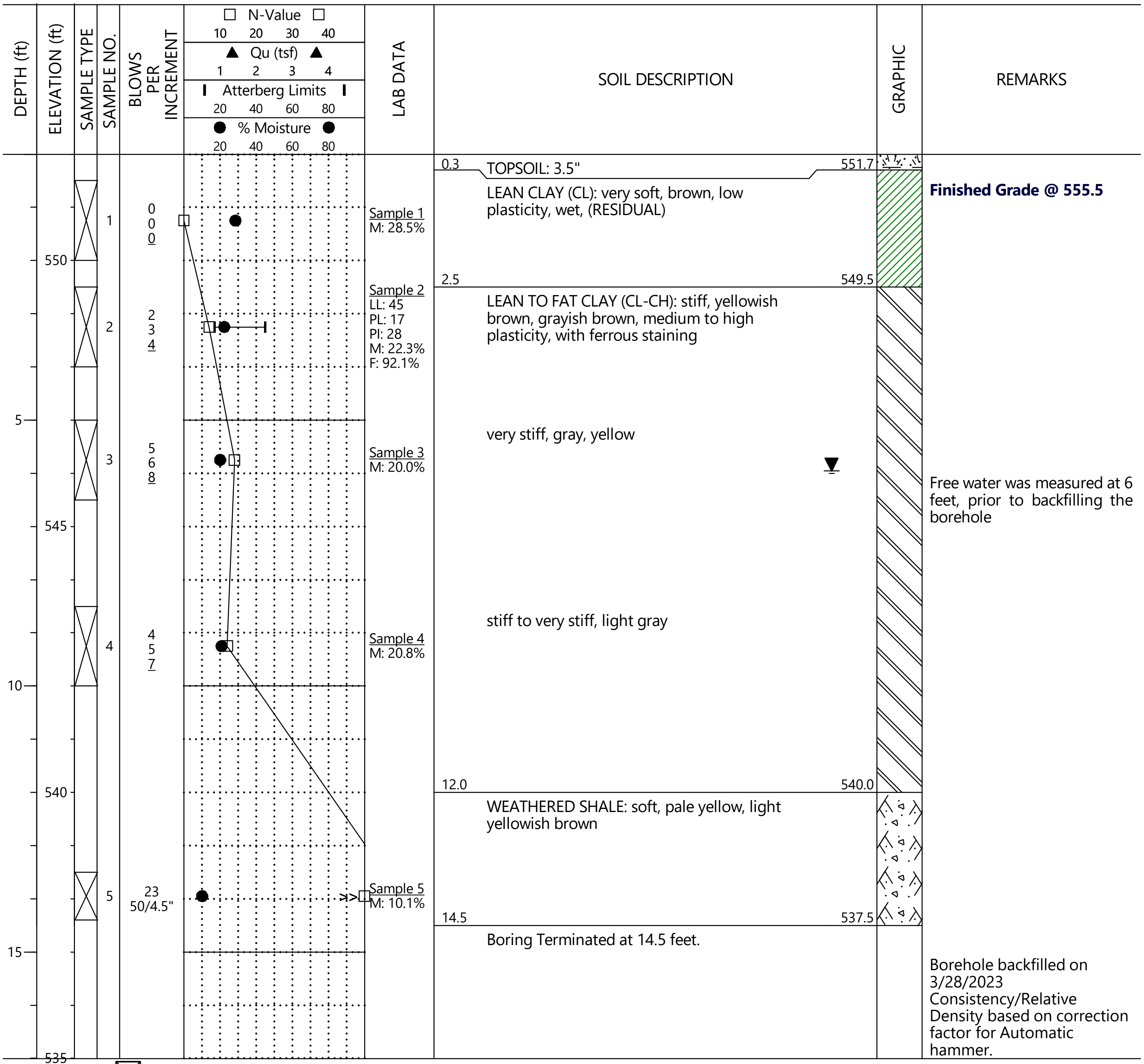
LOG OF BORING

Designation: B-04

Sheet 1 of 1

1403 South 70th East Avenue
Tulsa, OK 74112
Office: (918) 439-9005

PROJECT NAME: Cherokee Nation Housing, Hancock-Muskogee
PROJECT NUMBER: TU230050
DRILLING METHOD: Solid Flight Auger
EQUIPMENT USED: Diedrich D-50 ATV
HAMMER TYPE: Automatic
BORING LOCATION: 35.727197, -95.335422
LOCATION: Muskogee, Oklahoma
DATE DRILLED: 3/27/23
WEATHER: Clear
ELEVATION: 552
DRILL CREW: Building & Earth
LOGGED BY: J. Swyden



SAMPLE NO.	BLOWS PER INCREMENT	LAB DATA
1	0 0 0	Sample 1 M: 28.5%
2	2 3 4	Sample 2 LL: 45 PL: 17 PI: 28 M: 22.3% F: 92.1%
3	5 6 8	Sample 3 M: 20.0%
4	4 5 7	Sample 4 M: 20.8%
5	23 50/4.5"	Sample 5 M: 10.1%

0.3 TOPSOIL: 3.5" 551.7

LEAN CLAY (CL): very soft, brown, low plasticity, wet, (RESIDUAL)

2.5 549.5

LEAN TO FAT CLAY (CL-CH): stiff, yellowish brown, grayish brown, medium to high plasticity, with ferrous staining

very stiff, gray, yellow

stiff to very stiff, light gray

12.0 540.0

WEATHERED SHALE: soft, pale yellow, light yellowish brown

14.5 537.5

Boring Terminated at 14.5 feet.

Finished Grade @ 555.5

Free water was measured at 6 feet, prior to backfilling the borehole

Borehole backfilled on 3/28/2023
Consistency/Relative Density based on correction factor for Automatic hammer.

SAMPLE TYPE ☒ Split Spoon

N-VALUE	STANDARD PENETRATION RESISTANCE (AASHTO T-206)	**REC**	RECOVERY	**LL:** LIQUID LIMIT	**M:** NATURAL MOISTURE CONTENT
% MOISTURE	PERCENT NATURAL MOISTURE CONTENT	**RQD**	ROCK QUALITY DESIGNATION	**PL:** PLASTIC LIMIT	**F:** PERCENT PASSING NO. 200 SIEVE
▽	GROUNDWATER LEVEL IN THE BOREHOLE AT TIME OF DRILLING	**UD**	UNDISTURBED	**PI:** PLASTICITY INDEX	
▼	STABILIZED GROUNDWATER LEVEL	**Qu**	POCKET PENETROMETER UNCONFINED COMPRESSIVE STRENGTH		

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Tulsa, OK 74112
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Geotechnical, Environmental, and Materials Engineers

LOG OF BORING

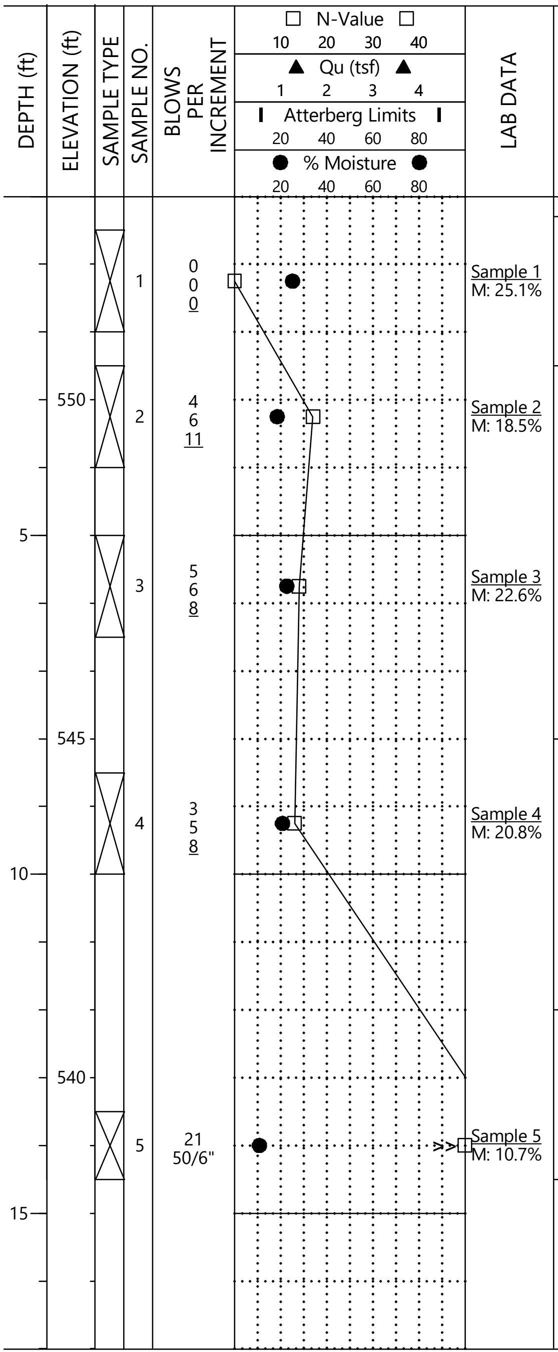
Designation: B-05

Sheet 1 of 1

PROJECT NAME: Cherokee Nation Housing, Hancock-Muskogee
PROJECT NUMBER: TU230050
DRILLING METHOD: Solid Flight Auger
EQUIPMENT USED: Diedrich D-50 ATV
HAMMER TYPE: Automatic
BORING LOCATION: 35.727251, -95.334670

LOCATION: Muskogee, Oklahoma
DATE DRILLED: 3/27/23
WEATHER: Clear
ELEVATION: 553
DRILL CREW: Building & Earth
LOGGED BY: J. Swyden

SAMPLE TYPE	SAMPLE NO.	BLOWS PER INCREMENT	LAB DATA
☒	1	0 0 0	Sample 1 M: 25.1%
☒	2	4 6 11	Sample 2 M: 18.5%
☒	3	5 6 8	Sample 3 M: 22.6%
☒	4	3 5 8	Sample 4 M: 20.8%
☒	5	21 50/6"	Sample 5 M: 10.7%



DEPTH (ft)	SOIL DESCRIPTION	ELEVATION (ft)	REMARKS
0.3	TOPSOIL: 3"	552.7	
2.5	LEAN CLAY (CL): very soft, brown, grayish brown, low plasticity, wet, (RESIDUAL)	550.5	**Finished Grade @ 554.4**
8.0	LEAN TO FAT CLAY (CL-CH): very stiff, yellowish brown, brown, medium to high plasticity, moist, with ferrous staining, (RESIDUAL) light gray, yellow	545.0	
12.0	SANDY LEAN CLAY (CL): very stiff, dark yellowish brown, medium plasticity, moist, with sandstone fragments and ferrous staining, (RESIDUAL) ▼	541.0	Free water was measured at 9 feet, prior to backfilling the borehole
14.5	WEATHERED SHALE: soft, light brown, light yellowish brown, with ferrous staining, (WEATHERED ROCK)	538.5	
	Boring Terminated at 14.5 feet.		Borehole backfilled on 3/28/2023. Consistency/Relative Density based on correction factor for Automatic hammer.

SAMPLE TYPE ☒ Split Spoon

N-VALUE	STANDARD PENETRATION RESISTANCE (AASHTO T-206)	**REC**	RECOVERY	**LL:**	LIQUID LIMIT **M:** NATURAL MOISTURE CONTENT
% MOISTURE	PERCENT NATURAL MOISTURE CONTENT	**RQD**	ROCK QUALITY DESIGNATION	**PL:**	PLASTIC LIMIT **F:** PERCENT PASSING NO. 200 SIEVE
▽	GROUNDWATER LEVEL IN THE BOREHOLE AT TIME OF DRILLING	**UD**	UNDISTURBED	**PI:**	PLASTICITY INDEX
▼	STABILIZED GROUNDWATER LEVEL	**Qu**	POCKET PENETROMETER UNCONFINED COMPRESSIVE STRENGTH		

BUILDING & EARTH
Geotechnical, Environmental, and Materials Engineers

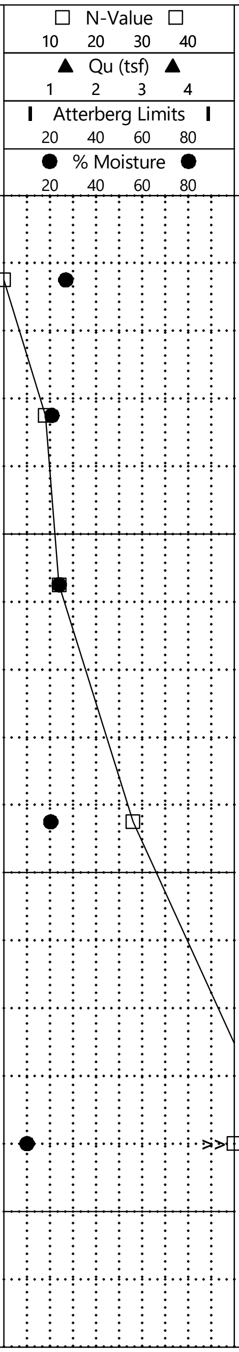
LOG OF BORING

Designation: B-06

Sheet 1 of 1

1403 South 70th East Avenue
Tulsa, OK 74112
Office: (918) 439-9005

PROJECT NAME:	Cherokee Nation Housing, Hancock-Muskogee	LOCATION:	Muskogee, Oklahoma
PROJECT NUMBER:	TU230050	DATE DRILLED:	3/27/23
DRILLING METHOD:	Solid Flight Auger	WEATHER:	Clear
EQUIPMENT USED:	Diedrich D-50 ATV	ELEVATION:	551.5
HAMMER TYPE:	Automatic	DRILL CREW:	Building & Earth
BORING LOCATION:	35.727236, -95.333958	LOGGED BY:	J. Swyden



DEPTH (ft)	ELEVATION (ft)	SAMPLE TYPE	SAMPLE NO.	BLOWS PER INCREMENT	LAB DATA	SOIL DESCRIPTION	REMARKS
0.3	551.2					TOPSOIL: 3"	
		☒	1	0 0 0	Sample 1 M: 26.9%	LEAN CLAY (CL): very soft, brown, yellow, red, low plasticity, wet, (RESIDUAL)	**Finished Grade @ 553.5**
2.5	549.0	☒	2	2 3 6	Sample 2 M: 20.9%	LEAN TO FAT CLAY (CL-CH): stiff, yellowish brown, brown, gray, medium to high plasticity, moist, with ferrous staining, (RESIDUAL)	
5		☒	3	4 5 7	Sample 3 M: 24.0%	stiff to very stiff, gray, grayish brown	
8.5	543.0	☒	4	5 11 17	Sample 4 M: 20.3%	SANDY LEAN CLAY (CL): hard, dark yellowish brown, medium plasticity, moist, with sandstone fragments and ferrous staining, (RESIDUAL) ▼	Free water was measured at 9 feet, prior to backfilling the borehole
12.0	539.5	☒	5	28 50/6"	Sample 5 M: 10.1%	WEATHERED SHALE: soft, light brown, (WEATHERED ROCK)	
14.5	537.0					Boring Terminated at 14.5 feet.	Borehole backfilled on 3/28/2023. Consistency/Relative Density based on correction factor for Automatic hammer.

Depth scale: 5, 10, 15. Elevation scale: 550, 545, 540, 535.

SAMPLE TYPE ☒ Split Spoon

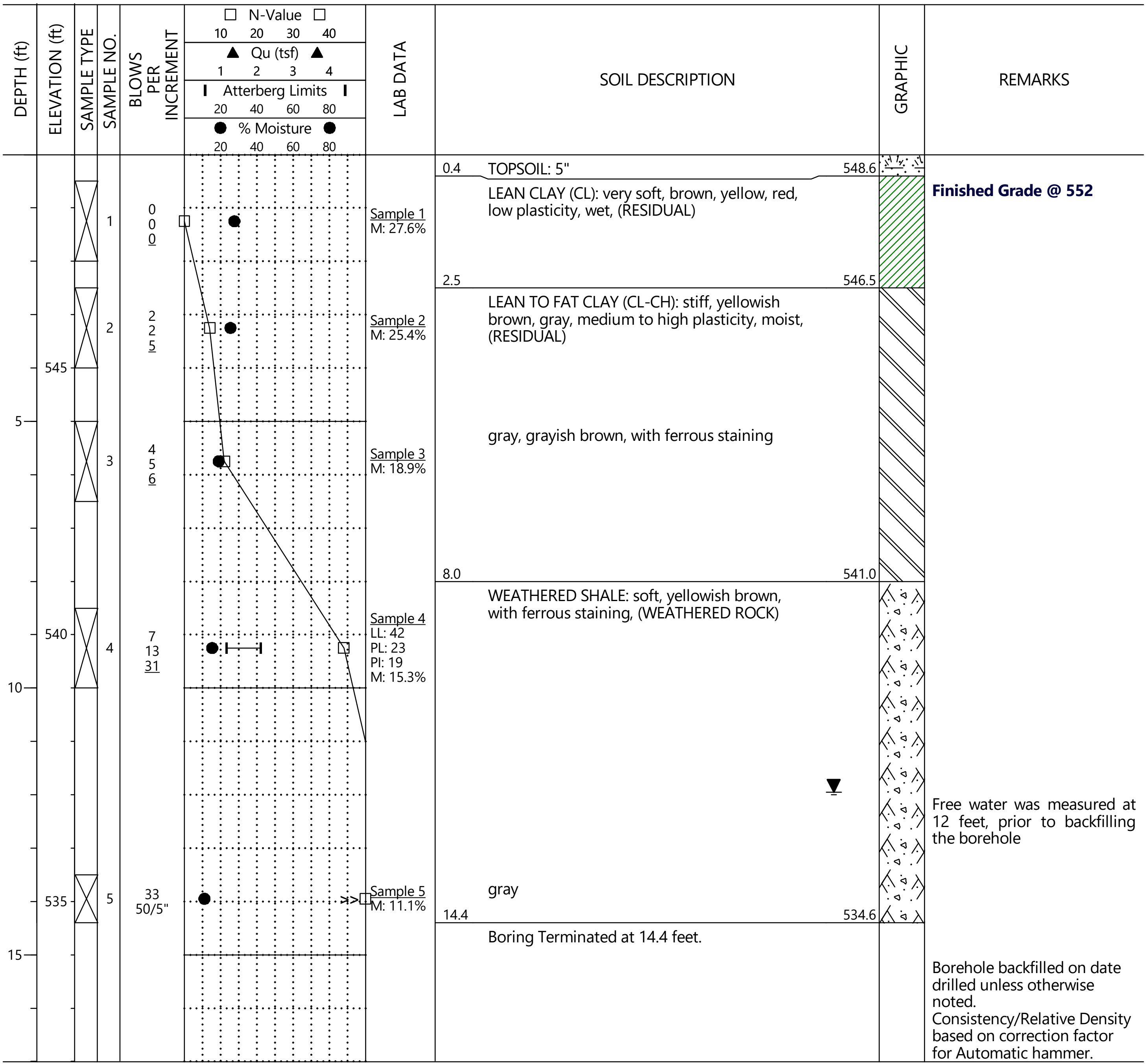
N-VALUE	STANDARD PENETRATION RESISTANCE (AASHTO T-206)	**REC**	RECOVERY	**LL:**	LIQUID LIMIT **M:** NATURAL MOISTURE CONTENT
% MOISTURE	PERCENT NATURAL MOISTURE CONTENT	**RQD**	ROCK QUALITY DESIGNATION	**PL:**	PLASTIC LIMIT **F:** PERCENT PASSING NO. 200 SIEVE
▽	GROUNDWATER LEVEL IN THE BOREHOLE AT TIME OF DRILLING	**UD**	UNDISTURBED	**PI:**	PLASTICITY INDEX
▼	STABILIZED GROUNDWATER LEVEL	**Qu**	POCKET PENETROMETER UNCONFINED COMPRESSIVE STRENGTH		

BUILDING & EARTH
Geotechnical, Environmental, and Materials Engineers

LOG OF BORING
Designation: B-07
Sheet 1 of 1

1403 South 70th East Avenue
Tulsa, OK 74112
Office: (918) 439-9005

PROJECT NAME:	Cherokee Nation Housing, Hancock-Muskogee	LOCATION:	Muskogee, Oklahoma
PROJECT NUMBER:	TU230050	DATE DRILLED:	3/28/23
DRILLING METHOD:	Solid Flight Auger	WEATHER:	Clear
EQUIPMENT USED:	Diedrich D-50 ATV	ELEVATION:	549
HAMMER TYPE:	Automatic	DRILL CREW:	Building & Earth
BORING LOCATION:	35.727246, -95.333209	LOGGED BY:	J. Swyden



Sample No.	Blows per Increment	Lab Data
1	0 0 0	Sample 1 M: 27.6%
2	2 2 5	Sample 2 M: 25.4%
3	4 5 6	Sample 3 M: 18.9%
4	7 13 31	Sample 4 LL: 42 PL: 23 PI: 19 M: 15.3%
5	33 50/5"	Sample 5 M: 11.1%

Depth	Soil Description	Elevation
0.4	TOPSOIL: 5"	548.6
2.5	LEAN CLAY (CL): very soft, brown, yellow, red, low plasticity, wet, (RESIDUAL)	546.5
8.0	LEAN TO FAT CLAY (CL-CH): stiff, yellowish brown, gray, medium to high plasticity, moist, (RESIDUAL) gray, grayish brown, with ferrous staining	541.0
14.4	WEATHERED SHALE: soft, yellowish brown, with ferrous staining, (WEATHERED ROCK) gray	534.6

Boring Terminated at 14.4 feet.

Finished Grade @ 552

Free water was measured at 12 feet, prior to backfilling the borehole

Borehole backfilled on date drilled unless otherwise noted.
Consistency/Relative Density based on correction factor for Automatic hammer.

SAMPLE TYPE ☒ Split Spoon

N-VALUE	STANDARD PENETRATION RESISTANCE (AASHTO T-206)	**REC**	RECOVERY	**LL:**	LIQUID LIMIT
% MOISTURE	PERCENT NATURAL MOISTURE CONTENT	**RQD**	ROCK QUALITY DESIGNATION	**PL:**	PLASTIC LIMIT
∇	GROUNDWATER LEVEL IN THE BOREHOLE AT TIME OF DRILLING	**UD**	UNDISTURBED	**PI:**	PLASTICITY INDEX
▼	STABILIZED GROUNDWATER LEVEL	**Qu**	POCKET PENETROMETER UNCONFINED COMPRESSIVE STRENGTH		

M: NATURAL MOISTURE CONTENT
F: PERCENT PASSING NO. 200 SIEVE

BUILDING & EARTH
Geotechnical, Environmental, and Materials Engineers

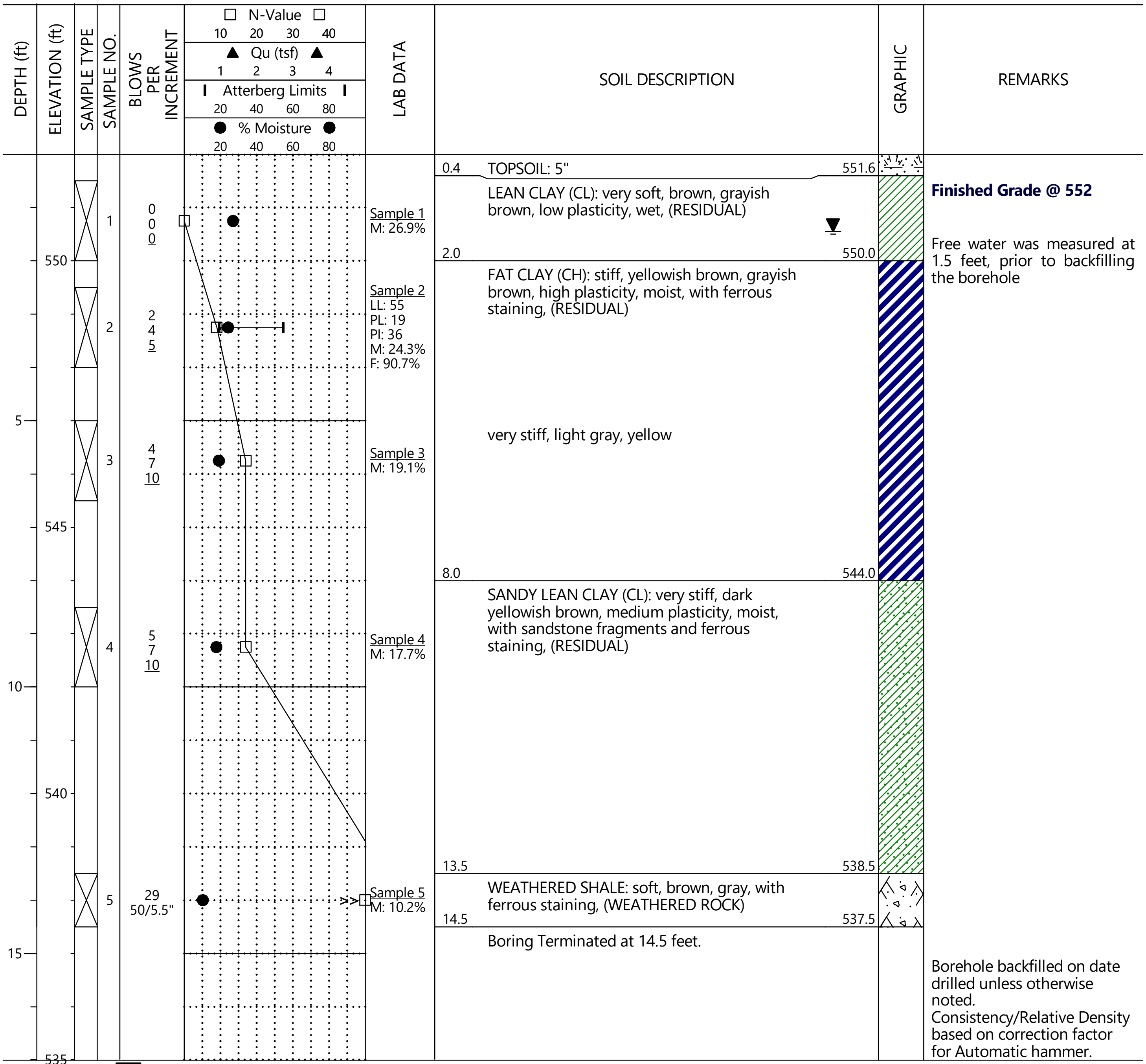
LOG OF BORING

Designation: B-08

Sheet 1 of 1

1403 South 70th East Avenue
Tulsa, OK 74112
Office: (918) 439-9005

PROJECT NAME:	Cherokee Nation Housing, Hancock-Muskogee	LOCATION:	Muskogee, Oklahoma
PROJECT NUMBER:	TU230050	DATE DRILLED:	3/28/23
DRILLING METHOD:	Solid Flight Auger	WEATHER:	Clear
EQUIPMENT USED:	Diedrich D-50 ATV	ELEVATION:	552
HAMMER TYPE:	Automatic	DRILL CREW:	Building & Earth
BORING LOCATION:	35.726923, -95.334310	LOGGED BY:	J. Swyden



SAMPLE NO.	BLOWS PER INCREMENT	LAB DATA
1	0 0 0	Sample 1 M: 26.9%
2	2 4 5	Sample 2 LL: 55 PL: 19 PI: 36 M: 24.3% F: 90.7%
3	4 7 10	Sample 3 M: 19.1%
4	5 7 10	Sample 4 M: 17.7%
5	29 50/5.5"	Sample 5 M: 10.2%

DEPTH (ft)	SOIL DESCRIPTION	ELEVATION (ft)
0.4	TOPSOIL: 5"	551.6
2.0	LEAN CLAY (CL): very soft, brown, grayish brown, low plasticity, wet, (RESIDUAL) ▼	550.0
8.0	FAT CLAY (CH): stiff, yellowish brown, grayish brown, high plasticity, moist, with ferrous staining, (RESIDUAL) very stiff, light gray, yellow	544.0
13.5	SANDY LEAN CLAY (CL): very stiff, dark yellowish brown, medium plasticity, moist, with sandstone fragments and ferrous staining, (RESIDUAL)	538.5
14.5	WEATHERED SHALE: soft, brown, gray, with ferrous staining, (WEATHERED ROCK)	537.5
	Boring Terminated at 14.5 feet.	

REMARKS

Finished Grade @ 552

Free water was measured at 1.5 feet, prior to backfilling the borehole

Borehole backfilled on date drilled unless otherwise noted.
Consistency/Relative Density based on correction factor for Automatic hammer.

SAMPLE TYPE ☒ Split Spoon

N-VALUE	STANDARD PENETRATION RESISTANCE (AASHTO T-206)	**REC**	RECOVERY	**LL:** LIQUID LIMIT	**M:** NATURAL MOISTURE CONTENT
% MOISTURE	PERCENT NATURAL MOISTURE CONTENT	**RQD**	ROCK QUALITY DESIGNATION	**PL:** PLASTIC LIMIT	**F:** PERCENT PASSING NO. 200 SIEVE
▽	GROUNDWATER LEVEL IN THE BOREHOLE AT TIME OF DRILLING	**UD**	UNDISTURBED	**PI:** PLASTICITY INDEX	
▼	STABILIZED GROUNDWATER LEVEL	**Qu**	POCKET PENETROMETER UNCONFINED COMPRESSIVE STRENGTH		

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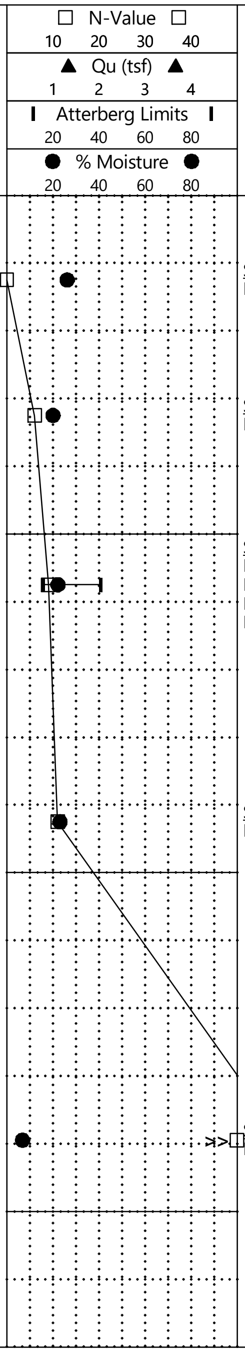
LOG OF BORING

Designation: B-09

Sheet 1 of 1

PROJECT NAME: Cherokee Nation Housing, Hancock-Muskogee
PROJECT NUMBER: TU230050
DRILLING METHOD: Solid Flight Auger
EQUIPMENT USED: Diedrich D-50 ATV
HAMMER TYPE: Automatic
BORING LOCATION: 35.726586, -95.335400

LOCATION: Muskogee, Oklahoma
DATE DRILLED: 3/27/23
WEATHER: Clear
ELEVATION: 550
DRILL CREW: Building & Earth
LOGGED BY: J. Swyden



DEPTH (ft)	ELEVATION (ft)	SAMPLE TYPE	SAMPLE NO.	BLOWS PER INCREMENT	LAB DATA	SOIL DESCRIPTION	REMARKS
	550					0.4 TOPSOIL: 5" 549.6	
		☒	1	0 0 0	Sample 1 M: 26.2%	LEAN CLAY (CL): very soft, brown, low plasticity, wet, (RESIDUAL)	**Finished Grade @ 549**
		☒	2	2 3 3	Sample 2 M: 20.0%	2.0 548.0 LEAN TO FAT CLAY (CL-CH): medium stiff to stiff, light brown, pale yellow, medium to high plasticity, moist, with ferrous staining, (RESIDUAL)	
5	545	☒	3	3 4 5	Sample 3 LL: 41 PL: 16 PI: 25 M: 22.1%	5.0 ▼ ▽545.0 LEAN CLAY (CL): stiff, light gray, olive yellow, medium to high plasticity, moist, with ferrous staining, (RESIDUAL)	Groundwater encountered at 5 feet (EL 545) at time of drilling and stabilized at 4.5 feet (EL 545.5).
		☒	4	3 4 7	Sample 4 M: 23.0%	8.0 542.0 SANDY LEAN CLAY (CL): stiff, yellowish brown, medium plasticity, moist, with sandstone fragments, (RESIDUAL)	
10	540						
		☒	5	30 50/5"	Sample 5 M: 6.8%	12.0 538.0 WEATHERED SHALE: soft, brown, light gray, with sandstone lenses, (WEATHERED ROCK)	
						14.4 535.6 Boring Terminated at 14.4 feet.	
15	535						Borehole backfilled on 3/28/2023. Consistency/Relative Density based on correction factor for Automatic hammer.

SAMPLE TYPE ☒ Split Spoon

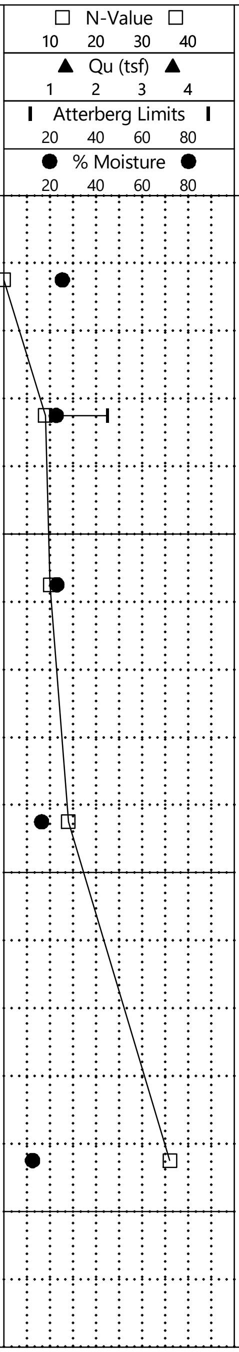
N-VALUE	STANDARD PENETRATION RESISTANCE (AASHTO T-206)	**REC**	RECOVERY	**LL:** LIQUID LIMIT	**M:** NATURAL MOISTURE CONTENT
% MOISTURE	PERCENT NATURAL MOISTURE CONTENT	**RQD**	ROCK QUALITY DESIGNATION	**PL:** PLASTIC LIMIT	**F:** PERCENT PASSING NO. 200 SIEVE
▽	GROUNDWATER LEVEL IN THE BOREHOLE AT TIME OF DRILLING	**UD**	UNDISTURBED	**PI:** PLASTICITY INDEX	
▼	STABILIZED GROUNDWATER LEVEL	**Qu**	POCKET PENETROMETER UNCONFINED COMPRESSIVE STRENGTH		

LOG OF BORING

Designation: B-10

Sheet 1 of 1

PROJECT NAME:	Cherokee Nation Housing, Hancock-Muskogee	LOCATION:	Muskogee, Oklahoma
PROJECT NUMBER:	TU230050	DATE DRILLED:	3/27/23
DRILLING METHOD:	Solid Flight Auger	WEATHER:	Clear
EQUIPMENT USED:	Diedrich D-50 ATV	ELEVATION:	551
HAMMER TYPE:	Automatic	DRILL CREW:	Building & Earth
BORING LOCATION:	35.726459, -95.334663	LOGGED BY:	J. Swyden



SAMPLE TYPE	SAMPLE NO.	BLOWS PER INCREMENT	LAB DATA
☒	1	0 0 0	Sample 1 M: 25.4%
☒	2	2 4 5	Sample 2 LL: 45 PL: 20 PI: 25 M: 22.8%
☒	3	4 4 6	Sample 3 M: 23.1%
☒	4	3 5 9	Sample 4 M: 16.4%
☒	5	10 18 18	Sample 5 M: 12.5%

DEPTH (ft)	SOIL DESCRIPTION	ELEVATION (ft)
0.4	TOPSOIL: 5"	550.6
2.5	LEAN CLAY (CL): very soft, brown, grayish brown, low plasticity, wet, (RESIDUAL) ▼	548.5
8.0	LEAN TO FAT CLAY (CL-CH): stiff, yellowish brown, gray, medium to high plasticity, moist, with ferrous staining, (RESIDUAL) stiff, mottled red	543.0
13.0	SANDY LEAN CLAY (CL): very stiff, yellowish brown, medium plasticity, moist, with sandstone fragments, (RESIDUAL)	538.0
15.0	WEATHERED SHALE: soft, yellowish brown, light gray, with ferrous staining, (WEATHERED ROCK)	536.0
	Boring Terminated at 15 feet.	

REMARKS

Finished Grade @ 552.75
Organic Content = 3.4%
Free water was measured at 1 feet, prior to backfilling the borehole

Borehole backfilled on 3/28/2023.
Consistency/Relative Density based on correction factor for Automatic hammer.

SAMPLE TYPE ☒ Split Spoon

N-VALUE	STANDARD PENETRATION RESISTANCE (AASHTO T-206)	**REC**	RECOVERY	**LL:** LIQUID LIMIT	**M:** NATURAL MOISTURE CONTENT
% MOISTURE	PERCENT NATURAL MOISTURE CONTENT	**RQD**	ROCK QUALITY DESIGNATION	**PL:** PLASTIC LIMIT	**F:** PERCENT PASSING NO. 200 SIEVE
▽	GROUNDWATER LEVEL IN THE BOREHOLE AT TIME OF DRILLING	**UD**	UNDISTURBED	**PI:** PLASTICITY INDEX	
▼	STABILIZED GROUNDWATER LEVEL	**Qu**	POCKET PENETROMETER UNCONFINED COMPRESSIVE STRENGTH		

BUILDING & EARTH
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LOG OF BORING

Designation: B-11

Sheet 1 of 1

1403 South 70th East Avenue
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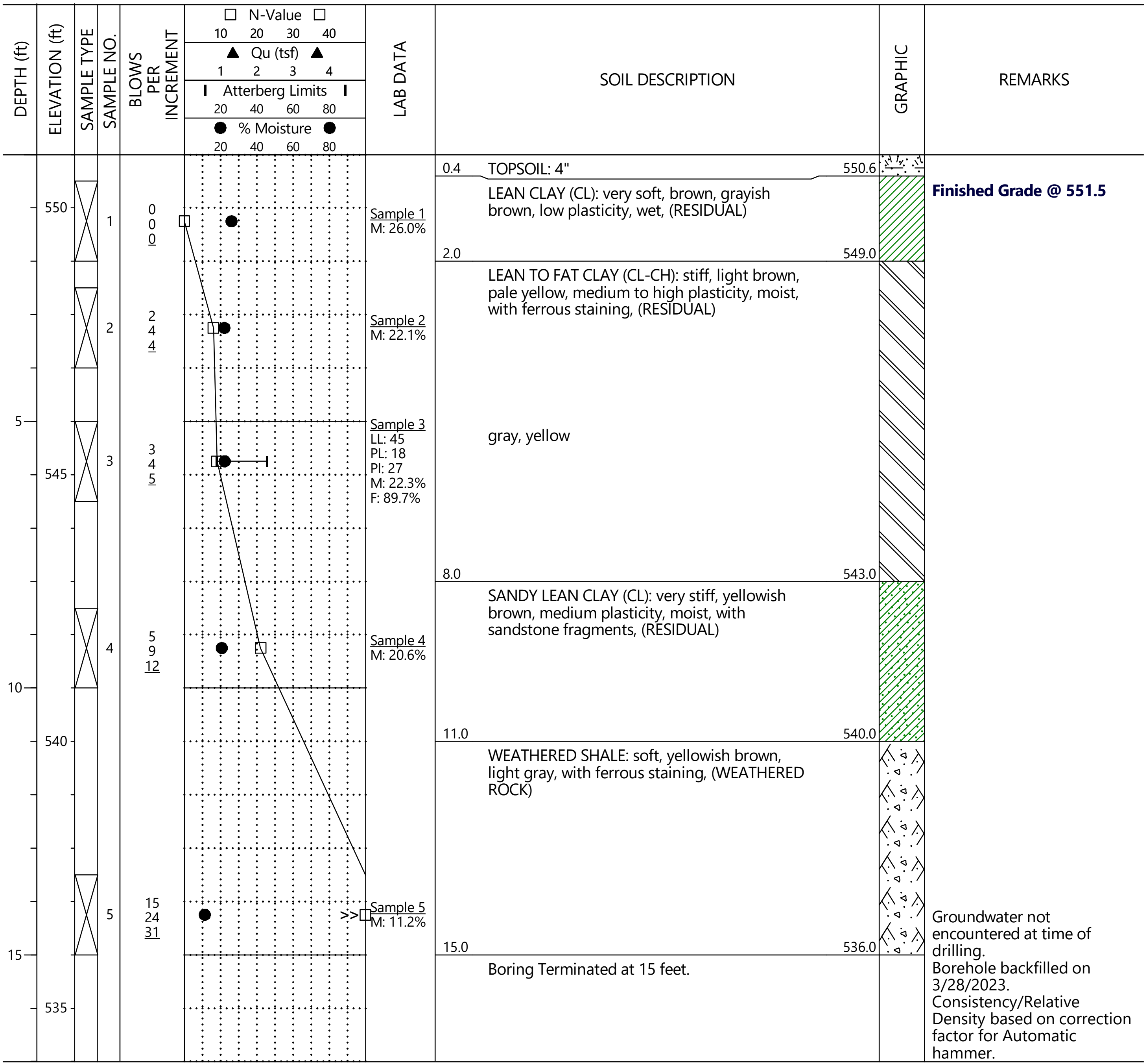
Office: (918) 439-9005

PROJECT NAME: Cherokee Nation Housing, Hancock-Muskogee
PROJECT NUMBER: TU230050
DRILLING METHOD: Solid Flight Auger
EQUIPMENT USED: Diedrich D-50 ATV
HAMMER TYPE: Automatic
BORING LOCATION: 35.726450, -95.333965

LOCATION: Muskogee, Oklahoma
DATE DRILLED: 3/27/23
WEATHER: Clear
ELEVATION: 551
DRILL CREW: Building & Earth
LOGGED BY: J. Swyden



DEPTH (ft)	ELEVATION (ft)	SAMPLE NO.	BLOWS PER INCREMENT	LAB DATA	SOIL DESCRIPTION	REMARKS
0.4	550.6				TOPSOIL: 4"	
2.0	549.0	1	0 0 0	Sample 1 M: 26.0%	LEAN CLAY (CL): very soft, brown, grayish brown, low plasticity, wet, (RESIDUAL)	**Finished Grade @ 551.5**
8.0	543.0	2	2 4 4	Sample 2 M: 22.1%	LEAN TO FAT CLAY (CL-CH): stiff, light brown, pale yellow, medium to high plasticity, moist, with ferrous staining, (RESIDUAL)	
		3	3 4 5	Sample 3 LL: 45 PL: 18 PI: 27 M: 22.3% F: 89.7%	gray, yellow	
11.0	540.0	4	5 9 12	Sample 4 M: 20.6%	SANDY LEAN CLAY (CL): very stiff, yellowish brown, medium plasticity, moist, with sandstone fragments, (RESIDUAL)	
15.0	536.0	5	15 24 31	Sample 5 M: 11.2%	WEATHERED SHALE: soft, yellowish brown, light gray, with ferrous staining, (WEATHERED ROCK)	Groundwater not encountered at time of drilling. Borehole backfilled on 3/28/2023. Consistency/Relative Density based on correction factor for Automatic hammer.
					Boring Terminated at 15 feet.	

SAMPLE TYPE ☒ Split Spoon

N-VALUE STANDARD PENETRATION RESISTANCE (AASHTO T-206)
% MOISTURE PERCENT NATURAL MOISTURE CONTENT
▽ GROUNDWATER LEVEL IN THE BOREHOLE AT TIME OF DRILLING
▼ STABILIZED GROUNDWATER LEVEL
REC RECOVERY
RQD ROCK QUALITY DESIGNATION
UD UNDISTURBED
Qu POCKET PENETROMETER UNCONFINED COMPRESSIVE STRENGTH
LL: LIQUID LIMIT
PL: PLASTIC LIMIT
PI: PLASTICITY INDEX
M: NATURAL MOISTURE CONTENT
F: PERCENT PASSING NO. 200 SIEVE

Geotechnical, Environmental, and Materials Engineers

LOG OF BORING

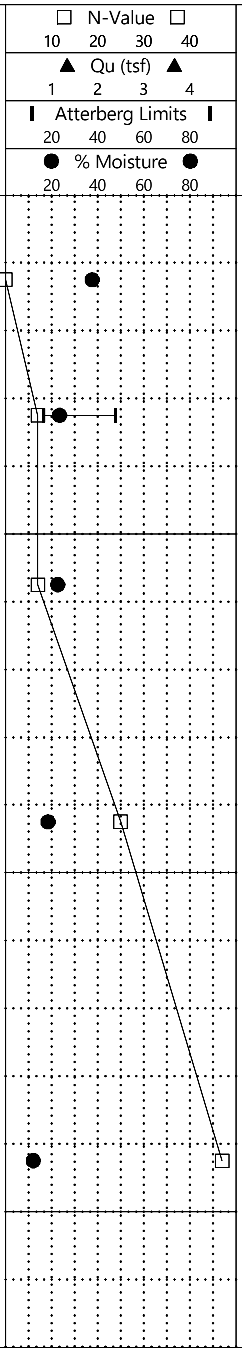
Designation: B-12

Sheet 1 of 1

1403 South 70th East Avenue
Tulsa, OK 74112
Office: (918) 439-9005

PROJECT NAME: Cherokee Nation Housing, Hancock-Muskogee
PROJECT NUMBER: TU230050
DRILLING METHOD: Solid Flight Auger
EQUIPMENT USED: Diedrich D-50 ATV
HAMMER TYPE: Automatic
BORING LOCATION: 35.726564, -95.333213

LOCATION: Muskogee, Oklahoma
DATE DRILLED: 3/28/23
WEATHER: Clear
ELEVATION: 547.5
DRILL CREW: Building & Earth
LOGGED BY: J. Swyden



DEPTH (ft)	ELEVATION (ft)	SAMPLE TYPE	SAMPLE NO.	BLOWS PER INCREMENT	LAB DATA	SOIL DESCRIPTION	REMARKS
0.4	547.1					TOPSOIL: 4"	
2.5	545.0	☒	1	0 0 0	Sample 1 M: 37.6%	LEAN CLAY (CL): very soft, brown, grayish brown, low plasticity, moist, (RESIDUAL)	
		☒	2	2 3 4	Sample 2 LL: 47 PL: 16 PI: 31 M: 23.4%	LEAN TO FAT CLAY (CL-CH): stiff, yellowish brown, gray, high plasticity, moist, (RESIDUAL)	
8.0	539.5	☒	3	2 3 4	Sample 3 M: 22.6%	with ferrous staining	**Finished Grade @ 542**
11.0	536.5	☒	4	7 10 15	Sample 4 M: 18.4%	SANDY LEAN CLAY (CL): hard, yellowish brown, medium plasticity, moist, with sandstone fragments, (RESIDUAL)	
15.0	532.5	☒	5	17 22 25	Sample 5 M: 12.0%	WEATHERED SHALE: soft, yellowish brown, light gray, with ferrous staining, (WEATHERED ROCK)	Groundwater not encountered at time of drilling. Borehole backfilled on date drilled unless otherwise noted. Consistency/Relative Density based on correction factor for Automatic hammer.
						Boring Terminated at 15 feet.	

SAMPLE TYPE ☒ Split Spoon

N-VALUE STANDARD PENETRATION RESISTANCE (AASHTO T-206)
% MOISTURE PERCENT NATURAL MOISTURE CONTENT
∇ GROUNDWATER LEVEL IN THE BOREHOLE AT TIME OF DRILLING
▼ STABILIZED GROUNDWATER LEVEL

REC RECOVERY
RQD ROCK QUALITY DESIGNATION
UD UNDISTURBED
Qu POCKET PENETROMETER UNCONFINED COMPRESSIVE STRENGTH

LL: LIQUID LIMIT
PL: PLASTIC LIMIT
PI: PLASTICITY INDEX
M: NATURAL MOISTURE CONTENT
F: PERCENT PASSING NO. 200 SIEVE

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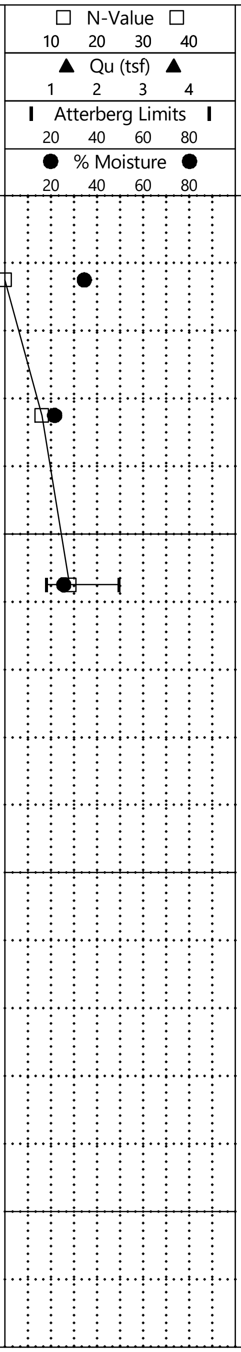
LOG OF BORING
Designation: P-01
Sheet 1 of 1

1403 South 70th East Avenue
Tulsa, OK 74112
Office: (918) 439-9005

PROJECT NAME:	Cherokee Nation Housing, Hancock-Muskogee	LOCATION:	Muskogee, Oklahoma
PROJECT NUMBER:	TU230050	DATE DRILLED:	3/28/23
DRILLING METHOD:	Solid Flight Auger	WEATHER:	Clear
EQUIPMENT USED:	Diedrich D-50 ATV	ELEVATION:	554
HAMMER TYPE:	Automatic	DRILL CREW:	Building & Earth
BORING LOCATION:	35.727533, -95.335129	LOGGED BY:	J. Swyden



DEPTH (ft)	ELEVATION (ft)	SAMPLE TYPE	SAMPLE NO.	BLOWS PER INCREMENT	LAB DATA	SOIL DESCRIPTION	REMARKS
0.4	553.6					TOPSOIL: 5"	**Finished Grade @ 554**
		☒	1	0 0 0	Sample 1 M: 34.5%	SILTY CLAY (CL-ML): very soft, brown, low plasticity, wet, (RESIDUAL)	
2.5	551.5						
		☒	2	2 4 4	Sample 2 M: 21.6%	LEAN TO FAT CLAY (CL-CH): stiff, yellowish brown, gray, high plasticity, moist, with ferrous staining, (RESIDUAL)	
5	550						
		☒	3	4 6 8	Sample 3 LL: 49 PL: 18 PI: 31 M: 25.5%	very stiff	
6.5	547.5					Boring Terminated at 6.5 feet.	
10	545						
15	540						Groundwater not encountered at time of drilling. Borehole backfilled on date drilled unless otherwise noted. Consistency/Relative Density based on correction factor for Automatic hammer.

SAMPLE TYPE ☒ Split Spoon

N-VALUE	STANDARD PENETRATION RESISTANCE (AASHTO T-206)	**REC**	RECOVERY	**LL:** LIQUID LIMIT	**M:** NATURAL MOISTURE CONTENT
% MOISTURE	PERCENT NATURAL MOISTURE CONTENT	**RQD**	ROCK QUALITY DESIGNATION	**PL:** PLASTIC LIMIT	**F:** PERCENT PASSING NO. 200 SIEVE
▽	GROUNDWATER LEVEL IN THE BOREHOLE AT TIME OF DRILLING	**UD**	UNDISTURBED	**PI:** PLASTICITY INDEX	
▼	STABILIZED GROUNDWATER LEVEL	**Qu**	POCKET PENETROMETER UNCONFINED COMPRESSIVE STRENGTH		

BUILDING & EARTH
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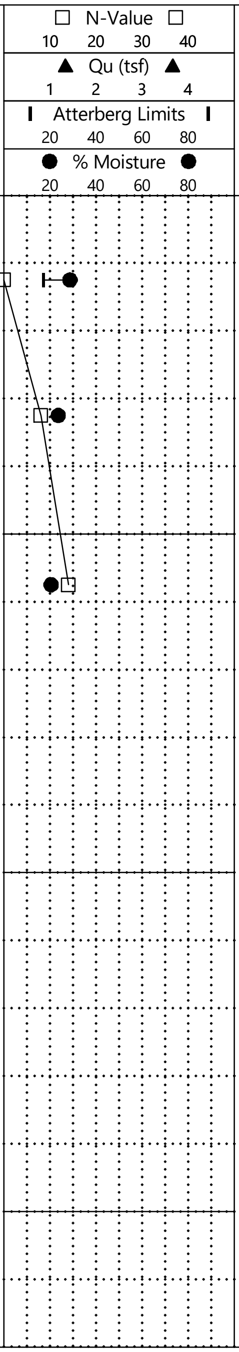
LOG OF BORING
Designation: P-02
Sheet 1 of 1

1403 South 70th East Avenue
Tulsa, OK 74112
Office: (918) 439-9005

PROJECT NAME:	Cherokee Nation Housing, Hancock-Muskogee	LOCATION:	Muskogee, Oklahoma
PROJECT NUMBER:	TU230050	DATE DRILLED:	3/28/23
DRILLING METHOD:	Solid Flight Auger	WEATHER:	Clear
EQUIPMENT USED:	Diedrich D-50 ATV	ELEVATION:	550.5
HAMMER TYPE:	Automatic	DRILL CREW:	Building & Earth
BORING LOCATION:	35.727511, -95.333494	LOGGED BY:	J. Swyden

DEPTH (ft)	ELEVATION (ft)	SAMPLE TYPE	SAMPLE NO.	BLOWS PER INCREMENT	LAB DATA	SOIL DESCRIPTION	REMARKS
0.4	550.1					TOPSOIL: 5"	
		☒	1	0 0 0	Sample 1 LL: 28 PL: 17 PI: 11 M: 28.7%	LEAN CLAY (CL): very soft, brown, low plasticity, wet, (RESIDUAL)	**Finished Grade @ 551**
2.5	548.0	☒	2	2 4 4	Sample 2 M: 23.6%	LEAN TO FAT CLAY (CL-CH): stiff, yellowish brown, grayish brown, medium to high plasticity, moist, (RESIDUAL)	
		☒	3	4 6 8	Sample 3 M: 20.5%	very stiff, yellow	
6.5	544.0					Boring Terminated at 6.5 feet.	



Groundwater not encountered at time of drilling.
Borehole backfilled on date drilled unless otherwise noted.
Consistency/Relative Density based on correction factor for Automatic hammer.

SAMPLE TYPE ☒ Split Spoon

N-VALUE STANDARD PENETRATION RESISTANCE (AASHTO T-206)
% MOISTURE PERCENT NATURAL MOISTURE CONTENT
▽ GROUNDWATER LEVEL IN THE BOREHOLE AT TIME OF DRILLING
▼ STABILIZED GROUNDWATER LEVEL
REC RECOVERY
RQD ROCK QUALITY DESIGNATION
UD UNDISTURBED
Qu POCKET PENETROMETER UNCONFINED COMPRESSIVE STRENGTH
LL: LIQUID LIMIT
PL: PLASTIC LIMIT
PI: PLASTICITY INDEX
M: NATURAL MOISTURE CONTENT
F: PERCENT PASSING NO. 200 SIEVE

BUILDING & EARTH
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LOG OF BORING

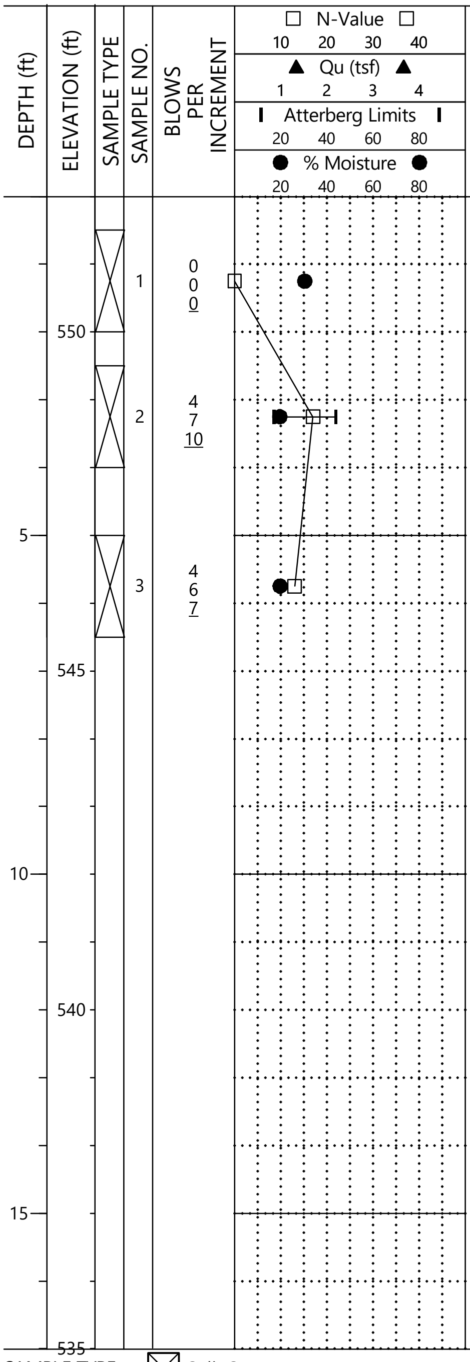
Designation: P-03

Sheet 1 of 1

1403 South 70th East Avenue
Tulsa, OK 74112
Office: (918) 439-9005

PROJECT NAME: Cherokee Nation Housing, Hancock-Muskogee
PROJECT NUMBER: TU230050
DRILLING METHOD: Solid Flight Auger
EQUIPMENT USED: Diedrich D-50 ATV
HAMMER TYPE: Automatic
BORING LOCATION: 35.726908, -95.335115

LOCATION: Muskogee, Oklahoma
DATE DRILLED: 3/27/23
WEATHER: Clear
ELEVATION: 552
DRILL CREW: Building & Earth
LOGGED BY: J. Swyden



DEPTH (ft)	ELEVATION (ft)	SAMPLE NO.	BLOWS PER INCREMENT	LAB DATA	SOIL DESCRIPTION	REMARKS
0.4	551.6				TOPSOIL: 5"	
		1	0 0 0	Sample 1 M: 30.4%	LEAN CLAY (CL): very soft, brown, low plasticity, wet, (RESIDUAL)	**Finished Grade @ 552**
2.5	549.5	2	4 7 10	Sample 2 LL: 44 PL: 17 PI: 27 M: 19.6% F: 89.5%	LEAN CLAY (CL): very stiff, grayish brown, medium to high plasticity, moist, with ferrous staining, (RESIDUAL)	
		3	4 6 7	Sample 3 M: 19.7%	yellowish brown	
6.5	545.5				Boring Terminated at 6.5 feet.	

Groundwater not encountered at time of drilling.
Borehole backfilled on date drilled unless otherwise noted.
Consistency/Relative Density based on correction factor for Automatic hammer.

SAMPLE TYPE ☒ Split Spoon

N-VALUE STANDARD PENETRATION RESISTANCE (AASHTO T-206)
% MOISTURE PERCENT NATURAL MOISTURE CONTENT
▽ GROUNDWATER LEVEL IN THE BOREHOLE AT TIME OF DRILLING
▼ STABILIZED GROUNDWATER LEVEL

REC RECOVERY
RQD ROCK QUALITY DESIGNATION
UD UNDISTURBED
Qu POCKET PENETROMETER UNCONFINED COMPRESSIVE STRENGTH

LL: LIQUID LIMIT
PL: PLASTIC LIMIT
PI: PLASTICITY INDEX

M: NATURAL MOISTURE CONTENT
F: PERCENT PASSING NO. 200 SIEVE

BUILDING & EARTH
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LOG OF BORING

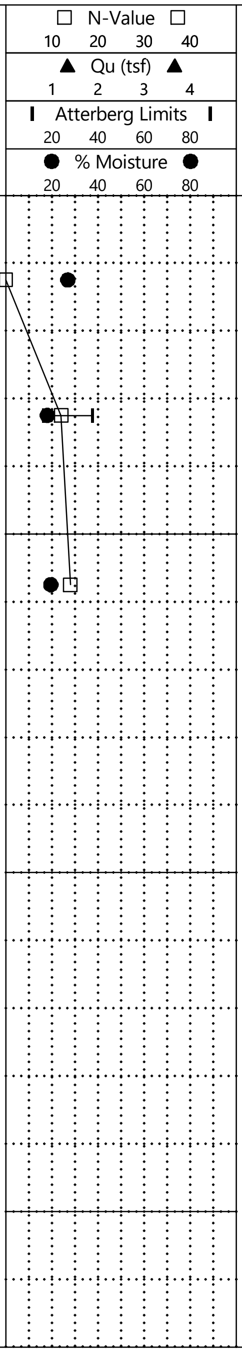
Designation: P-04

Sheet 1 of 1

1403 South 70th East Avenue
Tulsa, OK 74112
Office: (918) 439-9005

PROJECT NAME: Cherokee Nation Housing, Hancock-Muskogee
PROJECT NUMBER: TU230050
DRILLING METHOD: Solid Flight Auger
EQUIPMENT USED: Diedrich D-50 ATV
HAMMER TYPE: Automatic
BORING LOCATION: 35.726929, -95.333500
LOCATION: Muskogee, Oklahoma
DATE DRILLED: 3/28/23
WEATHER: Clear
ELEVATION: 550
DRILL CREW: Building & Earth
LOGGED BY: J. Swyden

DEPTH (ft)	ELEVATION (ft)	SAMPLE TYPE	SAMPLE NO.	BLOWS PER INCREMENT	LAB DATA	SOIL DESCRIPTION	REMARKS
0	550						
0.4	549.6					TOPSOIL: 5"	
						LEAN CLAY (CL): very soft, brown, grayish brown, low plasticity, wet, (RESIDUAL)	Finished Grade @ 549.5
		☒	1	0 0 0	Sample 1 M: 26.9%		
2.0	548.0					LEAN CLAY (CL): very stiff, grayish brown, medium plasticity, moist, with ferrous staining, (RESIDUAL)	
		☒	2	3 5 7	Sample 2 LL: 37 PL: 16 PI: 21 M: 18.0%		
5	545					yellowish brown, medium to high plasticity	
		☒	3	5 6 8	Sample 3 M: 19.5%		
6.5	543.5					Boring Terminated at 6.5 feet.	
10	540						
15	535						Groundwater not encountered at time of drilling. Borehole backfilled on date drilled unless otherwise noted. Consistency/Relative Density based on correction factor for Automatic hammer.



SAMPLE TYPE ☒ Split Spoon

N-VALUE STANDARD PENETRATION RESISTANCE (AASHTO T-206)
% MOISTURE PERCENT NATURAL MOISTURE CONTENT
∇ GROUNDWATER LEVEL IN THE BOREHOLE AT TIME OF DRILLING
▼ STABILIZED GROUNDWATER LEVEL
REC RECOVERY
RQD ROCK QUALITY DESIGNATION
UD UNDISTURBED
Qu POCKET PENETROMETER UNCONFINED COMPRESSIVE STRENGTH
LL: LIQUID LIMIT
PL: PLASTIC LIMIT
PI: PLASTICITY INDEX
M: NATURAL MOISTURE CONTENT
F: PERCENT PASSING NO. 200 SIEVE

Geotechnical, Environmental, and Materials Engineers

LOG OF BORING

Designation: P-05

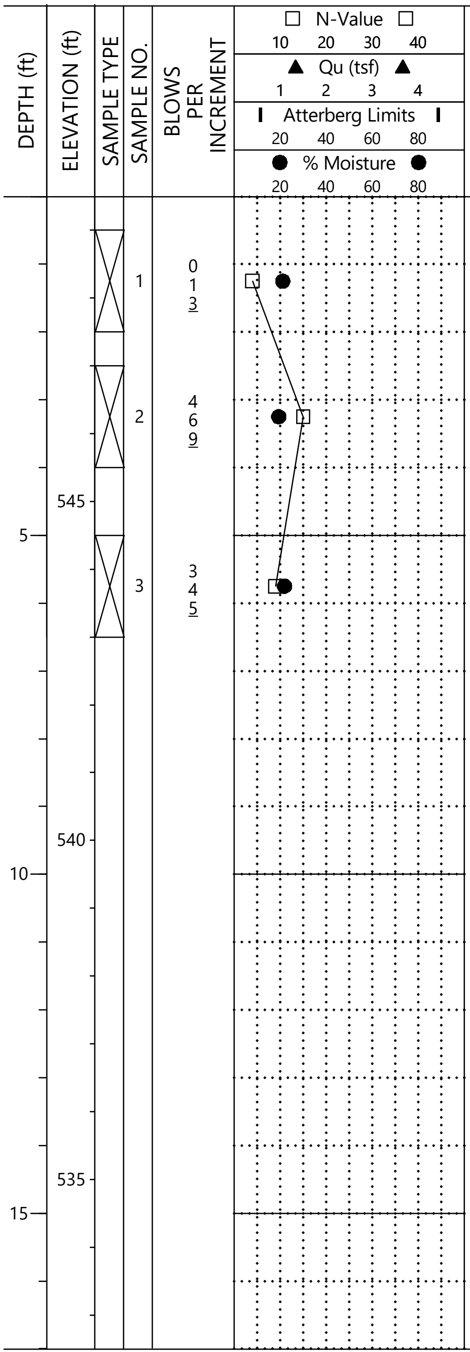
Sheet 1 of 1

1403 South 70th East Avenue
Tulsa, OK 74112
Office: (918) 439-9005

PROJECT NAME: Cherokee Nation Housing, Hancock-Muskogee
PROJECT NUMBER: TU230050
DRILLING METHOD: Solid Flight Auger
EQUIPMENT USED: Diedrich D-50 ATV
HAMMER TYPE: Automatic
BORING LOCATION: 35.726244, -95.335128

LOCATION: Muskogee, Oklahoma
DATE DRILLED: 3/27/23
WEATHER: Clear
ELEVATION: 549.5
DRILL CREW: Building & Earth
LOGGED BY: J. Swyden

DEPTH (ft)	ELEVATION (ft)	SAMPLE TYPE	SAMPLE NO.	BLOWS PER INCREMENT	LAB DATA	SOIL DESCRIPTION	GRAPHIC	REMARKS
0.4	549.1					TOPSOIL: 5"		
1.5	548.0	☒	1	0 1 3	Sample 1 M: 21.1%	LEAN CLAY (CL): very soft to soft, brown, low plasticity, moist, (RESIDUAL)		**Finished Grade @ 550**
		☒	2	4 6 9	Sample 2 M: 19.3%	LEAN TO FAT CLAY (CL-CH): medium stiff, yellowish brown, grayish brown, medium to high plasticity, moist, (RESIDUAL) very stiff, pale yellow, gray, with ferrous staining		
6.5	543.0	☒	3	3 4 5	Sample 3 M: 21.8%	stiff		
						Boring Terminated at 6.5 feet.		Groundwater not encountered at time of drilling. Borehole backfilled on date drilled unless otherwise noted. Consistency/Relative Density based on correction factor for Automatic hammer.



SAMPLE TYPE ☒ Split Spoon

N-VALUE STANDARD PENETRATION RESISTANCE (AASHTO T-206)
% MOISTURE PERCENT NATURAL MOISTURE CONTENT
▽ GROUNDWATER LEVEL IN THE BOREHOLE AT TIME OF DRILLING
▼ STABILIZED GROUNDWATER LEVEL
REC RECOVERY
RQD ROCK QUALITY DESIGNATION
UD UNDISTURBED
Qu POCKET PENETROMETER UNCONFINED COMPRESSIVE STRENGTH
LL: LIQUID LIMIT
PL: PLASTIC LIMIT
PI: PLASTICITY INDEX
M: NATURAL MOISTURE CONTENT
F: PERCENT PASSING NO. 200 SIEVE

Geotechnical, Environmental, and Materials Engineers

LOG OF BORING

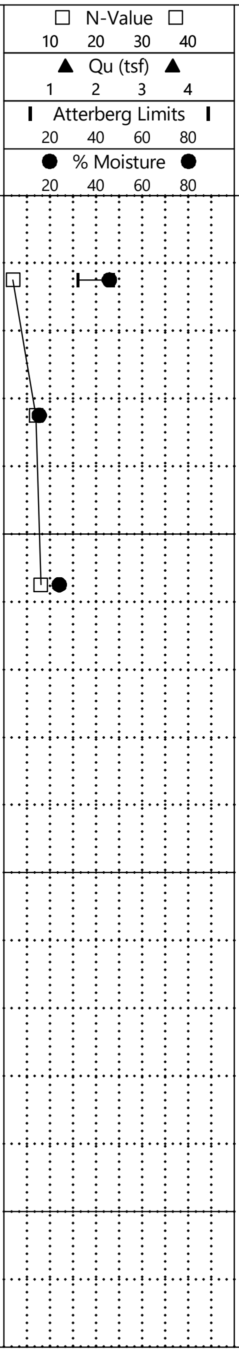
Designation: P-06

Sheet 1 of 1

1403 South 70th East Avenue
Tulsa, OK 74112
Office: (918) 439-9005

PROJECT NAME: Cherokee Nation Housing, Hancock-Muskogee
PROJECT NUMBER: TU230050
DRILLING METHOD: Solid Flight Auger
EQUIPMENT USED: Diedrich D-50 ATV
HAMMER TYPE: Automatic
BORING LOCATION: 35.726234, -95.333503

LOCATION: Muskogee, Oklahoma
DATE DRILLED: 3/28/23
WEATHER: Clear
ELEVATION: 548
DRILL CREW: Building & Earth
LOGGED BY: J. Swyden



DEPTH (ft)	ELEVATION (ft)	SAMPLE TYPE	SAMPLE NO.	BLOWS PER INCREMENT	LAB DATA	SOIL DESCRIPTION	GRAPHIC	REMARKS
0.5	547.5					TOPSOIL: 6"		
2.5	545.5	☒	1	1 1 1	Sample 1 LL: 47 PL: 32 PI: 15 M: 45.7%	LEAN CLAY (CL): soft, dark brown, medium plasticity, moist, with roots, (POSSIBLE FILL)		**Finished Grade @ 547**
		☒	2	2 3 4	Sample 2 M: 15.3%	LEAN TO FAT CLAY (CL-CH): stiff, yellowish brown, (RESIDUAL)		
6.5	541.5	☒	3	3 3 5	Sample 3 M: 24.0%	light gray, with ferrous staining		
						Boring Terminated at 6.5 feet.		Groundwater not encountered at time of drilling. Borehole backfilled on date drilled unless otherwise noted. Consistency/Relative Density based on correction factor for Automatic hammer.

Depth scale: 5, 10, 15. Elevation scale: 545, 540, 535.

SAMPLE TYPE ☒ Split Spoon

N-VALUE STANDARD PENETRATION RESISTANCE (AASHTO T-206)
% MOISTURE PERCENT NATURAL MOISTURE CONTENT
▽ GROUNDWATER LEVEL IN THE BOREHOLE AT TIME OF DRILLING
▼ STABILIZED GROUNDWATER LEVEL

REC RECOVERY
RQD ROCK QUALITY DESIGNATION
UD UNDISTURBED
Qu POCKET PENETROMETER UNCONFINED COMPRESSIVE STRENGTH

LL: LIQUID LIMIT
PL: PLASTIC LIMIT
PI: PLASTICITY INDEX

M: NATURAL MOISTURE CONTENT
F: PERCENT PASSING NO. 200 SIEVE

LABORATORY TEST PROCEDURES

A brief description of the laboratory tests performed is provided in the following sections.

DESCRIPTION OF SOILS (VISUAL-MANUAL PROCEDURE) (ASTM D2488)

The soil samples were visually examined by our engineer and soil descriptions were provided. Representative samples were then selected and tested in accordance with the aforementioned laboratory-testing program to determine soil classifications and engineering properties. This data was used to correlate our visual descriptions with the Unified Soil Classification System (USCS).

NATURAL MOISTURE CONTENT (ASTM D2216)

Natural moisture contents (M%) were determined on selected samples. The natural moisture content is the ratio, expressed as a percentage, of the weight of water in a given amount of soil to the weight of solid particles.

ATTERBERG LIMITS (ASTM D4318)

The Atterberg Limits test was performed to evaluate the soil's plasticity characteristics. The soil Plasticity Index (PI) is representative of this characteristic and is bracketed by the Liquid Limit (LL) and the Plastic Limit (PL). The Liquid Limit is the moisture content at which the soil will flow as a heavy viscous fluid. The Plastic Limit is the moisture content at which the soil is between "plastic" and the semi-solid stage. The Plasticity Index (PI = LL - PL) is a frequently used indicator for a soil's potential for volume change. Typically, a soil's potential for volume change increases with higher plasticity indices.

MATERIAL FINER THAN NO. 200 SIEVE BY WASHING (ASTM D1140)

Grain-size tests were performed to determine the partial soil particle size distribution. The amount of material finer than the openings on the No. 200 sieve (0.075 mm) was determined by washing soil over the No. 200 sieve. The results of wash #200 tests are presented on the boring logs included in this report and in the table of laboratory test results.

LOSS ON IGNITION TEST (LOI) (ASTM D2974)

LOI tests were performed on samples of near-surface soils. The ash content of a peat or organic soil sample is determined by igniting an oven-dried sample of the soil in a muffle furnace at 440°C (Method C) or 750°C (Method D). The substance remaining after ignition is the ash. The organic content is expressed as a percentage of the mass of the oven-dried sample.

LABORATORY TEST RESULTS

The results of the laboratory testing are presented in the following tables.

BORING NO.	DEPTH	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING #200 SIEVE	CLASSIFICATION
B-01	0.5 - 2.0	28.4	25	21	4		
B-01	2.5 - 4.0	19.1					
B-01	5.0 - 6.5	18.3					
B-01	8.5 - 10.0	22.5					
B-01	13.5 - 15.0	9.5					
B-02	0.5 - 2.0	29.1					
B-02	2.5 - 4.0	19.7					
B-02	5.0 - 6.5	20.6					
B-02	8.5 - 10.0	26.6					
B-02	13.5 - 14.5	10.5					
B-03	0.5 - 2.0	29.5					
B-03	2.5 - 4.0	24.7					
B-03	5.0 - 6.5	22.6	58	17	41	86	CH
B-03	8.5 - 10.0	22.9					
B-03	13.5 - 14.5	10.5					
B-04	0.5 - 2.0	28.5					
B-04	2.5 - 4.0	22.3	45	17	28	92	CL
B-04	5.0 - 6.5	20.0					
B-04	8.5 - 10.0	20.8					
B-04	13.5 - 14.4	10.1					
B-05	0.5 - 2.0	25.1					
B-05	2.5 - 4.0	18.5					
B-05	5.0 - 6.5	22.6					
B-05	8.5 - 10.0	20.8					
B-05	13.5 - 14.5	10.7					
B-06	0.5 - 2.0	26.9					
B-06	2.5 - 4.0	20.9					
B-06	5.0 - 6.5	24.0					
B-06	8.5 - 10.0	20.3					
B-06	13.5 - 14.5	10.1					
B-07	0.5 - 2.0	27.6					

TABLE L-1: General Soil Classification Test Results

Soils with a Liquid Limit (LL) greater than 50 and Plasticity Index (PI) greater than 25 usually exhibit significant volume change with varying moisture content and are considered to be highly plastic
(1) Indicates visual classification. WR indicates weathered rock.

LABORATORY TEST RESULTS

The results of the laboratory testing are presented in the following tables.

BORING NO.	DEPTH	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING #200 SIEVE	CLASSIFICATION
B-07	2.5 - 4.0	25.4					
B-07	5.0 - 6.5	18.9					
B-07	8.5 - 10.0	15.3	42	23	19		
B-07	13.5 - 14.4	11.1					
B-08	0.5 - 2.0	26.9					
B-08	2.5 - 4.0	24.3	55	19	36	91	CH
B-08	5.0 - 6.5	19.1					
B-08	8.5 - 10.0	17.7					
B-08	13.5 - 14.5	10.2					
B-09	0.5 - 2.0	26.2					
B-09	2.5 - 4.0	20.0					
B-09	5.0 - 6.5	22.1	41	16	25		
B-09	8.5 - 10.0	23.0					
B-09	13.5 - 14.4	6.8					
B-10	0.5 - 2.0	25.4					
B-10	2.5 - 4.0	22.8	45	20	25		
B-10	5.0 - 6.5	23.1					
B-10	8.5 - 10.0	16.4					
B-10	13.5 - 15.0	12.5					
B-11	0.5 - 2.0	26.0					
B-11	2.5 - 4.0	22.1					
B-11	5.0 - 6.5	22.3	45	18	27	90	CL
B-11	8.5 - 10.0	20.6					
B-11	13.5 - 15.0	11.2					
B-12	0.5 - 2.0	37.6					
B-12	2.5 - 4.0	23.4	47	16	31		
B-12	5.0 - 6.5	22.6					
B-12	8.5 - 10.0	18.4					
B-12	13.5 - 15.0	12.0					
P-01	0.5 - 2.0	34.5					
P-01	2.5 - 4.0	21.6					

TABLE L-1: General Soil Classification Test Results

Soils with a Liquid Limit (LL) greater than 50 and Plasticity Index (PI) greater than 25 usually exhibit significant volume change with varying moisture content and are considered to be highly plastic
(1) Indicates visual classification. WR indicates weathered rock.

LABORATORY TEST RESULTS

The results of the laboratory testing are presented in the following tables.

BORING NO.	DEPTH	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	% PASSING #200 SIEVE	CLASSIFICATION
P-01	5.0 - 6.5	25.5	49	18	31		
P-02	0.5 - 2.0	28.7	28	17	11		
P-02	2.5 - 4.0	23.6					
P-02	5.0 - 6.5	20.5					
P-03	0.5 - 2.0	30.4					
P-03	2.5 - 4.0	19.6	44	17	27	89	CL
P-03	5.0 - 6.5	19.7					
P-04	0.5 - 2.0	26.9					
P-04	2.5 - 4.0	18.0	37	16	21		
P-04	5.0 - 6.5	19.5					
P-05	0.5 - 2.0	21.1					
P-05	2.5 - 4.0	19.3					
P-05	5.0 - 6.5	21.8					
P-06	0.5 - 2.0	45.7	47	32	15		
P-06	2.5 - 4.0	15.3					
P-06	5.0 - 6.5	24.0					

TABLE L-1: General Soil Classification Test Results

Soils with a Liquid Limit (LL) greater than 50 and Plasticity Index (PI) greater than 25 usually exhibit significant volume change with varying moisture content and are considered to be highly plastic
(1) Indicates visual classification. WR indicates weathered rock.

Important Information about This

Geotechnical-Engineering Report

Subsurface problems are a principal cause of construction delays, cost overruns, claims, and disputes.

While you cannot eliminate all such risks, you can manage them. The following information is provided to help.

Geotechnical Services Are Performed for Specific Purposes, Persons, and Projects

Geotechnical engineers structure their services to meet the specific needs of their clients. A geotechnical-engineering study conducted for a civil engineer may not fulfill the needs of a constructor — a construction contractor — or even another civil engineer. Because each geotechnical- engineering study is unique, each geotechnical-engineering report is unique, prepared *solely* for the client. No one except you should rely on this geotechnical-engineering report without first conferring with the geotechnical engineer who prepared it. *And no one — not even you* — should apply this report for any purpose or project except the one originally contemplated.

Read the Full Report

Serious problems have occurred because those relying on a geotechnical-engineering report did not read it all. Do not rely on an executive summary. Do not read selected elements only.

Geotechnical Engineers Base Each Report on a Unique Set of Project-Specific Factors

Geotechnical engineers consider many unique, project-specific factors when establishing the scope of a study. Typical factors include: the client's goals, objectives, and risk-management preferences; the general nature of the structure involved, its size, and configuration; the location of the structure on the site; and other planned or existing site improvements, such as access roads, parking lots, and underground utilities. Unless the geotechnical engineer who conducted the study specifically indicates otherwise, do not rely on a geotechnical-engineering report that was:

- not prepared for you;
- not prepared for your project;
- not prepared for the specific site explored; or
- completed before important project changes were made.

Typical changes that can erode the reliability of an existing geotechnical-engineering report include those that affect:

- the function of the proposed structure, as when it's changed from a parking garage to an office building, or from a light-industrial plant to a refrigerated warehouse;
- the elevation, configuration, location, orientation, or weight of the proposed structure;
- the composition of the design team; or
- project ownership.

As a general rule, *always* inform your geotechnical engineer of project changes—even minor ones—and request an assessment of their impact. *Geotechnical engineers cannot accept responsibility or liability for problems that occur because their reports do not consider developments of which they were not informed.*

Subsurface Conditions Can Change

A geotechnical-engineering report is based on conditions that existed at the time the geotechnical engineer performed the study. *Do not rely on a geotechnical-engineering report whose adequacy may have been affected by*: the passage of time; man-made events, such as construction on or adjacent to the site; or natural events, such as floods, droughts, earthquakes, or groundwater fluctuations. *Contact the geotechnical engineer before applying this report to determine if it is still reliable.* A minor amount of additional testing or analysis could prevent major problems.

Most Geotechnical Findings Are Professional Opinions

Site exploration identifies subsurface conditions only at those points where subsurface tests are conducted or samples are taken. Geotechnical engineers review field and laboratory data and then apply their professional judgment to render an opinion about subsurface conditions throughout the site. Actual subsurface conditions may differ — sometimes significantly — from those indicated in your report. Retaining the geotechnical engineer who developed your report to provide geotechnical-construction observation is the most effective method of managing the risks associated with unanticipated conditions.

A Report's Recommendations Are Not Final

Do not overrely on the confirmation-dependent recommendations included in your report. *Confirmation-dependent recommendations are not final,* because geotechnical engineers develop them principally from judgment and opinion. Geotechnical engineers can finalize their recommendations *only* by observing actual subsurface conditions revealed during construction. *The geotechnical engineer who developed your report cannot assume responsibility or liability for the report's confirmation-dependent recommendations if that engineer does not perform the geotechnical-construction observation required to confirm the recommendations' applicability.*

A Geotechnical-Engineering Report Is Subject to Misinterpretation

Other design-team members' misinterpretation of geotechnical-engineering reports has resulted in costly problems. Confront that risk by having your geotechnical engineer confer with appropriate members of the design team after submitting the report. Also retain your geotechnical engineer to review pertinent elements of the design team's plans and specifications. Constructors can also misinterpret a geotechnical-engineering report. Confront that risk by having your geotechnical engineer participate in prebid and preconstruction conferences, and by providing geotechnical construction observation.

Do Not Redraw the Engineer's Logs

Geotechnical engineers prepare final boring and testing logs based upon their interpretation of field logs and laboratory data. To prevent errors or omissions, the logs included in a geotechnical-engineering report should *never* be redrawn for inclusion in architectural or other design drawings. Only photographic or electronic reproduction is acceptable, *but recognize that separating logs from the report can elevate risk.*

Give Constructors a Complete Report and Guidance

Some owners and design professionals mistakenly believe they can make constructors liable for unanticipated subsurface conditions by limiting what they provide for bid preparation. To help prevent costly problems, give constructors the complete geotechnical-engineering report, *but* preface it with a clearly written letter of transmittal. In that letter, advise constructors that the report was not prepared for purposes of bid development and that the report's accuracy is limited; encourage them to confer with the geotechnical engineer who prepared the report (a modest fee may be required) and/or to conduct additional study to obtain the specific types of information they need or prefer. A prebid conference can also be valuable. *Be sure constructors have sufficient time* to perform additional study. Only then might you be in a position to give constructors the best information available to you, while requiring them to at least share some of the financial responsibilities stemming from unanticipated conditions.

Read Responsibility Provisions Closely

Some clients, design professionals, and constructors fail to recognize that geotechnical engineering is far less exact than other engineering disciplines. This lack of understanding has created unrealistic expectations that have led to disappointments, claims, and disputes. To help reduce the risk of such outcomes, geotechnical engineers commonly include a variety of explanatory provisions in their reports. Sometimes labeled "limitations," many of these provisions indicate where geotechnical engineers' responsibilities begin and end, to help others recognize their own responsibilities and risks. *Read these provisions closely.* Ask questions. Your geotechnical engineer should respond fully and frankly.

Environmental Concerns Are Not Covered

The equipment, techniques, and personnel used to perform an *environmental* study differ significantly from those used to perform a *geotechnical* study. For that reason, a geotechnical-engineering report does not usually relate any environmental findings, conclusions, or recommendations; e.g., about the likelihood of encountering underground storage tanks or regulated contaminants. *Unanticipated environmental problems have led to numerous project failures.* If you have not yet obtained your own environmental information, ask your geotechnical consultant for risk-management guidance. *Do not rely on an environmental report prepared for someone else.*

Obtain Professional Assistance To Deal with Mold

Diverse strategies can be applied during building design, construction, operation, and maintenance to prevent significant amounts of mold from growing on indoor surfaces. To be effective, all such strategies should be devised for the *express purpose* of mold prevention, integrated into a comprehensive plan, and executed with diligent oversight by a professional mold-prevention consultant. Because just a small amount of water or moisture can lead to the development of severe mold infestations, many mold- prevention strategies focus on keeping building surfaces dry. While groundwater, water infiltration, and similar issues may have been addressed as part of the geotechnical- engineering study whose findings are conveyed in this report, the geotechnical engineer in charge of this project is not a mold prevention consultant; *none of the services performed in connection with the geotechnical engineer's study were designed or conducted for the purpose of mold prevention. Proper implementation of the recommendations conveyed in this report will not of itself be sufficient to prevent mold from growing in or on the structure involved.*

Rely, on Your GBC-Member Geotechnical Engineer for Additional Assistance

Membership in the Geotechnical Business Council of the Geoprofessional Business Association exposes geotechnical engineers to a wide array of risk-confrontation techniques that can be of genuine benefit for everyone involved with a construction project. Confer with you GBC-Member geotechnical engineer for more information.

GEOTECHNICAL
BUSINESS COUNCIL
of the Geoprofessional Business Association

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